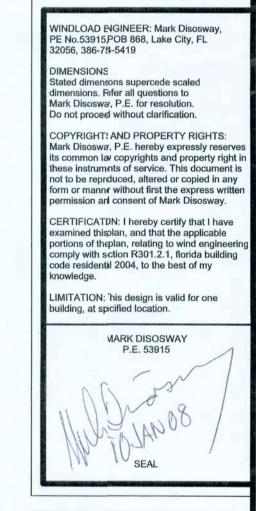


### **ELECTRICAL PLAN NOTES**

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4
  ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
  BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
  BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE E -5 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC
- E -10 CONDUCTORS ENTER THE BUILDING.
  SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

	ELECTRICAL LEGEND		
	CEILING FAN (PRE-WIRE FOR JIGHT KIT)		
90	DOUBLE SECURITY LIGHT		
	2X4 FLUORESCENT LIGHT FIXTURE		
0	RECESSED CAN LIGHT		
- ♦ ∰	BATH EXAUST FAN WITH LIGHT		
₩	BATH EXAUST FAN		
	LIGHT FIXTURE		
ф	DUPLEX OUTLET		
₩	220v OUTLET		
фан	GFI DUPLEX OUT.ET		
•	SMOKE DETECTOR		
\$	WALL SWITCH		
\$3	3 WAY WALL SWICH		
\$4	4 WAY WALL SWITCH		
∯ <sub>WP/GFI</sub>	WATER PROOF ŒI OUTLET		
$\nabla$	PHONE JACK		
0	TELEVISION JACK		
9	GARAGE DOOR (PENER		
M	WALL HEATER		



K & H Framing -Vinyl Siding, Inc.

> Gary Monk Residence

ADDRESS: Loti Century Oaks S/D Colunbia County, Florida

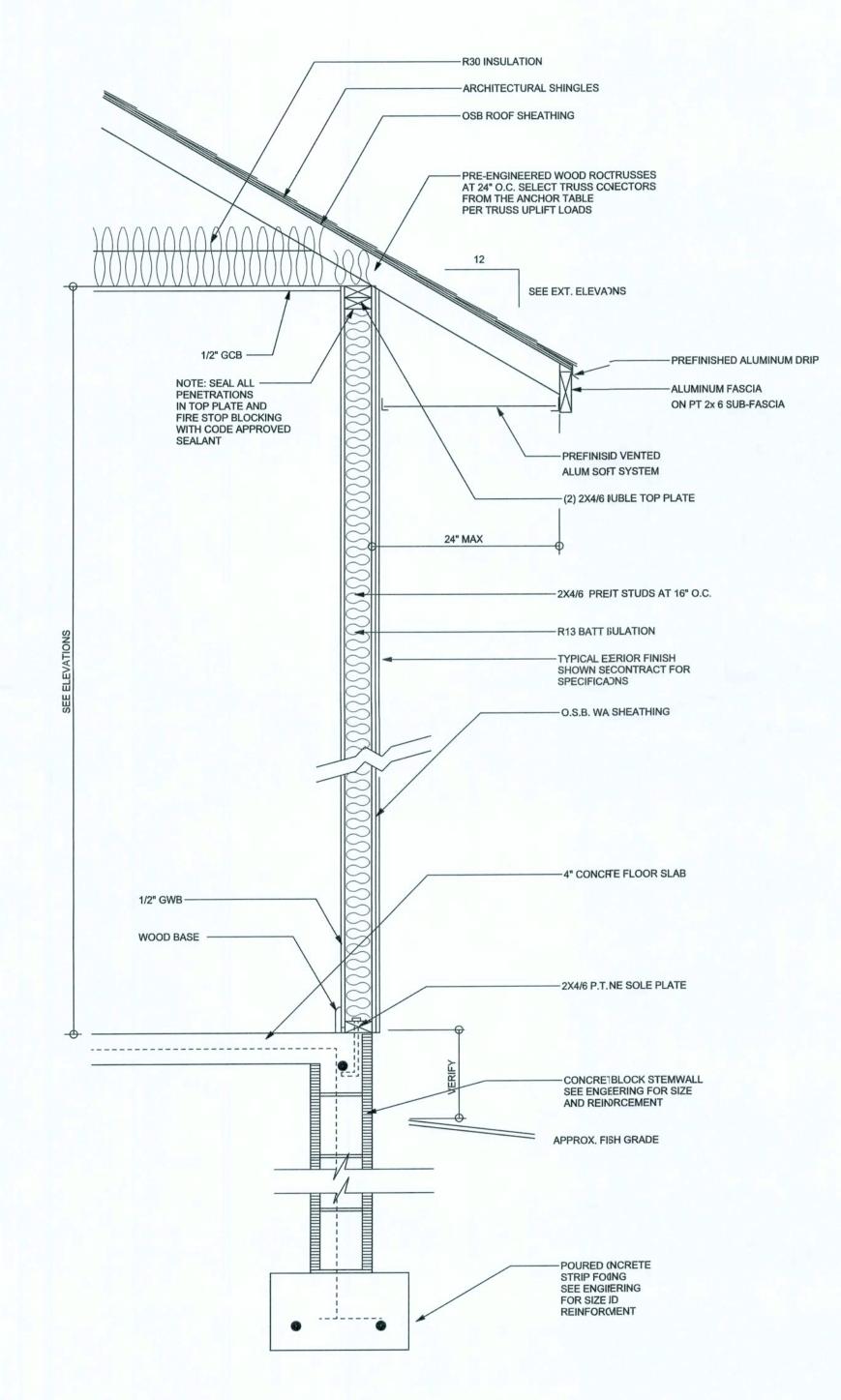
Mart Disosway P.E. F.O. Box 868 Lake City, Florida 32056 Phone (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: January 10, 2008 DRAWN EY: STRUCTURAL BY David Disosway David Disosway

