

DATE 08/25/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT

000027286

APPLICANT ADAM PAPKA PHONE 623-2383  
ADDRESS PO BOX 1921 LAKE CITY FL 32056  
OWNER ADAM PAPKA PHONE 623-2383  
ADDRESS 128 SW HOLLY GLEN LAKE CITY FL 32024  
CONTRACTOR ADAM PAPKA PHONE 623-2386  
LOCATION OF PROPERTY 47 S, L WATER AVE, L LITTLE RD, R HOLLY GLN, 3RD ON LEFT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 126200.00  
HEATED FLOOR AREA 1962.00 TOTAL AREA 2524.00 HEIGHT 24.00 STORIES 1  
FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 9/12 FLOOR SLAB  
LAND USE & ZONING AG-3 MAX. HEIGHT 35  
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00  
NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 01-5S-16-03390-018 SUBDIVISION  
LOT BLOCK PHASE UNIT TOTAL ACRES 1.00

000001662 CBC1253409  
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor  
WAIVER 08-530 BK WR N  
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: SECTION 14.9 SPECIAL FAMILY LOT PERMIT  
FLOOR ONE FOOT ABOVE THE ROAD

Check # or Cash 1520

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by  
Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by  
Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by  
Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by  
Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by  
M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by  
Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by  
M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE \$ 635.00 CERTIFICATION FEE \$ 12.62 SURCHARGE FEE \$ 12.62  
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$  
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 735.24  
INSPECTORS OFFICE L. J. L. CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Attention: Weegie

**Columbia County Building Department  
Culvert Waiver**

**Culvert Waiver No.  
000001662**

DATE: 08/25/2008 BUILDING PERMIT NO. 27286

APPLICANT ADAM PAKKA PHONE 623-2383

ADDRESS PO BOX 1921 LAKE CITY FL 32056

OWNER ADAM PAKKA PHONE 623-2383

ADDRESS 128 SW HOLLY GLEN LAKE CITY FL 32024

CONTRACTOR ADAM PAKKA PHONE 623-233

LOCATION OF PROPERTY 47 S, L WALTER AVE, L LITTLE RD, R HOLLEY GLEN, 3RD ON LEFT

PRIVATE ROAD

Blue Sign - GATED ROAD

SUBDIVISION/LOT/BLOCK/PHASE/UNIT \_\_\_\_\_

PARCEL ID # 01-5S-16-03390-018

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: \_\_\_\_\_

A SEPARATE CHECK IS REQUIRED  
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

**PUBLIC WORKS DEPARTMENT USE ONLY**

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE  
CULVERT WAIVER IS:

✓ APPROVED \_\_\_\_\_ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: This waiver is located on

PRIVATE RD.

SIGNED: [Signature] DATE: 8/27/08

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21  
Lake City, FL 32055  
Phone: 386-758-1008 Fax: 386-758-2160



## Columbia County Building Permit Application

ck 1520  
ck 1521 wain

For Office Use Only Application # 6808-01 Date Received 8/1/08 By GP Permit # 27286/1662  
Zoning Official BLK Date 12.08.08 Flood Zone X FEMA Map # N/A Zoning A-3  
Land Use A-3 Elevation N/A MFE 1st River N/A Plans Examiner (we) Date 8/12/08  
Comments Section 14.9 Special Family Lot Permit  
☐ NOC ☒ EH ☐ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel # \_\_\_\_\_  
☐ Dev Permit # \_\_\_\_\_ ☐ In Floodway ☒ Letter of Authorization from Contractor  
☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 08-530 Fax 752-2282Name Authorized Person Signing Permit Linda or Melanie Roder Phone 752-2281Address 387 SW Kemp Ct Lake City FL 32024Owners Name Adam Papka Phone 623-2383911 Address 128 SW Holly GLN Lake City FL 32024Contractors Name Adam Papka of Adam's Frames Construction Phone 623-2383Address P.O.B. 1921 Lake City FL 32056Fee Simple Owner Name & Address NABonding Co. Name & Address NAArchitect/Engineer Name & Address Evan Beamsley/Mark Disosway POB 868 Lake City FL 32056Mortgage Lenders Name & Address 1st Federal Lake City FLCircle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress EnergyProperty ID Number 01-55-16-03390-018 Estimated Cost of Construction 165KSubdivision Name NA Lot \_\_\_\_\_ Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase \_\_\_\_\_Driving Directions State Rd 47, L on Walter Ave, L on Little Road, R onHolly Glen, site on leftNumber of Existing Dwellings on Property 0Construction of Single family dwelling Total Acreage 1 Lot Size 1 acreDo you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 24'Actual Distance of Structure from Property Lines - Front 50' Side 16' Side 16' Rear 245'Number of Stories 1 Heated Floor Area 1962 Total Floor Area 4962 Roof Pitch 9-12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

left message 8/13/08

Columbia County Building Permit Application

Application # \_\_\_\_\_

**WARNING TO OWNER:** YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

**FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment**

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

**NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:**

**YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

**OWNERS AFFIDAVIT:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

[Signature]  
Owners Signature

Affirmed under penalty of perjury to by the Owner and subscribed before me this 28 day of July 2008  
Personally known ☒ or Produced Identification \_\_\_\_\_

[Signature]  
State of Florida Notary Signature (For the Owner)

SEAL:

NOTARY PUBLIC-STATE OF FLORIDA  
Linda R. Roder  
Commission #DD755608  
Expires: MAR. 24, 2012  
BONDED THRU ATLANTIC BONDING CO., INC.

**CONTRACTORS AFFIDAVIT:** By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

[Signature]  
Contractor's Signature (Permitee)

NOTARY PUBLIC-STATE OF FLORIDA  
Linda R. Roder  
Commission #DD755608  
Expires: MAR. 24, 2012  
BONDED THRU ATLANTIC BONDING CO., INC.

Contractor's License Number CBC 1253409  
Columbia County  
Competency Card Number \_\_\_\_\_

Affirmed under penalty of perjury to by the Contractor and subscribed before me this \_\_\_\_\_ day of \_\_\_\_\_ 2008



Adam Papka Permit

10/10/2007 12:01 PM FROM: 386-719-6708 Blake Construction Company TO: 752-2282 PAGE: 001 OF 001

### Notice of Authorization

I, Adam Papka, hereby authorize Linda Roder or Melanie Roder to be my Representative and act on my behalf in all aspects for applying for a Building Permit to be located in Columbia County.

X 

Contractor's Signature

7-22-08  
Date

Sworn to and Subscribed before me this 28 day of July, 2007  
by \_\_\_\_\_, who

☒ is Personally Known or

\_\_\_\_\_ has produced \_\_\_\_\_ as identification.

  
Notary Public

Notary Stamp

NOTARY PUBLIC-STATE OF FLORIDA  
Linda R. Roder  
Commission #DD755608  
Expires: MAR. 24, 2012  
BONDED THRU ATLANTIC BONDING CO., INC.

Prepared by & Return to:

Sierra Title, LLC  
619 SW Baya Drive, Suite 102  
Lake City, Florida 32025

File Number: 08-0274

Inst:200812013947 Date:7/25/2008 Time:2:19 PM  
Doc Stamp-Deed:273.00  
✓ DC, P. DeWitt Cason, Columbia County Page 1 of 2 B:1155 P:897

### General Warranty Deed

Made this June 27, 2008 A.D. By Robert Stewart and Ellen Stewart, Husband and Wife, hereinafter called the grantor, to Adam Papka and his wife, Leah Papka, whose post office address is: P.O. Box 2963, Lake City, Florida 32056, hereinafter called the grantee:

(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

**Witnesseth**, that the grantor, for and in consideration of the sum of Ten Dollars, (\$10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

See Attached Schedule "A"

**N.B.** The grantee Leah Papka is the sister of the grantor Ellen Stewart

Said property is not the homestead of the Grantor(s) under the laws and constitution of the State of Florida in that neither Grantor(s) or any members of the household of Grantor(s) reside thereon.

Parcel ID Number: 01-SS-16-03390-018

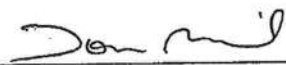
**Together** with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.


**To Have and to Hold**, the same in fee simple forever.

**And** the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31, 2007.

**In Witness Whereof**, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

  
Witness Printed Name Toni Ricard


  
Witness Printed Name Gabe Fitzhugh

State of Florida  
County of Columbia

  
Robert Stewart (Seal)  
Address:

  
Ellen Stewart (Seal)  
Address:

The foregoing instrument was acknowledged before me this 25<sup>th</sup> day of July, 2008, by Robert Stewart and Ellen Stewart, Husband and Wife, who are personally known to me or who has produced N/A as identification.

  
Notary Public  
Print Name: Toni Ricard  
My Commission Expires: 2-24-2012



**AFFIDAVIT OF SUBDIVIDED REAL PROPERTY  
FOR USE OF IMMEDIATE FAMILY MEMBERS  
FOR PRIMARY RESIDENCE**

STATE OF FLORIDA  
COUNTY OF COLUMBIA

BEFORE ME the undersigned Notary Public personally appeared.

Robert & Ellen Stewart, the Owner of the parent tract which has been subdivided for immediate family primary residence use, hereinafter the Owner, and Leah Papka, the family member of the Owner, who is the owner of the family parcel which is intended for immediate family primary residence use, hereafter the Family Member, and is related to the Owner as sister to Ellen Stewart, and both individuals being first duly sworn according to law, depose and say:

1. Both the Owner and the Family Member have personal knowledge of all matters set forth in this Affidavit.
2. The Owner holds fee simple title to certain real property situated in Columbia County, and more particularly described by reference to the Columbia county Property Appraiser Tax Parcel No. 01-55-16-03390-007.
3. The Owner has divided his parent parcel for use of immediate family members for their primary residence and the parcel divided and the remaining parent parcel are at least 1/2 acre in size. Immediate family is defined as grandparent, parent, step-parent, adopted parent, sibling, child, step-child, adopted child or grandchild.
4. The Family Member is a member of the Owner's immediate family, as set forth above, and holds fee simple title to certain real property divided from the Owner's parcel situated in Columbia County and more particularly described by reference to the Columbia County Property Appraiser Tax Parcel No. 01-55-16-03390-018.
5. No person or entity other than the Owner and Family Member claims or is presently entitled to the right of possession or is in possession of the property, and there are no tenancies, leases or other occupancies that affect the Property.
6. This Affidavit is made for the specific purpose of inducing Columbia County to recognize a family division for a family member on the parcel divided in accordance with Section 14.9 of the Columbia County Land Development Regulations.

7. This Affidavit is made and given by Affiants with full knowledge that the facts contained herein are accurate and complete, and with full knowledge that the penalties under Florida law for perjury include conviction of a felony of the third degree.

We Hereby Certify that the information contained in this Affidavit are true and correct.

X Ellie Stewart  
Owner

Ellen Stewart  
Typed or Printed Name

X Leah Papka  
Family Member

Leah Papka  
Typed or Printed Name

Subscribed and sworn to (or affirmed) before me this 29 day of July, 2008, by Ellen Stewart (Owner) who is personally known to me or has produced as identification.

Linda Roder  
Notary Public

NOTARY PUBLIC-STATE OF FLORIDA  
Linda R. Roder  
Commission #DD755608  
Expires: MAR. 24, 2012  
BONDED THRU ATLANTIC BONDING CO., INC.

Subscribed and sworn to (or affirmed) before me this 29 day of July, 2008, by Leah Papka (Family Member) who is personally known to me or has produced as identification.

Linda Roder  
Notary Public

NOTARY PUBLIC-STATE OF FLORIDA  
Linda R. Roder  
Commission #DD755608  
Expires: MAR. 24, 2012  
BONDED THRU ATLANTIC BONDING CO., INC.

Adam Papka

01-55-16-03390-018

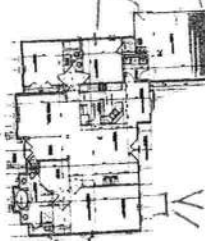
396.15

Water Ave

356.71

167'

245'



Owell

167'

356.69

399.96

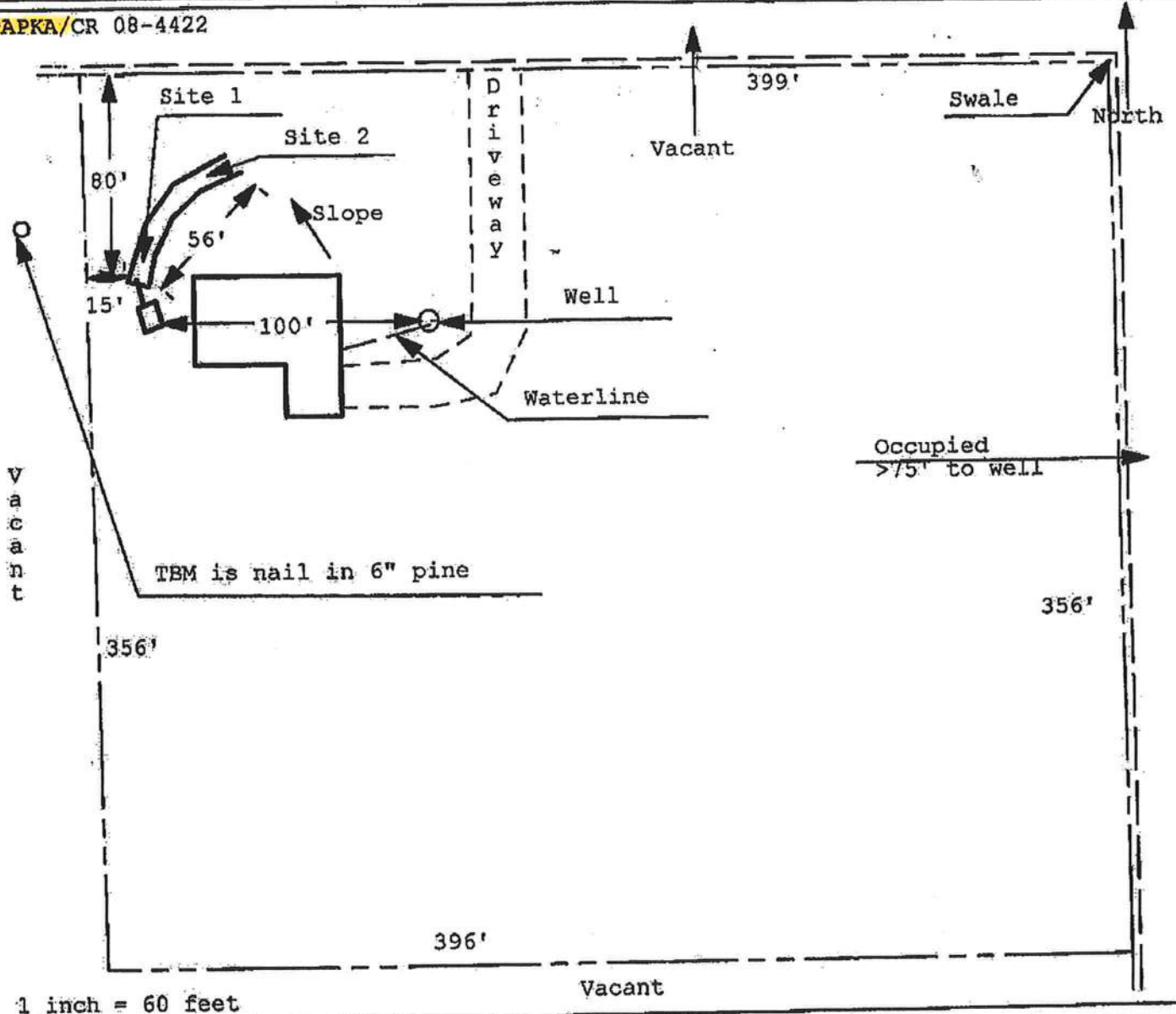
Holly Glen

N  
↑

**Application for Onsite Sewage Disposal System  
Construction Permit. Part II Site Plan**  
**Permit Application Number:** 08-0530

**ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT**

PAPKA/CR 08-4422



Site Plan Submitted By Paul Bluff Date 7/17/08  
 Plan Approved ☒ Not Approved ☐ Date 7/30/08

By Mark S. Lander

**Columbia CHD** CPHU

Notes:

# COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787

PHONE: (386) 758-1125 \* FAX: (386) 758-1365 \* Email: ron\_croft@columbiacountyfla.com

## Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 6/30/2008 DATE ISSUED: 7/1/2008

### ENHANCED 9-1-1 ADDRESS:

128 SW HOLLY

GLN

LAKE CITY FL 32024

### PROPERTY APPRAISER PARCEL NUMBER:

01-5S-16-03390-018

### Remarks:

Address Issued By:   
Columbia County 9-1-1 Addressing / GIS Department

**NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.**

1233

# **HALL'S PUMP & WELL SERVICE, INC.**

SPECIALIZING IN 4"-6" WELLS



DONALD AND MARY HALL  
OWNERS

PHONE (386) 752-1854  
FAX (386) 755-7022  
904 NW MAIN BLVD.  
LAKE CITY, FLORIDA 32056

July 25, 2008

Notice To All Contractors:  
~~Adam Papka & Linda Roder~~

Please be advised that due to the new building codes we will  
Use a large capacity diaphragm tank on all new well.  
This will insure a minimum of one (1) minute draw down or  
One (1) minute refill. If a smaller diaphragm tank is used then  
We will install a cycle stop valve which will produce the same  
Results. All wells will have a pump & tank combination that  
Will be sufficient enough for each situation.

If you have any questions please feel free to call our office.

Thank You,

Donald Hall

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs  
Residential Whole Building Performance Method A

Project Name:	802191 Adam's Framing & Construction	Builder:	Adam's Framing & Construction
Address:	128 SW Holly Gln	Permitting Office:	Columbia County
City, State:	Lake City, FL 32024-	Permit Number:	27286
Owner:		Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New	___
2. Single family or multi-family	Single family	___
3. Number of units, if multi-family	1	___
4. Number of Bedrooms	3	___
5. Is this a worst case?	No	___
6. Conditioned floor area (ft²)	1962 ft²	___
7. Glass type <sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)		___
a. U-factor:	Description Area	
(or Single or Double DEFAULT)	7a. (Dble Default)	309.2 ft²
b. SHGC:		
(or Clear or Tint DEFAULT)	7b. (Clear)	309.2 ft²
8. Floor types		
a. Slab-On-Grade Edge Insulation	R=0.0, 198.0(p) ft	___
b. N/A		___
c. N/A		___
9. Wall types		
a. Frame, Wood, Exterior	R=13.0, 1397.8 ft²	___
b. Frame, Wood, Adjacent	R=13.0, 124.0 ft²	___
c. N/A		___
d. N/A		___
e. N/A		___
10. Ceiling types		
a. Under Attic	R=30.0, 2204.0 ft²	___
b. N/A		___
c. N/A		___
11. Ducts		
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 170.0 ft	___
b. N/A		___
12. Cooling systems		
a. Central Unit	Cap: 46.0 kBtu/hr	___
	SEER: 13.00	___
b. N/A		___
c. N/A		___
13. Heating systems		
a. Electric Heat Pump	Cap: 46.0 kBtu/hr	___
	HSPF: 7.70	___
b. N/A		___
c. N/A		___
14. Hot water systems		
a. Electric Resistance	Cap: 40.0 gallons	___
	EF: 0.93	___
b. N/A		___
c. Conservation credits		___
(HR-Heat recovery, Solar		___
DHP-Dedicated heat pump)		___
15. HVAC credits		___
(CF-Ceiling fan, CV-Cross ventilation,		___
HF-Whole house fan,		___
PT-Programmable Thermostat,		___
MZ-C-Multizone cooling,		___
MZ-H-Multizone heating)		___

Glass/Floor Area: 0.16

Total as-built points: 27087

Total base points: 28477

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: \_\_\_\_\_

DATE: 7/24/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: \_\_\_\_\_

DATE: 7-28-08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: \_\_\_\_\_

DATE: \_\_\_\_\_



1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: 128 SW Holly Gln, Lake City, FL, 32024-

PERMIT #:

BASE				AS-BUILT						
GLASS TYPES										
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang		Area X SPM X SOF = Points			
					Ornt	Len	Hgt			
.18	1962.0	20.04	7077.3	Double, Clear	S	0.0	0.0	25.0	35.87	1.00 896.7
				Double, Clear	S	5.5	8.0	36.0	35.87	0.58 752.6
				Double, Clear	S	5.5	9.0	20.0	35.87	0.61 437.1
				Double, Clear	S	1.5	7.0	45.0	35.87	0.89 1443.8
				Double, Clear	W	1.5	7.0	30.0	38.52	0.94 1085.1
				Double, Clear	N	0.0	0.0	16.0	19.20	1.00 307.2
				Double, Clear	N	5.5	11.0	13.2	19.20	0.83 210.7
				Double, Clear	N	5.5	4.0	15.0	19.20	0.66 191.5
				Double, Clear	N	1.5	12.0	25.0	19.20	0.99 475.7
				Double, Clear	N	1.5	12.0	15.0	19.20	0.99 285.4
				Double, Clear	N	1.5	5.0	15.0	19.20	0.92 263.7
				Double, Clear	N	1.5	5.0	9.0	19.20	0.92 158.2
				Double, Clear	N	0.0	0.0	30.0	19.20	1.00 576.0
				Double, Clear	E	1.5	7.0	15.0	42.06	0.94 592.0
				As-Built Total:					309.2	7675.8
WALL TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points			
Adjacent	124.0	0.70	86.8	Frame, Wood, Exterior	13.0	1397.8	1.50	2096.7		
Exterior	1397.8	1.70	2376.3	Frame, Wood, Adjacent	13.0	124.0	0.60	74.4		
Base Total:				As-Built Total:					1521.8	2171.1
DOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points			
Adjacent	18.0	1.60	28.8	Exterior Insulated		20.0	4.10	82.0		
Exterior	40.0	4.10	164.0	Exterior Insulated		20.0	4.10	82.0		
				Adjacent Insulated		18.0	1.60	28.8		
Base Total:				As-Built Total:					58.0	192.8
CEILING TYPES Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points			
Under Attic	1962.0	1.73	3394.3	Under Attic	30.0	2204.0	1.73 X 1.00	3812.9		
Base Total:				As-Built Total:					2204.0	3812.9
FLOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points			
Slab	198.0(p)	-37.0	-7326.0	Slab-On-Grade Edge Insulation	0.0	198.0(p)	-41.20	-8157.6		
Raised	0.0	0.00	0.0							
Base Total:				As-Built Total:					198.0	-8157.6

# SUMMER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: 128 SW Holly Gln, Lake City, FL, 32024-

PERMIT #:

BASE			AS-BUILT		
INFILTRATION	Area X	BSPM = Points	Area X	SPM =	Points
1962.0	10.21	20032.0	1962.0	10.21	20032.0
<b>Summer Base Points: 25833.5</b>			<b>Summer As-Built Points: 25727.0</b>		
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (DM x DSM x AHU)
			X System Multiplier	X Credit Multiplier	= Cooling Points
25833.5	0.4266	11020.6	(sys 1: Central Unit 46000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS) 25727	1.00 (1.09 x 1.147 x 1.00)	0.263
			25727.0	1.00	1.250
				0.263	0.263
				1.000	1.000
					8444.5
					8444.5

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: 128 SW Holly Gln, Lake City, FL, 32024-

PERMIT #:

BASE				AS-BUILT							
<b>GLASS TYPES</b>											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	1962.0	12.74	4499.3	Double, Clear	S	0.0	0.0	25.0	13.30	1.00	332.4
				Double, Clear	S	5.5	8.0	36.0	13.30	2.12	1013.6
				Double, Clear	S	5.5	9.0	20.0	13.30	1.92	511.6
				Double, Clear	S	1.5	7.0	45.0	13.30	1.07	642.6
				Double, Clear	W	1.5	7.0	30.0	20.73	1.02	632.1
				Double, Clear	N	0.0	0.0	16.0	24.58	1.00	393.2
				Double, Clear	N	5.5	11.0	13.2	24.58	1.01	327.4
				Double, Clear	N	5.5	4.0	15.0	24.58	1.02	376.7
				Double, Clear	N	1.5	12.0	25.0	24.58	1.00	614.3
				Double, Clear	N	1.5	12.0	15.0	24.58	1.00	368.6
				Double, Clear	N	1.5	5.0	15.0	24.58	1.00	370.0
				Double, Clear	N	1.5	5.0	9.0	24.58	1.00	222.0
				Double, Clear	N	0.0	0.0	30.0	24.58	1.00	737.3
				Double, Clear	E	1.5	7.0	15.0	18.79	1.03	289.4
				<b>As-Built Total:</b>				<b>309.2</b>	<b>6831.3</b>		
<b>WALL TYPES</b> Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	124.0	3.60	446.4	Frame, Wood, Exterior	13.0		1397.8	3.40		4752.5	
Exterior	1397.8	3.70	5171.9	Frame, Wood, Adjacent	13.0		124.0	3.30		409.2	
<b>Base Total:</b> 1521.8 5618.3				<b>As-Built Total:</b>		1521.8		5161.7			
<b>DOOR TYPES</b> Area X BWPM = Points				Type			Area X WPM = Points				
Adjacent	18.0	8.00	144.0	Exterior Insulated			20.0	8.40		168.0	
Exterior	40.0	8.40	336.0	Exterior Insulated			20.0	8.40		168.0	
				Adjacent Insulated			18.0	8.00		144.0	
<b>Base Total:</b> 58.0 480.0				<b>As-Built Total:</b>		58.0		480.0			
<b>CEILING TYPES</b> Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1962.0	2.05	4022.1	Under Attic	30.0		2204.0	2.05 X 1.00		4518.2	
<b>Base Total:</b> 1962.0 4022.1				<b>As-Built Total:</b>		2204.0		4518.2			
<b>FLOOR TYPES</b> Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	198.0(p)	8.9	1762.2	Slab-On-Grade Edge Insulation	0.0		198.0(p)	18.80		3722.4	
Raised	0.0	0.00	0.0								
<b>Base Total:</b> 1762.2				<b>As-Built Total:</b>		198.0		3722.4			

# WINTER CALCULATIONS

## Residential Whole Building Performance Method A - Details

ADDRESS: 128 SW Holly Gln, Lake City, FL, 32024-

PERMIT #:

BASE				AS-BUILT							
INFILTRATION    Area X BWPM = Points				Area X    WPM    =    Points							
1962.0            -0.59            -1157.6				1962.0            -0.59            -1157.6							
Winter Base Points:			15224.2	Winter As-Built Points:			19556.1				
Total Winter Points	X	System Multiplier	= Heating Points	Total Component (System - Points)	X	Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X	Credit Multiplier	= Heating Points
15224.2		0.6274	9551.7	(sys 1: Electric Heat Pump 46000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 19556.1            1.000    (1.069 x 1.169 x 1.00) 0.443            1.000            10822.7 19556.1            1.00            1.250            0.443            1.000            10822.7							

# Residential Whole Building Performance Method A - Details

PERMIT #:

CODE COMPLIANCE STATUS											
BASE						AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
11021		9552		7905	28477	8444		10823		7820	27087

# PASS



# Code Compliance Checklist

## Residential Whole Building Performance Method A - Details

ADDRESS: 128 SW Holly Gln, Lake City, FL, 32024-

PERMIT #:

**6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

**6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)**

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

**ESTIMATED ENERGY PERFORMANCE SCORE\* = 84.0**

**The higher the score, the more efficient the home.**

, 128 SW Holly Gln, Lake City, FL, 32024-

1. New construction or existing	New	___	12. Cooling systems	
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 46.0 kBtu/hr
3. Number of units, if multi-family	1	___		SEER: 13.00
4. Number of Bedrooms	3	___	b. N/A	___
5. Is this a worst case?	No	___	c. N/A	___
6. Conditioned floor area (ft <sup>2</sup> )	1962 ft <sup>2</sup>	___		___
7. Glass type <sup>1</sup> and area: (Label reqd. by 13-104.4.5 if not default)		___	13. Heating systems	
a. U-factor:	Description Area		a. Electric Heat Pump	Cap: 46.0 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 309.2 ft <sup>2</sup>	___		HSPF: 7.70
b. SHGC:		___	b. N/A	___
(or Clear or Tint DEFAULT)	7b. (Clear) 309.2 ft <sup>2</sup>	___	c. N/A	___
8. Floor types		___		___
a. Slab-On-Grade Edge Insulation	R=0.0, 198.0(p) ft	___	14. Hot water systems	
b. N/A		___	a. Electric Resistance	Cap: 40.0 gallons
c. N/A		___		EF: 0.93
9. Wall types		___	b. N/A	___
a. Frame, Wood, Exterior	R=13.0, 1397.8 ft <sup>2</sup>	___	c. Conservation credits	___
b. Frame, Wood, Adjacent	R=13.0, 124.0 ft <sup>2</sup>	___	(HR-Heat recovery, Solar	___
c. N/A		___	DHP-Dedicated heat pump)	___
d. N/A		___	15. HVAC credits	___
e. N/A		___	(CF-Ceiling fan, CV-Cross ventilation,	___
10. Ceiling types		___	HF-Whole house fan,	___
a. Under Attic	R=30.0, 2204.0 ft <sup>2</sup>	___	PT-Programmable Thermostat,	___
b. N/A		___	MZ-C-Multizone cooling,	___
c. N/A		___	MZ-H-Multizone heating)	___
11. Ducts		___		___
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 170.0 ft	___		___
b. N/A		___		___

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: \_\_\_\_\_ Date: \_\_\_\_\_

Address of New Home: \_\_\_\_\_ City/FL Zip: \_\_\_\_\_



*\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar<sup>TM</sup> designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at [www.fsec.ucf.edu](http://www.fsec.ucf.edu) for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

**Linda Roder**

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**From:** "Jonathan Rocco" <jayrocco@hotmail.com>  
**To:** "Linda" <linda@nfpermitservices.com>  
**Sent:** Thursday, July 31, 2008 8:14 AM  
**Subject:** RE: Scanned from MFP-05240570 07/30/2008 16:46

Linda, the last warranty Deed I gave you where Mike Stewart purchased this property is blacked out.

I have called the property apprasier and they gave it to me. Please note that this is the parcel that Mike currently lives on!!

PID # 01-5S-16-03390-007

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From: jayrocco@hotmail.com  
To: linda@nfpermitservices.com  
Subject: FW: Scanned from MFP-05240570 07/30/2008 16:46  
Date: Wed, 30 Jul 2008 16:55:53 -0400

Linda, attached are 3 warranty deeds when this property was originally purchased. I got these deeds off of Prop App web site under Michael Stewart who was the original owner. Let me know if these work.

> Date: Wed, 30 Jul 2008 16:46:28 -0400  
> From: packages@sierratitleonline.com  
> Subject: Scanned from MAP-05240570 07/30/2008 16:46  
> To: jayrocco@hotmail.com  
>  
> Scanned from MAP-05240570.  
> Date: 07/30/2008 16:46  
> Pages:6  
> Resolution:200x200 DPI  
> -----

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Stay in touch when you're away with Windows Live Messenger. [IM anytime you're online.](#)

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Keep your kids safer online with Windows Live Family Safety. [Help protect your kids.](#)

8/22/05  
Doc. 1771.01

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID  
POST OFFICE BOX 1328  
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID  
POST OFFICE BOX 1328  
LAKE CITY, FL 32056-1328

File No. 05-503

Property Appraiser's  
Parcel Identification No.  
01-58-XXXXXXXXXX

Inst: XXXXXXXXXX Date: 08/22/2005 Time: 12:55  
Doc. Stamp, Deed: 1771.00  
*mk* DC, P. DeWitt Cason, Columbia County B:1055 P:2309

### WARRANTY DEED

THIS INDENTURE, made this 19th day of August 2005, BETWEEN WESTFIELD GROUP, LLLP, a Florida Limited Liability Limited Partnership, whose post office address is Post Office Box 3566, Lake City, Florida 32056, of the County of Columbia, State of Florida, grantor\*, and MICHAEL J. STEWART and his wife, EVE S. STEWART, whose post office address is Post Office Box 3265, Lake City, Florida 32056, of the County of Columbia, State of Florida, grantee\*.

WITNESSETH: that said grantor, for and in consideration of the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

#### TOWNSHIP 5 SOUTH - RANGE 16 EAST

SECTION 1: Commence at the Southeast corner of the NW 1/4 of Section 1, in Columbia County, Florida and run thence along the East line of said NW 1/4, N 1°41'37"E 240.00 feet to the POINT OF BEGINNING; continue along said East line of NW 1/4, N 1°41'37"E 654.06 feet; run thence N 88°55'15"W 1332.26 feet to a point on the West line of the SE 1/4 of the NW 1/4 of Section 1; run thence along said West line S 1°39'45"W 654.06 feet; run thence S 88°55'15"E 1331.90 feet to the POINT OF BEGINNING.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

SUBJECT TO: Mortgage held by Donald L. Haraway and his wife, Dianne C. Haraway, and Mildred L. Crews recorded in Official Records Book 961, Page 1933, public records of Columbia County, Florida, which said Mortgage the Grantor shall pay.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

\*"Grantor" and "grantee" are used for singular or plural, as context requires.

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in the presence of:

WESTFIELD GROUP, LLLP, a  
Florida Limited Liability  
Limited Partnership

  
First Witness

Terry McDavid  
(Printed Name)

  
Second Witness


Myrtle Ann McElroy  
(Printed Name)

By: 

Charles S. Sparks  
General Partner

By: 

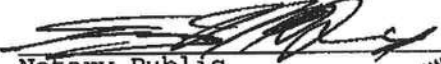
Scott D. Stewart  
General Partner

Inst:  Date: 08/22/2005 Time: 12:55  
Doc Stamp-Deed : 1771.00

DC, P. DeWitt Cason, Columbia County B: 1055 P: 2310

STATE OF FLORIDA  
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 19th day of August 2005, by CHARLES S. SPARKS and SCOTT D. STEWART, General Partners of WESTFIELD GROUP, LLLP, a Florida Limited Liability Partnership, on behalf of the partnership. They are personally known to me and did not take an oath.

  
Notary Public  
My commission expires:



Adam Papka

**COLUMBIA COUNTY BUILDING DEPARTMENT  
RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST  
FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006  
Supplements and One (1) and Two (2) Family Dwellings**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

**ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE- AND-TWO FAMILY DWELLINGS.**

**FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.**

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

**GENERAL REQUIREMENTS:**

- o Two (2) complete sets of plans containing the following:
- o All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- o Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- o Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

**Site Plan information including:**

- o Dimensions of lot or parcel of land
- o Dimensions of all building set backs
- o Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- o Provide a full legal description of property.

**Wind-load Engineering Summary, calculations and any details required:**

- o Plans or specifications must meet state compliance with FRC Chapter 3
- o The following information must be shown as per section FRC
- o Basic wind speed (3-second gust), miles per hour
- o Wind importance factor and nature of occupancy
- o Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- o The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m<sup>2</sup>), to be used for the design of exterior component and cladding materials not specifiably designed by the registered design professional.

**Elevations Drawing including:**

- o All side views of the structure
- o Roof pitch
- o Overhang dimensions and detail with attic ventilation
- o Location, size and height above roof of chimneys
- o Location and size of skylights with Florida Product Approval
- o Number of stories
- o e) Building height from the established grade to the roofs highest peak

### **Floor Plan including:**

- Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
  - All exterior and interior shear walls indicated
  - Shear wall opening shown (Windows, Doors and Garage doors)
  - Emergency escape and rescue opening in each bedroom (net clear opening shown)
  - Safety glazing of glass where needed
  - Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
  - Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
  - Plans must show and identify accessibility of bathroom (see FRC 322)
- All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

### **Foundation Plans Per FRC 403:**

- a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling.
- d) Assumed load-bearing value of soil \_\_\_\_\_ (psf)
- e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

### **CONCRETE SLAB ON GRADE Per FRC R506**

- Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports

### **PROTECTION AGAINST TERMITES Per FRC 320:**

- Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

### **Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606**

- Show all materials making up walls, wall height, and Block size, mortar type
- Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

**Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect**

### **Floor Framing System: First and/or second story**

- Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers
- Girder type, size and spacing to load bearing walls, stem wall and/or piers
- Attachment of joist to girder
- Wind load requirements where applicable
- Show required under-floor crawl space
- Show required amount of ventilation opening for under-floor spaces
- Show required covering of ventilation opening.
- Show the required access opening to access to under-floor spaces
- Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- Show Draft stopping, Fire caulking and Fire blocking
- Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- Provide live and dead load rating of floor framing systems (psf).

## **WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6**

- ✓ Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- ✓ Fastener schedule for structural members per table R602.3 (1) are to be shown.
- ✓ Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- ✓ Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- ✓ Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- ✓ Indicate where pressure treated wood will be placed.
- ✓ Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- ✓ A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

## **ROOF SYSTEMS:**

- ✓ Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- ✓ Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- ✓ Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- ✓ Provide dead load rating of trusses

## **Conventional Roof Framing Layout Per FRC 802:**

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- Valley framing and support details
- Provide dead load rating of rafter system.

