

JULIUS LEE PE.

RE 517536 - RONALD CLARK CONST - GARAGE ADDITION

**1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435**

Site Information:

Project Customer RONADL CLARK CONST Project Name 517536 Model Krychl Addition
Lot/Block Subdivision
Address 20403 NW 113th Way
City Alachua State FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name RONALD W CLARK License # CRC1326560
Address 15816 NW CR 1491
City ALACHUA State FL

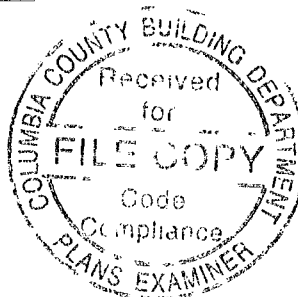
General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code FBC2010/TPI2007 Design Program: MiTek 20/20 7 3
Wind Code: ASCE 7-10 Wind Speed 130 mph Floor Load: N/A psf
Roof Load. 32 0 psf

This package includes 1 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet
conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

**In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany
this coversheet. The latest approval dates supersede and replace the previous drawings.**

No	Seal#	Truss Name	Date
1	I7210868	T01	9/5/013

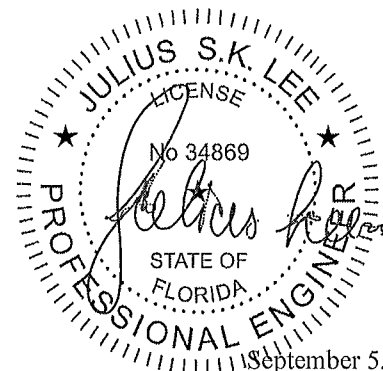


The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax)

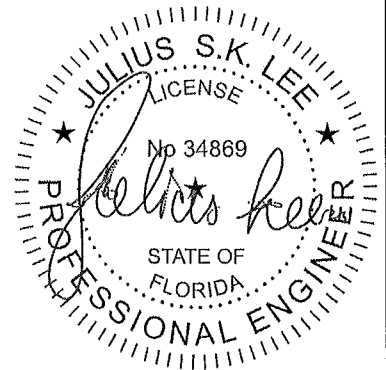
Truss Design Engineer's Name Julius Lee

My license renewal date for the state of Florida is February 28, 2015

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2



Job 517536	Truss T01	Truss Type Common Truss	Qty 13	Ply 1	RONALD CLARK CONST GARAGE ADDITION	17210868																																																																								
Builders FirstSource Lake City FL 32055					Job Reference (optional)																																																																									
<div style="text-align: right; font-size: small;">7 350 s Sep 27 2012 MiTek Industries, Inc. Thu Sep 05 10:55:50 2013 Page 1</div> <div style="text-align: center; font-size: x-small;">ID XTc08pmKY_hmuLirEGYvzc09Z-n3Eh0Y91aOAs1lweXaudpx2aBoVKpV8h_xsGhqygit</div>																																																																														
<table border="1" style="width:100%; border-collapse: collapse; font-size: x-small;"> <tr> <th colspan="2">LOADING (psf)</th> <th colspan="2">SPACING</th> <th colspan="2">CSI</th> <th colspan="2">DEFL</th> <th colspan="2">PLATES</th> <th colspan="2">GRIP</th> </tr> <tr> <td>TCLL</td><td>20 0</td> <td>Plates Increase</td><td>1 25</td> <td>TC</td><td>0 32</td> <td>in</td><td>(loc)</td> <td>I/defl</td><td>L/d</td> <td>MT20</td><td>244/190</td> </tr> <tr> <td>TCDL</td><td>7 0</td> <td>Lumber Increase</td><td>1 25</td> <td>BC</td><td>0 48</td> <td>Vert(TL)</td><td>-0 08</td> <td>8-12</td><td>>999</td> <td colspan="2"></td> </tr> <tr> <td>BCLL</td><td>0 0 *</td> <td>Rep Stress Incr</td><td>YES</td> <td>WB</td><td>0 15</td> <td>Vert(TL)</td><td>-0 15</td> <td>8-12</td><td>>999</td> <td colspan="2"></td> </tr> <tr> <td>BCDL</td><td>5 0</td> <td>Code FBC2010/TPI2007</td><td></td> <td>(Matrix-M)</td><td></td> <td>Horz(TL)</td><td>0 03</td><td>6</td><td>n/a</td> <td colspan="2"></td> </tr> <tr> <td colspan="10"></td> <td>Weight: 84 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP		TCLL	20 0	Plates Increase	1 25	TC	0 32	in	(loc)	I/defl	L/d	MT20	244/190	TCDL	7 0	Lumber Increase	1 25	BC	0 48	Vert(TL)	-0 08	8-12	>999			BCLL	0 0 *	Rep Stress Incr	YES	WB	0 15	Vert(TL)	-0 15	8-12	>999			BCDL	5 0	Code FBC2010/TPI2007		(Matrix-M)		Horz(TL)	0 03	6	n/a													Weight: 84 lb	FT = 20%
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REACTIONS (lb/size) 2=574/0-7 10 (min 0-1-8) 6=574/0-7 10 (min 0-1-8) Max Horz 2=118(LC 12) 6=318(LC 13) Max Uplift 2=318(LC 12) 6=318(LC 13) Max Grav 2=684(LC 2), 6=684(LC 2)																																																																														
FORCES (lb) Max. Comp./Max Ten All forces 250 (lb) or less except when shown TOP CHORD 2-3=-990/606, 3-4=-727/449 4-5=-727/448, 5-6=-990/606 BOT CHORD 2-9=-416/843, 8-9=-416/843, 6-8=-423/846 WEBS 4-8=-194/354 5-8=-308/288, 3-8=-308/288																																																																														
NOTES (8-10) 1) Unbalanced roof live loads have been considered for this design 2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf h=18ft, Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60 3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads 4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 318 lb uplift at joint 2 and 318 lb uplift at joint 6 7) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss. 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 9) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB 10) Truss Design Engineer Julius Lee PE Florida P E License No 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435																																																																														
LOAD CASE(S) Standard																																																																														



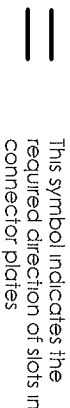
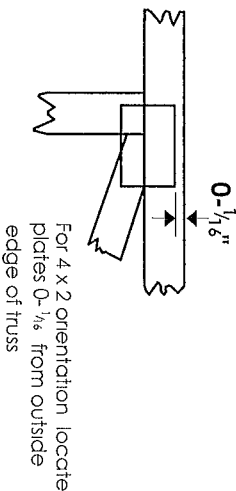
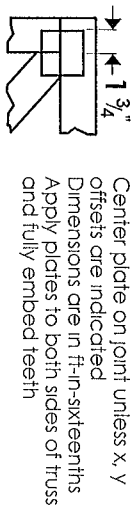
September 5,2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer. not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION

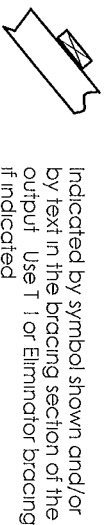


* Plate location details available in **Mitek 20/20** software or upon request.

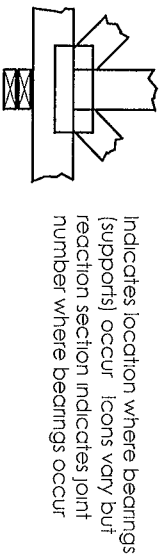
PLATE SIZE

4 X 4
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

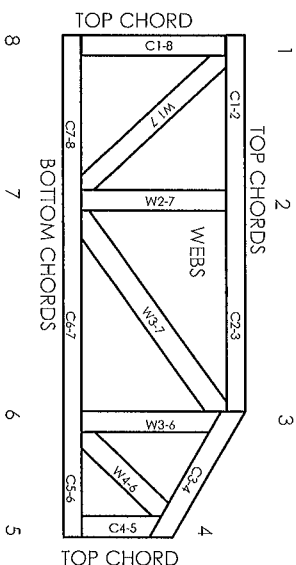
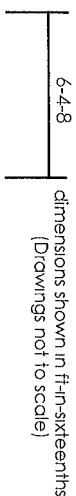


BEARING



Industry Standards:
ANSI/TPI1 National Design Specification for Metal Plate Connected Wood Truss Construction
DSB-89 Design Standard for Bracing
BCS11 Building Component Safety Information Guide to Good Practice for Handling
Installing & Bracing of Metal Plate
Connected Wood Trusses

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports

ESR-1311 ESR-1352, ER-5243 9604B
9730 95-43 96-31 9667A
NER-487 NER-561
95110 84-32, 96-67 ER-3907 9432A

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1109 Coastal Bay ,
Boynton Beach, FL 33435

General Safety Notes

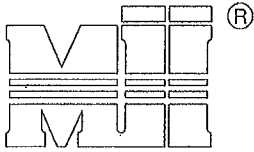
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system e.g. diagonal or X-bracing is always required. See BCS11
2. Truss bracing must be designed by an engineer. For wide truss spacing individual lateral braces themselves may require bracing or alternative T, I, or Eliminator bracing should be considered
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties
5. Cut members to bear tightly against each other
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements
12. Lumber used shall be of the species and size and in all respects equal to or better than that specified
13. Top chords must be sheathed or purlins provided at spacing indicated on design
14. Bottom chords require lateral bracing at 10 ft spacing, or less, if no ceiling is installed. Unless otherwise noted
15. Connections not shown are the responsibility of others
16. Do not cut or alter truss member or plate without prior approval of an engineer
17. Install and load vertically unless indicated otherwise
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



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MiTek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.

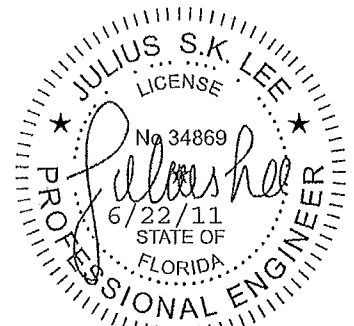
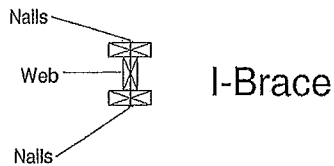
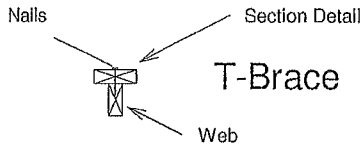
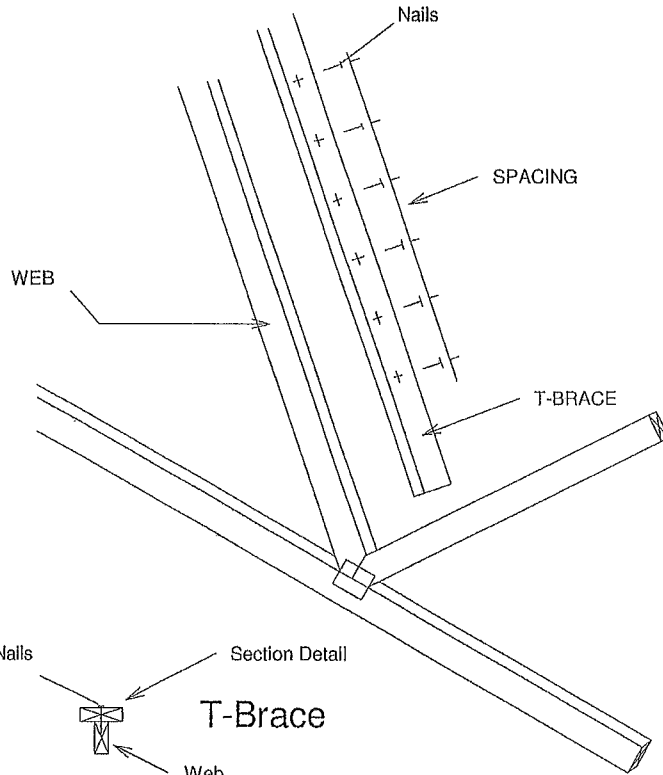
Note. Nail along entire length of T-Brace / I-Brace
(On Two-Ply's Nail to Both Plies)

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member



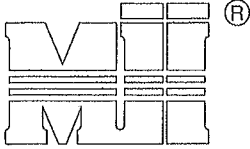
1109 COASTAL BAY
BOYNTON BC, FL 33435

JANUARY 1, 2009

LATERAL TOE-NAIL DETAIL

ST-TOENAIL_SP

MiTek Industries, Chesterfield, MO Page 1 of 1



MiTek Industries, Inc

NOTES

- 1 TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN)
- 2 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD
- 3 ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM	SYP	DF	HF	SPF	SPF-S
3.5" LONG	131	88.0	80.6	69.9	68.4	59.7
	135	93.5	85.6	74.2	72.6	63.4
	162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	128	74.2	67.9	58.9	57.6	50.3
	131	75.9	69.5	60.3	59.0	51.1
	148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

EXAMPLE

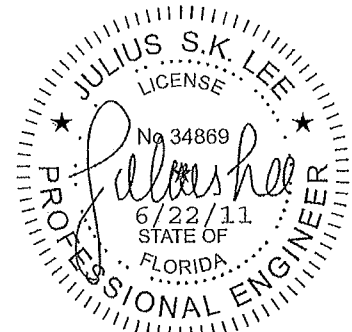
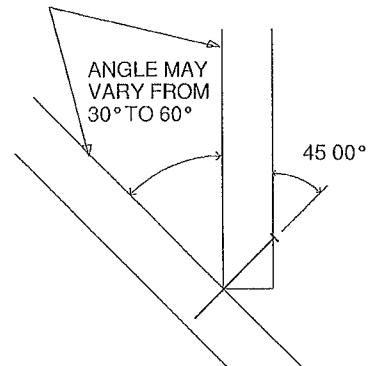
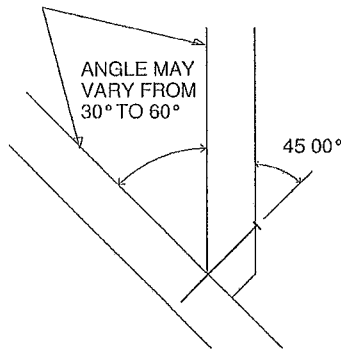
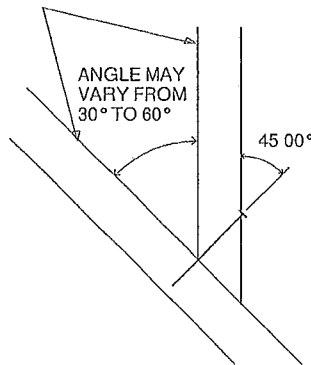
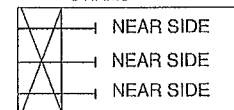
(3) 16d NAILS (162" diam x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

SIDE VIEW

3 NAILS

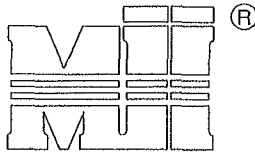


1109 COASTAL BAY
 BOYNTON BC, FL 33435

FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

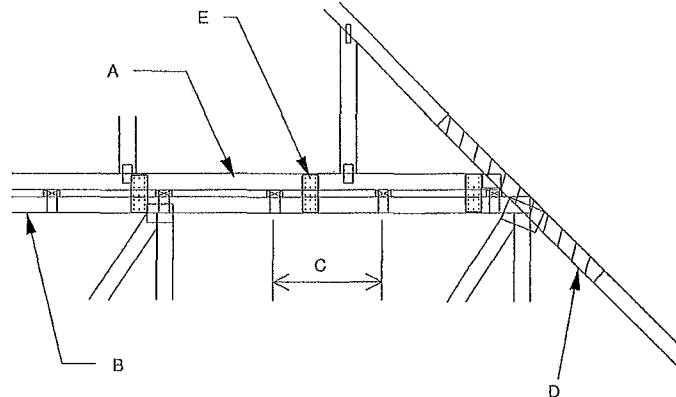


MITek Industries, Inc.

MITek Industries, Chesterfield, MO

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
 MAX MEAN ROOF HEIGHT = 30 FEET
 MAX TRUSS SPACING = 24' O.C.
 CATEGORY II BUILDING
 EXPOSURE B or C
 ASCE 7-10
 DURATION OF LOAD INCREASE 1.60
 DETAIL IS NOT APPLICABLE FOR TRUSSES
 TRANSFERRING DRAG LOADS (SHEAR TRUSSES)
 ADDITIONAL CONSIDERATIONS BY BUILDING
 ENGINEER/DESIGNER ARE REQUIRED

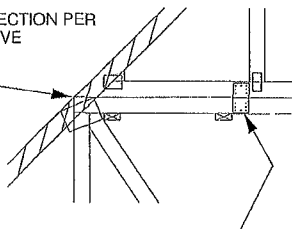
- A PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24' O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D 2" X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4' O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 115 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E FOR WIND SPEEDS BETWEEN 125 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



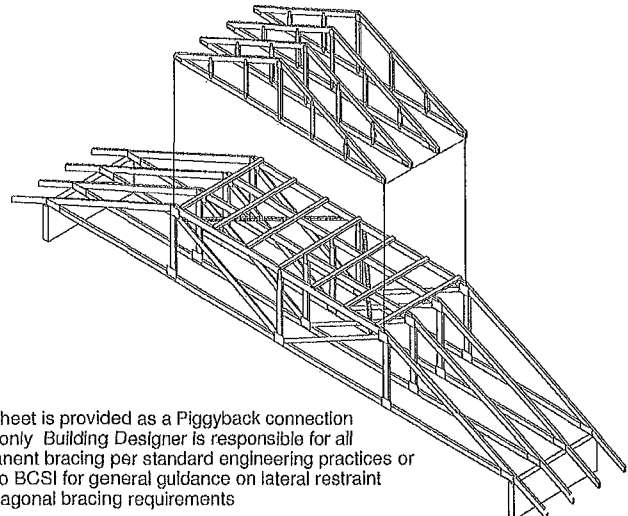
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER NOTE D ABOVE

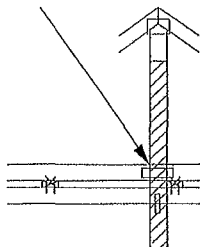


FOR ALL WIND SPEEDS, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE



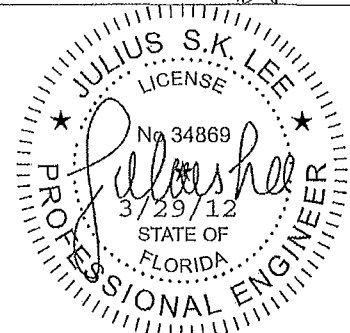
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

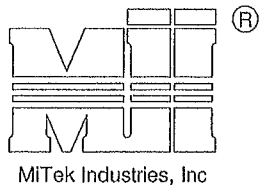


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB

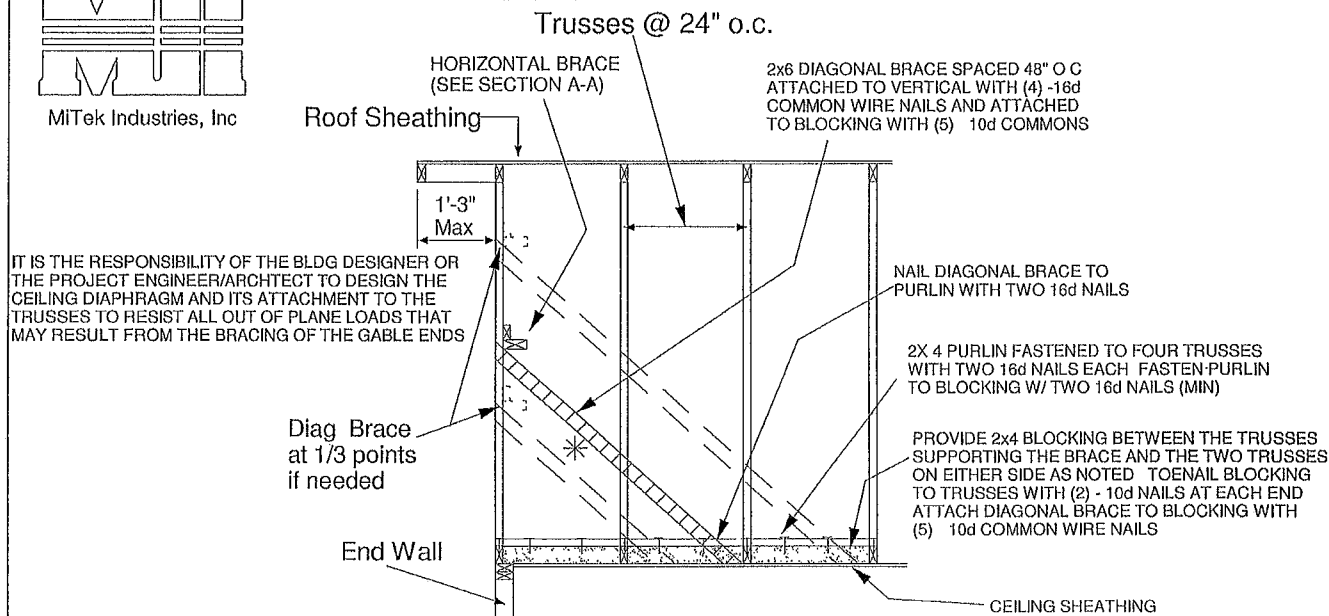
- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2" x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4' O.C. FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1' 15") REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED

METHOD 1 ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

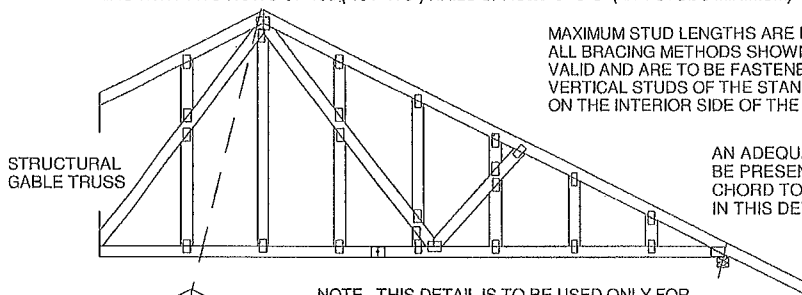
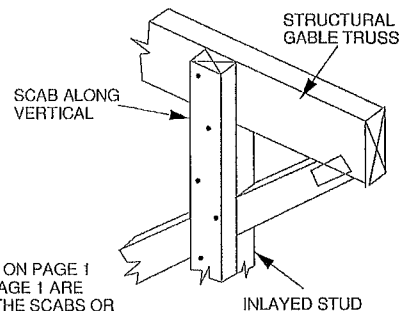
METHOD 2 ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6" O C

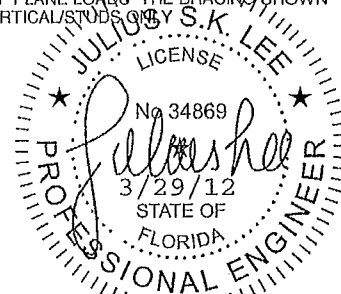
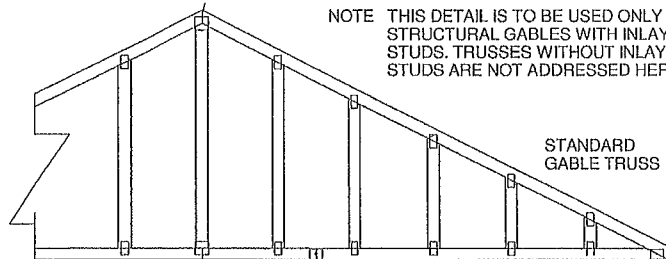
FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6" O C (2X 4 STUDS MINIMUM)

MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1
ALL BRACING METHODS SHOWN ON PAGE 1 ARE
VALID AND ARE TO BE FASTENED TO THE SCABS OR
VERTICAL STUDS OF THE STANDARD GABLE TRUSS
ON THE INTERIOR SIDE OF THE STRUCTURE



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST
BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM
CHORD TO RESIST ALL OUT OF PLANE LOADS THE BRACING SHOWN
IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY

NOTE THIS DETAIL IS TO BE USED ONLY FOR
STRUCTURAL GABLES WITH INLAVED
STUDS. TRUSSES WITHOUT INLAVED
STUDS ARE NOT ADDRESSED HERE



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6/12 PITCH - 2