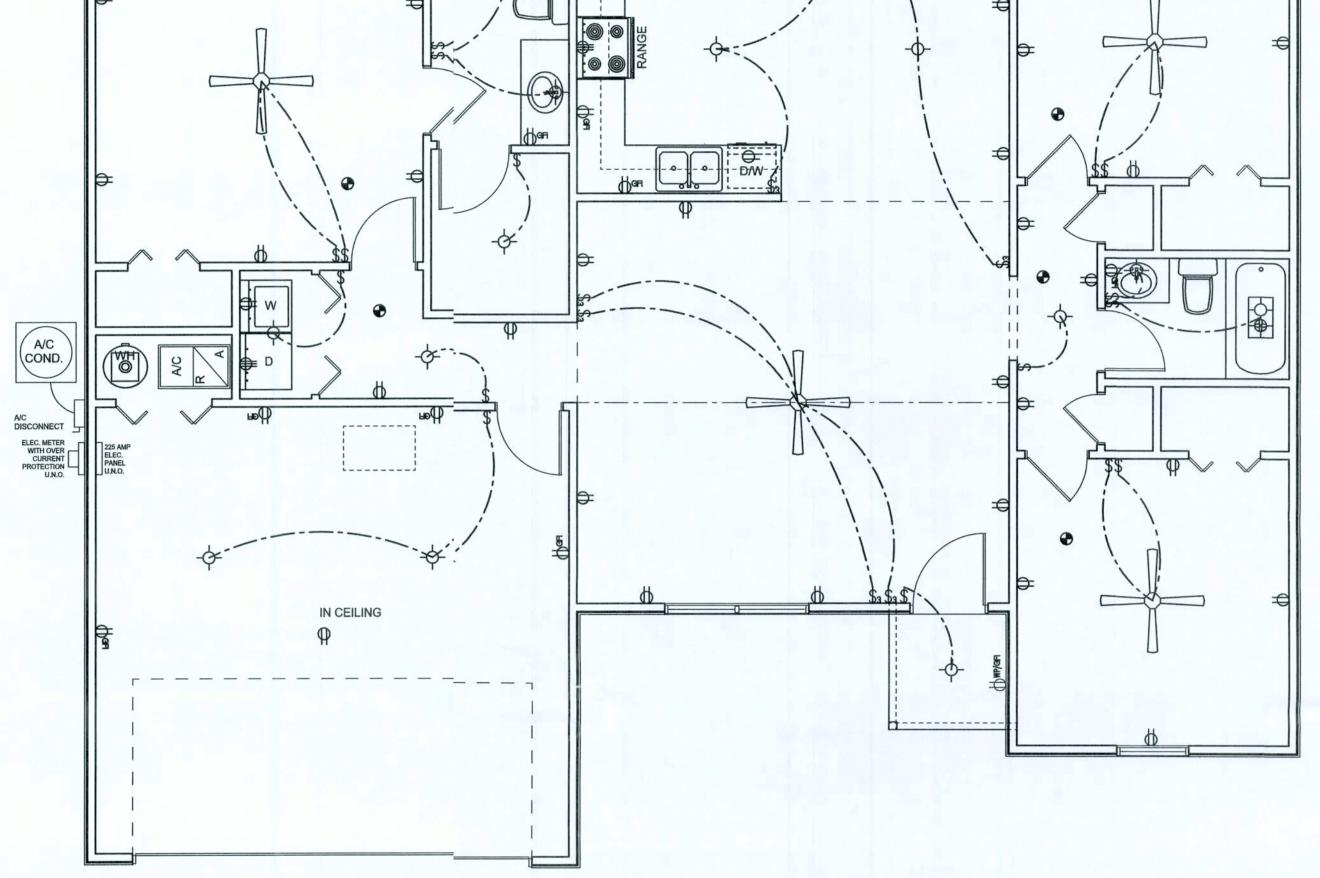
FINALS DATE: 10 / Jan ,08

> JOB NUMBER: 801041 DFAWING NUMBER