## **ROOF SHEATHING FRC Table R602,3(2) FRC 803**

- ✓ Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

## **ROOF ASSEMBLIES FRC Chapter 9**

- ✓ Include all materials which will make up the roof assemblies covering; with Florida Product Approval numbers for each component of the roof assemblies covering.

## **FCB Chapter 13 Florida Energy Efficiency Code for Building Construction**

- ✓ Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

## **HVAC information shown**

- ✓ Manual J sizing equipment or equivalent computation
- ✓ Exhaust fans locations in bathrooms

## **Plumbing Fixture layout shown**

- ✓ All fixtures waste water lines shall be shown on the foundation plan

## **Electrical layout shown including:**

- ✓ Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- ✓ Ceiling fans
- ✓ Smoke detectors
- ✓ Service panel, sub-panel, location(s) and total ampere ratings

- On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- Appliances and HVAC equipment and disconnects
- Arc Fault Circuits (AFCI) in bedrooms
- Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

#### **Private Potable Water**

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

#### **THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS**

- Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.** A development permit will also be required. The permit cost is \$50.00.
- Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

Adam Papka

# PRODUCT APPROVAL SPECIFICATION SHEET

**Location:**

**Project Name:**

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at [www.floridabuilding.org](http://www.floridabuilding.org)

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
<b>A. EXTERIOR DOORS</b>			
1. Swinging	Mayfair	entry door	FL 1311
2. Sliding			
3. Sectional	Raynor	garage door	FL 3070
4. Roll up			
5. Automatic			
6. Other			
<b>B. WINDOWS</b>			
1. Single hung	Danville	single hung windows	FL 1369
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
<b>C. PANEL WALL</b>			
1. Siding	Jones Hardie	hardiboard siding	FL 889-R1
2. Soffits	HST Blending products	aluminum & vinyl soffit	FL 5546 1 & 2
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
<b>D. ROOFING PRODUCTS</b>			
1. Asphalt Shingles	Tamko	30-year asphalt	FL 673
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives - Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
<b>E. SHUTTERS</b>			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
<b>F. SKYLIGHTS</b>			
1. Skylight			
2. Other			
<b>G. STRUCTURAL COMPONENTS</b>			
1. Wood connector/anchor	Simpson Strong	wood connectors/anchors	FL-1474
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
<b>H. NEW EXTERIOR ENVELOPE PRODUCTS</b>			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

*Linda Roder*

Contractor or Contractor's Authorized Agent Signature

Linda Roder

Print Name

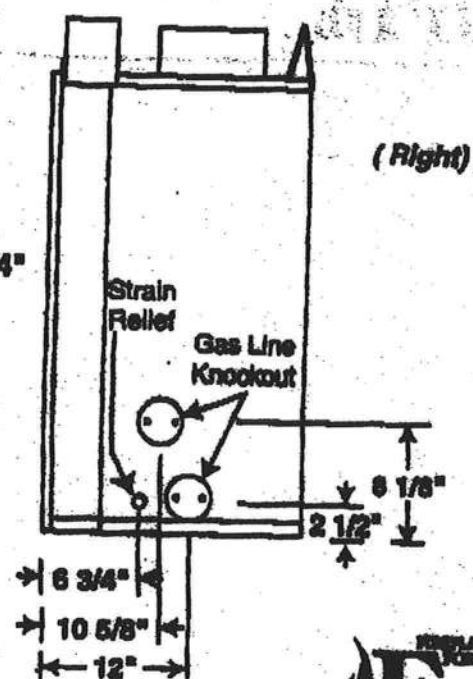
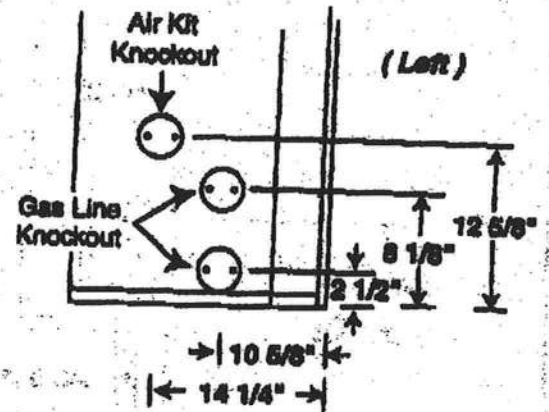
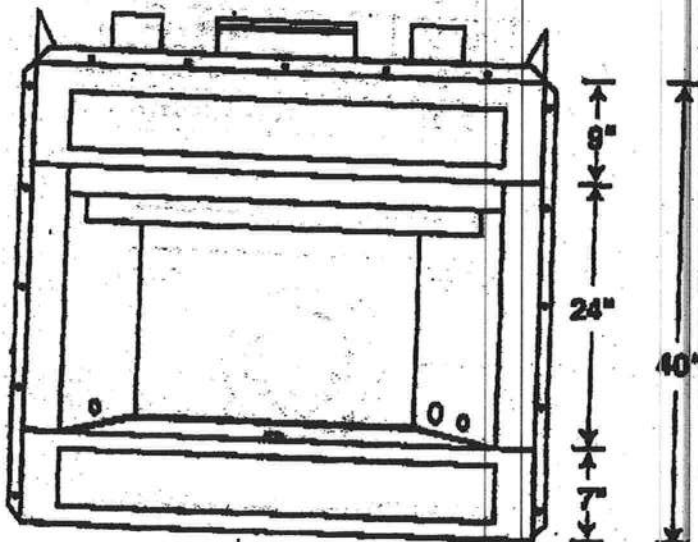
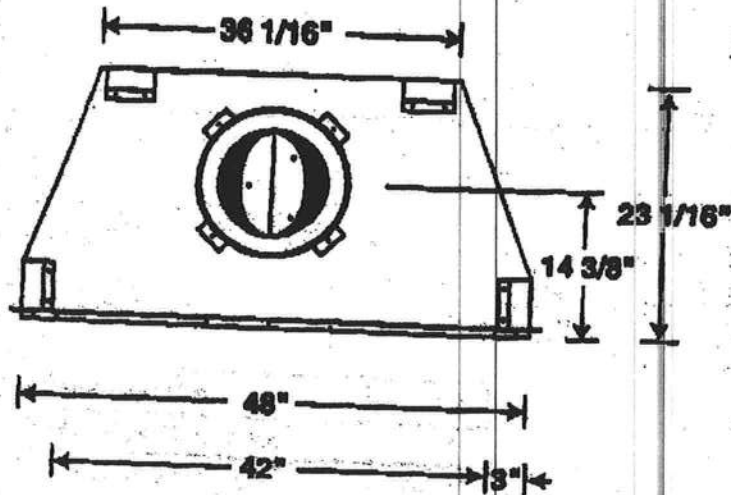
7-29-08

Date

# Craftsman

## 42" Woodburning Fireplace

Vent Pipe Size	10"
Min. Pipe Clearance	1"
Min. System Height	14' 6"
- w/ Single Offset	14' 6"
- w/ Two Offsets	22' 0"
Max. Dist. Between Elbows	6' 0"
Max. System Height	50' 0"

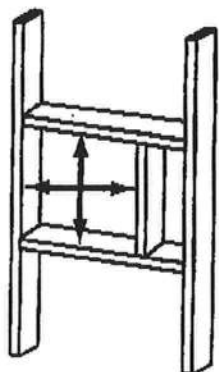


**Fmi**

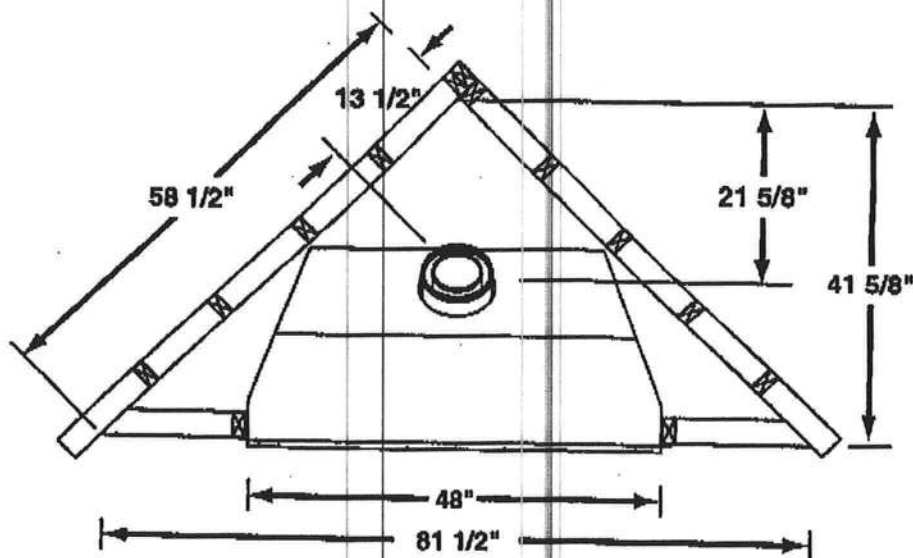
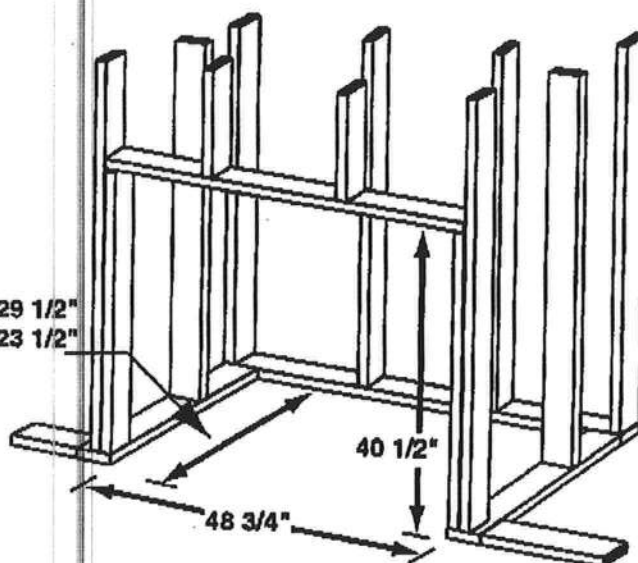
# Victorian

## 42" Direct Vent Fireplace

Vent Opening - 10 3/4" Square (I.D.)



Vertical Termination - 29 1/2"  
Horizontal Termination - 23 1/2"



**NOTE:**

Built-in Features Such as Mantels, Bookshelves, etc. Made of Combustible Materials Must Maintain Minimum Clearances from the Fireplace. See Installation Instructions for Complete Information

FIREPLACES  
FOR BUILDERS  
**Fmi**

# Residential System Sizing Calculation

## Summary

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

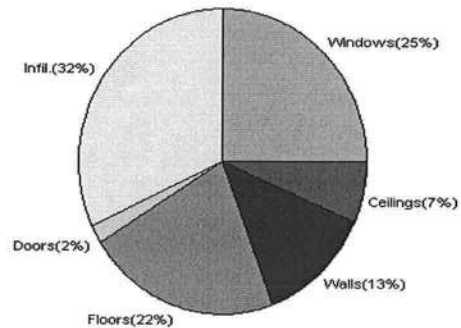
7/24/2008

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)			
Winter design temperature	33 F	Summer design temperature	92 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	37 F	Summer temperature difference	17 F
<b>Total heating load calculation</b>	<b>39395 Btuh</b>	<b>Total cooling load calculation</b>	<b>32676 Btuh</b>
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	116.8 46000	Sensible (SHR = 0.75)	134.7 34500
Heat Pump + Auxiliary(0.0kW)	116.8 46000	Latent	163.0 11500
		Total (Electric Heat Pump)	140.8 46000

## WINTER CALCULATIONS

Winter Heating Load (for 1962 sqft)

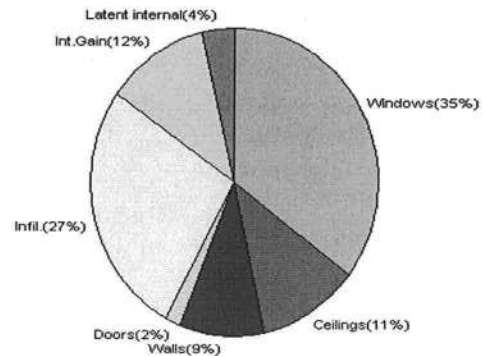
Load component		Load	
Window total	309 sqft	9953	Btuh
Wall total	1522 sqft	4998	Btuh
Door total	58 sqft	751	Btuh
Ceiling total	2204 sqft	2597	Btuh
Floor total	198 sqft	8645	Btuh
Infiltration	307 cfm	12451	Btuh
Duct loss		0	Btuh
<b>Subtotal</b>		<b>39395</b>	<b>Btuh</b>
Ventilation	0 cfm	0	Btuh
<b>TOTAL HEAT LOSS</b>		<b>39395</b>	<b>Btuh</b>



## SUMMER CALCULATIONS

Summer Cooling Load (for 1962 sqft)

Load component		Load	
Window total	309 sqft	11537	Btuh
Wall total	1522 sqft	3103	Btuh
Door total	58 sqft	568	Btuh
Ceiling total	2204 sqft	3650	Btuh
Floor total		0	Btuh
Infiltration	160 cfm	2982	Btuh
Internal gain		3780	Btuh
Duct gain		0	Btuh
Sens. Ventilation	0 cfm	0	Btuh
<b>Total sensible gain</b>		<b>25620</b>	<b>Btuh</b>
Latent gain(ducts)		0	Btuh
Latent gain(infiltration)		5856	Btuh
Latent gain(ventilation)		0	Btuh
Latent gain(internal/occupants/other)		1200	Btuh
<b>Total latent gain</b>		<b>7056</b>	<b>Btuh</b>
<b>TOTAL HEAT GAIN</b>		<b>32676</b>	<b>Btuh</b>



For Florida residences only

EnergyGauge® System Sizing

PREPARED BY:

DATE: 7/24/08

# System Sizing Calculations - Winter

## Residential Load - Whole House Component Details

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

7/24/2008

### Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	S	25.0	32.2	805 Btuh
2	2, Clear, Metal, 0.87	S	36.0	32.2	1159 Btuh
3	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
4	2, Clear, Metal, 0.87	S	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btuh
6	2, Clear, Metal, 0.87	N	16.0	32.2	515 Btuh
7	2, Clear, Metal, 0.87	N	13.2	32.2	425 Btuh
8	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
9	2, Clear, Metal, 0.87	N	25.0	32.2	805 Btuh
10	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
11	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
12	2, Clear, Metal, 0.87	N	9.0	32.2	290 Btuh
13	2, Clear, Metal, 0.87	N	30.0	32.2	966 Btuh
14	2, Clear, Metal, 0.87	E	15.0	32.2	483 Btuh
Window Total			309(sqft)		9953 Btuh
<b>Walls</b>	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1398	3.3	4590 Btuh
2	Frame - Wood - Adj(0.09)	13.0	124	3.3	407 Btuh
Wall Total			1522		4998 Btuh
<b>Doors</b>	Type		Area X	HTM=	Load
1	Insulated - Adjacent		18	12.9	233 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
Door Total			58		751Btuh
<b>Ceilings</b>	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2204	1.2	2597 Btuh
Ceiling Total			2204		2597Btuh
<b>Floors</b>	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	198.0 ft(p)	43.7	8645 Btuh
Floor Total			198		8645 Btuh
Zone Envelope Subtotal:					26944 Btuh
<b>Infiltration</b>	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	19620	307.4	12451 Btuh
<b>Ductload</b>	Partially sealed, R6.0, Supply(Attic), Return(Attic) (DLM of 0.00)				0 Btuh
<b>Zone #1</b>	Sensible Zone Subtotal				39395 Btuh

# Manual J Winter Calculations

## Residential Load - Component Details (continued)

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

7/24/2008

### WHOLE HOUSE TOTALS

	Subtotal Sensible	39395 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	39395 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
(Frame types - metal, wood or insulated metal)  
(U - Window U-Factor or 'DEF' for default)  
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



For Florida residences only

# System Sizing Calculations - Winter

## Residential Load - Room by Room Component Details

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

7/24/2008

### Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	S	25.0	32.2	805 Btuh
2	2, Clear, Metal, 0.87	S	36.0	32.2	1159 Btuh
3	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
4	2, Clear, Metal, 0.87	S	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	W	30.0	32.2	966 Btuh
6	2, Clear, Metal, 0.87	N	16.0	32.2	515 Btuh
7	2, Clear, Metal, 0.87	N	13.2	32.2	425 Btuh
8	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
9	2, Clear, Metal, 0.87	N	25.0	32.2	805 Btuh
10	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
11	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
12	2, Clear, Metal, 0.87	N	9.0	32.2	290 Btuh
13	2, Clear, Metal, 0.87	N	30.0	32.2	966 Btuh
14	2, Clear, Metal, 0.87	E	15.0	32.2	483 Btuh
Window Total			309(sqft)		9953 Btuh
<b>Walls</b>	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1398	3.3	4590 Btuh
2	Frame - Wood - Adj(0.09)	13.0	124	3.3	407 Btuh
Wall Total			1522		4998 Btuh
<b>Doors</b>	Type		Area X	HTM=	Load
1	Insulated - Adjacent		18	12.9	233 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
Door Total			58		751Btuh
<b>Ceilings</b>	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2204	1.2	2597 Btuh
Ceiling Total			2204		2597Btuh
<b>Floors</b>	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	198.0 ft(p)	43.7	8645 Btuh
Floor Total			198		8645 Btuh
Zone Envelope Subtotal:					26944 Btuh
<b>Infiltration</b>	Type	ACH X	Zone Volume	CFM=	
	Natural	0.94	19620	307.4	12451 Btuh
<b>Ductload</b>	Partially sealed, R6.0, Supply(Attic), Return(Attic) (DLM of 0.00)				0 Btuh
<b>Zone #1</b>	Sensible Zone Subtotal				39395 Btuh

# Manual J Winter Calculations

## Residential Load - Component Details (continued)

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

7/24/2008

### WHOLE HOUSE TOTALS

	Subtotal Sensible	39395 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	39395 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
(Frame types - metal, wood or insulated metal)  
(U - Window U-Factor or 'DEF' for default)  
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types )



For Florida residences only

# System Sizing Calculations - Summer

## Residential Load - Whole House Component Details

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

7/24/2008

### Component Loads for Whole House

Window	Type*		Overhang		Window Area(sqft)			HTM		Load	
	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	S	0ft.	0ft.	25.0	0.0	25.0	29	29	724	Btuh
2	2, Clear, 0.87, None,N,N	S	5.5ft	8ft.	36.0	0.0	36.0	29	29	1043	Btuh
3	2, Clear, 0.87, None,N,N	S	5.5ft	9ft.	20.0	0.0	20.0	29	29	579	Btuh
4	2, Clear, 0.87, None,N,N	S	1.5ft	7ft.	45.0	0.0	45.0	29	29	1303	Btuh
5	2, Clear, 0.87, None,N,N	W	1.5ft	7ft.	30.0	0.0	30.0	29	80	2385	Btuh
6	2, Clear, 0.87, None,N,N	N	0ft.	0ft.	16.0	0.0	16.0	29	34	538	Btuh
7	2, Clear, 0.87, None,N,N	N	5.5ft	11ft.	13.2	13.2	0.0	29	34	382	Btuh
8	2, Clear, 0.87, None,N,N	N	5.5ft	4ft.	15.0	15.0	0.0	29	34	434	Btuh
9	2, Clear, 0.87, None,N,N	N	1.5ft	12ft.	25.0	12.6	12.4	29	34	782	Btuh
10	2, Clear, 0.87, None,N,N	N	1.5ft	12ft.	15.0	7.5	7.5	29	34	469	Btuh
11	2, Clear, 0.87, None,N,N	N	1.5ft	5ft.	15.0	15.0	0.0	29	34	434	Btuh
12	2, Clear, 0.87, None,N,N	N	1.5ft	5ft.	9.0	9.0	0.0	29	34	261	Btuh
13	2, Clear, 0.87, None,N,N	N	0ft.	0ft.	30.0	0.0	30.0	29	34	1009	Btuh
14	2, Clear, 0.87, None,N,N	E	1.5ft	7ft.	15.0	0.0	15.0	29	80	1193	Btuh
Window Total					309 (sqft)					11537 Btuh	
Walls	Type	R-Value/U-Value			Area(sqft)			HTM		Load	
1	Frame - Wood - Ext	13.0/0.09			1397.8			2.1		2916 Btuh	
2	Frame - Wood - Adj	13.0/0.09			124.0			1.5		187 Btuh	
Wall Total					1522 (sqft)					3103 Btuh	
Doors	Type				Area (sqft)			HTM		Load	
1	Insulated - Adjacent				18.0			9.8		176 Btuh	
2	Insulated - Exterior				20.0			9.8		196 Btuh	
3	Insulated - Exterior				20.0			9.8		196 Btuh	
Door Total					58 (sqft)					568 Btuh	
Ceilings	Type/Color/Surface	R-Value			Area(sqft)			HTM		Load	
1	Vented Attic/DarkShingle	30.0			2204.0			1.7		3650 Btuh	
Ceiling Total					2204 (sqft)					3650 Btuh	
Floors	Type	R-Value			Size			HTM		Load	
1	Slab On Grade	0.0			198 (ft(p))			0.0		0 Btuh	
Floor Total					198.0 (sqft)					0 Btuh	
Zone Envelope Subtotal:										18858 Btuh	
Infiltration	Type	ACH			Volume(cuft)			CFM=		Load	
	SensibleNatural	0.49			19620			160.2		2982 Btuh	
Internal gain	Occupants			Btuh/occupant			Appliance		Load		
	6			X 230 +			2400		3780 Btuh		
Duct load	Partially sealed, R6.0, Supply(Attic), Return(Attic)							DGM = 0.00		0.0 Btuh	
Sensible Zone Load										25620 Btuh	

# Manual J Summer Calculations

## Residential Load - Component Details (continued)

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

7/24/2008

### WHOLE HOUSE TOTALS

<b>Whole House Totals for Cooling</b>	<b>Sensible Envelope Load All Zones</b>	<b>25620 Btuh</b>
	Sensible Duct Load	0 Btuh
	<b>Total Sensible Zone Loads</b>	<b>25620 Btuh</b>
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	<b>Total sensible gain</b>	<b>25620 Btuh</b>
	Latent infiltration gain (for 54 gr. humidity difference)	5856 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	<b>Latent total gain</b>	<b>7056 Btuh</b>
	<b>TOTAL GAIN</b>	<b>32676 Btuh</b>

\*Key: Window types (Pn - Number of panes of glass)  
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
(U - Window U-Factor or 'DEF' for default)  
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))  
(ExSh - Exterior shading device: none(N) or numerical value)  
(BS - Insect screen: none(N), Full(F) or Half(H))  
(Ornt - compass orientation)



For Florida residences only

# System Sizing Calculations - Summer

## Residential Load - Room by Room Component Details

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

7/24/2008

### Component Loads for Zone #1: Main

Window	Type*		Overhang		Window Area(sqft)			HTM		Load		
	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded			
1	2, Clear, 0.87, None,N,N	S	0ft.	0ft.	25.0	0.0	25.0	29	29	724	Btuh	
2	2, Clear, 0.87, None,N,N	S	5.5ft	8ft.	36.0	0.0	36.0	29	29	1043	Btuh	
3	2, Clear, 0.87, None,N,N	S	5.5ft	9ft.	20.0	0.0	20.0	29	29	579	Btuh	
4	2, Clear, 0.87, None,N,N	S	1.5ft	7ft.	45.0	0.0	45.0	29	29	1303	Btuh	
5	2, Clear, 0.87, None,N,N	W	1.5ft	7ft.	30.0	0.0	30.0	29	80	2385	Btuh	
6	2, Clear, 0.87, None,N,N	N	0ft.	0ft.	16.0	0.0	16.0	29	34	538	Btuh	
7	2, Clear, 0.87, None,N,N	N	5.5ft	11ft.	13.2	13.2	0.0	29	34	382	Btuh	
8	2, Clear, 0.87, None,N,N	N	5.5ft	4ft.	15.0	15.0	0.0	29	34	434	Btuh	
9	2, Clear, 0.87, None,N,N	N	1.5ft	12ft.	25.0	12.6	12.4	29	34	782	Btuh	
10	2, Clear, 0.87, None,N,N	N	1.5ft	12ft.	15.0	7.5	7.5	29	34	469	Btuh	
11	2, Clear, 0.87, None,N,N	N	1.5ft	5ft.	15.0	15.0	0.0	29	34	434	Btuh	
12	2, Clear, 0.87, None,N,N	N	1.5ft	5ft.	9.0	9.0	0.0	29	34	261	Btuh	
13	2, Clear, 0.87, None,N,N	N	0ft.	0ft.	30.0	0.0	30.0	29	34	1009	Btuh	
14	2, Clear, 0.87, None,N,N	E	1.5ft	7ft.	15.0	0.0	15.0	29	80	1193	Btuh	
Window Total					309 (sqft)					11537 Btuh		
Walls	Type	R-Value/U-Value			Area(sqft)			HTM		Load		
1	Frame - Wood - Ext	13.0/0.09			1397.8			2.1		2916 Btuh		
2	Frame - Wood - Adj	13.0/0.09			124.0			1.5		187 Btuh		
Wall Total					1522 (sqft)					3103 Btuh		
Doors	Type				Area (sqft)			HTM		Load		
1	Insulated - Adjacent				18.0			9.8		176 Btuh		
2	Insulated - Exterior				20.0			9.8		196 Btuh		
3	Insulated - Exterior				20.0			9.8		196 Btuh		
Door Total					58 (sqft)					568 Btuh		
Ceilings	Type/Color/Surface	R-Value			Area(sqft)			HTM		Load		
1	Vented Attic/DarkShingle	30.0			2204.0			1.7		3650 Btuh		
Ceiling Total					2204 (sqft)					3650 Btuh		
Floors	Type	R-Value			Size			HTM		Load		
1	Slab On Grade	0.0			198 (ft(p))			0.0		0 Btuh		
Floor Total					198.0 (sqft)					0 Btuh		
Zone Envelope Subtotal:										18858 Btuh		
Infiltration	Type	ACH			Volume(cuft)			CFM=		Load		
	SensibleNatural	0.49			19620			160.2		2982 Btuh		
Internal gain		Occupants			Btuh/occupant			Appliance		Load		
		6			X 230 +			2400		3780 Btuh		
Duct load	Partially sealed, R6.0, Supply(Attic), Return(Attic)							DGM = 0.00		0.0 Btuh		
	Sensible Zone Load										25620 Btuh	

# Manual J Summer Calculations

## Residential Load - Component Details (continued)

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191Adam'sFraming&Construction

Class 3 Rating  
Registration No. 0  
Climate: North

7/24/2008

### WHOLE HOUSE TOTALS

<b>Whole House Totals for Cooling</b>	<b>Sensible Envelope Load All Zones</b>	<b>25620 Btuh</b>
	Sensible Duct Load	0 Btuh
	<b>Total Sensible Zone Loads</b>	<b>25620 Btuh</b>
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	<b>Total sensible gain</b>	<b>25620 Btuh</b>
	Latent infiltration gain (for 54 gr. humidity difference)	5856 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	<b>Latent total gain</b>	<b>7056 Btuh</b>
	<b>TOTAL GAIN</b>	<b>32676 Btuh</b>

\*Key: Window types (Pn - Number of panes of glass)  
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)  
(U - Window U-Factor or 'DEF' for default)  
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))  
(ExSh - Exterior shading device: none(N) or numerical value)  
(BS - Insect screen: none(N), Full(F) or Half(H))  
(Omt - compass orientation)



For Florida residences only

# Residential Window Diversity

## MidSummer

128 SW Holly Gln  
Lake City, FL 32024-

Project Title:  
802191 Adam's Framing & Construction

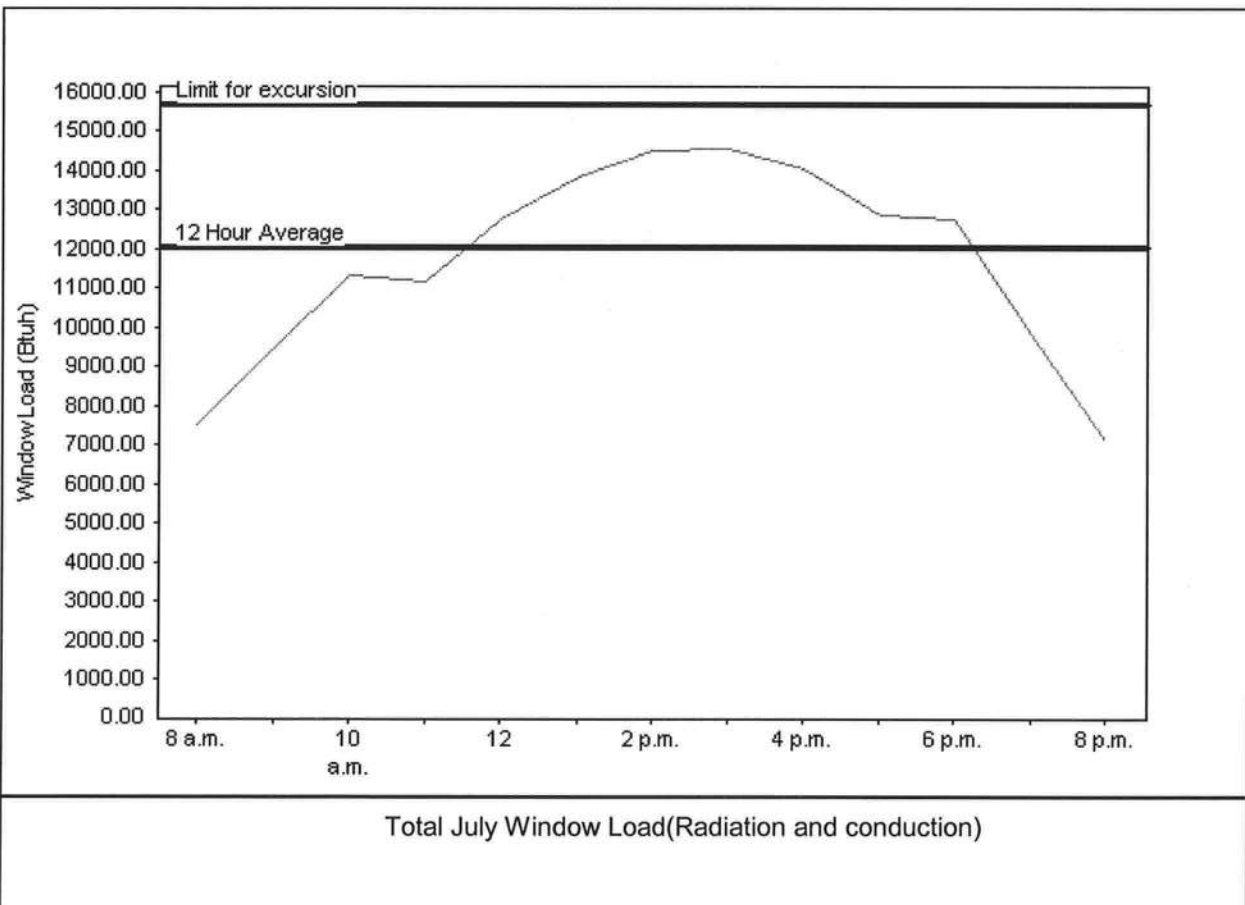
Class 3 Rating  
Registration No. 0  
Climate: North

7/24/2008

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	12055 Btu
Summer setpoint	75 F	Peak window load for July	14578 Btu
Summer temperature difference	17 F	Excursion limit(130% of Ave.)	15671 Btu
Latitude	29 North	Window excursion (July)	None

## WINDOW Average and Peak Loads



The midsummer window load for this house does not exceed the window load excursion limit.  
This house has adequate midsummer window diversity.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY:

DATE:

7/24/08



EnergyGauge® FLR2PB v4.1

# New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

**Public reporting burden** for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

27286

## Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.  
Company Address: P.O. Box 1795 City Lake City State FL Zip 32055  
Company Business License No. JB109476 Company Phone No. 386-755-3611 • 352-494-5751  
FHA/VA Case No. (if any) \_\_\_\_\_

## Section 2: Builder Information

Company Name: Adam Papko Company Phone No. \_\_\_\_\_

## Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 124 S.W. Holly Glen  
Lake City, FL  
Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other \_\_\_\_\_  
Approximate Depth of Footing: Outside 12 Inside 24 Type of Fill Asst.

## Section 4: Treatment Information

Date(s) of Treatment(s) 9-8-08  
Brand Name of Product(s) Used Bifent  
EPA Registration No. 53483-169  
Approximate Final Mix Solution % 1.6  
Approximate Size of Treatment Area: Sq. ft. 2524 Linear ft. 242 Linear ft. of Masonry Voids 242  
Approximate Total Gallons of Solution Applied 494  
Was treatment completed on exterior? ☐ Yes ☒ No  
Service Agreement Available? ☒ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) \_\_\_\_\_

Comments \_\_\_\_\_

Name of Applicator(s) Steve Brennan Certification No. (if required by State law) JB104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Steve Brennan Date 9-8-08

**Warning:** HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

form HUD-NPCA-99-B (04/2003)

This document prepared by:  
First Federal Bank of Florida  
Teresa Davis  
P. O. Box 2029  
Lake City, FL 32056


Inst:200812015818 Date:8/26/2008 Time:12:05 PM  
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1157 P:648


### NOTICE OF COMMENCEMENT

#### STATE OF FLORIDA COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following is provided in this Notice of Commencement:

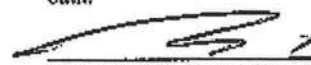
1. Description of Property: See attached Exhibit "A"
2. General Description of Improvement: construction of single family residence
3. Owner Information:
  - a. Name and Address: Adam and Leah Papka  
429 SW Meadow Terr  
Lake City, FL 32024
  - b. Interest in Property: 100%
  - c. Name and Address of Fee Simple Titleholder (if other than Owner)
4. Contractor (name and address): Adam's Framing and Construction  
P O Box 1921  
Lake City, FL 32056-1921
5. Surety:
  - a. NA
  - b. NA
6. Lender: First Federal Bank of Florida, PO Box 2029, Lake City, Florida, 32056
7. Persons within the State of Florida designated by Owner upon notices or other documents may be served as provided by Section 713.13(1)(a)(7), Florida Statutes: None
8. In addition to himself, the Owner designates Teresa Davis of First Federal Bank of Florida, PO Box 2029, Lake City, FL 32056 to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.
9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified):

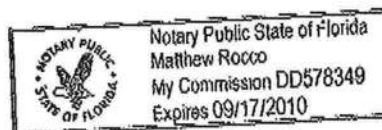
Adam Papka: 

Leah Papka: 

State of Florida  
County of Columbia

The foregoing instrument was acknowledged before me this 25th day of August, 2008 by Adam Papka and Leah Papka, who are personally known to me and who did not take an oath.

  
Notary Public, State of Florida



## EXHIBIT "A"

Commence at the Southeast corner of the NW 1/4, Section 1, Township 5 South, Range 16 East, Columbia County, Florida and thence run N 01°41'37" E, along the East line of said NW 1/4, 596.70 feet; thence N 88°45'17" W, 9.11 feet to the East maintained right of way of SW Little Road and the Point of Beginning; thence continue N 88°45'17" W, 234.96 feet; thence S 01°14'21" W, 189.88 feet; thence S 88°45'17" E, 232.93 feet to said East maintained right of way; thence N 01°51'07" E, along said East maintained right of way, 189.89 feet to the Point of Beginning.

Subject to and together with an ingress and egress easement (50 feet wide), lying 25 feet on each side of the following described centerline:

Commence at the Southeast corner of the NW 1/4, Section 1, Township 5 South, Range 16 East, Columbia County, Florida and run thence N 01°41'37" E, along the East line of said NW 1/4, 596.70 feet; thence N 88°45'17" W, 9.11 feet to the East maintained right of way of SW Little Road and the Point of Beginning of said centerline; thence continue N 88°45'17" W, 234.96 feet to the Point of Termination.



# CAL-TECH TESTING, INC.

## ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625  
4784 Rosselle St. • Jacksonville, FL 32254  
2230 Greensboro Hwy., Quincy, FL 32351

Lake City • (386) 755-3633

Fax • (386) 752-5456

Jacksonville • (904) 381-8901

Fax • (904) 381-8902

Quincy • (850) 442-3495

Fax • (850) 442-4008

JOB NO.: 08-477  
DATE TESTED: 9-8-08

## REPORT OF IN-PLACE DENSITY TEST

27286

ASTM METHOD: ☒ (D-2922) Nuclear ☐ (D-2937) Drive Cylinder ☐ Other

PROJECT: Holly Glen Lot 8B

CLIENT: Adams Framing & Const.

