DATE 08/09/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000026109

APPLICANT JAMES NORTON PHONE 752-3331

ADDRESS 3367 S US HIGHWAY 441 LAKE CITY FL 32025

OWNER JOE OSBURN PHONE 752-7544

ADDRESS 418 NW MAXMORE DRIVE LAKE CITY FL 32055

CONTRACTOR JAMES NORTON PHONE 752-3331

LOCATION OF PROPERTY HIGHWAY 41N, TL ON 25A, TR ON BELL,STRAIGHT AHEAD TO DEADEND

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 96700.00

HEATED FLOOR AREA 1934.00 TOTAL AREA 2876.00 HEIGHT ____ STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RR MAX. HEIGHT ____

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. ____

PARCEL ID 12-3S-16-02096-001 SUBDIVISION ____

LOT ____ BLOCK ____ PHASE ____ UNIT ____ TOTAL ACRES ____

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
		RB0031780	James H Norton

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	07-572	BK	JH	N

COMMENTS: NOC ON FILE, ONE FOOT ABOVE THE ROAD, MH MUST BE REMOVED 45 DAYS AFTER CO ISSUANCE

Check # or Cash 23659

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by

Foundation ____ date/app. by

Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by

Slab ____ date/app. by

Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by

Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by

Heat & Air Duct ____ date/app. by

Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by

C.O. Final ____ date/app. by

Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by

Pool ____ date/app. by

Reconnection ____ date/app. by

Pump pole ____ date/app. by

Utility Pole ____ date/app. by

M/H Pole ____ date/app. by

Travel Trailer ____ date/app. by

Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 485.00 CERTIFICATION FEE $ 14.38 SURCHARGE FEE $ 14.38

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ ____

FLOOD DEVELOPMENT FEE $ ____ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ ____ **TOTAL FEE** 588.76

INSPECTORS OFFICE Gale Tedder CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

CC# 23659

Columbia County Building Permit Application

For Office Use Only Application # 0708-03 Date Received 8/2/07 By GT Permit # 26109

Application Approved by - Zoning Official BLK Date 08.08.07 Plans Examiner OKJTH Date 8-3-07

Flood Zone X Development Permit N/A Zoning RR Land Use Plan Map Category Res. V.L. Dev.

Comments See ATTACHED LETTER REGARDING PLANS JTH If MH on property to be removed 45 day after CO being issued

☐ NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Permit

Name Authorized Person Signing Permit Charese Norton Fax 386-752-6427

Address 3367 S. US. HWY 441, Ste 101, Lake City, FL 32025 Phone 386-752-3331

Owners Name Joe Osburn Phone 386-752-7544

911 Address 418 NW Maxmore Dr., Lake City, FL 32055

Contractors Name James H. Norton Phone 386-752-3331

Address 3367 S. US HWY 441, Ste 101, Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Nicholas Paul Geisler, 1758 NW Brown Rd., Lc. FL 3205

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 12-3S-16-02096-001 Estimated Cost of Construction 125000.00

Subdivision Name — Lot — Block — Unit — Phase —

Driving Directions HWY 41 North, TL on CR25A, TR on Bell, Stay Straight (Maxmore) @ dead end

Type of Construction SFD, New home Constructn Number of Existing Dwellings on Property 0

Total Acreage 10 Lot Size — Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 498.51' Side 120' Side 449.55' Rear 130'

Total Building Height 19'2" Number of Stories 1 Heated Floor Area 1934 TOTAL 2,876 Roof Pitch 4/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James H. Norton
Owner Builder or Authorized Person by Notarized Letter

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 1 day of Aug 2007

Personally known ✓ or Produced Identification ____

PATRICIA T. PEELER
Notary Public, State of Florida
Comm. exp. Sep. 5, 2010
Comm. No. DD 579471

James H. Norton
Contractor Signature

Contractors License Number RB0031780

Competency Card Number 5553

NOTARY STAMP/SEAL

Patricia T. Peeler
Notary Signature

(Revised Sept. 2006)

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0708-03

Application Charese Norton, Contractor James Norton Owner Joe Osburn
Property: ID# 12-3s-16-02096-001

The submitted plans were designed by Architectural Drafting & Design, Inc.

Mr. Nicholas Paul Geisler, Architect signed and provides his Florida architect license number AR0007005 but did not affix his embossed seal on the plans. Mr. Mark Disosway P.E. 53951 has reviewed these plans, and assigned 706271as a job number with drawing sheet S1, S2 & S3 which were submitted with the plans.

When constructing this structure use only Mr. Disosway plans, sheets S1, S2 & S3 for structural requirement to comply with the Florida Building Codes.

Thank You:

Joe Haltiwanger
Plan Examiner
Columbia County Building
Department

Columbia County Property Appraiser

DB Last Updated: 5/11/2007

Parcel: 12-3S-16-02096-001 HX

2007 Proposed Values

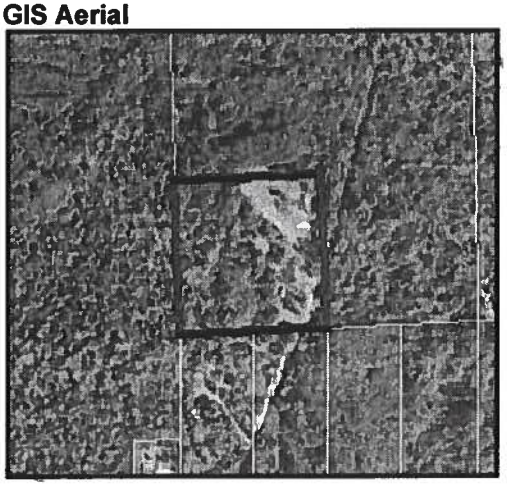
Tax Record | Property Card | Interactive GIS Map | Print

Search Result: 1 of 1

Owner & Property Info

Owner's Name	OSBURN JULIA R & JOE RANDALL		
Site Address	MAXMORE		
Mailing Address	418 NW MAXMORE DR LAKE CITY, FL 32055		
Use Desc. (code)	MOBILE HOM (000200)		
Neighborhood	12316.00	**Tax District**	3
UD Codes	MKTA06	**Market Area**	06
Total Land Area	10.000 ACRES		
Description	BEG SW COR OF NE1/4 OF SE1/4, RUN N 663.90 FT, E 636.95 FT, S 664.17 FT, W 633.75 FT TO POB. (BEING IN SW1/4 OF NE1/4 OF SE1/4. ORB 297-366, 35-655, 854-2151, QCD 1013-1766.		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (4)	$74,750.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (1)	$2,707.00
XFOB Value	cnt: (2)	$2,326.00
Total Appraised Value		$79,783.00

Just Value		$79,783.00
Class Value		$0.00
Assessed Value		$54,907.00
Exempt Value	(code: HX)	$25,000.00
Total Taxable Value		$29,907.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
3/9/1998	854/2151	WD	V	U	01	$0.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	MOBILE HME (000800)	1968	Below Avg. (03)	648	648	$2,707.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0021	BARN,FR AE	0	$1,030.00	1.000	24 x 15 x 0	(.00)
0031	BARN,MT AE	0	$1,296.00	288.000	12 x 24 x 0	(.00)

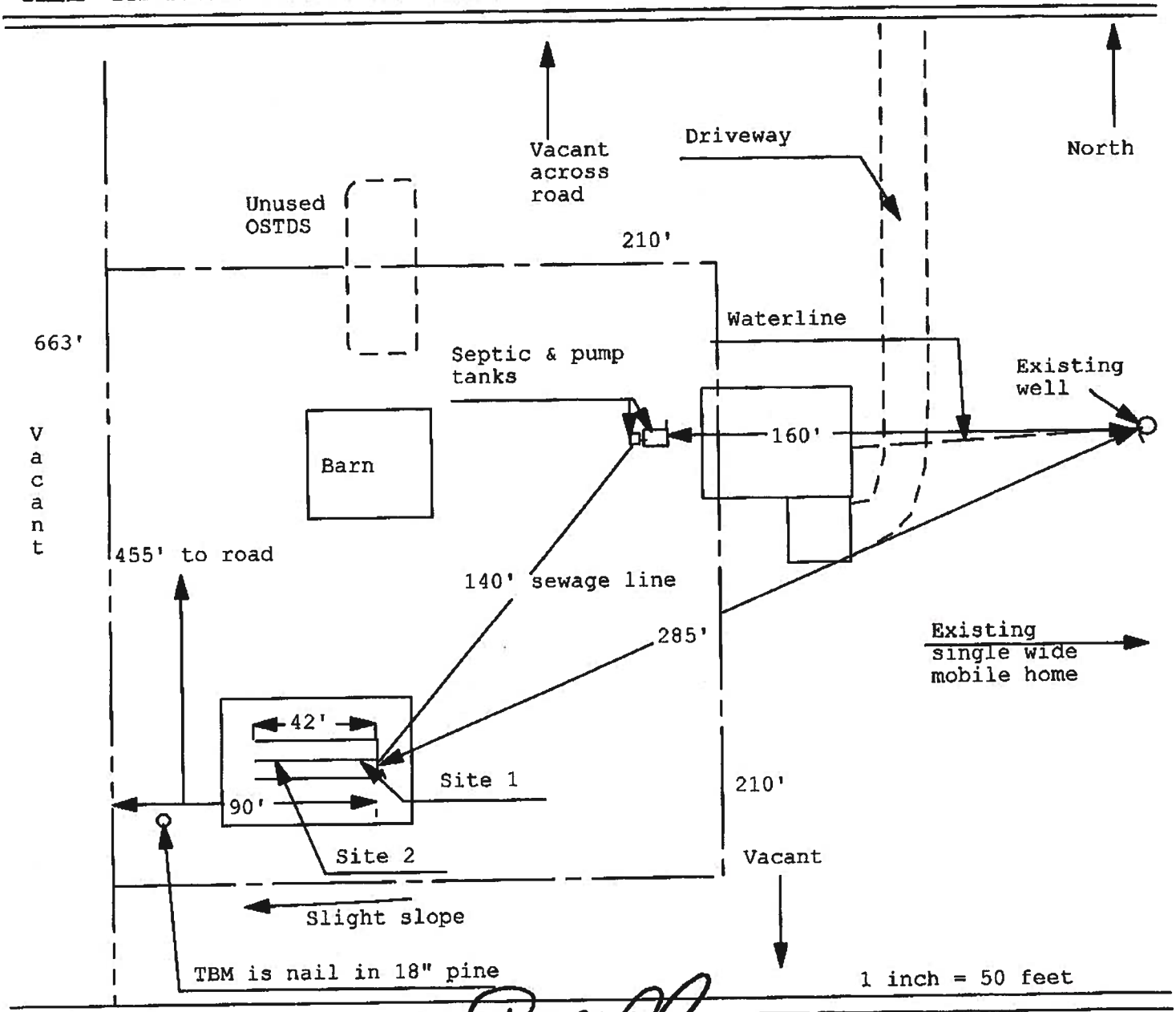
Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000200	MBL HM (MKT)	5.000 AC	1.00/1.00/1.00/.80	$7,200.00	$36,000.00
000200	MBL HM (MKT)	5.000 AC	1.00/1.00/1.00/.80	$7,200.00	$36,000.00
009945	WELL/SEPT (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	$2,000.00	$2,000.00
009947	SEPTIC (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	$750.00	$750.00

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07-0572

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul Lloyd **Date** 7/12/07

Plan Approved ✓ **Not Approved** ____ **Date** 7/18/07

By [signature] Columbia **CPHU**

Notes: _____

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 12-3S-16-02096-001

1. **Description of property: (legal description of the property and street address or 911 address)**
   418 Maxmore Dr., Lake City, FL 32055

Inst:200712017242 Date:8/1/2007 Time:8:46 AM
DC,P.DeWitt Cason ,Columbia County Page 1 of 1

2. **General description of improvement:** New Home Construction
3. **Owner Name & Address** Joe Osburn, 418 Maxmore Dr., Lake City, FL 32055
   Interest in Property
4. **Name & Address of Fee Simple Owner (if other than owner):** NA
5. **Contractor Name** James Norton **Phone Number** 386-752-3331
   Address 3367 S US Hwy 441, Suite 101, Lake City, FL 32025
6. **Surety Holders Name** NA **Phone Number**
   Address
   Amount of Bond
7. **Lender Name** NA **Phone Number**
   Address
8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name NA **Phone Number**
   Address
9. **In addition to himself/herself the owner designates** James H. Norton **of** Norton Home Imp. Co., Inc **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –**
   (a) 7. Phone Number of the designee 386-752-3331
10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording,**
    (Unless a different date is specified)

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

James R. Osburn
Signature of Owner

Sworn to (or affirmed) and subscribed before day of Aug 1, 2007

NOTARY STAMP/SEAL

PATRICIA T. PEELER
Notary Public, State of Florida
My comm. exp. Sep. 5, 2010
Comm. No. DD 579471

Patricia T. Peeler
Signature of Notary

THIS DOCUMENT PREPARED BY
FRANK M. GAFFORD
ATTORNEY AT LAW
228 EAST DUVAL STREET
LAKE CITY, FLORIDA

98-03725

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY, FL

1998 MAR 11 AM 9:03

RECORD VERIFIED
CLERK OF COURTS
COLUMBIA COUNTY, FLORIDA
BY _____ D.C.

WARRANTY DEED

THIS WARRANTY DEED, Made this 9th day of March, 1998, BETWEEN JULIA GEOGHAGAN OSBURN, a/k/a JULIA ROBERTA OSBURN, a/k/a JULIA OSBURN, whose address is Route 8, Box 1641, Lake City, Columbia County, Florida, grantor and JULIA GEOGHAGAN OSBURN, a/k/a JULIA ROBERTA OSBURN, a/k/a JULIA OSBURN, JOE RANDAL OSBURN, AND JEFF ALLEN OSBURN, whose Post Office is Route 8, Box 1641, Lake City, Florida, grantee,

WITNESSETH, that the said party of the first part, in consideration of the sum of TEN DOLLARS, ($10.00) to him in hand paid by the parties of the second part, does hereby grant and release unto the parties of the second part as joint tenants, with right of survivorship, and to their survivors, and to the heirs and assigns of such survivors, the following described land, situate, lying and being in the County of Columbia, State of Florida, to wit:

The SW1/4 of the NE1/4 of the SE1/4 of Section 12, Township 3 South Range 16 East.

The above parcel being more particularly described as follows:

Begin at the Southwest corner of the NE 1/4 of the SE 1/4, Section 12, Township 3 South, Range 16 East, and run North 0° 25' 18" West, along the West line of said NE 1/4 of the SE 1/4, 663.90 feet, thence North 87° 54' East, 636.95 feet,thence South 0°08'31" East, 664.17 feet, to the South line of said NE1/4 of the SE 1/4, thence South 87° 55' West, 633.75 feet along the South line of said NE 1/4 of the SE 1/4, to the POINT OF BEGINNING.

Being subject to a 30.00 foot road and utility easement in the Southeast portion. Containing 9.68 acres more or less.

Together with the appurtenances and all the estate and rights of the party of the first part in and to said premises.

IN WITNESS WHEREOF, Grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered
in our presence:

FRANK M. GAFFORD

Julia Geoghagan Osburn

JULIA GEOGHAGAN OSBURN
a/k/a JULIA ROBERTA OSBURN
a/k/a JULIA OSBURN

Doc. Stamps Tax $ .70
P. DeWitt Cason
Clerk of Court
By _____ D.C.

BK 0854 PG 2151

OFFICIAL RECORDS

BK 0854 PG 2152

OFFICIAL RECORDS

STATE OF FLORIDA
COUNTY OF COLUMBIA

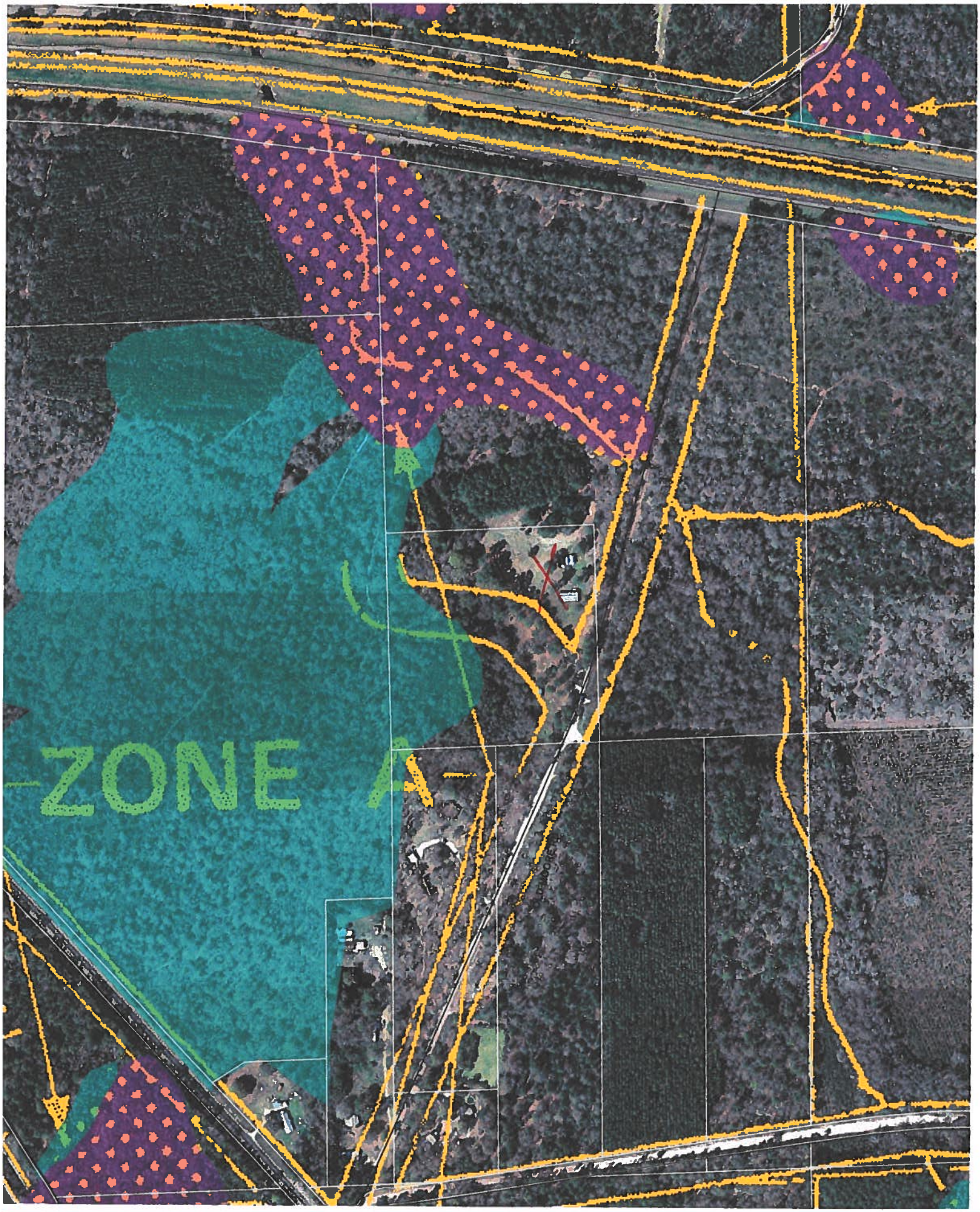
I HEREBY CERTIFY that on this day, before me, an officer duly qualified to take acknowledgments, personally appeared **JULIA GEOGHAGAN OSBURN, a/ka/ JULIA ROBERTA OSBURN, a/k/a JULIA OSBURN** to me known to be the person described in and who executed the foregoing instrument and acknowledged before me that they executed the same.

WITNESS my hand and official seal in the County and State last aforesaid this 9th day of March, 1998.

SUZETTE WOOLSEY
MY COMMISSION # CC 387270
EXPIRES: April 25, 1998
Bonded Thru Notary Public Underwriters

Suzett Woolsey
Notary Public:
SUZETTE WOOLSEY
My Commission expires:

JULIA OSBURN.WDEED.wpd



0708-03

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	Mr. Joe Osburn	Builder:	Norton
Address:		Permitting Office:	Columbia
City, State:	,	Permit Number:	2ce109
Owner:		Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 36.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 14.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	No		
6. Conditioned floor area (ft²)	1934 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAULT) 7a.	(Dble Default) 284.0 ft²	a. Electric Heat Pump	Cap: 36.0 kBtu/hr
b. SHGC:			HSPF: 8.70
(or Clear or Tint DEFAULT) 7b.	(Clear) 284.0 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=1.0, 208.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 80.0 gallons
a. Face Brick, Wood, Exterior	R=11.0, 1872.0 ft²		EF: 0.95
b. N/A		b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar	
10. Ceiling types		DHP-Dedicated heat pump)	
a. Under Attic	R=30.0, 1934.0 ft²	15. HVAC credits	CF,
b. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
c. N/A		HF-Whole house fan,	
11. Ducts		PT-Programmable Thermostat,	
a. Sup: Unc. Ret: Con. AH: Interior	Sup. R=6.0, 55.0 ft	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	

Glass/Floor Area: 0.15 Total as-built points: 22451 Total base points: 29429 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 7/31/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1934.0	20.04	6976.3	Double, Clear	N	1.3	5.0	6.0	19.20	0.93	107.3	
				Double, Clear	N	1.3	5.0	9.0	19.20	0.93	160.9	
				Double, Clear	N	5.7	7.0	45.0	19.20	0.74	642.0	
				Double, Clear	N	9.3	7.0	30.0	19.20	0.67	385.1	
				Double, Clear	N	1.3	7.0	30.0	19.20	0.97	556.4	
				Double, Clear	E	1.3	7.0	15.0	42.06	0.96	602.8	
				Double, Clear	E	1.3	6.0	8.0	42.06	0.93	313.8	
				Double, Clear	S	1.3	7.0	75.0	35.87	0.92	2475.0	
				Double, Clear	S	1.3	7.0	20.0	35.87	0.92	660.0	
				Double, Clear	S	7.3	7.0	30.0	35.87	0.51	552.8	
				Double, Clear	W	1.3	6.0	16.0	38.52	0.93	575.2	
				As-Built Total:				**284.0**			**7031.3**	

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Exterior	1872.0	1.70	3182.4	Face Brick, Wood, Exterior	11.0	1872.0	0.40	748.8
Adjacent	0.0	0.00	0.0					
Base Total:	**1872.0**		**3182.4**	**As-Built Total:**		**1872.0**		**748.8**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Exterior	80.0	4.10	328.0	Exterior Insulated	80.0	4.10	328.0
Adjacent	0.0	0.00	0.0				
Base Total:	**80.0**		**328.0**	**As-Built Total:**	**80.0**		**328.0**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	1934.0	1.73	3345.8	Under Attic	30.0	1934.0	1.73 X 1.00	3345.8
Base Total:	**1934.0**		**3345.8**	**As-Built Total:**		**1934.0**		**3345.8**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	208.0(p)	-37.0	-7696.0	Slab-On-Grade Edge Insulation	1.0	208.0(p	-39.87	-8292.3
Raised	0.0	0.00	0.0					
Base Total:			**-7696.0**	**As-Built Total:**		**208.0**		**-8292.3**

INFILTRATION	Area X	BSPM	= Points		Area X	SPM	= Points
	1934.0	10.21	19746.1		1934.0	10.21	19746.1

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 25882.7				**Summer As-Built Points:**						**22907.8**
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
				(sys 1: Central Unit 36000 btuh ,SEER/EFF(14.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS)						
				22908	1.00	(1.08 x 1.147 x 0.91)	0.244	0.950		5986.1
25882.7	**0.4266**		**11041.6**	**22907.8**	**1.00**	**1.128**	**0.244**	**0.950**		**5986.1**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1934.0	12.74	4435.0	Double, Clear	N	1.3	5.0	6.0	24.58	1.00	147.9	
				Double, Clear	N	1.3	5.0	9.0	24.58	1.00	221.8	
				Double, Clear	N	5.7	7.0	45.0	24.58	1.02	1123.5	
				Double, Clear	N	9.3	7.0	30.0	24.58	1.02	753.2	
				Double, Clear	N	1.3	7.0	30.0	24.58	1.00	738.0	
				Double, Clear	E	1.3	7.0	15.0	18.79	1.02	287.7	
				Double, Clear	E	1.3	6.0	8.0	18.79	1.03	154.6	
				Double, Clear	S	1.3	7.0	75.0	13.30	1.04	1041.9	
				Double, Clear	S	1.3	7.0	20.0	13.30	1.04	277.8	
				Double, Clear	S	7.3	7.0	30.0	13.30	2.81	1120.6	
				Double, Clear	W	1.3	6.0	16.0	20.73	1.02	337.6	
				As-Built Total:				**284.0**			**6204.7**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Exterior	1872.0	3.70	6926.4	Face Brick, Wood, Exterior			11.0	1872.0	3.50		6552.0	
Adjacent	0.0	0.00	0.0									
Base Total:	**1872.0**		**6926.4**	**As-Built Total:**				**1872.0**			**6552.0**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Exterior	80.0	8.40	672.0	Exterior Insulated				80.0	8.40		672.0	
Adjacent	0.0	0.00	0.0									
Base Total:	**80.0**		**672.0**	**As-Built Total:**				**80.0**			**672.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1934.0	2.05	3964.7	Under Attic			30.0	1934.0	2.05 X	1.00	3964.7	
Base Total:	**1934.0**		**3964.7**	**As-Built Total:**				**1934.0**			**3964.7**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	208.0(p)	8.9	1851.2	Slab-On-Grade Edge Insulation			1.0	208.0(p	15.63		3251.7	
Raised	0.0	0.00	0.0									
Base Total:			**1851.2**	**As-Built Total:**				**208.0**			**3251.7**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1934.0	-0.59	-1141.1					1934.0	-0.59		-1141.1	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**16708.3**	**Winter As-Built Points:**					**19504.1**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 36000 btuh ,EFF(8.7) Ducts:Unc(S),Con(R),Int(AH),R6.0					
			19504.1	1.000	(1.060 x 1.169 x 0.93)	0.392	1.000	8809.8
16708.3	**0.6274**	**10482.8**	**19504.1**	**1.00**	**1.152**	**0.392**	**1.000**	**8809.8**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	80.0	0.95	3		1.00	2551.79	1.00	7655.4
				As-Built Total:							**7655.4**

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points		Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	
11042	**10483**	**7905**	**29429**		**5986**	**8810**	**7655**	**22451**	

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 87.6

The higher the score, the more efficient the home.

1.	New construction or existing	New		12.	Cooling systems	
2.	Single family or multi-family	Single family		a.	Central Unit	Cap: 36.0 kBtu/hr
3.	Number of units, if multi-family	1				SEER: 14.00
4.	Number of Bedrooms	3		b.	N/A	
5.	Is this a worst case?	No				
6.	Conditioned floor area (ft²)	1934 ft²		c.	N/A	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)					
a.	U-factor:	Description Area		13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 284.0 ft²		a.	Electric Heat Pump	Cap: 36.0 kBtu/hr
b.	SHGC:					HSPF: 8.70
	(or Clear or Tint DEFAULT) 7b.	(Clear) 284.0 ft²		b.	N/A	
8.	Floor types					
a.	Slab-On-Grade Edge Insulation	R=1.0, 208.0(p) ft		c.	N/A	
b.	N/A					
c.	N/A			14.	Hot water systems	
9.	Wall types			a.	Electric Resistance	Cap: 80.0 gallons
a.	Face Brick, Wood, Exterior	R=11.0, 1872.0 ft²				EF: 0.95
b.	N/A			b.	N/A	
c.	N/A					
d.	N/A			c.	Conservation credits	
e.	N/A				(HR-Heat recovery, Solar	
10.	Ceiling types				DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1934.0 ft²		15.	HVAC credits	CF,
b.	N/A				(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A				HF-Whole house fan,	
11.	Ducts				PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Con. AH: Interior	Sup. R=6.0, 55.0 ft			MZ-C-Multizone cooling,	
b.	N/A				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

BUILDING INPUT SUMMARY REPORT

PROJECT

Title:	Mr. Joe Osburn	**Family Type:**	Single	**Address Type:**	Street Address
Owner:	(blank)	**New/Existing:**	New	**Lot #:**	N/A
# of Units:	1	**Bedrooms:**	3	**Subdivision:**	N/A
Builder Name:	(blank)	**Conditioned Area:**	1934	**Platbook:**	N/A
Climate:	North	**Total Stories:**	1	**Street:**	(blank)
Permit Office:	Columbia	**Worst Case:**	No	**County:**	(blank)
Jurisdiction #:	(blank)	**Rotate Angle:**	(blank)	**City, St, Zip:**	, ,

FLOORS

#	Floor Type	R-Val	Area/Perimeter	Units
1	Slab-On-Grade Edge Insulation	1.0	208.0(p) ft	1

DOORS

#	Door Type	Orientation	Area	Units
1	Insulated	Exterior	20.0 ft²	4

CEILINGS

#	Ceiling Type	R-Val	Area	Base Area	Units
1	Under Attic	30.0	1934.0 ft²	1934.0 ft²	1

Credit Multipliers: None

COOLING

#	System Type	Efficiency	Capacity
1	Central Unit	SEER: 14.00	36.0 kBtu/hr

Credit Multipliers: Ceil Fn

WALLS

#	Wall Type	Location	R-Val	Area	Units
1	Face Brick - Wood	Exterior	11.0	1872.0 ft²	1

HEATING

#	System Type	Efficiency	Capacity
1	Electric Heat Pump	COP: 8.70	36.0 kBtu/hr

Credit Multipliers: None

WINDOWS

#	Panes	Tint	Ornt	Area	OH Length	OH Hght	Units
1	Double	Clear	N	6.0 ft²	1.3 ft	5.0 ft	1
2	Double	Clear	N	9.0 ft²	1.3 ft	5.0 ft	1
3	Double	Clear	N	15.0 ft²	5.7 ft	7.0 ft	3
4	Double	Clear	N	15.0 ft²	9.3 ft	7.0 ft	2
5	Double	Clear	N	15.0 ft²	1.3 ft	7.0 ft	2
6	Double	Clear	E	15.0 ft²	1.3 ft	7.0 ft	1
7	Double	Clear	E	8.0 ft²	1.3 ft	6.0 ft	1
8	Double	Clear	S	15.0 ft²	1.3 ft	7.0 ft	5
9	Double	Clear	S	10.0 ft²	1.3 ft	7.0 ft	2
10	Double	Clear	S	15.0 ft²	7.3 ft	7.0 ft	2
11	Double	Clear	W	16.0 ft²	1.3 ft	6.0 ft	1

DUCTS

#	Supply Location	Return Location	Air Handler Location	Supply R-Val	Supply Length
1	Uncond.	Cond.	Interior	6.0	55.0 ft

Credit Multipliers: None

WATER

#	System Type	EF	Cap.	Conservation Type	Con. EF
1	Electric Resistance	0.95	80.0	None	0.00

REFR.

#	Use Default?	Annual Operating Cost	Electric Rate
1	Yes	N/A	N/A

Residential System Sizing Calculation

Summary

Project Title:
Mr. Joe Osburn

Code Only
Professional Version
Climate: North

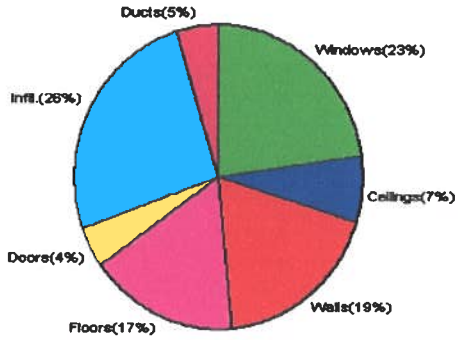
7/5/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**35146**	**Btuh**	**Total cooling load calculation**	**29503**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	102.4	36000	Sensible (SHR = 0.75)	126.0	27000
Heat Pump + Auxiliary(0.0kW)	102.4	36000	Latent	111.5	9000
			Total (Electric Heat Pump)	122.0	36000

WINTER CALCULATIONS

Winter Heating Load (for 1934 sqft)

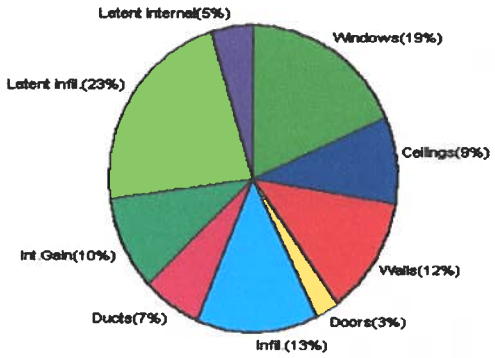
Load component			Load	
Window total	284	sqft	8037	Btuh
Wall total	1872	sqft	6552	Btuh
Door total	80	sqft	1466	Btuh
Ceiling total	1934	sqft	2514	Btuh
Floor total	208	ft	5928	Btuh
Infiltration	209	cfm	8974	Btuh
Subtotal			**33472**	**Btuh**
Duct loss			1674	Btuh
TOTAL HEAT LOSS			**35146**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1934 sqft)

Load component			Load	
Window total	284	sqft	5469	Btuh
Wall total	1872	sqft	3632	Btuh
Door total	80	sqft	811	Btuh
Ceiling total	1934	sqft	2746	Btuh
Floor total			0	Btuh
Infiltration	193	cfm	3822	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**19480**	**Btuh**
Duct gain			1948	Btuh
Total sensible gain			**21428**	**Btuh**
Latent gain(infiltration)			6695	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**8075**	**Btuh**
TOTAL HEAT GAIN			**29503**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: _____

DATE: _____

System Sizing Calculations - Winter

Residential Load - Component Details

Project Title:
Mr. Joe Osburn

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

7/5/2007

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	6.0	28.3	170 Btuh
2	2, Clear, Metal, DEF	N	9.0	28.3	255 Btuh
3	2, Clear, Metal, DEF	N	45.0	28.3	1274 Btuh
4	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
5	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
6	2, Clear, Metal, DEF	E	15.0	28.3	424 Btuh
7	2, Clear, Metal, DEF	E	8.0	28.3	226 Btuh
8	2, Clear, Metal, DEF	S	75.0	28.3	2122 Btuh
9	2, Clear, Metal, DEF	S	20.0	28.3	566 Btuh
10	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
11	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
	Window Total		284		8037 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	11.0	1872	3.5	6552 Btuh
	Wall Total		1872		6552 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		80	18.3	1466 Btuh
	Door Total		80		1466Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1934	1.3	2514 Btuh
	Ceiling Total		1934		2514Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	1	208.0 ft(p)	28.5	5928 Btuh
	Floor Total		208		5928 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	19340(sqft)	129	5542 Btuh
	Mechanical			80	3432 Btuh
	Infiltration Total			209	8974 Btuh

Totals for Heating	**Subtotal**	**33472 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1674 Btuh**
	Total Btuh Loss	**35146 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Project Title:
Mr. Joe Osburn

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 7/5/2007

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, B, N	N	1.33	5	6.0	0.0	6.0	15	15	90	Btuh
2	2, Clear, DEF, B, N	N	1.33	5	9.0	0.0	9.0	15	15	135	Btuh
3	2, Clear, DEF, B, N	N	5.66	7	45.0	0.0	45.0	15	15	675	Btuh
4	2, Clear, DEF, B, N	N	9.33	7	30.0	0.0	30.0	15	15	450	Btuh
5	2, Clear, DEF, B, N	N	1.33	7	30.0	0.0	30.0	15	15	450	Btuh
6	2, Clear, DEF, B, N	E	1.33	7	15.0	0.0	15.0	15	46	690	Btuh
7	2, Clear, DEF, B, N	E	1.33	6	8.0	0.0	8.0	15	46	368	Btuh
8	2, Clear, DEF, B, N	S	1.33	7	75.0	75.0	0.0	15	24	1125	Btuh
9	2, Clear, DEF, B, N	S	1.33	7	20.0	20.0	0.0	15	24	300	Btuh
10	2, Clear, DEF, B, N	S	7.33	7	30.0	30.0	0.0	15	24	450	Btuh
11	2, Clear, DEF, B, N	W	1.33	6	16.0	0.0	16.0	15	46	736	Btuh
	Window Total				284					5469	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Exterior	11.0	1872.0	1.9	3632	Btuh
	Wall Total		1872.0		3632	Btuh

Doors	Type	Area	HTM	Load	
1	Insulated - Exter	80.0	10.1	811	Btuh
	Door Total	80.0		811	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1934.0	1.4	2746	Btuh
	Ceiling Total		1934.0		2746	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	1.0	208.0 ft(p)	0.0	0	Btuh
	Floor Total		208.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.35	19340	113.0	2238	Btuh
	Mechanical			80	1584	Btuh
	Infiltration Total			193	3822	Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		6	X	300	+	1200	3000	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title:
Mr. Joe Osburn

Code Only
Professional Version
Climate: North

7/5/2007

Totals for Cooling	Subtotal	19480	Btuh
	Duct gain(using duct multiplier of 0.10)	1948	Btuh
	Total sensible gain	21428	Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	6695	Btuh
	Latent occupant gain (6 people @ 230 Btuh per person)	1380	Btuh
	Latent other gain	0	Btuh
	TOTAL GAIN	29503	Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

** LAMAR BOOZER **		PROJECT:	CUSTOM
900 EAST PUTNAM STREET		CLIENT:	NORTON
LAKE CITY, FL 32055		DATE:	8 01 07

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS — DESIGNER: LAMAR BOOZER

#0708-03

CLIENT INFORMATION:

NAME: NORTON
ADDRESS:
CITY, STATE: LAKE CITY, FLORIDA

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN, = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	85	2,773	0	2,964	2,964
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,219	4,389	0	2,400	2,400
11-C DOOR METAL POLYSTYRENE CORE	40	846	0	462	462
16-G CEILING R-30 INSULATION	1,934	2,627	0	2,627	2,627
22-A SLAB ON GRADE NO EDGE INSUL	111	4,046	0	0	0
SUBTOTALS FOR STRUCTURE:	3,389	14,681	0	8,453	8,453
PEOPLE	10	0	0	3,000	3,000
APPLIANCES	0	0	800	1,500	2,300
DUCTWORK	0	734	0	1,841	1,841
INFILTRATION W.CFM: 0.0 S.CFM: 235.9	0	0	7,859	5,449	13,308
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				20,243	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		15,415	8,659	20,243	28,902

SUPPLY CFM AT 20 DEG DT:	920	CFM PER SQUARE FOOT:	0.520
SQUARE FT. OF ROOM AREA:	1,934	SQUARE FOOT PER TON:	734.482

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 15.415 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 3.409 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

Live Oak
PEST CONTROL, INC.

17856 U.S. 129 South
McAlpin, FL 32062 - 2561

Robert F. McGranahan
President

Jeffrey D. Lee
Entomologist

FAX COVER SHEET

DATE: 9/18/07

To: John Kerce

From:

Comments:

Notice of Prevention for Subterranean Termites
(As required by Florida Building Code (FBC) 104.2.6)

Live Oak PEST CONTROL, INC.
17856 U.S. 129 • McALPIN, FLORIDA 32062
(386) 362-3887 • 1-800-771-3887 • Fax: (386) 364-3529

JOE OSBURN PERMIT # [redacted] 00002610 9

418 MAXMORE DR. LAKE CITY, FL 32055
Address of Treatment or Lot/Block of Treatment

9/18/07 Date	9:00 Time	HOUSTON & AARON Applicator
PREVAIL Product Used	CYPERMETHRIN Chemical used *(active ingredient)*	507.5 Number of gallons applied
.125% Percent Concentration	2915 Area treated *(square feet)*	360 Linear feet treated

HORIZONTAL & VERTICAL
Stage of treatment *(Horizontal, Vertical, Adjoining Slab, retreat of disturbed area)*

As per 104.2.6 - If soil chemical barrier method for Subterranean termite prevention is used, final exterior treatment shall be completed prior to final building approval.
If this notice is for the final exterior treatment, initial and date this line. _____

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0708-03

Application Charese Norton, Contractor James Norton Owner Joe Osburn
Property: ID# 12-3s-16-02096-001

The submitted plans were designed by Architectural Drafting & Design, Inc.

Mr. Nicholas Paul Geisler, Architect signed and provides his Florida architect license number AR0007005 but did not affix his embossed seal on the plans. Mr. Mark Disosway P.E. 53951 has reviewed these plans, and assigned 706271as a job number with drawing sheet S1, S2 & S3 which were submitted with the plans.

When constructing this structure use only Mr. Disosway plans, sheets S1, S2 & S3 for structural requirement to comply with the Florida Building Codes.

Thank You:

Joe Haltiwanger
Plan Examiner
Columbia County Building
Department

Builders FirstSource

Project Information for: L247360

Builder: Norton Builders
Address: 418 Northwest Maxmore Drive
.... Lake City, FL 32055
County: Columbia
Truss Count: 18
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
James H. Norton Florida Contractor License No. RB0031780
Address: Route 28 Box 388A Lake City, Florida 32025

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1866292	T01	7/20/07
2	J1866293	T01G	7/20/07
3	J1866294	T02	7/20/07
4	J1866295	T03	7/20/07
5	J1866296	T03G	7/20/07
6	J1866297	T04	7/20/07
7	J1866298	T04G	7/20/07
8	J1866299	T05	7/20/07
9	J1866300	T05G	7/20/07
10	J1866301	T06	7/20/07
11	J1866302	T06G	7/20/07
12	J1866303	T07	7/20/07
13	J1866304	T08	7/20/07
14	J1866305	T08G	7/20/07
15	J1866306	T09	7/20/07
16	J1866307	T09G	7/20/07
17	J1866308	T10	7/20/07
18	J1866309	T11	7/20/07

Project Information for: L247360

Builder: Norton Builders
Address: 418 Northwest Maxmore Drive
.... Lake City, FL 32055
County: Columbia
Truss Count: 18
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

July 20,2007

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
James H. Norton Florida Contractor License No. RB0031780
Address: Route 28 Box 388A Lake City, Florida 32025

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1866292	T01	7/20/07
2	J1866293	T01G	7/20/07
3	J1866294	T02	7/20/07
4	J1866295	T03	7/20/07
5	J1866296	T03G	7/20/07
6	J1866297	T04	7/20/07
7	J1866298	T04G	7/20/07
8	J1866299	T05	7/20/07
9	J1866300	T05G	7/20/07
10	J1866301	T06	7/20/07
11	J1866302	T06G	7/20/07
12	J1866303	T07	7/20/07
13	J1866304	T08	7/20/07
14	J1866305	T08G	7/20/07
15	J1866306	T09	7/20/07
16	J1866307	T09G	7/20/07
17	J1866308	T10	7/20/07
18	J1866309	T11	7/20/07

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866292
L247360	T01	COMMON	11	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Jul 20 09:52:47 2007 Page 1

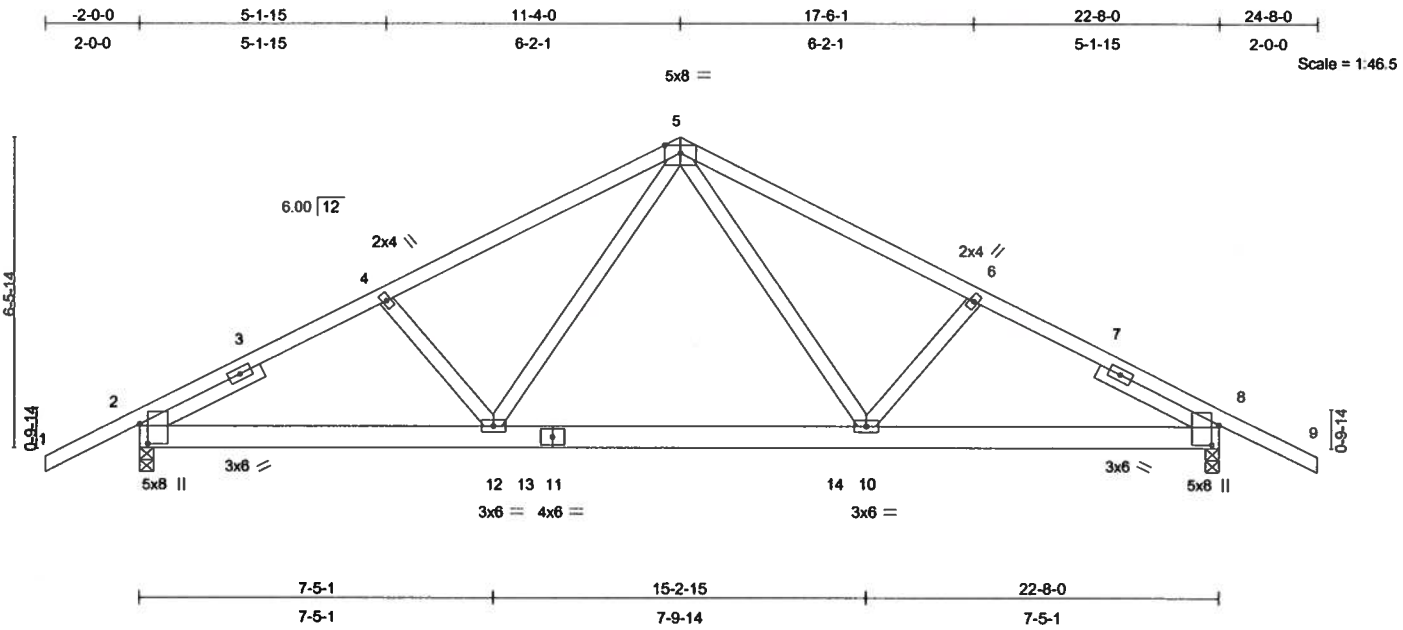


Plate Offsets (X,Y): [2:0-4-15,0-1-15], [8:0-4-15,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	0.29	10-12	>934	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.20	10-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.83	Horz(TL)	-0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 2-9-15, Right 2 X 4 SYP No.3 2-9-15

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-0 oc bracing.

REACTIONS (lb/size) 2=1028/0-3-8, 8=1028/0-3-8
Max Horz 2=93(load case 6)
Max Uplift 2=-660(load case 6), 8=-660(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-3=-1572/2075, 3-4=-1513/2086, 4-5=-1416/2082, 5-6=-1416/2082, 6-7=-1513/2088, 7-8=-1571/2075, 8-9=0/30
BOT CHORD 2-12=-1676/1311, 12-13=-1099/950, 11-13=-1099/950, 11-14=-1099/950, 10-14=-1099/950, 8-10=-1676/1311
WEBS 4-12=-197/140, 5-12=-908/513, 5-10=-908/513, 6-10=-197/140

JOINT STRESS INDEX
2 = 0.96, 2 = 0.75, 3 = 0.00, 4 = 0.34, 5 = 0.58, 6 = 0.34, 7 = 0.00, 8 = 0.96, 8 = 0.75, 10 = 0.42, 11 = 0.34 and 12 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces , and for MWFRS for reactions specified.

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866292
L247360	T01	COMMON	11	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 660 lb uplift at joint 2 and 660 lb uplift at joint 8.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-9=-54, 2-13=-10, 13-14=-70(F=-60), 8-14=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866293
L247360	T01G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Jul 20 09:55:30 2007 Page 1

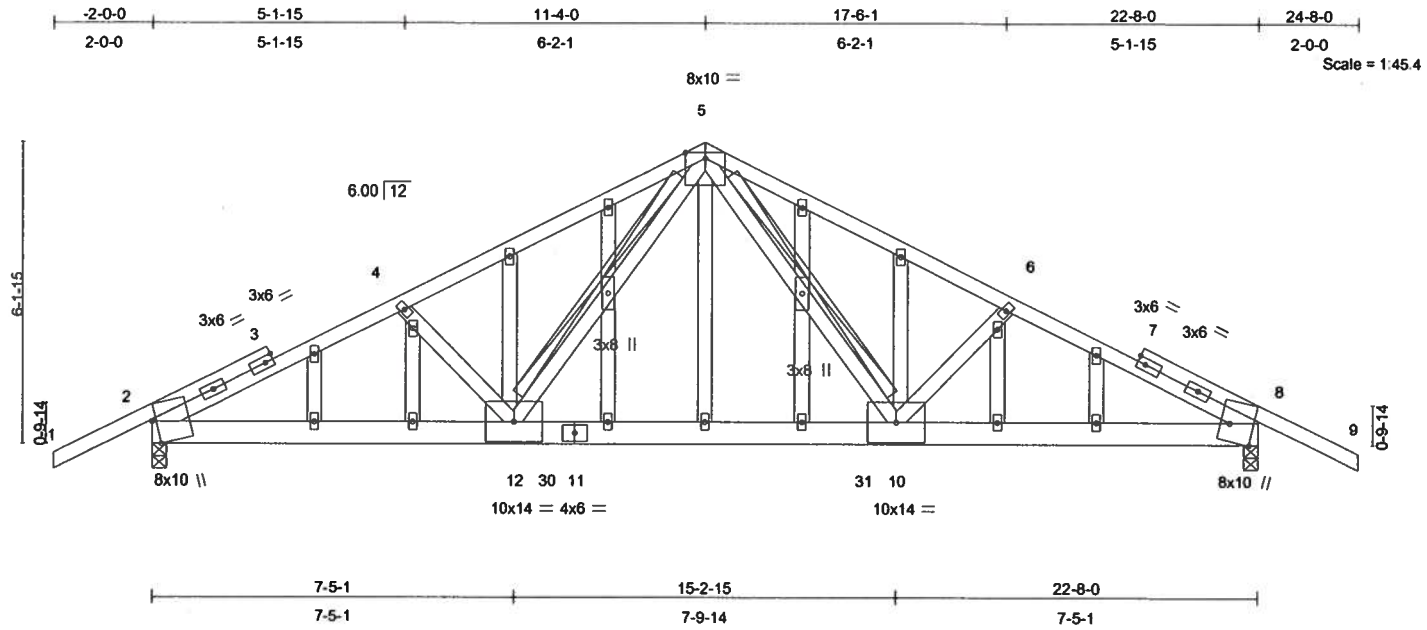


Plate Offsets (X,Y): [2:0-5-14,Edge], [8:0-4-5,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	0.30	10-12	>920	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.20	10-12	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.35	Horz(TL)	-0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D *Except* 1-3 2 X 4 SYP No.2, 7-9 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-12, 5-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1731/0-3-8, 8=1731/0-3-8
Max Horz 2=103(load case 6)
Max Uplift 2=-1331(load case 6), 8=-1331(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-41/63, 2-3=-2720/3654, 3-4=-2481/3496, 4-5=-2312/3304, 5-6=-2312/3304, 6-7=-2481/3496, 7-8=-2720/3654, 8-9=-41/63
BOT CHORD 2-12=-3055/2296, 12-30=-1936/1545, 11-30=-1936/1545, 11-31=-1936/1545, 10-31=-1936/1545, 8-10=-3055/2296
WEBS 4-12=-561/657, 5-12=-1211/718, 5-10=-1211/718, 6-10=-561/657

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Key Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX

2 = 0.63, 3 = 0.00, 3 = 0.42, 3 = 0.54, 4 = 0.37, 5 = 0.99, 6 = 0.37, 7 = 0.00, 7 = 0.54, 7 = 0.42, 8 = 0.63, 10 = 0.35, 11 = 0.44, 12 = 0.35, 13 = 0.34, 14 = 0.59, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.59, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34 and 29 = 0.34

July 20,2007

Continued on page 2

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866293
L247360	T01G	GABLE	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 5.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1331 lb uplift at joint 2 and 1331 lb uplift at joint 8.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-30=-10, 30-31=-40(F=-30), 8-31=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866294
L247360	T02	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Jul 20 09:56:04 2007 Page 1

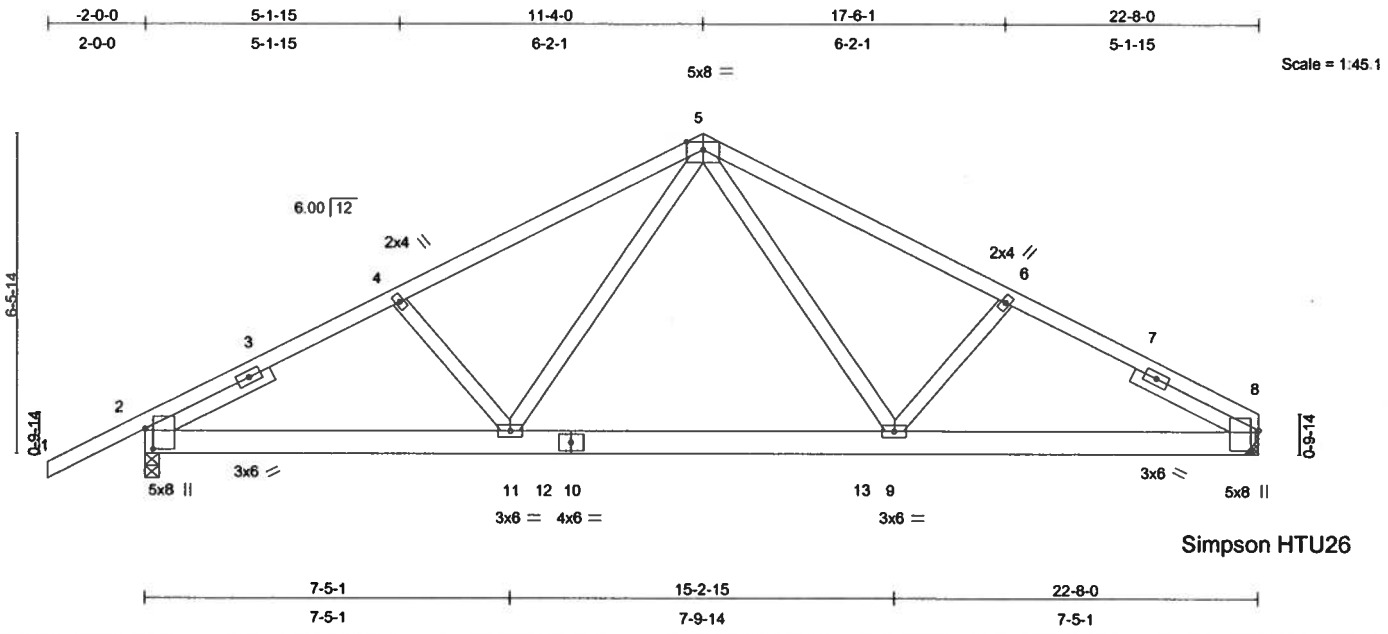


Plate Offsets (X,Y): [2:0-4-15,0-1-15], [8:0-4-15,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.29	9-11	>941	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.20	9-11	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.86	Horz(TL)	-0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 134 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 2-9-15, Right 2 X 4 SYP No.3 2-9-15

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-1 oc bracing.

REACTIONS (lb/size) 8=916/Mechanical, 2=1033/0-3-8
Max Horz 2=101(load case 6)
Max Uplift 8=-572(load case 7), 2=-660(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-3=-1591/2088, 3-4=-1521/2098, 4-5=-1425/2095, 5-6=-1441/2119, 6-7=-1540/2128, 7-8=-1612/2113
BOT CHORD 2-11=-1733/1319, 11-12=-1159/959, 10-12=-1159/959, 10-13=-1159/959, 9-13=-1159/959, 8-9=-1764/1339
WEBS 4-11=-197/140, 5-11=-903/512, 5-9=-942/536, 6-9=-207/155

JOINT STRESS INDEX
2 = 0.97, 2 = 0.76, 3 = 0.00, 4 = 0.34, 5 = 0.58, 6 = 0.34, 7 = 0.00, 8 = 0.97, 8 = 0.76, 9 = 0.42, 10 = 0.35 and 11 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces , and for MWFRS for reactions specified.

Julius Lee
Truss Design Engineer
Florida PE No. 31869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866294
L247360	T02	COMMON	2	1	Job Reference (optional)

NOTES

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 572 lb uplift at joint 8 and 660 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-8=-54, 2-12=-10, 12-13=-70(F=-60), 8-13=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866295
L247360	T03	COMMON	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:25 2007 Page 1

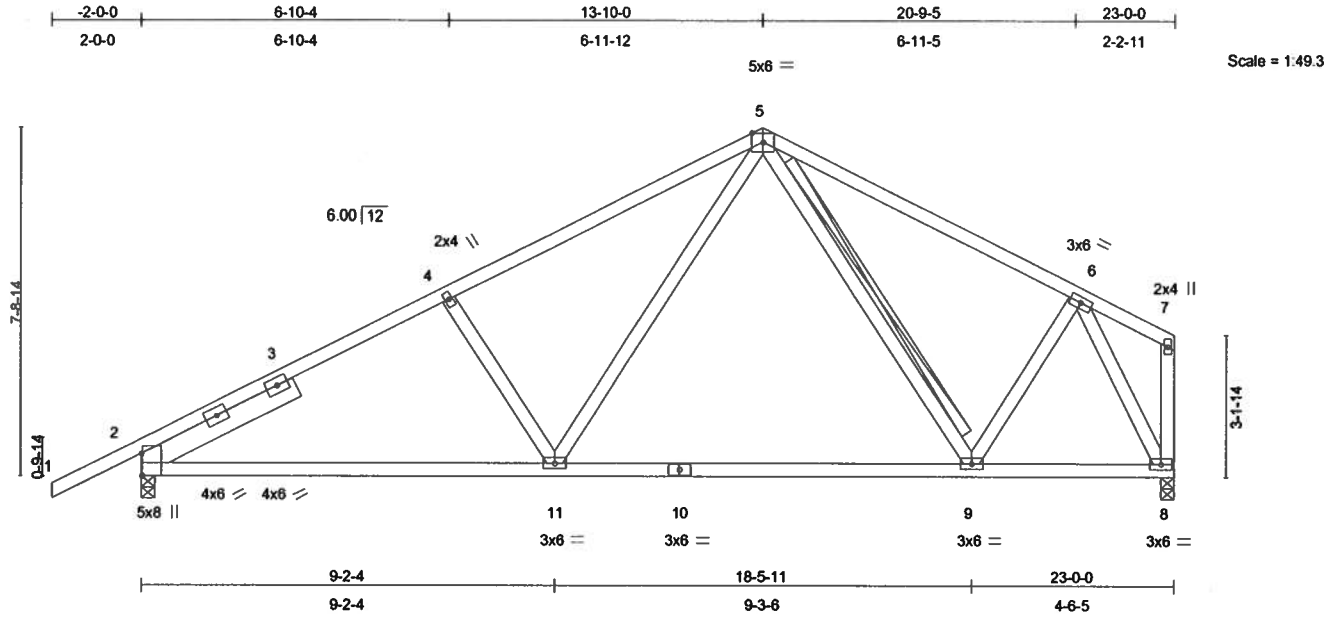


Plate Offsets (X,Y): [2:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.12	9-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.22	9-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 133 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-10-3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-5-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=844/0-3-8, 8=727/0-3-8
Max Horz 2=172(load case 6)
Max Uplift 2=-251(load case 6), 8=-144(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1139/588, 3-4=-975/613, 4-5=-933/613, 5-6=-629/433, 6-7=-21/81, 7-8=-38/58
BOT CHORD 2-11=-559/909, 10-11=-278/560, 9-10=-278/560, 8-9=-232/379
WEBS 4-11=-303/300, 5-11=-233/414, 5-9=-149/72, 6-9=-16/251, 6-8=-839/520

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.56, 2 = 0.21, 2 = 0.21, 3 = 0.00, 4 = 0.33, 5 = 0.70, 6 = 0.35, 7 = 0.33, 8 = 0.45, 9 = 0.42, 10 = 0.26 and 11 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866295
L247360	T03	COMMON	4	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 144 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866296
L247360	T03G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:27 2007 Page 1

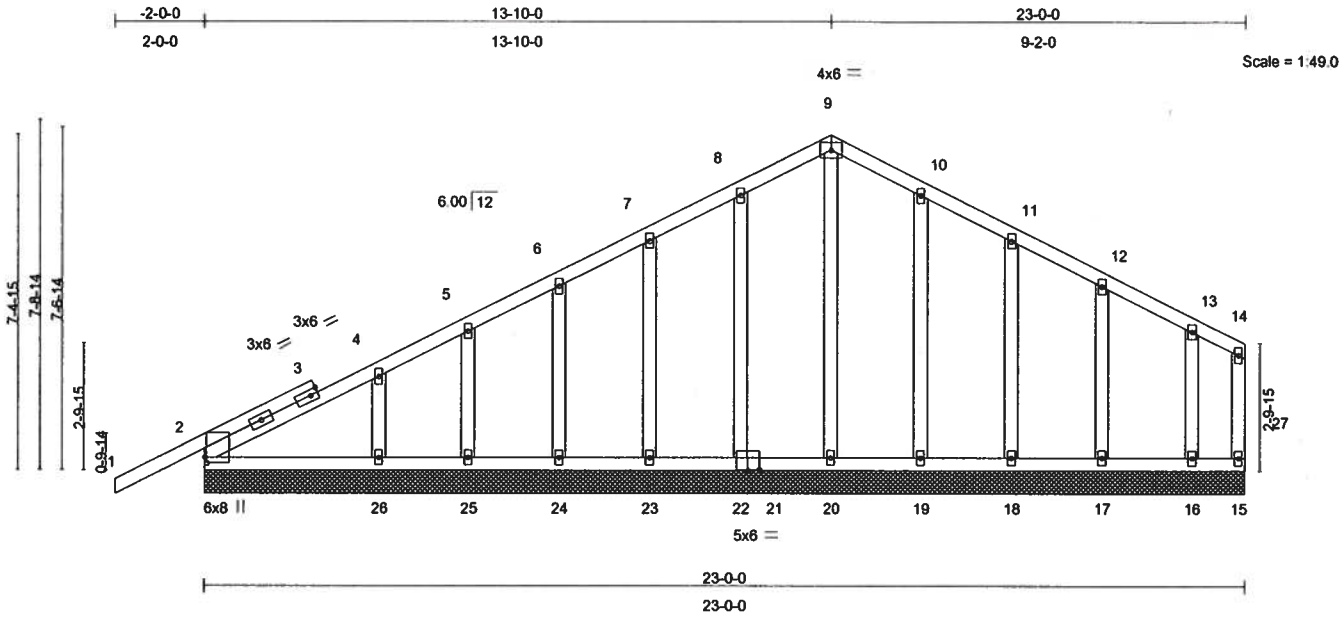


Plate Offsets (X,Y): [2:0-1-8,0-0-8], [21:0-3-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.04	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.20	Horz(TL)	0.00	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 150 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=509/23-0-0, 15=56/23-0-0, 20=240/23-0-0, 22=249/23-0-0, 23=247/23-0-0, 24=254/23-0-0, 25=221/23-0-0, 26=344/23-0-0, 19=249/23-0-0, 18=246/23-0-0, 17=258/23-0-0, 16=189/23-0-0

Max Horz 2=214(load case 6)

Max Uplift 2=-196(load case 6), 15=-8(load case 6), 22=-113(load case 6), 23=-122(load case 6), 24=-121(load case 6), 25=-112(load case 6), 26=-158(load case 6), 19=-110(load case 7), 18=-123(load case 7), 17=-121(load case 7), 16=-122(load case 7)

Max Grav 2=509(load case 1), 15=56(load case 1), 20=240(load case 1), 22=253(load case 10), 23=247(load case 1), 24=254(load case 10), 25=221(load case 1), 26=344(load case 10), 19=253(load case 11), 18=246(load case 1), 17=258(load case 1), 16=192(load case 11)

Julius Lee
Truss Design Engineer
Florida PE No. 31889
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Continued on page 2

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-12/54, 2-3=-134/53, 3-4=-144/75, 4-5=-70/71, 5-6=-56/102, 6-7=-58/139, 7-8=-58/180, 8-9=-59/231, 9-10=-59/231, 10-11=-58/180, 11-12=-59/121, 12-13=-55/62, 13-14=-26/28, 15-27=-45/18, 14-27=-45/18

BOT CHORD 2-26=-1/7, 25-26=-1/7, 24-25=-1/7, 23-24=-1/7, 22-23=-1/7, 21-22=-1/7, 20-21=-1/7, 19-20=-1/7, 18-19=-1/7, 17-18=-1/7, 16-17=-1/7, 15-16=-1/7

WEBS 9-20=-221/0, 8-22=-233/166, 7-23=-227/183, 6-24=-233/181, 5-25=-205/165, 4-26=-314/235, 10-19=-233/166, 11-18=-226/182, 12-17=-237/184, 13-16=-183/156

JOINT STRESS INDEX

2 = 0.87, 3 = 0.00, 3 = 0.36, 3 = 0.36, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.33, 8 = 0.33, 9 = 0.24, 10 = 0.33, 11 = 0.33, 12 = 0.33 , 13 = 0.33, 14 = 0.33, 15 = 0.33, 15 = 0.00, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.29, 22 = 0.00, 23 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.00 and 27 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2, 8 lb uplift at joint 15, 113 lb uplift at joint 22, 122 lb uplift at joint 23, 121 lb uplift at joint 24, 112 lb uplift at joint 25, 158 lb uplift at joint 26, 110 lb uplift at joint 19, 123 lb uplift at joint 18, 121 lb uplift at joint 17 and 122 lb uplift at joint 16.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-9=-114(F=-60), 9-14=-114(F=-60), 2-15=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866297
L247360	T04	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:27 2007 Page 1

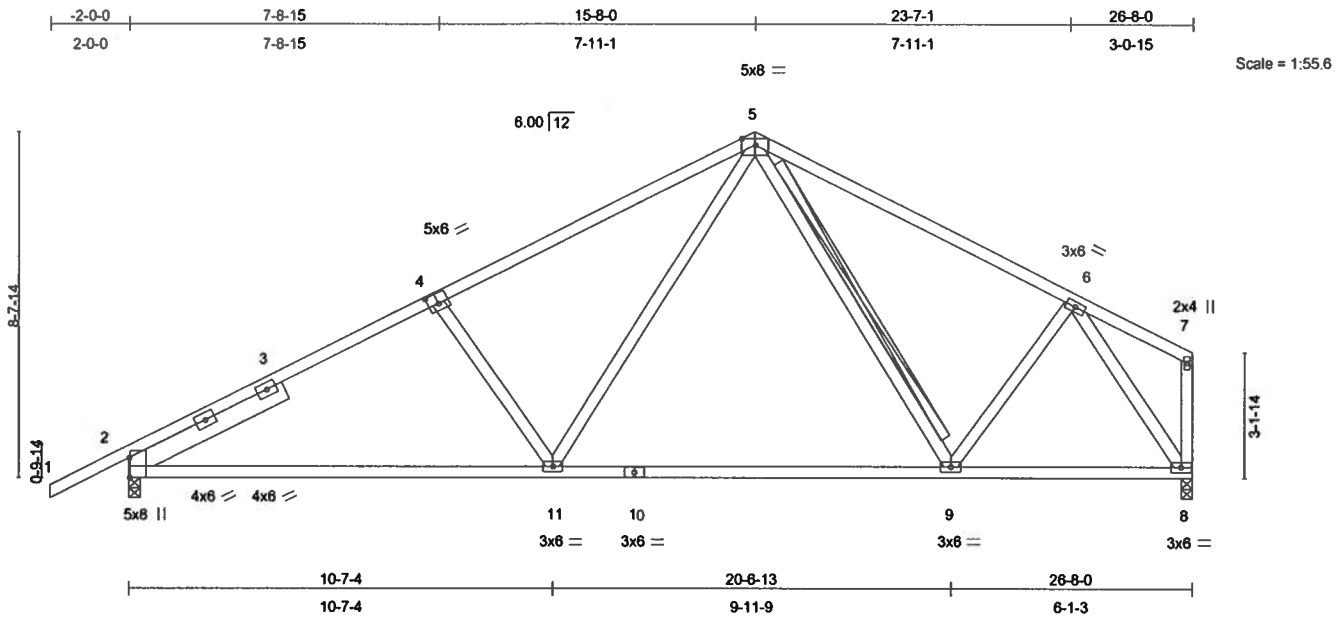


Plate Offsets (X,Y): [2:0-5-15,Edge], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.16	2-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.29	2-11	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 4-4-3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=961/0-3-8, 8=845/0-3-8
Max Horz 2=182(load case 6)
Max Uplift 2=-277(load case 6), 8=-169(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1325/710, 3-4=-1164/739, 4-5=-1104/724, 5-6=-810/550, 6-7=-22/84 , 7-8=-29/23
BOT CHORD 2-11=-659/1087, 10-11=-328/680, 9-10=-328/680, 8-9=-324/539
WEBS 4-11=-356/348, 5-11=-265/469, 5-9=-119/78, 6-9=-5/231, 6-8=-982/607

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Key Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX
2 = 0.74, 2 = 0.24, 2 = 0.24, 3 = 0.00, 4 = 0.70, 5 = 0.66, 6 = 0.36, 7 = 0.33, 8 = 0.42, 9 = 0.43, 10 = 0.22 and 11 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866297
L247360	T04	COMMON	6	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 2 and 169 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866298
L247360	T04G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:29 2007 Page 1

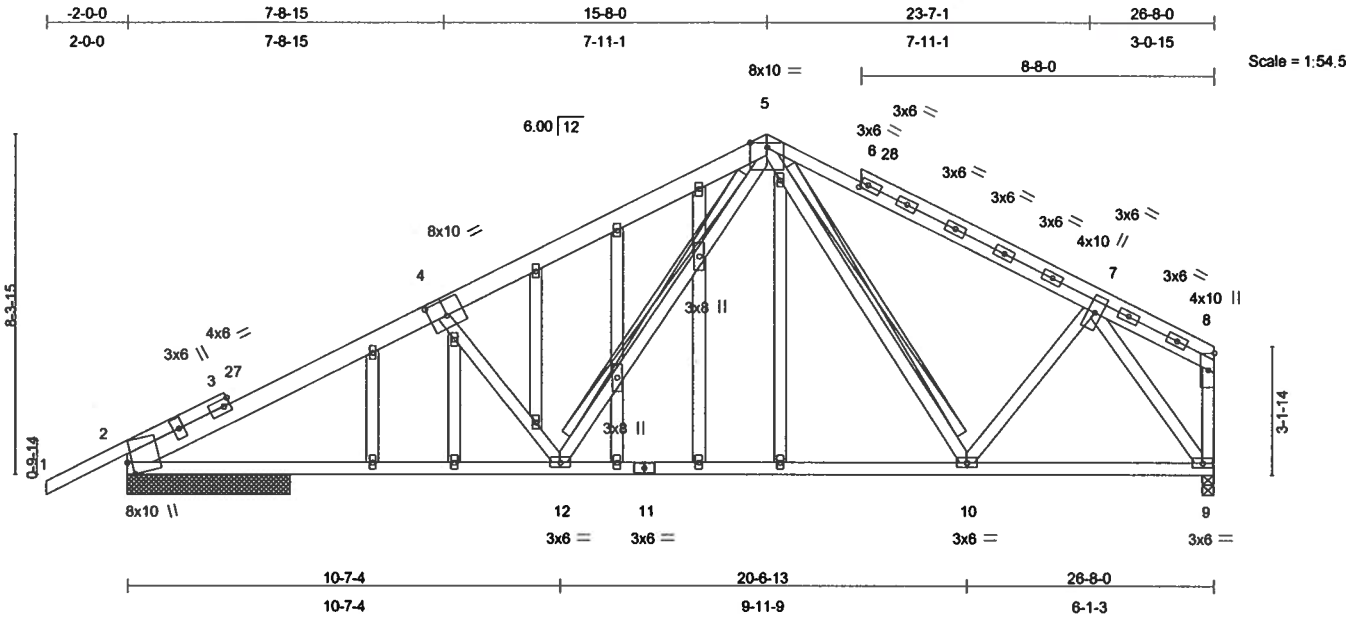


Plate Offsets (X,Y): [2:0-3-15,Edge], [4:0-5-0,0-4-8], [6:0-2-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.19	2-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.37	2-12	>866	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.66	Horz(TL)	0.08	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 216 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2 *Except* 4-5 2 X 6 SYP No.1D, 2-4 2 X 6 SYP No.1D
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-7-6 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 5-0-4 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-12, 5-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2029/3-11-8, 9=1338/0-3-8
Max Horz 2=232(load case 6)
Max Uplift 2=-780(load case 6), 9=-480(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-12/54, 2-3=-3147/1737, 3-27=-2984/1698, 4-27=-2942/1683, 4-5=-2511/1484, 5-28=-1191/852, 6-28=-1260/869, 6-7=-1398/863, 7-8=-43/44, 8-9=-52/8
BOT CHORD	2-12=-1544/2631, 11-12=-725/1379, 10-11=-725/1379, 9-10=-518/874
WEBS	4-12=-1096/779, 5-12=-672/1180, 5-10=-417/209, 7-10=-158/554, 7-9=-1540/927

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Ray Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX

2 = 0.76, 3 = 0.00, 3 = 0.72, 3 = 0.59, 4 = 0.70, 5 = 0.77, 6 = 0.00, 6 = 0.32, 6 = 0.32, 6 = 0.32, 6 = 0.32, 6 = 0.32, 7 = 0.59, 7 = 0.21, 7 = 0.21, 8 = 0.35, 9 = 0.55, 10 = 0.42, 11 = 0.44, 12 = 0.87, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.64, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.64, 25 = 0.33 and 26 = 0.33

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866298
L247360	T04G	GABLE	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 780 lb uplift at joint 2 and 480 lb uplift at joint 9.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-27=-114(F=-60), 5-27=-141(F=-87), 5-28=-141(F=-87), 8-28=-54, 2-9=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES.
L247360	T05	COMMON	6	1	J1866299
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:30 2007 Page 1

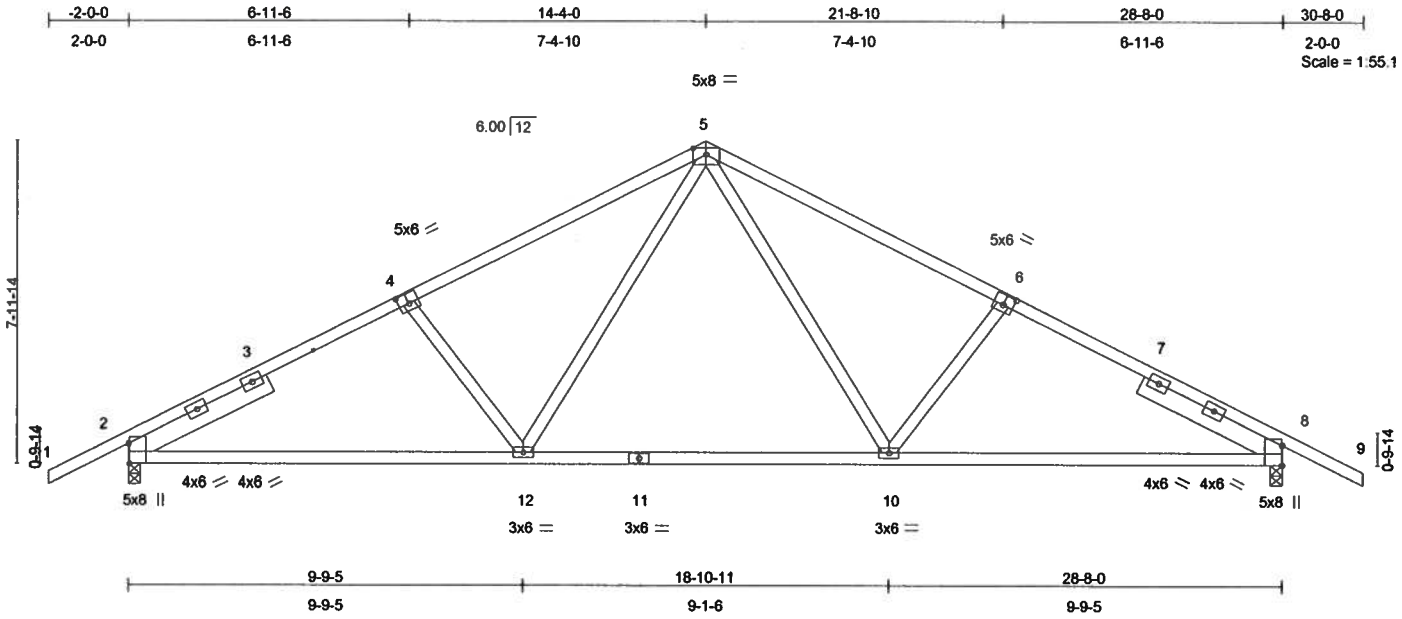


Plate Offsets (X,Y): [2:0-5-15,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.13	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.24	8-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.30	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-10-13, Right 2 X 6 SYP No.1D 3-10-13

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-3 oc bracing.

REACTIONS (lb/size) 2=1025/0-3-8, 8=1025/0-3-8
Max Horz 2=109(load case 6)
Max Uplift 2=-286(load case 6), 8=-286(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1471/806, 3-4=-1393/832, 4-5=-1263/810, 5-6=-1263/810, 6-7=-1393/832, 7-8=-1470/806, 8-9=0/26
BOT CHORD 2-12=-560/1215, 11-12=-274/876, 10-11=-274/876, 8-10=-560/1215
WEBS 4-12=-307/299, 5-12=-214/394, 5-10=-214/394, 6-10=-307/299

JOINT STRESS INDEX
2 = 0.71, 2 = 0.27, 2 = 0.27, 3 = 0.00, 4 = 0.56, 5 = 0.60, 6 = 0.56, 7 = 0.00, 8 = 0.71, 8 = 0.27, 8 = 0.27, 10 = 0.42, 11 = 0.34 and 12 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.

Julius Lee
Truss Design Engineer
Florida PE No. 31555
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866299
L247360	T05	COMMON	6	1	Job Reference (optional)

NOTES

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2 and 286 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866300
L247360	T05G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:32 2007 Page 1

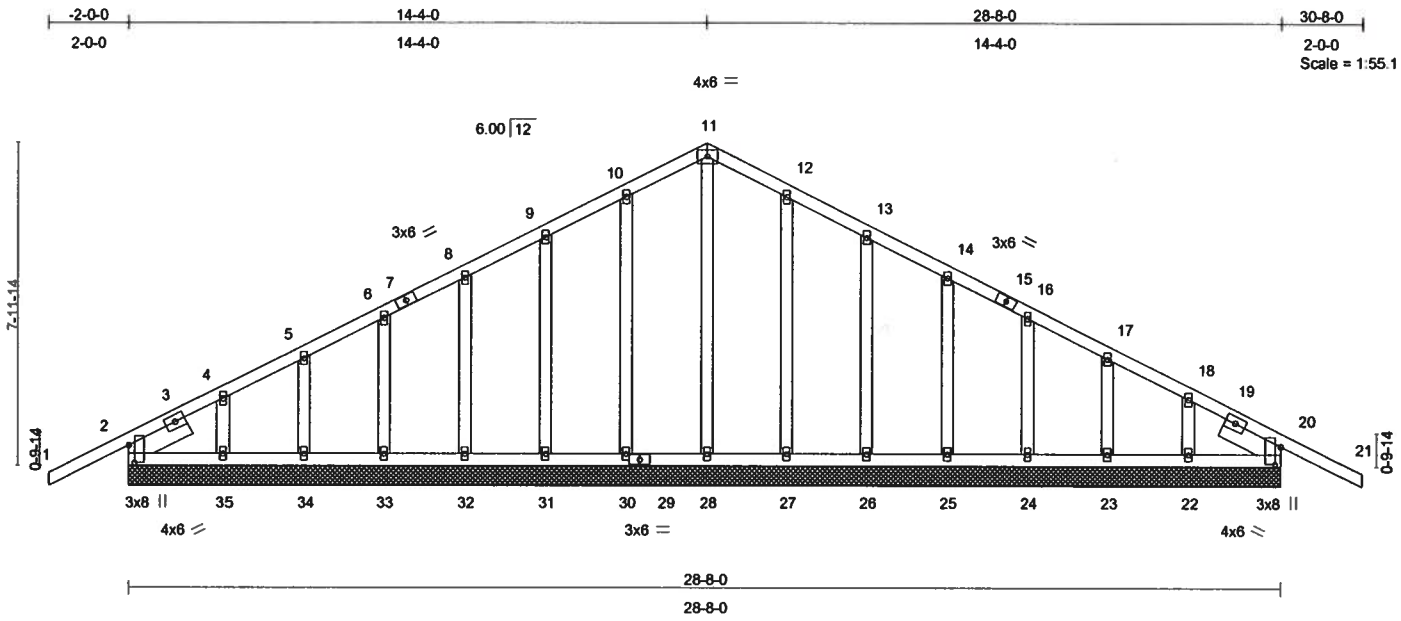


Plate Offsets (X,Y): [2:0-5-3,0-1-11], [20:0-5-3,0-1-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.04	21	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.06	21	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.01	20	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3
SLIDER	Left 2 X 6 SYP No.1D 1-7-12, Right 2 X 6 SYP No.1D 1-7-12

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=475/28-8-0, 20=475/28-8-0, 28=232/28-8-0, 30=248/28-8-0, 31=248/28-8-0, 32=249/28-8-0, 33=243/28-8-0, 34=267/28-8-0, 35=159/28-8-0, 27=248/28-8-0, 26=248/28-8-0, 25=249/28-8-0, 24=243/28-8-0, 23=267/28-8-0, 22=159/28-8-0

Max Horz 2=127(load case 6)

Max Uplift 2=-166(load case 6), 20=-201(load case 7), 30=-110(load case 6), 31=-124(load case 6), 32=-119(load case 6), 33=-118(load case 6), 34=-125(load case 6), 35=-100(load case 6), 27=-107(load case 7), 26=-125(load case 7), 25=-119(load case 7), 24=-118(load case 7), 23=-128(load case 7), 22=-87(load case 7)

Max Grav 2=475(load case 1), 20=475(load case 1), 28=232(load case 1), 30=252(load case 10), 31=248(load case 1), 32=249(load case 10), 33=243(load case 10), 34=267(load case 1), 35=160(load case 10), 27=252(load case 11), 26=248(load case 1), 25=249(load case 11), 24=243(load case 11), 23=267(load case 1), 22=160(load case 11)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866300
L247360	T05G	GABLE	1	1	Job Reference (optional)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-12/54, 2-3=-159/62, 3-4=-131/57, 4-5=-85/60, 5-6=-70/98, 6-7=-68/133, 7-8=-18/129, 8-9=-69/169, 9-10=-69/228, 10-11=-69/277, 11-12=-69/277, 12-13=-69/228, 13-14=-69/168, 14-15=-18/100, 15-16=-68/112, 16-17=-70/56, 17-18=-63/42, 18-19=-75/11, 19-20=-159/15, 20-21=-12/54

BOT CHORD 2-35=0/163, 34-35=0/163, 33-34=0/163, 32-33=0/163, 31-32=0/163, 30-31=0/163, 29-30=0/163, 28-29=0/163, 27-28=0/163, 26-27=0/163, 25-26=0/163, 24-25=0/163, 23-24=0/163, 22-23=0/163, 20-22=0/163

WEBS 11-28=-212/0, 10-30=-232/163, 9-31=-228/185, 8-32=-229/178, 6-33=-225/177, 5-34=-243/189, 4-35=-154/128, 12-27=-232/163, 13-26=-228/185, 14-25=-229/178, 16-24=-225/177, 17-23=-243/189, 18-22=-154/128

JOINT STRESS INDEX

2 = 0.77, 2 = 0.12, 3 = 0.00, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.15, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.24, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.15, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.00, 20 = 0.77, 20 = 0.12, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.33, 28 = 0.33, 29 = 0.15, 30 = 0.33, 31 = 0.33, 32 = 0.33, 33 = 0.33, 34 = 0.33 and 35 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 166 lb uplift at joint 2, 201 lb uplift at joint 20, 110 lb uplift at joint 30, 124 lb uplift at joint 31, 119 lb uplift at joint 32, 118 lb uplift at joint 33, 125 lb uplift at joint 34, 100 lb uplift at joint 35, 107 lb uplift at joint 27, 125 lb uplift at joint 26, 119 lb uplift at joint 25, 118 lb uplift at joint 24, 128 lb uplift at joint 23 and 87 lb uplift at joint 22.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

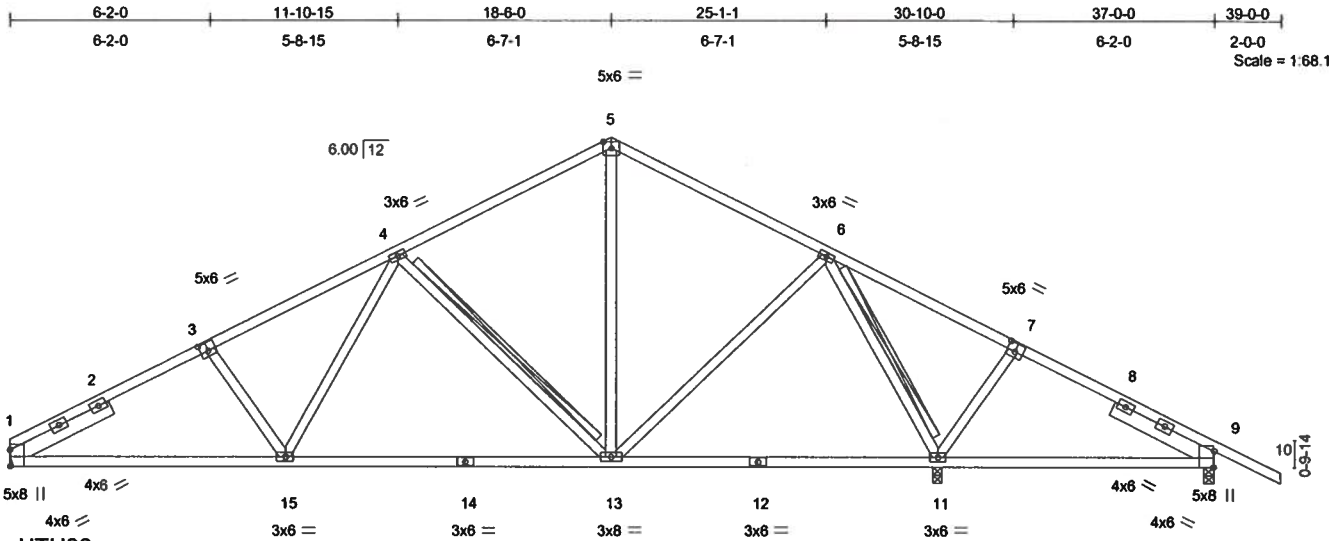
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-11=-114(F=-60), 11-21=-114(F=-60), 2-20=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES.
L247360	T06	COMMON	6	1	J1866301
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:33 2007 Page 1



Simpson HTU26

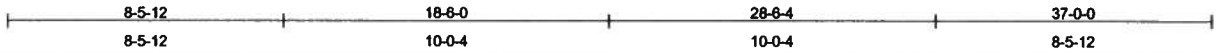


Plate Offsets (X,Y): [1:0-5-15,Edge], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [9:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.22	9-11	>464	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.29	13-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-5-9, Right 2 X 6 SYP No.1D 3-5-9

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-13, 6-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=874/Mechanical, 11=1339/0-3-8, 9=263/0-3-8
Max Horz 1=-140(load case 7)
Max Uplift 1=-212(load case 6), 11=-308(load case 7), 9=-295(load case 7)
Max Grav 1=874(load case 1), 11=1339(load case 1), 9=339(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1428/836, 2-3=-1358/855, 3-4=-1255/842, 4-5=-709/583, 5-6=-708/581, 6-7=0/322, 7-8=-82/202, 8-9=-171/184, 9-10=0/26
BOT CHORD 1-15=-594/1182, 14-15=-373/930, 13-14=-373/930, 12-13=-53/298, 11-12=-53/298, 9-11=-154/74
WEBS 3-15=-210/247, 4-15=-174/344, 4-13=-533/439, 5-13=-220/296, 6-13=-21/402, 6-11=-1116/526, 7-11=-307/338

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866301
L247360	T06	COMMON	6	1	Job Reference (optional)

JOINT STRESS INDEX

1 = 0.55, 1 = 0.26, 1 = 0.26, 2 = 0.00, 3 = 0.61, 4 = 0.39, 5 = 0.58, 6 = 0.39, 7 = 0.61, 8 = 0.00, 9 = 0.55, 9 = 0.26, 9 = 0.26, 11 = 0.44, 12 = 0.48, 13 = 0.56, 14 = 0.48 and 15 = 0.44

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 212 lb uplift at joint 1, 308 lb uplift at joint 11 and 295 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES.
L247360	T06G	GABLE	1	1	J1866302
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Jul 20 09:58:24 2007 Page 1

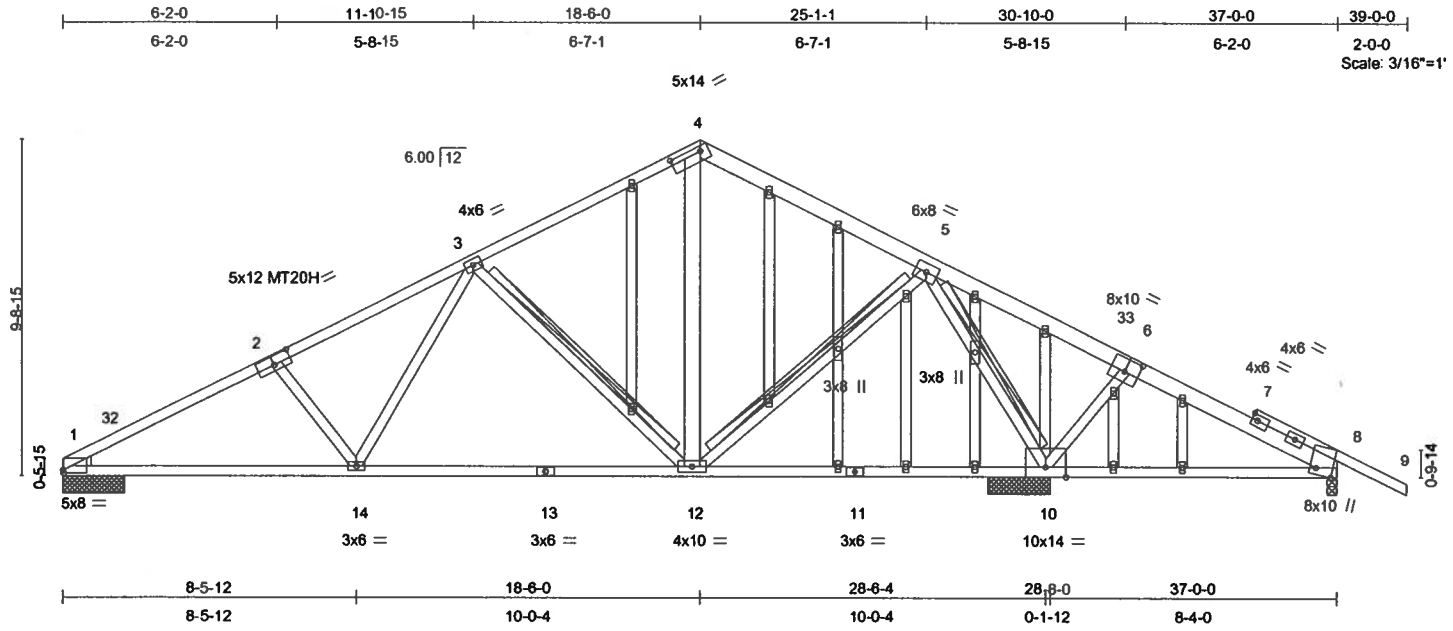


Plate Offsets (X,Y): [1:0-0-4,0-1-0], [2:0-6-0,0-3-0], [4:0-11-0,0-1-12], [6:0-5-0,0-4-8], [8:0-2-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	0.28	1-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.31	12-14	>999	240	MT20H	187/143
BCLL	10.0	* Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.08	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 277 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 4-6 2 X 6 SYP No.1D, 6-8 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except* 4-12 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-5-7 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-12, 5-12, 5-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1945/1-9-8, 8=303/0-3-8, 10=3293/1-9-8
Max Horz 1=-163(load case 7)
Max Uplift 1=-1110(load case 6), 8=-377(load case 7), 10=-1905(load case 7)
Max Grav 1=1945(load case 1), 8=396(load case 11), 10=3293(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-32=-3447/3238, 2-32=-3290/3136, 2-3=-2973/2871, 3-4=-1615/1641, 4-5=-1618/1640, 5-33=-995/1242, 6-33=-768/914, 6-7=-590/750, 7-8=-486/634, 8-9=-31/54
BOT CHORD 1-14=-2619/2868, 13-14=-1865/2156, 12-13=-1865/2156, 11-12=-342/531, 10-11=-342/531, 8-10=-562/535
WEBS 2-14=-619/729, 3-14=-643/702, 3-12=-1241/1317, 4-12=-325/368, 5-12=-758/991, 5-10=-3003/2830, 6-10=-726/855

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1326 Crestal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX

1 = 0.79, 2 = 0.95, 3 = 0.52, 4 = 0.94, 5 = 0.71, 6 = 0.53, 7 = 0.00, 7 = 0.37, 7 = 0.37, 8 = 0.49, 10 = 0.53, 11 = 0.52, 12 = 0.75, 13 = 0.78, 14 = 0.56, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.35, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.77, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34 and 31 = 0.34

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866302
L247360	T06G	GABLE	1	1	Job Reference (optional)

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are MT20 plates unless otherwise indicated.
6) All plates are 2x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1110 lb uplift at joint 1, 377 lb uplift at joint 8 and 1905 lb uplift at joint 10.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-32=-114(F=-60), 4-32=-141(F=-87), 4-33=-141(F=-87), 9-33=-114(F=-60), 1-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Ray Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866303
L247360	T07	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:35 2007 Page 1

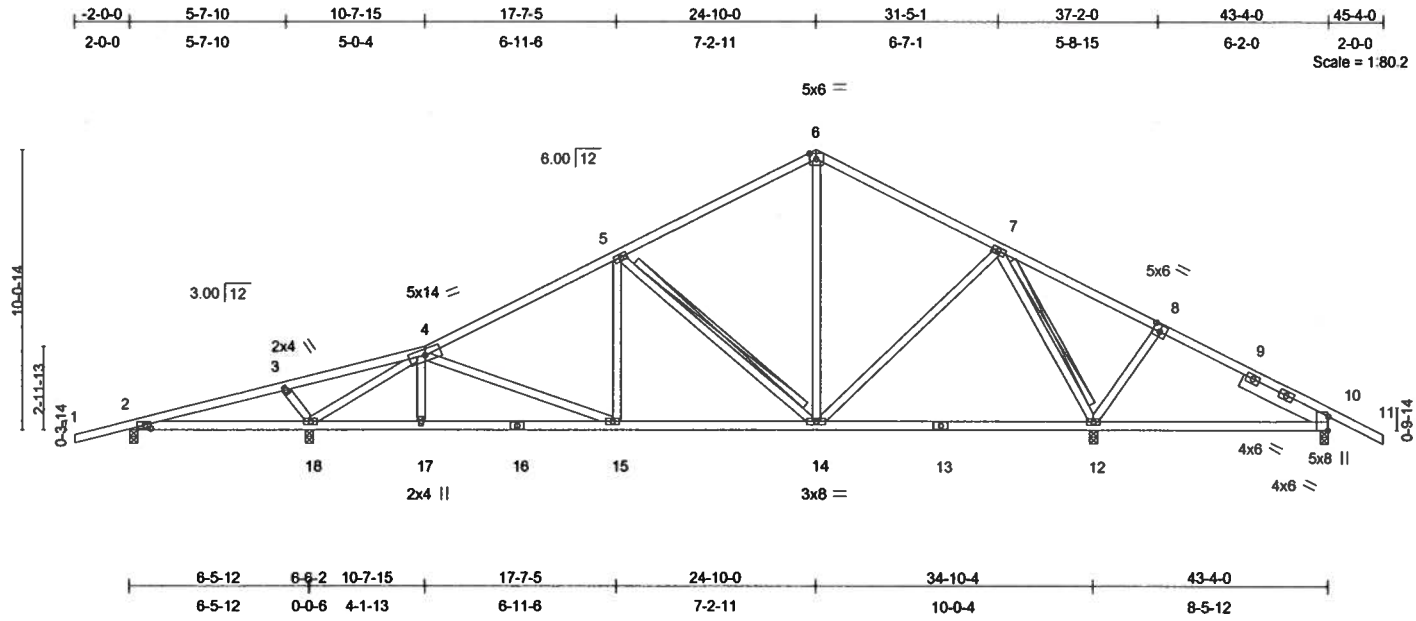


Plate Offsets (X,Y): [2:0-2-12,0-1-8], [8:0-3-0,0-3-0], [10:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.21	10-12	>478	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.29	12-14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.03	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 239 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 3-5-9

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-14, 7-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=218/0-3-8, 18=1203/0-3-8, 12=1296/0-3-8, 10=270/0-3-8
Max Horz 2=-133(load case 7)
Max Uplift 2=-232(load case 4), 18=-399(load case 6), 12=-303(load case 7), 10=-296(load case 7)
Max Grav 2=228(load case 10), 18=1203(load case 1), 12=1296(load case 1), 10=346(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-124/293, 3-4=-198/463, 4-5=-1083/690, 5-6=-686/555, 6-7=-673/558, 7-8=0/305, 8-9=-97/194, 9-10=-186/176, 10-11=0/26
BOT CHORD 2-18=-247/188, 17-18=-434/956, 16-17=-438/954, 15-16=-438/954, 14-15=-346/899, 13-14=-41/289, 12-13=-41/289, 10-12=-146/87
WEBS 3-18=-299/247, 4-18=-1634/907, 4-17=0/155, 4-15=-89/98, 5-15=-8/221, 5-14=-504/404, 6-14=-187/265, 7-14=-14/380, 7-12=-1064/503, 8-12=-306/337

Julius Lee
Truss Design Engineer
Florida PE No. 31666
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Continued on page 2

JOINT STRESS INDEX

2 = 0.36, 3 = 0.33, 4 = 0.31, 5 = 0.39, 6 = 0.62, 7 = 0.39, 8 = 0.61, 9 = 0.00, 10 = 0.55, 10 = 0.11, 10 = 0.11, 12 = 0.44, 13 = 0.53, 14 = 0.56, 15 = 0.34, 16 = 0.40, 17 = 0.33 and 18 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 2, 399 lb uplift at joint 18, 303 lb uplift at joint 12 and 296 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866304
L247360	T08	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:37 2007 Page 1

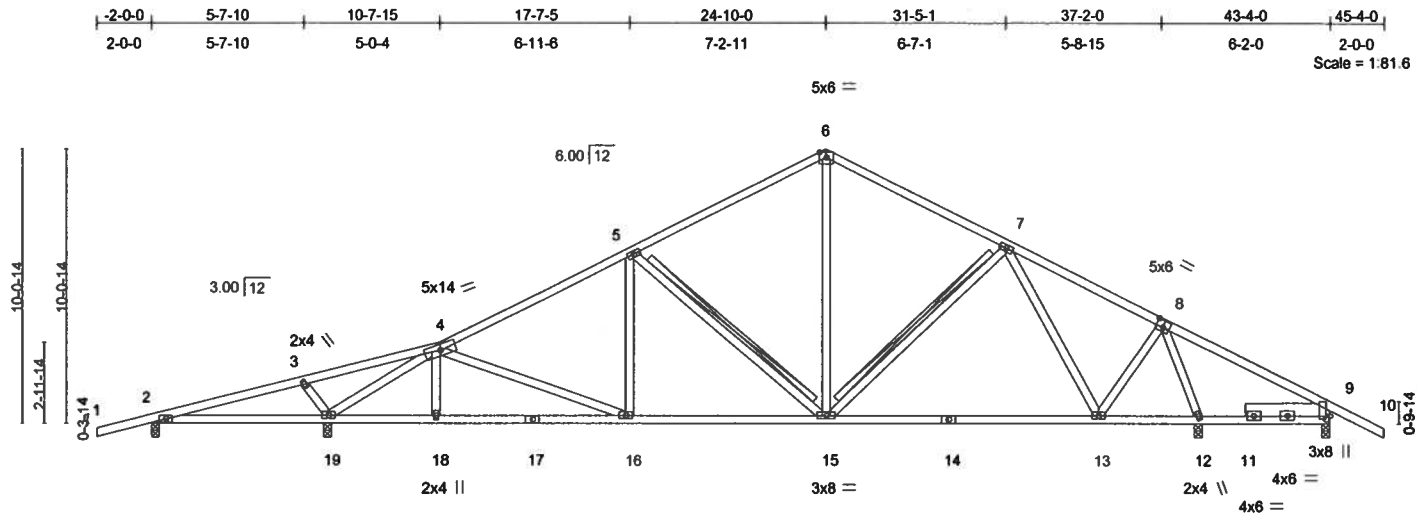


Plate Offsets (X,Y): [8:0-3-0,0-3-0], [9:0-1-8,0-1-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.19	13-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.36	13-15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 244 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Right 2 X 6 SYP No.1D 3-1-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-15, 7-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=193/0-3-8, 19=1345/0-3-8, 9=10/0-3-8, 12=1440/0-3-8
Max Horz 2=-133(load case 7)
Max Uplift 2=-232(load case 4), 19=-414(load case 6), 9=-156(load case 7), 12=-341(load case 7)
Max Grav 2=211(load case 10), 19=1345(load case 1), 9=113(load case 11), 12=1440(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-172/385, 3-4=-246/556, 4-5=-1289/774, 5-6=-914/655, 6-7=-901/658, 7-8=-569/404, 8-9=-305/612, 9-10=0/26
BOT CHORD 2-19=-335/234, 18-19=-481/1076, 17-18=-485/1074, 16-17=-485/1074, 15-16=-421/1083, 14-15=-210/711, 13-14=-210/711, 12-13=-49/107, 11-12=-457/382, 9-11=-457/382
WEBS 3-19=-302/249, 4-19=-1885/1020, 4-18=0/155, 4-16=-42/68, 5-16=0/201, 5-15=-487/385, 6-15=-277/411, 7-15=-105/173, 7-13=-566/327, 8-13=-265/701,

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33426

July 20,2007

JOINT STRESS INDEX

2 = 0.39, 3 = 0.33, 4 = 0.35, 5 = 0.39, 6 = 0.63, 7 = 0.39, 8 = 0.61, 9 = 0.84, 9 = 0.11, 9 = 0.11, 11 = 0.00, 12 = 0.55, 13 = 0.51 , 14 = 0.58, 15 = 0.56, 16 = 0.34, 17 = 0.44, 18 = 0.33 and 19 = 0.53

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 2, 414 lb uplift at joint 19, 156 lb uplift at joint 9 and 341 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1326 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866305
L247360	T08G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:38 2007 Page 1

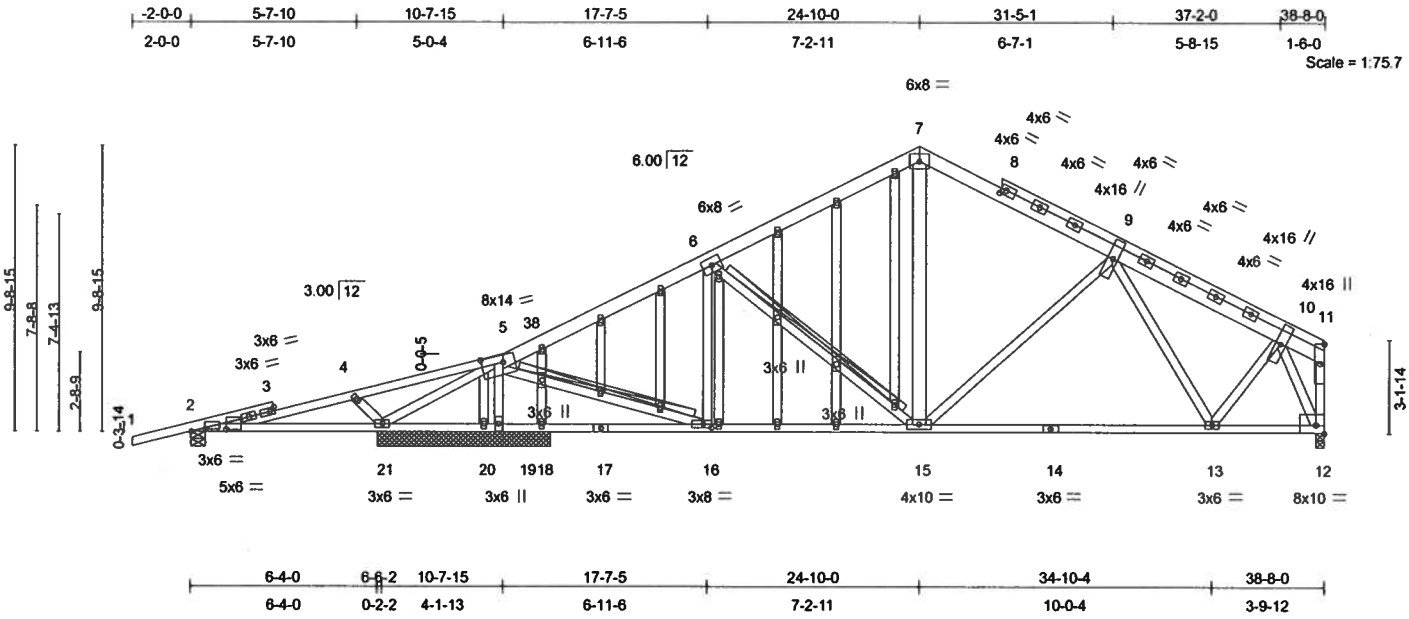


Plate Offsets (X,Y): [2:0-5-12,0-1-0], [2:1-2-4,0-0-12], [5:0-9-0,0-3-0], [8:0-2-0,0-2-0], [12:Edge,0-3-8], [16:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.18	13-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.34	13-15	>943	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.56	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 328 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except* 2-5 2 X 4 SYP No.1D, 1-3 2 X 4 SYP No.2 8-11 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except* 7-15 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-8-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-0-13 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-16, 6-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=604/0-6-0, 21=704/5-11-8, 19=1976/5-11-8, 12=1323/0-3-8, 20=49/5-11-8, 18=101/5-11-8
Max Horz 2=249(load case 6)
Max Uplift 2=-499(load case 4), 21=-514(load case 4), 19=-1300(load case 6), 12=-668(load case 7)
Max Grav 2=606(load case 10), 21=706(load case 10), 19=1976(load case 1), 12=1323(load case 1), 20=100(load case 2), 18=190(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-30/53, 2-3=-104/57, 3-4=-108/47, 4-5=-457/456, 5-38=-2132/1916, 6-38=-1946/1799, 6-7=-1715/1604, 7-8=-1316/1344, 8-9=-1528/1430, 9-10=-1102/916, 10-11=-40/40, 11-12=-51/32
BOT CHORD 2-21=-110/51, 20-21=-43/64, 19-20=-43/64, 18-19=-74/87, 17-18=-74/87, 16-17=-74/87, 15-16=-1555/1747, 14-15=-994/1252, 13-14=-994/1252, 12-13=-370/463
WEBS

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 31989
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Builders FirstSource

JOINT STRESS INDEX

2 = 0.66, 2 = 0.62, 3 = 0.00, 3 = 0.53, 3 = 0.53, 4 = 0.33, 5 = 0.42, 6 = 0.17, 7 = 0.86, 8 = 0.00, 8 = 0.32, 8 = 0.25, 8 = 0.25, 9 = 0.44, 9 = 0.22, 9 = 0.22, 9 = 0.22, 9 = 0.22, 10 = 0.44, 11 = 0.19, 12 = 0.23, 13 = 0.56, 14 = 0.61, 15 = 0.41, 16 = 0.67, 17 = 0.15, 18 = 0.33, 19 = 0.62, 20 = 0.33, 21 = 0.35, 22 = 0.43, 23 = 0.33, 24 = 0.33, 25 = 0.33, 26 = 0.33, 27 = 0.33, 28 = 0.33, 29 = 0.33, 30 = 0.71, 31 = 0.33, 32 = 0.33, 33 = 0.33, 34 = 0.33, 35 = 0.71, 36 = 0.33 and 37 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 499 lb uplift at joint 2, 514 lb uplift at joint 21, 1300 lb uplift at joint 19 and 668 lb uplift at joint 12.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-114(F=-60), 5-38=-114(F=-60), 7-38=-141(F=-87), 7-8=-141(F=-87), 8-11=-54, 2-12=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866306
L247360	T09	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:39 2007 Page 1

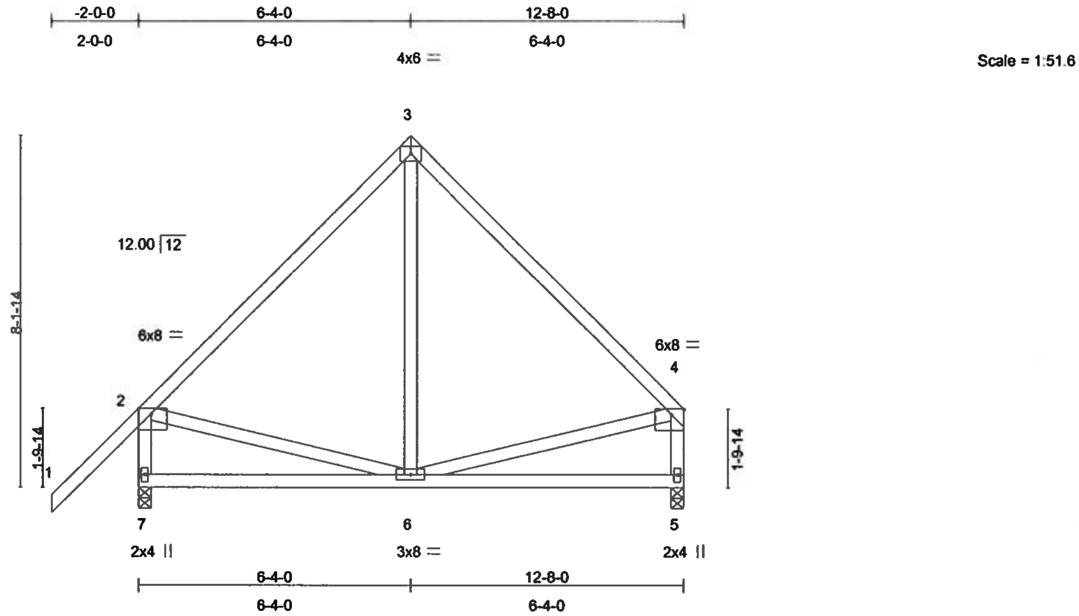


Plate Offsets (X,Y): [2:0-3-8,Edge], [4:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.02	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.04	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 84 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 7=522/0-3-8, 5=386/0-3-8
Max Horz 7=277(load case 5)
Max Uplift 7=-157(load case 6), 5=-79(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/82, 2-3=-365/216, 3-4=-362/211, 2-7=-490/344, 4-5=-352/201
BOT CHORD 6-7=-309/273, 5-6=-101/131
WEBS 3-6=-39/164, 2-6=-122/234, 4-6=-127/236

JOINT STRESS INDEX
2 = 0.62, 3 = 0.67, 4 = 0.62, 5 = 0.73, 6 = 0.16 and 7 = 0.73

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES.
L247360	T09	COMMON	1	1	J1866306
					Job Reference (optional)

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 157 lb uplift at joint 7 and 79 lb uplift at joint 5.

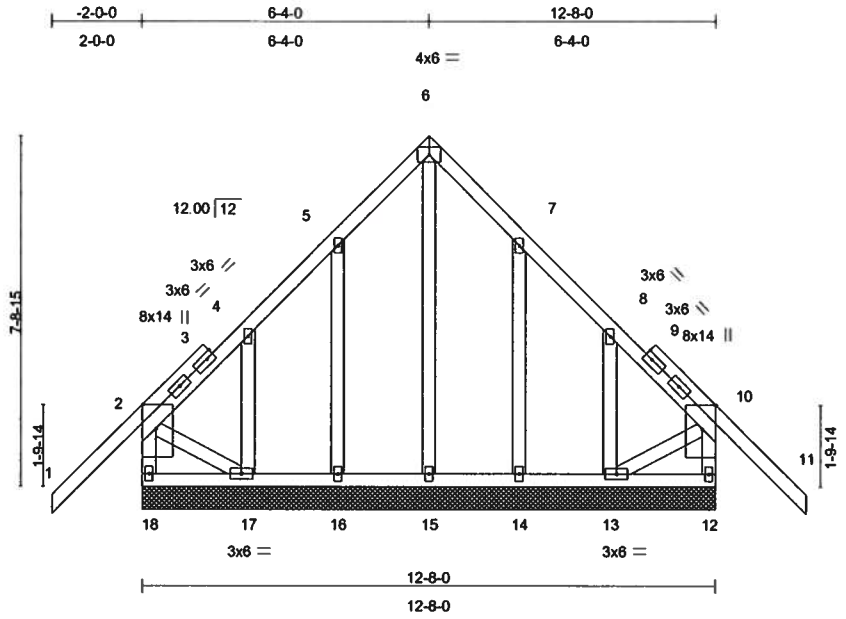
LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866307
L247360	T09G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jul 20 09:14:41 2007 Page 1



Scale = 1:49.0

Plate Offsets (X,Y): [2:0-6-6,Edge], [10:0-6-6,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.06	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.10	10-11	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.19	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 106 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 18=503/12-8-0, 12=503/12-8-0, 15=223/12-8-0, 16=259/12-8-0, 17=138/12-8-0, 14=259/12-8-0, 13=138/12-8-0
Max Horz 18=288(load case 5)
Max Uplift 18=-211(load case 4), 12=-197(load case 5), 15=-2(load case 5), 16=-217(load case 6), 17=-250(load case 6), 14=-216(load case 7), 13=-241(load case 7)
Max Grav 18=503(load case 1), 12=503(load case 1), 15=223(load case 1), 16=263(load case 10), 17=138(load case 10), 14=263(load case 11), 13=138(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-30/159, 2-3=-149/135, 3-4=-152/129, 4-5=-95/108, 5-6=-97/184, 6-7=-97/184, 7-8=-88/76, 8-9=-112/93, 9-10=-149/105, 10-11=-30/159, 2-18=-500/215, 10-12=-500/204
BOT CHORD	17-18=-233/296, 16-17=-55/292, 15-16=-55/292, 14-15=-55/292, 13-14=-55/292, 12-13=-64/116
WEBS	6-15=-202/13, 5-16=-245/235, 4-17=-155/131, 7-14=-245/233, 8-13=-155/131, 2-17=-105/227, 10-13=-87/214

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866307
L247360	T09G	GABLE	1	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.53, 3 = 0.00, 3 = 0.26, 3 = 0.26, 4 = 0.09, 5 = 0.14, 6 = 0.13, 7 = 0.14, 8 = 0.09, 9 = 0.00, 9 = 0.26, 9 = 0.26, 10 = 0.53, 12 = 0.30, 13 = 0.10, 14 = 0.13, 15 = 0.07, 16 = 0.13, 17 = 0.10 and 18 = 0.30

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
8) Gable studs spaced at 2-0-0 oc.
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 18, 197 lb uplift at joint 12, 2 lb uplift at joint 15, 217 lb uplift at joint 16, 250 lb uplift at joint 17, 216 lb uplift at joint 14 and 241 lb uplift at joint 13.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-2=-114(F=-60), 2-6=-114(F=-60), 6-10=-114(F=-60), 10-11=-114(F=-60), 12-18=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866308
L247360	T10	COMMON	1	2	Job Reference (optional)

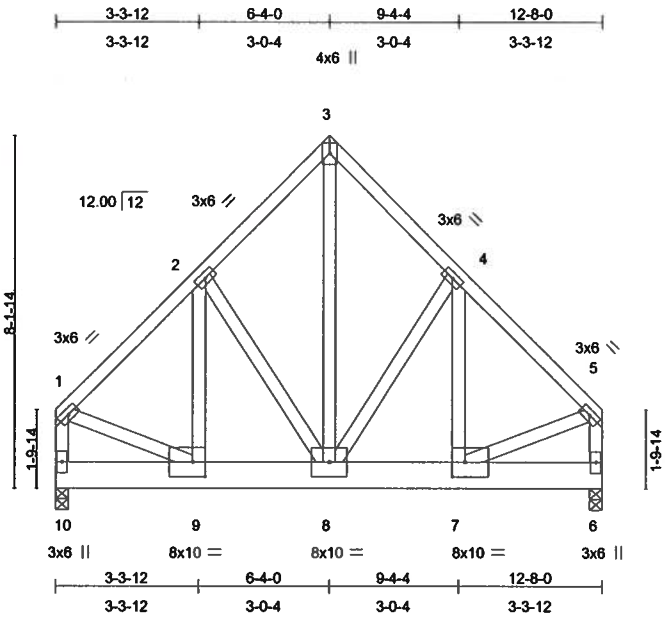


Plate Offsets (X,Y): [7:0-3-8,0-4-0], [9:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.02	8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.05	8	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.48	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 235 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=3861/0-3-8, 6=3861/0-3-8
Max Horz 10=-231(load case 3)
Max Uplift 10=-1103(load case 6), 6=-1103(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2898/853, 2-3=-2317/751, 3-4=-2317/751, 4-5=-2898/853, 1-10=-2928/841, 5-6=-2928/841
BOT CHORD 9-10=-280/215, 8-9=-667/1988, 7-8=-569/1988, 6-7=-63/215
WEBS 2-9=-277/868, 2-8=-680/296, 3-8=-970/3006, 4-8=-680/296, 4-7=-279/868, 1-9=-559/1953, 5-7=-561/1953

JOINT STRESS INDEX
1 = 0.67, 2 = 0.34, 3 = 0.40, 4 = 0.34, 5 = 0.67, 6 = 0.38, 7 = 0.15, 8 = 0.27, 9 = 0.15 and 10 = 0.38

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Julius Lee
Truss Design Engineer
Florida PE No. [illegible]
[illegible] Coastal Bay Blvd
Boynton Beach, FL [illegible]

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES.
L247360	T10	COMMON	1	2	J1866308 Job Reference (optional)

NOTES

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1103 lb uplift at joint 10 and 1103 lb uplift at joint 6.
8) Girder carries tie-in span(s): 37-0-0 from 0-0-0 to 12-8-0

LOAD CASE(S) Standard

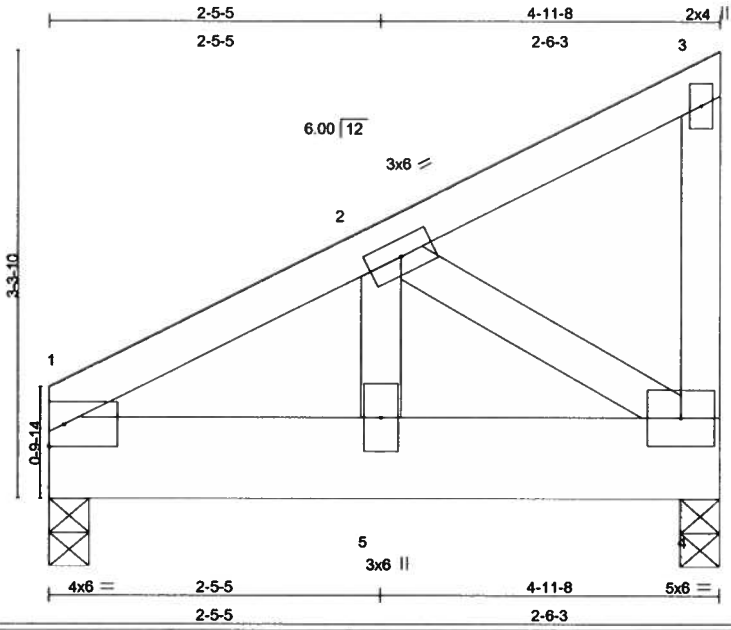
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-54, 3-5=-54, 6-10=-570(F=-560)

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

July 20,2007

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866309
L247360	T11	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Jul 20 10:00:29 2007 Page 1



Scale = 1:16.4

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.02	5	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.34	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=1206/0-3-8, 4=1206/0-3-8
Max Horz 1=89(load case 5)
Max Uplift 1=-307(load case 5), 4=-360(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1093/256, 2-3=-32/20, 3-4=-57/30
BOT CHORD 1-5=-274/881, 4-5=-274/881
WEBS 2-5=-276/1052, 2-4=-1058/329

JOINT STRESS INDEX
1 = 0.54, 2 = 0.78, 3 = 0.02, 4 = 0.26 and 5 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 1 and 360 lb uplift at joint 4.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Truss Design Engineer
Florida PE No. 31989
1400 Coastal Ray Blvd
Boynton Beach, FL 33435

July 20,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	NORTON BLDG. - OSBURN RES. J1866309
L247360	T11	MONO TRUSS	1	1	Job Reference (optional)

LOAD CASE(S) Standard

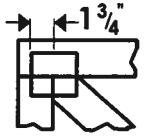
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-463(F=-453), 1-3=-54

Julius Lee
Truss Design Engineer
Florida PE No. 31869
1106 Coastal Bay Blvd
Boynton Beach, FL 33436

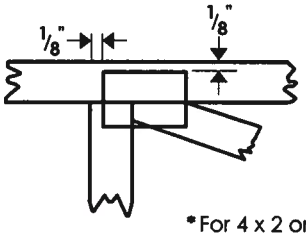
July 20,2007

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

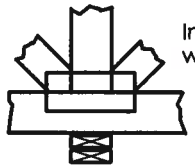
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

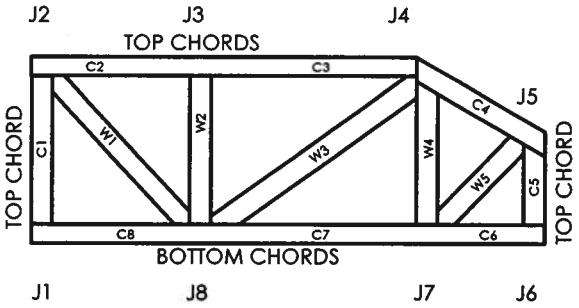
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 6"	8' 6"	11' 6"	11' 6"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 6"	6' 6"	8' 3"	8' 6"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	8' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 8"	8' 8"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 8"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 8"	8' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 8"	8' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 8"	8' 2"	10' 6"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

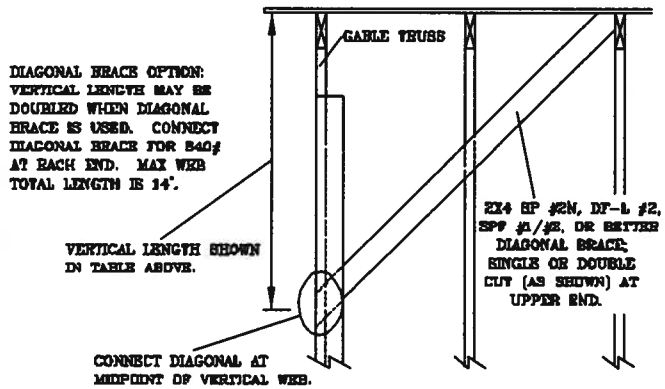
SOUTHERN PINE
#3
STUD
STANDARD

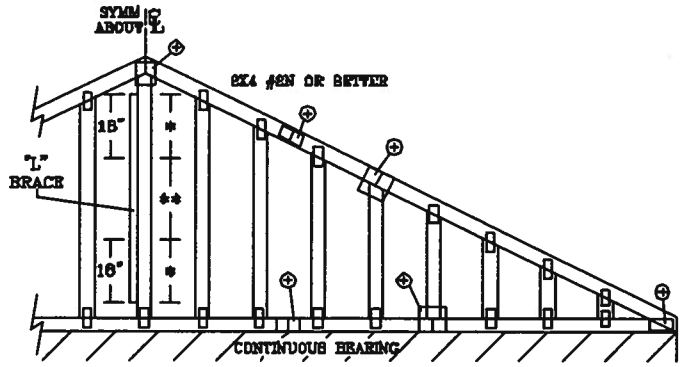
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#2
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PRAX, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 16 E HT
-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 6"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STUD	3' 3"	4' 6"	4' 8"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	5' 6"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	8' 4"	8' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	8' 4"	8' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 8"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 6"	6' 6"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 8"	5' 8"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#2
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

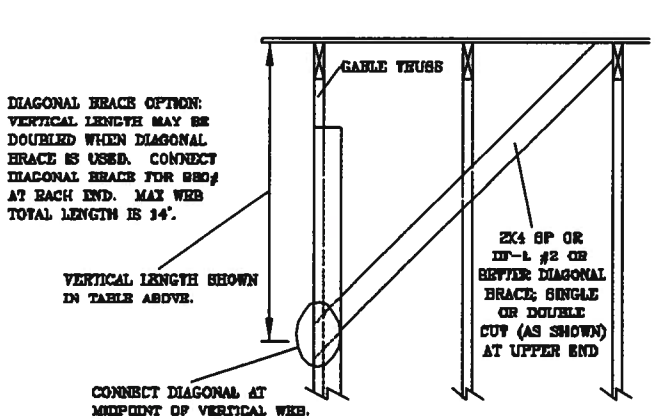
PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

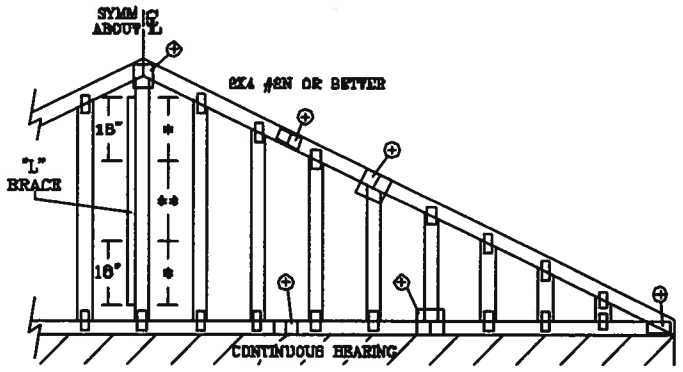
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 880# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING!! TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

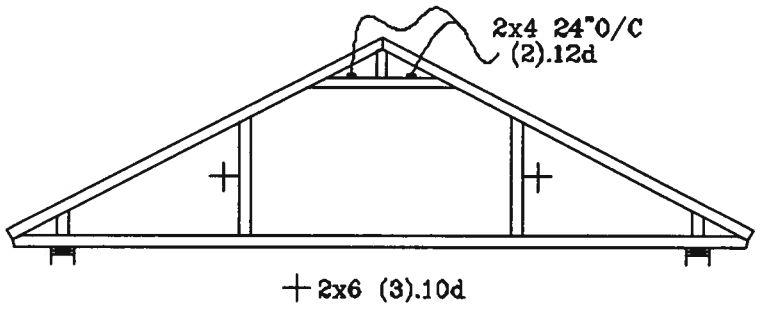
No: 34669
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

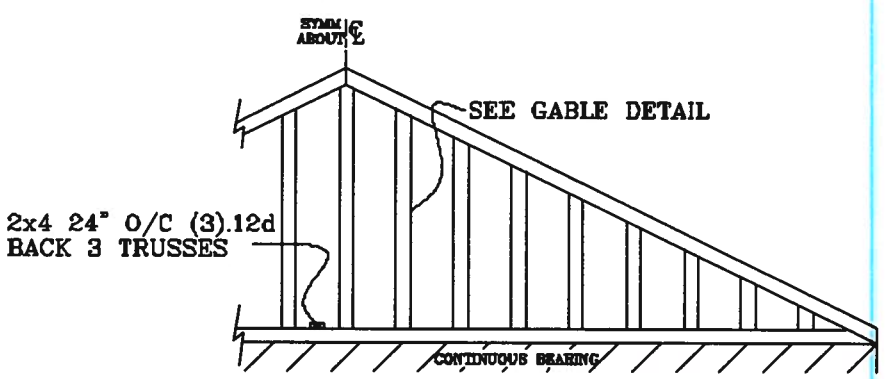
MAX. SPACING 24.0"

REF ASCE7-02-GAB13090
DATE 11/26/03
DWG MTEK STD GABLE 90' E W7
-ENG

TYPICAL ATTIC TRUSS BRACING

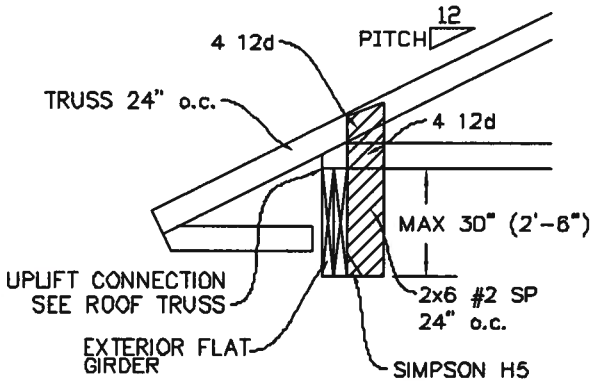


GABLE END TRUSS DETAIL

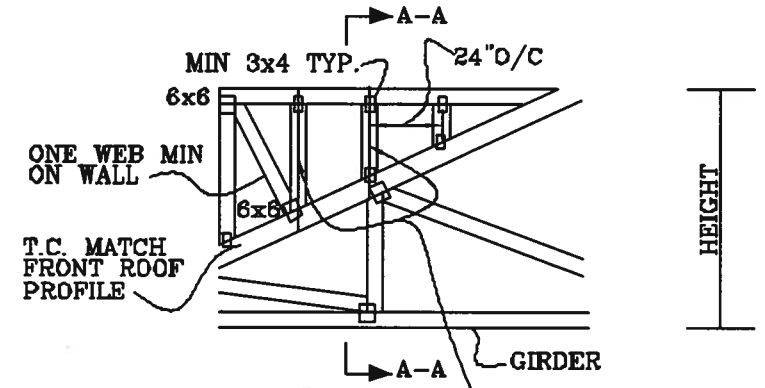


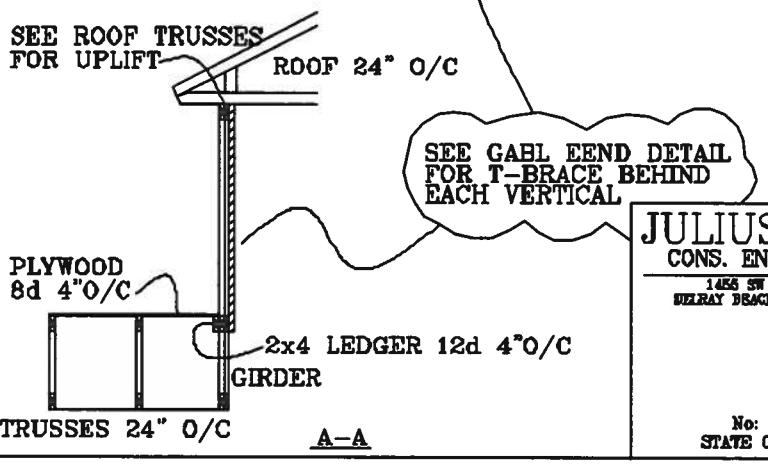
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

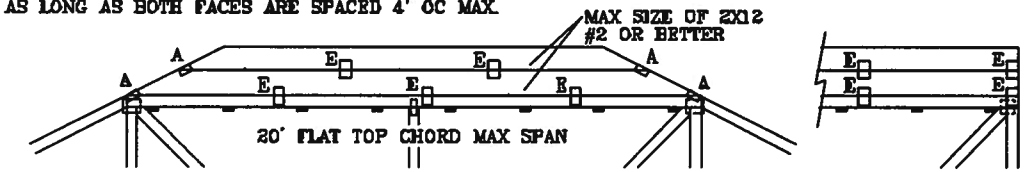
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

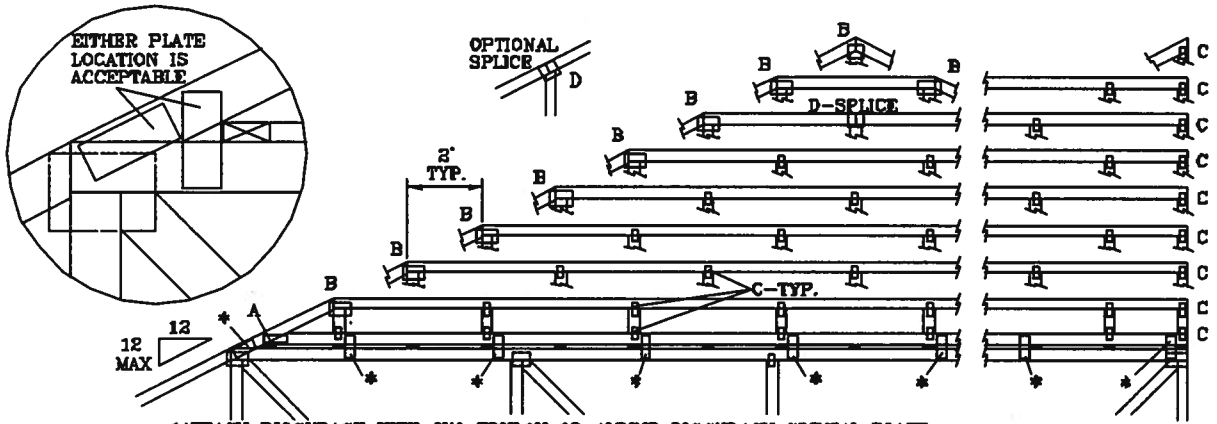
110 MPH WIND, 30' MEAN HGT, ASCE 7-93, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

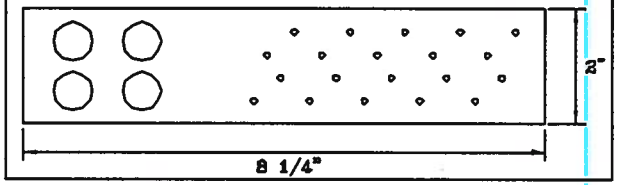
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X8	5X8	5X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X8
E	4X8 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 11/26/03
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

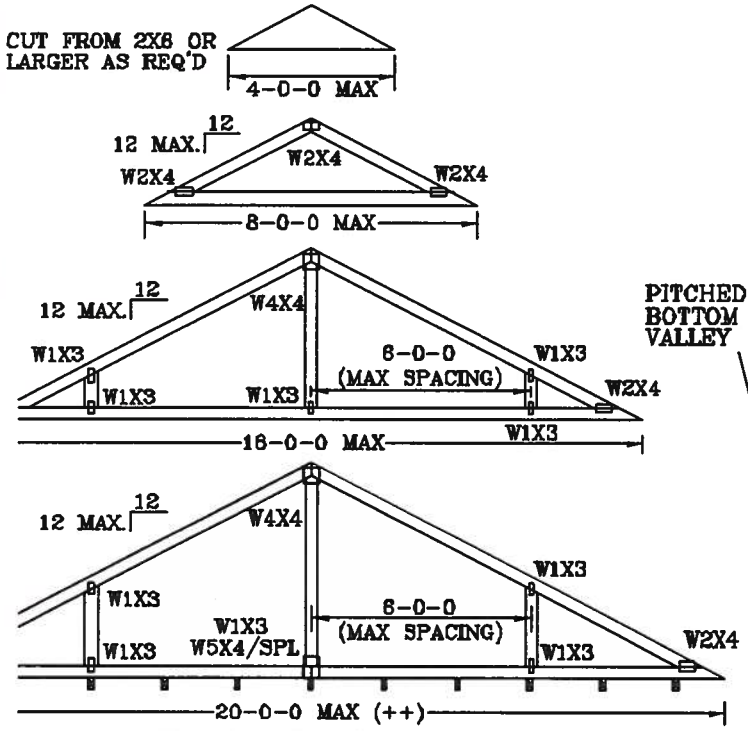
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

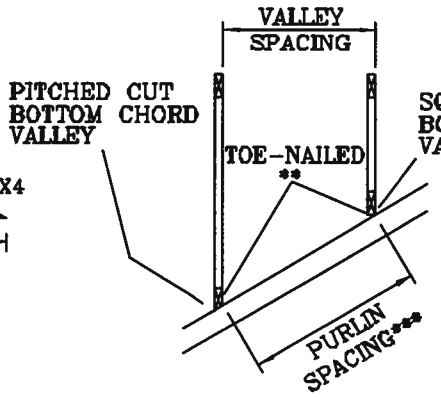
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

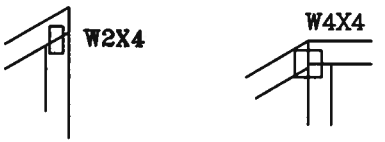
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

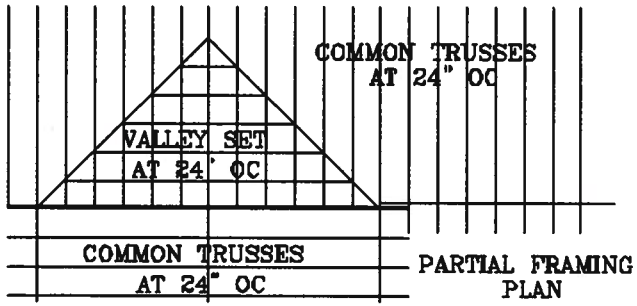
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DURFAC. 1.25		1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

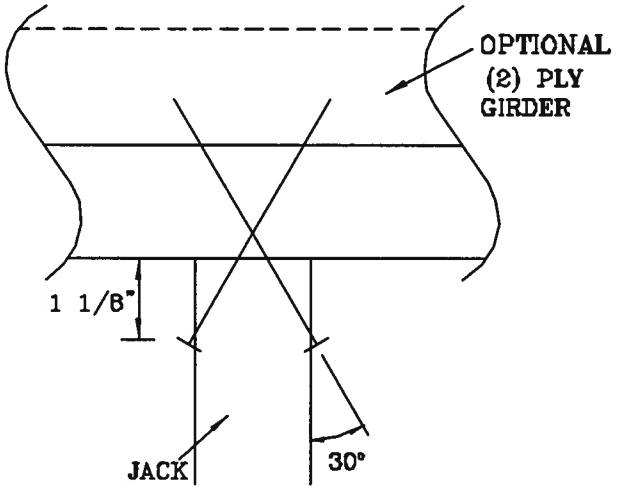
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

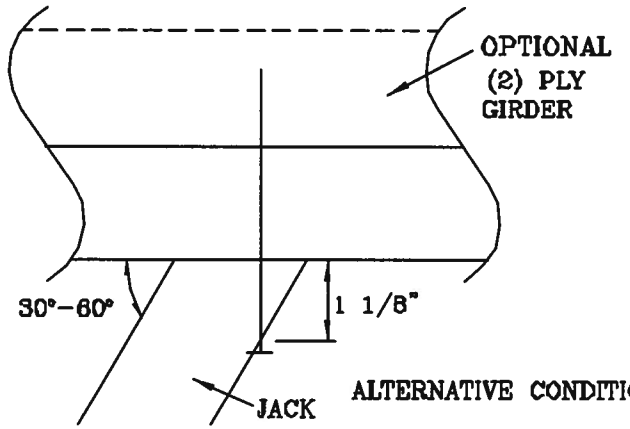
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

==WARNING== TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34669
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

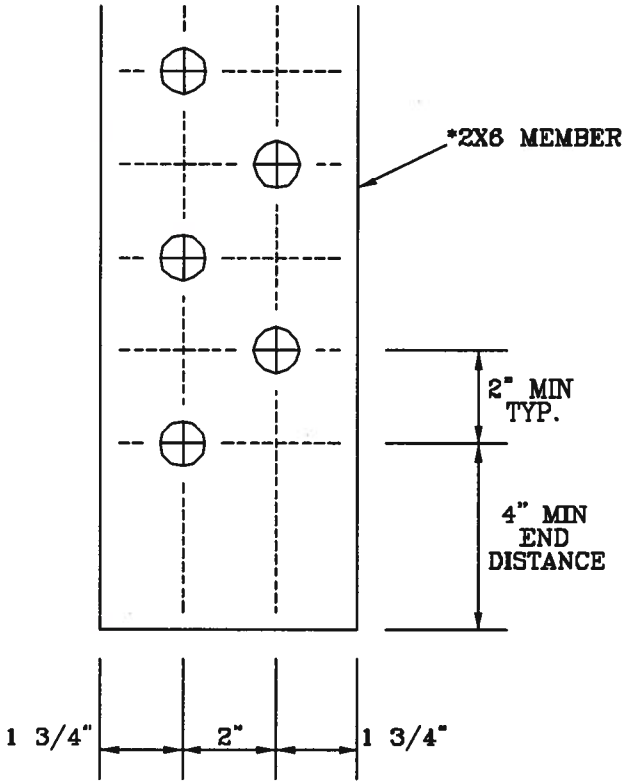
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

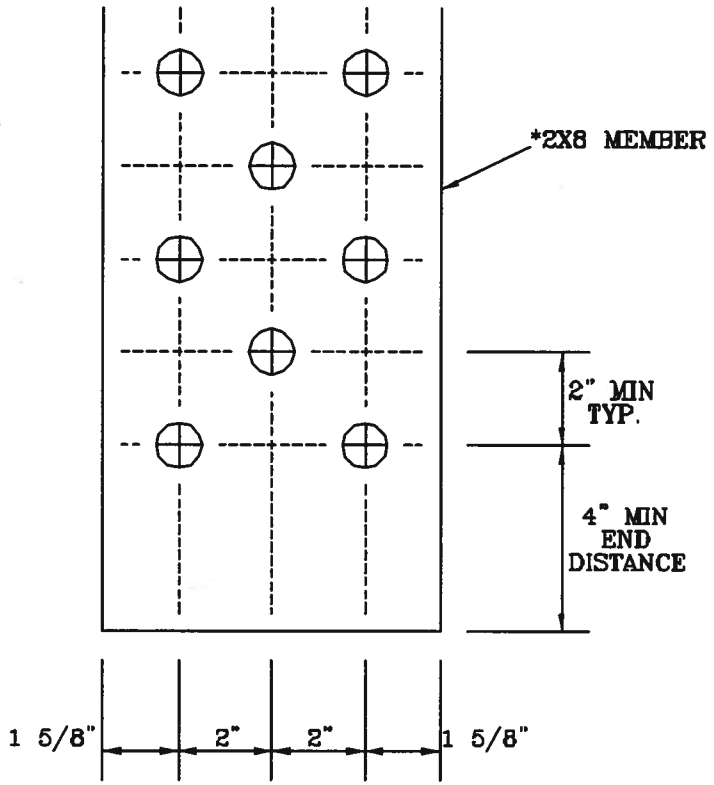
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

DIRECTION OF GRAIN AND LOAD



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A626.016

==WARNING== TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

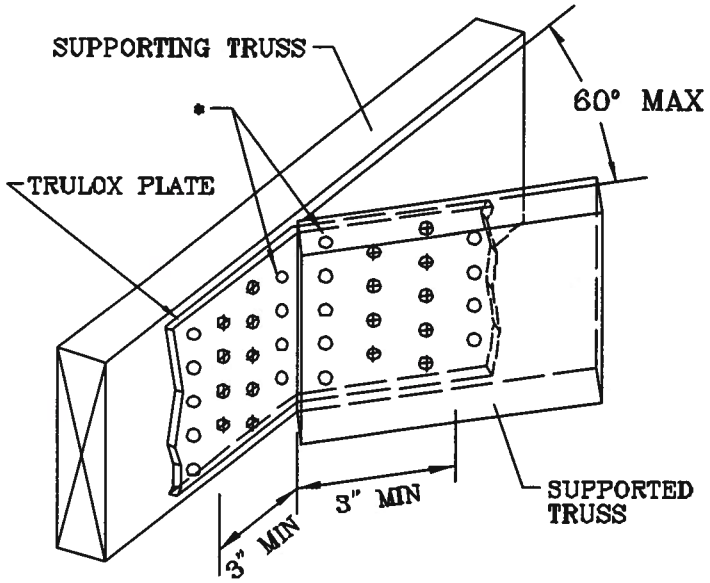
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

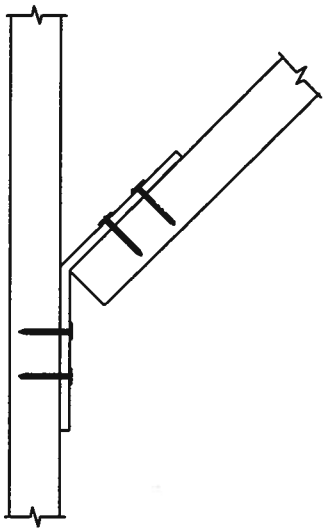
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

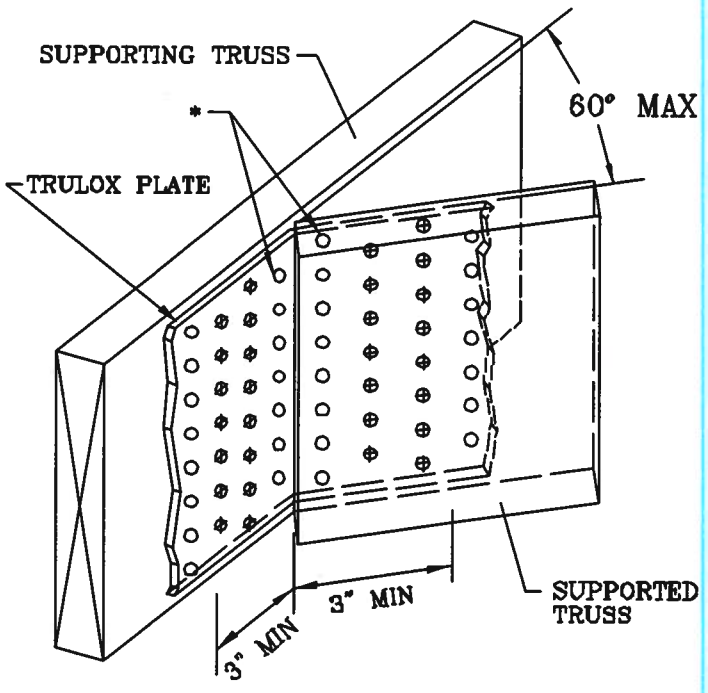
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	16	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

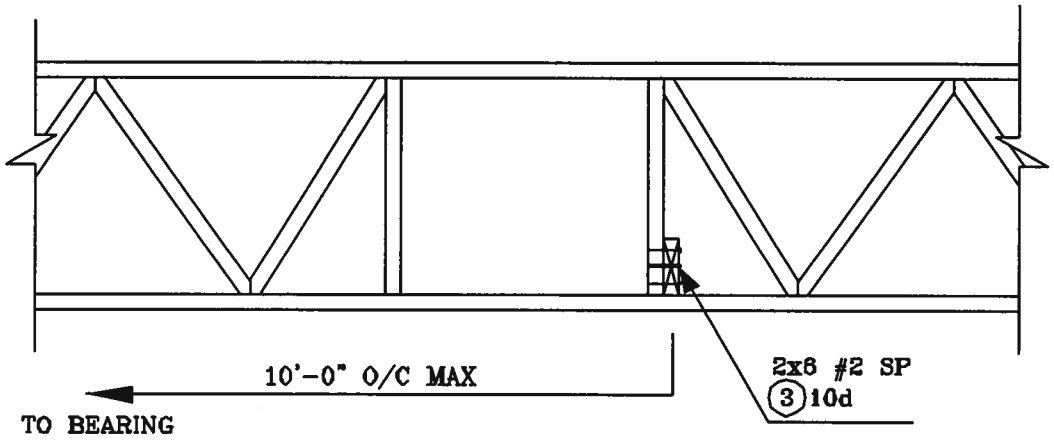
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2381

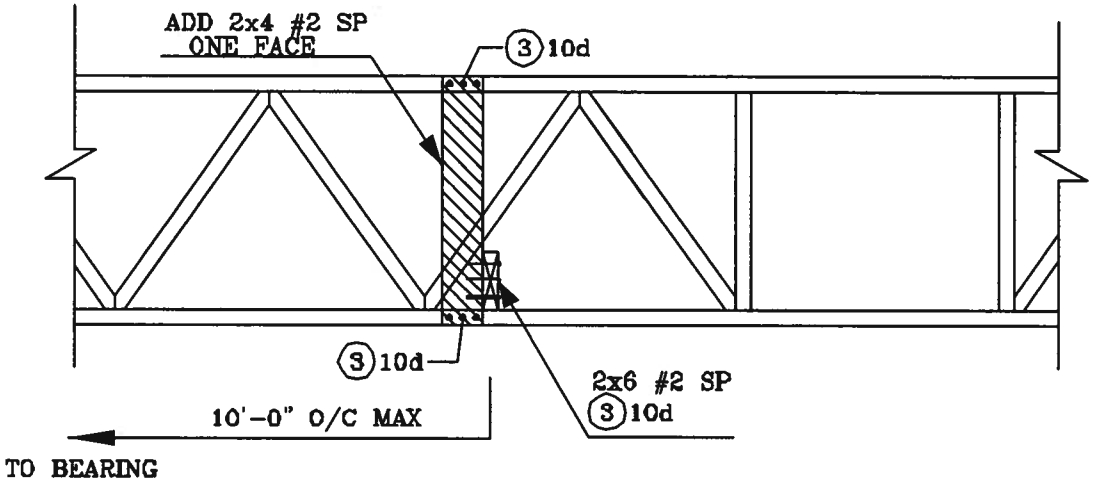
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: Series 8900
TYPE: PVC Fixed Window

Title of Test	Results
AAMA Rating	F-C50 60 x 78
Uniform Load Deflection Test Pressure	±50.0 psf
Air Infiltration	<0.01 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	±75.0 psf
Corner Weld Test	Pass
Forced Entry Resistance	Grade 40

Reference should be made to full report for test specimen description and data.

Report No: 02-46046.01
Report Date: 07/23/03
Expiration Date: 07/17/07

Architectural Testing

AAMA/WDMA 101/I.S. 2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
4661 Burbank Road, P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-46046.01
Test Date:	07/17/03
Report Date:	07/23/03
Expiration Date:	07/17/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies, to perform testing on Series 8900 PVC Fixed window. The sample tested successfully met the performance requirements for a F-C50 60 x 78 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimens were evaluated in accordance with AAMA/WDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: Series 8900

Type: PVC Fixed Window

Overall Size: 4' 11-3/4" wide by 6' 5-3/4" high

Area: 32.3 ft²

Finish: All vinyl was white.

Glazing Details: The window utilized a nominal 3/4" thick insulating glass unit fabricated from two nominal double strength sheets of annealed glass separated by a desiccant filled metal spacer system. The glass was set from the interior against a silicone sealant backbedding. PVC glazing stops were utilized on the interior.

Frame Construction: The frame corners were miter cut and welded.

Installation: The window was installed within a nominal 2" by 8" SPF wood test buck. The window was anchored to the buck with #8 by 1-5/8" wood screws spaced 6" from each corner and 8" to 10" on center. Silicone sealant was used to seal the window to the test buck.

849 Western Avenue North
Saint Paul, MN 55117-5245
phone: 651.636.3835
fax: 651.636.3843
www.archtest.com

Test Results: The results are tabulated as follows:

Paragraph	Title of Test – Test Method	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-91 (See Note #1)		
	@ 1.57 psf (25 mph)	<0.01 cfm/ft^2	0.30 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	<0.01 cfm/ft^2	--

Note #1: The tested specimen meets (or exceeds) the performance levels specified in AAMA/WDMA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance per ASTM E 547-00 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.1.7	Welded Corner Test	Pass	<100% break on weld
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type D		
	Grade 40		
	Lock Manipulation Test	No entry	No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00 and 331-00		
	WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3)		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 60 seconds)		
	@ 50.0 psf (positive)	0.04"	No Damage
	@ 50.0 psf (negative)	0.03"	No Damage
4.4.2	Uniform Load Structural per ASTM E 330-97		
	(Measurements reported were taken in between the anchor points)		
	(Loads were held for 10 seconds)		
	@ 75.0 psf (positive)	<0.01"	0.16" max.
	@ 75.0 psf (negative)	<0.01"	0.16" max.

Note #3: The Uniform Load Deflection test is not an AAMA/WDMA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Eric J. Schoenthaler
Technician

Daniel A. Johnson
Regional Manager

EJS/mb
02-46046.01

Architectural Testing

AAMA/WDMA 101/I.S. 2-97
TEST REPORT

Rendered to:

JORDAN COMPANIES

SERIES/MODEL: 8540
TYPE: PVC Casement Window

Title of Test	Results
AAMA/WDMA Rating	C-R40 (36 x 72)
Uniform Load Deflection Test Pressure	± 40.0 psf
Air Infiltration	0.08 cfm/ft^2
Water Resistance Test Pressure	7.5 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Forced Entry Resistance	Pass Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48974.01
Report Date: 02/06/04
Expiration Date: 02/06/08

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3835
fax: 651.636.3843
www.architest.com

Architectural Testing

AAMA/WDMA 101/I.S.2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No:	02-48974.01
Test Dates:	01/13/04
Thru:	02/06/04
Report Date:	02/12/04
Expiration Date:	02/06/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8540 Casement Window. The sample tested successfully met the performance requirements for a C-R40 36 x 72 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/I.S. 2-97, *"Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors."*

Test Specimen Description:

Series/Model: 8540

Type: PVC Casement Window

Overall Size: 3' 0" wide by 6' 0" high

Sash Size: 2' 10-1/4" wide by 5' 10-1/4" high

Finish: All PVC was white.

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two double-strength annealed sheets and a desiccant-filled metal spacer system. The glass was set from the exterior against a bed of silicone with PVC stops used on the exterior.

849 Western Avenue North
Saint Paul, Minnesota 55117
phone: 651.636.3635
fax: 651.636.3643
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.460" high pile with center fin	1 Row	Perimeter of sash exterior
Foam-filled vinyl bulb gasket	1 Row	Perimeter of sash interior
1/4" EPDM rubber bulb	1 Row	Perimeter of frame

Frame Construction: Frame corners were miter-cut and welded.

Sash Construction: Sash corners were miter-cut and welded.

Hardware:

Dual arm roto-operator	1	Sill
4-point lock with keepers on the sash	1	Locking jamb
Casement hinges	2	Top and bottom corner of sash on hinge side
Metal snubbers	2	24" from top and bottom on hinge side

Installation: The unit was installed into a grade 2 SPF 2" by 8" wood test buck and secured with 1-5/8" screws through the nail fin spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results:

The results are tabulated as follows.

Paragraph	Title of Test	Results	Allowed
2.1.2	Air Infiltration per ASTM E 283-01 (See Note #1)		
	@ 1.57 psf (25 mph)	0.08 cfm/ft^2	0.3 cfm/ft^2 max.
	@ 6.24 psf (50 mph)	0.13 cfm/ft^2	--

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA101/I.S.2-97 for air infiltration.

Test Results: (Continued)

Paragraph	Title of Test	Results	Allowed
2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		

Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."

2.2.5.6.1	Vertical Deflection Test @ 45lbs	0.09"	0.71"
2.2.5.6.2	Hardware Load Test @ 5lbs/ft²	No damage	No damage
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type B		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests B1 through B3	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance:

4.3	Water Resistance per ASTM E 547-00		
	WTP = 7.5 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3)		
	(Measurements reported were taken on the top rail)		
	(Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.10"	(See Note #3)
	@ 40.0 psf (negative)	0.30"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97		
	(Measurements reported were taken on the top rail)		
	(Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.01"	0.136" max.
	@ 60.0 psf (negative)	0.01"	0.136" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

PLS/jb
02-48974.01

DOCUMENT CONTROL ADDENDUM 02-48974.00

Current Issue Date: 02/12/04

Report No. 02-48974.01

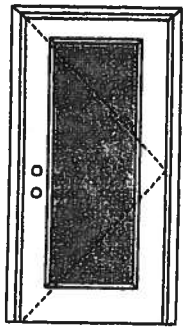
Requested by: Darrel Booth, Jordan Companies
Purpose: AAMA/WDMA 101/I.S. 2-97 testing on a Jordan 8540 Casement
Issue Date: 02/12/04
Comments: Reports and drawings forwarded to ALI for AAMA certification.

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


160 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors
Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


304 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

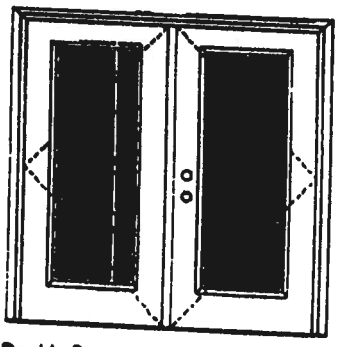
Exclusively from Masonite®
Masonite International Corporation

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited wind uplift design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


130, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


180 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 5-panel with scroll.

Johnson Entry Systems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

XX
Glazed Inswing Unit

COP-WI-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS:


404 Series


410 Series


430 Series

FULL GLASS:


100 Series


114, 120, 122 Series


152 Series


160 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Bathy

State of Florida, Professional Engineer
Kurt Batthazor, P.E. – License Number 56533

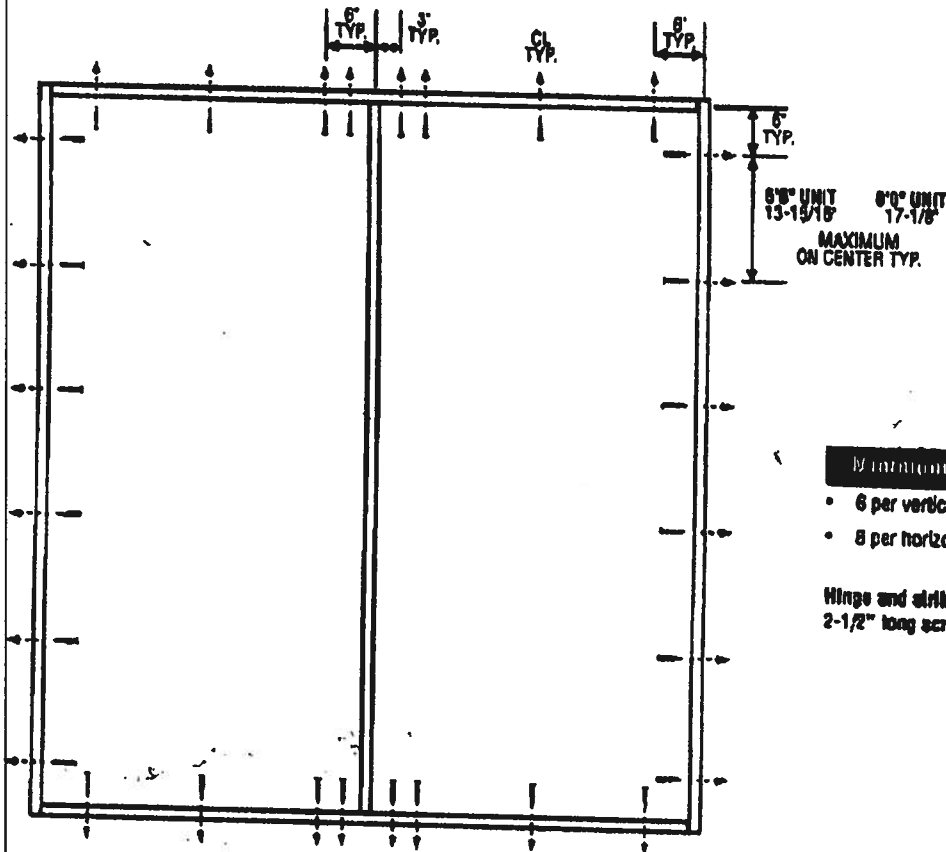
Johnson™ EntrySystems

March 29, 2002

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☐ NA	☐	d) Location, size and height above roof of chimneys
☐ NA	☐	e) Location and size of skylights
☑	☐	f) Building height
☑ (1)	☐	e) Number of stories

Floor Plan including:

- ☑ ☐ a) Rooms labeled and dimensioned
- ☑ ☐ b) Shear walls
- ☑ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☑ ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☐ NA ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☑ ☐ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- ☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☑ ☐ b) All posts and/or column footing including size and reinforcing
- ☑ ☐ c) Any special support required by soil analysis such as piling
- ☑ ☐ d) Location of any vertical steel

Roof System:

- ☑ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- ☑ ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☐ NA ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☑ ☐ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ NA ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ NA ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ NA ☐ b) Floor joist size and spacing

☐ NA ☐ c) Girder size and spacing

☐ NA ☐ d) Attachment of joist to girder

☐ NA ☐ e) Wind load requirements where applicable

☑ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☑ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

☑ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☐ **Private Potable Water**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

EXISTING WELL

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 12-3S-16-02096-001 **Building permit No.** 000026109

Use Classification SFD,UTILITY **Fire:** 0.00

Permit Holder JAMES NORTON **Waste:**

Owner of Building JOE OSBURN **Total:** 0.00

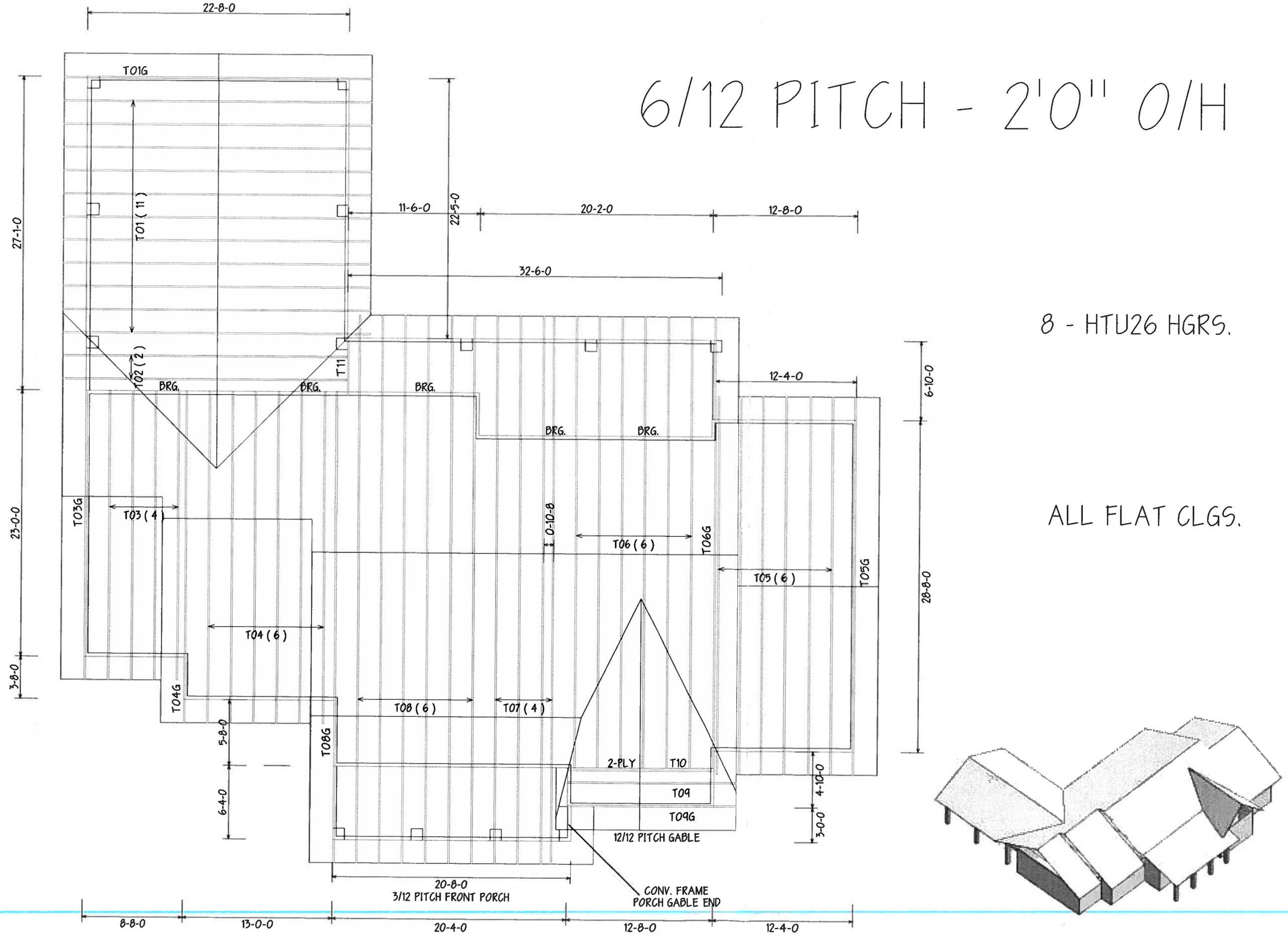
Location: 418 NW MAXMORE DRIVE, LAKE CITY, FL

Date: 06/05/2008

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

6/12 PITCH - 2'0" O/H



8 - HTU26 HGRS.

ALL FLAT CLGS.

BEARING HEIGHT SCHEDULE

	9' 1-1/8"

NOTES:

1) REFER TO HID 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA42Z UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: NORTON BLDG.		
LEGAL ADDRESS: OSBURN RES.		
MODEL: CUSTOM		REVISION: SCALE: NTS
DATE: 7-20-07	DRAWN BY: K.L.H.	JOB #: L247360