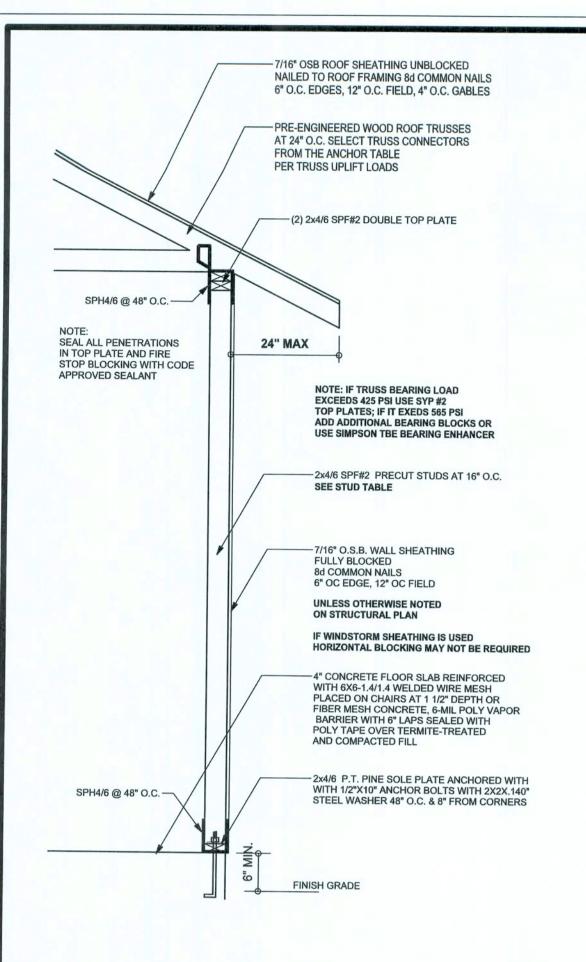
> > **A-2** OF 5 SHEETS



TYPICAL DESIGN WALL SECTION NON - STRUCTURAL DATA SCALE: 1" = 1'- 0"