GENERAL CONTRACTOR: SAC EARTHWORK CONTRACTOR:

SOIL USE (SEE NOTE): (1) SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: F. Geiger

MODIFIED (ASTM D-1557): ☒ STANDARD (ASTM D-698): ☒

TEST NO.	TEST LOCATION	TEST:	PROCTOR NO.	WET DENS. LBS./CU. FT.	DRY DENS. LBS./CU. FT.	MOIST PERCENT	% MAX. DENS.
		DEPTH ELEV. LIFT					
1	10' E X 8' N of SW corner	12" *	*	108.8	103.3	5.3	96.5
2	12' W X 14' N of SE corner	1	1	109.2	104.7	4.3	97.8
3	Approx Ctr of Rd	1	1	110.0	106.3	3.5	99.4

REMARKS:

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
*	#2 Dan Register Pit	107.0	11.2

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other  
The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

# COLUMBIA COUNTY FLORIDA DEPARTMENT OF BUILDING AND ZONING

## OCCUPANCY

### COLUMBIA COUNTY, FLORIDA

#### Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 01-5S-16-03390-018

Building permit No. 000027286

Use Classification SFD, UTILITY

Fire: 44.94

Permit Holder ADAM PAPKA

Waste: 117.25

Owner of Building ADAM PAPKA

Total: 162.19

Location: 128 SW HOLLY GLEN, LAKE CITY, FL

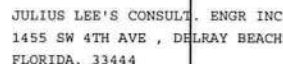
Date: 03/25/2009

*Harry Dieke*

Building Inspector

POST IN A CONSPICUOUS PLACE  
(Business Places Only)





**Note:** Refer to individual truss design drawings for special loading conditions, design criteria, truss geometry, lumber, and plate information.

License # : 34869

*By julius lee at 7:08 pm, Jul 23, 2008*

This truss specification package consists of this index sheet and 55 truss design drawings. This signed and sealed index sheet indicates acceptance of my professional engineering responsibility solely for listed truss design drawings. The suitability and use of each truss component for any particular building is the responsibility of the building designer per ANSI/TPI 1-1995, Section 2.

[illegible]

<b>Project Information:</b>			Builder: ADAMS FRAMING AND CONSTRUCTION			Builders FirstSource		
			Model: custom			2525 E. Duval St.		
Builders FirstSource Job #: L273732						Lake City, FL 32055		

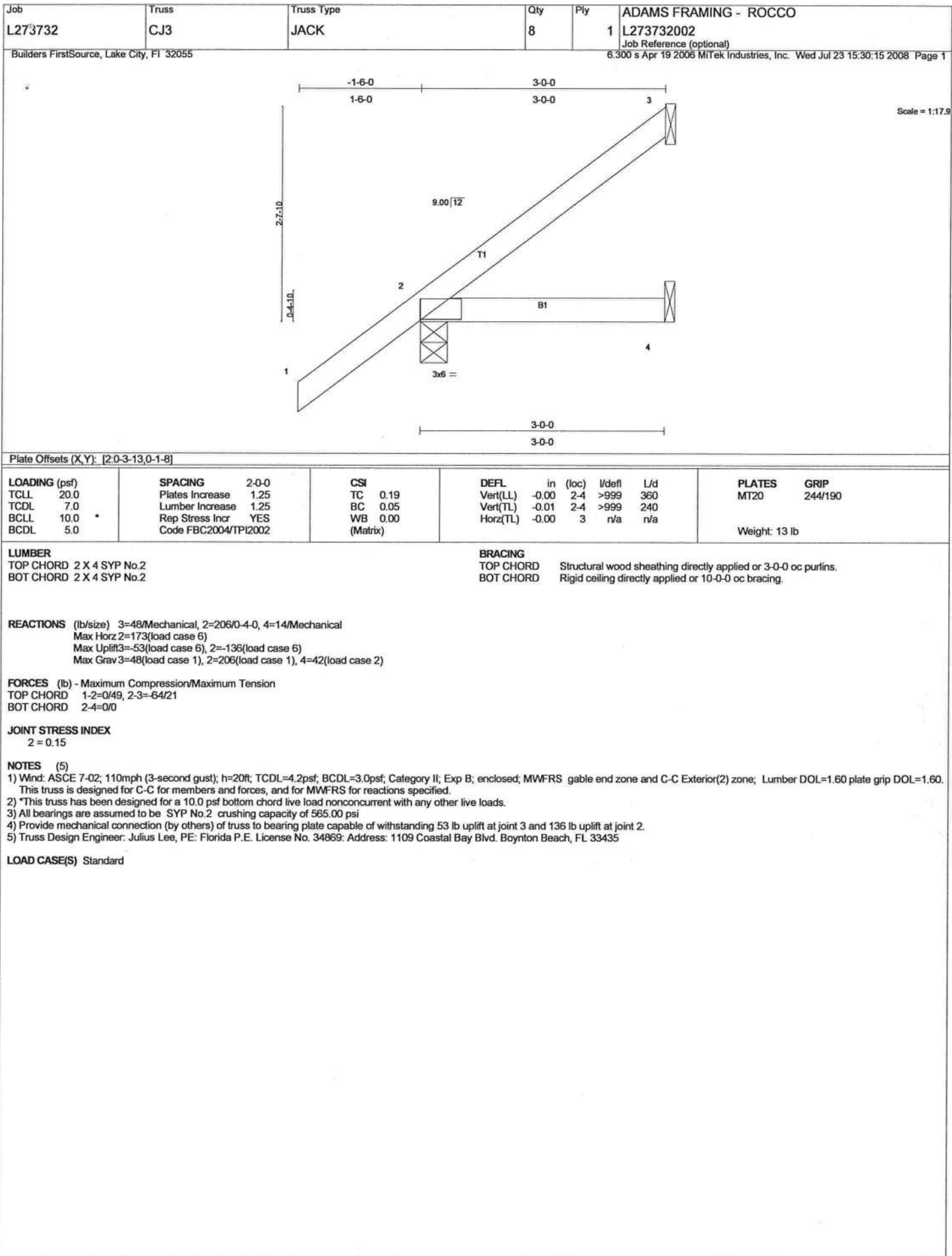


To whom it may concern,

This letter is intended to address the issue of warning notes on 7' jack trusses. I have reviewed the jack truss and it passes without modification for any jack up to 7' with a total loading not to exceed 55# and a maximum overhang of 2'. Below is a copy of note you will see on the jack. This letter will act as an approval for the truss mentioned above.

**\*\*\*Design Problems\*\*\* Review Required/ Max Deflection In Panel Exceeded: A-B**





Job L273732	Truss CJ5	Truss Type JACK	Qty 2	Ply 1	ADAMS FRAMING - ROCCO L273732004 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:16 2008 Page 1		

Scale = 1/25.2

Plate Offsets (X,Y): [2-0-3-13,0-1-8]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCCL 20.0	Plates Increase	1.25	TC 0.23	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.17	Vert(TL)	-0.05	2-4	>999	240		
BCCL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCCL 5.0	Code FBC2004/TP12002		(Matrix)							
Weight: 20 lb										

<b>LUMBER</b>		<b>BRACING</b>	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=118/Mechanical, 2=254/Mechanical, 4=24/Mechanical  
 Max Horz 2=242(load case 6)  
 Max Uplift 3=137(load case 6), 2=120(load case 6)  
 Max Grav 3=118(load case 1), 2=254(load case 1), 4=73(load case 2)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/48, 2-3=-118/56  
 BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**  
 2 = 0.17

**NOTES** (5)  
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60.  
 This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 3 and 120 lb uplift at joint 2.  
 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

Job L273732	Truss EJ1	Truss Type MONO TRUSS	Qty 2	Ply 1	ADAMS FRAMING - ROCCO L273732006 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:18 2008 Page 1		

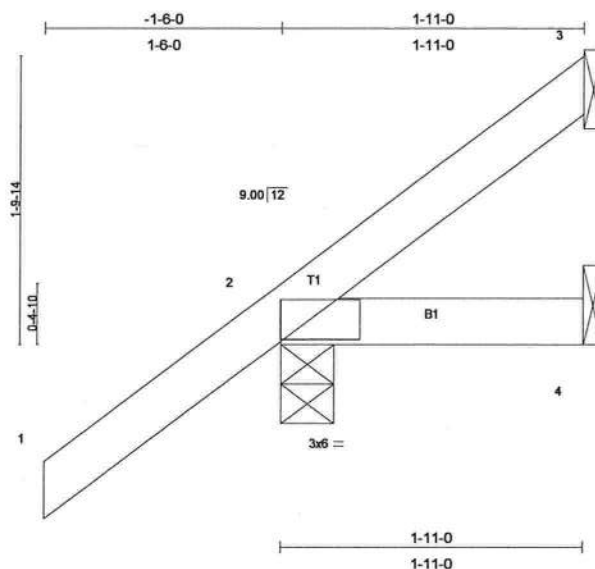


Plate Offsets (X,Y): [2-0-3-13,0-1-8]

<b>LOADING (psf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCCL 20.0	Plates Increase 1.25	TC 0.17	Vert(LL) -0.00 2 >999 360	MT20	244/190
TCCL 7.0	Lumber Increase 1.25	BC 0.02	Vert(TL) -0.00 2-4 >999 240		
BCCL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCCL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 10 lb	

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 1-11-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=177/0-4-0, 4=9/Mechanical, 3=17/Mechanical  
Max Horz 2=137(load case 6)  
Max Uplift 2=-144(load case 6), 3=-23(load case 7)  
Max Grav 2=177(load case 1), 4=28(load case 2), 3=25(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/47, 2-3=-53/15  
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**  
2 = 0.13

**NOTES** (5)  
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60.  
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 2 and 23 lb uplift at joint 3.  
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:19 2008 Page 1

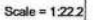


**Weight:** 9 lb

TOP CHORD	Structural wood sheathing directly applied or 1-8-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

LOAD CASE(S) Standard

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:21 2008 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.32	Vert(LL) 0.05 1-3	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.33	Vert(TL) -0.09 1-3	>605	240		
BCLL 10.0	Rep Stress Incr NO	WB 0.00	Horz(TL) -0.00 2	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)					
						Weight: 17 lb	

<b>BRACING</b>	
<b>TOP CHORD</b>	Structural wood sheathing directly applied or 5-0-0 oc purlins.
<b>BOT CHORD</b>	Rigid ceiling directly applied or 10-0-0 oc bracing.

**NOTES** (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate gird DOL=1.60.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

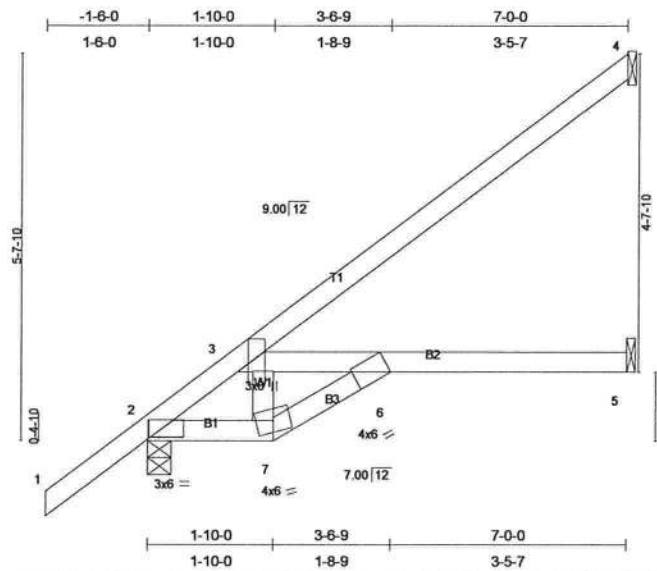
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 1, 158 lb uplift at joint 2 and 42 lb uplift at joint 3.

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869. Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard  
1) Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (plf)  
Vert: 1-2=-54, 1-3=-56(F=46)

Job L273732	Truss EJ7A	Truss Type SPECIAL	Qty 6	Ply 1	ADAMS FRAMING - ROCCO L273732012 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:22 2008 Page 1		



Scale = 1/32\"/>

Plate Offsets (X,Y): [2'-0-6-3,0-0-6], [3'-0-4-0,0-0-8]

LOADING (psf)	SPACING	CSI	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	TC (LL)	0.12	5-6	>699	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.44	Vert(TL)	-0.17	5-6	>471	240		
BCLL 10.0	Rep Stress Incr YES	WB 0.03	Horz(TL)	0.05	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
									Weight: 31 lb

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

**REACTIONS** (lb/size) 4=134/Mechanical, 2=333/0-4-0, 5=79/Mechanical  
Max Horz 2=223(load case 6)  
Max Uplift 4=-97(load case 6), 2=-68(load case 6), 5=-11(load case 6)  
Max Grav 4=134(load case 1), 2=333(load case 1), 5=120(load case 2)

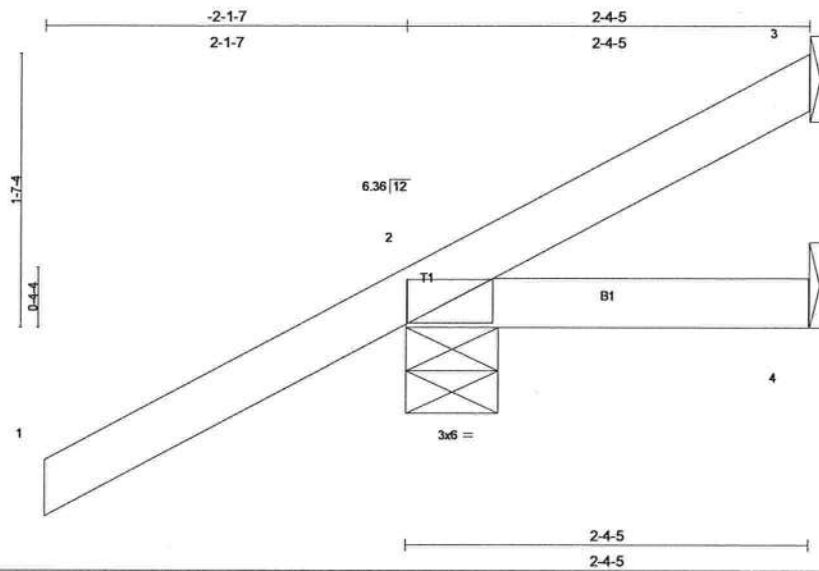
**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/49, 2-3=-336/0, 3-4=-126/62  
BOT CHORD 2-7=-261/262, 6-7=-200/230, 3-6=-232/197, 5-6=0/0  
WEBS 3-7=-60/129

**JOINT STRESS INDEX**  
2 = 0.68, 3 = 0.58, 6 = 0.42 and 7 = 0.22

**NOTES** (5)  
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 4, 68 lb uplift at joint 2 and 11 lb uplift at joint 5.  
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

Job L273732	Truss HJ2A	Truss Type JACK	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732014 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:23 2008 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.03	Vert(TL)	-0.00	2-4	>999	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 11 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 2-4-5 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS (lb/size)

3=-44/Mechanical, 2=226/0-6-7, 4=6/Mechanical  
Max Horz 2=88(load case 5)  
Max Uplift 3=-44(load case 1), 2=-251(load case 5), 4=-22(load case 3)  
Max Grav 3=73(load case 5), 2=226(load case 1), 4=26(load case 2)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-50/38  
BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.11

#### NOTES (6)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 3, 251 lb uplift at joint 2 and 22 lb uplift at joint 4.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

#### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

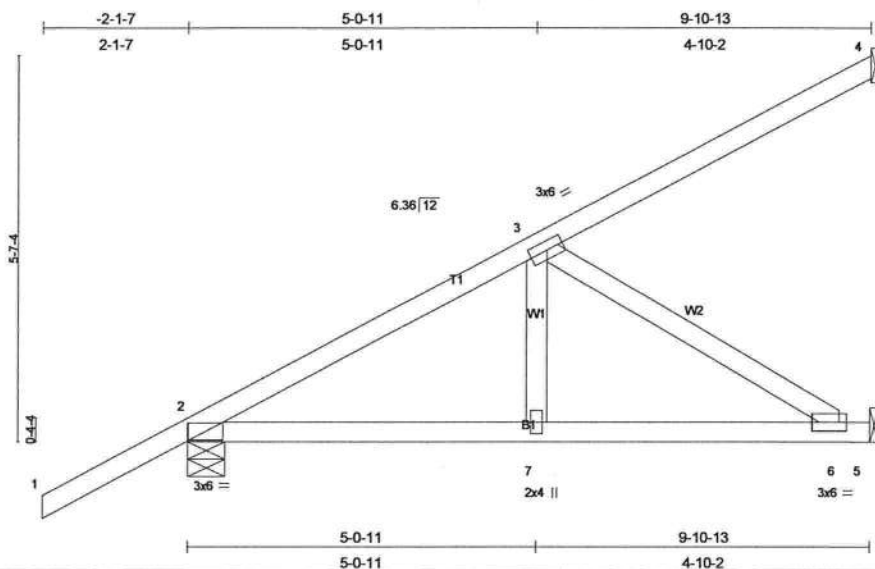
Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-5(F=24, B=24)-to-3=-45(F=5, B=5), 2=0(F=5, B=5)-to-4=-8(F=1, B=1)

Job L273732	Truss HJ9	Truss Type MONO TRUSS	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732016 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2008 MITek Industries, Inc. Wed Jul 23 15:30:24 2008 Page 1		



<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b> GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.45	Vert(LL) 0.03 6-7 >999 360	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.21	Vert(TL) -0.06 6-7 >999 240	
BCLL 10.0	Rep Stress Incr NO	WB 0.22	Horz(TL) 0.01 5 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 46 lb

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.1D  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS (lb/size) 4=233/Mechanical, 2=409/0-6-7, 5=262/Mechanical

Max Horz 2=390(load case 5)  
Max Uplift 4=257(load case 5), 2=-158(load case 5), 5=-140(load case 5)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-460/0, 3-4=-156/80  
BOT CHORD 2-7=-283/382, 6-7=-283/382, 5-6=0/0  
WEBS 3-7=0/197, 3-6=-450/333

#### JOINT STRESS INDEX

2 = 0.67, 3 = 0.17, 6 = 0.13 and 7 = 0.14

#### NOTES (6)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 lb uplift at joint 4, 158 lb uplift at joint 2 and 140 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

#### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:26 2008 Page 1

Job <b>L273732</b>	Truss <b>T01</b>	Truss Type <b>HIP</b>	Qty <b>1</b>	Ply <b>1</b>	<b>ADAMS FRAMING - ROCCO</b> <b>L273732020</b> Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:28 2008 Page 1		

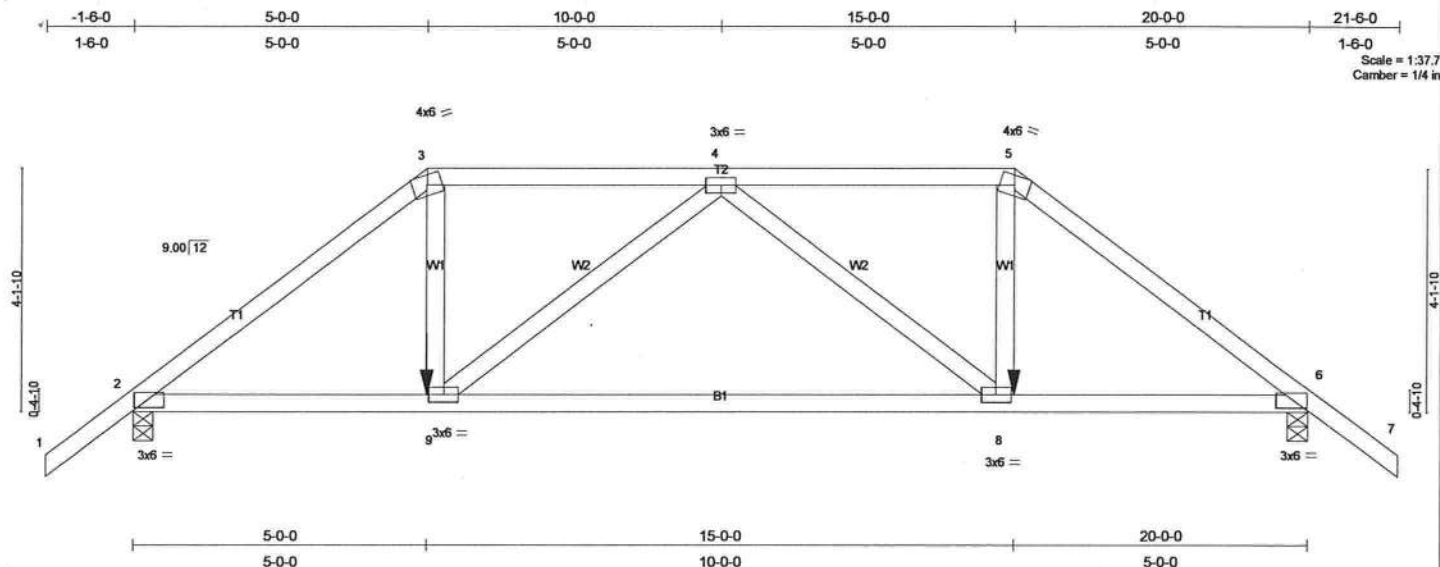


Plate Offsets (X,Y): [2:0-6-3,0-0-10], [6:0-6-3,0-0-10]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.33	Vert(LL) -0.17 8-9 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.61	Vert(TL) -0.41 8-9 >570 240		
BCLL 10.0 *	Rep Stress Incr NO	WB 0.29	Horz(TL) 0.04 6 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			
				Weight: 98 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-11-11 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 7-4-7 oc bracing.

#### REACTIONS

(lb/size) 2=1118/0-4-0, 6=1118/0-4-0  
Max Horz 2=-104(load case 3)  
Max Uplift 2=-457(load case 5), 6=-457(load case 6)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/49, 2-3=-1534/651, 3-4=-1166/566, 4-5=-1166/566, 5-6=-1534/651, 6-7=0/49  
BOT CHORD 2-9=-541/1144, 8-9=-723/1459, 6-8=-452/1144  
WEBS 3-9=-246/576, 4-9=-429/334, 4-8=-429/334, 5-8=-246/576

#### JOINT STRESS INDEX

2 = 0.71, 3 = 0.50, 4 = 0.35, 5 = 0.50, 6 = 0.71, 8 = 0.38 and 9 = 0.38

#### NOTES (10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 2 and 457 lb uplift at joint 6.
- Girder carries hip end with 5-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 187 lb down and 130 lb up at 15-0-0, and 187 lb down and 130 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

#### LOAD CASE(S) Standard

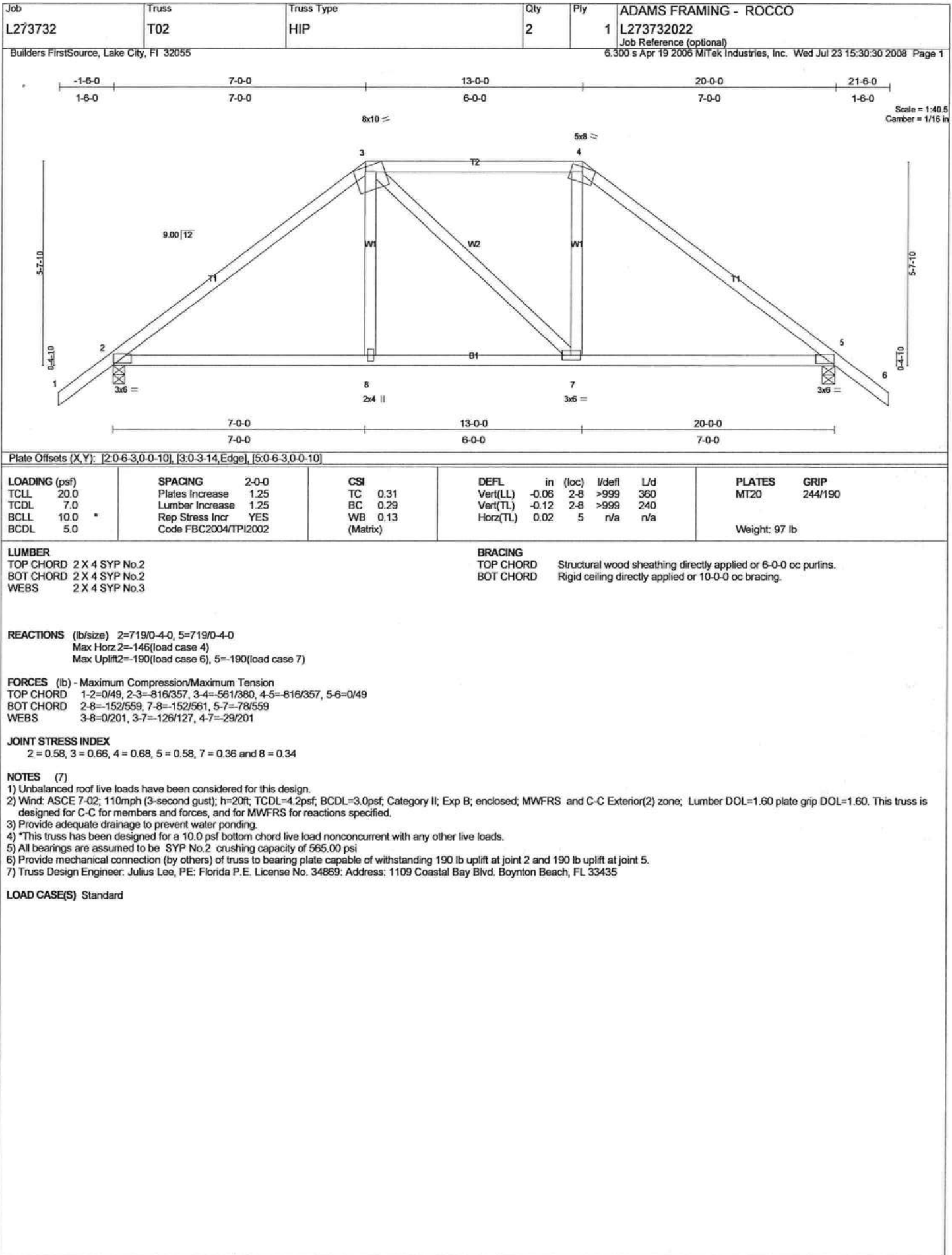
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

#### Uniform Loads (plf)

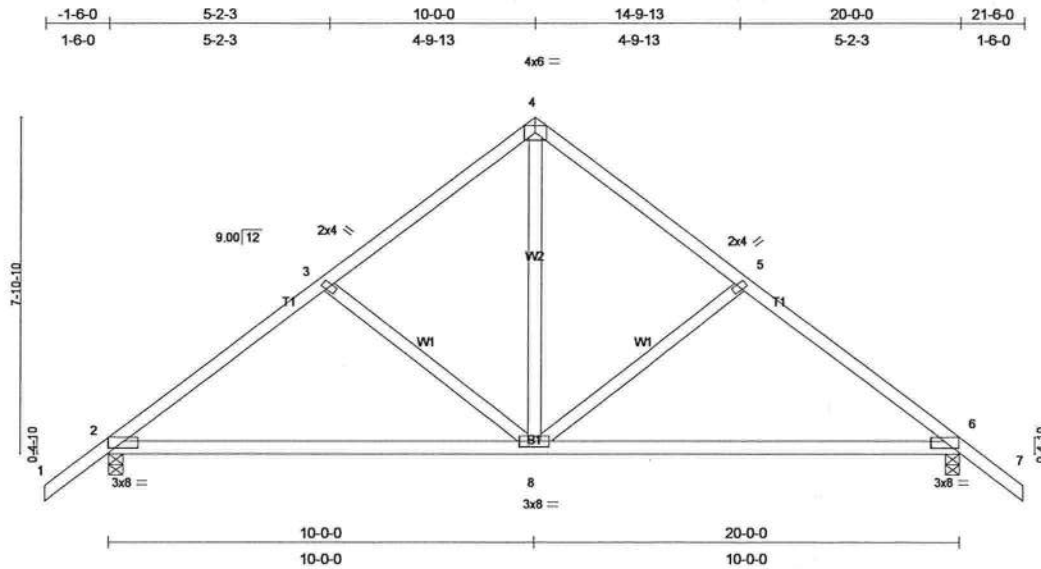
Vert: 1-3=-54, 3-5=-90(F=-36), 5-7=-54, 2-9=-10, 8-9=-17(F=-7), 6-8=-10

#### Concentrated Loads (lb)

Vert: 9=-187(F) 8=-187(F)



Job L273732	Truss T04	Truss Type COMMON	Qty 2	Ply 1	ADAMS FRAMING - ROCCO L273732024 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:31 2008 Page 1		



Scale = 1/52.0  
Camber = 1/8 in

Plate Offsets (X,Y): [2-0-8-3,0-1-6], [6-0-8-3,0-1-6]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL) -0.15	6-8 >999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL) -0.27	6-8 >870 240
BCLL 10.0	Rep Stress Incr	YES	WB 0.21	Horz(TL) 0.02	6 n/a n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)		
					PLATES GRIP
					MT20 244/190
					Weight: 102 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=719/0-4-0, 6=719/0-4-0
Max Horz 2=-208(load case 4)
Max Uplift 2=-202(load case 6), 6=-202(load case 7)

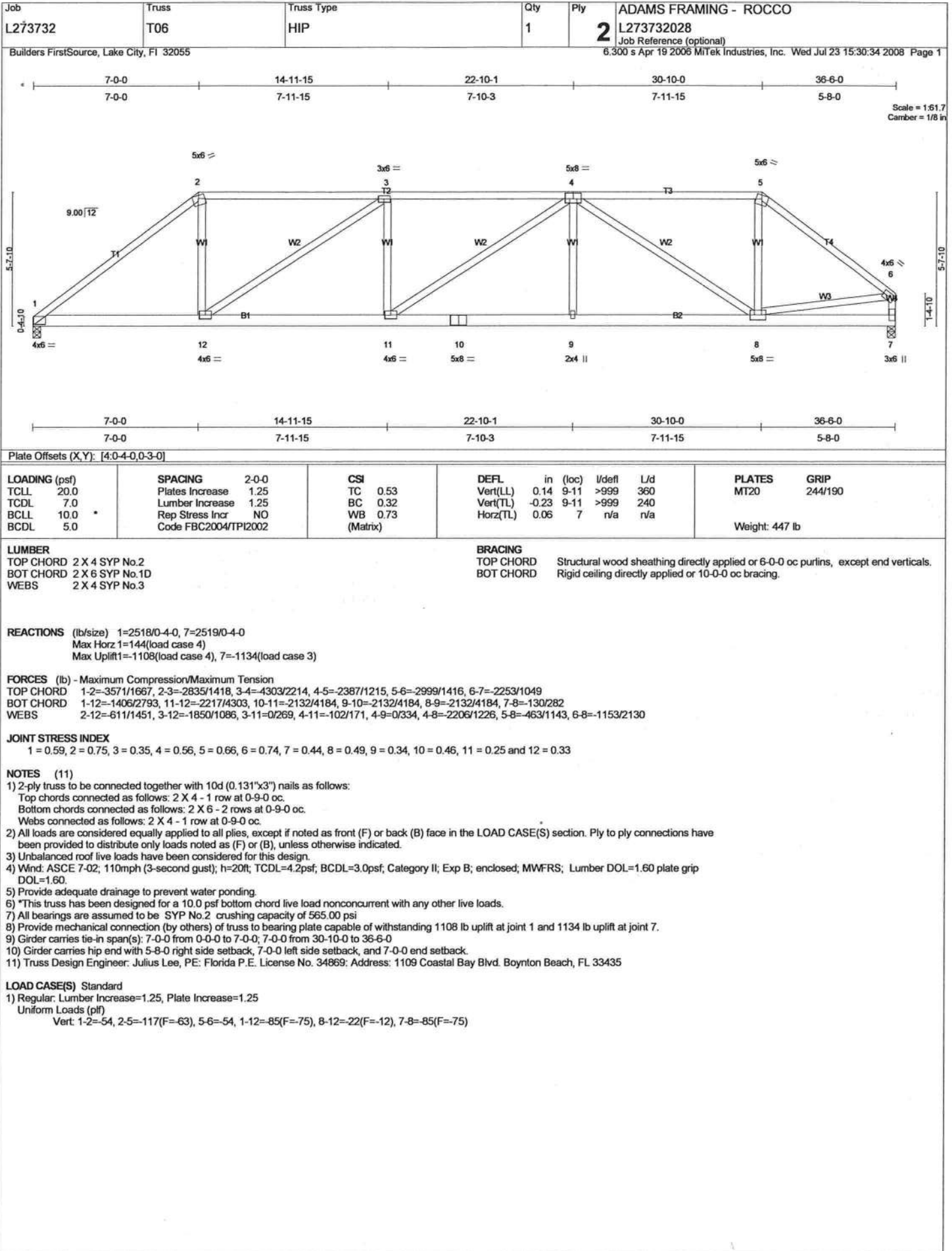
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-833/372, 3-4=-615/340, 4-5=-615/340, 5-6=-833/372, 6-7=0/49
BOT CHORD 2-8=-132/597, 6-8=-124/597
WEBS 3-8=-232/228, 4-8=-215/430, 5-8=-232/228

JOINT STRESS INDEX
2 = 0.72, 3 = 0.34, 4 = 0.42, 5 = 0.34, 6 = 0.72 and 8 = 0.57

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 2 and 202 lb uplift at joint 6.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
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Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:33 2008 Page 1



Job <b>L273732</b>	Truss <b>T08</b>	Truss Type <b>HIP</b>	Qty <b>1</b>	Ply <b>1</b>	<b>ADAMS FRAMING - ROCCO</b> <b>L273732030</b> Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:36 2008 Page 1		

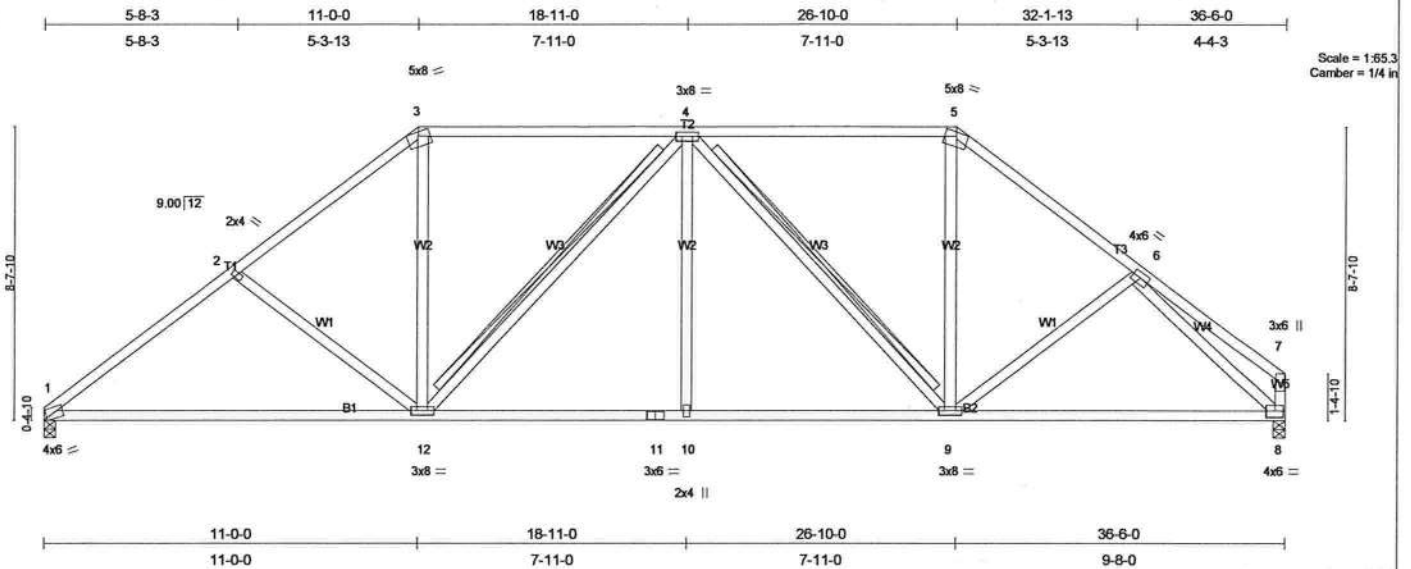


Plate Offsets (X,Y): [1:0-1-2,Edge]					
<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	2-0-0	TC 0.75	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.68	Vert(LL) -0.30 1-12 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.74	Vert(TL) -0.56 1-12 >773 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.07 8 n/a n/a		
	Code FBC2004/TPI2002				
				Weight: 215 lb	

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-5-4 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	Rigid ceiling directly applied or 7-10-12 oc bracing.
WEBS 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 4-12, 4-9
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.

