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WIND ANALYSIS -- 120MPH Wind Velocity or as interpolated
2023 8th edition Florida Building Code
*Calculations as per Section **1609ASCE 7-22***

Prepared By
James Zaleski PE 51544

FOR:
TJ PIZZAGALLI
245 SW COLONY GLEN
LAKE CITY, FL 32024

Prepared by (print legibly): James Zaleski
Design Professional FL Lic. #: 51544

Importance factor: 1.0 Building Category: ENCLOSED
Wind Exposure (s): B Risk Category II
Internal Pressure Coefficient +/-.18

Mean Roof Height ***16.47*** **End Zone Length 6.0 feet**

JAMES A. ZALESKI LICENSE No. 51544 STATE OF FLORIDA PROFESSIONAL ENGINEER

_ MAX OVERHANG 1.5 FT MAX

MANUFACTURED TRUSSES TO BE USED
Roof Slope =~~4~~/12 – 7/12

TRUSS SPAN/LOCATION HURRICANE CLIPS
HC MODEL–1 Simpson H-10A IN ALL AREAS

ROOF SHEATHING MATERIAL –7/16" OSB
NAILING – 8D RING SHANK

NAILING PATTERN
EDGES-
6" O.C FIELD – 6" O.C

4" O.C FIRST ROW AT ALL EAVES

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Plan May Be Mirrored at Contractors Option

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Wall Exterior Panel – Sheath with 7/16" OSB
– 2 X 4 STUDS AT 16" O.C. UP TO 10 FEET

– 2 X 4 STUDS AT 12" O.C. UP TO 12 FEET

– 2 X 6 STUDS AT 16" O.C. UP TO 16 FEET

ALL WALLS OVER 10 FEET TO HAVE 2 ROWS OF BLOCKING

SEE ATTACHED DETAILS

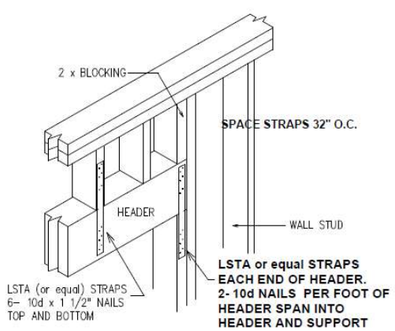
POSTS USE SIMPSON ABU BASE WITH 2-LSTA24 STRAPS AT TOP AND 2 SIMPSON SDWC 15600 SCREWS FROM POST TO BEAM

MIN NAIL PENETRATION – 1-1/2"
Nail Type 8D
Edge Nail Spacing 4" o.c
Intermediate Nail Spacing 8" o.c
SIMPSON SDWC15600 SCREWS AT THE TOP OF STUDS AND SIMPSON SDWC15450 SCREWS AT THE BOTTOM OF STUDS AT ALL CORNERS AND 48" O.C

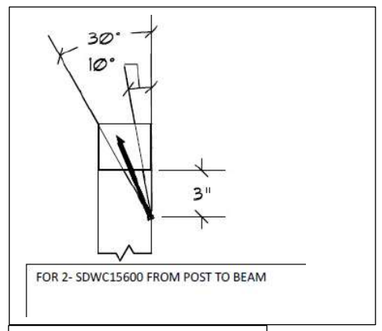
SIMPSON SPH STRAPS MAY BE USED IN LIEU OF SCREWS

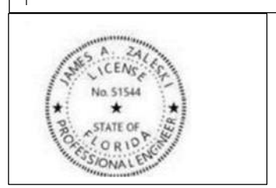
BEAM TO WALL/CORNER CONNECTION – POCKET AND NAIL INTO WALL W/ (10) 16 PENNY NAILS, STRAP W/ SIMPSON H7Z.

10" 'J' BOLT MINIMUM 48" O.C. w/
3 GA. x 3" x 3" BEARING PLATE



HEADER CONN.





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USE (3) 2x HEADERS WITH 2x6 WALLS AND (2) 2x HEADERS WITH 2x4 WALLS.
VERIFY HEADER SIZE W/ OPENING FRAMING & HEADER SCHEDULE ON THIS SHEET.
ASSEMBLE ALL HEADERS WITH 16D NAILS STAGGERED 6" O.C. W/ 2" EDGE DISTANCE
ALL HEADERS SHALL BE 2x12 U.O.N.
ATTACH ALL LVL BEAMS W/ SIMPSON SDWS 0.220 SCREWS (OR EQUAL) STAGGERED W/ 1–1/2" MIN. EDGE DISTANCE.

- (2) ROWS @ 24" O.C. < 10" LVL
- (3) ROWS @ 24" O.C. > 10" LVL
- 2 & 3 PLY – 3–1/2" LONG STAGGER INSTALLED FROM BOTH OUTER PLIES
- 4 PLY – 6" LONG STAGGER INSTALLED FROM BOTH OUTER PLIES

FOR WINDOWS PLACED WITHIN 3'–0" OF EXTERIOR CORNERS, WALL STUDS SHALL BE SPACED @ 8" O.C.

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COMPONENTS AND CLADDING PRESSURES: (WORST CASE LOADS MAY BE USED)

COMPONENTS AND CLADDING

ZONE per

SEE ATTACHED

MAIN WIND FORCE RESISTING SYSTEMS (MWFRS) (WORST CASE LOADS MAY BE USED)

SEE ATTACHED

All Load Bearing and Shear Walls To be Framed as per FBC

Alternative Hurricane Clips are acceptable as long as they meet the requirements shown

See Attached header schedule

PROVIDE GABLE END BRACING DETAIL, all vaulted or high ceilings shall be balloon framed to the ceiling diaphram.

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NOTES: PLEASE READ & complete all blanks!!!!

1. ***See floor plan for wall bracing locations or circle 100% if structural sheathing is required on all exterior walls, with the nailing pattern indicated above.***
2. ***There are _ _, there are not X interior shear walls, locate interior shear walls on plan.***
3. ***Gable ends required to be sheathed with same material as shear wall? Yes or (No) circle one)***
4. ***Wall sheathing used in lieu of vertical straps: Nailing @ N/A o.c. along top & bottom plates***
5. ***Provide detail for 2 story bldgs showing continuous load path between 2nd floor stud & 1st floor studs.***
6. ***Provide additional information for column base & column/beam connection if required for porches.***
7. ***Provide calculations or documentation to substantiate method used as an attachment to this form(SEE PLANS)***

Instructions:

1. The form should be completed & signed, sealed & dated by a Fla. licensed engineer or architect.
2. Since more than one methodology for determination of wind forces is permitted under Section 1609ASCE7-22, to comply with State Building Codes a space has been provided to indicate method used.
3. Wind Analysis Forms submitted & permitted to be used as Master Plans will be for identical plans only, minor deviations such as door swings. Any deviation from the exterior form, opening sizes or locations will not be permitted unless noted by the design professional.

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ALL JOIST HANGERS SHALL BE SIMPSON LU26 HANGER OR EQUAL.

DECKING TO BE 5/4 P.T. WOOD OR BETTER USE 2– 1/4"x 2" DECK SCREWS PER JOIST OR (2– 10d x 3" RING SHANK NAILS)

ABOVE IS MINIMUM REQUIREMENTS FOR THE RESISTANCE OF WIND PRESSURES

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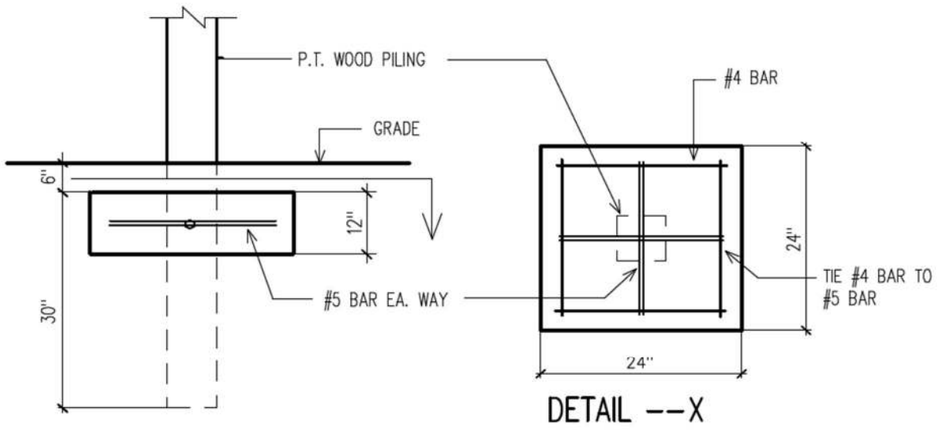
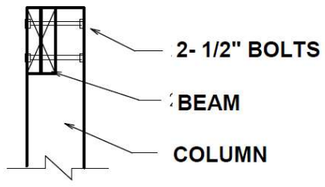