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"



# EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

ONE STORY WALL SECTION

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

(2) 2X12 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

**ANCHOR BOLT** 

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

SIMPSON H2.5A U.N.O. -

(2) SIMPSON LSTA21-

AND (8) -16d TO POST

w/ (8) -16d TO HEADER

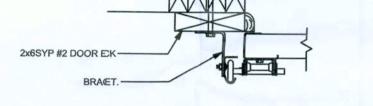
SEE STRUCTURAL PLAN

# — (2) 2X4 SPF #2 TOP PLATE (2) SIMPSON SPH4 w/ (6: 10d--SIMPSON SPH4 @ 48" O.C. (2) SIMPSON LSTA2w/ (8) -16d TO HEAD® AND (8) -16d TO STUPACK -(2) 2X12 SYP #2 HEADER U.N.O SEE STRUCTURAL PLAN (2) KINGS STUDS--(2) JACKS STUDS w/ (2) ROWS 10d! w/ (2) ROWS 10d @ 12" O.C. EACH SE 12" O.C. EACH SIDE SIMPSON LTTI31 --w/ (18) - 10d & 5/8" x 10" ANCHOR BCT -FOUNDATION SEE SEE FOOTING DETAILS

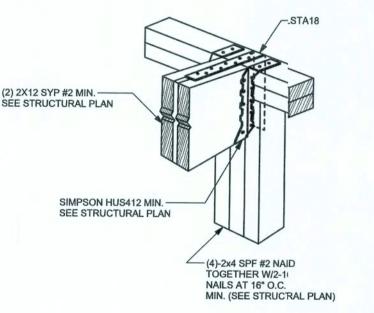
# TYPICAL GARGE DOOR HEADER STRAPING DETAIL

#### 2 SYP #2 GARAGE DOOR BUCK ATTACHMENT AACH GARAGE DOOR BUCK TO STUD PACK AT EH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SEWS w/ 1" WASHER LAG SCREWS MAY BE CINTERSUNK, HORIZONTAL JAMBS DO NOT TINSFER LOAD, CENTER LAG SCREWS OR SGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

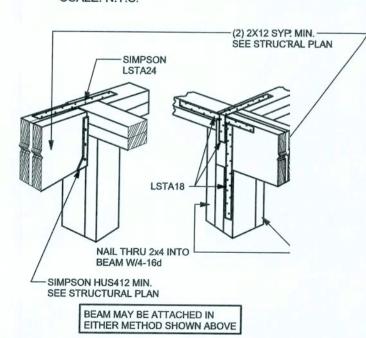
OOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



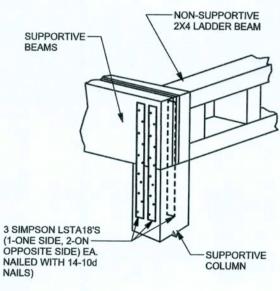
# **GARAGEOOR BUCK INSTALLATION DETAIL**



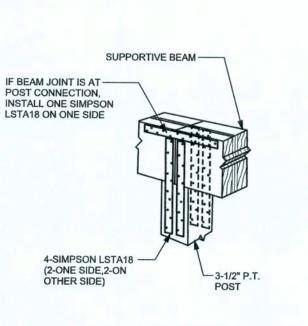
#### **BEAM MID-WALL CONNECION DETAIL** SCALE: N.T.S.