**REACTIONS** (lb/size) 1=1158/0-4-0, 8=1158/0-4-0  
Max Horz 1=230(load case 5)  
Max Uplift 1=-243(load case 5), 8=-240(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-1665/830, 2-3=-1445/790, 3-4=-1098/716, 4-5=-1016/677, 5-6=-1335/736, 6-7=-319/126, 7-8=-242/135  
BOT CHORD 1-12=-605/1267, 11-12=-510/1294, 10-11=-510/1294, 9-10=-510/1294, 8-9=-466/999  
WEBS 2-12=-226/257, 3-12=-217/489, 4-12=-390/233, 4-10=0/196, 4-9=-488/248, 5-9=-178/425, 6-9=-110/144, 6-8=-1201/626

**JOINT STRESS INDEX**  
1 = 0.81, 2 = 0.34, 3 = 0.63, 4 = 0.57, 5 = 0.68, 6 = 0.35, 7 = 0.32, 8 = 0.71, 9 = 0.57, 10 = 0.34, 11 = 0.43 and 12 = 0.57

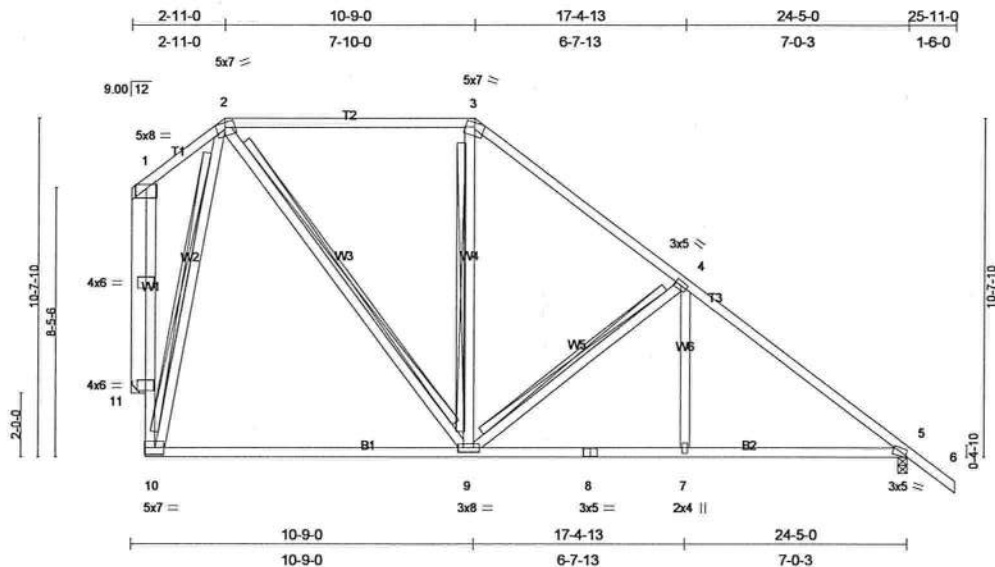
**NOTES** (7)  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
3) Provide adequate drainage to prevent water ponding.  
4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 1 and 240 lb uplift at joint 8.  
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

Job L273732	Truss T10	Truss Type HIP	Qty 5	Ply 1	ADAMS FRAMING - ROCCO L273732032 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 Mittek Industries, Inc. Wed Jul 23 15:30:38 2008 Page 1



Scale = 1/8 in  
Camber = 1/8 in

Plate Offsets (X,Y): [5:0-0-13,Edge]

<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	2-0-0	TC 0.74	in (loc) l/defl L/d	MT20	244/190
TCCL 7.0	Plates Increase 1.25	BC 0.49	Vert(LL) -0.19 9-10 >999 360		
BCCL 10.0	Lumber Increase 1.25	WB 0.46	Vert(TL) -0.35 9-10 >815 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.11 5 n/a n/a		
	Code FBC2004/TP12002			Weight: 178 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3 \*Except\*  
W3 2 X 4 SYP No.2, W1 2 X 4 SYP No.2  
OTHERS 2 X 6 SYP No.1D

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 5-9-11 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 2-3.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
WEBS T-Brace: 2 X 4 SYP No.3 - 2-9, 3-9, 4-9, 2-10  
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.  
Brace must cover 90% of web length.

#### REACTIONS

(lb/size) 5=849/0-3-8, 11=762/Mechanical  
Max Horz 11=-348(load case 7)  
Max Uplift 5=-251(load case 7), 11=-212(load case 7)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-47/154, 2-3=-448/400, 3-4=-666/383, 4-5=-1034/408, 5-6=0/48, 10-11=-276/782, 1-11=-63/120  
BOT CHORD 9-10=-61/282, 8-9=-125/733, 7-8=-125/733, 5-7=-125/733  
WEBS 2-9=-270/472, 3-9=-70/133, 4-9=-361/312, 4-7=0/199, 2-10=-750/330

#### JOINT STRESS INDEX

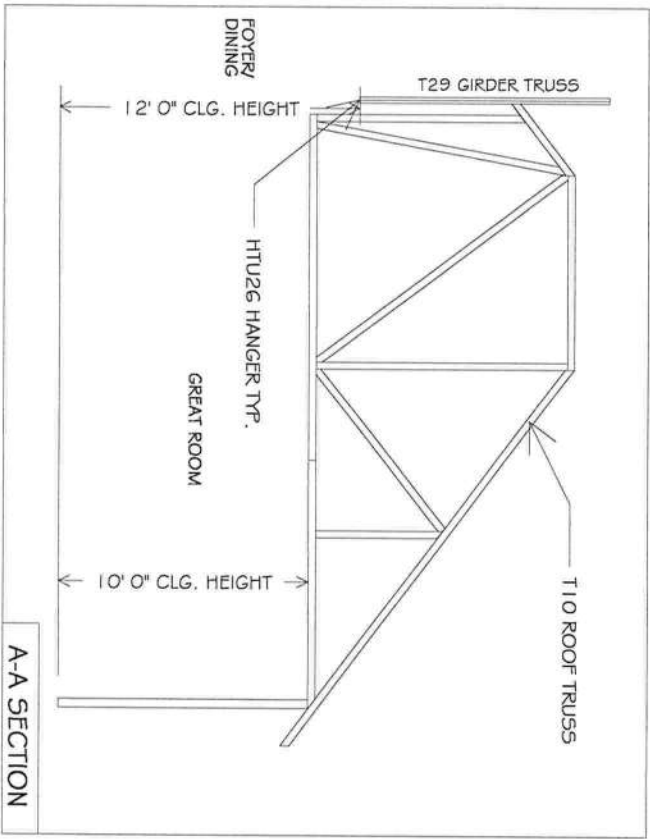
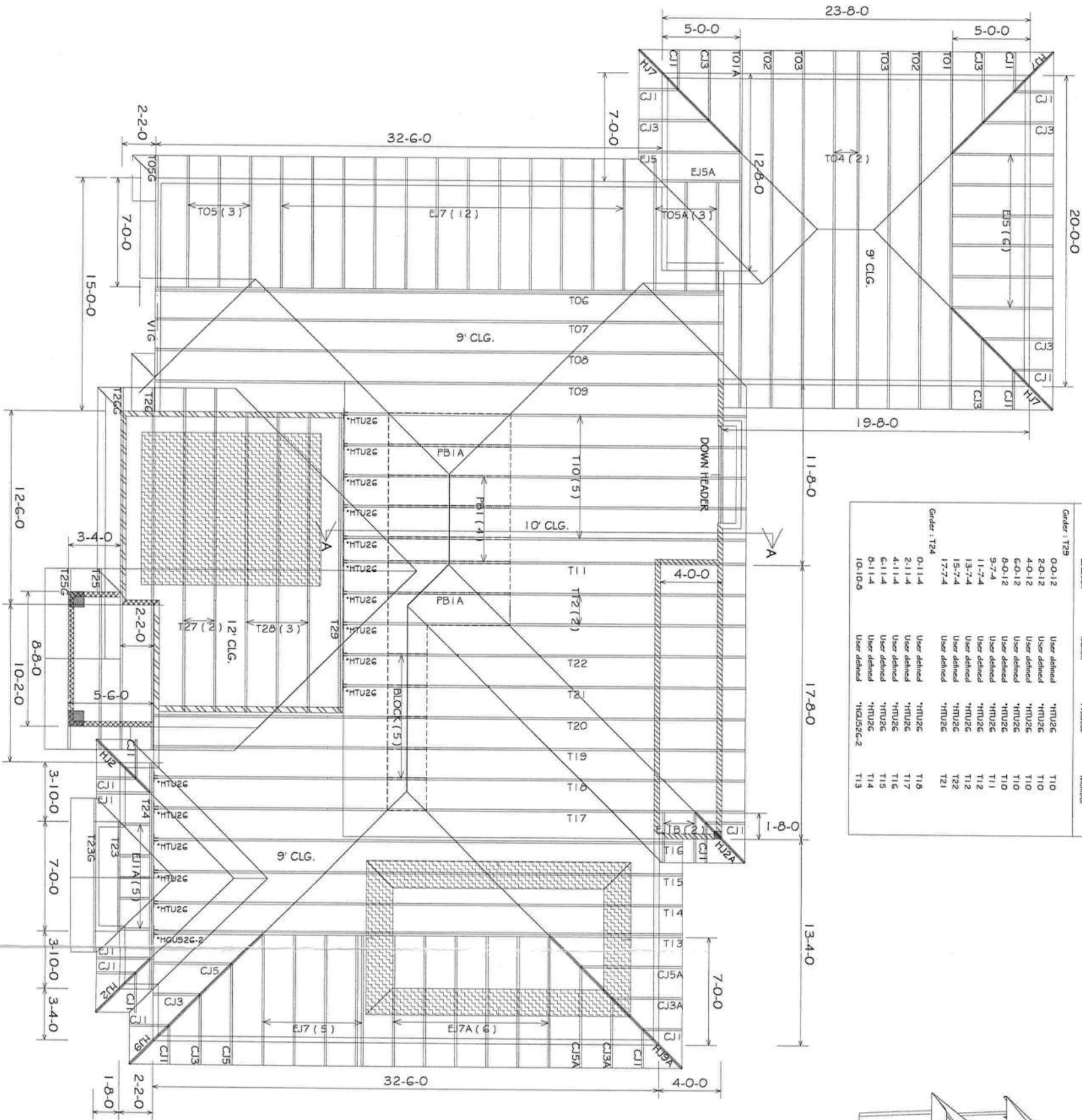
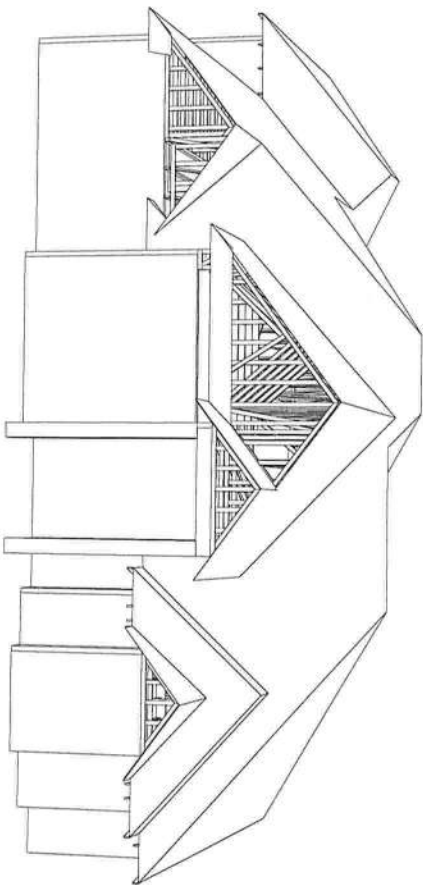
1 = 0.29, 2 = 0.69, 3 = 0.74, 4 = 0.52, 5 = 0.71, 7 = 0.34, 8 = 0.24, 9 = 0.58, 10 = 0.69, 11 = 0.00, 11 = 0.32 and 11 = 0.32

#### NOTES (8-9)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft, TCCL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 5 and 212 lb uplift at joint 11.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

ROOF/LOOK TRUSS HANGER LIST				
Truss	Location	Manuf.	Product	Member
Gorder 1 T29	9-0-12	User defined	*HTU26	T10
	2-0-12	User defined	*HTU26	T10
	4-0-12	User defined	*HTU26	T10
	6-0-12	User defined	*HTU26	T10
	8-0-12	User defined	*HTU26	T10
	9-7-4	User defined	*HTU26	T11
	11-7-4	User defined	*HTU26	T12
	13-7-4	User defined	*HTU26	T22
	15-7-4	User defined	*HTU26	T21
	17-7-4	User defined	*HTU26	T21
Gorder 1 T24	0-11-4	User defined	*HTU26	T18
	2-11-4	User defined	*HTU26	T17
	4-11-4	User defined	*HTU26	T16
	6-11-4	User defined	*HTU26	T15
	8-11-4	User defined	*HTU26	T14
	10-10-8	User defined	*HTU26-2	T13



BEARING HEIGHT SCHEDULE	
	9' 0"
	10' 0"
	12' 0"
	14' 0"
INDICATES TRAYBOX CLG.	

NOTES:

- 1) REFER TO HD 91 (RECOMMENDATIONS FOR HANGING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL W01 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2.4G MAXIMUM SPACING. UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACE/MENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEAM UP.
- 7) ALL ROOF TRUSS HANGERS TO BE SAMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SAMPSON THA422 UNLESS OTHERWISE NOTED.
- 8) BEAMS/JOISTS/INTEL (EOK) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR IDENTIFICATION OF TRUSSES AND JOISTS. ALL REVISIONS ARCHITECTURAL OR OTHER TRUSS LAYOUTS, REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE ADAMANT CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Typical beam size: \_\_\_\_\_

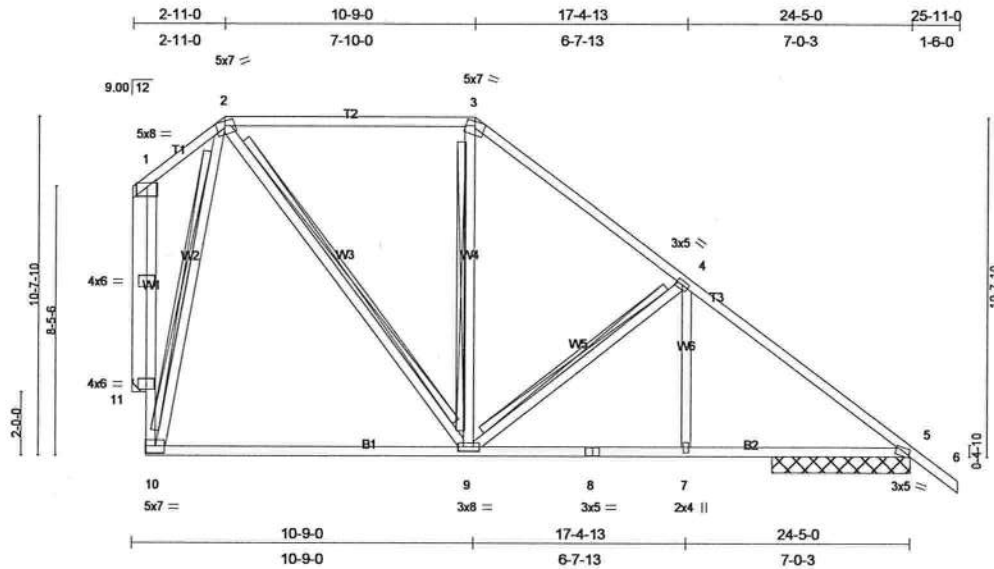
Approved by: \_\_\_\_\_ Date: \_\_\_\_\_



TRUSS INFORMATION:  
ROOF PITCH: 9/12  
CEILING: TRAYS @ MASTER & DINING  
OVERHANG: 1' 6"  
HANGER LIST:  
15 - HTU26  
1 - HGUS26-2 (CONNECTS T13 TO T24)  
VALLEY:  
BY OTHERS  
NOTE: ALL HEADERS AND BEAMS BY OTHERS  
ADDITIONAL BEARING WALLS @ DINING/FOYER &  
AROUND GARAGE.

ADAMS FRAMING	
LAKE CITY, FL	
SCALE: NTS	
DATE: 04/10/03	JOB #: 1273732

Job L273732	Truss T11	Truss Type HIP	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732033 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:39 2008 Page 1		



Scale = 1/8" = 1'-0"  
Camber = 1/8" in

Plate Offsets (X,Y): [5:0-0-13,Edge]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.74	Vert(LL) -0.20 9-10 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.49	Vert(TL) -0.35 9-10 >816 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.47	Horz(TL) 0.11 5 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
Weight: 178 lb					

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3 \*Except\*  
W3 2 X 4 SYP No.2, W1 2 X 4 SYP No.2  
OTHERS 2 X 6 SYP No.10

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 5-9-2 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 2-3.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
WEBS T-Brace: 2 X 4 SYP No.3 - 2-9, 3-9, 4-9, 2-10  
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.  
Brace must cover 90% of web length.

**REACTIONS** (lb/size) 5=846/4-4-0, 11=767/Mechanical  
Max Horz 11=-348(load case 7)  
Max Uplift5=-246(load case 7), 11=-213(load case 7)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-47/154, 2-3=-453/402, 3-4=-673/387, 4-5=-1051/417, 5-6=0/47, 10-11=-279/787, 1-11=-63/120  
BOT CHORD 9-10=60/281, 8-9=-135/752, 7-8=-135/752, 5-7=-135/752  
WEBS 2-9=-273/478, 3-9=-65/136, 4-9=-379/321, 4-7=0/202, 2-10=-755/332

#### JOINT STRESS INDEX

1 = 0.29, 2 = 0.69, 3 = 0.73, 4 = 0.52, 5 = 0.79, 7 = 0.34, 8 = 0.25, 9 = 0.58, 10 = 0.69, 11 = 0.00, 11 = 0.32 and 11 = 0.32

#### NOTES (8-9)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 5 and 213 lb uplift at joint 11.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

**LOAD CASE(S)** Standard

Job <b>L273732</b>	Truss <b>T09</b>	Truss Type <b>SPECIAL</b>	Qty <b>1</b>	Ply <b>1</b>	ADAMS FRAMING - ROCCO <b>L273732031</b> Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:37 2008 Page 1

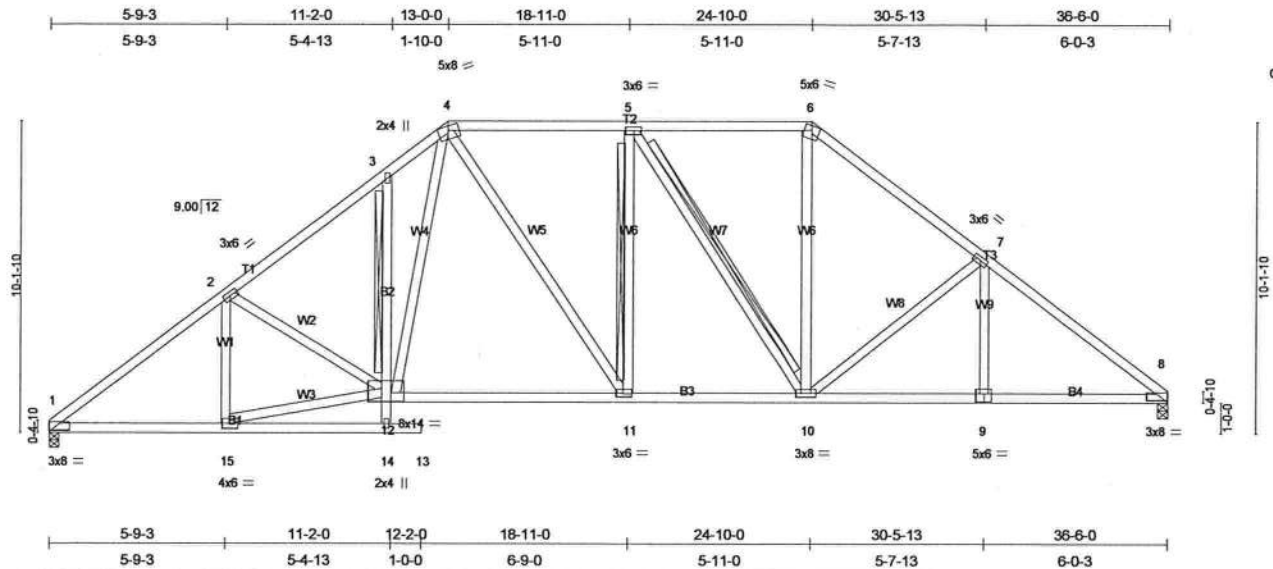


Plate Offsets (X,Y): [1:0-8-3,0-0-10], [8:0-8-3,0-0-14], [9:0-3-0,0-3-0], [12:0-5-0,0-3-5]

LOADING (psf)	SPACING	2:0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.11	11-12	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.24	11-12	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.47	Horz(TL)	0.09	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 240 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2 \*Except\*  
B2 2 X 4 SYP No.3  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-2 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:  
T-Braces: 2 X 4 SYP No.3 - 3-12  
T-Braces: 2 X 4 SYP No.3 - 5-11, 5-10  
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.  
Brace must cover 90% of web length.

#### REACTIONS

(lb/size) 1=1165/0-4-0, 8=1161/0-4-0  
Max Horz 1=271(load case 5)  
Max Uplift1=210(load case 6), 8=203(load case 7)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1737/804, 2-3=-1624/839, 3-4=-1543/955, 4-5=-1213/775, 5-6=-1057/702, 6-7=-1405/778, 7-8=-1726/810  
BOT CHORD 1-15=-577/1303, 14-15=-40/33, 13-14=0/0, 12-14=0/111, 3-12=-153/184, 11-12=-337/1103, 10-11=-390/1213, 9-10=-534/1290, 8-9=-534/1290  
WEBS 2-15=-174/119, 12-15=-551/1293, 2-12=-114/187, 4-12=-347/533, 4-11=-193/306, 5-11=-178/203, 5-10=-388/211, 6-10=-245/478, 7-10=-312/280, 7-9=0/194

#### JOINT STRESS INDEX

1 = 0.87, 2 = 0.44, 3 = 0.43, 4 = 0.49, 5 = 0.43, 6 = 0.58, 7 = 0.44, 8 = 0.73, 9 = 0.47, 10 = 0.59, 11 = 0.43, 12 = 0.27, 14 = 0.64 and 15 = 0.59

#### NOTES (7)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1 and 203 lb uplift at joint 8.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job <b>L273732</b>	Truss <b>T07</b>	Truss Type <b>HIP</b>	Qty <b>1</b>	Ply <b>1</b>	ADAMS FRAMING - ROCCO <b>L273732029</b> Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:35 2008 Page 1		

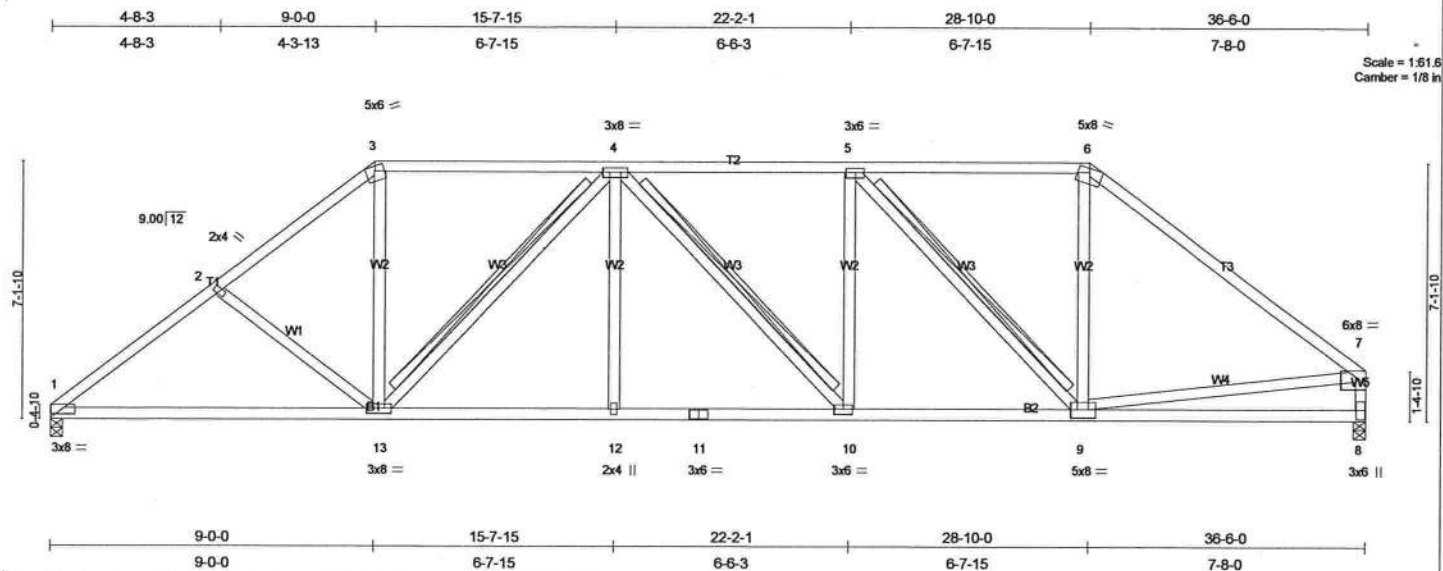


Plate Offsets (X,Y): [1:0-8-3,0-1-6], [7:0-3-8,Edge]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	-0.14 1-13 >999	360	MT20 244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.29 1-13 >999	240	
BCCL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.07 8 n/a	n/a	
BCDL	5.0	Code FBC2004/TP12002		(Matrix)					Weight: 215 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-7-3 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	Rigid ceiling directly applied or 7-9-2 oc bracing.
WEBS 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 4-13, 4-10, 5-9
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.

**REACTIONS** (lb/size) 1=1158/0-4-0, 8=1158/0-4-0  
Max Horz 1=188(load case 5)  
Max Uplift 1=287(load case 5), 8=287(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-1692/848, 2-3=-1518/819, 3-4=-1175/727, 4-5=-1490/890, 5-6=-1061/681, 6-7=-1439/715, 7-8=-1113/585  
BOT CHORD 1-13=-634/1289, 12-13=-669/1532, 11-12=-669/1532, 10-11=-669/1532, 9-10=-643/1490, 8-9=-226/273  
WEBS 2-13=-161/199, 3-13=-276/565, 4-13=-588/278, 4-12=0/177, 4-10=-96/66, 5-10=-10/222, 5-9=-695/307, 6-9=-139/447, 7-9=-333/820

**JOINT STRESS INDEX**  
1 = 0.73, 2 = 0.34, 3 = 0.55, 4 = 0.57, 5 = 0.38, 6 = 0.73, 7 = 0.71, 8 = 0.47, 9 = 0.37, 10 = 0.38, 11 = 0.55, 12 = 0.34 and 13 = 0.57

**NOTES** (7)  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
3) Provide adequate drainage to prevent water ponding.  
4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 1 and 287 lb uplift at joint 8.  
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

6,300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:33 2008 Page 1

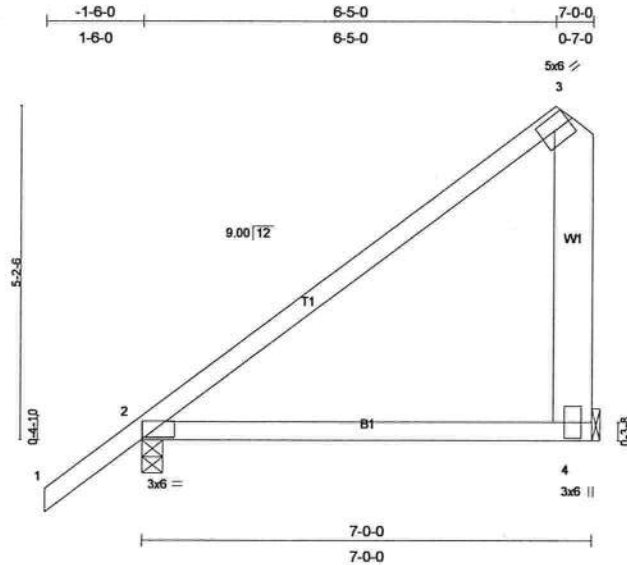


Weight: 51 lb

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

1) Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (plf)  
Vert: 1-7=-64(F=-10), 7-8=-64(F=-10), 2-9=-10

Job L273732	Truss T05	Truss Type COMMON	Qty 3	Ply 1	ADAMS FRAMING - ROCCO L273732025 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6:300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:32 2008 Page 1		



Scale = 1/32.5

Plate Offsets (X,Y): [2:0-6,3:0-0-6], [3:Edge,0-2-8]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) -0.03 2-4 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.19	Vert(TL) -0.06 2-4 >999 240		
BCLL 10.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 4 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 40 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 8 SYP No.1D

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(lb/size) 2=310/0-4-0, 4=191/Mechanical  
Max Horz 2=214(load case 6)  
Max Uplift 2=80(load case 6), 4=103(load case 6)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/49, 2-3=-175/37, 3-4=-140/189  
BOT CHORD 2-4=-57/60

#### JOINT STRESS INDEX

2 = 0.57, 3 = 0.59 and 4 = 0.40

#### NOTES (5)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 2 and 103 lb uplift at joint 4.
- 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L273732	Truss T03	Truss Type HIP	Qty 2	Ply 1	ADAMS FRAMING - ROCCO L273732023 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:31 2008 Page 1		

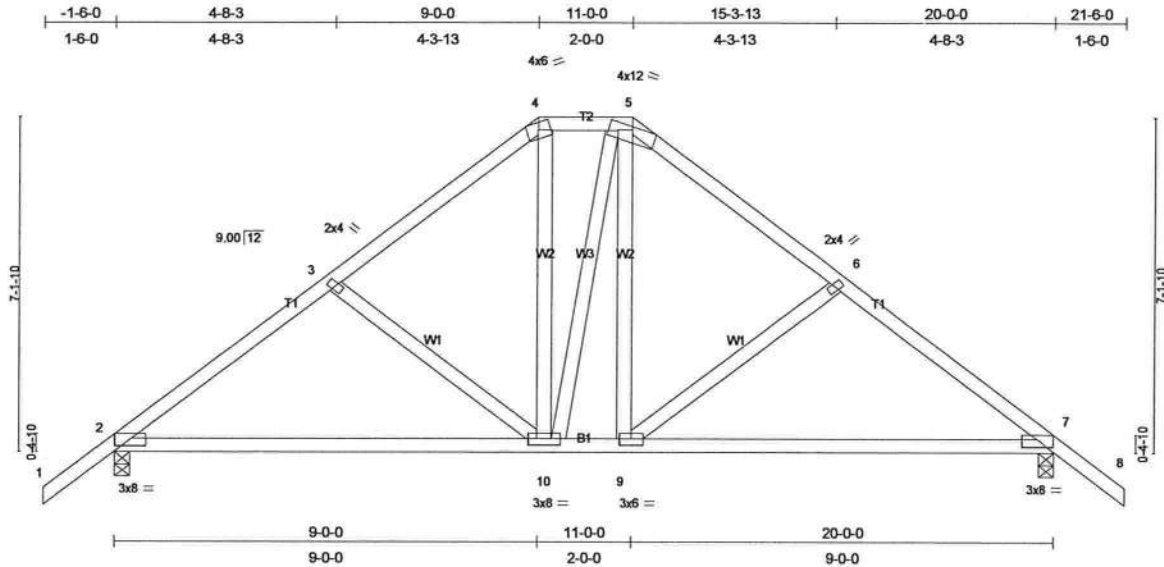


Plate Offsets (X,Y): [2-0-8-3,0-1-6], [7-0-8-3,0-1-6]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.21	Vert(LL)	-0.13	7-9	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.40	Vert(TL)	-0.24	7-9	>987	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.12	Horz(TL)	0.02	7	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 118 lb	

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=719/0-4-0, 7=719/0-4-0  
Max Horz 2=188(load case 5)  
Max Uplift 2=199(load case 6), 7=199(load case 7)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/49, 2-3=843/384, 3-4=644/352, 4-5=456/345, 5-6=643/352, 6-7=842/384, 7-8=0/49  
BOT CHORD 2-10=-157/610, 9-10=-26/453, 7-9=-141/610  
WEBS 3-10=-201/208, 4-10=-88/208, 5-10=-123/123, 5-9=-91/213, 6-9=-203/210

**JOINT STRESS INDEX**  
2 = 0.71, 3 = 0.34, 4 = 0.31, 5 = 0.60, 6 = 0.34, 7 = 0.72, 9 = 0.35 and 10 = 0.60

#### NOTES (7)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 2 and 199 lb uplift at joint 7.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

Job <b>L273732</b>	Truss <b>T01A</b>	Truss Type <b>HIP</b>	Qty <b>1</b>	Ply <b>1</b>	ADAMS FRAMING - ROCCO <b>L273732021</b> Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:29 2008 Page 1		

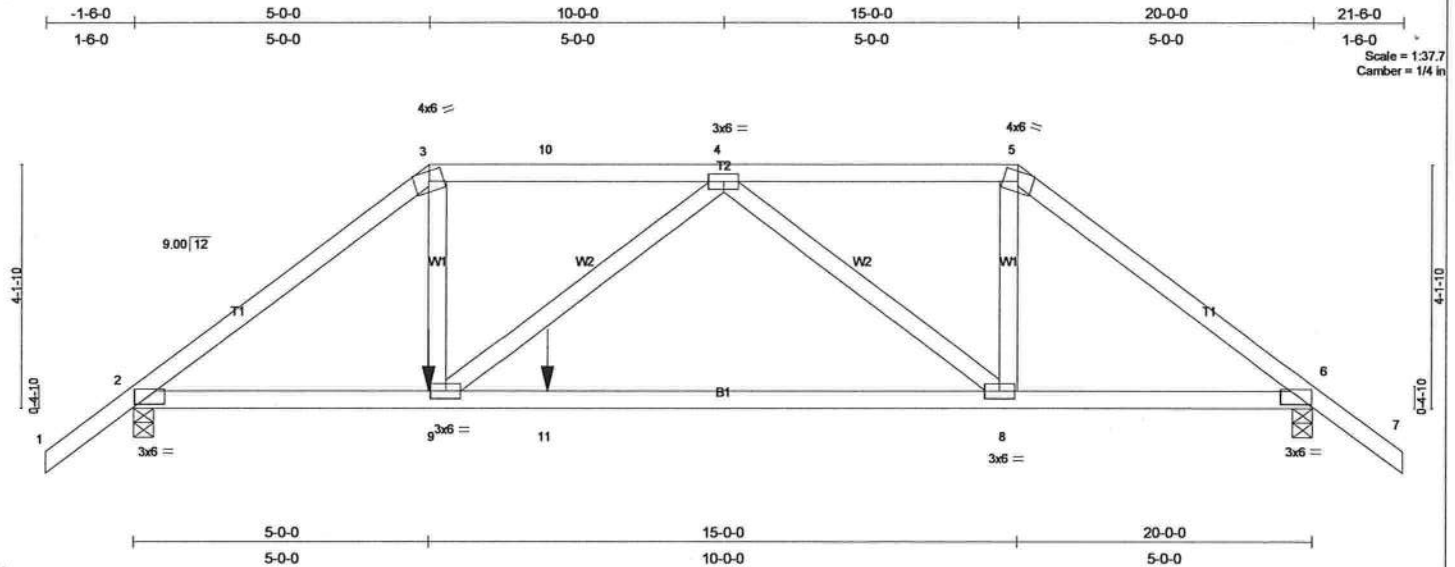


Plate Offsets (X,Y): [2-0-6-3,0-0-10], [6-0-6-3,0-0-10]					
<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	2-0-0	TC 0.22	in (loc) l/defl L/d	MT20	244/190
TCCL 7.0	Plates Increase 1.25	BC 0.85	Vert(LL) -0.23 8-9 >999 360		
BCCL 10.0 *	Lumber Increase 1.25	WB 0.32	Vert(TL) -0.49 8-9 >482 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.04 6 n/a n/a		
	Code FBC2004/TP12002			Weight: 98 lb	

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-11-14 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 9-3-0 oc bracing.
WEBS 2 X 4 SYP No.3	

**REACTIONS** (lb/size) 2=1081/0-4-0, 6=879/0-4-0  
 Max Horz 2=105(load case 4)  
 Max Uplift 2=-364(load case 4), 6=-252(load case 6)