COLUMN BASE DETAIL



BEAM TO COLUMN CONNECTION

TYPICAL DECK CONNECTION DETAILS

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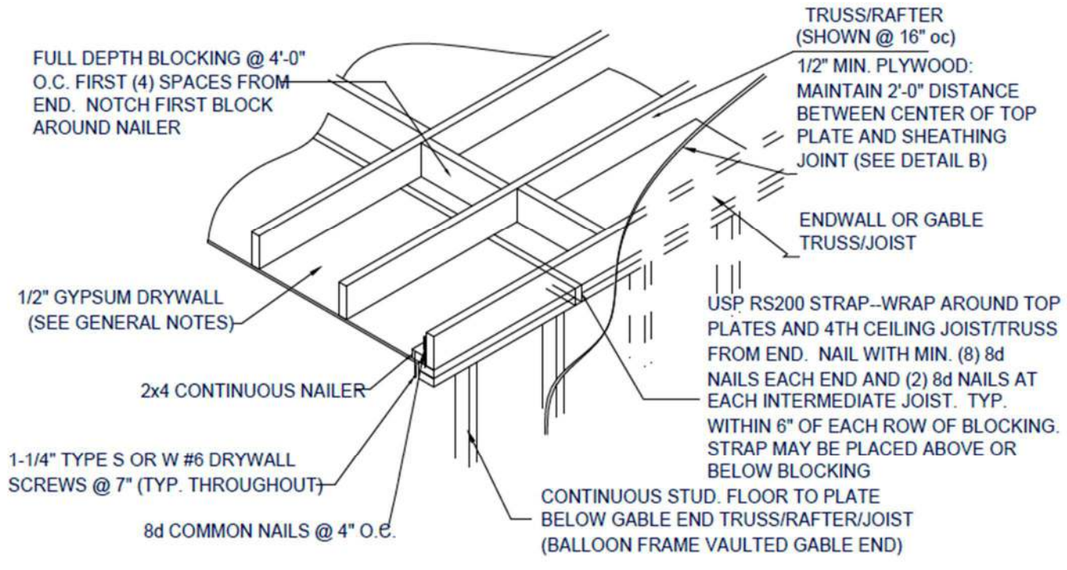

ENDWALL CEILING CONNECTION

Header Table		
Span (FT.)	Header Size	(2 x) cripples per end
0' - 4'-0"	2-2x10 w/7/16" OSB Flitch Plate	1
4'-0" - 9'-4"	2-2x10 w/7/16" OSB Flitch Plate	2
9'-4" - 12'-0"	2-2x10 w/7/16" OSB Flitch Plate	3
12'-0" - 15'-4"	3 1/2" X 11 7/8" LVL (or equal)	3

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MecaWind v2502

Calculations Prepared by:
JAMES ZALESKI P.E 51544
2305 HAVERHILL RD
TALLAHASSEE, FL, 32312
Date: Apr 28, 2025
File Location: Current Project Not Saved

General:

Reference Abbreviations: T: Table, F: Figure, E: Equation, §: Section

Wind Load Standard	=	ASCE 7-22	Basic Wind Speed	=	120.0 mph
Exposure Classification	=	B	Risk Category	=	II
Structure Type	=	Building	Design Basis for Wind Pressures	=	ASD
MWFRS Analysis Method	=	Ch 27	C&C Analysis Method	=	Ch 30 Pt 1
Dynamic Type of Structure	=	Rigid	Show Advanced Options	=	False

Building:

Roof	=	Roof Type	=	Gabled	Encl	=	Enclosure Classification	=	Enclosed
Help	=	Help on Building Roof Type	=	Help	R_{Ht}	=	Ridge Height	=	23.933 ft
E_{Ht}	=	Eave Height	=	9.000 ft	W	=	Building Width	=	51.200 ft
L	=	Building Length	=	50.250 ft	Pitch	=	Pitch of Roof	=	6.61 :12
θ	=	Slope of Roof	=	28.857 °	OH	=	Overhang Configuration	=	All Soffit
Par	=	Parapet	=	None	z_i	=	Highest Opening Elevation	=	0.0000 ft
HT_{over}	=	Override Mean Roof Height	=	False	Ht_{man}	=	Mean Roof Height	=	16.467 ft
RA_{over}	=	Override Roof Area	=	False	GC_{pi_o}	=	Override GC_{pi} value	=	False
IsElev	=	Building is Elevated	=	False		=			

Exposure Constants [T:26.11-1]:

α = 3-s Gust-speed exponent	=	7.500	Z_g = Nominal Ht of Boundary Layer	=	3280.000 ft
â = Reciprocal of α	=	0.133	b = 3 sec gust speed factor	=	0.840
α_m = Mean hourly Wind-Speed Exponent	=	0.222	b_m = Mean hourly Windspeed Exponent	=	0.470
c = Turbulence Intensity Factor	=	0.300	ε = Integral Length Scale Exponent	=	0.3333

Overhang Inputs:

Std	=	Overhangs on all sides are the same	=	True
OHType	=	Type of Roof Wall Intersections	=	Soffit
OH	=	Overhang of Roof Beyond Wall	=	1.500 ft

Main Wind Force Resisting System (MWFRS) Wind Calculations per Ch 27

REVIEWED BY JAMES ZALESKI P.E.

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TALLAHASSEE, FL 323132

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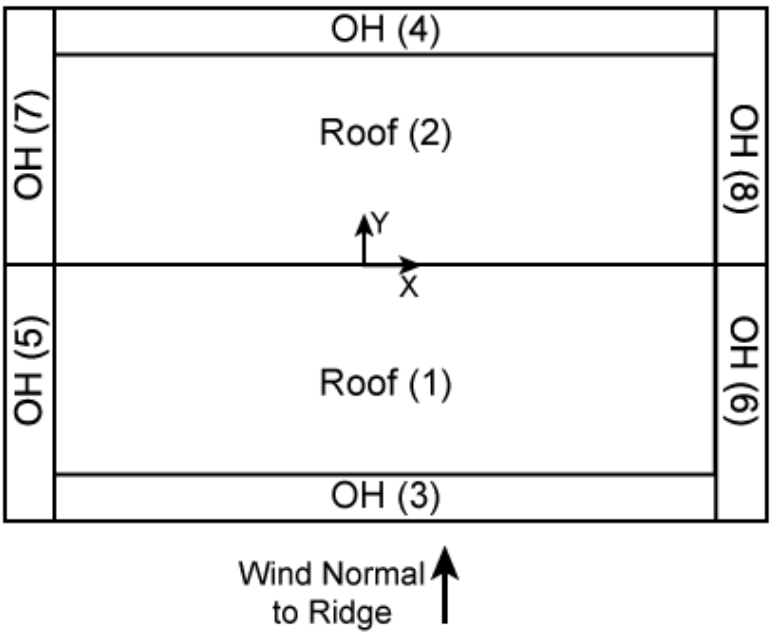

h = Mean structure height	= 16.467 ft	$K_h = 2.41 \cdot (Z/Z_g)^{2/\alpha}$ T:26.10-1	= 0.587
K_{zt} = No Topographic Feature	= 1.000	K_d = Directionality Factor T:26.6-1	= 0.85
GC_{pi} = ± Internal Press Coef T:26.13-1	= ±0.18	LF = ASD Load Factor	= 0.60
K_e = Ground Elev Factor T:26.10-1	= 1.000	$q_h = .00256 \cdot K_h \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF$ E:26.10-1	= 12.99 psf
q_{in} = Negative Internal Pressure: q_h	= 12.99 psf	q_{ip} = For $+GC_{pi}$ use q_h	= 12.99 psf
A_{roof} = Roof Area	= 3,295.34 ft²		

MWFRS Wind Loads [Normal to Ridge]

h = Mean Roof Height of Building	= 16.4667 ft	R_{Ht} = Ridge Height Of Roof	= 23.9333 ft
B = Building Width Normal To Wind	= 50.2500 ft	L = Building Width Parallel To Wind	= 51.2000 ft
L/B = Ratio: L/B	= 1.019	h/L = Ratio: h/L	= 0.322
θ = Slope of Roof	= 28.86 °	G = Gust Factor: Min(G_1, G_2)	= 0.850
Cp_{WW} = Windward Wall Coefficient	= 0.800	Cp_{LW} = Leeward Wall Coefficient	= -0.496
Cp_{SW} = Side Wall Coefficient	= -0.700		

Wall Wind Pressures [Normal to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC_{pi}	q_i psf	K_z	K_{zt}	q_z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
9.000	+0.18	12.99	0.573	1.000	12.67	5.34	-6.64	-8.56	11.98	9.60
9.000	-0.18	12.99	0.573	1.000	12.67	9.31	-2.67	-4.58	11.98	9.60

K_z	*= $2.41 \cdot (Z/Z_g)^{2/\alpha}$*	*K_{zt}*	*= No Topographic Feature*
GC_{pi}	*= +Internal Coef T:26.13-1*	*q_z*	*= $.00256 \cdot K_z \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF$ E:26.10-1*
q_{ip}	*= For $+GC_{pi}$ use q_h*	*q_{in}*	*= Negative Internal Pressure: q_h*
Side	*= $q_h \cdot K_d \cdot G \cdot Cp_{SW} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1*	*Leeward*	*= $q_h \cdot K_d \cdot G \cdot Cp_{LW} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1*
Windward	*= $q_z \cdot K_d \cdot G \cdot Cp_{WW} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1*	*Total*	*= Windward - Leeward*
+Press	*= Pressure Acting Toward Surface*	*-Press*	*= Pressure Acting Away from Surface*
§27.1.5	*= MWFRS Min Wall Pressure = 9.60 psf*		

Roof Wind Pressures [Normal to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC_{pi}	C_{pMin}	C_{pMax}	P_{min} psf	P_{max} psf	P_{min} psf
Overhang	Leeward	7,8	All	All	28.86	N	0	-0.6	-0.6	-5.63	-5.63	4.80
Overhang	Windward	5,6	All	All	28.86	N	0	0.271	-0.207	2.55	-1.94	4.80
Overhang	Leeward	4	All	All	28.86	N	+0.18	-0.6	-0.6	-7.62	-7.62	4.80
Overhang	Windward	3	All	All	28.86	N	+0.18	0.271	-0.207	0.56	-3.93	4.80
Roof	Leeward	2	All	All	28.86	N	+0.18	-0.6	-0.6	-7.62	-7.62	4.80
Roof	Windward	1	All	All	28.86	N	+0.18	0.271	-0.207	0.56	-3.93	4.80
Soffit	Bottom	3	All	All	0.0	N/A	+0.18	0.8	0.8	5.52	5.52	4.80
Overhang	Leeward	4	All	All	28.86	N	-0.18	-0.6	-0.6	-3.64	-3.64	4.80

Overhang	Windward	3	All	All	28.86	N	-0.18	0.271	-0.207	4.53	0.05	4.80
Roof	Leeward	2	All	All	28.86	N	-0.18	-0.6	-0.6	-3.64	-3.64	4.80
Roof	Windward	1	All	All	28.86	N	-0.18	0.271	-0.207	4.53	0.05	4.80
Soffit	Bottom	3	All	All	0.0	N/A	-0.18	0.8	0.8	9.50	9.50	4.80

Roof Pressures based upon Ch 27:

Component = The building component for pressures
Start = Start Dist from Windward Edge
C_{pMin} = Smallest Coefficient Magnitude
P_{min} = $q_h \cdot K_d \cdot G \cdot C_{pMin} - q_{ip} \cdot K_d \cdot GC_{piE:27.3-1}$
GC_{pi} = +Internal Coef T:26.13-1
P_{min} = Min Press projected on vertical plane §27.1.5
§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf
-Press = Pressure Acting Away from Surface

Location = Reference Graphic in Output for Values
End = End Dist from Windward Edge
C_{pMax} = Largest Coefficient Magnitude
P_{max} = $q_h \cdot K_d \cdot G \cdot C_{pMax} - q_{in} \cdot K_d \cdot GC_{piE:27.3-1}$
Basis = P=Parallel to Ridge: N=Normal to Ridge
θ = Roof Slope Relative to Wind
+Press = Pressure Acting Toward Surface

• The smaller uplift pressures due to C_{pMin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

MWFRS Wind Loads [Parallel to Ridge]

h = Mean Roof Height of Building = 16.4667 ft
B = Building Width Normal To Wind = 51.2000 ft
L/B = Ratio: L/B = 0.981
θ = Slope of Roof = 28.86 °
Cp_{WW} = Windward Wall Coefficient = 0.800
Cp_{SW} = Side Wall Coefficient = -0.700

R_{Ht} = Ridge Height Of Roof = 23.9333 ft
L = Building Width Parallel To Wind = 50.2500 ft
h/L = Ratio: h/L = 0.328
G = Gust Factor: Min(G_1, G_2) = 0.850
Cp_{LW} = Leeward Wall Coefficient = -0.500

Wall Wind Pressures [Parallel to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC_{pi}	q_i psf	K_z	K_{zt}	q_z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
23.933	+0.18	12.99	0.649	1.000	14.35	6.31	-6.68	-8.56	12.99	9.60
16.467	+0.18	12.99	0.587	1.000	12.99	5.52	-6.68	-8.56	12.20	9.60
9.000	+0.18	12.99	0.573	1.000	12.67	5.34	-6.68	-8.56	12.02	9.60
23.933	-0.18	12.99	0.649	1.000	14.35	10.28	-2.71	-4.58	12.99	9.60
16.467	-0.18	12.99	0.587	1.000	12.99	9.50	-2.71	-4.58	12.20	9.60
9.000	-0.18	12.99	0.573	1.000	12.67	9.31	-2.71	-4.58	12.02	9.60

K_z = $2.41 \cdot (Z/Z_g)^{2/\alpha}$
GC_{pi} = +Internal Coef T:26.13-1
q_{ip} = For +GC_{pi} use q_h
Side = $q_h \cdot K_d \cdot G \cdot Cp_{SW} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1
Windward = $q_z \cdot K_d \cdot G \cdot Cp_{WW} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1
+Press = Pressure Acting Toward Surface
§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf

K_{zt} = No Topographic Feature
q_z = $.00256 \cdot K_z \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF_{E:26.10-1}$
q_{in} = Negative Internal Pressure: q_h
Leeward = $q_h \cdot K_d \cdot G \cdot Cp_{LW} - q_{ip} \cdot K_d \cdot (GC_{pi+})$ E:27.3-1
Total = Windward - Leeward
-Press = Pressure Acting Away from Surface

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Roof Wind Pressures [Parallel to Ridge]
All wind pressures include a Load Factor (LF) of 0.6

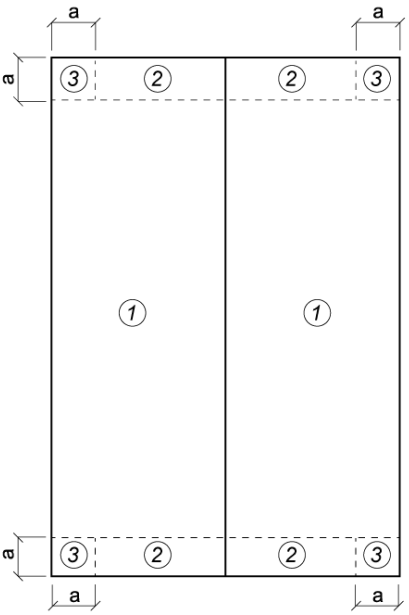
Component	Description	Location	Start ft	End ft	θ °	Basis	GC_{pi}	C_{pMin}	C_{pMax}	P_{min} psf	P_{max} psf	P_{min} psf
Overhang	Overhang 0 to h/2	5,7	0.000	1.500	0.0	P	0	-0.9	-0.18	-8.45	-1.69	4.80
Overhang	Overhang ≥ 2•h	6,8	51.750	53.250	0.0	P	0	-0.3	-0.18	-2.82	-1.69	4.80
Overhang Bottom	Bottom	5,7	All	All	0.0	N/A	0	0.8	0.8	7.51	7.51	4.80
Overhang	Overhang 0 to h	3,4	1.500	16.467	0.0	P	+0.18	-0.9	-0.18	-10.43	-3.68	4.80
Overhang	Overhang h to 2•h	3,4	16.467	32.933	0.0	P	+0.18	-0.5	-0.18	-6.68	-3.68	4.80
Overhang	Overhang ≥ 2•h	3,4	32.933	51.750	0.0	P	+0.18	-0.3	-0.18	-4.80	-3.68	4.80
Roof	Roof 0 to h	1,2	1.500	16.467	0.0	P	+0.18	-0.9	-0.18	-10.43	-3.68	4.80
Roof	Roof h to 2•h	1,2	16.467	32.933	0.0	P	+0.18	-0.5	-0.18	-6.68	-3.68	4.80
Roof	Roof ≥ 2•h	1,2	32.933	51.750	0.0	P	+0.18	-0.3	-0.18	-4.80	-3.68	4.80
Overhang	Overhang 0 to h	3,4	1.500	16.467	0.0	P	-0.18	-0.9	-0.18	-6.46	0.30	4.80
Overhang	Overhang h to 2•h	3,4	16.467	32.933	0.0	P	-0.18	-0.5	-0.18	-2.71	0.30	4.80
Overhang	Overhang ≥ 2•h	3,4	32.933	51.750	0.0	P	-0.18	-0.3	-0.18	-0.83	0.30	4.80
Roof	Roof 0 to h	1,2	1.500	16.467	0.0	P	-0.18	-0.9	-0.18	-6.46	0.30	4.80
Roof	Roof h to 2•h	1,2	16.467	32.933	0.0	P	-0.18	-0.5	-0.18	-2.71	0.30	4.80
Roof	Roof ≥ 2•h	1,2	32.933	51.750	0.0	P	-0.18	-0.3	-0.18	-0.83	0.30	4.80

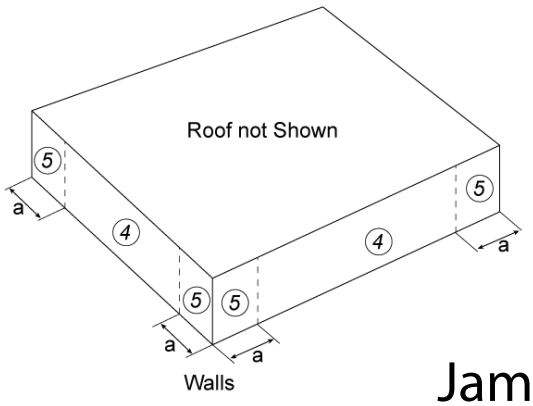
Roof Pressures based upon Ch 27:

Component	= *The building component for pressures*	*Location*	= *Reference Graphic in Output for Values*
Start	= *Start Dist from Windward Edge*	*End*	= *End Dist from Windward Edge*
C_{pMin}	= *Smallest Coefficient Magnitude*	C_{pMax}	= *Largest Coefficient Magnitude*
P_{min}	= $q_h \cdot K_d \cdot G \cdot C_{pMin} - q_{ip} \cdot K_d \cdot GC_{piE:27.3-1}$	P_{max}	= $q_h \cdot K_d \cdot G \cdot C_{pMax} - q_{in} \cdot K_d \cdot GC_{piE:27.3-1}$
GC_{pi}	= *+Internal Coef* $_{T:26.13-1}$	*Basis*	= *P=Parallel to Ridge: N=Normal to Ridge*
P_{min}	= *Min Press projected on vertical plane* $_{§27.1.5}$	θ	= *Roof Slope Relative to Wind*
§27.1.5	= *MWFRS Min Wall Pressure = 9.60 psf*	*+Press*	= *Pressure Acting Toward Surface*
-Press	= *Pressure Acting Away from Surface*		

• The smaller uplift pressures due to C_{pMin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

Components and Cladding (C&C) Wind Loads per Ch 30 Pt 1 Roof & Wall





h = Mean structure height	= 16.467 ft	$K_h = 2.41 \cdot (Z/Z_g)^{2/\alpha}$	= 0.587
K_{zt} = No Topographic Feature	= 1.000	K_d = Directionality Factor $_{T:26.6-1}$	= 0.85
GC_{pi} = ± Internal Press Coef $_{T:26.13-1}$	= ±0.18	LF = ASD Load Factor	= 0.60
K_e = Ground Elev Factor $_{T:26.10-1}$	= 1.000	$q_h = .00256 \cdot K_h \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF_{E:26.10-1}$	= 12.99 psf
θ = Slope of Roof	= 28.86 °	a_1 = Min(0.1•B, 0.4•h)	= 5.025 ft
a = Max(a_1, 0.04•B, 3 ft [0.9 m])	= 5.025 ft		

C&C Wind Roof & Wall Detailed per Ch 30 Pt 1
All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft²	1/3 Rule	Reference	GC_{pi}	GC_{pd}	GC_{pu}	P_{down} psf	P_{uplift} psf
Zone 1	1	1.0000	1.0000	1.00	No	F:30.3-2D	±0.18	0.90	-1.80	11.93	-21.86
Zone 2	2	1.0000	1.0000	1.00	No	F:30.3-2D	±0.18	0.90	-2.00	11.93	-24.07
Zone 3	3	1.0000	1.0000	1.00	No	F:30.3-2D	±0.18	0.90	-2.50	11.93	-29.59
Zone 4	4	1.0000	1.0000	1.00	No	F:30.3-1	±0.18	1.00	-1.10	13.03	-14.13
Zone 5	5	1.0000	1.0000	1.00	No	F:30.3-1	±0.18	1.00	-1.40	13.03	-17.45

GC_{pd}	= *Down (+) External Coefficient*	GC_{pu}	= *Uplift (-) External Coefficient*
P_{down}	= $q_h \cdot K_d \cdot [GC_{pd} - GC_{pi}]$ $_{[E:30.3-1]}$	P_{uplift}	= $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ $_{[E:30.3-1]}$
+Press	= *Pressure Acting Toward Surface*	*-Press*	= *Pressure Acting Away from Surface*
§30.2.2	= *C&C Min Pressure = 9.60 psf*	*Zone*	= *Applicable Zone per Figure*
Width	= *Width of Component*	*Span*	= *Span of Component*
Area	= *Span • Width*	*1/3 Rule*	= *Width limited to Span/3*
GC_{pi}	= *Internal Coef* $_{T:26.13-1}$	*Reference*	= *Applicable Reference from Standard*

C&C Wind Roof & Wall Overhangs Detailed per Ch 30 Pt 4
All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft²	1/3 Rule	Reference	GC_{pi}	GC_{pd}	GC_{pu}	P_{down} psf	P_{uplift} psf
Zone 1_OHS	1_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	±0.18	0.00	-2.80	9.60	-32.91
Zone 2_OHS	2_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	±0.18	0.00	-3.00	9.60	-35.11

Zone 3_OHS	3_OHS	1.0000	1.0000	1.00	No	F:30.3-2D/F:30.3-1	±0.18	0.00	-3.50	9.60	-40.63

GC_{pd}	= Down (+) External Coefficient	GC_{pu}	= Uplift (-) External Coefficient
P_{down}	= $q_h \cdot K_d \cdot [GC_{pd} - GC_{pi}]$ E:30.7-1	P_{uplift}	= $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ E:30.7-1
+Press	= Pressure Acting Toward Surface	-Press	= Pressure Acting Away from Surface
§30.2.2	= C&C Min Pressure = 9.60 psf	Zone	= Applicable Zone per Figure
Width	= Width of Component	Span	= Span of Component
Area	= Span • Width	1/3 Rule	= Width limited to Span/3
GC_{pi}	= Internal Coef T:26.13-1	Reference	= Applicable Reference from Standard
#_OHS	= Roof Zone # on Overhang Soffit	Soffit	= Soffit present so use building GC_{pi}

Warnings & Notes:
Overhang GCp determined from adding applicable roof GCp on top to applicable Wall GCp on bottom

-

C&C Wind Roof & Wall Summary per Ch 30 Pt 1

Zone	Reference	P_{max} A ≤ 10 ft² psf	P_{min} A ≤ 10 ft² psf	P_{max} A = 20 ft² psf	P_{min} A = 20 ft² psf	P_{max} A = 50 ft² psf	P_{min} A = 50 ft² psf
1	F:30.3-2D	11.93	-21.86	10.90	-18.54	9.60	-14.15
2	F:30.3-2D	11.93	-24.07	10.90	-21.52	9.60	-18.14
3	F:30.3-2D	11.93	-29.59	10.90	-25.76	9.60	-20.69
4	F:30.3-1	13.03	-14.13	12.44	-13.55	11.67	-12.77
5	F:30.3-1	13.03	-17.45	12.44	-16.27	11.67	-14.72
1_OHS	F:30.3-2D/F:30.3-1	9.60	-32.91	9.60	-28.99	9.60	-23.82
2_OHS	F:30.3-2D/F:30.3-1	9.60	-35.11	9.60	-31.97	9.60	-27.82
3_OHS	F:30.3-2D/F:30.3-1	9.60	-40.63	9.60	-36.22	9.60	-30.37

Zone	Reference	P_{max} A = 100 ft² psf	P_{min} A = 100 ft² psf	P_{max} A = 200 ft² psf	P_{min} A = 200 ft² psf	P_{max} A > 500 ft² psf	P_{min} A > 500 ft² psf
1	F:30.3-2D	9.60	-10.82	9.60	-10.82	9.60	-10.82
2	F:30.3-2D	9.60	-15.58	9.60	-13.03	9.60	-13.03
3	F:30.3-2D	9.60	-16.86	9.60	-13.03	9.60	-13.03
4	F:30.3-1	11.08	-12.18	10.49	-11.60	9.72	-10.82
5	F:30.3-1	11.08	-13.55	10.49	-12.37	9.72	-10.82
1_OHS	F:30.3-2D/F:30.3-1	9.60	-19.91	9.60	-19.33	9.60	-18.55
2_OHS	F:30.3-2D/F:30.3-1	9.60	-24.68	9.60	-21.54	9.60	-20.76
3_OHS	F:30.3-2D/F:30.3-1	9.60	-25.95	9.60	-21.54	9.60	-20.76

P_{max}	= Maximum Pressure	P_{min}	= Minimum Pressure
Area	= Span • Width	Width	= Width of Component
Span	= Span of Component	Reference	= Applicable Reference from Standard
§30.2.2	= C&C Min Pressure = 9.60 psf	Interpolate	= Interpolate for Areas between columns

REVIEWED BY JAMES ZALESKI P.E.

2305 HAVEFRHILL RD

TALLAHASSEE, FL 323132

PH 850-766-7778

James A Zaleski

Digitally signed by James A Zaleski
Date: 2025.04.28 09:05:11 -04'00'

FOR DIGITAL SIGNATURES&SEAL: This item has been digitally signed and sealed by JAMES ZALESKI P.E. 51544 on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on electronic copies.

ROOF SHEATHING MATERIAL – 7/16" OSB
NAILING – 8D RING SHANK

NAILING PATTERN
EDGES- 6" O.C *FIELD – 6" O.C*

4" O.C FIRST ROW AT ALL EAVES

REVIEWED BY JAMES ZALESKI P.E.
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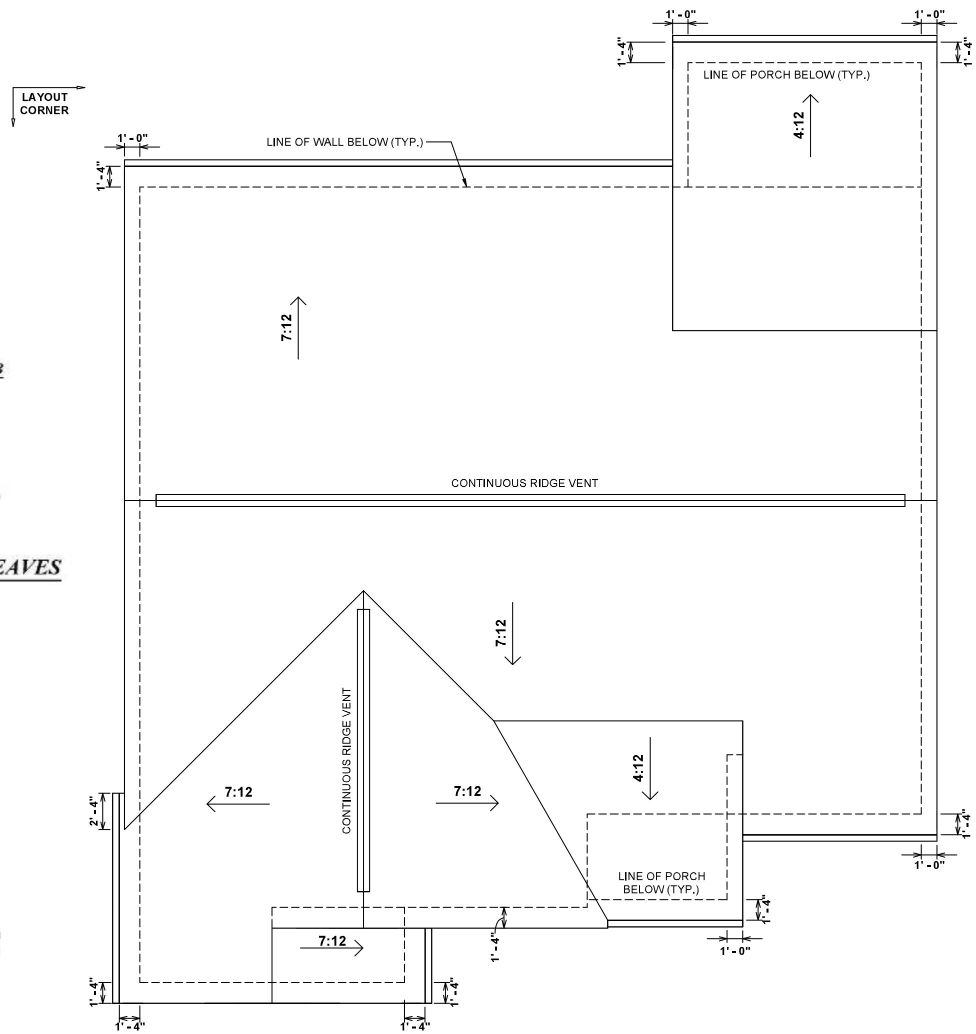
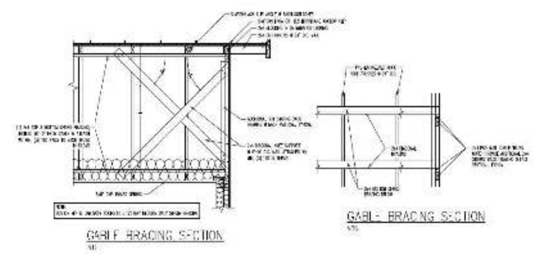


1 ROOF OVERVIEW
1/8" = 1'-0"

4:12 O.W.H. = 3 15/16"
7:12 O.W.H. = 7 7/8"
UNLESS OTHERWISE NOTED ON ROOF OVERVIEW



FLOOR & ROOF TRUSS NOTES:

1. ALL FLOOR TRUSS AND / OR ROOF FRAMING IS TO BE DETERMINED BY THE TRUSS MANUFACTURER AND TRUSS DRAWINGS SHALL BE SIGNED AND SEALED BY THE SAME. FRAMING LAYOUTS CONTAINED IN THESE DRAWINGS ARE TO BE CONSIDERED A PROPOSED SCHEMATIC REPRESENTATION ONLY AND FINAL MANUFACTURER DESIGN MAY VARY FROM THAT SHOWN. THE TRUSS MANUFACTURER'S CALCULATED SIZE AND SPACING OF PRE-ENGINEERED TRUSSES SHALL TAKE PRECEDENCE OVER WHAT HAS BEEN PROVIDED. THE OWNER AND / OR GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL TRAY, CATHEDRAL, AND OTHER DIMENSIONAL CEILING ASPECTS OF THIS PROJECT THAT MAY OR MAY NOT BE SHOWN ON PLANS PRIOR TO FABRICATION. IT IS RECOMMENDED THAT THE OWNER AND /OR GENERAL CONTRACTOR COORDINATE AND UNDERSTAND ALL ASPECTS OF THE SPECIFIED TRUSS PACKAGE LAYOUT BEFORE COMMENCING WITH INSTALLATION.
2. DESIGN OF WOOD FLOOR TRUSSES (IF APPLICABLE) & ROOF TRUSSES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, AND SUPPORT REACTIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECT'S STATE AND SUBMITTED TO BUILDING DEPARTMENT FOR PERMITTING. ANY REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS WITH REGARD TO TRUSS CONFIGURATION ONLY.
3. ERECTION AND BRACING OF PREFABRICATED WOOD TRUSSES SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS OF THE TRUSS MANUFACTURER AND THE TRUSS PLATE INSTITUTE'S "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS".
4. SECURE EACH COMMON ROOF TRUSS/RAFTER TO TOP PLATE WITH ONE OF THE FOLLOWING SIMPSON ANCHOR CLIPS AT ALL BEARING POINTS: H-10A (PREFERRED), (2) H2.5A, H7, OR MTS16. USE SIMPSON H-7 AT GIRDER TRUSSES. PROVIDE A MINIMUM OF TWO STUDS UNDER GIRDER TRUSS END BEARING.
5. TRUSS TO TRUSS CONNECTIONS SHALL BE VERIFIED & PROVIDED BY THE TRUSS DESIGNER.
6. CONTRACTOR TO PROVIDE ALL BLOCKING BETWEEN TRUSSES AND / OR RAFTER FRAMING.
7. CONTRACTOR TO REFER TO F.B.C. 2023 & WFCM 2015 FOR FRAMING REQUIREMENTS OF WOOD FRAMED WALL SYSTEMS.
8. TRUSS MANUFACTURER SHALL REFER TO ANY MEP DRAWINGS FOR OTHER ITEMS OR APPENDAGES THAT MAY EFFECT THE TRUSS LOADING. ANY SUCH ITEMS SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER / ENGINEER OF RECORD.

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

PRELIMINARY CONSTRUCTION PLANS

REV #	DESCRIPTION	DATE

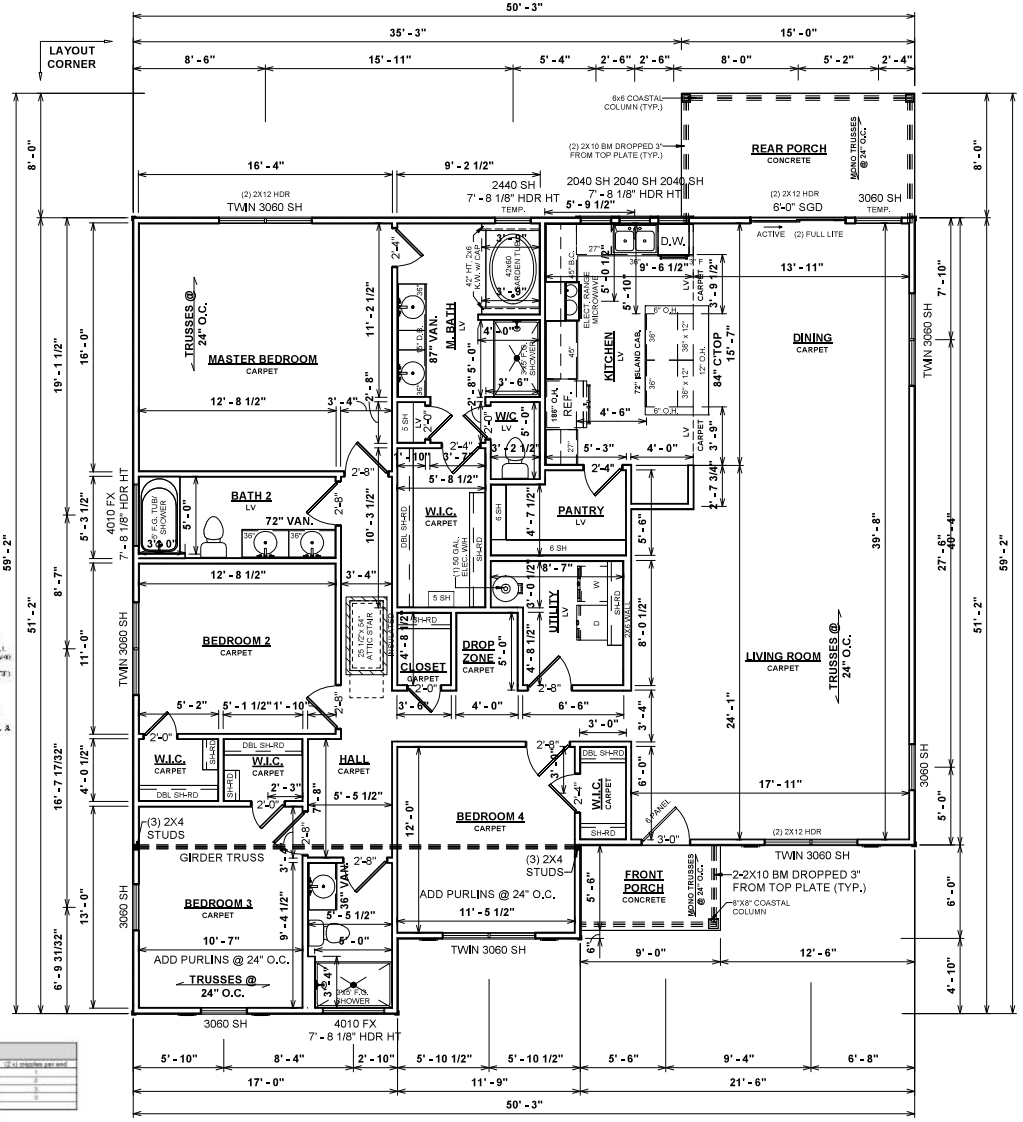
REVISION SCHEDULE

AREAS:	
FIRST FLOOR HEATED	2,281 SF
	2,281 SF
FRONT PORCH	50 SF
REAR PORCH	120 SF
	170 SF
TOTAL UNDER ROOF	2,451 SF

THE: MURRAY
FOR: TJ PIZZAGALLI
245 SW COLONY GLEN
LAKE CITY, FL 32024
SOLD BY: L. HALLER
OFFICE: GAINESVILLE
JOB #: 59-25-008
2x4 EXTERIOR WALLS
FOUNDATION TYPE: MONOSLAB
DRAWN BY: FABIO D.
CHECKED BY: X. CHECKER
PRINTED: 3/28/2025 1:13:31 PM
PROPERTY OF: AMERICA'S HOME PLACE
SHEET #: E-3
ROOF OVERVIEW
SCALE: 1/8" = 1'-0"
PAPER SIZE: 11" x 17"

James
A
Zaleski

Digitally signed by James A Zaleski
Date: 2025.04.28 09:04:31 -04'00'



1 FIRST FLOOR PLAN
1/8" = 1'-0"

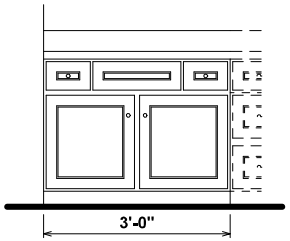
BEAM SPAN TABLE		
Span (FT.)	Header Size	(2) jt studs per end
0 - 3'-0"	2-2x12 w/7/16" OSB Filler Plate	1
3'-01" - 5'-0"	2-2x12 w/7/16" OSB Filler Plate	2
5'-01" - 10'-0"	2-2x12 w/7/16" OSB Filler Plate	3
10'-01" - 12'-0"	2-1/2" LVL (1.750" DF) (or equal)	3

GENERAL NOTES:

- MINIMUM 9'-1 1/8" CEILING HEIGHT ON FIRST FLOOR U.N.O.
- ALL LOAD BEARING WALLS AND EXTERIOR OPENINGS TO HAVE (2) 2x10 HEADERS U.N.O.
- STANDARD FIRST FLOOR WINDOW HEADER HEIGHTS SHALL BE FRAMED DOWN 2'-2" FROM T.O.P., U.N.O.
- WINDOW HEADERS FOR WINDOWS LOCATED ABOVE KITCHEN SINK SHALL BE FRAMED DOWN 1'-5" FROM T.O.P., U.N.O.
- STANDARD SECOND FLOOR WINDOW HEADER HEIGHTS SHALL BE FRAMED DOWN 1'-2" FROM T.O.P., U.N.O.
- 7/16" O.S.B. AND HOUSEWRAP REQUIRED.
- DIMENSIONS ARE TO SHEATHING EXTERIOR; SUBTRACT 1/2" FROM DIMENSIONS FOR EXTERIOR WINDOW AND DOOR FRAMING LOCATION IF OPENINGS ARE FRAMED BEFORE SHEATHING INSTALLATION.
- ALL INTERIOR DOORS ARE EITHER CENTERED ON WALLS OR ROUGH OPENING STARTED MINIMUM OF 4" FROM ADJOINING WALL UNLESS OTHERWISE DIMENSIONED.
- NUMBER OF STAIR TREADS AND RISERS MAY VARY AS A RESULT OF LOCAL BUILDING CODES, STANDARDS AND FINAL GRADE.
- ALL SHELVES TO BE 12" DEEP U.N.O.
- HEIGHT OFF FLOOR TO BE:
  1. SINGLE 68"
  2. DOUBLE 42" AND 84"
- ALL PLUMBING FIXTURES SHOWN ARE A REPRESENTATION OF SIZE AND LOCATION ONLY. ACTUAL STYLE AND BRAND OF FIXTURES MAY VARY PER OFFICE LOCATION.
- ALL TUBS / SHOWERS TO HAVE NAILERS AT FLANGE.
- INSTALL A 24" WIDE WALKWAY FROM ATTIC ACCESS TO FURNACE PLATFORM.
- PORCH, STOOP, & DECK HANDRAILS NOT INCLUDED W/ SLAB FOUNDATION.
- RAILINGS ARE A FORCED OPTION WHEN PORCH IS OVER 30" HIGH FROM FINISHED GRADE.
- SCHLUTER DITRA UNCOUPLING AND WATER PROOFING MEMBRANE TO BE USED ON ALL TILE SHOWER INSTALLATIONS.

TERMITE PROTECTION

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION.



2 891.1 36" VANITY ELEVATION
1/2" = 1'-0"

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

PRELIMINARY CONSTRUCTION PLANS

PAPER SIZE: 11" x 17"

SCALE: As indicated

REVISION SCHEDULE			
REV #	DESCRIPTION	DATE	

AREAS:	
FIRST FLOOR HEATED	2,281 SF
	2,281 SF
FRONT PORCH	50 SF
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SOLD BY: L. HALLER

JOB #: 59-25-008
2x4 EXTERIOR WALLS
FOUNDATION TYPE: MONOSLAB

DRAWN BY: FABIO D.
CHECKED BY: X. CHECKER
PRINTED: 3/28/2025 1:13:33 PM

PROPERTY OF:

SHEET # F-2

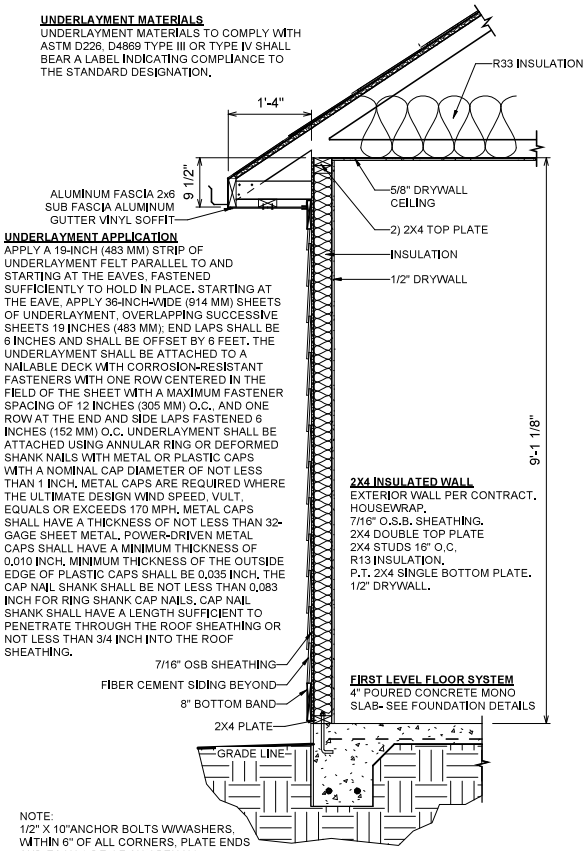
FIRST FLOOR PLAN

James A Zaleski

Digitally signed by James A Zaleski
Date: 2025.04.28 09:02:56 -04'00'

ROOF SYSTEM
ROOF COVERING AS SPECIFIED DOUBLE LAYER OF SYNTHETIC UNDERLAYMENT WITH 7/16" OSB SHEATHING NAILED AND CLIPPED
PRE ENGINEERED ROOF TRUSSES PER PRINT
R-33 INSULATION IN FLAT AREAS
R-30 INSULATION IN SLOPED AREAS

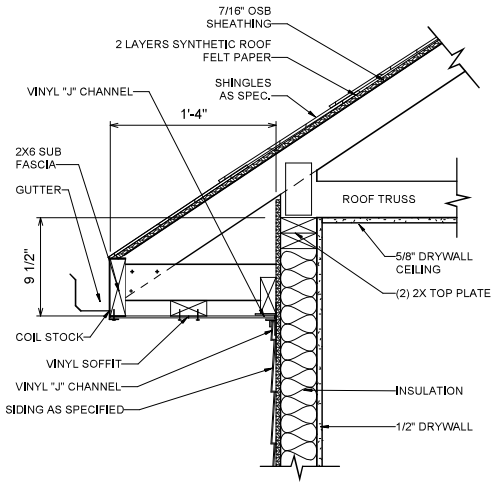
UNDERLAYMENT MATERIALS
UNDERLAYMENT MATERIALS TO COMPLY WITH ASTM D226, D4869 TYPE III OR TYPE IV SHALL BEAR A LABEL INDICATING COMPLIANCE TO THE STANDARD DESIGNATION.



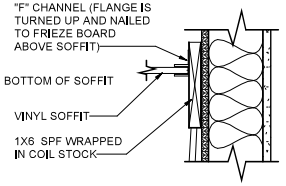
UNDERLAYMENT APPLICATION
APPLY A 19-INCH (483 MM) STRIP OF UNDERLAYMENT FELT PARALLEL TO AND STARTING AT THE EAVES, FASTENED SUFFICIENTLY TO HOLD IN PLACE. STARTING AT THE EAVE, APPLY 36-INCH-WIDE (914 MM) SHEETS OF UNDERLAYMENT, OVERLAPPING SUCCESSIVE SHEETS 19 INCHES (483 MM); END LAPS SHALL BE 6 INCHES AND SHALL BE OFFSET BY 6 FEET. THE UNDERLAYMENT SHALL BE ATTACHED TO A NAILABLE DECK WITH CORROSION-RESISTANT FASTENERS WITH ONE ROW CENTERED IN THE FIELD OF THE SHEET WITH A MAXIMUM FASTENER SPACING OF 12 INCHES (305 MM) O.C., AND ONE ROW AT THE END AND SIDE LAPS FASTENED 6 INCHES (152 MM) O.C. UNDERLAYMENT SHALL BE ATTACHED USING ANNULAR RING OR DEFORMED SHANK NAILS WITH METAL OR PLASTIC CAPS WITH A NOMINAL CAP DIAMETER OF NOT LESS THAN 1 INCH. METAL CAPS ARE REQUIRED WHERE THE ULTIMATE DESIGN WIND SPEED, VULT, EQUALS OR EXCEEDS 170 MPH. METAL CAPS SHALL HAVE A THICKNESS OF NOT LESS THAN 32-GAGE SHEET METAL. POWER-DRIVEN METAL CAPS SHALL HAVE A MINIMUM THICKNESS OF 0.010 INCH. MINIMUM THICKNESS OF THE OUTSIDE EDGE OF PLASTIC CAPS SHALL BE 0.035 INCH. THE CAP NAIL SHANK SHALL BE NOT LESS THAN 0.083 INCH FOR RING SHANK CAP NAILS. CAP NAIL SHANK SHALL HAVE A LENGTH SUFFICIENT TO PENETRATE THROUGH THE ROOF SHEATHING OR NOT LESS THAN 3/4 INCH INTO THE ROOF SHEATHING.

NOTE:
1/2" X 10"ANCHOR BOLTS W/WASHERS, WITHIN 6" OF ALL CORNERS, PLATE ENDS AND EACH SIDE OF AN OPENING, THEN SPACED A MINIMUM OF 4'-0" O.C. OR AS REQUIRED.

1 1 STORY WALL CONC. SIDING ON SLAB - FL
1/2" = 1'-0"



2 400.1 RAFTER TAIL AT CONC. SIDING WALL - FL
1" = 1'-0"



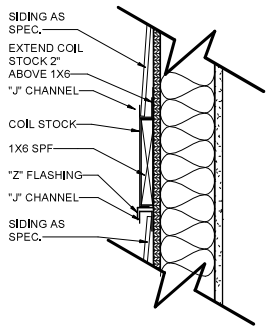
3 FRIEZE DETAIL AT SIDING
1 1/2" = 1'-0"

Wall Exterior Panel - Sheath with 7/16" OSB
- 2 X 4 STUDS at 12" O.C. UP TO 10 FEET
- 2 X 6 STUDS at 12" O.C. UP TO 12 FEET
- 2 X 6 STUDS AT 16" O.C. UP TO 16 FEET
ALL WALLS OVER 10 FEET TO HAVE 2 ROWS OF BLOCKING

SEE ATTACHED DETAILS

POSTS USE SIMPSON ABU BASE WITH 2 STRAP STRAPS AT TOP AND 2 SIMPSON SDWC 15600 DRILLWS FROM POST TO BEAM

MIN NAIL PENETRATION - 1-1/2"
Nail Type SD
Edge Nail Spacing 4" o.c.
Intermediate Nail Spacing 8" o.c.
SIMPSON SDWC15600 SCREWS AT THE TOP OF STUDS AND SIMPSON SDWC22450 SCREWS AT THE BOTTOM OF STUDS AT ALL CORNERS AND 48" O.C.



4 LINEAL BAND DETAIL CONC. SIDING
1 1/2" = 1'-0"

REVIEWED BY JAMES ZALESKI P.E.
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TALLAHASSEE, FL 323132
PH 850-766-7778

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JAMES ZALESKI P.E. ON THE DATE ADJACENT TO THE SEAL. PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

OPT. PAPER SIZE: for 1/4" = 1'-0" on 24" x 36" PAPER SIZE ENLARGE PRINTS TO 200%

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PAPER SIZE: 11" x 17"

SCALE: As indicated

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THE: MURRAY

FOR: TJ PIZZAGALLI

245 SW COLONY GLEN
LAKE CITY, FL 32024

OFFICE: GAINESVILLE

SOLD BY: L. HALLER

JOB #: 59-25-008

2x4 EXTERIOR WALLS

FOUNDATION TYPE: MONOSLAB

DRAWN BY: FABIO D.

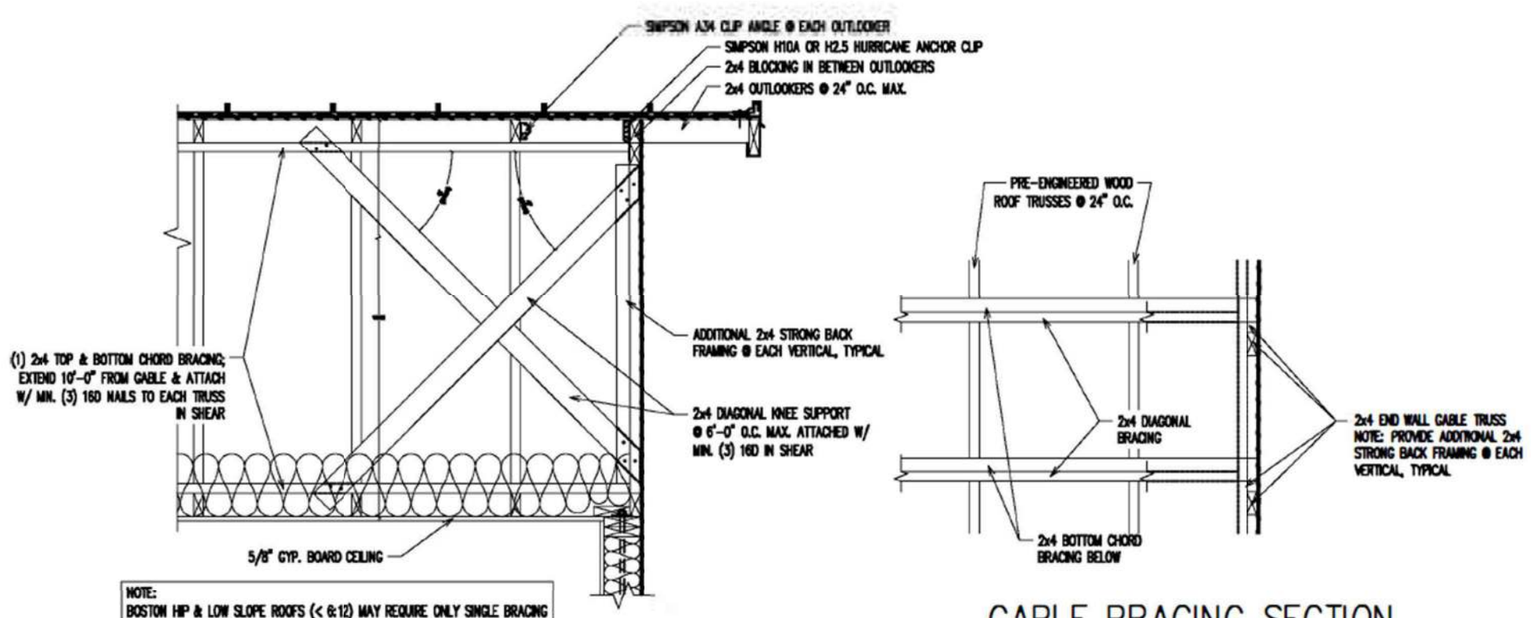
CHECKED BY: X. CHECKER

PRINTED: 3/28/2025 1:13:36 PM

PROPERTY OF:

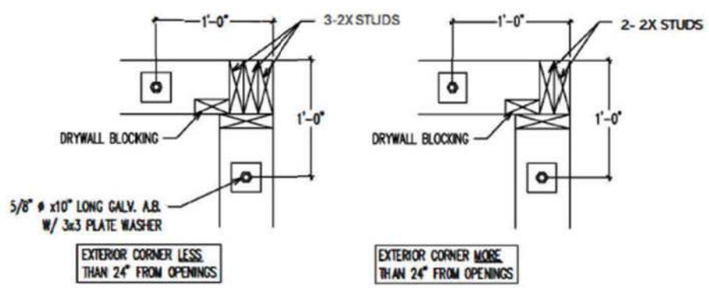
SHEET #: S-3

MONOSLAB WALL SECTIONS - CONCRETE SIDING



GABLE BRACING SECTION

NTS



CORNER FRAMING PLAN SECTIONS

NTS

INSTALL 5/8"ø x10" J-BOLT ANCHORS W/ MIN. 7" EMBEDMENT @ 32" O.C. FOR ALL LOAD BEARING WALLS.

JAMES ZALESKI P.E. 51544
2305 HAVERHILL RD
TALLAHASSEE, FL 32312
PH 850-766-7778

ALL FLOOR TRUSS AND /O R ROOFFRAMING S TO BE DETERMINED BY THE TRUSS MANUFACTURER AND TRUSS DRAWINGS SHALL BE SIGNED AND SEALED BY THE SAME FRAMING LAYOUTS CONTAINED IN TIHESE DRAWINGS ARE TO BE CONSIDERED A PROPOSED SCHEMATIC REPRESENTATION ONLY AND FINAL MANUFACTURER DESIGN MAY VARY FROM THAT SHOWN. THE TRUSS MANUFACTIURER'S CALCULATED SIZE AND SPACING OF PRE- ENGINEERED TRUSSES SHALL TAKE PRECEDENCE OVER WHAT HAS BEEN PROPOSED. THE OWNERAND / OR GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF AIL. TRAY, CATHEDRAL, AND OTHER DIMENSIONAL CEILING ASPECTS OF THIS PROJECT TH AT MAY OR MAY NOT BE SHOWN ON PLANS PRIOR TO FABRICATION. IT IS RECOMMENDED THAT THE OWNER AND / OR GENERAL. CONTRACTOR COORDINATE AND UNDERSTAND ALL ASPECTS OF THE SPECIFIED TRUSS PACKAGE LAYOUT BEFO RE COMMENCING WITH INSTALLATION.

DESIGN OF WOOD FLOOR TRUSSES (IF APPLICABLE) & ROOF TRUSSES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, AND SUPPORT REACTIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE PROJECTSTATE AND SUBMITTIED TO BUILDING DEPARTMENT FOR PERMITTING. ANY REVIEW OF SHOP DRAWINGS SHAIL. BEFOR CONFORMANCE WITH TH CONTRACT DOCUMENTS WITH REGARD TO TRUSS CONFIGURATION ONLY.

ERECTION AND BRACING OF PREFABRICATED WOOD TRUSSES SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS OF THE TRUSS MANUFACTURER AND TH TRUSS PLATE INSTITUTE'S "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS".

SECURE EACH COMMON ROOFTRUSS/RAFTER TO TOP PLATE WITH ONE OF THE FOLLOWING SIMPSON ANCHOR CLIPS AT AIL. BEARING POINTS:

H- 10A (PREFERRED), (2) H2.5A, H7, OR MTS16.

USE SIMPSON H-7 AT GIRDER TRUSSES. PROVIDE A MINIMUM OF TWO STUDS UNDER GIRDER TRUSS END BEARING.

TRUSS TO TRUSS CONNECTIONS SHALL BE VERIFIED & PROVIDED BY THE TRUSS DESIGNER.

CONTRACTOR TO PROVIDE AIL. BLOCKING BETWEEN TRUSSES AND / OR RAFTER FRAMING.

CONTRACTOR TO REFER TO F.B.C. 2023 & WFCM 2018 FOR FRAMINGREQUIREMENTS OF WOOD FRAMED WALL SYSTEMS.

TRUSS MANUFACTURER SHAIL. REFER TO ANY MEP DRAWINGS FOR OTHER ITEMS OR APPENDAGES IHAT MAY EFFECT THETRUSS LOADING. ANY SUCH ITEMSSHALL BE BROUGHT TO IHE ATTENTION OF IHE DESIGNER / ENGINEER OF RECORD.

BEAM SPAN TABLE

Span (FT.)	Header Size	(2 x) cripples per end
0' - 4'-0"	2-2x10 w/7/16" OSB Flitch Plate	1
4'-0" - 9'-4"	2-2x10 w/7/16" OSB Flitch Plate	2
9'-4" - 12'-0"	2-2x10 w/7/16" OSB Flitch Plate	3
12'-0" - 15'-4"	3 1/2" X 11 7/8" LVL (or equal)	3

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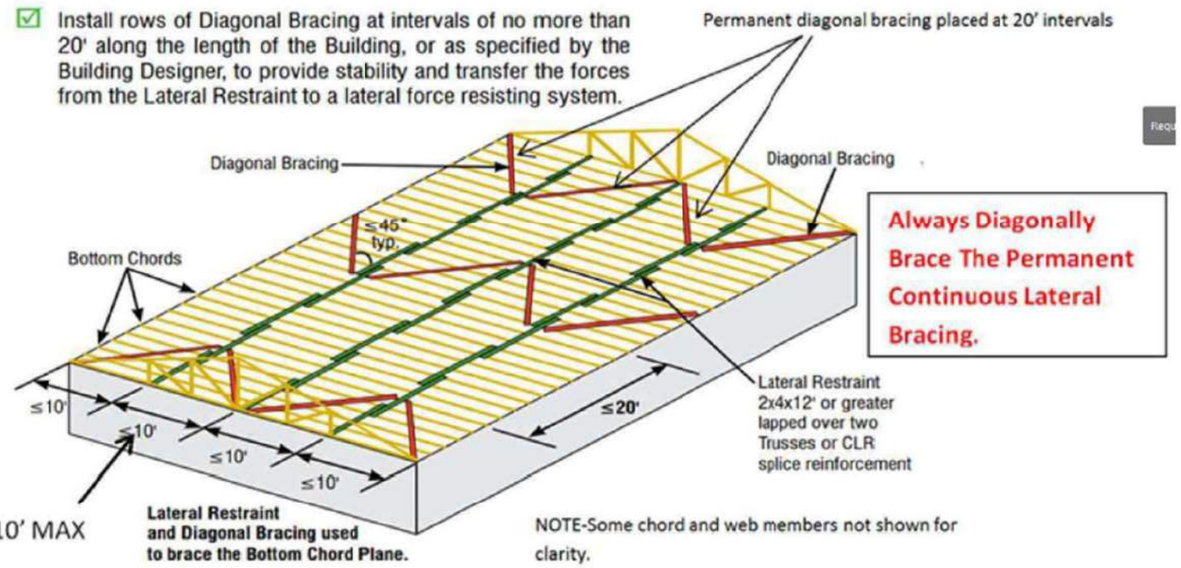
James A Zaleski

Digitally signed by James A Zaleski

Date: 2025.04.28 09:02:20 -04'00'



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ES ZALESKI P.E. 51544 2305 HAVERHILL RD TALLAHASSEE, FL 32312 PH 850-766-7778

Install Simpson sheathing clip PSCL @ 24" c.c. for roof sheathing.
Gable ends per attached details. For vaulted ceilings, balloon framing is required.
Provide continuous structural sheathing on gable ends and block all edges on sheathing.

James A Zaleski

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HEADER SIZE AND STRAPPING CHART

SPAN	HEADER SIZE	QUANTITY OF JACK STUDS AT EACH END	QUANTITY OF KING STUDS AT EACH END	STRAPPING TO JACK STUDS AT EACH END TOP AND BOTTOM	STRAPPING TO KING STUDS AT EACH END TOP AND BOTTOM
0'-0" TO 7'-6"	2 - 2X10" WITH ½" PLATE	1	1	1 SIMPSON MSTA24	1 SIMPSON SPH4
7'-6" – 11'-3"	2 -2X12" WITH ½" PLATE OR 4-2 X 10" WITH ½" PLATE	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4
11'-3" – 14'-0"	2- 1 ¾" X 9 ¼" LVL	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4
IN LIEU OF STRAPPING USE A SDWC15600 AT THE TOP OF EACH JACK AND KING STUD AND ONE SDWC15450 AT THE BASE OF EACH JACK AND KING STUD					

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	Shear Wall Panel Type "B" Specifications	
1 Face	Stud Spacing	16" O.C.
	Panel Grade	OSB Sheathing
	Minimum Panel Thickness	7/16"
	Minimum Nail Penetration in Framing	1 1/2"
	Nail Type	8d Common
	Edge Nail Spacing	4"
	Intermediate Nail Spacing (field)	12"
	Total Panel Shear Capacity	350 plf