BEAM CORNER CONNECTION. ETAIL



SUPPORTIVE POST TO BEAM **DETAIL FOR SINGLE BEAM** SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

# **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.

FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**WASHERS**: WASHERS USED WITH 1/2" BOLTS TO BE 2"  $\times$  2"  $\times$  9/64"; WITH 5/8" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 3/4" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 7/8" BOLTS TO BE 3"  $\times$  3"  $\times$  5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY** 

GRADE & SPECIES: TABLE

SYP #2

SYP #2

SYP #2

24F-V3 SP

TIMBERSTRAND

MICROLAM

PARALAM

DOUBLE 2x4 SPF TOP PLATE NAILECET TOGETHER W/2-16d NAILS AT 16" O 4' MIN. LAP w/ (12) - 16d OR 4" LAP w/ w/

CS20 w/ (4) - 16d &(14) - 10d INTERIOR CEILING AS ---SPECIFIED ON FLOOR PLAN

ALL STUDS TO BE 2x4 -

CONTINUOUS FRAMME TO

IF TRUSS TO WALL STRAPS ARE E NAILED TO THE HEADER THE SPH4/6 @ 41/48" O.C.

CEILING DIAPHRAGGM DETAIL

-SPH4/6 Alall Openings (U.N.O.)-

CRIPP-PLES IF REQUIRED

(4) .131 x x 3 1/4" GUN NAILS

TOE NAVAILED THRU SILL -

INTO JAJACK STUD U.N.O.

TYPICAL L STRAPPING (U.N.O.)

—SPH4/6 AL\LL OPENINGS (U.N.O.)——

(1) 2X6 SPF #2<sub>!2</sub> SILL UP TO 11'-0" U.N.O.

TYPICAL HEADBER STRAPING DETAIL

(1) 2X4 SPF #2#2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 1, 10'-0" WALL HIGHT U.N.O.)

(SEE STITRUCTURAL PLAN)

SP;PH4/6 @ 48" O.C. (U.N.O.)

SPF NAILED TO TOP

SCALE: N.T.S.

ARE NOT REQUIRED

(6) .131 x 3 1/4" GUN NAILS-

INTO KING STUD

TOE NAILED THRU HEADER

AND BOTTOM PLATES

CONTINUOUS FRAME -

TO TOP PLATE AT **BOTTOM CHORD OF TRUSS** 

2x10

2x12

GLB

Fb (p<sub>(psi)</sub> | E (10<sup>6</sup>psi)

1.6

1.6

1.8

1.7

1.9

120200

105)50

160(00

PRE ENGINEERED ROCOOF TRUSS -

SPECIFICA	ER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR LLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SIT BACKFILL HE	E CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND GHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MA REQUIREMEN	TERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 ITS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE	ONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL AD ENGINEER IMMEDIATELY.
DESIGN, PLA	RUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS EMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, USS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL CATIONS.

# **ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

< 420	VPLIFT LBS. SPF		TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 455		H5A	3-8d	3-8d	
< 360	< 265	H5	4-8d	4-8d	
	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	-
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED I 12" EMBEDMEN
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED F 12" EMBEDMEN
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED F 12" EMBEDMEN
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED F
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
	331	STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d	-	1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		3/0 AB
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		4 (0)1 ( =
< 2300	< 2300	ABU66			1/2" AB
< 2320	< 2320	ABU88	12-16d 18 - 16d		1/2" AB 2-5/8" AB

# ROOF SYSTEM DESIGN

(6) .131 x 3 1/4" GUN NAILS

INTO KING STUD

TOE NAILED THRU HEADER

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS. BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

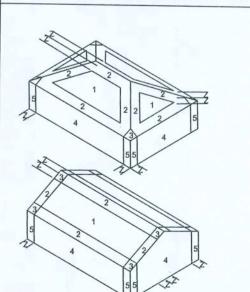
1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B WIND IMPORTANCE FACTOR = 1.0

BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES

**DESIGN DATA** 

6.) MEAN ROOF HEIGHT = <30 FT .) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) Zone Effective Wind Area (ft2)



STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

		19.9	-21.8	18.1	-18.1
2 2	2	19.9	-25.5	18.1	-21.8
1 1 2 7	2 O'hg		-40.6		-40.6
2 2 2 5	3	19.9	-25.5	18.1	-21.8
4	3 O'hg		-68.3		-42.4
111 4	4	21.8	-23.6	18.5	-20.4
555	5	21.8	-29.1	18.5	-22.6
	Doors 8			21.8	-29.1
2	Wors (Zone	t Cas			
2 3	8x7 Gara			19.5	-22.9
4 /3/ 4 5	16x7 Ga			18.5	-21.0
55 22					
N LOADS					
R 40 PSF (ALL OTHER DWELLING ROOMS)					
20 DCE (CLEEDING DOCUME)					

DESIGN FLOOR 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER)

FINALS DATE: 10 / Jan / 08

JOB NUMBER: 80 041 DRAWING NUMBER

OF 5 SHEETS

**MASONRY NOTES:** 

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

DIMENSIONS: Stated dimensions spercede scaled dimensions. Refer al questions to Mark Disosway, P.Efor resolution. Do not proceed withut clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. nereby expressly reserve its common law copyights and property right in these instruments of ervice. This documen not to be reproduced altered or copied in any form or manner without first the express writter

> CERTIFICATION: I breby certify that I have xamined this plan, ad that the applicable portions of the plan, elating to wind enginee comply with section [301.2.1, florida building code residential 2004 to the best of my

permission and consnt of Mark Disosway.

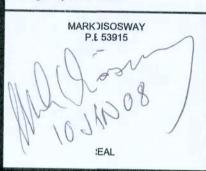
WINDLOAD ENGINIER: Mark Disosway, PE No.53915, POB 68, Lake City, FL

32056, 386-754-541

LIMITATION: This deign is valid for one building, at specified ocation.

REVISONS

SOFTPLAN



K & H Framing Vinyl Sding, Inc.

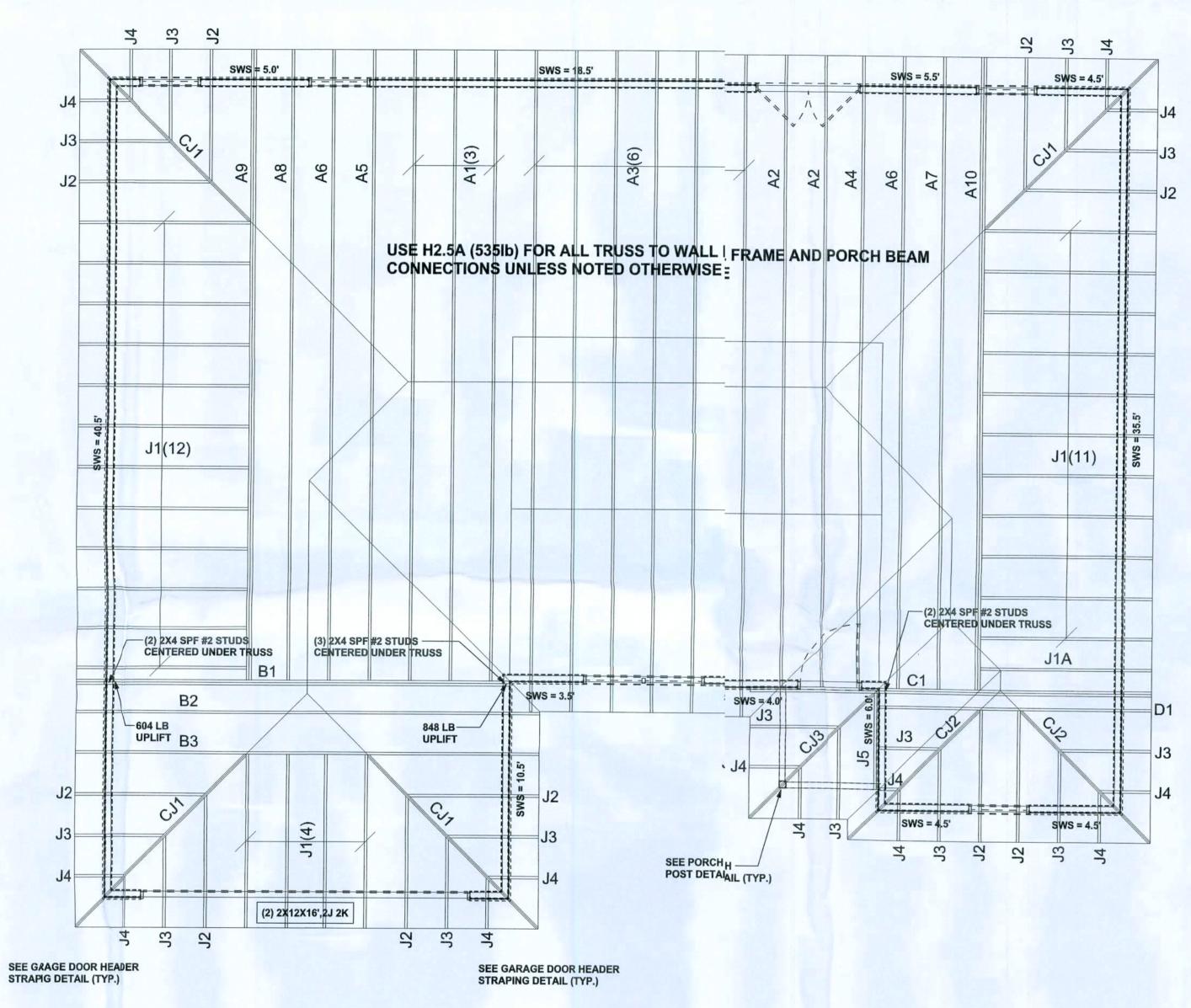
Residence ADIRESS: Lot 5 Centry Oaks S/D