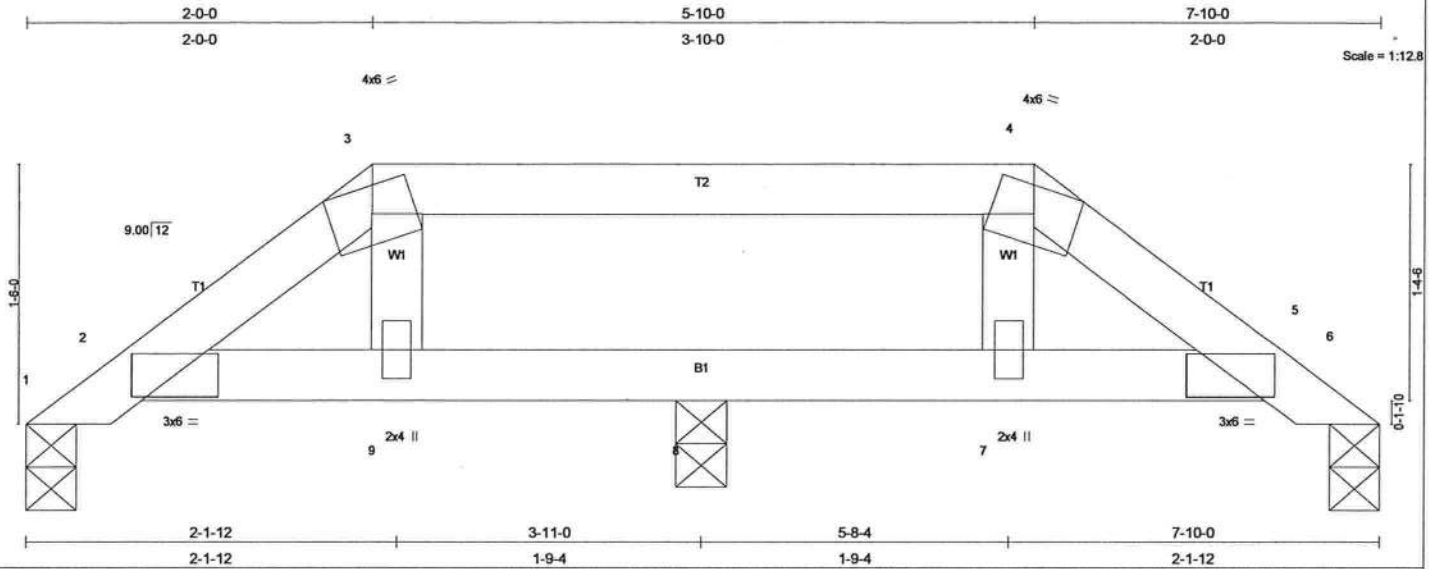
**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/49, 2-3=-1529/551, 3-10=-1156/481, 4-10=-1155/481, 4-5=-860/318, 5-6=-1159/348, 6-7=0/49  
 BOT CHORD 2-9=-458/1133, 9-11=-449/1186, 8-11=-449/1186, 6-8=-209/844  
 WEBS 3-9=-210/604, 4-9=-93/91, 4-8=-470/299, 5-8=-143/434

**JOINT STRESS INDEX**  
 2 = 0.73, 3 = 0.43, 4 = 0.35, 5 = 0.43, 6 = 0.73, 8 = 0.39 and 9 = 0.39

- NOTES** (10)
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
  - 3) Provide adequate drainage to prevent water ponding.
  - 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
  - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 364 lb uplift at joint 2 and 252 lb uplift at joint 6.
  - 7) Girder carries hip end with 13-0-0 right side setback, 5-0-0 left side setback, and 5-0-0 end setback.
  - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 187 lb down and 130 lb up at 5-0-0, and 250 lb down and 91 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
  - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
  - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard  
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25  
 Uniform Loads (plf)  
 Vert: 1-3=-54, 3-10=-90(F=-36), 5-10=-54, 5-7=-54, 2-9=-10, 9-11=-17(F=-7), 6-11=-10  
 Concentrated Loads (lb)  
 Vert: 9=-187(F) 11=-250(F)

Job L273732	Truss PB1A	Truss Type HIP PIGGYBACK	Qty 2	Ply 1	ADAMS FRAMING - ROCCO L273732019 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:27 2008 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	Vert(LL)	-0.01	9	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.16	Vert(TL)	-0.01	9	>999	240		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.02	Horz(TL)	0.01	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 25 lb	

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=176/0-3-8, 6=176/0-3-8, 8=133/0-3-8  
Max Horz 1=-39(load case 4)  
Max Uplift 1=-44(load case 5), 6=-50(load case 4), 8=-24(load case 5)

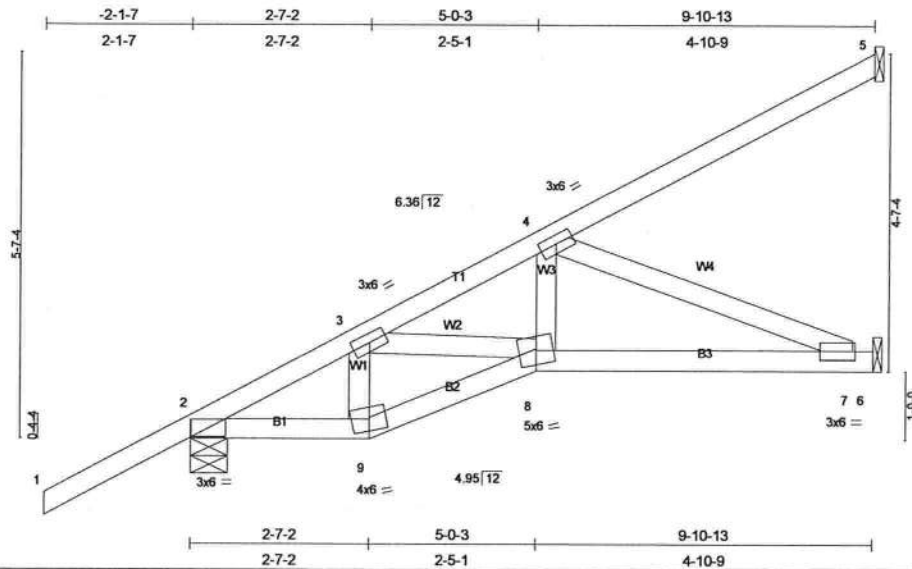
**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-97/78, 2-3=-250/207, 3-4=-213/216, 4-5=-250/207, 5-6=-97/78  
BOT CHORD 2-9=-155/220, 8-9=-145/213, 7-8=-145/213, 5-7=-155/220  
WEBS 3-9=-54/68, 4-7=-54/68

**JOINT STRESS INDEX**  
2 = 0.21, 3 = 0.28, 4 = 0.28, 5 = 0.21, 7 = 0.04 and 9 = 0.04

**NOTES** (9)  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
3) Provide adequate drainage to prevent water ponding.  
4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
6) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.  
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 50 lb uplift at joint 6 and 24 lb uplift at joint 8.  
8) SEE MiTEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS  
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

Job L273732	Truss HJ9A	Truss Type SPECIAL	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732017 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:25 2008 Page 1		



<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.02 7-8 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.27	Vert(TL) -0.06 7-8 >999 240		
BCLL 10.0	Rep Stress Incr NO	WB 0.28	Horz(TL) 0.01 6 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
Weight: 50 lb					

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 9-2-5 oc bracing.
WEBS 2 X 4 SYP No.3	

**REACTIONS** (lb/size) 5=224/Mechanical, 2=409/0-6-7, 6=271/Mechanical  
Max Horz 2=387(load case 5)  
Max Uplift 5=245(load case 5), 2=158(load case 5), 6=152(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/53, 2-3=352/0, 3-4=688/174, 4-5=150/76  
BOT CHORD 2-9=162/268, 8-9=164/286, 7-8=469/604, 6-7=0/0  
WEBS 3-9=157/117, 3-8=321/369, 4-8=0/255, 4-7=651/505

**JOINT STRESS INDEX**  
2 = 0.57, 3 = 0.18, 4 = 0.22, 7 = 0.18, 8 = 0.48 and 9 = 0.20

**NOTES** (6)  
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.  
2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 5, 158 lb uplift at joint 2 and 152 lb uplift at joint 6.  
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).  
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard  
1) Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (plf)  
Vert: 1-2=-54  
Trapezoidal Loads (plf)  
Vert: 2=-4(F=25, B=25)-to-5=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-9=6(F=2, B=2), 9=6(F=2, B=2)-to-8=-12(F=-1, B=-1), 8=-12(F=-1, B=-1)-to-6=-25(F=-7, B=-7)

Job L273732	Truss HJ7	Truss Type MONO TRUSS	Qty 3	Ply 1	ADAMS FRAMING - ROCCO L273732015 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:24 2008 Page 1		

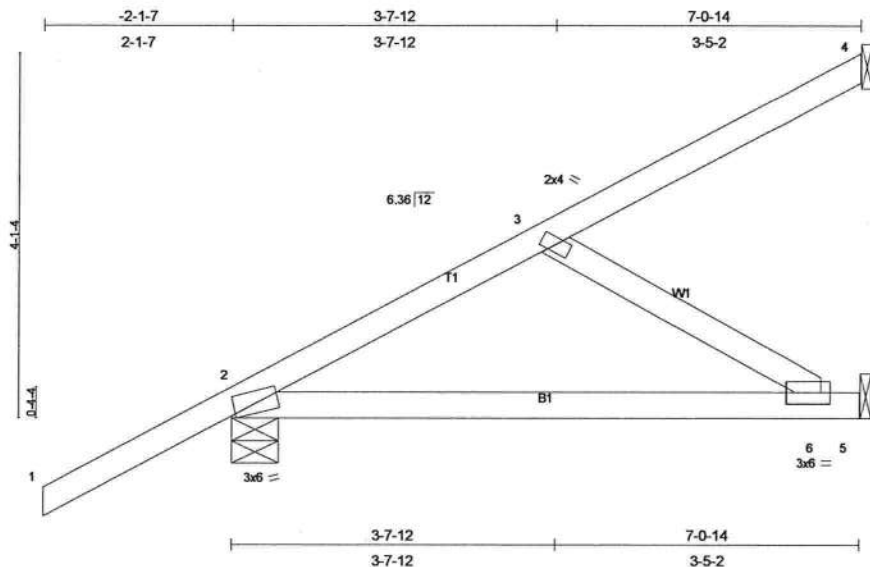


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL)	-0.07	2-6	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.27	Vert(TL)	-0.11	2-6	>714	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.05	Horz(TL)	0.00	5	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 31 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(lb/size) 4=125/Mechanical, 2=287/0-6-6, 5=110/Mechanical  
Max Horz 2=220(load case 5)  
Max Uplift 4=-135(load case 5), 2=-157(load case 5), 5=-33(load case 5)  
Max Grav 4=125(load case 1), 2=287(load case 1), 5=129(load case 2)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-188/0, 3-4=-81/43  
BOT CHORD 2-6=-116/147, 5-6=0/0  
WEBS 3-6=-172/136

#### JOINT STRESS INDEX

2 = 0.87, 3 = 0.08 and 6 = 0.05

#### NOTES (6)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 lb uplift at joint 4, 157 lb uplift at joint 2 and 33 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

#### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

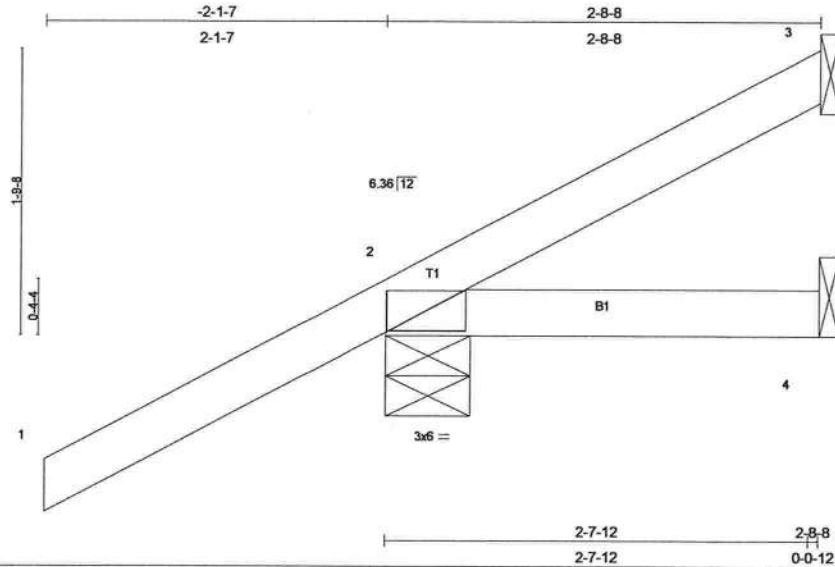
Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-95(F=-21, B=-21), 2=0(F=5, B=5)-to-5=-18(F=-4, B=-4)

Job L273732	Truss HJ2	Truss Type JACK	Qty 2	Ply 1	ADAMS FRAMING - ROCCO L273732013 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:22 2008 Page 1		



<b>LOADING (psf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.33	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.04	Vert(LL) -0.00 2-4 >999 360		
BCLL 10.0	Rep Stress Incr NO	WB 0.00	Vert(TL) -0.00 2-4 >999 240		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 12 lb	

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-8-8 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

<b>REACTIONS</b> (lb/size)	3=-28/Mechanical, 2=218/0-6-7, 4=7/Mechanical
Max Horz 2=90(load case 5)	
Max Uplift 3=-28(load case 1), 2=-211(load case 5)	
Max Grav 3=62(load case 3), 2=218(load case 1), 4=30(load case 2)	

<b>FORCES (lb)</b> - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-45/31
BOT CHORD 2-4=0/0

<b>JOINT STRESS INDEX</b>
2 = 0.10

<b>NOTES</b> (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 211 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

<b>LOAD CASE(S)</b> Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=4(F=25, B=25)-to-3=-45(F=5, B=5), 2=0(F=5, B=5)-to-4=-8(F=1, B=1)

Job L273732	Truss EJ7	Truss Type MONO TRUSS	Qty 17	Ply 1	ADAMS FRAMING - ROCCO L273732011 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:21 2008 Page 1		

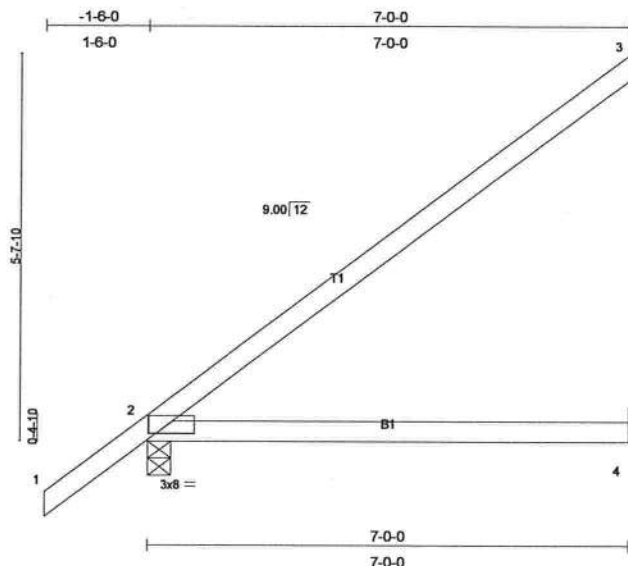


Plate Offsets (X,Y): [2-0-8-3-0-1-2]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	0.10	2-4	>786	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.29	Vert(TL)	-0.17	2-4	>474	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 27 lb										

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(lb/size) 3=156/Mechanical, 2=318/0-4-0, 4=49/Mechanical  
Max Horz 2=223(load case 6)  
Max Uplift 3=117(load case 6), 2=77(load case 6)  
Max Grav 3=156(load case 1), 2=318(load case 1), 4=95(load case 2)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/49, 2-3=155/73  
BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.72

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 3 and 77 lb uplift at joint 2.
- 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L273732	Truss EJ5	Truss Type MONO TRUSS	Qty 7	Ply 1	ADAMS FRAMING - ROCCO L273732009 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:20 2008 Page 1		

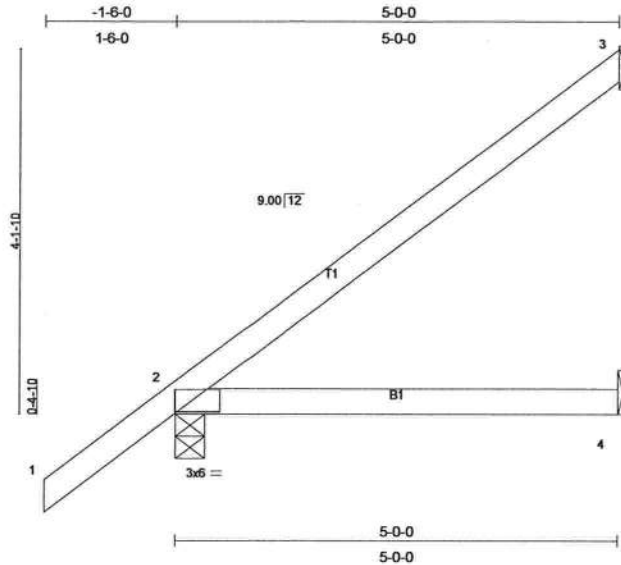


Plate Offsets (X,Y): [2-0-3-13,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 20 lb

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purtins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=113/Mechanical, 2=258/0-4-0, 4=24/Mechanical  
Max Horz 2=242(load case 6)  
Max Uplift 3=-134(load case 6), 2=-125(load case 6)  
Max Grav 3=113(load case 1), 2=258(load case 1), 4=72(load case 2)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=0/49, 2-3=-117/54  
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**  
2 = 0.18

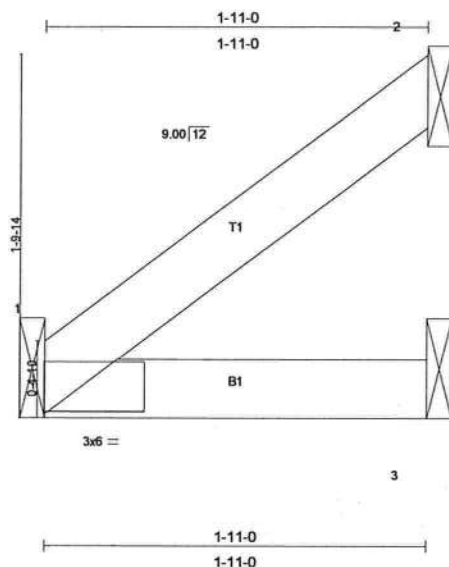
**NOTES** (5)  
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 125 lb uplift at joint 2.  
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

Job L273732	Truss EJ1A	Truss Type MONO TRUSS	Qty 5	Ply 1	ADAMS FRAMING - ROCCO L273732007 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:19 2008 Page 1



Scale = 1:11.1

Plate Offsets (X,Y): [1:0-3-13,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.05	Vert(LL)	-0.00	1	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.03	Vert(TL)	-0.00	1-3	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 7 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 1-11-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(lb/size) 1=61/Mechanical, 3=9/Mechanical, 2=51/Mechanical  
Max Horz 1=69(load case 6)  
Max Uplift 1=-3(load case 6), 2=-63(load case 6)  
Max Grav 1=61(load case 1), 3=28(load case 2), 2=51(load case 1)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-52/25  
BOT CHORD 1-3=0/0

#### JOINT STRESS INDEX

1 = 0.04

#### NOTES (5)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3 lb uplift at joint 1 and 63 lb uplift at joint 2.
- 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:17 2008 Page 1



LOAD CASE(S) Standard

Job L273732	Truss CJ3A	Truss Type SPECIAL	Qty 2	Ply 1	ADAMS FRAMING - ROCCO L273732003 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6:300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:16 2008 Page 1		

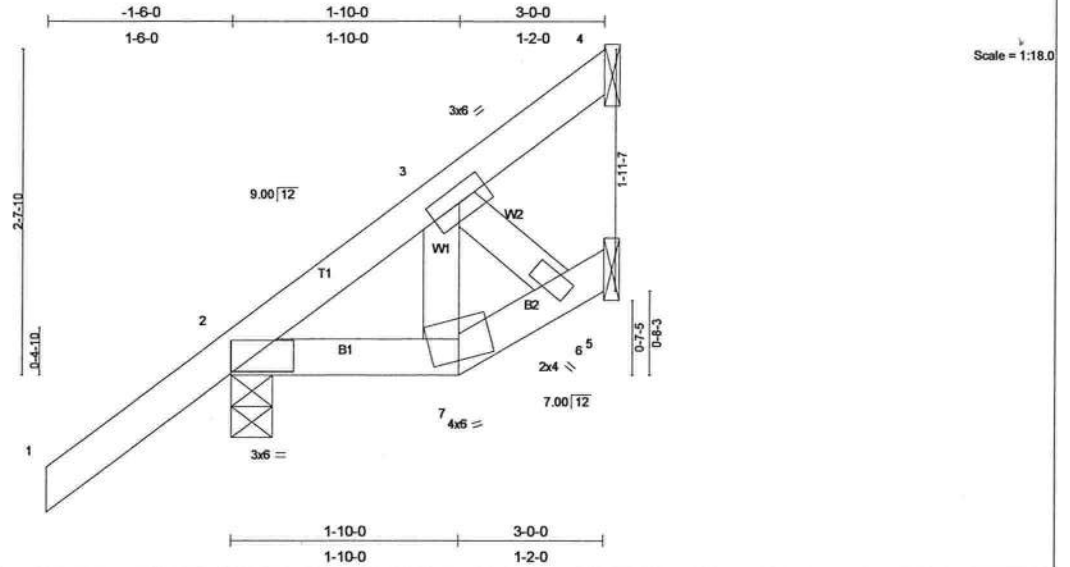


Plate Offsets (X,Y): [2-0-3-13,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.19	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.02	Vert(TL)	-0.00	2	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.01	Horz(TL)	-0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
Weight: 17 lb										

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS (lb/size) 4=41/Mechanical, 2=206/0-4-0, 5=21/Mechanical

Max Horz 2=173(load case 6)  
Max Uplift 4=51(load case 6), 2=-136(load case 6), 5=-14(load case 7)  
Max Grav 4=41(load case 1), 2=206(load case 1), 5=42(load case 2)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/49, 2-3=-80/0, 3-4=-41/20  
BOT CHORD 2-7=-18/30, 6-7=-18/31, 5-6=-8/21  
WEBS 3-7=-0/35, 3-6=-38/21

#### JOINT STRESS INDEX

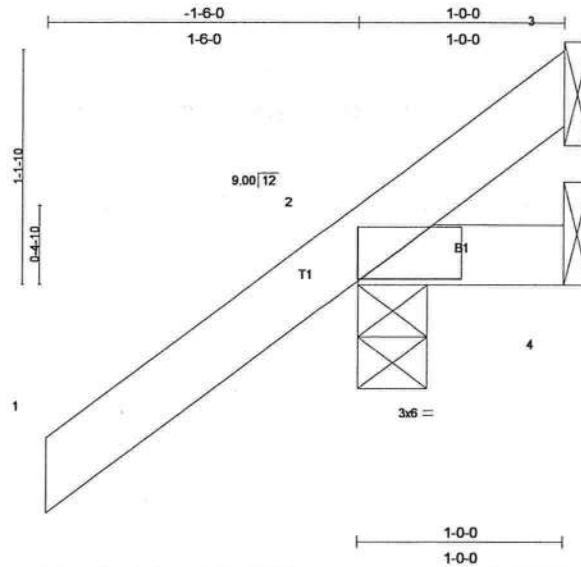
2 = 0.15, 3 = 0.03, 6 = 0.01 and 7 = 0.02

#### NOTES (5)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 4, 136 lb uplift at joint 2 and 14 lb uplift at joint 5.
- 5) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

#### LOAD CASE(S) Standard

Job L273732	Truss CJ1	Truss Type JACK	Qty 16	Ply 1	ADAMS FRAMING - ROCCO L273732001 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:14 2008 Page 1		



Scale = 1:10.7

Plate Offsets (X,Y): [2-0-3-13,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.18	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 6 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(lb/size) 2=180/0-4-0, 4=5/Mechanical, 3=-41/Mechanical  
Max Horz 2=106(load case 6)  
Max Uplift 2=-206(load case 6), 4=-11(load case 4), 3=-41(load case 1)  
Max Grav 2=180(load case 1), 4=14(load case 2), 3=74(load case 6)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-59/56  
BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.14

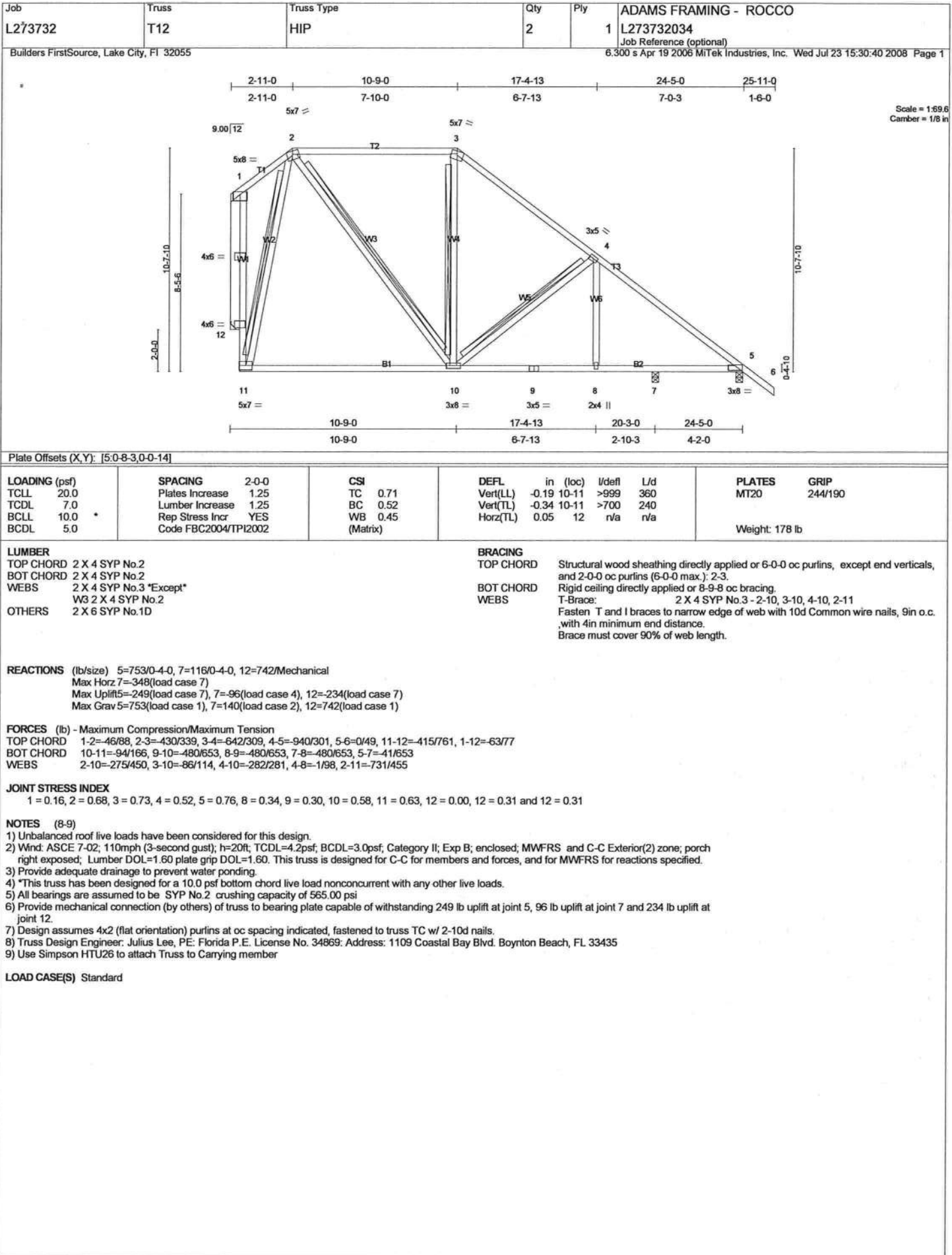
#### NOTES (5)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 2, 11 lb uplift at joint 4 and 41 lb uplift at joint 3.
- 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

#### LOAD CASE(S) Standard







Job L273732	Truss T14	Truss Type SPECIAL	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732036 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MITek Industries, Inc. Wed Jul 23 15:30:42 2008 Page 1

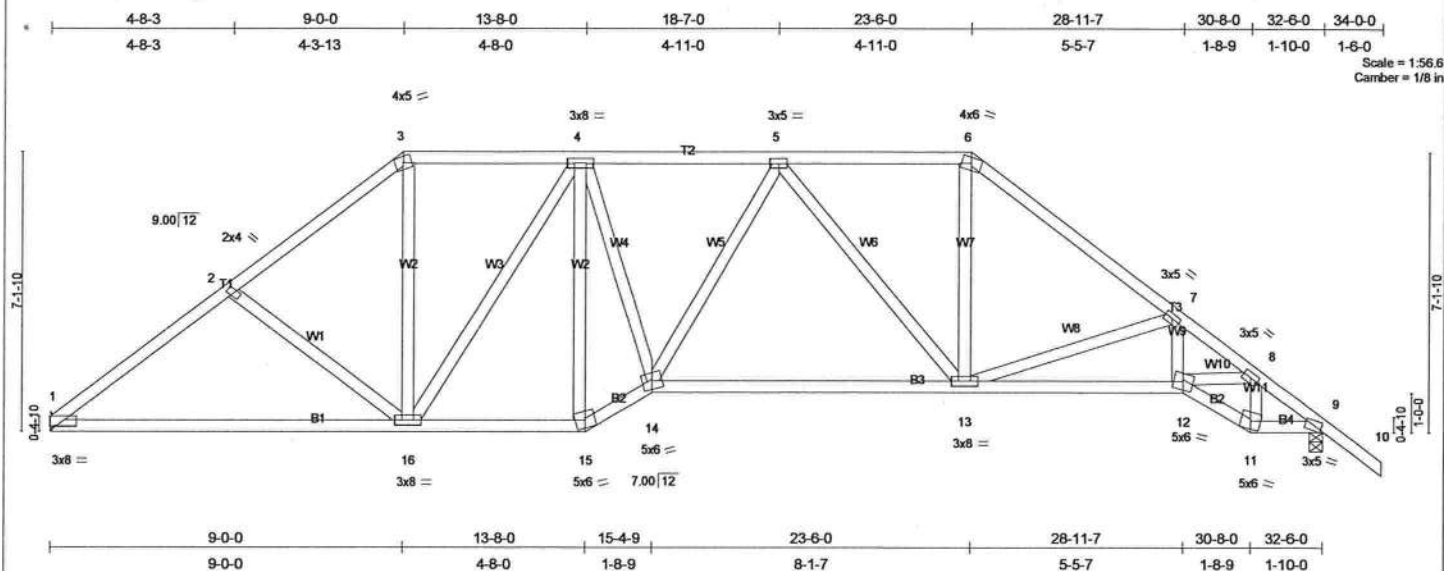


Plate Offsets (X,Y): [1:0-8-3,0-1-6], [9:0-0-13,Edge]

<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.28	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.49	Vert(LL) -0.15 1-16 >999 360		
BCLL 10.0	Rep Stress Incr YES	WB 0.56	Vert(TL) -0.29 1-16 >999 240		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	Horz(TL) 0.12 9 n/a n/a		
				Weight: 199 lb	

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 4-1-7 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 7-10-10 oc bracing.

**REACTIONS** (lb/size) 1=1028/Mechanical, 9=1122/0-4-0  
Max Horz 1=-210(load case 4)  
Max Uplift 1=-221(load case 5), 9=-249(load case 7)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-1488/740, 2-3=-1306/706, 3-4=-995/631, 4-5=-1404/789, 5-6=-1166/674, 6-7=-1520/738, 7-8=-2252/982, 8-9=-1449/622, 9-10=0/49  
BOT CHORD 1-16=-430/1135, 15-16=-368/1190, 14-15=-410/1368, 13-14=-433/1392, 12-13=-645/1765, 11-12=-357/1137, 9-11=-333/1045  
WEBS 2-16=-191/217, 3-16=-251/502, 4-16=-434/227, 4-15=-668/196, 4-14=-188/750, 5-14=-29/103, 5-13=-440/243, 6-13=-231/566, 7-13=-647/364, 7-12=-96/451, 8-12=-348/830, 8-11=-567/220

**JOINT STRESS INDEX**  
1 = 0.73, 2 = 0.34, 3 = 0.54, 4 = 0.94, 5 = 0.45, 6 = 0.56, 7 = 0.52, 8 = 0.52, 9 = 0.77, 11 = 0.30, 12 = 0.56, 13 = 0.57, 14 = 0.77, 15 = 0.34 and 16 = 0.59

**NOTES** (7-8)  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
3) Provide adequate drainage to prevent water ponding.  
4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 1 and 249 lb uplift at joint 9.  
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435  
8) Use Simpson HTU26 to attach Truss to Carrying member

**LOAD CASE(S)** Standard

Job

Truss

Truss Type

Qty

Ply

ADAMS FRAMING - ROCCO

L273732

T16

HIP

1

1

L273732038

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:44 2008 Page 1

6-8-3

13-0-0

19-6-0

25-9-13

32-6-0

34-0-0

6-8-3

6-3-13

6-6-0

6-3-13

6-8-3

1-6-0

6x10

4x6

3

4

T2

9.00/12

3x5

3x5

3x5

10-1-10

10-1-10

1

2

3

4

5

6

7

0-4-10

0-4-10

3x6

12

11

10

9

8

3x6

2x4

3x5

3x5

2x4

6-8-3

13-0-0

19-6-0

25-9-13

32-6-0

6-8-3

6-3-13

6-6-0

6-3-13

6-8-3

Plate Offsets (X,Y): [1:0-8-3,0-0-14], [6:0-6-3,0-0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.33	Vert(LL)	0.08	1-12	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.37	Vert(TL)	-0.13	1-12	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.27	Horz(TL)	0.06	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 193 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 4-9-13 oc purlins.

Rigid ceiling directly applied or 9-11-5 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 2-11, 3-9, 5-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.

with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1028/Mechanical, 6=1122/0-4-0

Max Horz 1=-293(load case 4)

Max Uplift 1=-198(load case 6), 6=-271(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1499/690, 2-3=-1136/650, 3-4=-827/605, 4-5=-1133/646, 5-6=-1484/670, 6-7=0/49

BOT CHORD 1-12=-359/1107, 11-12=-359/1107, 10-11=-182/827, 9-10=-182/827, 8-9=-337/1091, 6-8=-337/1091

WEBS 2-12=0/215, 2-11=-361/323, 3-11=-155/339, 3-9=-148/150, 4-9=-147/338, 5-9=-342/297, 5-8=0/211

JOINT STRESS INDEX

1 = 0.74, 2 = 0.52, 3 = 0.80, 4 = 0.68, 5 = 0.52, 6 = 0.69, 8 = 0.34, 9 = 0.59, 10 = 0.28, 11 = 0.40 and 12 = 0.34

NOTES (7-8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TC/DL=4.2psf; BC/DL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

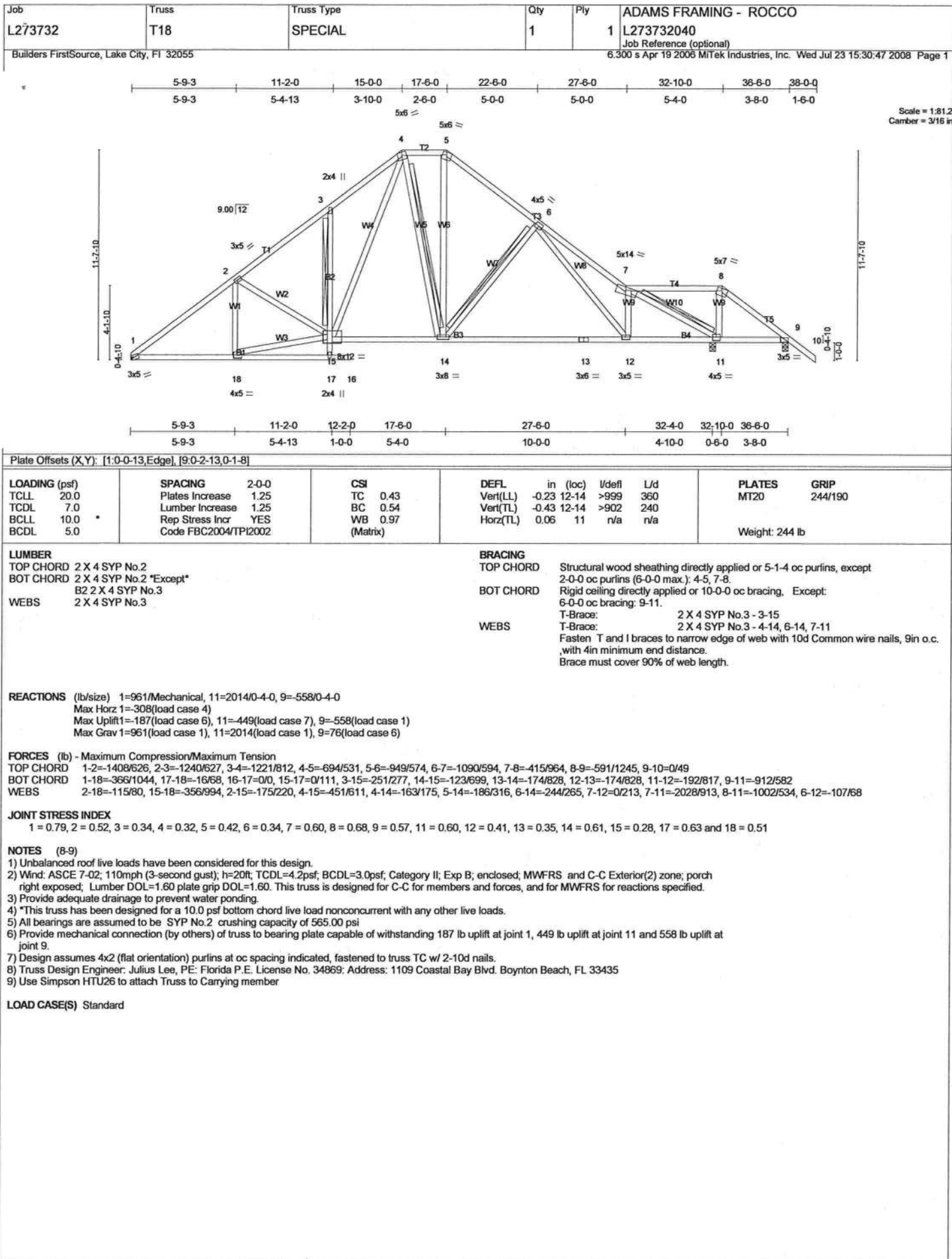
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint 1 and 271 lb uplift at joint 6.

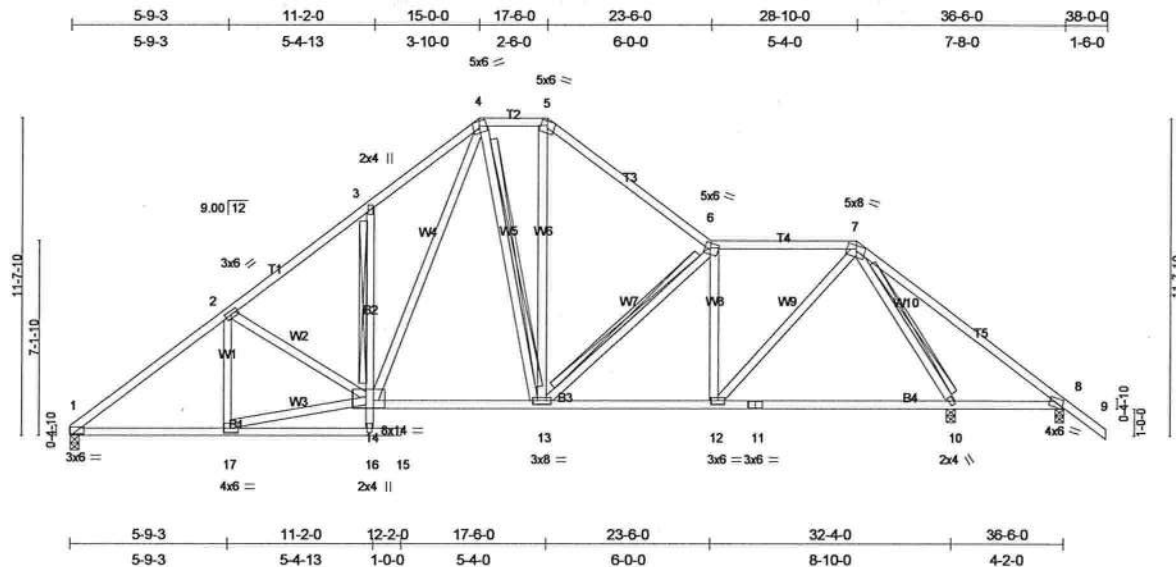
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



Job L273732	Truss T20	Truss Type SPECIAL	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732042 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:49 2008 Page 1		



6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:52 2008 Page 1



Plate Offsets (X,Y): [7:0-1-1,0-0-6]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.46	Vert(LL)	-0.05 9-10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.24	Vert(TL)	-0.09 9-10	>999	240		
BCLL 10.0	Rep Stress Incr YES	WB 0.47	Horz(TL)	0.01 14	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)					Weight: 212 lb	

## LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 6 SYP No.1D

## BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 2-3, 4-5.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 4-12, 5-10, 2-13 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

## REACTIONS

(lb/size) 9=784/0-4-0, 7=198/0-4-0, 14=629/Mechanical  
Max Horz 9=-348(load case 7)  
Max Uplift 9=-299(load case 7), 7=-80(load case 5), 14=-197(load case 7)  
Max Grav 9=784(load case 1), 7=199(load case 11), 14=629(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=55/67, 2-3=217/202, 3-4=310/212, 4-5=352/265, 5-6=562/199, 6-7=186/59, 7-8=0/49, 13-14=274/546, 1-14=83/84  
BOT CHORD 12-13=67/124, 11-12=194/313, 10-11=194/313, 9-10=189/46, 7-9=8/251  
WEBS 2-12=241/431, 3-12=66/78, 4-12=413/317, 4-10=107/82, 5-10=125/170, 6-10=68/354, 6-9=728/479, 2-13=538/289

### JOINT STRESS INDEX

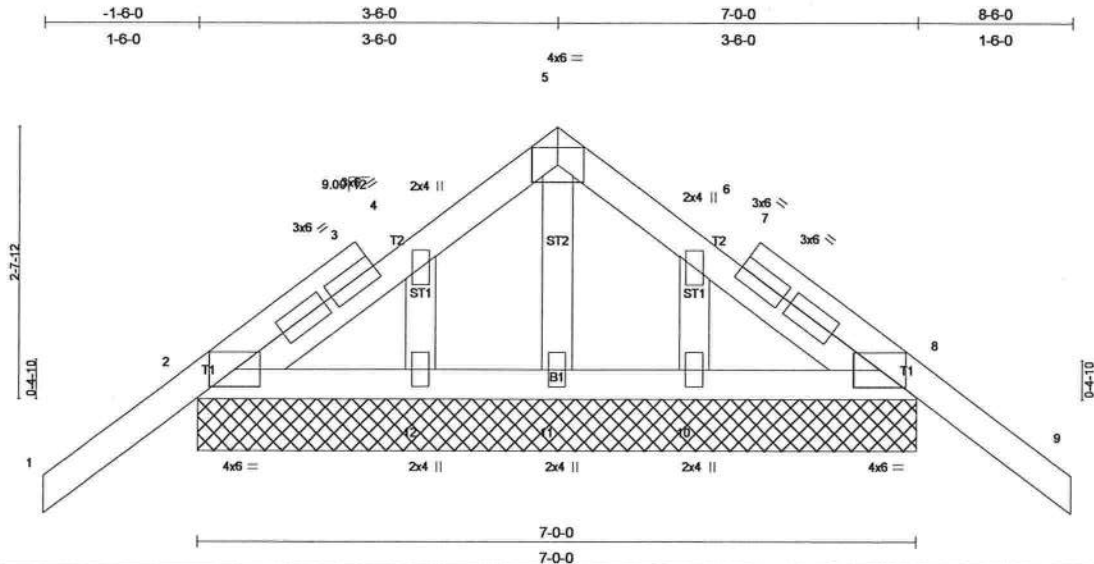
1 = 0.16, 2 = 0.32, 3 = 0.31, 4 = 0.55, 5 = 0.97, 6 = 0.52, 7 = 0.76, 9 = 0.34, 10 = 0.60, 11 = 0.18, 12 = 0.61, 13 = 0.48, 14 = 0.00, 14 = 0.26 and 14 = 0.26

## NOTES (8-9)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCLD=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 9, 80 lb uplift at joint 7 and 197 lb uplift at joint 14.
- 7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 9) Use Simpson HTU26 to attach Truss to Bearing member

## LOAD CASE(S) Standard

Job L273732	Truss T23G	Truss Type GABLE	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732046 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:53 2008 Page 1		



<b>LOADING</b> (psf)	<b>SPACING</b> 2'-0"	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.16	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.03	Vert(LL) -0.01 9 n/r 120		
BCLL 10.0	Rep Stress Incr YES	WB 0.02	Vert(TL) -0.01 9 n/r 90		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.00 8 n/a n/a		
				Weight: 40 lb	

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.
OTHERS 2 X 4 SYP No.3	

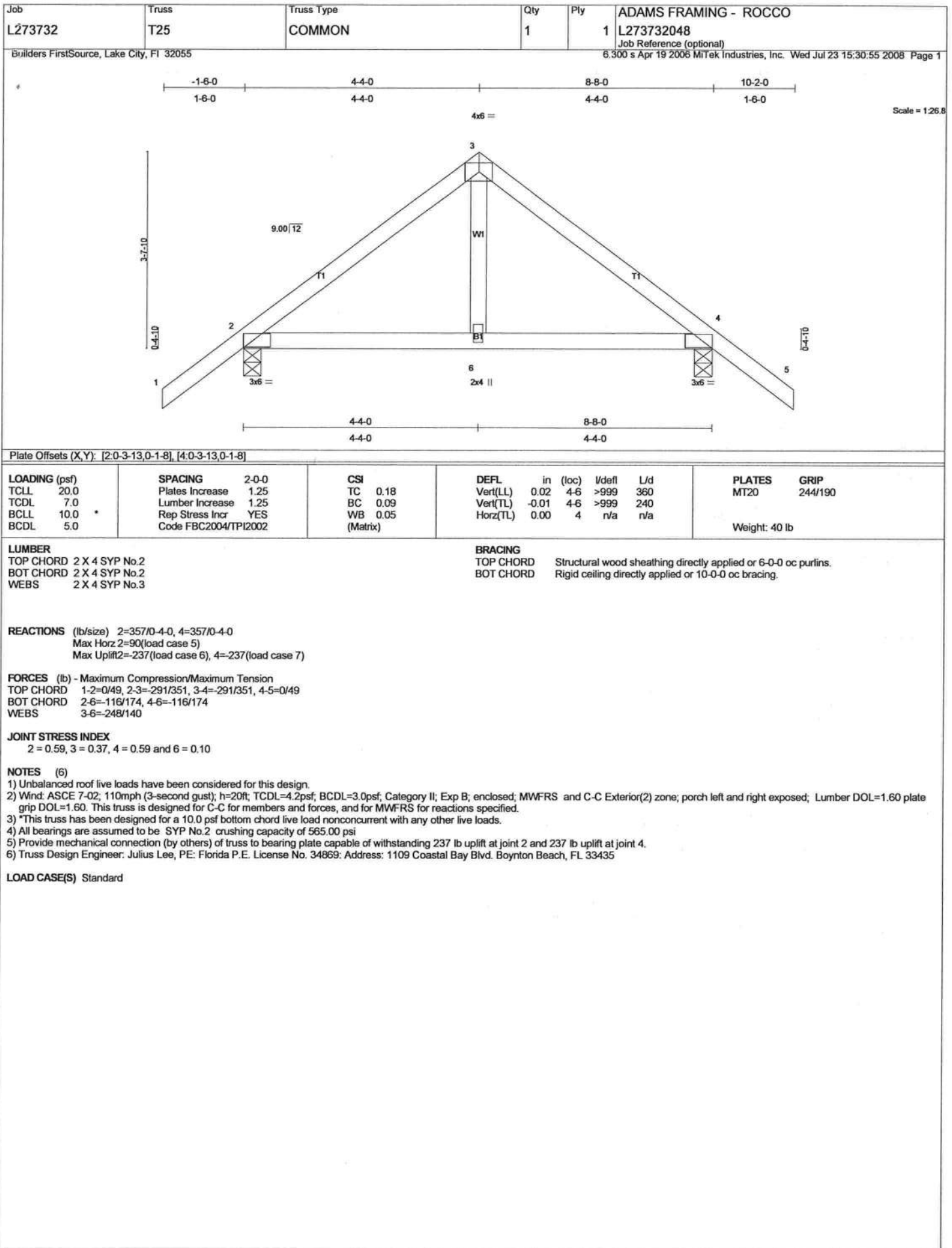
<b>REACTIONS</b> (lb/size)	2=181/7'-0", 8=181/7'-0", 11=72/7'-0", 12=88/7'-0", 10=88/7'-0"
Max Horz 2=-86(load case 4)	
Max Uplift 2=-158(load case 6), 8=-173(load case 7), 11=-1(load case 5), 12=-36(load case 7), 10=-38(load case 4)	
Max Grav 2=181(load case 1), 8=181(load case 1), 11=72(load case 1), 12=90(load case 10), 10=90(load case 11)	

<b>FORCES</b> (lb) - Maximum Compression/Maximum Tension	
TOP CHORD 1-2=0/47, 2-3=-52/56, 3-4=-38/67, 4-5=-35/90, 5-6=-35/90, 6-7=-6/46, 7-8=-52/39, 8-9=0/47	
BOT CHORD 2-12=-3/91, 11-12=-3/91, 10-11=-3/91, 8-10=-3/91	
WEBS 5-11=-58/3, 4-12=-77/60, 6-10=-77/60	

<b>JOINT STRESS INDEX</b>	
2 = 0.69, 3 = 0.00, 3 = 0.16, 3 = 0.16, 4 = 0.04, 5 = 0.06, 6 = 0.04, 7 = 0.00, 7 = 0.16, 7 = 0.16, 8 = 0.69, 10 = 0.03, 11 = 0.02 and 12 = 0.03	

- NOTES** (9)
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
  - 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 5) Gable requires continuous bottom chord bearing.
  - 6) Gable studs spaced at 1'-4" oc.
  - 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
  - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 2, 173 lb uplift at joint 8, 1 lb uplift at joint 11, 36 lb uplift at joint 12 and 38 lb uplift at joint 10.
  - 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

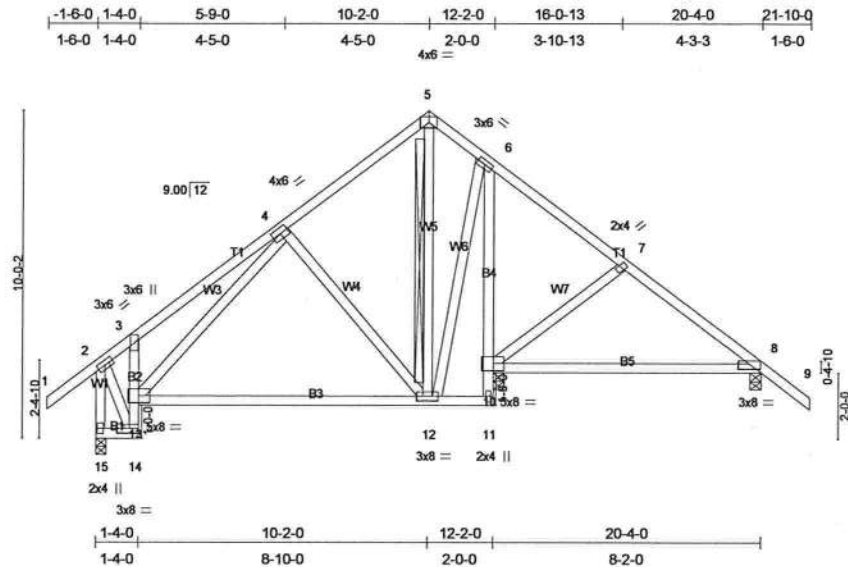
**LOAD CASE(S)** Standard



Job L273732	Truss T26	Truss Type SPECIAL	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732050 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 M/Tek Industries, Inc. Wed Jul 23 15:30:57 2008 Page 1



Scale = 1/8" = 1'-0"  
Camber = 1/16" in

Plate Offsets (X,Y): [8-0-8-3,0-1-6]

<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.20	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.63	Vert(LL) 0.20 8-10 >483 360		
BCLL 10.0	Rep Stress Incr YES	WB 0.20	Vert(TL) -0.19 12-13 >741 240		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.03 10 n/a n/a		
				Weight: 145 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2 \*Except\*  
B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3  
WEBS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:  
10-0-0 oc bracing: 12-13.  
WEBS T-Brace: 2 X 4 SYP No.3 - 5-12  
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.  
Brace must cover 90% of web length.

#### REACTIONS

(lb/size) 15=462/0-4-0, 10=656/0-4-0, 8=341/0-4-0  
Max Horz 15=262(load case 5)  
Max Uplift 15=-108(load case 6), 10=-296(load case 6), 8=-256(load case 7)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-196/174, 3-4=-323/270, 4-5=-161/157, 5-6=-107/179, 6-7=-65/117, 7-8=-248/131, 8-9=0/49, 2-15=-506/255  
BOT CHORD 14-15=-222/231, 13-14=-175/82, 3-13=-134/111, 12-13=-108/240, 11-12=-102/244, 10-11=-61/3, 6-10=-510/224, 8-10=-17/148  
WEBS 5-12=-117/92, 7-10=-192/255, 2-14=-26/254, 4-12=-212/258, 4-13=-246/198, 6-12=-180/392

#### JOINT STRESS INDEX

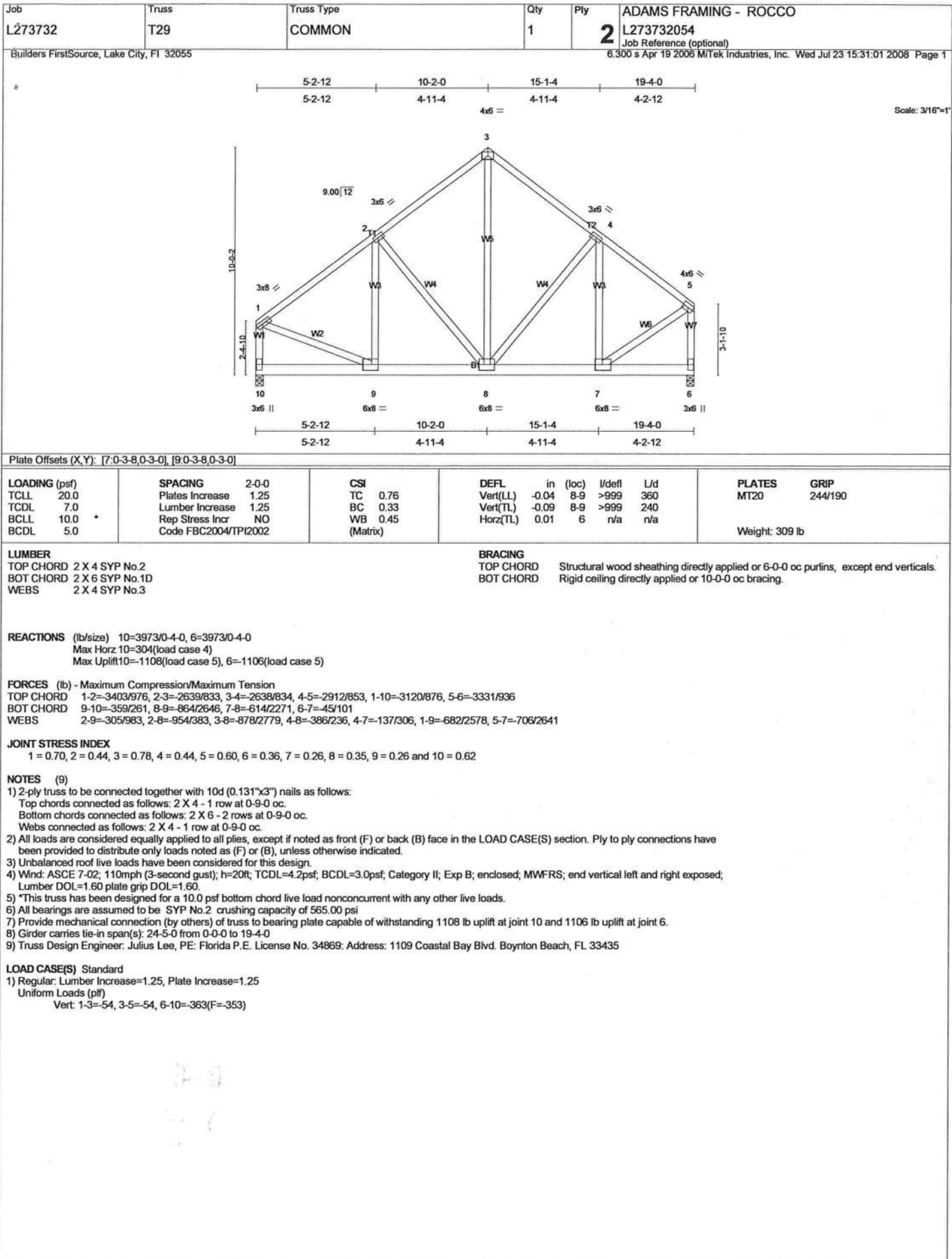
2 = 0.61, 3 = 0.36, 4 = 0.29, 5 = 0.29, 6 = 0.55, 7 = 0.34, 8 = 0.77, 10 = 0.77, 11 = 0.34, 12 = 0.61, 13 = 0.59, 14 = 0.67 and 15 = 0.34

#### NOTES (6)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 15, 296 lb uplift at joint 10 and 256 lb uplift at joint 8.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

LOAD CASE(S) Standard



## #2 HIP OR COMMON TRUSS

2' TYP  
MAX

I

UPLIFT: 400# or Less

UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND  
SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

UPLIFT: 400# or Less

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



BC LIVE LOAD IS NON CONCURRENT 10\*

CORNER SET  
SETBACK

7'0" MAX

**WARNING==** THESE RESIST EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC01-1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 589 DOWNEY RD., SUITE 200, MARYSVILLE, VA 52719 AND VITA CORD TRUSS COMPANY OF AMERICA, 6500 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROTECT ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROTECT ATTACHED CEILING.

**PRODUCT==** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR APPLICABLE PROVISIONS OF NIS NATIONAL DESIGN & BRACING OF TRUSSES. DESIGN CONDITIONS WILL BE FAVORABLE PROVIDED NO UNSUITABLE DESIGN BY ARCHITECT AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF E0/B/16/G6 (V.H.S) ASH A655 GRABIE (D/0.01) DESIGNED INSTITUTION FOR BRACING ULS-2. EACH INSPECTION OF THE TRUSS SHALL BE PER ANEX A4 OF TPI-1-8002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING REGISTRATION. THE TRUSS COMPONENT DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/TPI 1 SEC. 2.

CONS. EDGINGERS, P. L.

DELRAY BEACH, FL 33444-2101

STATE OF

	DUR.	FAC.	1.26
S.PACING	2'	MAX	

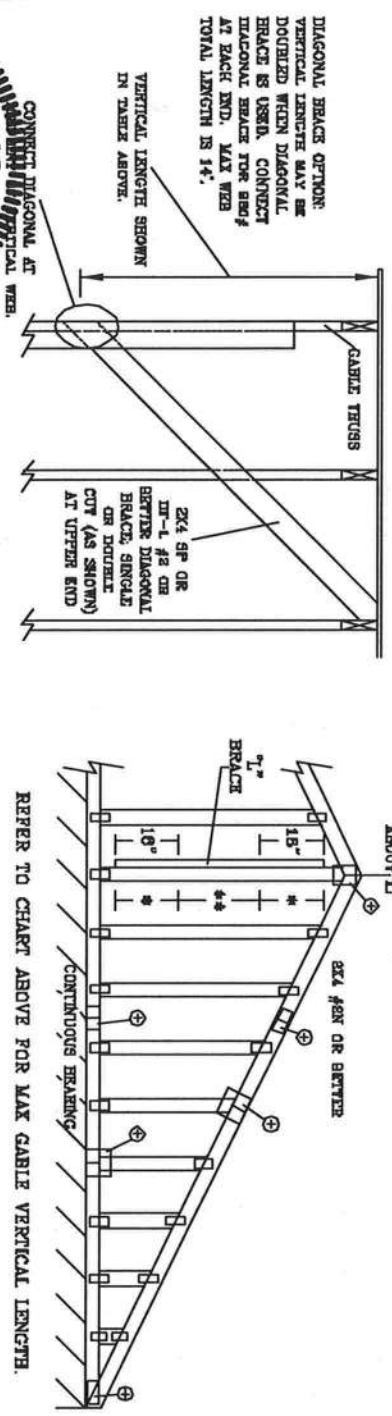
REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	

**REVIEWED**  
By Julius lee at 10:52 am, Jun 27, 2008



ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																
CABLE VERTICAL SPACING	2x4 SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 "L" BRACE * (1) 2x4 "L" BRACE * (2) 2x4 "L" BRACE ** (1) 2x6 "L" BRACE * (2) 2x6 "L" BRACE * (2) 2x8 "L" BRACE **												
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"			
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"			
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"			
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"	12' 3"	12' 3"		
		#4	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	12' 3"			
	SPF	#2	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	12' 3"			
		#3	3' 3"	4' 6"	4' 6"	5' 11"	6' 0"	7' 10"	8' 0"	9' 4"	9' 4"	12' 3"	12' 3"			
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	6' 0"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 3"			
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	6' 11"	8' 1"	8' 0"	10' 10"	10' 10"			
		#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"			
16" O.C.	SPF	#2	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"			
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"			
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"			
		STANDARD	3' 4"	4' 6"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"			
		#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	8' 7"	11' 9"	12' 8"	14' 0"	14' 0"			
	SPF	#2	3' 11"	6' 4"	6' 10"	7' 4"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"			
		#3	3' 8"	5' 7"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"			
		STUD	3' 8"	5' 8"	5' 8"	6' 7"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"			
		STANDARD	3' 6"	4' 9"	4' 9"	6' 3"	6' 3"	8' 3"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"			
		#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"			
24" O.C.	SPF	#2	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"			
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"			
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"			
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"			
		#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"			
	SPF	#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"			
		#3	4' 2"	6' 6"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"			
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"			
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
		#1	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			



BRACING GROUP SPECIES AND GRADES:		GROUP A:		GROUP B:	
SPRICE-PINE-YR		HOL-FIR		DOUGLAS FIR-LARCH	
#1 / #2	STUD	#2	STUD	#3	STUD
#3	STUD	#3	STUD	#3	STUD
STUD	STUD	STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD	STANDARD	STANDARD

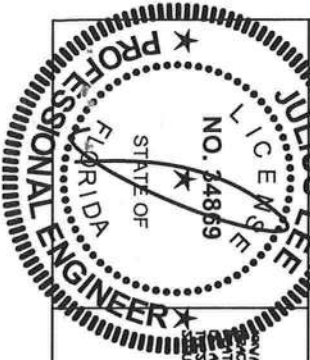
CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS PER 160 P.F. OVER CONTINUOUS BRACING (6 PSF TO DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLEAKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 10d NAILS.	
* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES, SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SERVICE
LESS THAN 4' 0"	1x4 OR 2x4
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2x4
GREATER THAN 11' 8"	2x6

+ REFER TO COMMON TRUSS DESIGN FOR PLATE, SPICE, AND BEEL PLATES	
--	--



WORKMAN TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST-1-93 CARRYING CAPACITY INFORMATION, PUBLISHED BY THE TRUSS INSTITUTE, 382 BROADVIEW DR., SUITE 200, MARIETTA, GA 30067 AND VITA (WOOD TRUSS COUNCIL) 1000 W. 10TH AVE., SUITE 100, DENVER, CO 80202 FOR SAFETY PRACTICES PRIOR TO PERFORMING STRUCTURAL ANALYSIS AND DESIGN. ALL TRUSSES MUST HAVE A PERMANENTLY ATTACHED STRUCTURAL PLATE AND EITHER GROUND SHALL HAVE A PERMANENTLY ATTACHED ROAD GROUND.

REVIEWED  
By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S  
CONS. ENGINEERS P.A.  
1456 SW 4th AVENUE  
DELRAY BEACH, FL 33444-8161

No. 34869  
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF  
MAX. SPACING 24.0"

REF ASCE 7-02-GAB10030  
DATE 11/26/03  
DWG DATE STD GAB1 30' x 17'  
-ENG

TOP CHORD 2X4 #2 OR BETTER  
BOT CHORD 2X4 #2 OR BETTER  
WEBS 2X4 #3 OR BETTER

# PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

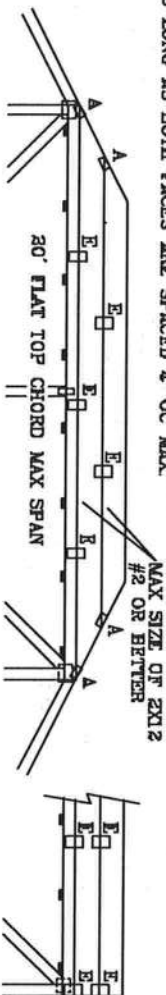
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST  
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF  
110 MPH WIND, 30' MEAN HGT, PBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF  
WIND TC DL=5 PSF, WIND BC DL=5 PSF

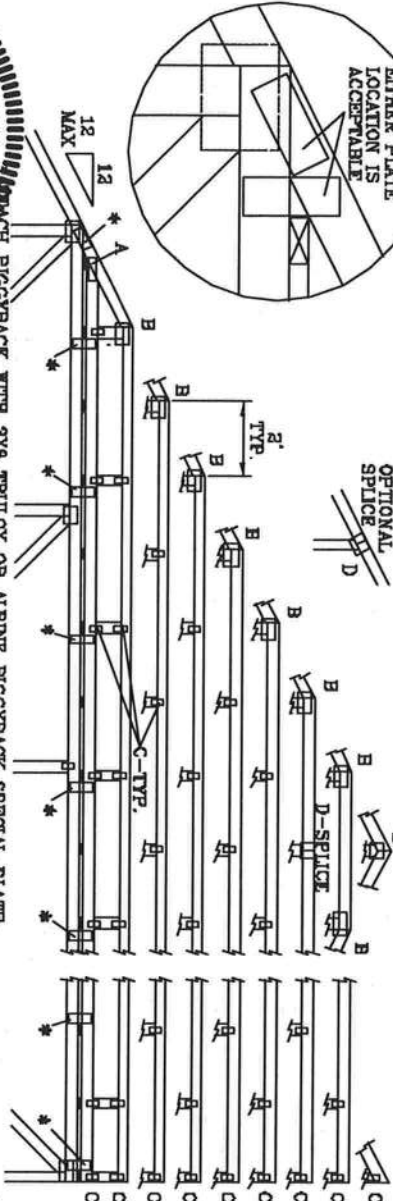
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



ETHER PLATE LOCATION IS ACCEPTABLE

OPTIONAL SPLICE



THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

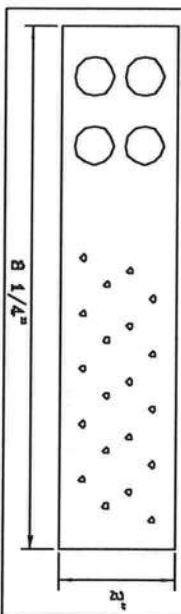
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	6X6	6X8	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X6	6X8	5X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (B) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80X LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4" OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80X LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

\* PIGGYBACK SPECIAL PLATE

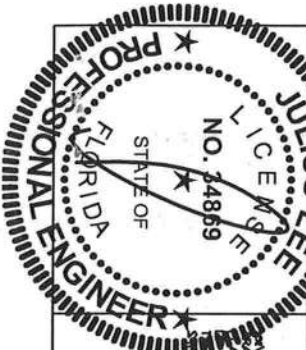
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



**JULIUS LEE'S**  
CONS. ENGINEERS P.A.  
1449 SW 42nd AVENUE  
DEERBEEK, FL 33441-2161

MAX LOADING  
65 PSF AT  
1.33 DUR. FAC.  
60 PSF AT  
1.25 DUR. FAC.  
47 PSF AT  
1.15 DUR. FAC.  
SPACING 24.0"

REF PIGGYBACK  
DATE 09/12/07  
DRWG/ITEK STD PIGGY  
-ENG JL



**REVIEWED**  
By Julius Lee at 11:59 am, Jun 11, 2008

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AP&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

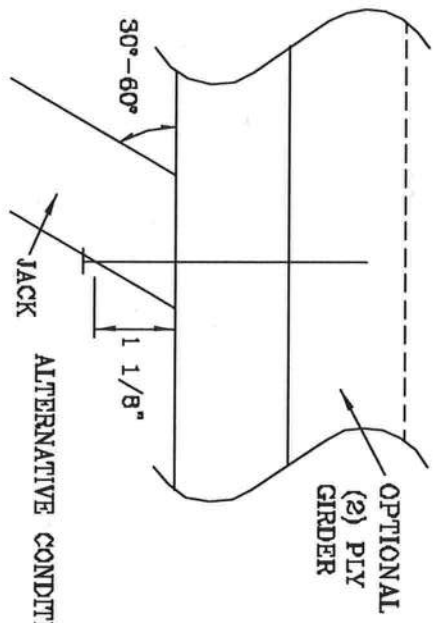
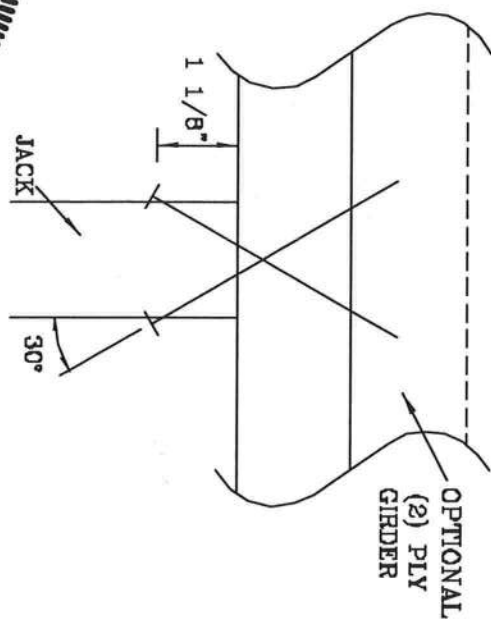
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

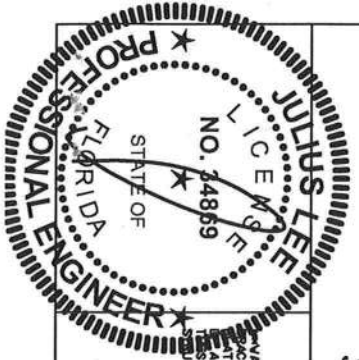
NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	187#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS SAFETY. TRUSS SAFETY PRACTICES, PUBLISHED BY THE TRUSS ASSOCIATION, 6500 ENTERPRISE DR., SUITE 200, NATION, VA 22641-1103. THESE PRACTICES SHOULD BE FOLLOWED FOR ALL TRUSS STRUCTURAL PANELS AND BRACING AND OTHER CHORD SHALL HAVE A PERMANENTLY ATTACHED

REVIEWED  
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S  
CONS. ENGINEERS P.A.  
1405 EY 4th AVENUE  
DELMAR BEACH, FL 33444-2161

No. 34869  
STATE OF FLORIDA

TC IL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC IL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

# TRULOX CONNECTION DETAIL

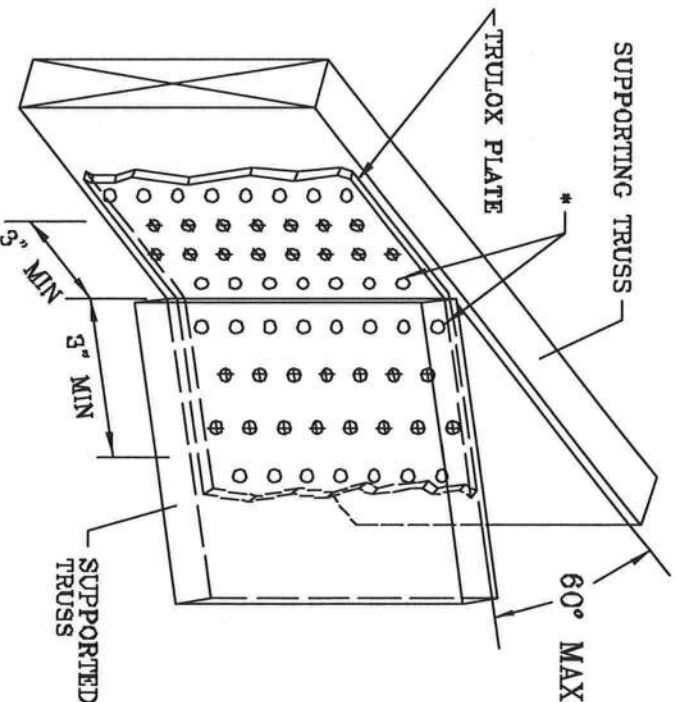
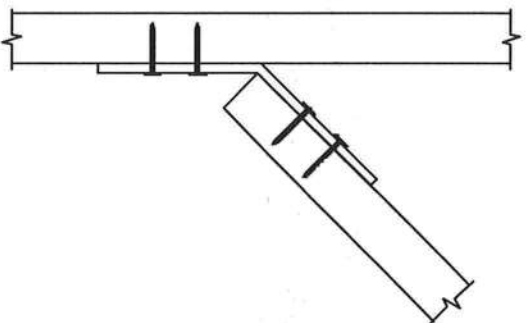
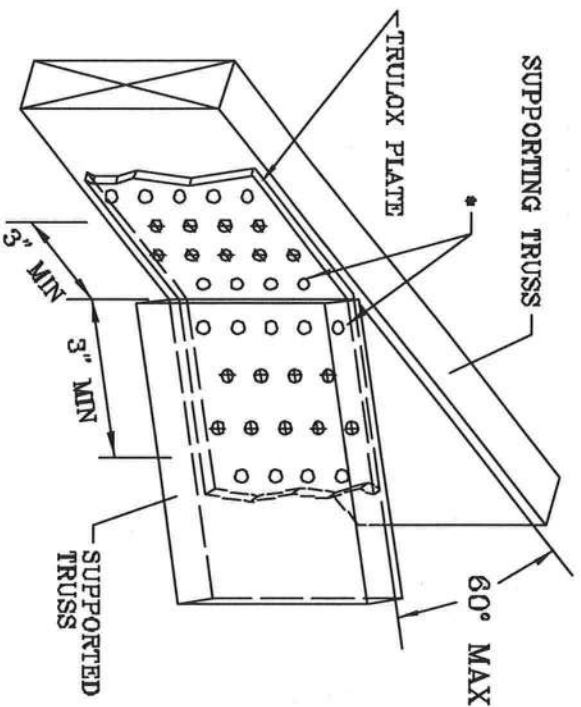
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

\* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

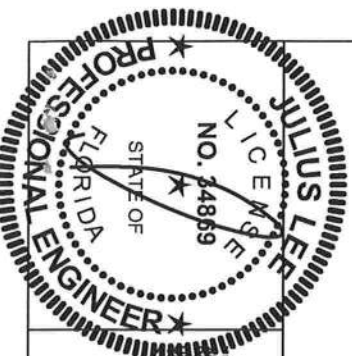
TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350 #
5X6	15	990 #

MINIMUM 5X6 TRULOX PLATE

REVIEWED  
By Julius Lee at 11:58 am, Jun 11, 2008

THIS DRAWING REPLACES DRAWINGS 1.156,989 1.158,989/R  
1.154,944 1.152,217 1.152,017 1.159,154 & 1.151,524

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DC21-1-03 BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURING INSTITUTE, 366 JENNIFER DR., SUITE 500, NATION, VA 28755 AND VITA CYCLO TRUSS COUNCIL, 1000 N. 10TH AVE., SUITE 100, NATION, VA 28755 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



JULIUS LEE'S  
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE  
DELRAY BEACH, FL 33444-3811

Not: 34869  
STATE OF FLORIDA

REF TRULOX

DATE 11/26/03

DRWG CTRULOX1103

-ENG JL

# MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

## Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail <sup>(1)</sup>	2	12"	370	<b>280</b>	280	<b>245</b>		
	3	12"	555	<b>415</b>	415	<b>370</b>		
1/2" A307 Through Bolts <sup>(2)(4)</sup>	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" <sup>(4)</sup>	2	24"	680	<b>510</b>	510	<b>455</b>		
		19.2"	850	<b>640</b>	640	<b>565</b>		
		16"	1,020	<b>765</b>	765	<b>680</b>		
SDS 1/4" x 6" <sup>(3)(4)</sup>	2	24"				<b>455</b>	<b>465</b>	<b>455</b>
		19.2"				<b>565</b>	<b>580</b>	<b>565</b>
		16"				<b>680</b>	<b>695</b>	<b>680</b>
USP WS35 <sup>(4)</sup>	2	24"	480	<b>360</b>	360	<b>320</b>		
		19.2"	600	<b>450</b>	450	<b>400</b>		
		16"	715	<b>540</b>	540	<b>480</b>		
USP WS6 <sup>(3)(4)</sup>	2	24"				<b>350</b>	<b>525</b>	<b>350</b>
		19.2"				<b>440</b>	<b>660</b>	<b>440</b>
		16"				<b>525</b>	<b>790</b>	<b>525</b>
3 3/4" TrussLok <sup>(4)</sup>	2	24"	635	<b>475</b>	475	<b>425</b>		
		19.2"	795	<b>595</b>	595	<b>530</b>		
		16"	955	<b>715</b>	715	<b>635</b>		
5" TrussLok <sup>(4)</sup>	2	24"		<b>500</b>	500	<b>445</b>	<b>480</b>	<b>445</b>
		19.2"		<b>625</b>	625	<b>555</b>	<b>600</b>	<b>555</b>
		16"		<b>750</b>	750	<b>665</b>	<b>725</b>	<b>665</b>
6 3/4" TrussLok <sup>(4)</sup>	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

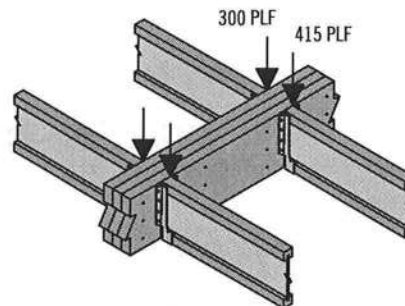
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

## General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

## Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

### Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

# MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

## Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 <sup>(1)</sup>	4	1,915	1,435 <sup>(4)</sup>	1,435	1,275	1,660 <sup>(2)</sup>	1,405 <sup>(2)</sup>
	6	2,870	2,150 <sup>(4)</sup>	2,150	1,915	2,785 <sup>(2)</sup>	2,110 <sup>(2)</sup>
	8	3,825	2,870 <sup>(4)</sup>	2,870	2,550	3,715 <sup>(2)</sup>	2,810 <sup>(2)</sup>
	4	2,545	1,910 <sup>(4)</sup>	1,910	1,695	1,925 <sup>(2)</sup>	1,775 <sup>(2)</sup>
3 3/8" or 5" TrussLok™	6	3,815	2,860 <sup>(4)</sup>	2,860	2,545	2,890 <sup>(2)</sup>	2,665 <sup>(2)</sup>
	8	5,090	3,815 <sup>(4)</sup>	3,815	3,390	3,855 <sup>(2)</sup>	3,550 <sup>(2)</sup>

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(2) 6" long screws required.

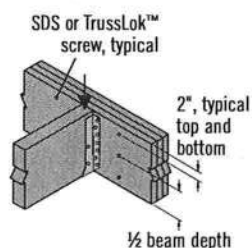
(3) 5" long screws required.

(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

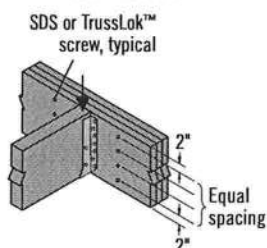
See General Notes on page 38

## Connections

### 4 or 6 or Screw Connection

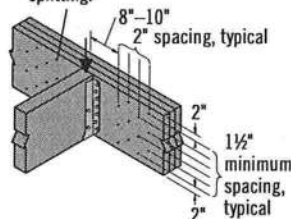


### 8 Screw Connection



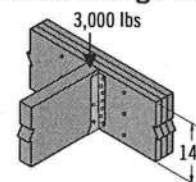
### Nail Connection

10d (0.128" x 3") nails, typical. Stagger to prevent splitting.



There must be an equal number of nails on each side of the connection

## Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

# MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

## 1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d-16d (0.148"-0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

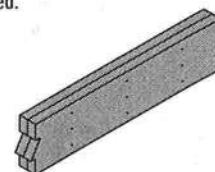
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

## 3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

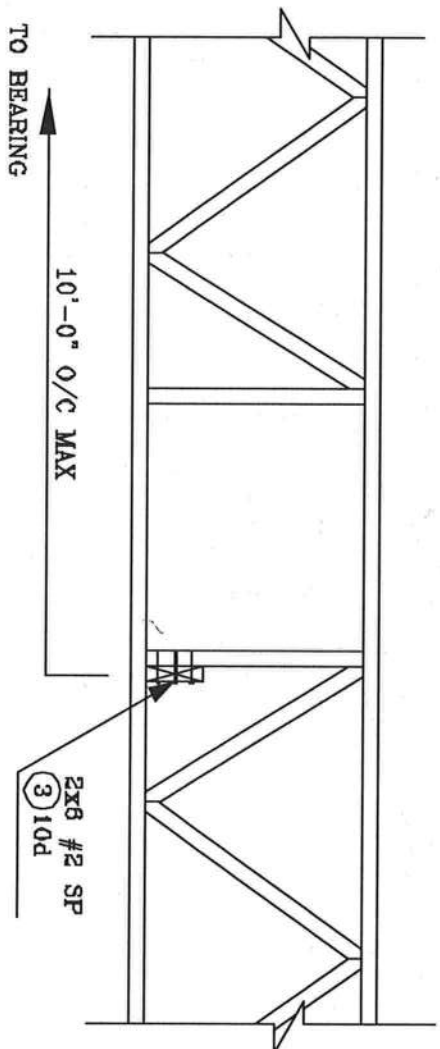
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



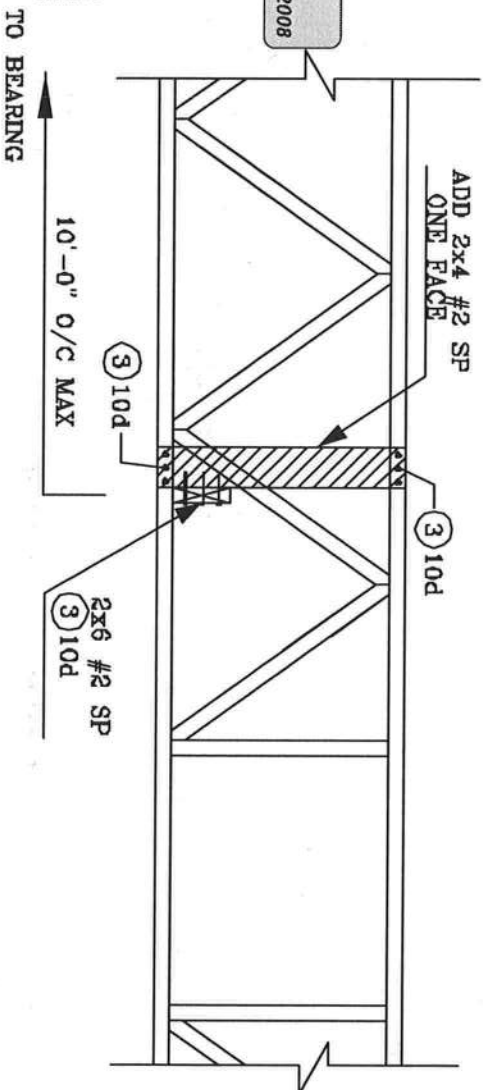
Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6

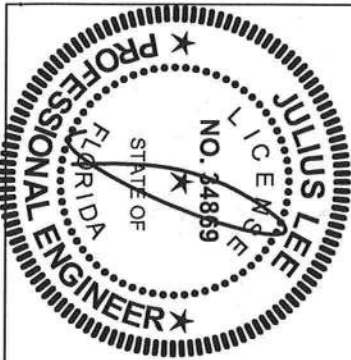
# STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



## ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



**REVIEWED**  
By Julius Lee at 11:58 am, Jun 11, 2008



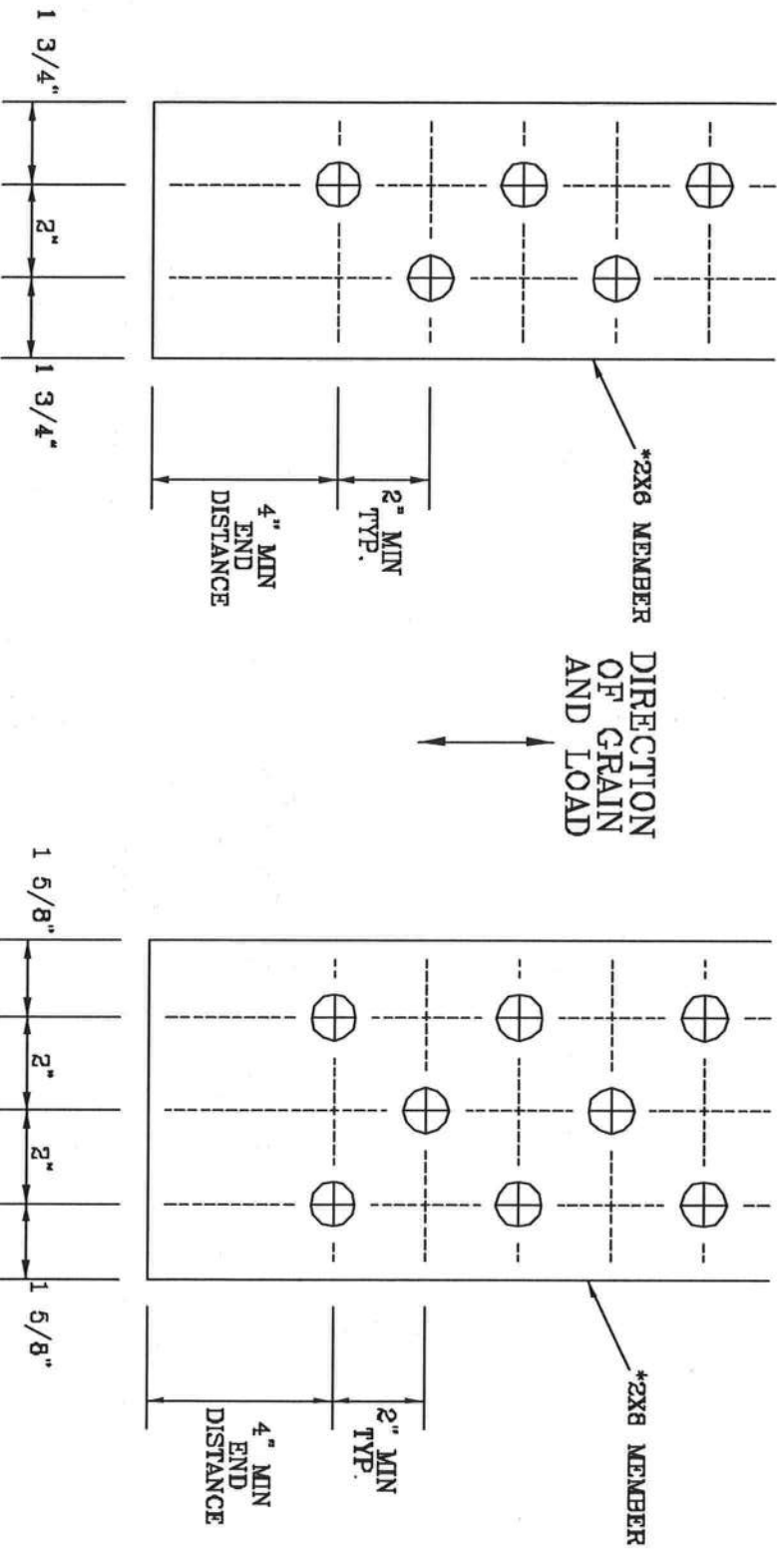
**JULIUS LEE'S**  
CONS. ENGINEERS P.A.  
1425 SW 4th AVENUE  
DISSAULT BEACH, FL 33444-2161

No: 34869  
STATE OF FLORIDA

# 1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

\* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.  
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.  
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.01.6

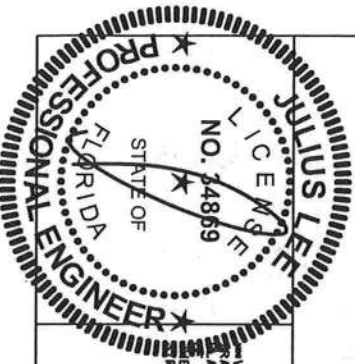
WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND RIGGING. REFER TO 2001 IBC BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS AND JOIST MANUFACTURERS ASSOCIATION (TJMA), 1400 ST. 4TH AVENUE, SUITE 200, WASHINGTON, VA 22191 AND VITA CYCLED TRUSS COUNCIL, 1400 ST. 4TH AVENUE, SUITE 200, WASHINGTON, VA 22191. THESE PRACTICES PREFER TO PERFORMING THESE FUNCTIONS. THESE OTHER PRACTICES SHALL HAVE A PROPERLY ATTACHED ROAD DETAIL.

JULIUS LEE'S  
CONS. ENGINEERS P.A.  
1400 ST. 4TH AVENUE  
DELTAT BLDG. FL 39444-2101

TC IL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC IL	PSF	ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008



TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.  
BOT CHORD 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.  
WEBS 2X4 SP #3 OR BETTER.

\* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE),

ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR  
FBC 2004 110 MPH. ASCE 7-02 110 MPH WIND OR (3) 16d FOR  
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED  
BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=6 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "J"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

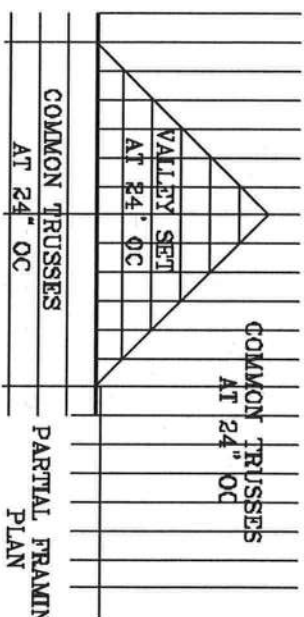
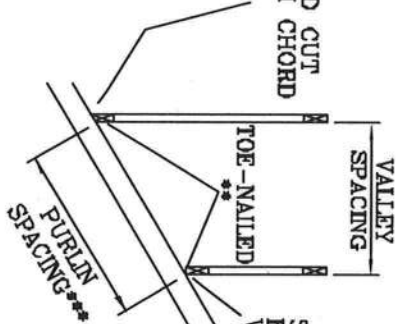
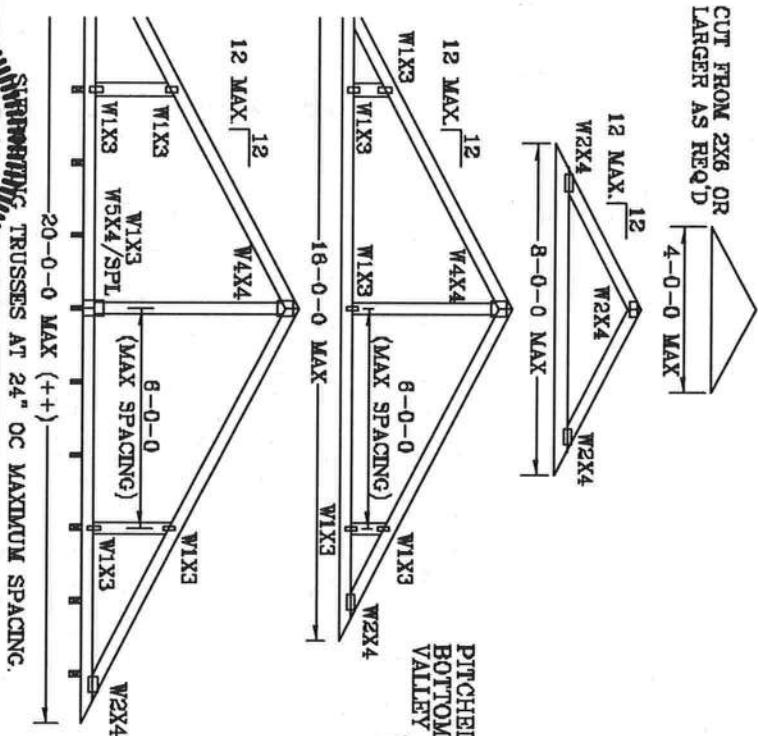
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH  
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS  
INSTALLATION  
OR

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR  
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

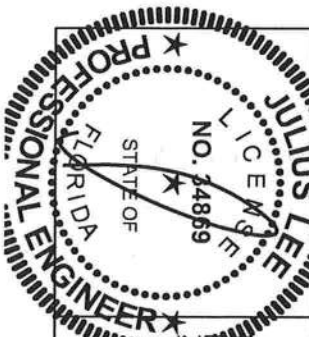
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



TRUSSES AT 24" OC MAXIMUM SPACING

**THIS DRAWING REPLACES DRAWING A105**

PERSONNEL WHO ARE RESPONSIBLE FOR THE DESIGN, FABRICATING, INSTALLING AND MAINTAINING OF BUILDING EQUIPMENT. THE JOURNAL, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 380 DORFMAN DR., SUITE 200, WESTPORT, MA 01886, IS A MUST-READ FOR ALL TRUSS CONSTRUCTION PROFESSIONALS. THE JOURNAL PROVIDES THE LATEST INFORMATION ON THE LATEST TECHNOLOGY AND DESIGN PRACTICES. THE JOURNAL IS A MUST-READ FOR ALL TRUSS CONSTRUCTION PROFESSIONALS. THE JOURNAL IS A MUST-READ FOR ALL TRUSS CONSTRUCTION PROFESSIONALS.



REVIEWED

By julius lee at 11:59 am, Jun 11, 2008

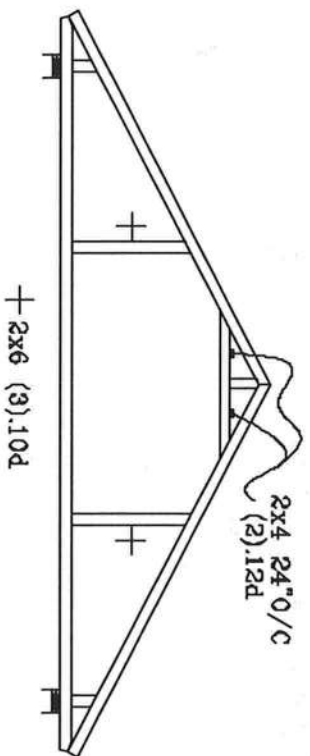
**JULIUS LEE'S  
CONS. ENGINEERS P.A.**

1455 SW 4th Avenue  
Delray Beach, FL 33446-2161

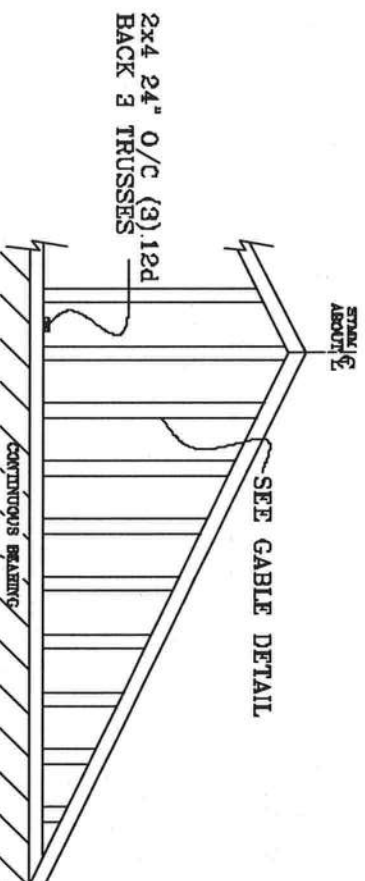
1455 SW 4th AVENUE  
LRY BEACH, FL 3344-2161

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DUR.FAC.	1.25	1.25			
SPACING	24"				

# TYPICAL ATTIC TRUSS BRACING

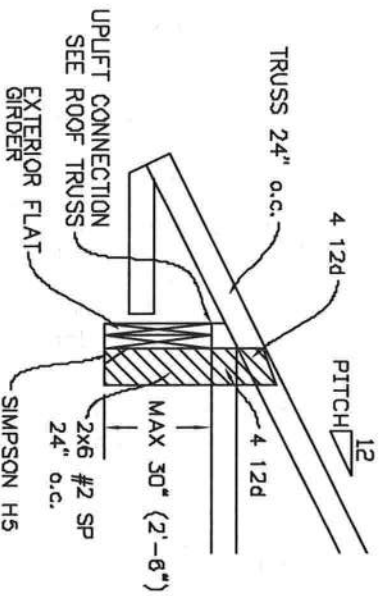


# GABLE END TRUSS DETAIL

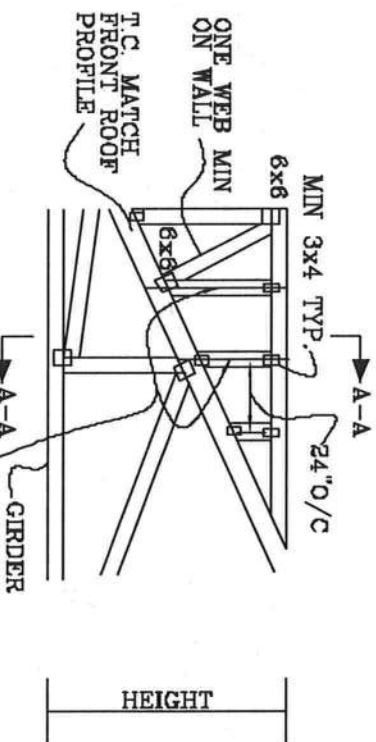


MINIMUM BR BRACING ON GABLE TRUSSES OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

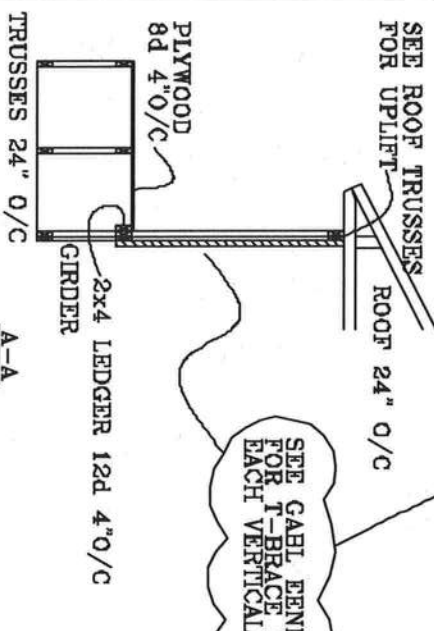
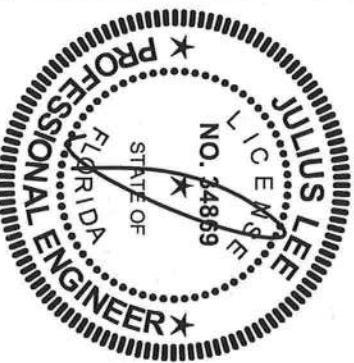
# TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



# TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



REVIEWED  
By Julius Lee at 11:59 am, Jun 11, 2008

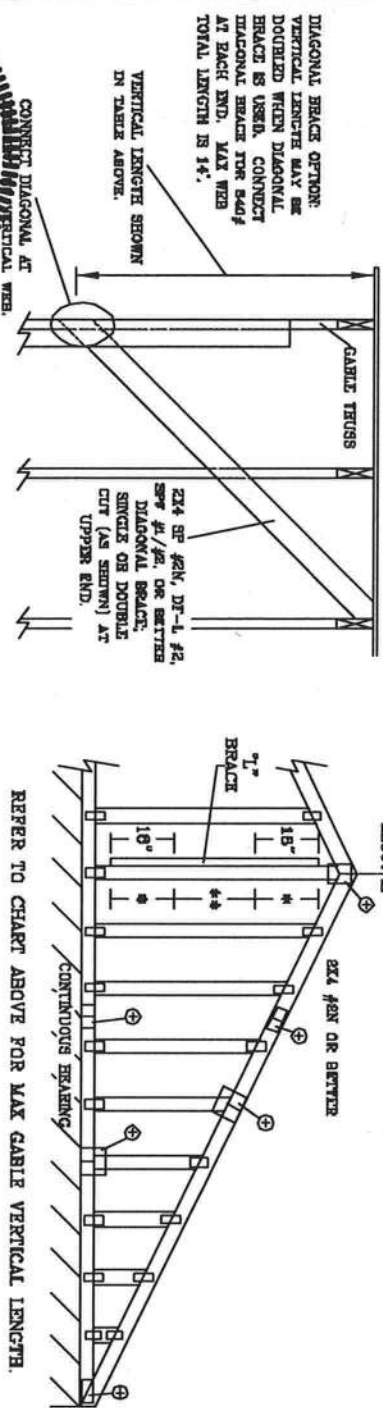


SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL

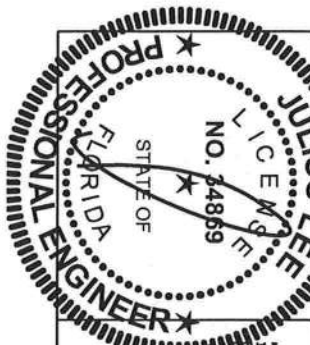
JULIUS LEE'S  
CONS. ENGINEERS P.A.  
1425 SW 4th AVENUE  
DISSAULT BEACH, FL 33444-2661

No. 34869  
STATE OF FLORIDA

MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE **	
SPACING	CABLE VERTICAL SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"				
	STUD	#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"				
	HF	STANDARD	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"				
	SP	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 3"	10' 10"	11' 6"	12' 11"	13' 11"				
	DFL	#2	3' 7"	6' 10"	6' 3"	6' 11"	7' 6"	8' 3"	8' 3"	10' 10"	11' 6"	12' 11"	13' 11"				
16" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
	STUD	#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
	HF	STANDARD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
	SP	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 2"	9' 5"	9' 5"	12' 5"	13' 5"	14' 0"	14' 0"				
	DFL	#2	4' 0"	8' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 5"	12' 5"	13' 5"	14' 0"	14' 0"				
24" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
	STUD	#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
	HF	STANDARD	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
	SP	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 2"	9' 5"	9' 5"	12' 5"	13' 5"	14' 0"	14' 0"				
	DFL	#2	4' 0"	8' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 5"	12' 5"	13' 5"	14' 0"	14' 0"				



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BEARING (6 PER VC DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4" O" OUTLOOKERS WITH 2" O" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 104 NAILS.	
* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES: SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	



REVIEWED  
By Julius Lee at 12:00 pm, Jun 11, 2008

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESI 1-60 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS MANUFACTURING, 5831 DOWNEY DR., SUITE 200, MARIETTA, GA 30067, FOR SAFETY PRACTICES PRIOR TO PERFORMING STRUCTURAL WORK. STRUCTURAL PANELS AND JOINTS SHOULD HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S  
CONS. ENGINEERS P.A.  
1435 SW 4th AVENUE  
DELAND BEACH, FL 33444-8161

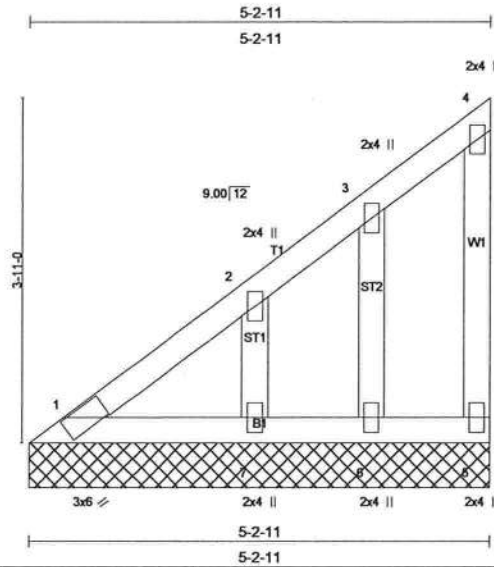
REF ASCE 7-02-CAB13015  
DATE 11/26/03  
DRWG AUTHOR STD GABLE 15 E HT  
-ENG

MAX. TOT. LD. 60 PSF  
MAX. SPACING 24.0"

Job L273732	Truss V1G	Truss Type GABLE	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732055 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:31:02 2008 Page 1



Scale = 1:25.2

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc)	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.05	Vert(LL) n/a - n/a	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.02	Vert(TL) n/a - n/a		
BCLL 10.0	Rep Stress Incr YES	WB 0.04	Horz(TL) 0.00 n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 27 lb	

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-2-11 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	

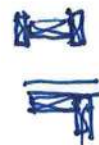
**REACTIONS** (lb/size) 1=56/5-2-11, 5=33/5-2-11, 6=71/5-2-11, 7=138/5-2-11  
 Max Horz 1=161(load case 6)  
 Max Uplift 5=-30(load case 6), 6=-64(load case 6), 7=-124(load case 6)  
 Max Grav 1=70(load case 6), 5=33(load case 1), 6=71(load case 1), 7=138(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=-183/40, 2-3=-82/14, 3-4=-28/13, 4-5=-28/37  
 BOT CHORD 1-7=0/0, 6-7=0/0, 5-6=0/0  
 WEBS 3-6=-60/78, 2-7=-116/152

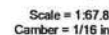
**JOINT STRESS INDEX**  
 1 = 0.08, 2 = 0.08, 3 = 0.04, 4 = 0.02, 5 = 0.02, 6 = 0.04 and 7 = 0.09

**NOTES** (6)  
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60.  
 This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.  
 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 3) Gable requires continuous bottom chord bearing.  
 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 5, 64 lb uplift at joint 6 and 124 lb uplift at joint 7.  
 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard



6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:31:00 2008 Page 1



<b>LOADING (psf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.65	Vert(LL) -0.11 11-12 >999 360	MT20	244/190
TCOL 7.0	Lumber Increase 1.25	BC 0.87	Vert(TL) -0.20 11-12 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.48	Horz(TL) 0.09 7 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)		Weight: 146 lb	

<b>BRACING</b>	
<b>TOP CHORD</b>	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
<b>BOT CHORD</b>	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 12-13.
	T-Brace: 2 X 4 SYP No.3 - 5-10
<b>JOINTS</b>	1 Brace at Jt(s): 10

**FORCES (lb) - Maximum Compression/Maximum Tension**  
 TOP CHORD 1-2=336/178, 2-3=516/328, 3-4=523/350, 4-5=502/407, 5-6=579/307, 1-14=689/287, 6-7=596/292  
 BOT CHORD 13-14=275/273, 12-13=338/172, 2-12=177/190, 11-12=234/466, 10-11=80/374, 8-10=70/54, 8-9=0/0, 7-8=117/11  
 WEBS 3-12=223/204, 3-11=187/229, 4-11=359/403, 5-11=141/230, 1-13=198/470, 6-8=79/332

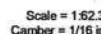
JOINT STRESS INDEX  
1 = 0.44, 2 = 0.47, 3 = 0.29, 4 = 0.29, 5 = 0.49, 6 = 0.51, 7 = 0.82, 8 = 0.60, 10 = 0.86, 11 = 0.68, 12 = 0.56, 13 = 0.73 and 14 = 0.34

NOTES (6)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust);  $h=20ft$ ;  $TCDF=4.2psf$ ;  $BCDL=3.0psf$ ; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 14 and 110 lb uplift at joint 7.
- 6) Truss Design Engineer, Julius Lee, P.E. Florida P.E. License No. 34869. Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:58 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	0.24 11-13	>429	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL)	-0.18 11-13	>573	240		
BCLL 10.0	Rep Stress Incr	NO	WB 0.41	Horz(TL)	0.03 11	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)					Weight: 181 lb	

TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
	B2 2 X 4 SYP No.3, B4 2 X 4 SYP No.3		6-0-0 oc bracing: 17-18.
WEBS	2 X 4 SYP No.3	T-Brace:	2 X 4 SYP No.3 - 8-13
OTHERS	2 X 4 SYP No.3		

REACTIONS (lb/size) 11=552/0-4-0, 18=212/1-4-0, 16=723/1-4-0, 13=436/0-10-0, 17=194/1-4-0  
Max Horz 18=316(load case 5)  
Max Uplift 11=412(load case 7), 18=566(load case 4), 16=431(load case 6), 13=352(load case 7), 17=471(load case 5)  
Max Grav 11=552(load case 1), 18=619(load case 5), 16=723(load case 1), 13=437(load case 11), 17=409(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=4/56, 2-3=529/227, 3-4=149/136, 4-5=137/225, 5-6=316/230, 6-7=158/171, 7-8=226/228, 8-9=313/216, 9-10=500/322, 10-11=555/327, 11-12=4/58, 2-18=545/474  
 BOT CHORD 17-18=265/268, 16-17=0/0, 3-16=270/256, 15-16=163/246, 14-15=93/244, 13-14=71/92, 8-13=198/132, 11-13=183/401  
 WEBS 9-13=266/348, 2-17=355/459, 6-15=102/101, 5-16=129/223, 5-16=533/253

JOINT STRESS INDEX  
2 = 1.00, 3 = 0.51, 4 = 0.00, 5 = 0.29, 6 = 0.34, 7 = 0.54, 8 = 0.53, 9 = 0.34, 10 = 0.00, 10 = 0.28, 10 = 0.21, 11 = 0.55, 13 = 0.71, 14 = 0.60, 15 = 0.44, 16 = 0.64, 17 = 0.49, 18 = 0.47, 19 = 0.74, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.42, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.64, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34 and 41 = 0.34

NOTES (11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust);  $h=20ft$ ;  $TCOL=4.2psf$ ;  $BCDL=3.0psf$ ; Category II; Exp B; enclosed, MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 16 considers parallel to grain value using ANSITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 412 lb uplift at joint 11, 566 lb uplift at joint 18, 431 lb uplift at joint 16, 352 lb uplift at joint 13 and 471 lb uplift at joint 17.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard  
1) Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (plf)  
Vert: 1-2=64/(F=-10), 2-7=64/(F=-10), 7-12=64/(F=-10), 17-18=10, 14-16=10, 11-13=10

Job L273732	Truss T25G	Truss Type GABLE	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732049 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:56 2008 Page 1

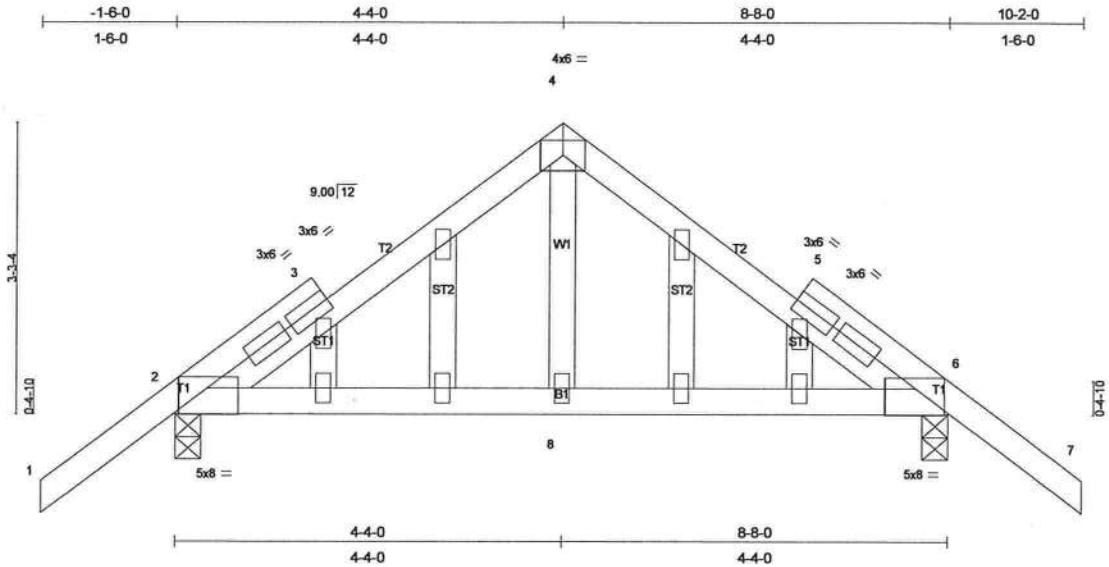


Plate Offsets (X,Y): [2:0-3-13,Edge], [6:0-3-13,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.24	Vert(LL)	0.02	2-8	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(TL)	-0.01	2-8	>999	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.05	Horz(TL)	0.00	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 51 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3  
OTHERS 2 X 4 SYP No.3

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS

(lb/size) 2=415/0-3-8, 6=415/0-3-8  
Max Horz 2=-108(load case 4)  
Max Uplift 2=-369(load case 6), 6=-369(load case 7)

#### FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6/57, 2-3=-356/430, 3-4=-291/427, 4-5=-291/427, 5-6=-356/430, 6-7=-6/57  
BOT CHORD 2-8=-209/233, 6-8=-209/233  
WEBS 4-8=-257/139

#### JOINT STRESS INDEX

2 = 0.58, 3 = 0.00, 3 = 0.25, 3 = 0.25, 4 = 0.45, 5 = 0.00, 5 = 0.25, 5 = 0.25, 6 = 0.58, 8 = 0.10, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00 and 16 = 0.00

#### NOTES (10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS: gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 1-4-0 oc.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 2 and 369 lb uplift at joint 6.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

#### LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (plf)  
Vert: 2-6=-10, 1-4=-64(F=-10), 4-7=-64(F=-10)

Job <b>L273732</b>	Truss <b>T24</b>	Truss Type <b>HIP</b>	Qty <b>1</b>	Ply <b>2</b>	ADAMS FRAMING - ROCCO <b>L273732047</b> Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:54 2008 Page 1		

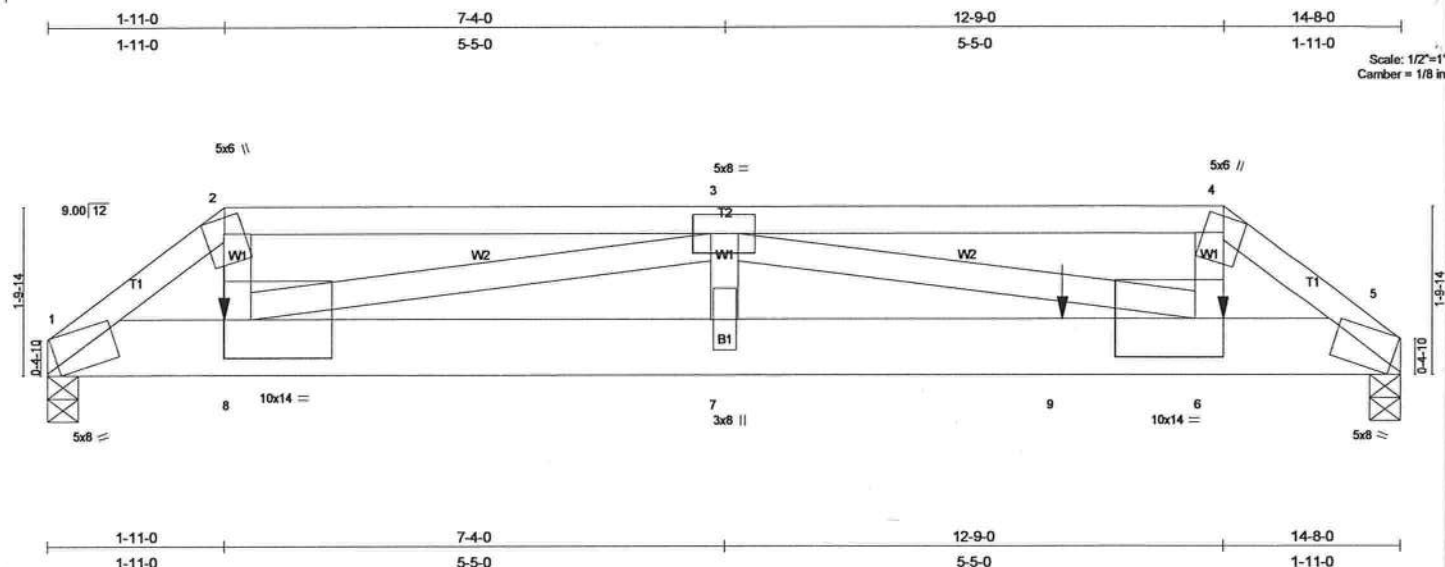


Plate Offsets (X,Y): [6-0-3-8,0-5-0], [8-0-3-8,0-5-0]					
<b>LOADING</b> (psf)	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	2-0-0	TC 0.36	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.59	Vert(LL) -0.16 6-7 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.75	Vert(TL) -0.30 6-7 >570 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.05 5 n/a n/a		
	Code FBC2004/TPI2002			Weight: 176 lb	

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-7-4 oc purlins.
BOT CHORD 2 X 8 SYP 2400F 2.0E	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

**REACTIONS** (lb/size) 1=4252/0-4-0, 5=4063/0-4-0  
Max Horz 1=-38(load case 3)  
Max Uplift 1=-1534(load case 4), 5=-1465(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-6979/2524, 2-3=-5987/2191, 3-4=-6883/2518, 4-5=-8048/2914  
BOT CHORD 1-8=-2043/5565, 7-8=-4253/11666, 7-9=-4253/11666, 6-9=-4253/11666, 5-6=-2311/6388  
WEBS 2-8=-1402/3938, 3-8=-5884/2164, 3-7=-874/2546, 3-6=-4957/1825, 4-6=-1653/4628

**JOINT STRESS INDEX**  
1 = 0.71, 2 = 0.95, 3 = 0.71, 4 = 0.95, 5 = 0.71, 6 = 0.71, 7 = 0.41 and 8 = 0.71

- NOTES** (13)
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2127 lb down and 776 lb up at 11-0-0, and 17 lb down and 12 lb up at 12-9-0, and 17 lb down and 12 lb up at 1-11-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
  - 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.  
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.  
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
  - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2127 lb down and 776 lb up at 11-0-0, and 17 lb down and 12 lb up at 12-9-0, and 17 lb down and 12 lb up at 1-11-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
  - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
  - Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
  - Provide adequate drainage to prevent water ponding.
  - \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1534 lb uplift at joint 1 and 1465 lb uplift at joint 5.
  - Girder carries tie-in span(s): 32-6-0 from 0-0-0 to 11-0-0
  - Girder carries hip end with 1-11-0 right side setback, 1-11-0 left side setback, and 2-4-0 end setback.
  - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2127 lb down and 776 lb up at 11-0-0, and 17 lb down and 12 lb up at 12-9-0, and 17 lb down and 12 lb up at 1-11-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
  - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

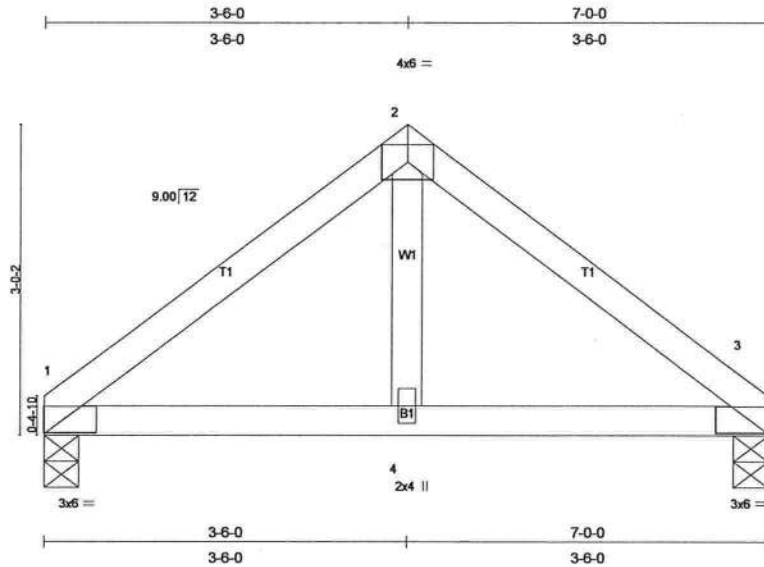
**LOAD CASE(S)** Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (plf)  
Vert: 1-2=-54, 2-4=-55(F=-1), 4-5=-54, 1-8=-493(F=-483), 8-9=-493(F=-483), 6-9=-10(F=-0), 5-6=-10  
Concentrated Loads (lb)  
Vert: 8=-17(F) 6=-17(F) 9=-2127(F)

Job L273732	Truss T23	Truss Type COMMON	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732045 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:52 2008 Page 1



Scale = 1:21.5

Plate Offsets (X,Y): [1:0-3-13,0-1-8], [3:0-3-13,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.09	Vert(LL)	-0.00	1-4	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.08	Vert(TL)	-0.01	1-4	>999	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.04	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 28 lb	

**LUMBER**  
TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3

**BRACING**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 1=216/0-4-0, 3=216/0-4-0  
Max Horz 1=-75(load case 3)  
Max Uplift 1=-43(load case 5), 3=-43(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-261/65, 2-3=-261/65  
BOT CHORD 1-4=-20/165, 3-4=-20/165  
WEBS 2-4=0/118

**JOINT STRESS INDEX**  
1 = 0.27, 2 = 0.15, 3 = 0.27 and 4 = 0.09

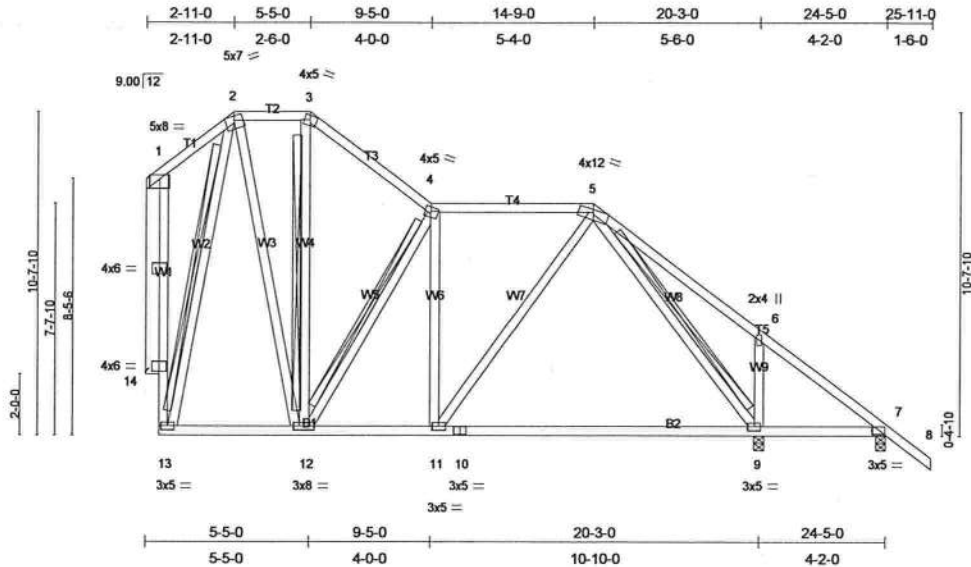
**NOTES** (8)  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.  
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi  
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 1 and 43 lb uplift at joint 3.  
6) Girder carries tie-in span(s): 2-4-0 from 0-0-0 to 7-0-0  
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).  
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard  
1) Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (plf)  
Vert: 1-2=-54, 2-3=-54, 1-3=-11(F=-1)

Job L273732	Truss T21	Truss Type SPECIAL	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732043 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:51 2008 Page 1



Scale = 1/32  
Camber = 3/16 in

Plate Offsets (X,Y): [7:0-5-3,0-0-6]

<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc)	<b>l/defl</b>	<b>L/d</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plates Increase 1.25	TC 0.26	Vert(LL) -0.23 9-11	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.49	Vert(TL) -0.40 9-11	>585	240		
BCLL 10.0	Rep Stress Incr YES	WB 0.53	Horz(TL) 0.01 14	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)					
						Weight: 208 lb	

#### LUMBER

TOP CHORD 2 X 4 SYP No.2  
BOT CHORD 2 X 4 SYP No.2  
WEBS 2 X 4 SYP No.3  
OTHERS 2 X 6 SYP No.1D

#### BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 2-3, 4-5.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
WEBS T-Brace: 2 X 4 SYP No.3 - 3-12, 4-12, 5-9, 2-13  
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.  
Brace must cover 90% of web length.

#### REACTIONS

(lb/size) 9=751/0-4-0, 7=226/0-4-0, 14=634/Mechanical  
Max Horz 9=-348(load case 7)  
Max Uplift 9=-270(load case 7), 7=-98(load case 5), 14=-202(load case 7)  
Max Grav 9=751(load case 1), 7=226(load case 11), 14=634(load case 1)

#### FORCES

(lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-59/69, 2-3=-219/209, 3-4=-346/192, 4-5=-436/219, 5-6=-145/79, 6-7=-164/54, 7-8=0/49, 13-14=-275/547, 1-14=-88/88  
BOT CHORD 12-13=-68/123, 11-12=-269/437, 10-11=-293/347, 9-10=-293/347, 7-9=-12/248  
WEBS 2-12=-267/442, 3-12=-56/59, 4-12=-443/288, 4-11=-34/111, 5-11=-20/202, 5-9=-499/170, 6-9=-257/318, 2-13=-530/295

#### JOINT STRESS INDEX

1 = 0.16, 2 = 0.32, 3 = 0.35, 4 = 0.66, 5 = 0.62, 6 = 0.34, 7 = 0.64, 9 = 0.41, 10 = 0.52, 11 = 0.42, 12 = 0.61, 13 = 0.72, 14 = 0.00, 14 = 0.26 and 14 = 0.26

#### NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 270 lb uplift at joint 9, 98 lb uplift at joint 7 and 202 lb uplift at joint 14.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job <b>L273732</b>	Truss <b>T19</b>	Truss Type <b>SPECIAL</b>	Qty <b>1</b>	Ply <b>1</b>	<b>ADAMS FRAMING - ROCCO</b> <b>L273732041</b> Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:48 2008 Page 1		

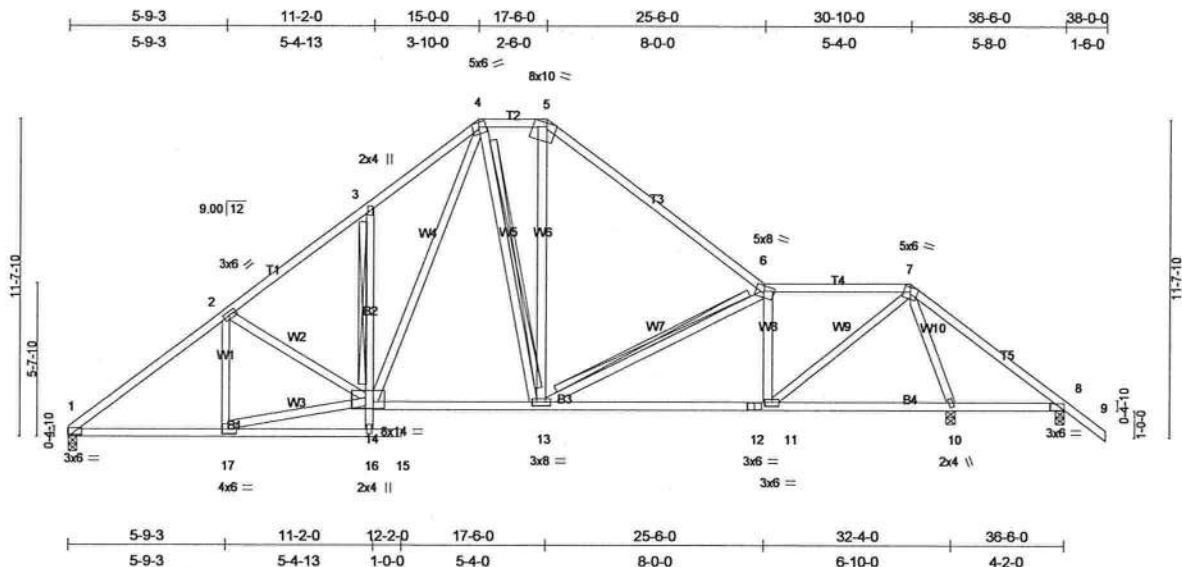


Plate Offsets (X,Y): [1:0-6-3,0-0-10], [5:0-3-14,Edge], [8:0-3-13,0-1-8], [14:0-4-11,0-2-15]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.51	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.34	Vert(LL) -0.10 11-13 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.99	Vert(TL) -0.21 11-13 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.05 10 n/a n/a		
	Code FBC2004/TP12002			Weight: 240 lb	

<b>LUMBER</b>	<b>BRACING</b>
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-0-8 oc purlins, except 2-0-0 oc purlins (5-8-5 max.): 4-5, 6-7.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
B2 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 3-14
WEBS 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 4-13, 6-13
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.

**REACTIONS** (lb/size) 1=985/0-4-0, 10=1632/0-4-0, 8=-201/0-4-0  
Max Horz 1=-308(load case 4)  
Max Uplift 1=-192(load case 6), 10=-327(load case 7), 8=-227(load case 10)  
Max Grav 1=985(load case 1), 10=1632(load case 1), 8=5(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-1445/655, 2-3=-1286/659, 3-4=-1274/850, 4-5=-747/580, 5-6=-1058/578, 6-7=-1018/594, 7-8=-295/788, 8-9=0/49  
BOT CHORD 1-17=-388/1072, 16-17=-25/62, 15-16=0/0, 14-16=0/111, 3-14=-262/289, 13-14=-142/730, 12-13=-349/1044, 11-12=-349/1044, 10-11=-52/127, 8-10=-548/389  
WEBS 2-17=-122/83, 14-17=-369/1028, 2-14=-169/214, 4-14=-460/630, 4-13=-218/239, 5-13=-135/286, 6-13=-378/328, 6-11=-777/446, 7-11=-601/1316, 7-10=-1641/775

**JOINT STRESS INDEX**  
1 = 0.71, 2 = 0.44, 3 = 0.34, 4 = 0.34, 5 = 0.77, 6 = 0.91, 7 = 0.51, 8 = 0.65, 10 = 0.63, 11 = 0.79, 12 = 0.43, 13 = 0.61, 14 = 0.17, 16 = 0.62 and 17 = 0.47

- NOTES** (8)
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - Provide adequate drainage to prevent water ponding.
  - \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 1, 327 lb uplift at joint 10 and 227 lb uplift at joint 8.
  - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
  - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

**LOAD CASE(S)** Standard

Job <b>L273732</b>	Truss <b>T17</b>	Truss Type <b>SPECIAL</b>	Qty <b>1</b>	Ply <b>1</b>	ADAMS FRAMING - ROCCO <b>L273732039</b> Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:45 2008 Page 1		

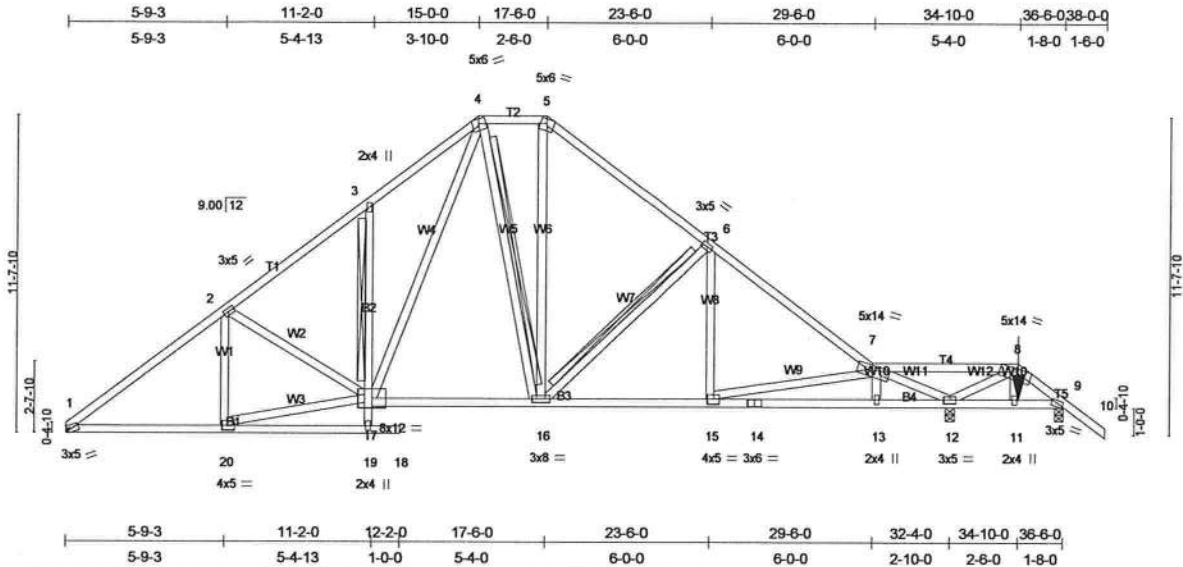


Plate Offsets (X,Y): [1:0-0-13,Edge], [8:0-5-14,Edge], [9:0-0-13,Edge]					
LOADING (psf)	SPACING	2:0-0	CSI	DEFL	in (loc)
TCLL 20.0	Plates Increase	1.25	TC 0.47	Vert(LL)	-0.08 18 >999 360
TCDL 7.0	Lumber Increase	1.25	BC 0.34	Vert(TL)	-0.16 16-17 >999 240
BCLL 10.0	Rep Stress Incr	NO	WB 0.64	Horz(TL)	0.06 12 n/a n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)		
			PLATES		GRIP
			MT20		244/190
			Weight: 245 lb		

<b>LUMBER</b>		<b>BRACING</b>	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-0-10 oc purlins, except 2-0-0 oc purlins (6-0-0 max.); 4-5, 7-8.
BOT CHORD	2 X 4 SYP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 11-12,9-11.
WEBS	2 X 4 SYP No.3	WEBS	T-Brace: 2 X 4 SYP No.3 - 3-17 T-Brace: 2 X 4 SYP No.3 - 4-16, 6-16 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance. Brace must cover 90% of web length.

<b>REACTIONS</b> (lb/size)	
1=978/Mechanical, 12=1691/0-4-0, 9=-233/0-4-0	
Max Horz 1=-308(load case 3)	
Max Uplift 1=-192(load case 5), 12=-378(load case 6), 9=-233(load case 1)	
Max Grav 1=978(load case 1), 12=1691(load case 1), 9=1(load case 5)	

<b>FORCES</b> (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-1436/273, 2-3=-1275/292, 3-4=-1258/428, 4-5=-721/254, 5-6=-999/276, 6-7=-1266/241, 7-8=-260/1424, 8-9=-83/629, 9-10=0/49
BOT CHORD	1-20=-286/1066, 19-20=-20/52, 18-19=0/0, 17-19=0/110, 3-17=-254/195, 16-17=-118/721, 15-16=-94/939, 14-15=-102/790, 13-14=-102/790, 12-13=-97/789, 11-12=-424/136, 9-11=-414/135
WEBS	2-20=-122/74, 17-20=-270/1032, 2-17=-170/146, 4-17=-298/629, 4-16=-173/178, 5-16=-101/313, 6-16=-322/204, 6-15=0/163, 7-15=-6/181, 7-13=-7/116, 7-12=-2443/501, 8-12=-1132/250, 8-11=-92/12

<b>JOINT STRESS INDEX</b>	
1 = 0.79, 2 = 0.52, 3 = 0.34, 4 = 0.33, 5 = 0.40, 6 = 0.52, 7 = 0.88, 8 = 0.78, 9 = 0.73, 11 = 0.34, 12 = 0.80, 13 = 0.34, 14 = 0.30, 15 = 0.32, 16 = 0.61, 17 = 0.28, 19 = 0.61 and 20 = 0.53	

<b>NOTES</b> (11-12)	
1) Unbalanced roof live loads have been considered for this design.	
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.	
3) Provide adequate drainage to prevent water ponding.	
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.	
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi	
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 1, 378 lb uplift at joint 12 and 233 lb uplift at joint 9.	
7) Girder carries hip end with 1-8-0 right side setback, 29-6-0 left side setback, and 2-4-0 end setback.	
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.	
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 15 lb down and 10 lb up at 34-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.	
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).	
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435	
12) Use Simpson HTU26 to attach Truss to Carrying member	

<b>LOAD CASE(S)</b> Standard	
1) Regular: Lumber Increase=1.25, Plate Increase=1.25	
Uniform Loads (plf)	
Vert: 1-4=-54, 4-5=-54, 5-7=-54, 7-8=-55(F=-1), 8-10=-54, 1-19=-10, 18-19=-10, 13-17=-10, 11-13=-10(F=-0), 9-11=-10	
Concentrated Loads (lb)	
Vert: 11=-15(F)	

Job L273732	Truss T15	Truss Type SPECIAL	Qty 1	Ply 1	ADAMS FRAMING - ROCCO L273732037 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Jul 23 15:30:43 2008 Page 1

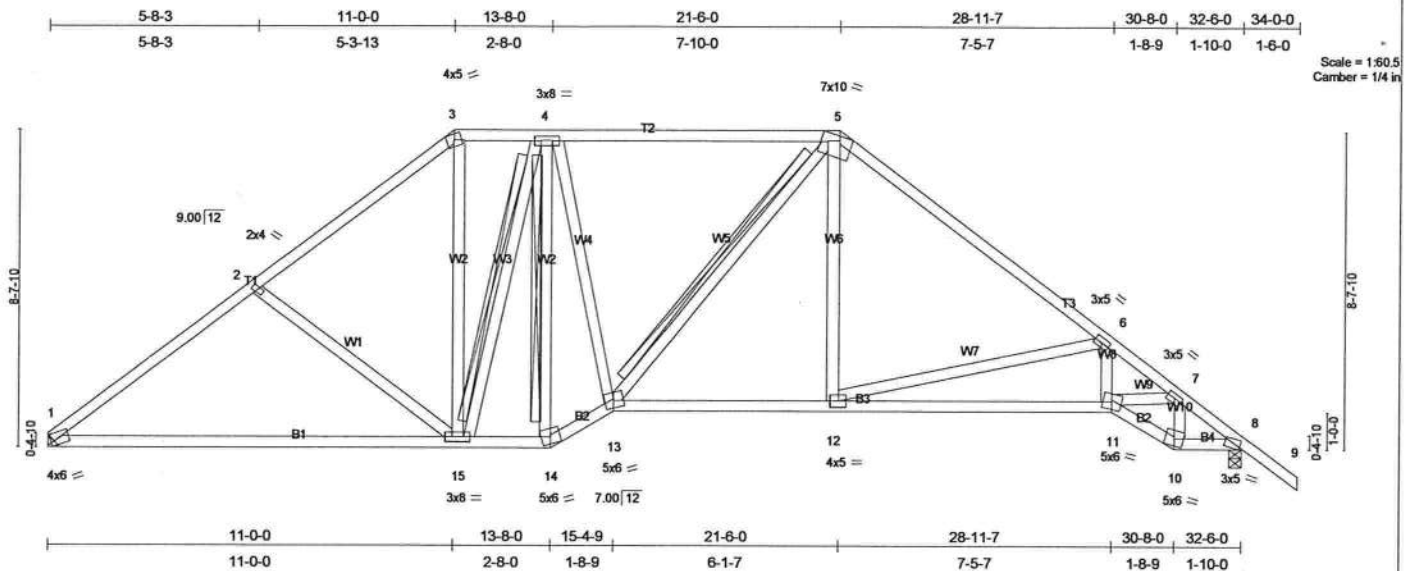


Plate Offsets (X,Y): [1:0-1-2,Edge], [5:0-3-14,Edge], [8:0-0-13,Edge]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d			PLATES GRIP		
TCLL	20.0	Plates Increase 1.25		TC	0.45	Vert(LL)	-0.29	1-15	>999	360	MT20 244/190
TCDL	7.0	Lumber Increase 1.25		BC	0.67	Vert(TL)	-0.55	1-15	>706	240	
BCLL	10.0	Rep Stress Incr YES		WB	0.82	Horz(TL)	0.12	8	n/a	n/a	
BCDL	5.0	Code FBC2004/TP12002		(Matrix)							
						Weight: 208 lb					

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-10-12 oc purlins.
BOT CHORD 2 X 4 SYP No.2	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
WEBS 2 X 4 SYP No.3	9-8-1 oc bracing: 1-15
	7-8-0 oc bracing: 11-12.
	T-Brace: 2 X 4 SYP No.3 - 4-15, 4-14, 5-13
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.
	,with 4in minimum end distance.
	Brace must cover 90% of web length.

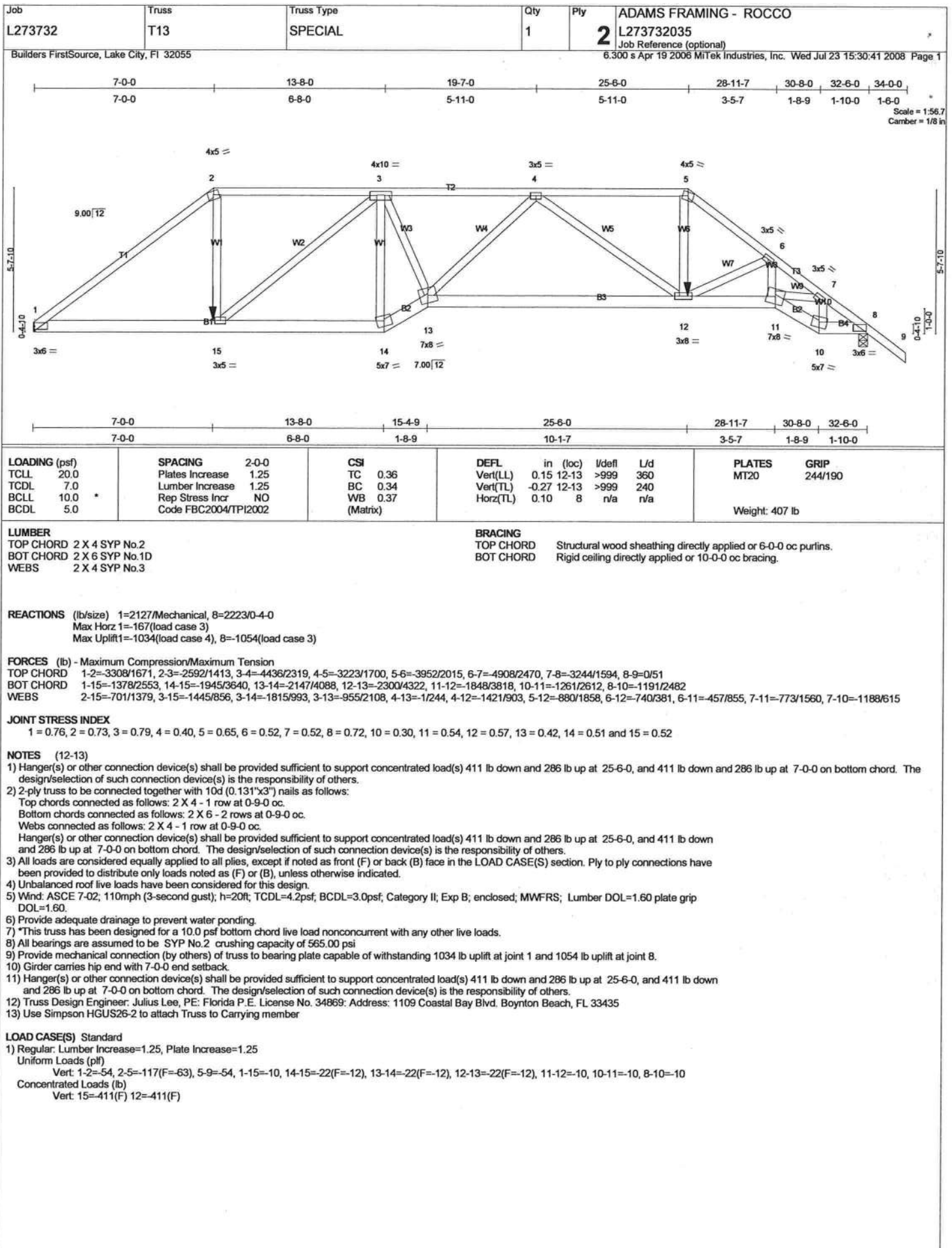
REACTIONS (lb/size) 1=1028/Mechanical, 8=1122/0-4-0  
Max Horz 1=-251(load case 4)  
Max Uplift 1=-188(load case 6), 8=-261(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension  
TOP CHORD 1-2=-1464/722, 2-3=-1233/675, 3-4=-915/616, 4-5=-1109/676, 5-6=-1418/686, 6-7=-2292/977, 7-8=-1434/604, 8-9=0/49  
BOT CHORD 1-15=-402/1114, 14-15=-262/988, 13-14=-292/1129, 12-13=-225/1054, 11-12=-676/1820, 10-11=-325/1121, 8-10=-308/1025  
WEBS 2-15=-266/281, 3-15=-268/529, 4-15=-380/266, 4-14=-570/116, 4-13=-70/533, 5-13=-155/205, 5-12=-72/361, 6-12=-794/465, 6-11=-51/497, 7-11=-397/908, 7-10=-574/182

JOINT STRESS INDEX  
1 = 0.77, 2 = 0.34, 3 = 0.61, 4 = 0.62, 5 = 0.91, 6 = 0.52, 7 = 0.52, 8 = 0.77, 10 = 0.29, 11 = 0.69, 12 = 0.33, 13 = 0.37, 14 = 0.30 and 15 = 0.61

- NOTES (7-8)
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
  - Provide adequate drainage to prevent water ponding.
  - \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 1 and 261 lb uplift at joint 8.
  - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
  - Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



**FEEs:**

ROAD IMPACT FEE 10100003632400	<u>\$1,046.00</u>	CODE	<u>210</u>	UNIT	<u>1</u>
EMS IMPACT FEE 10300003632210	<u>\$29.88</u>				
FIRE PROTECTION IMPACT FEE 10200003632220	<u>\$78.63</u>				
CORRECTIONS IMPACT FEE 00100003632200	<u>\$409.16</u>				
SCHOOL IMPACT FEE 00100003632900	<u>\$1,500.00</u>				
<b>TOTAL FEES CHARGED</b>	<u>\$3,063.67</u>	CHECK NUMBER	<u>1519</u>		