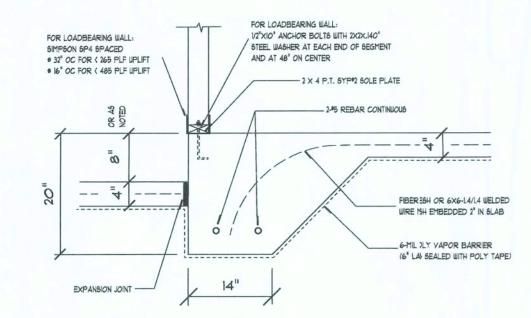


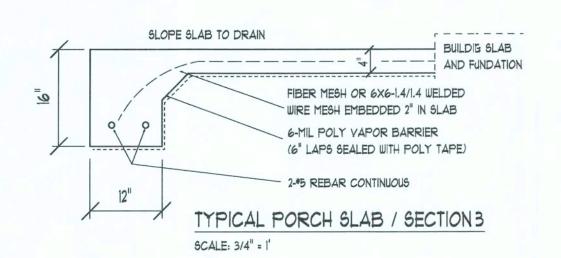
TYPICAL INTERIOR BEARING WALL

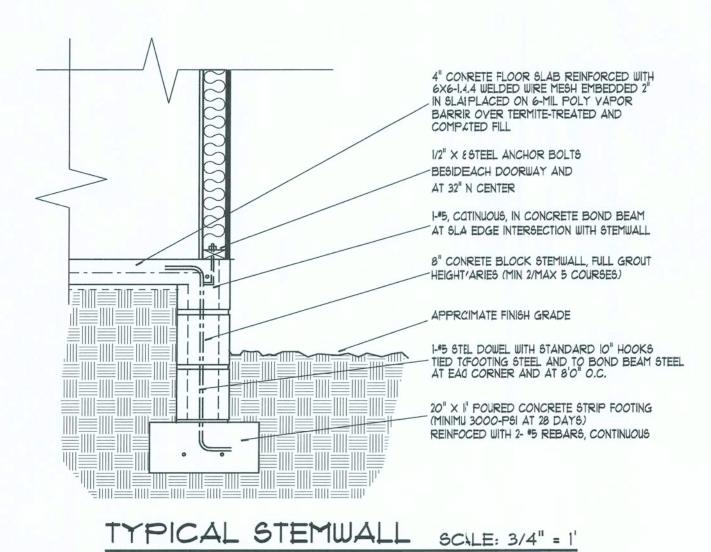
SCALE: 1" = 1'

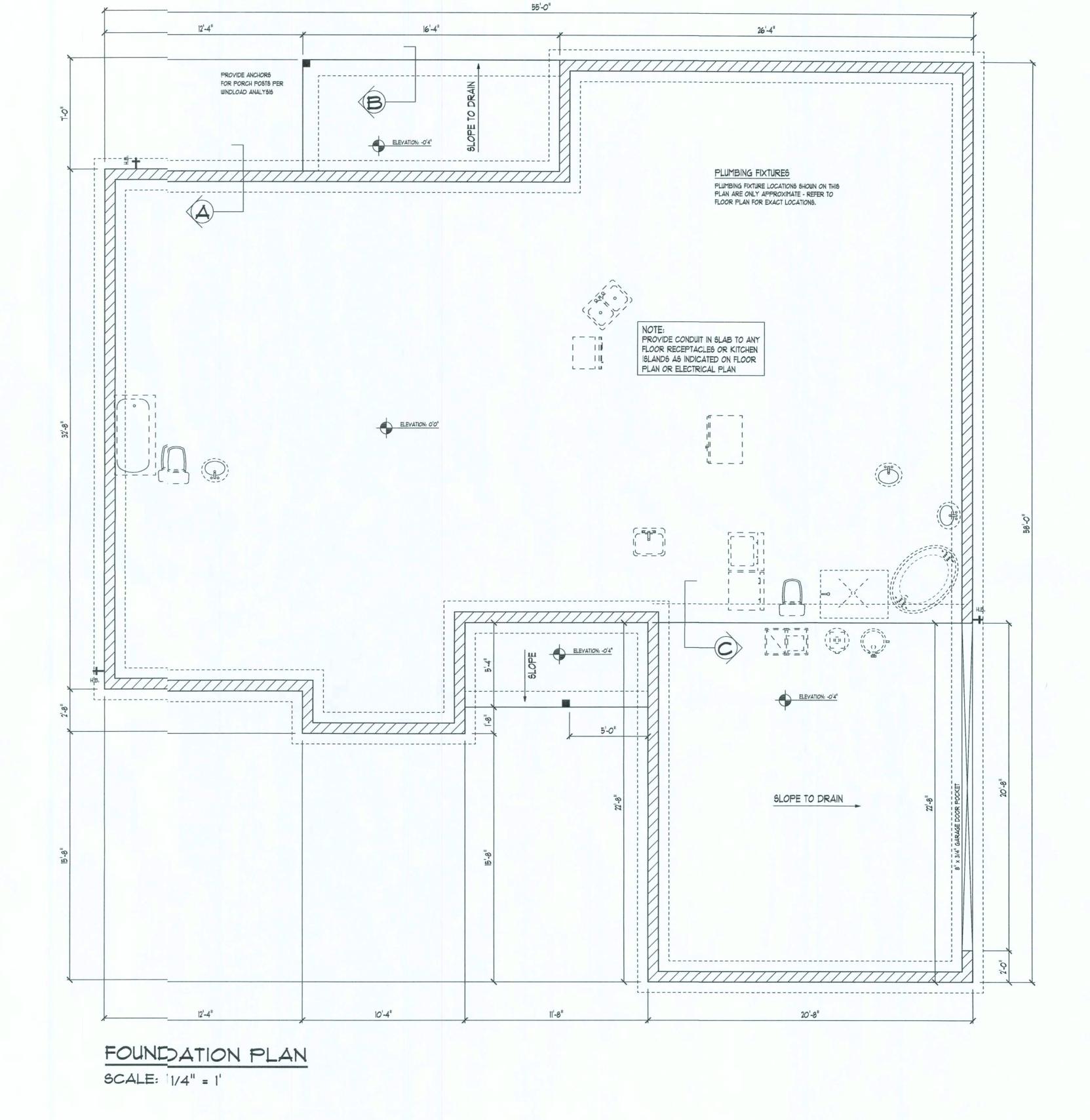
PLACE AS SHOWN ON FLOOR PLAN, AND ALSO REFER TO ROOF TRUSS ENGINEERING FOR OTHER INTERIOR BEARING LOCATIONS



TYPICAL MONOLITHIC SLAB / SECION C







September 26, 2006

D.D.S. STUDIOS

Lake City IL. 32056 (386) 754-2181

SHEET NUMBER

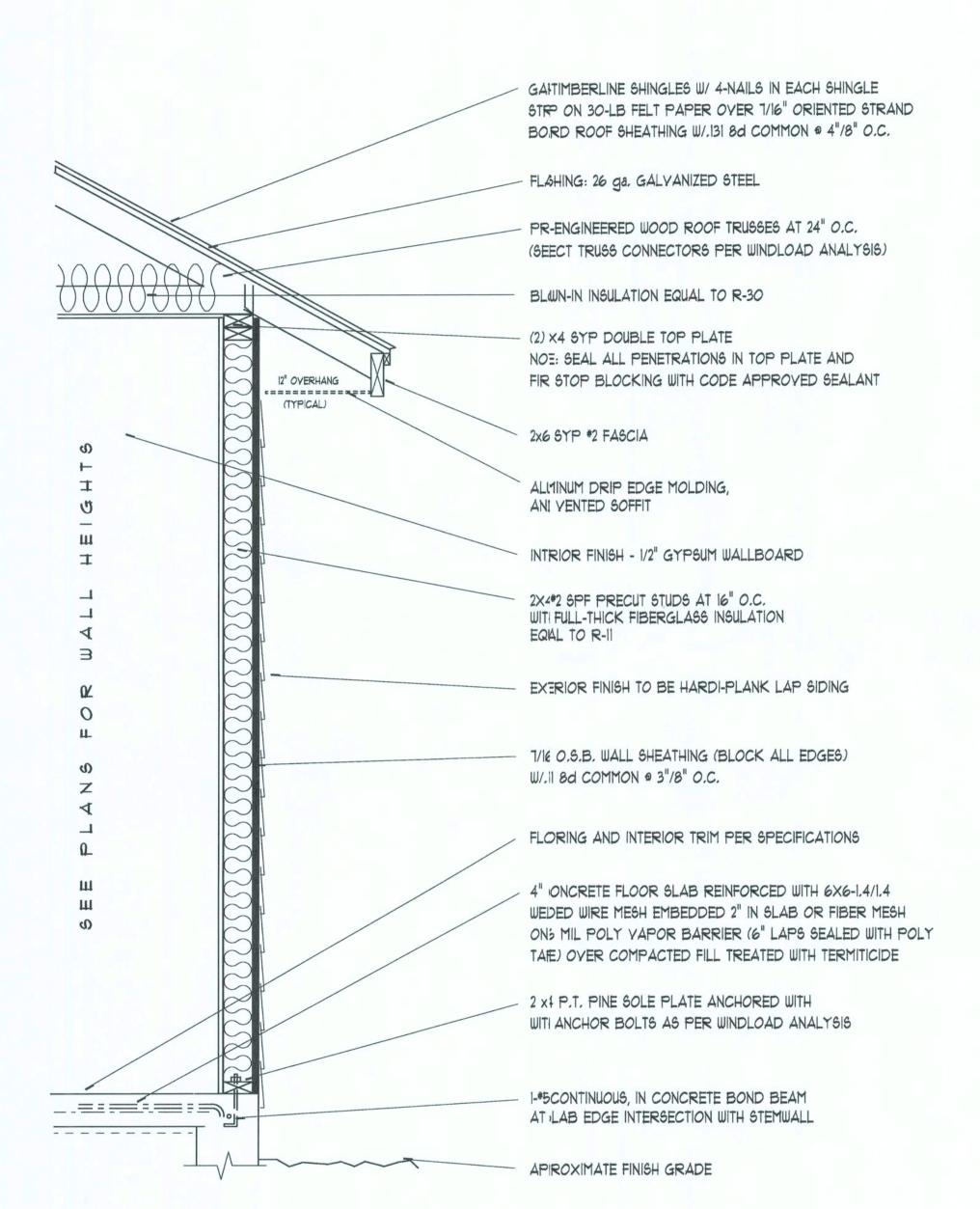
2 a 4

All work shall comply with the standard building code, and all applicable local codes and odinances.

Contractor shill verify all dimensions pror to commencing construction.

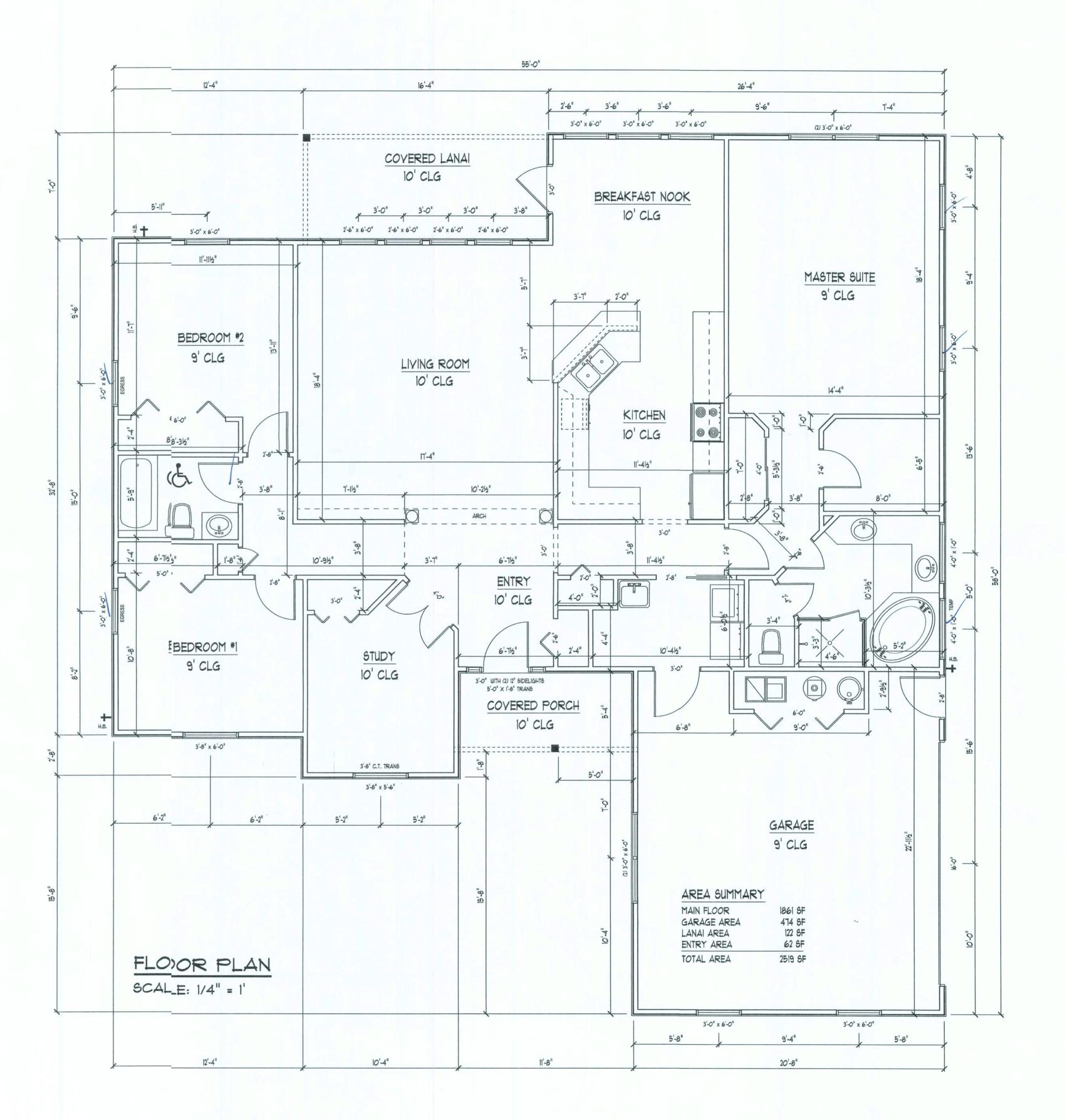
P.O. Box 73

ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRICEDENCE OVER SCALED DIMENSIONS



TYPICAL WALL SECTION

SCALE: 1" = 1'0"



September 26, 2006

Studios

D.D.B. STUDIOS

P.O. Box 273

Lake City FL. 32056
(386) 154-0181

O. Box 273 Ke City FL. 32056 86) 154-0181

CALLAWAY, LOT (4)

SITE CONDITIONS, IT IS THE TRUCTURE IS BUILT IN STRICT

E, AND FEDERALL, IF YOUR CITY

DONE LOCALLY BY A QUALIFIED

CAL

CAL

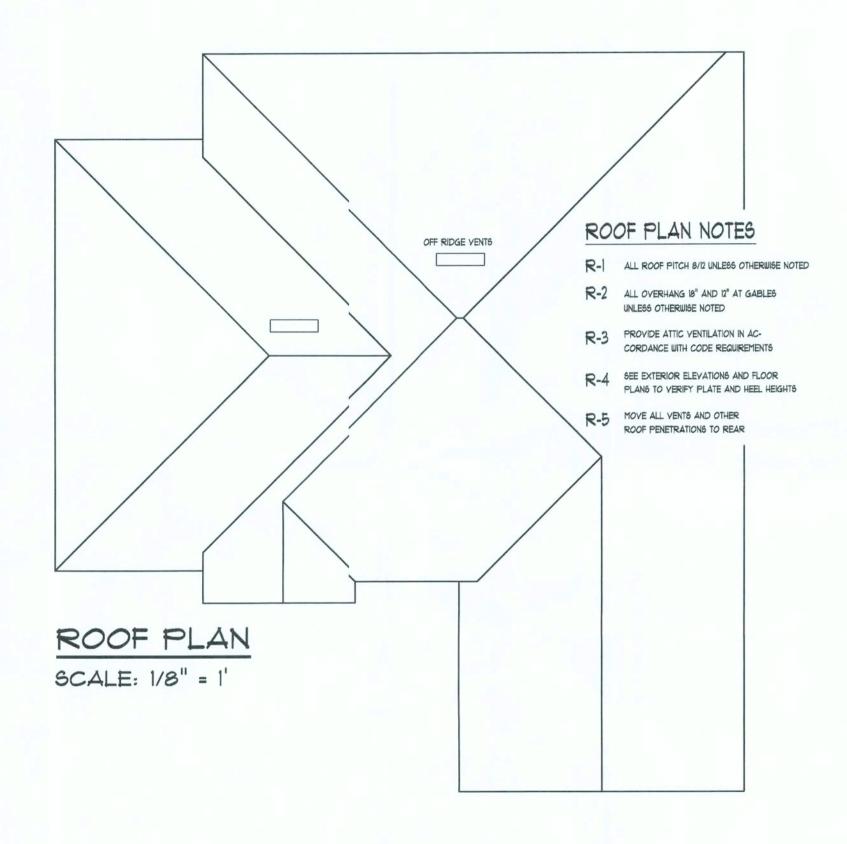
LAKE CITY, EL AT THE TIME THEY ARE DRAFTED. DUE TO VARYING STATULES AND RECULATIONS, DOES STUDIOS CANNOT WARRANT COMPLIANCE WOCAL, AND NATIONAL CODES IN YOUR AREA OR WITH YOUR PARTICULAR ESPONSIBILITY OF THE PURCHASER AND/OR BUILDER TO SEE THAT THE SOMPLIANCE WITH ALL GOVERNING MUNICIPAL CODES (CITY, COUNTY, STATE REQUIRES AN ENGINEER'S STAMP, YOU WILL NEED TO HAVE THIS

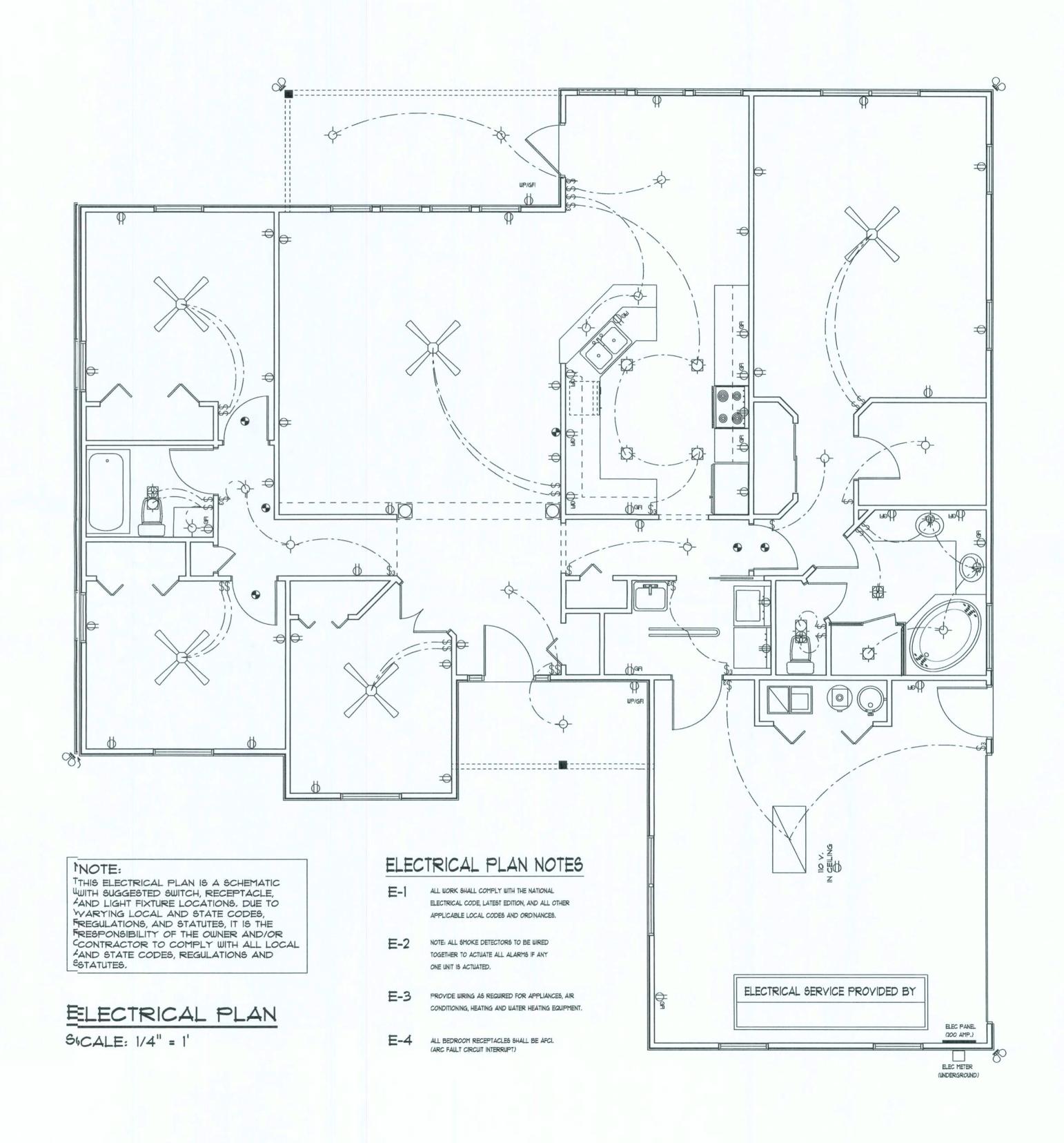
PICAL WALL SECTION

3 of 4

All work shall comply with the stancard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.





E BY COMPASS BUILDERS:

LANIE AND STUDIOS

VAY, LOT & 43

2000 DDS STUDIOS

VAY, LOT & 43

2000 DDS STUDIOS

THE LANI

CALLAWAY, LOT & 43

COPYRIGHT: 2000 DDS STUDIOS

OUR PLANS ARE DRAFTED FOR AVERAGE SITE CONDITIONS AND COMPLIANCE WITH APPLICABLE COD IN LAKE CITY, FL. AT THE TIME THEY ARE DRAFTED. DUE TO VARYING STATE, LOCAL, AND NATIONAL CO RULES AND REGULATIONS, DDS STUDIOS CANNOT WARRANT COMPLIANCE WITH ALL APPLICABLE STATE, LOCAL, AND NATIONAL CODES IN YOUR AREA OR WITH YOUR PARTICULAR SITE CONDITIONS, IT IS THE RESPONSIBILITY OF THE PURCHASER AND/OR BUILDER TO SEE THAT THE STRUCTURE IS BUILT IN STRICT COMPLIANCE WITH ALL COVERNING MUNICIPAL CODES (CITY, COUNTY, STATE, AND FEDERALL), IF YOUR OR STATE REQUIRES AN ENGINEES STAMP YOU WILL NEED TO HAVE THIS DONE LOCALLY BY A QUALTE.

CTRICAL PLAN

ROOF FLAN

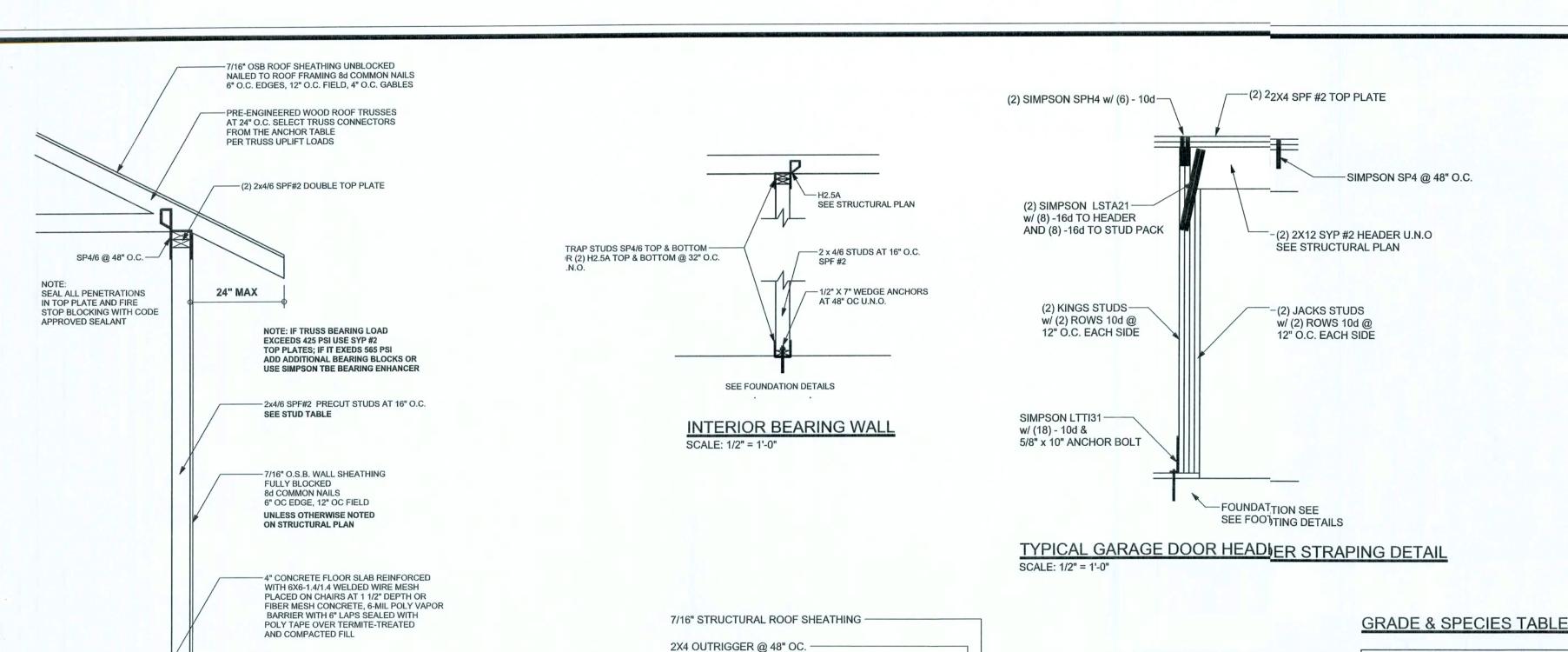
SHEET NUMBER

4 of 4

All work shall comply with the standard building code, and all applicable local codes and ordinaices.

Contractor shall verify all dimensions prior to

commencing constuction.



- HURRICANE CLIP H-2.5 OR I EQUAL BLOCKING REQUIRED BETWEEN OUTRIGGERS -2X4 BARGE RAFTER CONT. (3) .131 X 3 1/4 " GUN NAILS -SHINGLE STRIP 2X4 BLOCKING @ SHEATHING JOINT 4' FROM GABLE END -- FASCIA TOP CHORD OF GABLE ENGD TRUSS 2X4 SCAB CONT. DP TO DROP 3 1/2" CHORD@ 8' FRONGABLE -CONT. 2X4 SCAB FROM TORP TO BOTTOM CHORD @ X-BRACCING 4 - 10d NAILS OR 4 .131"x 3.25" (PROVIDE ADDITIONAL 2X4'\$'S @ TYPICAL AT ALL CONECTIONS L VERTICAL IF HIGHER THAN 1 48". TO FORM AN "L" SHAPE.) 2X4 SCAB IF VERTWEB IS NOT PRESENT -TOE NAIL TRUSS TO DOUBLILE PLATE w/ 16d COM @8" OC.: CONT. 2X4X8' #2 SP LATERAL BOTTOM CHORD OF GABLEF END TRUSS BRACE @ 48" OC. -- 2 - 2X4 TOP PLATE SIMPSON LSTA 24 @ 48" OCC. 2X4 BLOCKING @ 3" OC. BETWEEN GABLE ND FIRST -2X4 STUDS @16" OC. TRUSS. 2X4 X-BRACE @ 6'-0" OC. -

TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

PSL PARALAM 2900 2.0 C-BRACCING AL 2X4'\$'S @ THAN 1 48", APE.) DOUBLIE 28" OC.; DOUBLE 2x4 SPF TOP PLATE NAILED TOGETHER W/2-16d NAILS AT 16" O.C. 4' MIN. LAP w/ (12) - 16d OR 4" LAP w/ CS20 w/ (4) - 16d &(14) - 10d INTERIOR CEILING AS SPECIFIED ON FLOOR PLAN CONTINUOUS FRAME TO TOP PLATE AT BOTTOM CHORD OF TRUSS

AND BOTTOM PLATES

WITH 2-16d NAILS

| GLB |

CONTINUOUS FRAME
TO TOP PLATE AT
BOTTOM CHORD OF TRUSS

ALL STUDS TO BE 2x4
SPF NAILED TO TOP

SYP #2

SYP #2

SYP #2

24F-V3 SP

TIMBERSTRAND

MICROLAM

Fb (psi) E (10⁶ psi)

1.6

1.6

1.6

1.8

1.9

1200

1050

975

2400

1600

CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

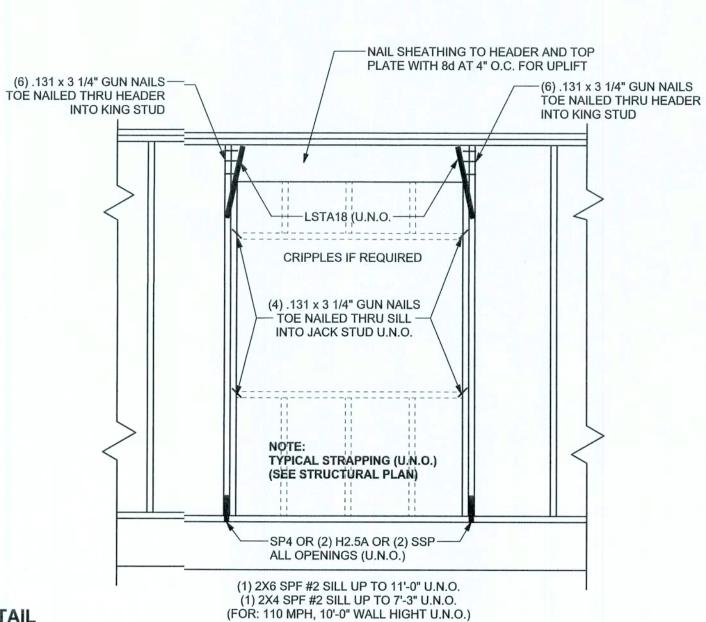
NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	DER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AF FALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	IATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 ENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE TH	CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU HE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL LOAD ENGINEER IMMEDIATELY.
DESIGN, PL TRUSS-TO-	E TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS ACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL OCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements			
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi			
2.1	Mortar	ASTM C 270, Type N, UNO			
2.2	Grout	ASTM C 476, admixtures require approva			
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block			
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"			
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)			
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS			
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS			
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.			
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not			

detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

			. O I ENILO	TO KALIERIKUSS	10 21002
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	Н3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS TO STUDS

PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled

WINDLOAD ENGINEER: Mark Disosway,

dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly rese

REVISIONS

SOFTPIAN

form or manner without first the express writter permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building

code residential 2004, to the best of my

its common law copyrights and property right in these instruments of service. This document is

not to be reproduced, altered or copied in any

Knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Compass Builders

Spec House Lot 43 Callaway S/D

> ADDRESS: Lot 43 Callaway S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
October 16, 2006

DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE:

JOB NUMBER: 610053 DRAWING NUMBER

S-1

OF 3 SHEETS

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING EXAMPLE 16" O.C. x 0.85 = 13.6" O.C. SIMPSON H2.5A U.N.O. -SEE STRUCTURAL PLAN (2) 2X10 SYP #2 U.N.O. SEE STRUCTURAL PLAN (2) SIMPSON LSTA21w/ (8) -16d TO HEADER AND (8) -16d TO POST 6X6 SYP #2 POST -SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT —SEE FOOTING DETAILS TYPICAL PORCH POST DETAIL SCALE: 1/2" = 1'-0"

-2x4/6 P.T. PINE SOLE PLATE ANCHORED WITH

WITH 1/2"X10" ANCHOR BOLTS WITH 2X2X.140"

STEEL WASHER 48" O.C. & 8" FROM CORNERS

SP4/6 @ 48" O.C. -

SCALE: 3/4" = 1'-0"

ONE STORY WALL SECTION

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT

(1) 2x6 @ 16" OC TO 18'-10' STUD HEIGHT

TOGETHER W/2-16d
NAILS AT 16" O.C.
MIN. (SEE STRUCTURAL PLAN)

BEAM MID-VALL CONNECTION DETAIL

SCALE: N.T.S.

(2) 2X12 SYP #2 MIN.
SEE STRUCTURAL PLAN

SIMPSON
LSTA24

NAIL THRU 2x4VTO
BEAM W/4-16d

SIMPSON HUS412 MIN.
SEE STRUCTURAL PLAN

BEAM MAY BE ATACHED IN
EITHER METHODHOWN ABOVE

-(4)-2x4 SPF #2 NAILED

(2) 2X12 SYP #2 MIN. —— SEE STRUCTURAL PLAN

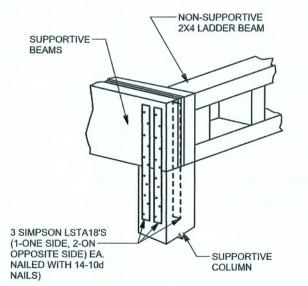
SIMPSON HUS412 MIN.

SEE STRUCTURAL PLA

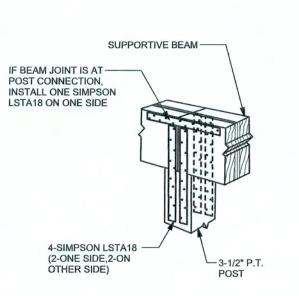
BEAM MAY BE ATACHED IN EITHER METHODHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

SCALE: N.T.S.



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL SCALE: N.T.S.

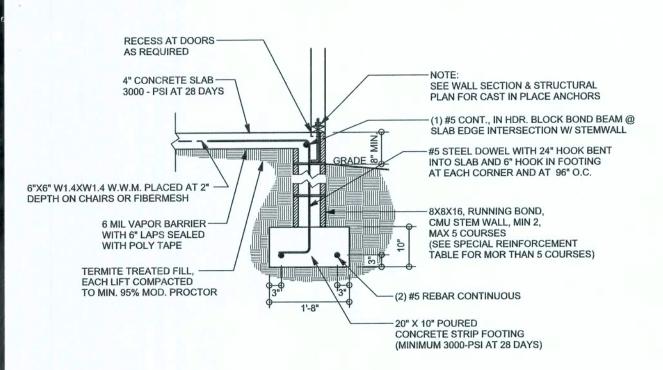
TYPICAL HEADER STRAPING DETAIL

DESIGN DATA

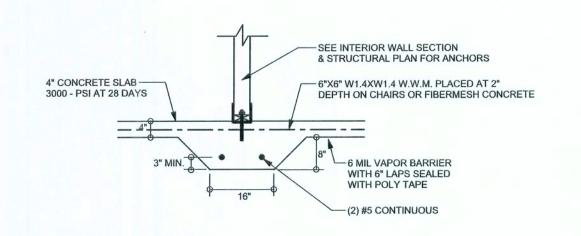
WIND LOADS PER FLORIDA BUILDING CODE 2					
ENCLOSED SIMPLE DIAPHRAGM BUILDINGS I MEAN ROOF HEIGHT NOT EXCEEDING LEAST ON UPPER HALF OF HILL OR ESCARPMENT 60 SLOPE AND UNOBSTRUCTED UPWIND FOR 50	HORIZONTAL D OFT IN EXP. B. 3	IMEN	SION I FXP	OR 60) FT; NOT ND >10%
BUILDING IS NOT IN THE HIGH VELOCITY HUR	RICANE ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEBRIS	S REGION				
1.) BASIC WIND SPEED = 110 MPH					
2.) WIND EXPOSURE = B					
3.) WIND IMPORTANCE FACTOR = 1.0					
4.) BUILDING CATEGORY = II					
5.) ROOF ANGLE = 10-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT = N//	A (ENCLOSED B	HILDI	NG)		
8.) COMPONENTS AND CLADDING DESIGN V				R301.	2(2))
	Zone	Effect	ive W	ind Are	ea (ft2)
			0		100
	1	19.9	-21.8	18.1	-18.1
2 2	2	19.9	-25.5	18.1	-21.8
5	2 O'hg		-40.6		-40.6
2 2 2 5	3	19.9	-25.5	18.1	-21.8
4 3 4	3 O'hg	04.0	-68.3		-42.4
	5		-23.6		-20.4
**	5	21.8	-29.1	18.5	-22.6
The state of the s	Doors	& Wind	lows	21.8	-29.1
24 2		st Cas	-		
		5, 10			
5 2 3	8x7 Gar 16x7 Ga			19.5 18.5	-22.9 -21.0

~	4 2/ 4 5	16x7 Garage Door	18.5	-21.0
	55 22			

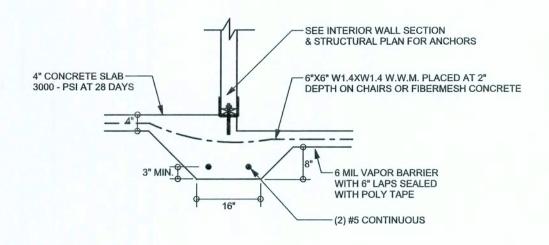
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			
NOT IN I	FLOOD ZONE (BUILDER TO VERIFY)			



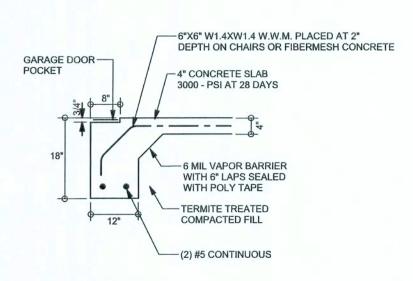
F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



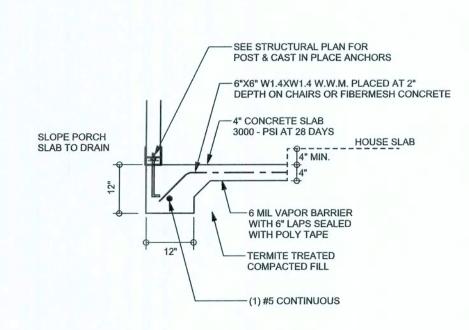
F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



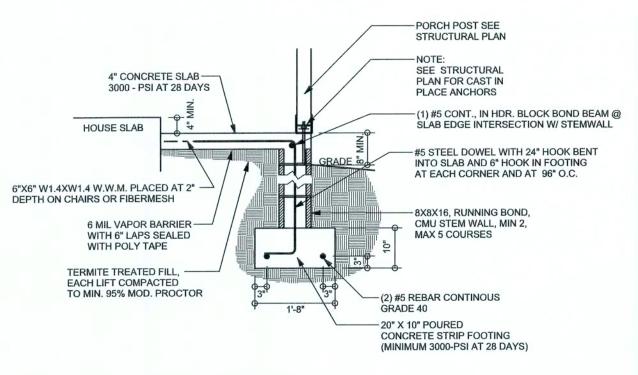
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

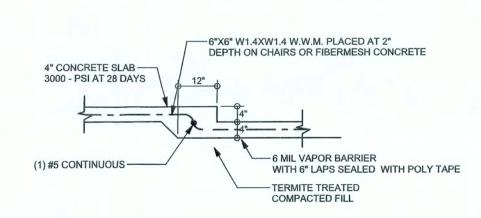


F12 OPTIONAL STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

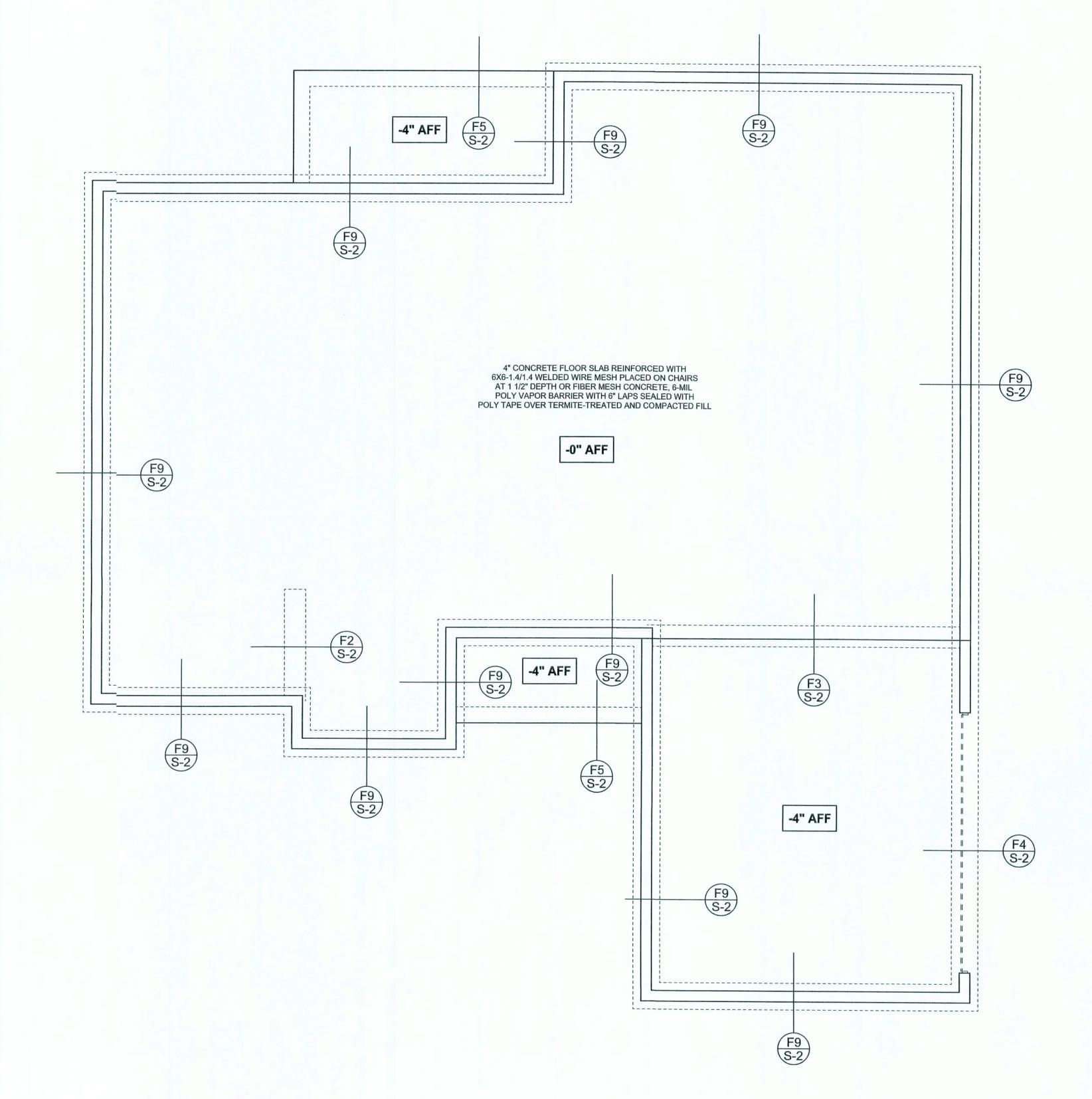
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F6 TYPICAL NON - BEARING STEP FOOTING



REVISIONS

SOFTPIXN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Compass Builders

MARK DISOSWAY P.E. 53915

Spec House Lot 43 Callaway S/D

> ADDRESS: Lot 43 Callaway S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
October 16, 2006

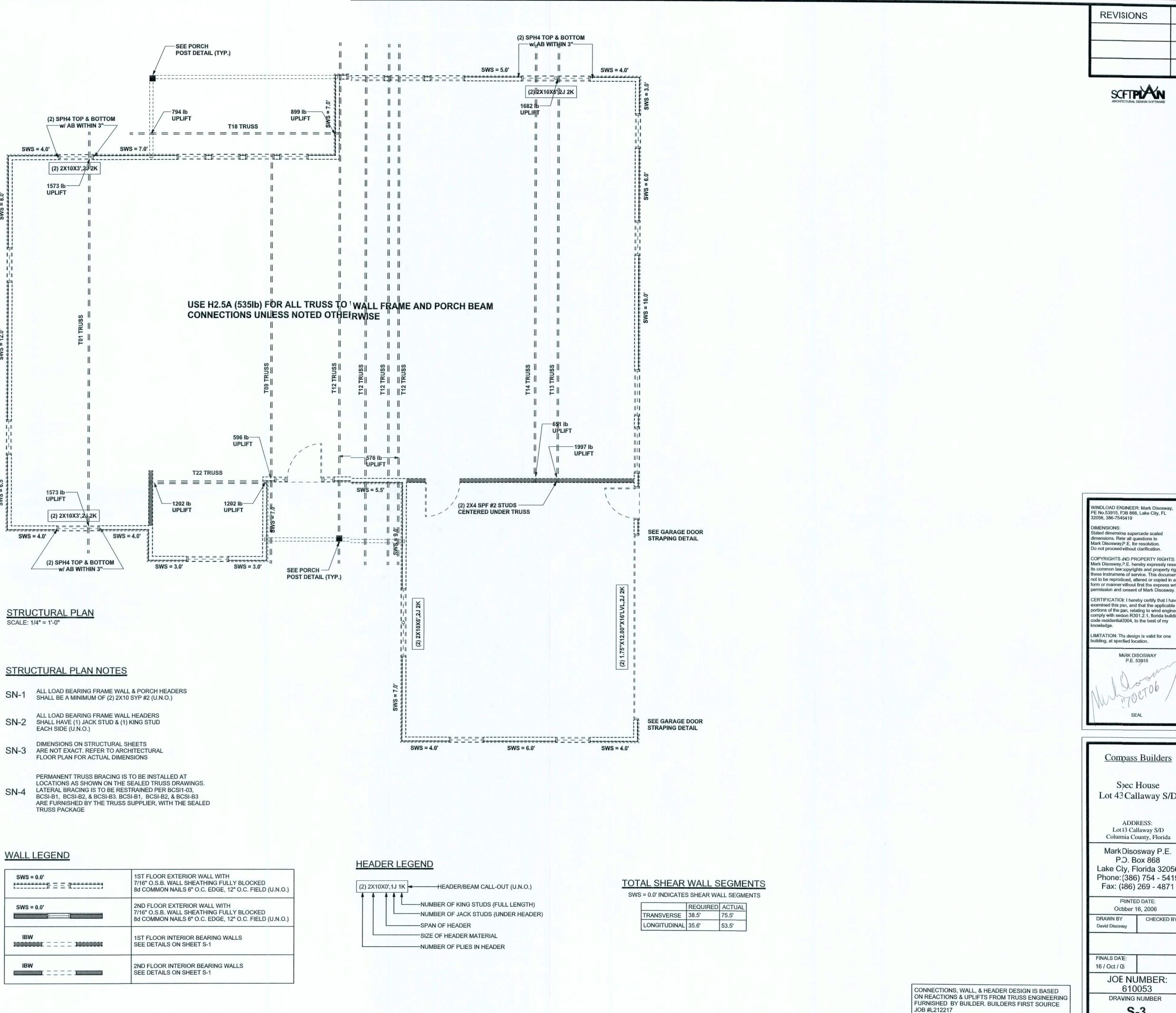
DRAWN BY: CHECKED BY:

David Disosway

FINALS DATE: 16 / Oct / 06

JOB NUMBER: 610053 DRAWING NUMBER

> S-2 OF 3 SHEETS



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LIMITATION: The design is valid for one building, at specfied location.

MARK DISOSWAY P.E. 53915

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FINALS DATE:

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S-3

OF 3 SHEETS