GaryMonk

Columbia County, Florida Mark Disssway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (38i) 754 - 5419 Fax: (386) 269 - 4871

PRINTID DATE: January 10, 2008 DRAWN BY: STRUCTURAL BY David Disosway David Disosway

REVISIONS



#### STRUCTURAL PLAN SCALE: '/4" = 1'-0"

### STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

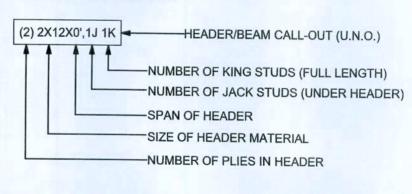
SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

# WALL LEGEND

SWS = 0.0'	1ST IT FLOOR EXTERIOR WALL
SWS = 0.0'	2ND D FLOOR EXTERIOR WALL
IBW	1ST IT FLOOR INTERIOR BEARING WALL
IBW	2ND 5 FLOOR INTERIOR BEARING WALL

## HEADER LEGEND



# TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.6'	92.5'
LONGITUDINAL	28.6'	50.0'

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB368, Lake City, FL DIMENSIONS: Stated dimensions spercede scaled dimensions. Refer a questions to Mark Disosway, P.E for resolution. Do not proceed withut clarification. COPYRIGHTS ANDPROPERTY RIGHTS: Mark Disosway, P.E hereby expressly reserves its common law coprights and property right in these instruments of service. This document is not to be reproduced altered or copied in any form or manner without first the express written permission and conent of Mark Disosway. CERTIFICATION: I lereby certify that I have examined this plan, and that the applicable portions of the plan, elating to wind engineering comply with section \(\frac{3}{3}\)01.2.1, florida building code residential 200, to the best of my LIMITATION: This design is valid for one building, at specified ocation. MARKDISOSWAY P.i. 53915

> K & HFraming -Vinyl Siding, Inc.

3EAL

Gary Monk Residence

ADDRESS: Lot 5 Cenury Oaks S/D Columbia County, Florida

Mark Dissway P.E. P.O. 3ox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

January10, 2008 DRAWN BY: STRUCTURAL BY: David Disosway David Disosway

FINALS DATE: 10 / Jan / 08

JOB NUMBER:

801041 DRAWIN3 NUMBER

> **S-3** OF 53HEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS