# **Julius Lee**

RE: 314347 - Huber Res. / Roof

## 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Site Information:

Project Customer: ADAM'S FRAMING Project Name: 314347 Model: HUBER RES.

Lot/Block:

Subdivision:

Address: 412 SW HIGHPOINT GLENN

City: COLUMBIA CTY

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: ADAM R. PAPKA

License #: CBC1253409

Address: P.O. BOX 1921

City: LAKE CITY,

State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002

Wind Code: ASCE 7-05 Wind Speed: 110 mph

Design Program: MiTek 20/20 7.1

Roof Load: 32.0 psf

Floor Load: N/A psf

This package includes 96 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

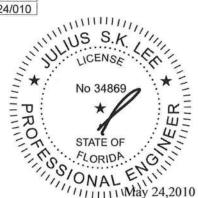
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	14338064	CJ	5/24/010	18	14338081	CJ4A	5/24/010
2	14338065	CJ1	5/24/010	19	14338082	CJ4B	5/24/010
3	14338066	CJ1A	5/24/010	20	14338083	CJ4C	5/24/010
4	14338067	CJ1B	5/24/010	21	14338084	CJ5	5/24/010
5	14338068	CJ1D	5/24/010	22	14338085	CJ5A	5/24/010
6	14338069	CJ1E	5/24/010	23	14338086	CJ5B	5/24/010
7	14338070	CJ1F	5/24/010	24	14338087	CJ5C	5/24/010
8	14338071	CJ2	5/24/010	25	14338088	CJ5D	5/24/010
9	14338072	CJ2A	5/24/010	26	14338089	CJ5E	5/24/010
10	14338073	CJ3	5/24/010	27	14338090	CJ5F	5/24/010
11	14338074	CJ3A	5/24/010	28	14338091	CJ5G	5/24/010
12	14338075	CJ3B	5/24/010	29	14338092	CJ5H	5/24/010
13	14338076	CJ3C	5/24/010	30	14338093	CJ5I	5/24/010
14	14338077	CJ3D	5/24/010	31	14338094	CJ5J	5/24/010
15	14338078	CJ3E	5/24/010	32	14338095	CJ5K	5/24/010
16	14338079	CJ3F	5/24/010	33	14338096	CJ5L	5/24/010
17	14338080	CJ4	5/24/010	34	14338097	CJ5M	5/24/010

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



1 of 4

Julius Lee

RE: 314347 - Huber Res. / Roof

## Site Information:

Project Customer: ADAM'S FRAMING Project Name: 314347 Model: HUBER RES.

Lot/Block: Address: 412 SW HIGHPOINT GLENN Subdivision:

State: FL City: COLUMBIA CTY

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
35	14338098	CJ5N	5/24/010	78	14338141	T44	5/24/010
36	14338099	CJ5O	5/24/010	79	14338142	T45	5/24/010
37	14338100	CJ6	5/24/010	80	14338143	T46	5/24/010
38	14338101	EJ6	5/24/010	81	14338144	T47	5/24/010
39	14338102	EJ7	5/24/010		14338145	T48	5/24/010
40	14338103	EJ7A	5/24/010	83	14338146	T49	5/24/010
41	14338104	EJ7C	5/24/010	84	14338147	T50	5/24/010
42	14338105	FG4	5/24/010	85	14338148	T51	5/24/010
43	14338106	HJ4	5/24/010	86	14338149	T52	5/24/010
44	14338107	HJ5	5/24/010	87	14338150	T53	5/24/010
45	14338108	HJ6	5/24/010	88	14338151	T54	5/24/010
46	14338109	HJ8	5/24/010	89	14338152	T55	5/24/010
47	14338110	HJ8A	5/24/010	90	14338153	T56	5/24/010
48	14338111	HJ9	5/24/010	91	14338154	T57	5/24/010
49	14338112	HJ9A	5/24/010	92	14338155	T58	5/24/010
50	14338113	HJ9B	5/24/010	93	14338156	T59	5/24/010
51	14338114	T01	5/24/010	94	14338157	T60	5/24/010
52	14338115	T02	5/24/010	95	14338158	T61	5/24/010
53	14338116	T03	5/24/010	96	14338159	T62	5/24/010
54	14338117	T04	5/24/010				
55	14338118	T05	5/24/010				
56	14338119	T06	5/24/010				
57	14338120	T07	5/24/010				
58	14338121	T08	5/24/010				
59	14338122	T09	5/24/010				
60	14338123	T10	5/24/010				
61	14338124	T11	5/24/010	Ī			
62	14338125	T12	5/24/010				
63	14338126	T13	5/24/010				
64	14338127	T14	5/24/010				
65	14338128	T15	5/24/010				
66	14338129	T32	5/24/010				
67	14338130	T33	5/24/010				
68	14338131	T34	5/24/010				
69	14338132	T35	5/24/010				
70	14338133	T36	5/24/010				
71	14338134	T37	5/24/010				
72	14338135	T38	5/24/010	-			
73	14338136	T39	5/24/010	75			
74	14338137	T40	5/24/010				
75	14338138	T41	5/24/010				
76	14338139	T42	5/24/010				
77	14338140	T43	5/24/010	-			

Job Truss Type Truss Qty Huber Res. / Roof 314347 CJ JACK Job Reference (optional) s Oct 1 2009 MiTek Industr ies, Inc. Mon May 24 11:03:32 2010 Page 1

Builders FrstSource, Lake City, FL 32055

1-0-0 -2-3-8 2-3-8

Scale = 1:32.5

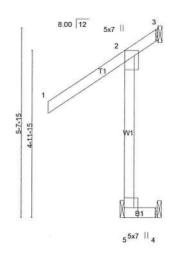


Plate Offsets (X.Y): [2:0-4-3.Edge]

LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.67	Vert(LL)	0.00	5	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.69	Vert(TL)	0.00	5	>999	240	1,000,000	
BCLL	0.0	•	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.41	3	n/a	n/a		
BCDL	5.0	- 1	Code FBC2007/TF	12002	(Matr	rix)	Wind(LL)	0.00	4-5	>999	240	Weight: 14 lb	e.

LUMBER

TOP CHORD 2 X 4 SYP No 2 BOT CHORD 2 X 4 SYP No.2 WEBS

2 X 4 SYP No.1D

BRACING

TOP CHORD BOT CHORD

end verticals

Structural wood sheathing directly applied or 1-0-0 oc purlins, except

Rigid ceiling directly applied or 9-9-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (Ib/size) 5=349/Mechanical, 4=-12/Mechanical, 3=-152/Mechanical Max Horz 5=179(LC 6)

Max Uplift4=-497(LC 6), 3=-152(LC 1) Max Grav 5=377(LC 6), 4=8(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-5=-329/120

NOTES (9-10)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

The Fabrication Tolerance at joint 2 = 0%
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

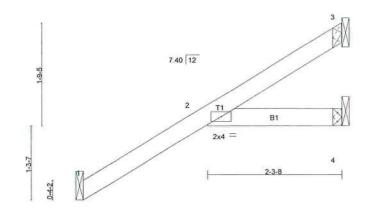
Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.

Job Truss Truss Type Qty Huber Res. / Roof 14338065 314347 CJ1 JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:33 2010 Page 1 Builders FrstSource, Lake City, FL 32055

2-9-14 2-9-14 1-7-1

Scale = 1:18.8



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.42	Vert(LL)	-0.04	2	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.08	2	>635	240		
BCLL	0.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.08	4	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	212002	(Matr	ix)	Wind(LL)	0.14	2	>365	240	Weight: 11 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 1-7-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=141/Mechanical, 4=11/Mechanical, 3=128/Mechanical

Max Horz 1=211(LC 6)

Max Uplift 1=-121(LC 6), 4=-39(LC 4), 3=-206(LC 6)

Max Grav 1=141(LC 1), 4=34(LC 2), 3=128(LC 1)

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 1, 39 lb uplift at joint 4 and 206 lb uplift at joint 3.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240	10000000000	
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TF	P12002	(Matr	nix)	Wind(LL)	0.00	2	>999	240	Weight: 7 lb	

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=158(LC 6)

Max Uplift 2=-431(LC 6), 4=-17(LC 4), 3=-90(LC 1)

Max Grav 2=256(LC 1), 4=14(LC 2), 3=202(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

(8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable ent zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 2, 17 lb uplift at joint 4 and 90 lb uplift at joint 3.

"Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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Qty Huber Res. / Roof Job Truss Truss Type 14338067 314347 CJ1B JACK lob Reference (optional) s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:33 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-8 1-0-0 2-0-8 1-0-0 Scale = 1:13.7 3x5 || 8.00 12 2-1-0 T1 W1 2x4 || LOADING (psf) PLATES GRIP DEFL SPACING CSI 2-0-0 I/defl 1./d 244/190 TC Vert(LL) >999 360 MT20 TCLL 20.0 Plates Increase 1.25 0.53 0.00 5 7.0 1.25 BC 0.09 240 TCDL Lumber Increase Vert(TL) 0.00 >999 Rep Stress Incr BCLL 0.0 YES WR 0.00 Horz(TL) -0.01 3 n/a n/a

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

5.0

Wind(LL) BRACING

TOP CHORD

0.00

5 >999

**BOT CHORD** 

Structural wood sheathing directly applied or 1-0-0 oc purlins, except end verticals

Weight: 9 lb

Rigid ceiling directly applied or 10-0-0 oc bracing.

240

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=299/0-5-0, 4=-23/Mechanical, 3=-103/Mechanical

Code FBC2007/TPI2002

Max Horz 5=165(LC 6)

Max Uplift5=-290(LC 6), 4=-59(LC 6), 3=-103(LC 1)

Max Grav 5=299(LC 1), 4=5(LC 2), 3=132(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-5=-267/411

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27fl; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

(Matrix)

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads, 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

- 5) Refer to girder(s) for truss to truss connections.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

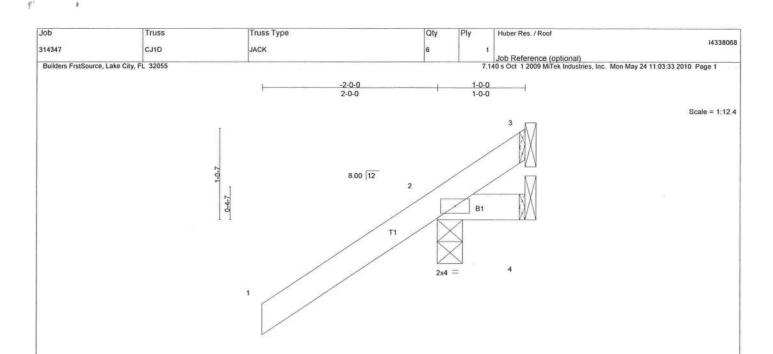
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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May 24,2010

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/ITI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute. 583 D'Onofrio Drive. Madison. WI 53719.



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.00	2	>999	360	MT20	244/190	
CDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240	7.0000/120019		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a			
BCDL	5.0	Code FBC2007/T	PI2002	(Mati	rix)	Wind(LL)	0.00	2	>999	240	Weight: 7 lb		

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical

Max Horz 2=158(LC 6)

Max Uplift 2=-434(LC 6), 4=-17(LC 4), 3=-90(LC 1)

Max Grav 2=256(LC 1), 4=14(LC 2), 3=205(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES (8-9)

  1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable eng zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 434 lb uplift at joint 2, 17 lb uplift at joint 4 and 90 lb uplift at joint 3.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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Truss Type Qty Huber Res. / Roof Job 14338069 314347 CJ1E JACK

Builders FrstSource, Lake City, FL 32055

Job Reference (optional) 7 140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:33 2010 Page 1



Scale = 1:31.7

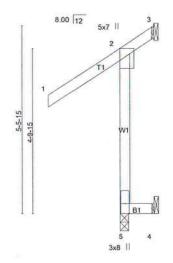


Plate Offsets (X,Y): [2:0-4-3,Edge]

LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.59	Vert(LL)	0.00	5	>999	360	MT20	244/190
CDL	7.0	. 1	Lumber Increase	1.25	BC	0.61	Vert(TL)	0.00	5	>999	240	1,00000000	
BCLL	0.0		Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.35	3	n/a	n/a	10.11.00.00.00.00.00.00.00.00.00.00.00.0	
BCDL	5.0		Code FBC2007/TF	12002	(Matr	rix)	Wind(LL)	0.00	4-5	>999	240	Weight: 14 lb	K@

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.1D WEBS

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins, except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=299/0-3-0, 4=-9/Mechanical, 3=-118/Mechanical

Max Horz 5=165(LC 6)

Max Uplift4=-443(LC 6), 3=-164(LC 6) Max Grav 5=362(LC 6), 4=9(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-281/74

NOTES (9-10)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) The Fabrication Tolerance at joint 2 = 12%
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Reveter.

May 24,2010

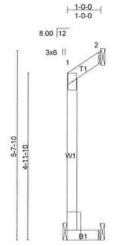
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. ARRING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE. Design volid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. not truss designer. Bracing shows is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Intermation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Truss Type Qty Ply Huber Res / Roof 14338070 CJIF

Builders FrstSource, Lake City, FL 32055

Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:33 2010 Page 1

Scale = 1:32.4



LOADING (psf) SPACING CSI DEFL PLATES GRIP 2-0-0 I/defi L/d (loc) TCLL 20.0 Plates Increase 1.25 TC 0.99 Vert(LL) -0.00 >999 360 MT20 244/190 1 25 BC. TCDL 70 Lumber Increase 0.85 Vert(TL) -0.00 4 >999 240 MT20H 187/143 0.0 0.00 BCLL Rep Stress Incr WB -0.49 Horz(TL) n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.01 >999 240 Weight: 10 lb 3-4

5x10 MT20H ||

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins, except

Rigid ceiling directly applied or 5-3-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 4=27/Mechanical, 3=4/Mechanical, 2=23/Mechanical

Max Horz 4=187(LC 6)

Max Uplift 4=-416(LC 4), 3=-380(LC 6), 2=-279(LC 6) Max Grav 4=617(LC 6), 3=277(LC 4), 2=155(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-4=-393/143, 1-2=-250/85

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are MT20 plates unless otherwise indicated.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2

LOAD CASE(S) Standard

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular, building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Ray Phyl. Design Phyl. Design PROTESTATE STATE S

May 24,201

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual bullding component, Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to inset stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, defivery, erection and bracing, consult. ANS/IPI Quality Citleria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Type Qty Ply Huber Res. / Roof Truss 1433807 314347 CJ2 JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:34 2010 Page 1 Builders FrstSource, Lake City, FL 32055

Scale = 1:21.5

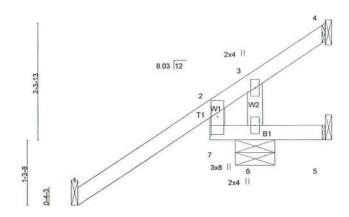


Plate Offsets (X V): [7:0-4-4 0-1-8]

OADIN	IG (psf)	- 1	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.23	Vert(LL)	-0.00	7	>999	360	MT20	244/190
CDL	7.0		Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	7	>999	240		
BCLL	0.0		Rep Stress Incr	NO	WB	0.03	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	P12002	(Matr	ix)	Wind(LL)	0.00	7	>999	240	Weight: 14 lb	)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 2-3-4 oc purlins, except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings Mechanical except (jt=length) 6=0-9-4.

(lb) - Max Horz 6=201(LC 5)

Max Uplift All uplift 100 lb or less at joint(s) 4, 5, 6 except 1=-128(LC 5) Max Grav All reactions 250 lb or less at joint(s) 1, 4, 5 except 6=313(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5, 6 except (jt=lb) 1=128

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 65 lb down and 37 lb up at 0-0-0, and 65 lb down and 50 lb up at -2-6-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54, 2-4=-54, 5-7=-10

Concentrated Loads (lb) Vert: 2=-65(F) 1=-65

rs. JULIUS S.K No 3 P. STAIL STA

May 24,2010

♠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for taleratis support of individual web members only. Additional temporary bracing to inset stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMS/ITPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee

Qty Job Truss Truss Type Ply Huber Res. / Roof 14338072 314347 CJZA JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:34 2010 Page 1 Builders FrstSource, Lake City, FL 32055

> -2-3-8 3-0-0 2-3-8 3-0-0

> > Scale = 1:39.1

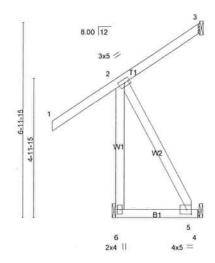


Plate Offsets (X,Y): [5:0-2-0,0-2-0]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.00	5-6	>999	360	MT20	244/190	
CDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.01	5-6	>999	240	0.0000000000000000000000000000000000000		
BCLL	0.0	Rep Stress Incr	YES	WB	0.24	Horz(TL)	-0.05	3	n/a	n/a			
BCDL	5.0	Code FBC2007/TF	PI2002	(Matr	rix)	Wind(LL)	0.03	5-6	>999	240	Weight: 29 lb	0	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins, except

end verticals

**BOT CHORD** Rigid ceiling directly applied or 8-4-8 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

U

REACTIONS (lb/size) 6=278/Mechanical, 3=18/Mechanical, 4=14/Mechanical

Max Horz 6=414(LC 6)

Max Uplift 6=-173(LC 4), 3=-32(LC 7), 4=-617(LC 6) Max Grav 6=278(LC 1), 3=51(LC 4), 4=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD

2-6=-421/131

**BOT CHORD** 5-6=-449/43

WEBS

2-5=-90/949

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

- o) Refer to girder(s) for truss to truss connections.
  6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 6=173 | VIIII | VIIII
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34869 ROMAL E

May 24,201

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design per amenders and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toleral support of Individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

5.0

Wind(LL) BRACING 2-4

>999

240

0.01

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Weight: 14 lb

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=240(LC 6) Max Uplift3=-50(LC 7), 2=-333(LC 6), 4=-48(LC 4) Max Grav 3=43(LC 4), 2=250(LC 1), 4=42(LC 2)

Code FBC2007/TPI2002

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

(Matrix)

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=333

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular

building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

with State U 305 STATE OF IIIIONAL

Job Truss Type Qty Huber Res. / Roof Truss 14338074 314347 CJ3A JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:34 2010 Page 1 Builders FrstSource, Lake City, FL 32055 3-0-0 2-0-8 Scale = 1:20.4 8.00 12 3x8 || B1 2x6 ||

LOADIN	G (psf)	- 1	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.72	Vert(LL)	-0.00	4-5	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.00	4-5	>999	240	attraction.	
BCLL	0.0		Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.03	3	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	PI2002	(Matr	rix)	Wind(LL)	0.01	4-5	>999	240	Weight: 15 lb	

### LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 5=254/0-5-0, 3=39/Mechanical, 4=4/Mechanical Max Horz 5=285(LC 6) Max Uplift5=-193(LC 6), 3=-90(LC 6), 4=-29(LC 6)

Max Grav 5=254(LC 1), 3=39(LC 1), 4=37(LC 2)

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-230/273

### NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

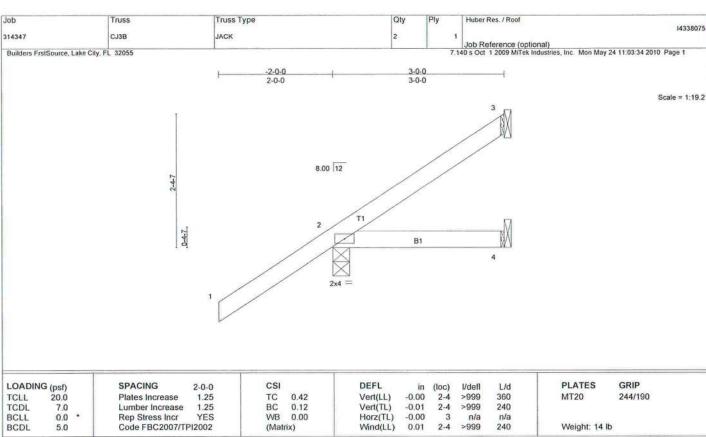
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 STATE OF FLORIDA SIONAL THUMAL

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with Millek connectors. This design is based only upon porometers shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/IPI Quality Citetia, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical

Max Horz 2=241(LC 6)
Max Uplift 3=-49(LC 7), 2=-335(LC 6), 4=-48(LC 4)
Max Grav 3=43(LC 4), 2=250(LC 1), 4=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable eng zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=335

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular

building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

with State POF I NO NAL

Job Truss Truss Type Qty Huber Res. / Roof 14338076 314347 CJ3C SPECIAL Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:35 2010 Page 1 Builders FrstSource, Lake City, FL 32055 1-0-0 2-0-0 2-3-8 Scale = 1:20.2 1-2-12 8.00 12 0-4-7 7.00 12 1-0-0

Plate O	ffsets (>	K,Y	(): [2	2:0-2-4,0-2-0]										
LOADIN	IG (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	)		Plates Increase	1.25	TC	0.41	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	)		Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	0.0	)		Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	)		Code FBC2007/	TPI2002	(Mati	ix)	Wind(LL)	0.00	2	****	240	Weight: 22 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP No.1D BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-3-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 3=36/Mechanical, 2=261/0-5-8, 4=15/Mechanical Max Horz 2=247(LC 6)

Max Uplift3=-69(LC 7), 2=-271(LC 6)

Max Grav 3=44(LC 4), 2=261(LC 1), 4=45(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard PROTING STAIL

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Miffek connectors. This design is based only upon parameters shown, and is for an individual building component.

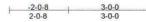
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not frus designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult. AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.

1109 Coastal Bay Blvd. Boynton, FL 33435

Job Qty Huber Res. / Roof Truss Type 14338077 314347 CJ3D JACK

Builders FrstSource, Lake City, FL 32055

lob Reference (optional)
Is Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:35 2010 Page 1



Scale = 1:38.2

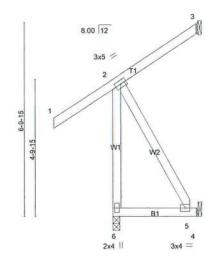


Plate Offsets (X,Y): [5:0-0-0,0-0-0]

LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	- 1	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.00	5-6	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.01	5-6	>999	240	1980,403,500,500	
BCLL	0.0		Rep Stress Incr	YES	WB	0.22	Horz(TL)	-0.04	3	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	212002	(Matr	ix)	Wind(LL)	0.03	5-6	>999	240	Weight: 28 lb	0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 3-0-0 oc purlins, except

Rigid ceiling directly applied or 8-7-14 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=254/0-3-0, 3=29/Mechanical, 4=14/Mechanical

Max Horz 6=394(LC 6)

Max Uplift6=-163(LC 4), 3=-41(LC 7), 4=-568(LC 6) Max Grav 6=254(LC 1), 3=46(LC 4), 4=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-6=-403/122 5-6=-427/44

BOT CHORD WEBS

2-5=-91/877

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

- o) Refer to girder(s) for truss to truss connections.
  6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 6=163 (CENS).
  7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and dark.
  8) This manufactured product is designed as as in the signed as a sig

- building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

  9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435—

LOAD CASE(S) Standard

No 348 FLORING ONAL E U

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TPI Quality Citleria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Type Qty luber Res. / Roof 14338078 314347 CJ3E JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:35 2010 Page 1

Builders FrstSource, Lake City, FL 32055

Scale = 1:38.9

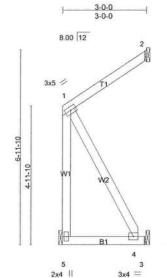


Plate Offsets (X,Y): [4:0-0-0,0-0-0]

LOADIN	IG (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d
TCLL	20.0		Plates Increase	1.25	TC	0.64	Vert(LL)	-0.00	4-5	>999	360
TCDL	7.0		Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.01	4-5	>999	240
BCLL	0.0	*	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.03	2	n/a	n/a
BCDL	5.0		Code FBC2007/TF	P12002	(Mati	rix)	Wind(LL)	0.01	4-5	>999	240

**PLATES** GRIP MT20 244/190

Weight: 24 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=89/Mechanical, 2=75/Mechanical, 3=14/Mechanical

Max Horz 5=269(LC 6)

Max Uplift5=-115(LC 4), 2=-116(LC 6), 3=-320(LC 6) Max Grav 5=300(LC 6), 2=75(LC 1), 3=148(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

1-5=-483/136 4-5=-284/82

BOT CHORD

WEBS

1-4=-173/597

## NOTES

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 2) This truss has been designed for a live toau ...
  fit between the bottom chord and any other members.

  4) All bearings are assumed to be SYP No.2.
  5) Refer to girder(s) for truss to truss connections.
  6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except to 2=116, 3=320.
  7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
  8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
  9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435—P. CASE(S) Standard

THINDING S.L. No 34669 STATE OF SIONAL FLORIDA MILLIAM

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TPI Quality Citeria, DSB-89 and BCS11 Building Component Safety Information available from Truss Pfate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Qty Huber Res. / Roof Job Truss Type 14338079 314347 CJ3F JACK

Builders FrstSource, Lake City, FL 32055

Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:35 2010 Page 1



Scale = 1:41.2

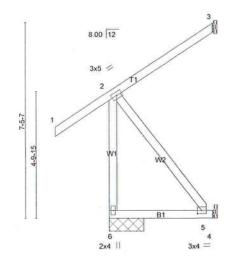


Plate Offsets	(X,Y):	[5:0-1-13	3,0-1-81	
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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.01	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.02	5-6	>999	240	SAMATRASIS	
BCLL	0.0	Rep Stress Incr	YES	WB	0.19	Horz(TL)	-0.03	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TF	PI2002	(Matr	ix)	Wind(LL)	0.03	5-6	>999	240	Weight: 32 lb	)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-11-3 oc purlins, except end verticals

BOT CHORD Rigid ceiling directly applied or 8-8-7 oc bracing.

> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=272/1-3-8, 3=66/Mechanical, 4=19/Mechanical

Max Horz 6=433(LC 6)

Max Uplift 6=-79(LC 4), 3=-78(LC 6), 4=-433(LC 6) Max Grav 6=272(LC 1), 3=66(LC 1), 4=70(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-6=-271/94 TOP CHORD

BOT CHORD 5-6=-470/48

WEBS 2-5=-79/778

NOTES

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 3 except (jt=lb) 4=433.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular \* building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

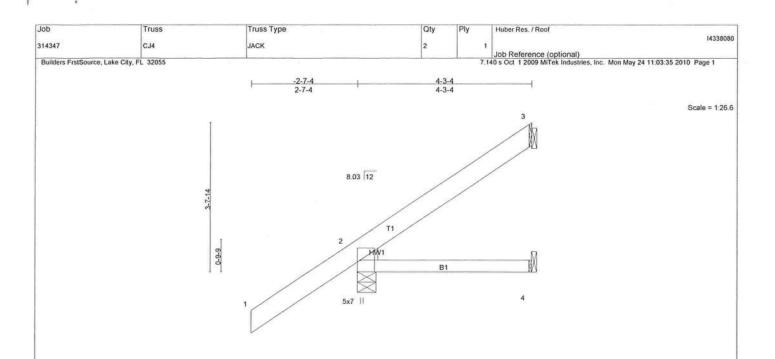
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435—

LOAD CASE(S) Standard

WILLIAM S.K. No 348 U

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult. ANSI/IPII Quality Criteria, DSB-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	2-4	>999	240		
BCLL	0.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TF	12002	(Matr	rix)	Wind(LL)	0.00	2	****	240	Weight: 27 lb	

TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-3-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=-8/Mechanical, 2=566/0-5-8, 4=20/Mechanical Max Horz 2=323(LC 5) Max Uplift3=-64(LC 6), 2=-504(LC 5)

Max Grav 3=53(LC 3), 2=566(LC 1), 4=60(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- fit between the bottom chord and any other members
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=504
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 118 lb up at 0-0-0, and 85 lb down and 65 lb up at -2-7-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

  10) This manufactured product is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

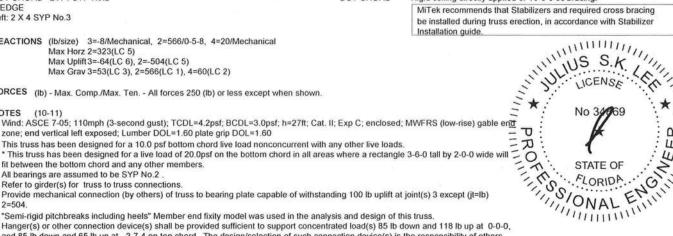
## LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-3=-54, 2-4=-10

Concentrated Loads (lb)

Vert: 1=-85(F) 2=-85(F)



May 24,2010

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🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information

available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Qty Job Truss Type Huber Res. / Roo 14338081 314347 CJ4A SPECIAL Job Reference (optional) Is Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:36 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-2-11 2-2-11 2-6-10 2-6-10 4-7-4 2-0-10 Scale = 1:23.5 3x5 < 7.34 12 2-8-9 3 3x5 ⋍ **B3** WZ 4x12 = 6 1-2-12 M3 1-4-0 3x4 = U **B1** 8 2x4 || 3x4 2-6-10 2-6-10 4-7-4 2-0-10 LOADING (psf) SPACING CSI DEFL **PLATES** GRIP L/d (loc) TC BC TCLL 20.0 Plates Increase 1.25 0.54 Vert(LL) -0.00 8 >999 360 MT20 244/190 TCDL 70 Lumber Increase 1.25 0.06 Vert(TL) -0.00 6-7 >999 240 0.0 WB 0.07 Horz(TL) -0.01 n/a BCLL Rep Stress Incr YES n/a BCDL Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.00 8 >999 240 Weight: 33 lb 5.0 BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 4-7-4 oc purlins, except BOT CHORD 2 X 4 SYP No.2 \*Except\* Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD B2: 2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 9=303/0-5-8, 4=63/Mechanical, 5=43/Mechanical Max Horz 9=330(LC 6) Max Uplift9=-246(LC 6), 4=-102(LC 6), 5=-61(LC 7) Max Grav 9=303(LC 1), 4=63(LC 1), 5=69(LC 2) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-9=-289/533 7-9=-435/0 TOP CHORD WEBS NOTES (8-9) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SYP No.2 . 4=102.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1100 Country 1000 In 1000 PR. STA. STA. FLORID. LOAD CASE(S) Standard

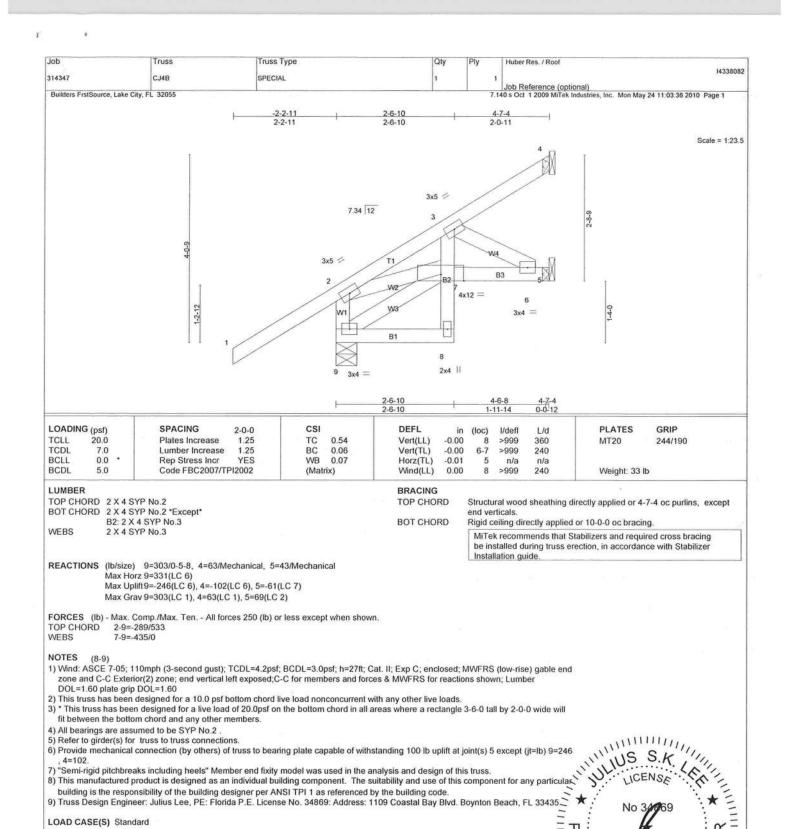
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

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AMSI/TPU (Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters have milet Pale Mil

LOAD CASE(S) Standard

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

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May 24,2010

STATE OF

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SIONAL FLORIDA

Job Truss Truss Type Qty Huber Res. / Root 314347 CJ4C SPECIAL Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:36 2010 Page 1 Builders FrstSource, Lake City, FL 32055 Scale = 1:21.4 7.34 12 3x5 / 0-11-4 B1 4 3 4x5 = 2x4 | Plate Offsets (X,Y): [4:0-0-3,0-2-0] LOADING (psf) SPACING CSI DEFL PLATES GRIP 2-0-0 (loc) l/defl 1 /d Vert(LL) -0.02 >999 360 MT20 244/190 TCLL Plates Increase TC 0.28 20.0 1.25 4-5 TCDL 7.0 1.25 BC Vert(TL) -0.03 4-5 >999 240 Lumber Increase 0.13 BCLL 00 Rep Stress Incr YES WR 0.05 Horz(TL) -0.00 2 n/a n/a Code FBC2007/TPI2002 240 Weight: 21 lb 0.00 4-5 >999 (Matrix) Wind(LL) BCDL 5.0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 4-5-1 oc purlins, except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilize Installation guide.

REACTIONS (lb/size) 5=135/Mechanical, 2=114/Mechanical, 3=21/Mechanical Max Horz 5=163(LC 6)

Max Uplift5=-25(LC 6), 2=-165(LC 6), 3=-15(LC 6)

Max Grav 5=135(LC 1), 2=114(LC 1), 3=63(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3 except (jt=lb) 2=165.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34909

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May 24,2010

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual bullding component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insus stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult. ANSI/IPI Quality Citeria, DSB-89 and BCS11 Building Component Safety Information. available from trust Plate Institute. 583 D'Onofrio Drive, Madison. WI 53719.

Job Truss Truss Type Qty Huber Res. / Roof 314347 CJ5 Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:36 2010 Page 1 Builders FrstSource, Lake City, FL 32055 5-4-9 5-4-9 2-2-0 2-2-0 Scale = 1:25.7 7.40 12 HW1 0-4-12 B1 5x8 || 0-10-9

Plate Offsets (X,Y): [2:0-4-5,0-0-7], [2:0-0-10,0-10-0] LOADING (psf) SPACING CSI DEFL PLATES GRIP TCLL 20.0 Plates Increase 1.25 TC 0.63 Vert(LL) -0.032-4 >999 360 MT20 244/190 BC TCDL 7.0 1.25 0.38 2-4 -0.06 >999 240 Lumber Increase Vert(TL) 0.0 Rep Stress Incr WB 0.00 Horz(TL) -0.00 BCLL n/a n/a

BCDL LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

5.0

WEDGE

Left: 2 X 8 SYP No.1D

Wind(LL) BRACING

0.15

2-4 >416

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 5-4-9 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

240

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

Weight: 24 lb

REACTIONS (lb/size) 3=104/Mechanical, 4=25/Mechanical, 2=325/0-6-10

Code FBC2007/TPI2002

Max Horz 2=324(LC 6)

Max Uplift3=-148(LC 6), 4=-87(LC 4), 2=-396(LC 6)

Max Grav 3=104(LC 1), 4=76(LC 2), 2=325(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

(Matrix)

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

4) All bearings are assumed to be SYP No.2

- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=148

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

PRO No 10NAL minn

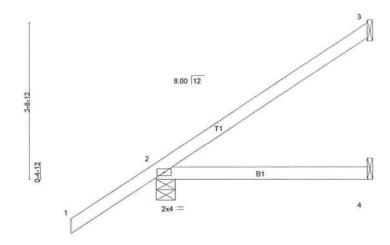
May 24,2010

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and reper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to have stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, qualify control, storage, delivery, erection and bracing, consult. AMS/IPI/Quality Citeria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive. Madison, WI 53719.

Job Truss Truss Type Qty Huber Res. / Roof 14338085 314347 CJ5A JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:36 2010 Page 1 Builders FrstSource, Lake City, FL 32055

> 2-0-1 5-0-0

> > Scale = 1:25.8



LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.44	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240	110000000040	
BCLL	0.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	PI2002	(Matr	ix)	Wind(LL)	0.00	2	****	240	Weight: 20 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 3=98/Mechanical, 2=300/0-5-8, 4=24/Mechanical

Max Horz 2=325(LC 6)

Max Uplift3=-148(LC 6), 2=-275(LC 6)

Max Grav 3=98(LC 1), 2=300(LC 1), 4=71(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

.... Stabilizer

..... Stabilizer

LICENSE ...... NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp U; encoded and content of the second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp U; encoded and content of the second gust of the second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp U; encoded and second gust of the second gust of t Ш NG

Job Truss Truss Type Qty Huber Res. / Roof 314347 CJ5B Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:37 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 5-0-0 2-0-0 5-0-0 Scale = 1:25.8 8.00 12 0-4-11 2x4 =

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.05	2-4	>999	240	22722755	
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TP	12002	(Matr	nix)	Wind(LL)	0.12	2-4	>483	240	Weight: 20 lb	

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical Max Horz 2=324(LC 6)

Max Uplift3=-153(LC 6), 2=-350(LC 6), 4=-83(LC 4) Max Grav 3=103(LC 1), 2=295(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable eng zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=153

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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Qty Job Truss Truss Type Huber Res. / Roof 14338087 Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:37 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-8 5-0-0 2-0-8 5-0-0 Scale = 1:28.4 8.00 12 3x5 / 1-5-0 W2 **B**1 2x4 || LOADING (psf) SPACING DEFL PLATES GRIP CSI 2-0-0 I/defl L/d 244/190 TC MT20 TCLL 20.0 Plates Increase 1.25 0.47 Vert(LL) -0.03 5-6 >999 360 1.25 BC TCDL 7.0 Lumber Increase 0.16 Vert(TL) -0.05 5-6 >999 240 BCLL 00 Rep Stress Incr YES WR 0.11 Horz(TL) -0.01 n/a nla Code FBC2007/TPI2002 BCDL 5.0 (Matrix) Wind(LL) 0.01 5-6 >999 240 Weight: 28 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 BRACING

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-10-10 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=298/0-5-0, 3=102/Mechanical, 4=24/Mechanical

Max Horz 6=369(LC 6)

Max Uplift6=-197(LC 6), 3=-139(LC 6), 4=-73(LC 6) Max Grav 6=298(LC 1), 3=102(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-6=-274/237 BOT CHORD 5-6=-409/0 WEBS 2-5=0/423

NOTES (8-9)

- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3)\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 6=197, 3=139.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSLTPL1 as referenced by the building code.
- building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

  9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34909

STATE OF

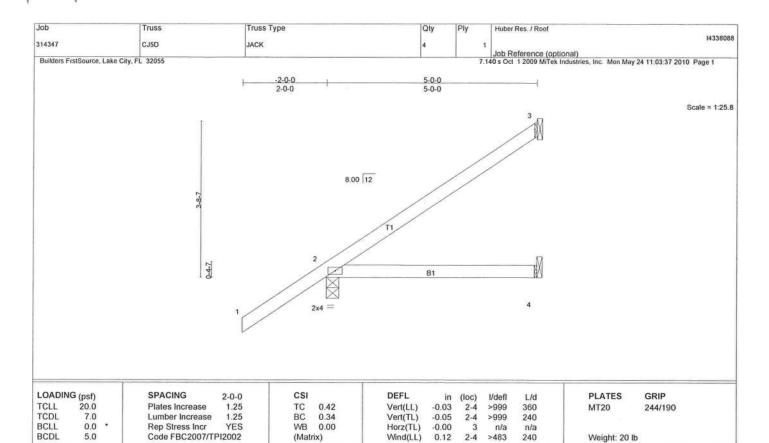
FLORIDA

May

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not first designer. Bracing shown is for lateral support of Individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult. AMSI/TII Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive. Madison, WI 53719.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING -

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

U

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=325(LC 6)

Max Uplift3=-151(LC 6), 2=-351(LC 6), 4=-83(LC 4) Max Grav 3=103(LC 1), 2=295(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jl=lb) 3=151, 2=351.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

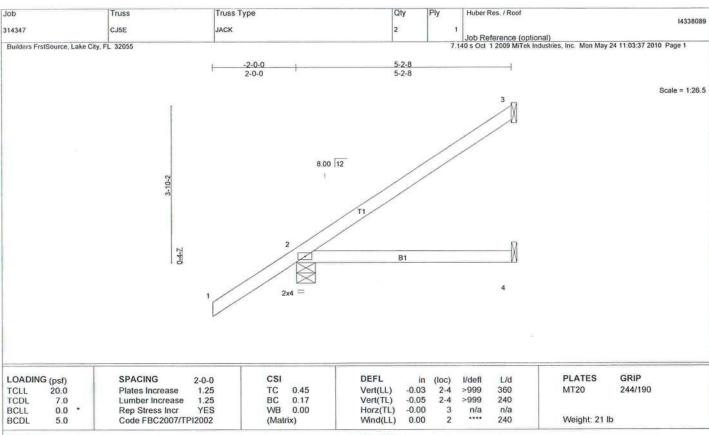
May 24,2010

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitlek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TRI Quality Criteria, DSB-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-2-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

U

D

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

No 34769

REACTIONS (lb/size) 3=105/Mechanical, 2=305/0-5-8, 4=25/Mechanical

Max Horz 2=334(LC 6)

Max Uplift3=-157(LC 6), 2=-276(LC 6)

Max Grav 3=105(LC 1), 2=305(LC 1), 4=74(LC 2)

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=157, 2=276.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

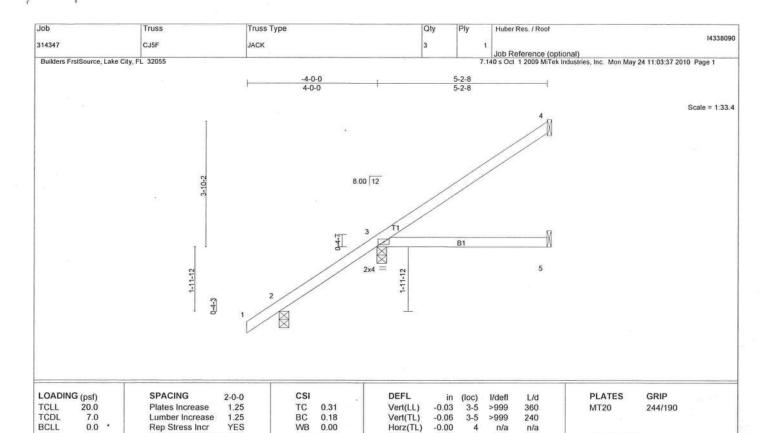
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

May 24,2010

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BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

5.0

BRACING

Wind(LL)

TOP CHORD BOT CHORD

0.00

3

Structural wood sheathing directly applied or 5-2-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

240

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

U

D

Weight: 24 lb

STATE OF

FLORIDA

REACTIONS All bearings Mechanical except (jt=length) 3=0-3-8, 2=0-3-8.

Code FBC2007/TPI2002

(lb) - Max Horz 3=405(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) except 4=-177(LC 6), 3=-112(LC 6), 2=-206(LC 6) Max Grav All reactions 250 lb or less at joint(s) 4, 5, 2 except 3=315(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 —

(Matrix)

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 4, 112 lb uplift at joint 3 and 206 lb uplift at joint 2.

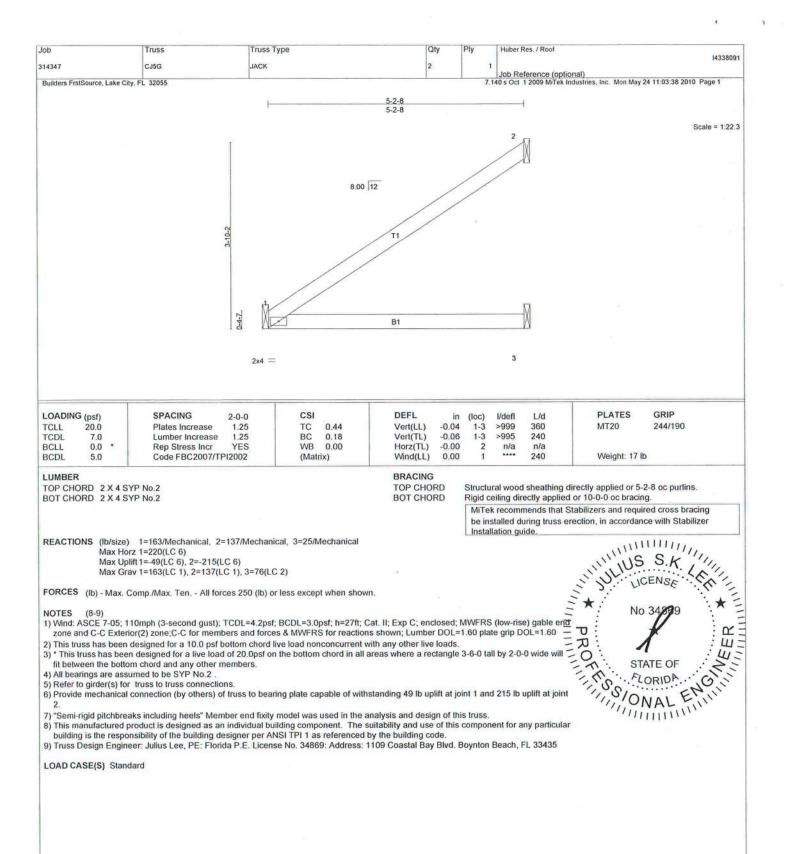
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

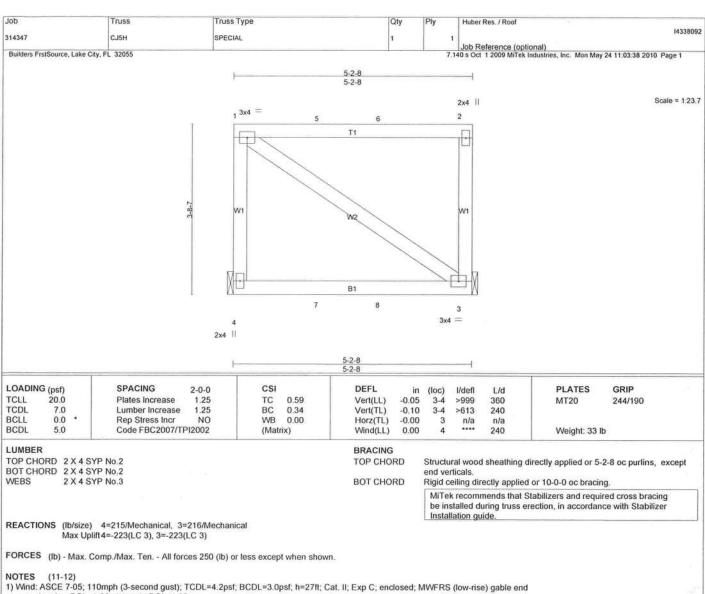
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard





- zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SYP No.2
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 4 and 223 lb uplift at joint 3.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

Concentrated Loads (lb)

Vert: 5=-45(F) 6=-45(F) 7=-14(F) 8=-14(F)

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

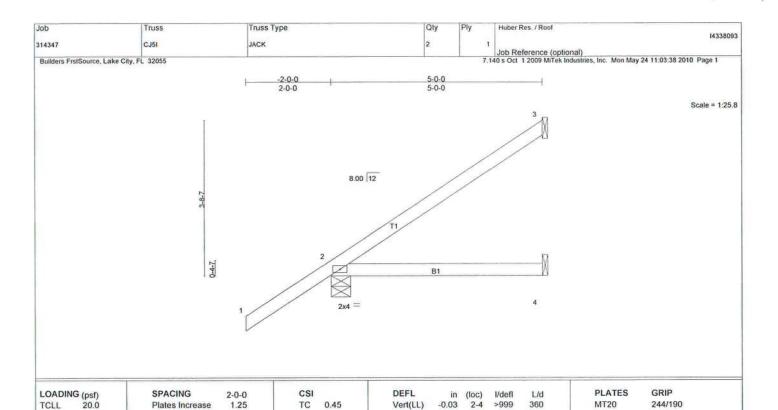
Uniform Loads (plf)

Vert: 1-2=-54, 3-4=-10

Concentrated STATE STATE STATE 10/4

May 24,2010

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. NANNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the source of the effect. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute, S83 D'Onofrio Drive, Madison, WI 53719.



BCDL LUMBER

TCDL

BCLL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

7.0

0.0

5.0

Wind(LL) BRACING

Vert(TL)

Horz(TL)

-0.05

-0.00

0.00

2-4

3

2

>999

n/a

240

n/a

240

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing e with Stabil be installed during truss erection, in accordance with Stabilizer Installation guide.

Weight: 20 lb

REACTIONS (lb/size) 3=99/Mechanical, 2=300/0-5-8, 4=24/Mechanical

Lumber Increase

Code FBC2007/TPI2002

Rep Stress Incr

Max Horz 2=325(LC 6)

Max Uplift3=-146(LC 6), 2=-276(LC 6)

Max Grav 3=99(LC 1), 2=300(LC 1), 4=71(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

BC

WB 0.00

(Matrix)

0.16

1.25

YES

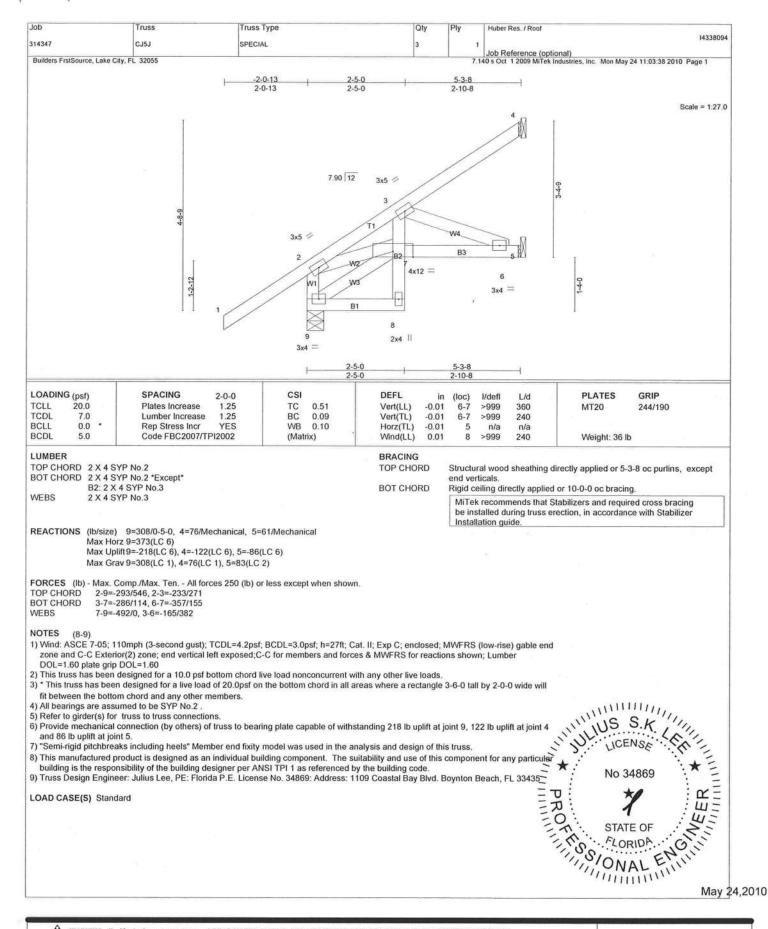
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

NOTES (8-9)

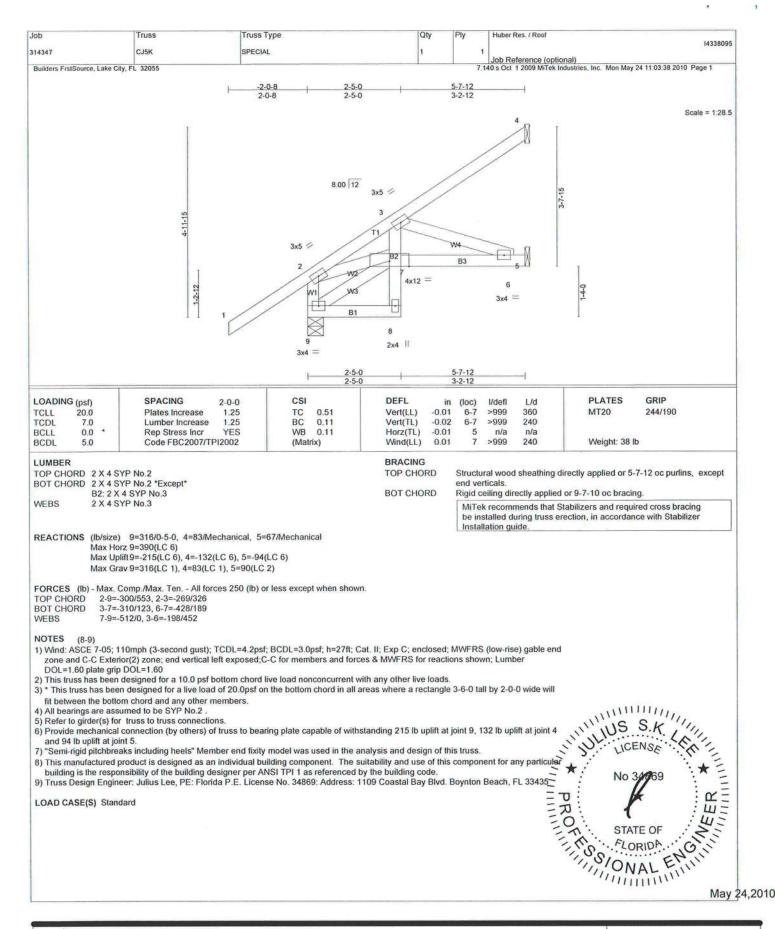
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp. 0, vol. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gup. 2 cone and C-C Exterior(2) zone; C-C for members and cone and co



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

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Job Truss Truss Type Qty Huber Res. / Roof 14338096 314347 CJ5L JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:39 2010 Page 1

Builders FrstSource, Lake City, FL 32055



Scale = 1:46.6

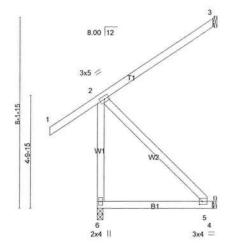


Plate Offsets (X,Y): [5:0-1-8,0-1-8]

LOADING	(psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.03	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.05	5-6	>999	240		
BCLL	0.0	Rep Stress Incr	YES	WB	0.19	Horz(TL)	-0.04	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TP	12002	(Matr	rix)	Wind(LL)	0.15	5-6	>374	240	Weight: 36 lb	EC.

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WERS 2 X 4 SYP No.3

BOT CHORD

BRACING

TOP CHORD

end verticals Rigid ceiling directly applied or 7-2-9 oc bracing.

ס

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Structural wood sheathing directly applied or 5-0-0 oc purlins, except

REACTIONS (lb/size) 6=298/0-3-0, 3=102/Mechanical, 4=24/Mechanical

Max Horz 6=478(LC 6)

Max Uplift6=-153(LC 4), 3=-139(LC 6), 4=-457(LC 6) Max Grav 6=298(LC 1), 3=102(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-6=-274/73 BOT CHORD 5-6=-520/51

WEBS 2-5=-73/738

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 6, 139 lb uplift at joint 3 and 457 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435—

LOAD CASE(S) Standard

3 THINITUS S.K No 34 R. SI. SI. PLORIL. SINEF

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding laborication, quality control, storage, delivery, erection and bracing, consult. AMSI/ITI Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.

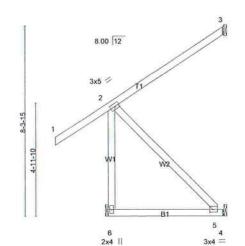


Plate Offsets (X,Y): [5:0-1-9,0-1-8] LOADING (psf) SPACING CSI DEFL PLATES GRIP (loc) I/defl L/d 20.0 Plates Increase 1.25 TC 0.56 Vert(LL) -0.03 5-6 >999 360 MT20 244/190 TCLL TCDL 7.0 Lumber Increase 1.25 BC 0.46 Vert(TL) -0.05 5-6 >999 240 WB BCLL 0.0 Rep Stress Incr YES 0.19 Horz(TL) -0.053 n/a n/a Code FBC2007/TPI2002 Weight: 37 lb (Matrix) Wind(LL) 0.16 5-6 >361 240 BCDL 5.0

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 2 X 4 SYP No.3 BRACING

**BOT CHORD** 

TOP CHORD

Structural wood sheathing directly applied or 5-0-8 oc purlins, except end verticals.

Rigid ceiling directly applied or 7-0-6 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=316/Mechanical, 3=98/Mechanical, 4=24/Mechanical

Max Horz 6=496(LC 6)

Max Uplift6=-159(LC 4), 3=-129(LC 6), 4=-482(LC 6) Max Grav 6=316(LC 1), 3=98(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-6=-292/79 BOT CHORD 5-6=-540/50 WEBS 2-5=-72/775

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 6, 129 lb uplift at joint 3 and 482 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular **x** building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435-

LOAD CASE(S) Standard

icular \* No 34899

No 34899

STATE OF

FLORIDA

May

May 24,2010

Scale = 1:47.5

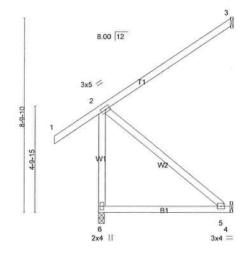
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Job Truss Truss Type Qty Huber Res. / Roof Ply 14338098 314347 CJ5N JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:39 2010 Page 1

Builders FrstSource, Lake City, FL 32055



Scale = 1:49.4



LOADIN	IG (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.51	Vert(LL)	-0.05	5-6	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.09	5-6	>773	240	100000000000000000000000000000000000000	
BCLL	0.0	*	Rep Stress Incr	YES	WB	0.18	Horz(TL)	-0.05	3	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	212002	(Mati	rix)	Wind(LL)	0.26	5-6	>268	240	Weight: 40 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 5-11-8 oc purlins, except

**BOT CHORD** 

Rigid ceiling directly applied or 7-2-13 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=325/0-3-0, 3=133/Mechanical, 4=29/Mechanical

Max Horz 6=518(LC 6)

Max Uplift6=-157(LC 4), 3=-189(LC 6), 4=-438(LC 6) Max Grav 6=325(LC 1), 3=133(LC 1), 4=86(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD BOT CHORD 2-6=-296/59 5-6=-559/55 WEBS 2-5=-71/728

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Bovolog Particular August Particular August Particular STATE OF

May 24,2010

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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of individual two members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult.

ANSI/TPI1 Quality Criteria, DSS-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.

Job Truss Type Qty Huber Res. / Roof 14338099 JACK 314347 CJ5O Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:39 2010 Page 1

Builders FrstSource, Lake City, FL 32055



Scale = 1:47.3

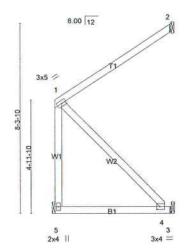


Plate Of	fsets (X,Y)	: [4:0	)-1-9,0-1-8]										
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.38	Vert(LL)	-0.03	4-5	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	4-5	>999	240	0.04000000	
BCLL	0.0	93	Rep Stress Incr	YES	WB	0.08	Horz(TL)	-0.01	2	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	212002	(Matr	ix)	Wind(LL)	0.02	4-5	>999	240	Weight: 33 lb	)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

end verticals Rigid ceiling directly applied or 10-0-0 oc bracing.

**BOT CHORD** 

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

Structural wood sheathing directly applied or 5-0-0 oc purlins, except

REACTIONS (lb/size) 5=153/Mechanical, 2=129/Mechanical, 3=24/Mechanical

Max Horz 5=202(LC 6)

Max Uplift 2=-199(LC 6), 3=-186(LC 6)

Max Grav 5=153(LC 1), 2=129(LC 1), 3=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

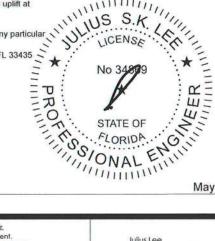
WEBS 1-4=-25/322

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

  8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

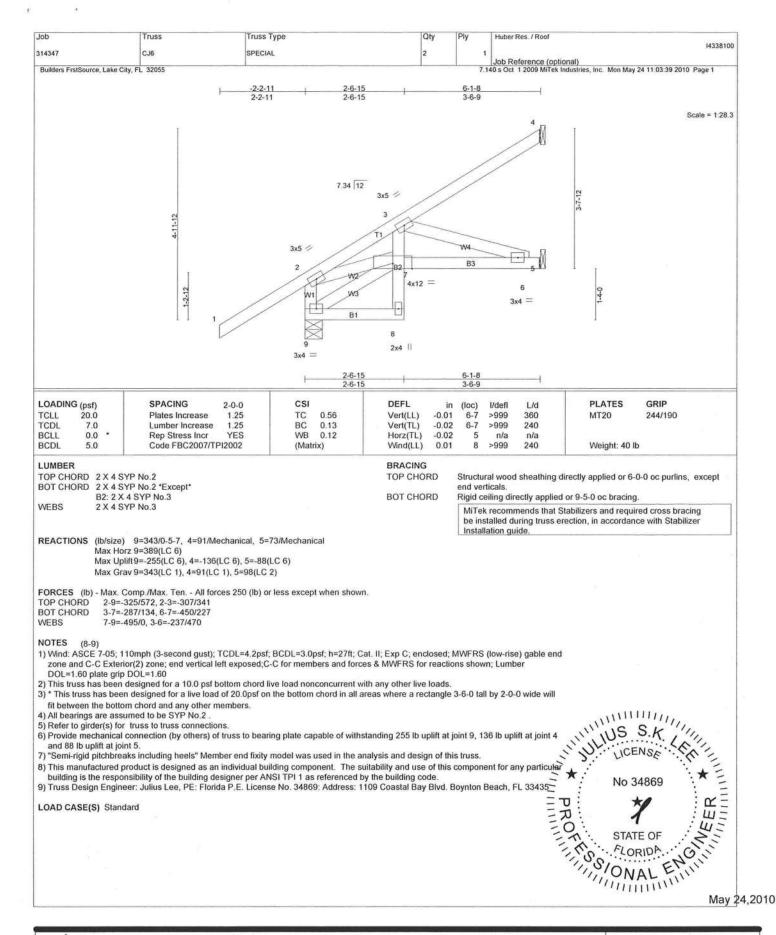
  9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Rlyd. Royalog Book.

LOAD CASE(S) Standard



May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. ■ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 BEFORE USE. Design valid for use only with Milles, connectors. This design is based only upon parameters shown, and is for an individual bullding component. Applicability of design paramenters and proper incorporation of component is responsibility of bullding designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control, storage, delivery, erection and bracing, consult — ANS/IPI Quality Criteria, DSB-89 and BCS11 Building Component Safely Information—available from truss Flate Institute, 583 D'Onofrio Drive. Madison. WI 53719.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-74-73 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flobrication, qualify control, storage, delivery, erection and bracing, consult. ANSI/IPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison. WI 53719.

Job Truss Type Qty Ply Huber Res. / Roof Truss 14338101 EJ6 MONO TRUSS 314347 Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:40 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-8 2-0-8 6-11-8 Scale = 1:34.7 8.00 12 5x6 / W2 B1 2x4 || Plate Offsets (X,Y): [2:0-3-0,0-1-12] LOADING (psf) SPACING CSI DEFL PLATES GRIP 2-0-0 l/deft in (loc) L/d TC 0.65 Vert(LL) >999 MT20 244/190 TCLL Plates Increase -0.08 5-6 360 20.0 1.25 TCDL 7.0 Lumber Increase 1.25 BC 0.27 Vert(TL) -0.145-6 >584 240 BCLL 0.0 Rep Stress Incr WB 0.15 Horz(TL) -0.01 YES n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.02 5-6 >999 240 Weight: 38 lb LUMBER BRACING Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD 2 X 4 SYP No.2 end verticals 2 X 4 SYP No.2 \*Except\* **BOT CHORD** Rigid ceiling directly applied or 8-3-9 oc bracing. WEBS W2: 2 X 4 SYP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 3=143/Mechanical, 6=353/0-5-0, 4=54/Mechanical Max Horz 6=337(LC 6) Max Uplift3=-144(LC 6), 6=-143(LC 6), 4=-46(LC 6) Max Grav 3=143(LC 1), 6=353(LC 1), 4=107(LC 2) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-6=-314/257 **BOT CHORD** 5-6=-585/143 WEBS 2-5=-146/595 NOTES 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

- fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 143 lb uplift at joint 6 and 46 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular \* building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

  9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435—

LOAD CASE(S) Standard

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May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not frus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality controls storage, delivery, erection and bracing, consult. AMSI/THI Quality Criteria, DSS-89 and BCS11 Building Component Safety Information.

Job Truss Truss Type Qty Huber Res. / Roof 12 314347 F.J7 MONO TRUSS Job Reference (optional) Builders FrstSource, Lake City, FL 32055 7.140 s Jun 24 2009 MiTek Industries, Inc. Mon May 24 14:50:40 2010 Page 1 7-0-0 -2-0-0 2-0-0 7-0-0 Scale = 1 31 5 0-4-11

Plate Off	ets (X,)	(): [2	2:0-1-8,0-1-8]										
LOADING	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defi	L/d	PLATES	GRIP
TCLL	20.0	- 1	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.08	2-4	>948	360	MT20	244/190
TCDL	7.0	- 1	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.16	2-4	>491	240	10-00000000	
BCLL	0.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	- 1	Code FBC2007/TF	212002	(Matr	ix)	Wind(LL)	0.42	2-4	>193	240	Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

(lb/size) 3=154/Mechanical, 2=355/0-5-8, 4=41/Mechanical

Max Horz 2=294(LC 6)

Max Uplift 3=-162(LC 6), 2=-309(LC 6), 4=-98(LC 4) Max Grav 3=154(LC 1), 2=355(LC 1), 4=95(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

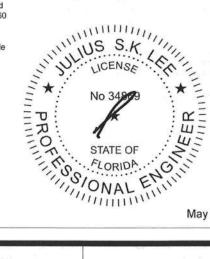
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 3, 309 lb uplift at joint 2 and 98 lb uplift at joint 4.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

  7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL

LOAD CASE(S) Standard



May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control storage, delivery, erection and bracing, consult. ANS/IPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate institute, 583 D'Onotrio Drive, Madison, WI 53719.

Job Truss Truss Type Qty 14338103 EJ7A MONO TRUSS 314347 Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:40 2010 Page 1 Builders FrstSource, Lake City, FL 32055 7-0-0 7-0-0 2-0-0 2-0-0 Scale: 3/8"=1" 8.00 12 Plate Offsets (X,Y): [2:0-2-4,0-2-0] DEFL **PLATES** GRIP LOADING (psf) SPACING I/defl L/d 2-0-0 (loc) 360 MT20 244/190 Plates Increase 1.25 TC 0.64 Vert(LL) -0.01 >999 TCLL 20.0 1.25 BC 0.25 Vert(TL) -0.02 4-5 >999 240 TCDL 7.0 Lumber Increase 0.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 n/a n/a BCLL Weight: 27 lb BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.01 2-5 >999 240 BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD

**BOT CHORD** 

REACTIONS All bearings Mechanical except (jt=length) 2=0-3-8, 5=0-9-4.

(lb) - Max Horz 2=294(LC 6)

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

Max Uplift All uplift 100 lb or less at joint(s) 5 except 3=-151(LC 6), 2=-207(LC 6)

Max Grav All reactions 250 lb or less at joint(s) 3, 4, 5 except 2=266(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 3=151

"Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular

building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

nticular NO 34999

ROSTATE OF FLORIDA SINEF

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

May 24,2010

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1109 Coastal Bay Blvd. Boynton, FL 33435

Job Truss Truss Type Qty Ply Huber Res. / Roof 314347 EJ7C MONO TRUSS 10 Job Reference (optional) Builders FrstSource, Lake City, FL 32055 7.140 s Jun 24 2009 MiTek Industries, Inc. Mon May 24 14:53:12 2010 Page 1 2-0-0 7-0-0 7-0-0 2-0-0 Scale = 1:31.5 0-4-7

100111-0011	sets (X,	T	:0-1-15,0-1-8]										
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.61	Vert(LL)	-0.09	2-4	>914	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.17	2-4	>471	240	5.30 PM PMSER	
BCLL	0.0		Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2007/TF	212002	(Matr	ix)	Wind(LL)	0.44	2-4	>185	240	Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=157/Mechanical, 2=352/0-3-8, 4=42/Mechanical

Max Horz 2=295(LC 6)

Max Uplift 3=-163(LC 6), 2=-306(LC 6), 4=-99(LC 4) Max Grav 3=157(LC 1), 2=352(LC 1), 4=96(LC 2)

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 3, 306 lb uplift at joint 2 and 99 lb uplift at joint 4.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any
  particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
   Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL

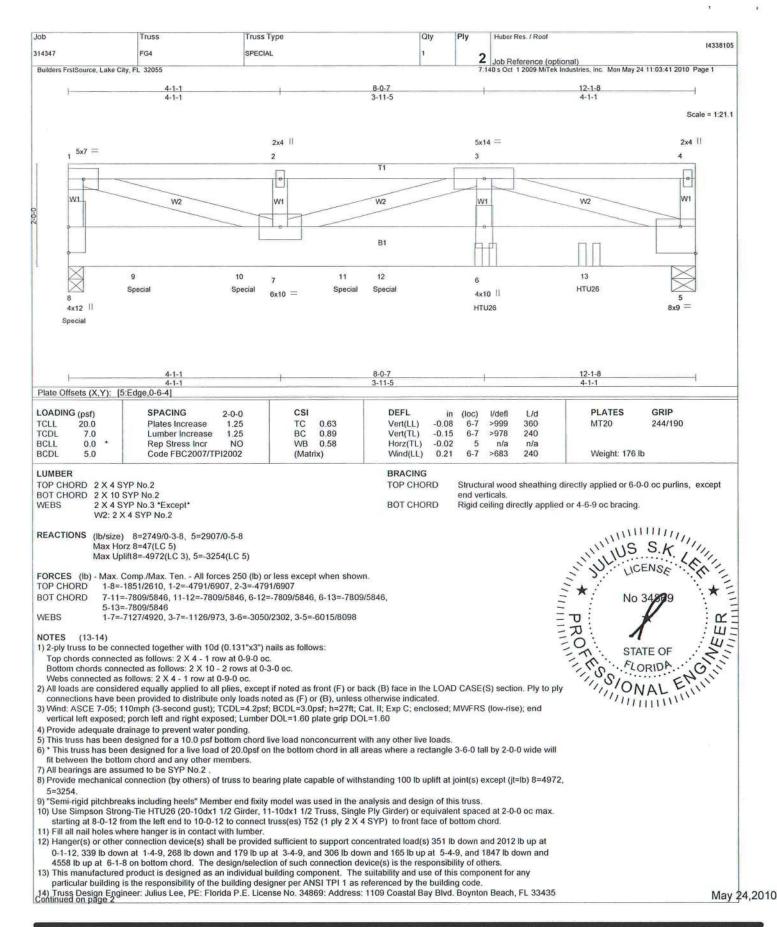
LOAD CASE(S) Standard



May 24,2010

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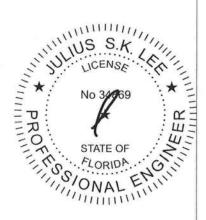
ANSI/ITB Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, Wi 53719.

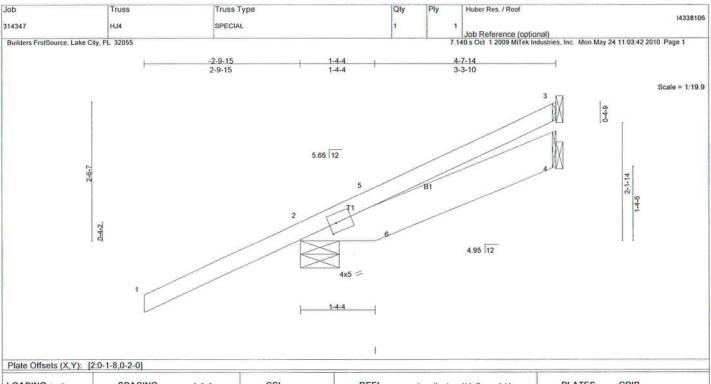


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b	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	143381
1347	FG4	SPECIAL	1	2	Job Reference (ontional)	
ilders FrstSource,	Lake City, FL 32055			7.	Job Reference (optional) 140 s Oct 1 2009 MiTek Industries, Inc. Mon May	24 11:03:41 2010 Page 2
	PARAGONICA PENERSON CON					
AD CASE(S)	Standard per Increase=1.25, Plate Inc	croase=1 25				
Uniform Loads	(plf)	Stedde-1.20				
Vert: 1 Concentrated	1-4=-54, 5-8=-10					
Vert: 8	3=-351(F) 6=-857(F) 9=-339	9(F) 10=-268(F) 11=-306(F) 12=-1847(F	) 13=-930(F)			
			AND CONTRACTOR OF THE SECOND			





LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.39	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
CDL	7.0		Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.00	2-4	>999	240		
BCLL	0.0		Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	- 1	Code FBC2007/TF	212002	(Matr	nix)	Wind(LL)	0.00	2-4	>999	240	Weight: 28 lb	)

LUMBER

TOP CHORD 2 X 4 SYP M 31 BOT CHORD 2 X 8 SYP No.1D BRACING

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 4-7-14 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=32/Mechanical, 2=308/0-8-8, 4=19/Mechanical

Max Horz 2=248(LC 5)

Max Uplift 3=-83(LC 6), 2=-485(LC 5)

Max Grav 3=32(LC 1), 2=308(LC 1), 4=55(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (11-12)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=485

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 37 lb up at 1-5-12, and 37 lb up at 1-5-12 on top chord, and 23 lb up at 1-5-12, and 23 lb up at 1-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

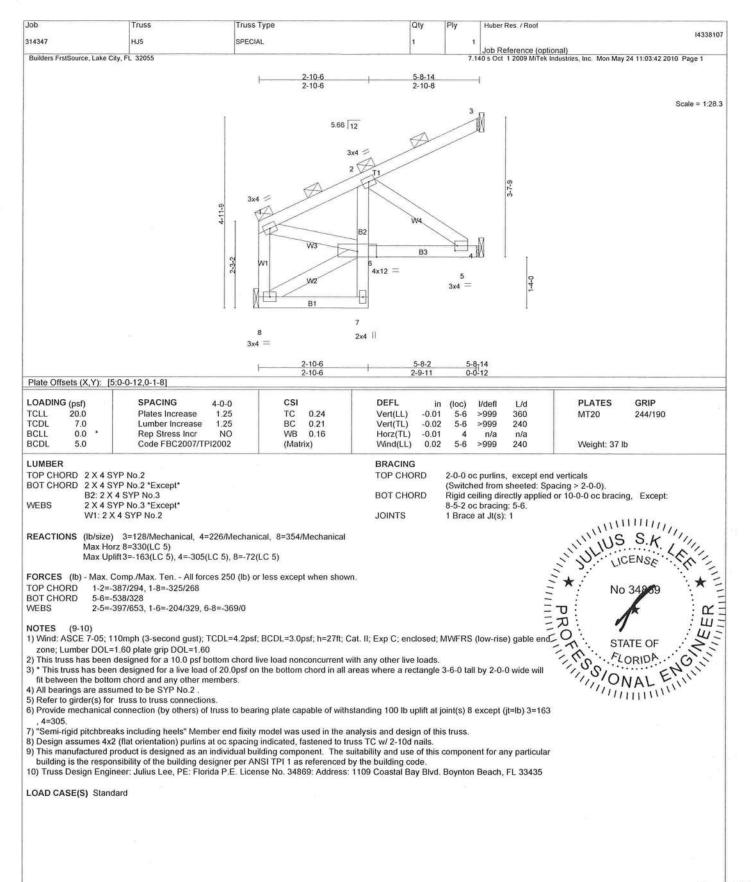
Vert. 1-3=-54, 2-4=-10

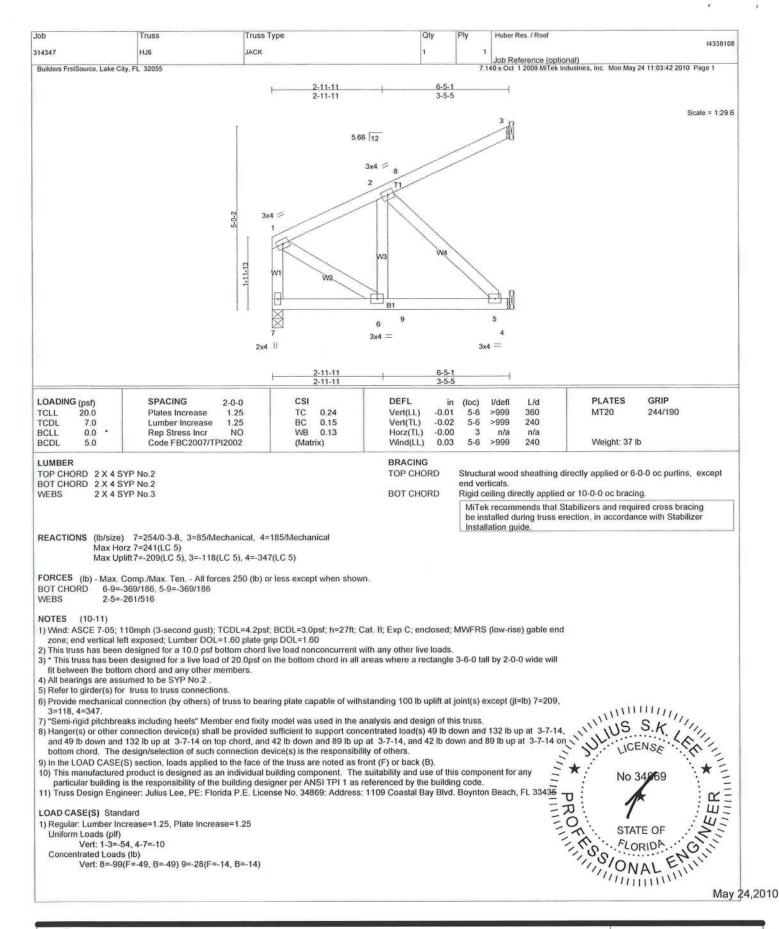
Concentrated Loads (lb) Vert: 5=75(F=37, B=37) 6=10(F=5, B=5)

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May 24,2010

■ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown for lateral support of individual web members only. Additional temporary bracing to inverse stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/IPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information. available from fruss Plate Institute, 583 D'Onofrio Drive, Medison, WI 53719.





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Job Truss Truss Type Qty Huber Res. / Roof 14338109 нив 314347 MONO TRUSS Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:42 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -3-2-14 3-2-14 8-0-2 4-2-14 Scale = 1:49.1 13 5.66 12 4x5 < 11 3 9 10 4x6 = 4-11-15 7 16 14 6 18 8 4x10 8x9 3x4 = Plate Offsets (X,Y): [2:0-2-15,0-2-0], [6:0-2-2,0-4-0] LOADING (psf) SPACING CSI DEFL PLATES GRIP 2-0-0 (loc) I/defl L/d TCLL 20.0 0.39 Plates Increase 1.25 TC Vert(LL) -0.00 6-7 >999 360 MT20 244/190 BC TCDL 7.0 Lumber Increase 1.25 0.36 -0.01 Vert(TL) 6-7 >999 240 0.0 BCLL Rep Stress Incr WB 0.94 -0.01 Horz(TL) n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.05 6-7 >999 240 Weight: 82 lb LUMBER BRACING TOP CHORD 2 X 6 SYP No.1D TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.3 \*Except\* WEBS BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. W1: 2 X 4 SYP No.2 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 4=171/Mechanical, 8=361/Mechanical, 6=102/Mechanical Max Horz 8=520(LC 5) Max Uplift 4=-248(LC 5), 8=-2006(LC 3), 6=-1945(LC 5) Max Grav 4=171(LC 1), 8=361(LC 1), 6=195(LC 2) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-9=-87/849, 9-10=-84/791, 3-10=-99/750, 2-8=-362/1448 TOP CHORD 8-14=-447/0, 14-15=-447/0, 7-15=-447/0, 7-16=-819/51, 16-17=-819/51, 17-18=-819/51, **BOT CHORD** 6-18=-819/51 WEBS 3-7=-1434/109, 3-6=-93/1497, 2-7=-910/88 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will 6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=248.

8=2006, 6=1945.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this location of this location device(s) shall be provided sufficient to support the support of the location of this location device (s) shall be provided sufficient to support the support of the location of this location device (s) shall be provided sufficient to support the support of the location of this location device (s) shall be provided sufficient to support the support of the location of this location of the location of the location device (s) shall be provided sufficient to support the support of the location of the location of the location of the location device (s) shall be provided sufficient to support the location of the location device (s) shall be provided sufficient to support the location of the location 1-4-15, 14 lb down and 25 lb up at 3-10-11, 23 lb down and 36 lb up at 4-2-15, and 48 lb down and 120 lb up at 6-8-10, and 44 lb dewn and 110 lb up at 7-0-14 on top chord, and 417 lb up at 1-0-12, 471 lb up at 1-4-15, 12 lb down and 540 lb up at 3-10-11, 12 lb down Ш D and 588 lb up at 4-2-15, and 42 lb down and 437 lb up at 6-8-10, and 42 lb down and 462 lb up at 7-0-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 13) Use Simpson SUR\L210 to attach Truss to Carrying member JT8 May 24,2010 COAD CASE (S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE FAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

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AMSI/TRI Quality Critical, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.

	Truss	Truss Type	Qty	Ply	Huber Res. / Roof		14338109
	HJ8	MONO TRUSS	1	1	Job Reference (	optional)	
AD CASE(S) Standard Regular: Lumber Increas Jniform Loads (pif) Vert: 1-2=-54, 2 Concentrated Loads (lb)	se=1.25, Plate Increase=1.2			7.1		(optional) Tek Industries, Inc. Mon May 24 11:03:42	2010 Page 2
						No 34869  TO STATE OF LORIDA  STATE OF LORIDA	NEER * WATER
						A CONAL	EMILITY

Joh Truss Truss Type Huber Res. / Roof 14338110 314347 HJ8A MONO TRUSS Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:43 2010 Page 1 Builders FrstSource, Lake City, FL 32055 4-0-13 4-0-13 8-1-9 4-0-12 Scale = 1:49 1 5.66 12 3x4 = 3x6 = 8-9-4 4-11-4 5 6 2x4 || 6x8 3x4 = 4-0-13 Plate Offsets (X,Y): [5:0-2-0,0-3-0] LOADING (psf) SPACING CSI DEFL PLATES 2-0-0 GRIP in (loc) I/defl L/d TCLL 20.0 TC Plates Increase 1.25 0.28 Vert(LL) -0.00 5-6 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.06 -0.01 Vert(TL) 5-6 >999 240 0.0 BCLL Rep Stress Incr WB 0.19 NO Horz(TL) -0.01 n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) 0.01 Wind(LL) 5-6 >999 Weight: 67 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.3 \*Except\* WEBS BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. W1: 2 X 4 SYP No.2 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 3=156/Mechanical, 7=172/Mechanical, 4=187/Mechanical Max Horz 7=241(LC 5) Max Uplift3=-222(LC 5), 4=-280(LC 5) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-5=-201/381 NOTES (10-12)1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SYP No.2 5) Refer to girder(s) for truss to truss connections. particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Trapezoidal Loads (plf) 6) Refer to girder(s) for truss to truss connections. Vert: 1=0(F=27, B=27)-to-3=-110(F=-28, B=-28), 7=0(F=5, B=5)-to-4=-20(F=-5, B=-5)

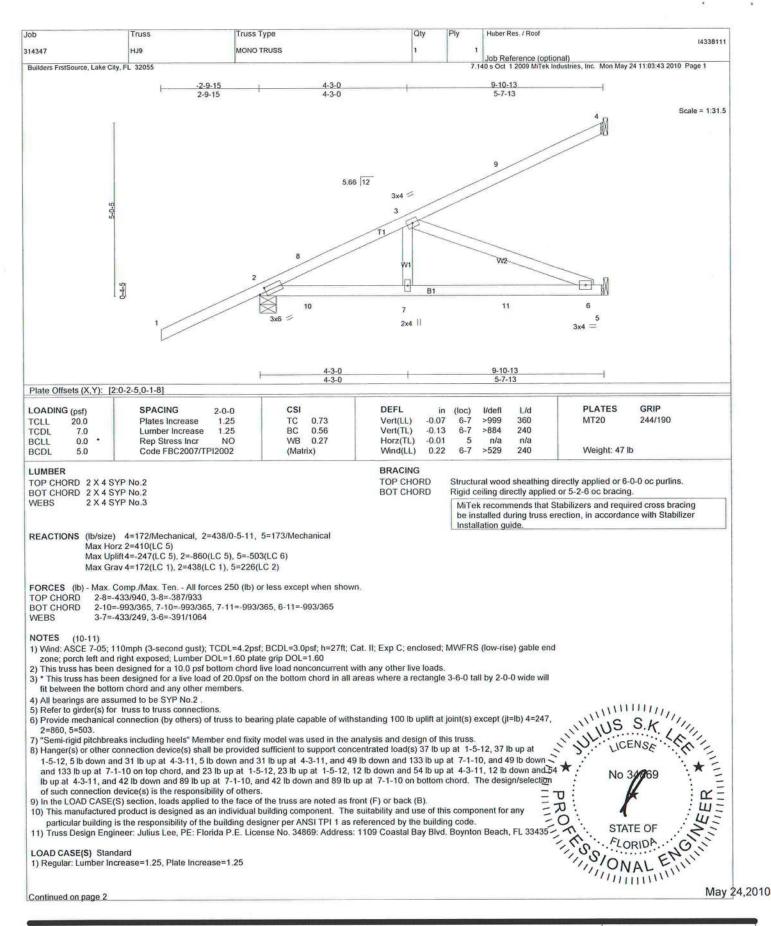
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE FAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult.

ANSI/ITI Qualify Critical, DSS-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435



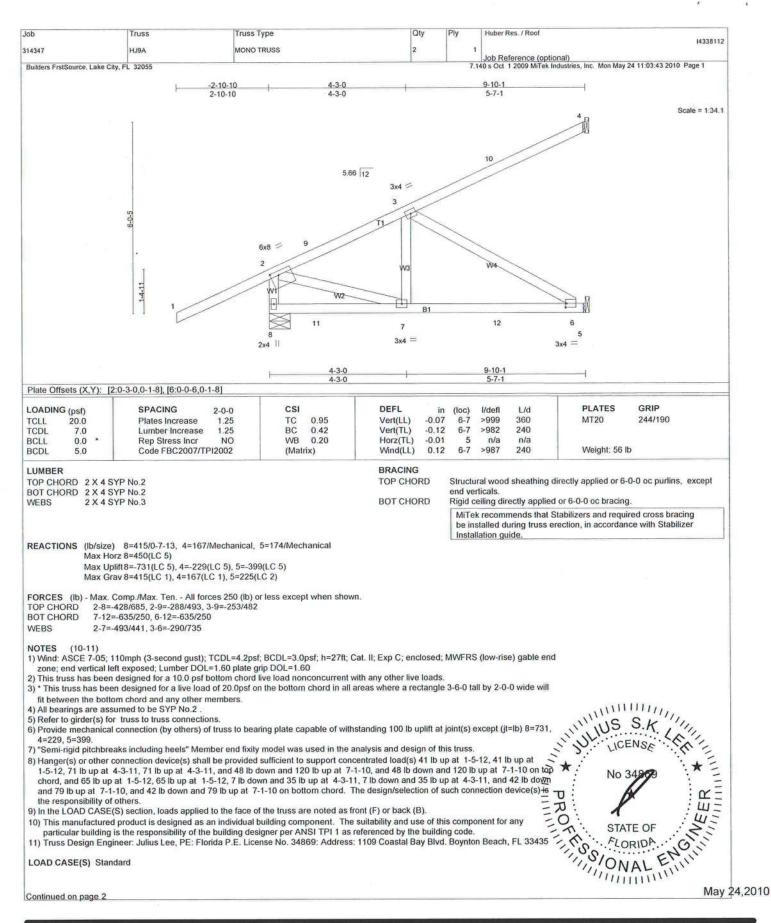
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b	Truss	Truss Type	Qty	Ply	Huber Res. / Roof		
4347	нла	MONO TRUSS	1		1 Joh Deference (cotional)		143381
ilders FrstSource, L	ake City, FL 32055			7	Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries	s, Inc. Mon May 24 11:03:43 2010	Page 2
	20 6 10						
AD CASE(S) : Iniform Loads							
	-4=-54, 2-5=-10						
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)						
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	e-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- oncentrated L	-4=-54, 2-5=-10 oads (lb)	4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		
Vert: 1- concentrated L	-4=-54, 2-5=-10 oads (lb)	F-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)		





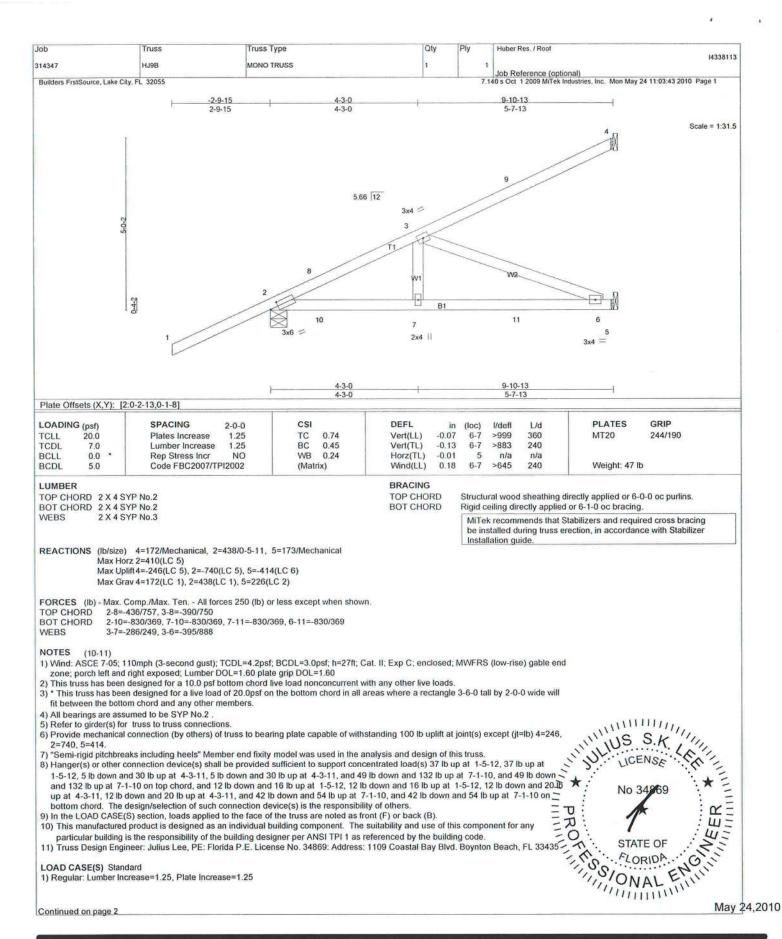
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

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AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

)	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	
347	нј9А	MONO TRUSS	2	1		1433811
ilders FrstSource, Lake City,	FL 32055			7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:	03:43 2010 Page 2
Jniform Loads (plf) Vert: 1-2=-54, Concentrated Loads (I	ase=1.25, Plate Increase 2-4=-54, 5-8=-10 b)	e=1.25 5) 9=81(F=41, B=41) 10=-97(F=-48, B	i=-48) 11=26(F=13, B=	=13) 12=-2	28(F=-14, B=-14)	
					NO 348	K. LENI
					No 348	369 ★ C
					No 348  PRO STATE  FLORIT	OF A
					777 ONA	THINK'S

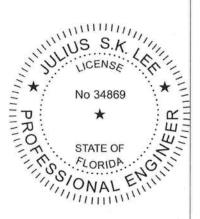


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

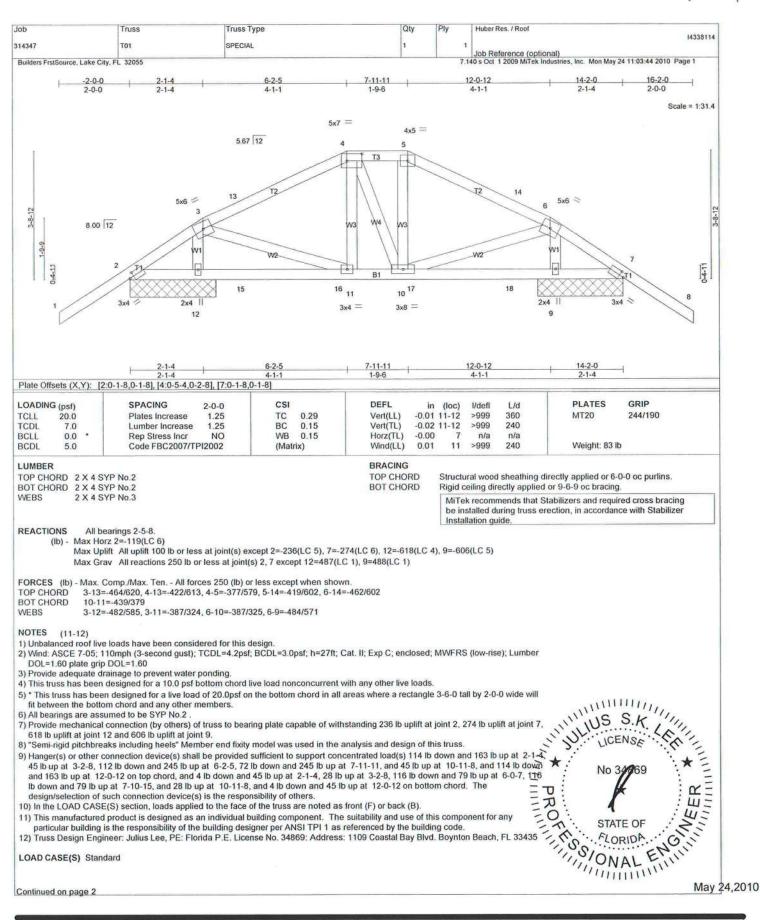
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Job	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	1100011
314347	нуэв	MONO TRUSS	1	1	1 Joh Beforense (estimativ	1433811
Builders FrstSource	Lake City, FL 32055			7	Job Reference (optional) 140 s Oct 1 2009 MiTek Industries, Inc. Mon May	y 24 11:03:43 2010 Page 2
LOAD CASE(S) Uniform Load						
Official Load						
Vert:	1-4=-54, 2-5=-10					
Vert: Concentrated	Loads (lb)	-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	3(F=-14, B=-14)	
Vert: Concentrated	Loads (lb)	-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	3(F=-14, B=-14)	
Vert: Concentrated	Loads (lb)	-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	B(F=-14, B=-14)	
Vert: Concentrated	Loads (lb)	-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-2	8(F=-14, B=-14)	
Vert: Concentrated	Loads (lb)	-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B=	=5) 11=-20	8(F=-14, B=-14)	
Vert: Concentrated	Loads (lb)	-4, B=-4) 8=75(F=37, B=37) 9=-99(F=-49	, B=-49) 10=10(F=5, B	=5) 11=-20	8(F=-14, B=-14)	







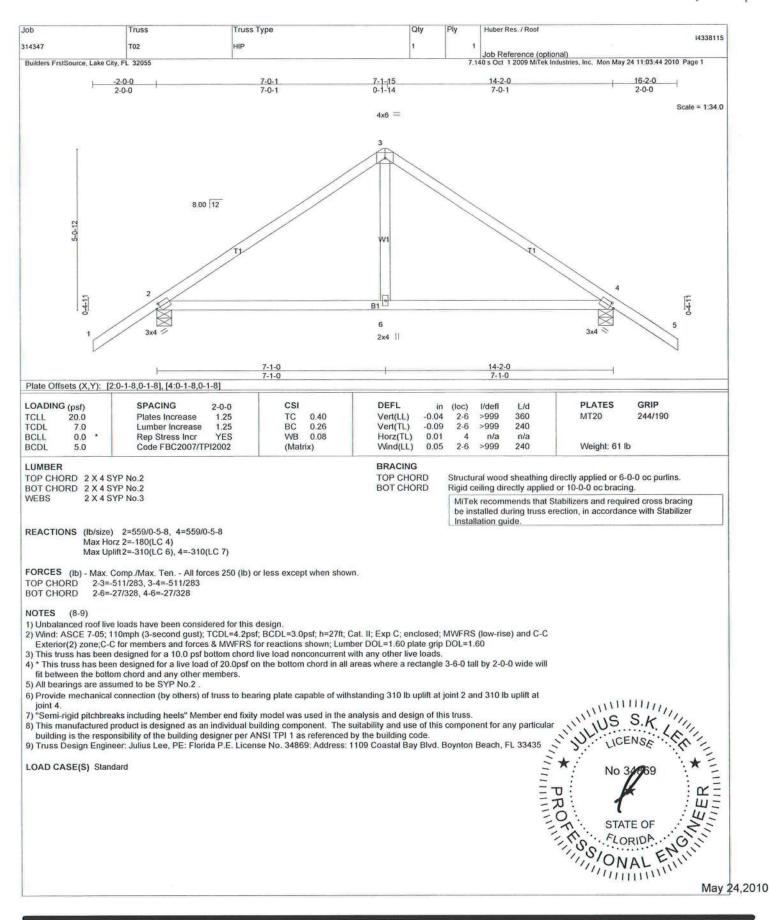
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AMSI/TIP Quality Citeria, DSS-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.

	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	
347	TOI	SPECIAL	1	1		1433811
ders FrstSource, L	Lake City, FL 32055			7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon M	ay 24 11:03:44 2010 Page 2
niform Loads Vert: 1 oncentrated t	er Increase=1.25, Plate Incr (plf) -3=-54, 3-4=-54, 4-5=-54, 5- Loads (lb)		3=16(F) 14=16(F) 15=17(	F) 16=-39(	(F) 17=-10(F) 18=17(F)	
						6
						S S.K
					PRO	TATE OF CORIDA CONTINUITY
					11/08/0	NAL ENTIN

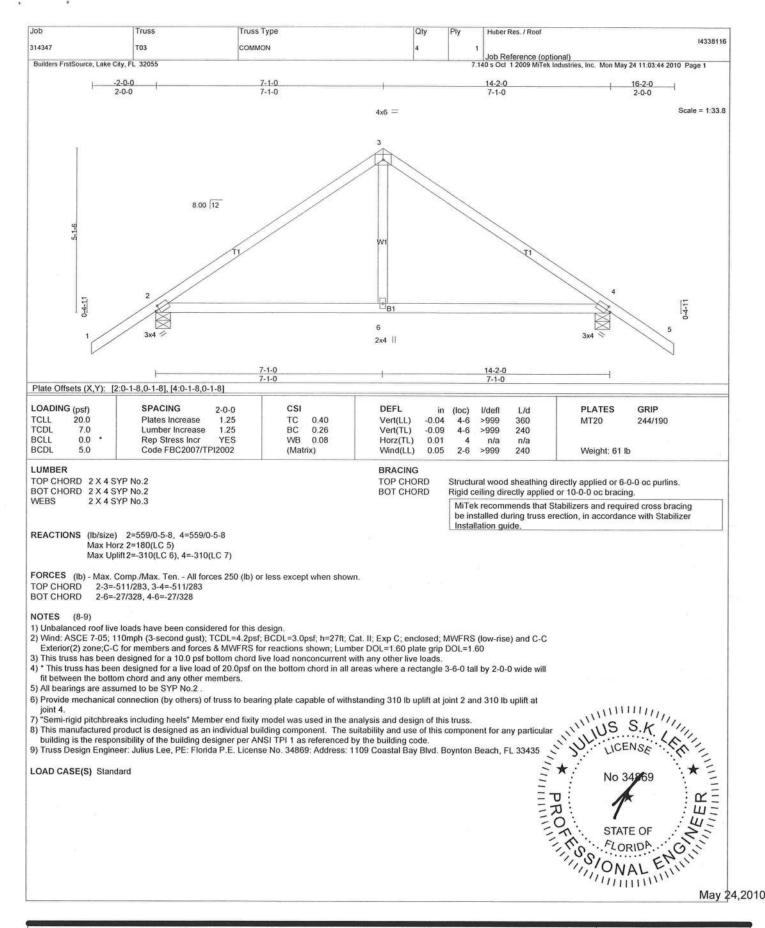


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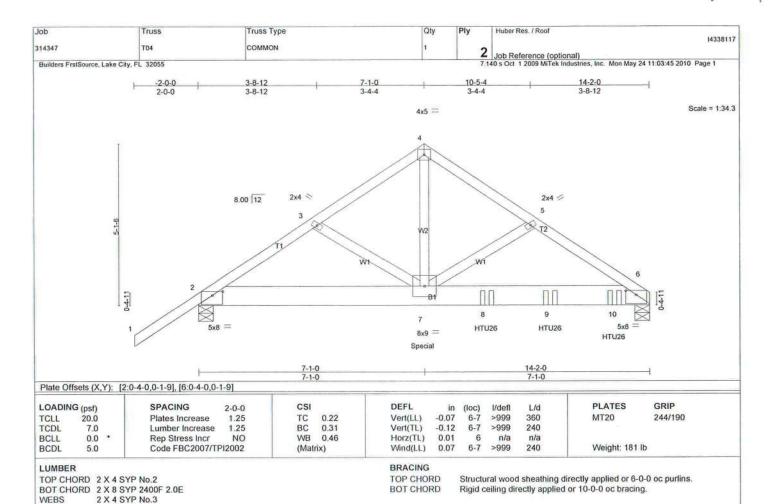
Applicability of design parameters and proper incorporation of component is responsibility of building designer not fluss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control, storage, delivery, erection and bracing, consult.

AMSI/TRI ADSI-89 and BCS11 Building Component available from Truss Plate Institute, 583 D'Onofrio Drive, Madison. WI 53719.



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REACTIONS (lb/size) 6=2975/0-5-8, 2=1802/0-5-8

Max Horz 2=213(LC 4)

Max Uplift6=-1712(LC 6), 2=-1364(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-2962/2098, 3-4=-2819/2101, 4-5=-2839/2087, 5-6=-2999/2134 2-7=-1721/2380, 7-8=-1713/2463, 8-9=-1713/2463, 9-10=-1713/2463, 6-10=-1713/2463 **BOT CHORD** 

WEBS 4-7=-2215/2894

## NOTES (13-14)

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) All bearings are assumed to be SYP No.2

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1712 lb uplift at joint 6 and 1364 lb uplift at joint 2.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 9-0-12 from the left end to 13-0-12 to connect truss(es) T06 (1 ply 2 X 4 SYP) to back face of bottom chord.

11) Fill all nail holes where hanger is in contact with lumber.

12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1469 lb down and 1714 lb up at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Flonda P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2

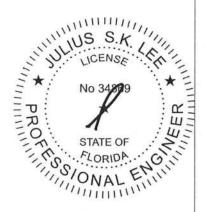
PROPINE STATE
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ONAL MOIN

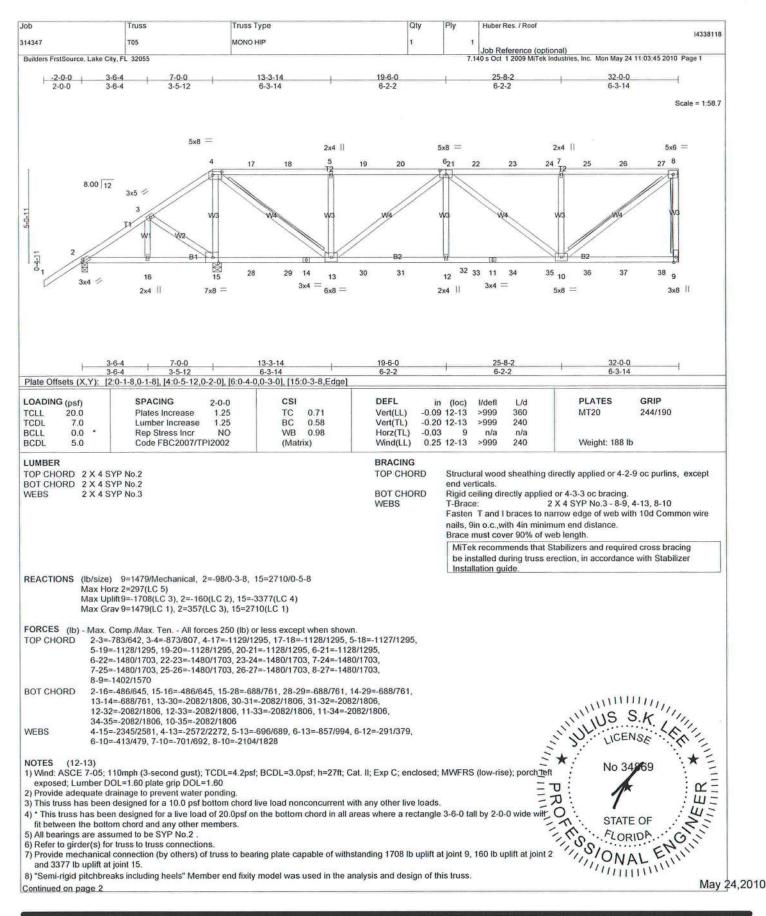
May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control, storage, delivery, erection and bracing, consult. AMS/ITIN Design Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply		Huber Res. / Roof	
314347	T04	COMMON	1		2	Job Reference (optional)	14338117
Builders FrstSource, L  LOAD CASE(S)	No. 2010 10 10 10 10 10 10 10 10 10 10 10 10			,	7.14	0 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:45	2010 Page 2
	er Increase=1.25, Plate In	crease=1 25					
Uniform Loads		1.20					
Vert: 1-	4=-54, 4-6=-54, 2-6=-10						
Concentrated L	anda (Ih)						





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AMSI/TRUS ADSB-89 and BCSI1 Building Component available from Truss Plate Institute, 583 D'Onofro Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	Huber Res. / Roof		
314347	T05	MONO HIP	1	1	Name and the same and	4338118	
	155000	000000000000000000000000000000000000000		100	Job Reference (optional)		
Builders EretSource Lake City E	22055			7 14	Or Oct 1 2000 MiTch Industries Inc. Man Hou 24 11/02/45 2010 Dane		

- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 217 lb down and 370 lb up at 7-0-0, 92 lb down and 131 lb up at 9-0-12, 100 lb down and 142 lb up at 11-0-12, 100 lb down and 142 lb up at 13-0-12, 100 lb down and 142 lb up at 15-0-12, 100 lb down and 142 lb up at 17-0-12 up at 19-0-12, 100 lb down and 142 lb up at 21-0-12, 100 lb down and 142 lb up at 25-0-12, 100 lb down and 142 lb up at 25-0-12, 100 lb down and 142 lb up at 25-0-12, 100 lb down and 142 lb up at 27-0-12, and 100 lb down and 142 lb up at 29-0-12, and 100 lb down and 142 lb up at 31-0-12 on top chord, and 261 lb down and 613 lb up at 7-0-0, 26 lb down at 9-0-12, 65 lb down and 104 lb up at 11-0-12, 65 lb down and 104 lb up at 13-0-12, 65 lb down and 104 lb up at 15-0-12, 65 lb down and 104 lb up at 17-0-12, 65 lb down and 104 lb up at 19-0-12, 65 lb down and 104 lb up at 21-0-12, 65 lb down and 104 lb up at 23-0-12, 65 lb down and 104 lb up at 25-0-12, 65 lb down and 104 lb up at 27-0-12, and 65 lb down and 104 lb up at 29-0-12, and 65 lb down and 104 lb up at 31-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

  13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

## LOAD CASE(S) Standard

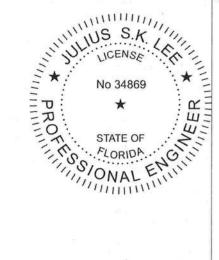
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

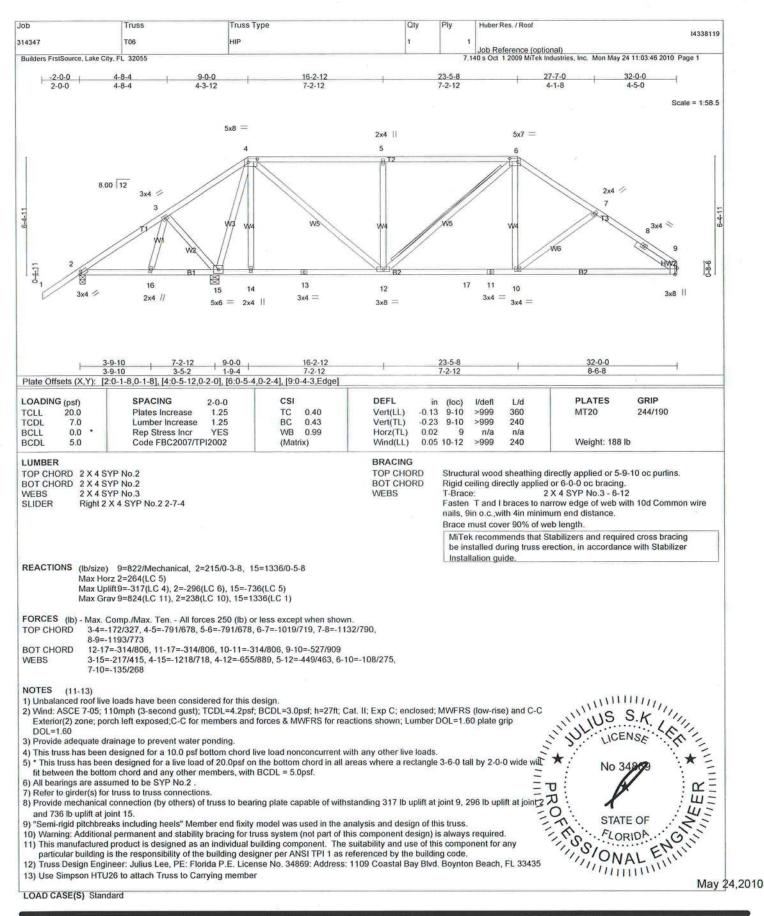
Uniform Loads (plf)

Vert: 1-4=-54, 4-8=-54, 2-9=-10

Concentrated Loads (lb)

Vert: 4=-217(F) 15=-194(F) 13=-31(F) 5=-100(F) 17=-92(F) 18=-100(F) 19=-100(F) 20=-100(F) 21=-100(F) 22=-100(F) 23=-100(F) 24=-100(F) 25=-100(F) 26=-100(F) 27=-100(F) 28=-5(F) 29=-31(F) 30=-31(F) 31=-31(F) 32=-31(F) 33=-31(F) 34=-31(F) 35=-31(F) 36=-31(F) 37=-31(F) 38=-31(F)

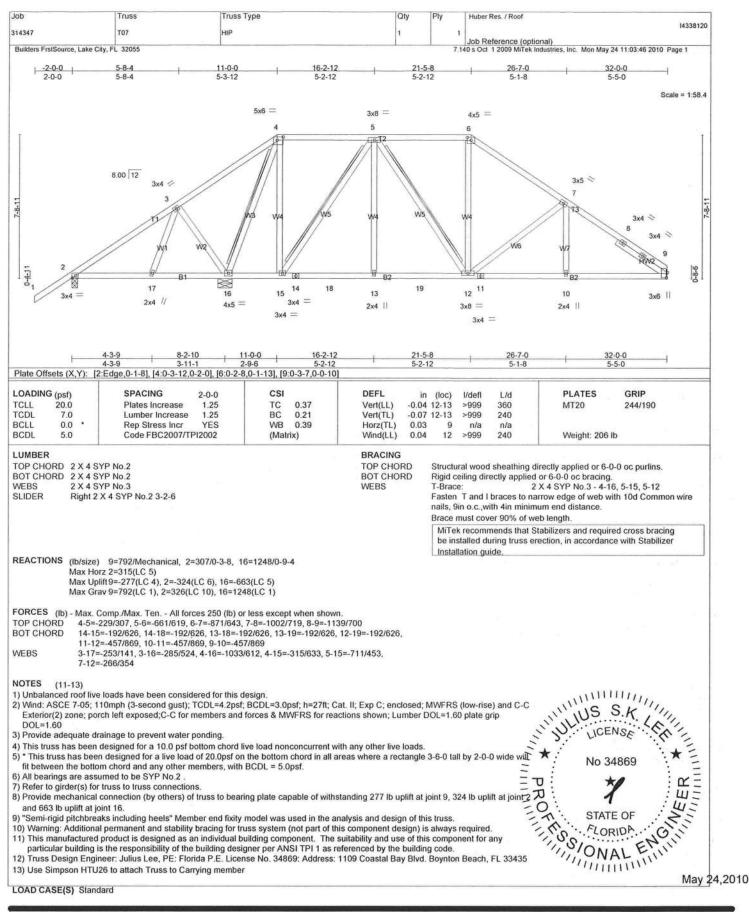




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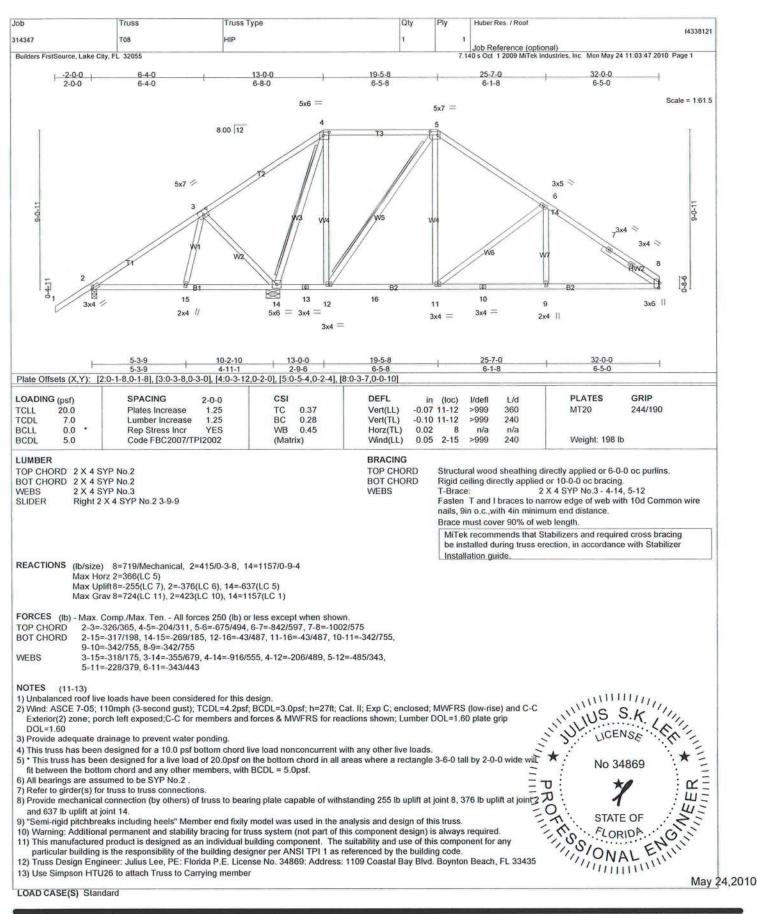
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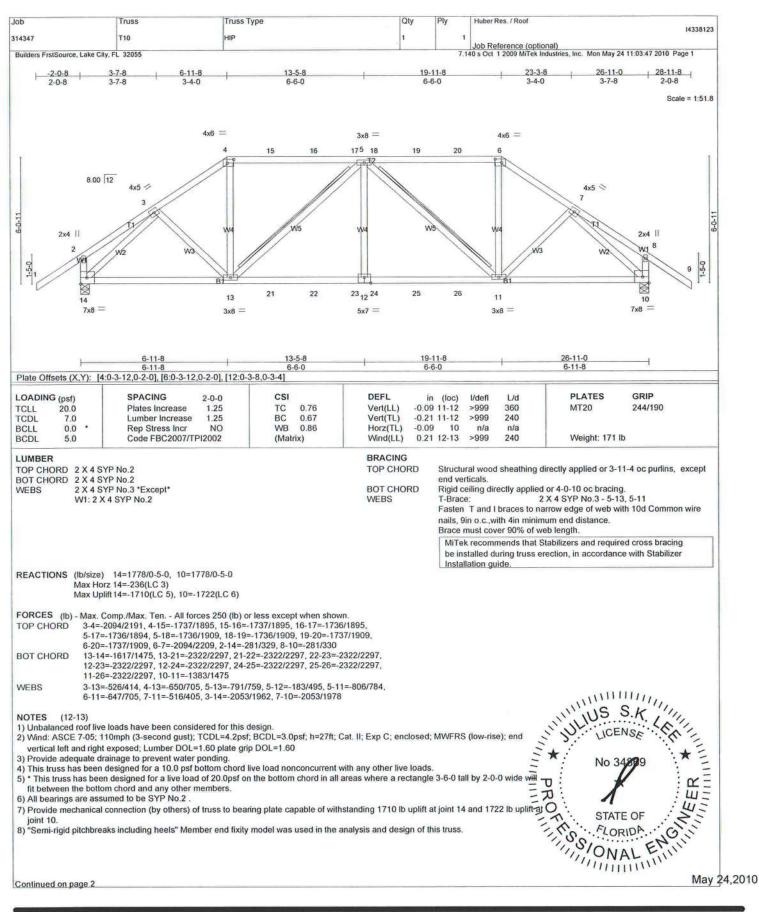
Job Truss Truss Type Qty Huber Res. / Roof 14338122 314347 T09 MONO TRUSS Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:47 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 6-0-8 12-0-0 2-0-0 Scale = 1:50.0 2x4 || 8.00 12 3x5 🗸 0-4-11 6 3x4 / 3x5 = 2x4 || Plate Offsets (X,Y): [2:0-1-8,0-1-8] LOADING (psf) SPACING CSI 2-0-0 DEFL PLATES GRIP I/defl (loc) L/d TCLL 20.0 Plates Increase 1.25 TC 0.39 Vert(LL) -0.03 5-6 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.30 Vert(TL) -0.05 2-6 >999 240 0.0 BCLL Rep Stress Incr YES WB 0.32 Horz(TL) -0.01 5 n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.11 5-6 >999 240 Weight: 70 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No 2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2 X 4 SYP No.2 end verticals WEBS 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 6-9-5 oc bracing. WEBS 2 X 4 SYP No.3 - 4-5 T-Brace: Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. REACTIONS (lb/size) 5=364/Mechanical, 2=501/0-3-8 Max Horz 2=438(LC 6) Max Uplift 5=-478(LC 6), 2=-403(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-458/502 **BOT CHORD** 2-6=-781/307, 5-6=-781/307 **WEBS** 3-6=-521/207, 3-5=-368/939 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C 5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 478 lb uplift at joint 5 and 403 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this true.
8) Warning: Additional permanent and stability bracing for truss system (not part of this combuilding is the responsibility of the building. Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip 6) Provide media...
joint 2.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in ...
8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

STATE OF 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 

FLORIDA. May 24,2010

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Job	Truss	Truss Type	Qtv	Plv	Huber Res. / Roof	
314347	T10	нр	1			14338123
	1.10		l'		Job Reference (optional)	
Builders Eart Course Lake City	EI 220EE			7.14	10 - Oot 1 2000 MiTch lad office has Man Man	24 44-02-40 2040 Dece 2

NOTES (12-13)

- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 241 lb down and 340 lb up at 6-11-8, 89 lb down and 124 lb up at 9-0-4, 89 lb down and 124 lb up at 13-10-12, 89 lb down and 124 lb up at 13-10-12, and 89 lb down and 124 lb up at 13-10-12, and 201 lb down and 334 lb up at 19-11-8 on top chord, and 271 lb down and 457 lb up at 6-11-8, 77 lb down and 52 lb up at 9-0-4, 77 lb down and 52 lb up at 13-0-4, 77 lb down and 52 lb up at 13-0-4, 77 lb down and 52 lb up at 13-0-4, 77 lb down and 52 lb up at 17-10-12, and 271 lb down and 52 lb up at 19-10-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

## LOAD CASE(S) Standard

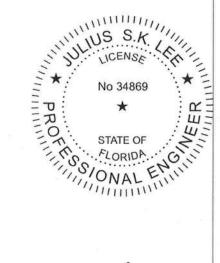
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

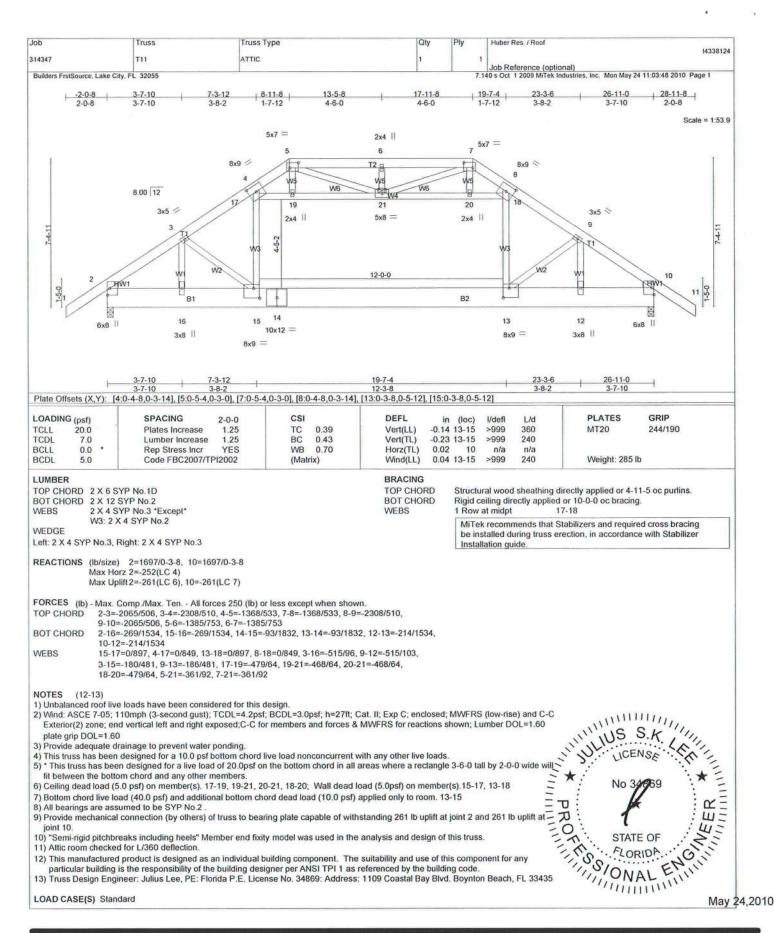
Vert: 1-2=-54, 2-4=-54, 4-6=-54, 6-8=-54, 8-9=-54, 10-14=-10

Concentrated Loads (lb)

Vert: 4=-201(F) 6=-201(F) 13=-208(F) 11=-208(F) 15=-89(F) 16=-89(F) 17=-89(F) 18=-89(F) 19=-89(F) 20=-89(F) 21=-44(F) 22=-44(F) 23=-44(F) 25=-44(F) 25=-44(F) 26=-44(F) 26=-44(F

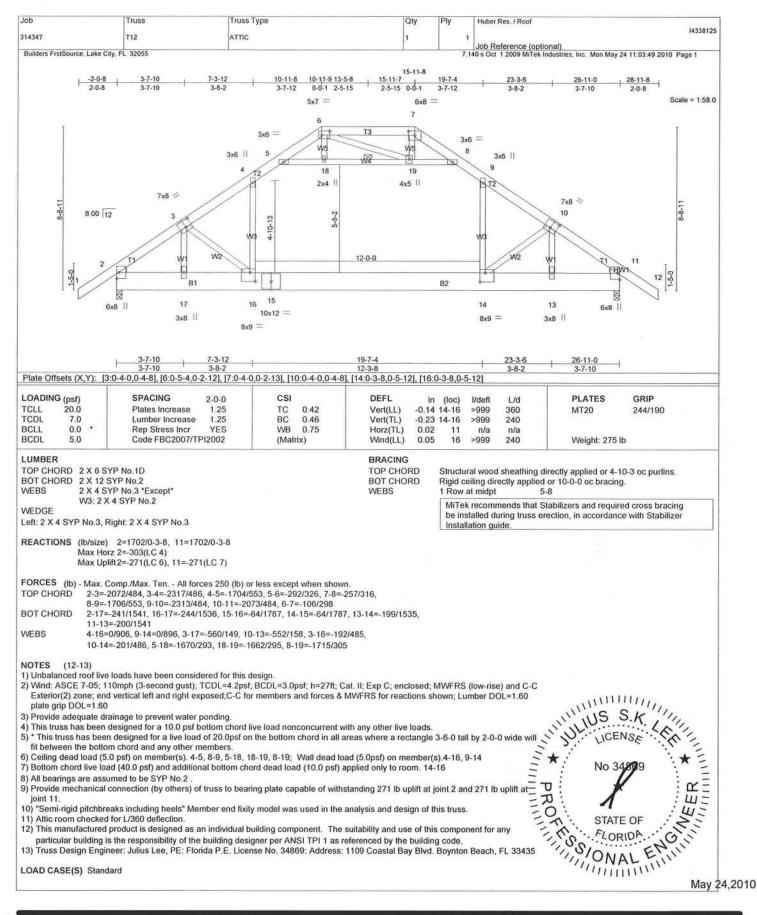






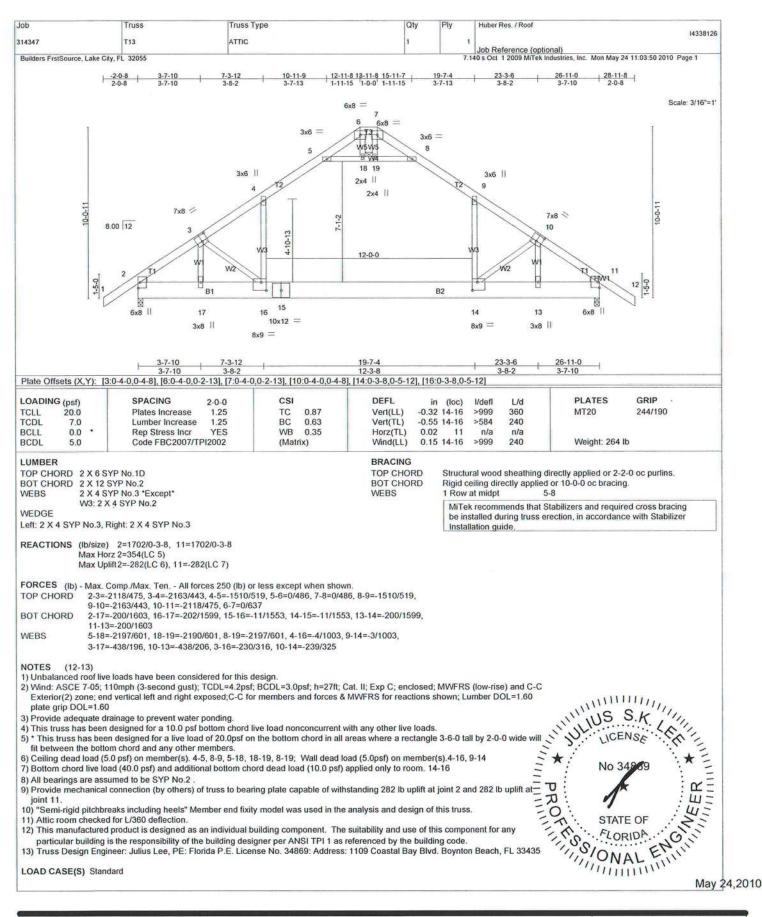
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Job Truss Type Truss Qty Huber Res. / Roof Ply 14338127 314347 T14 ATTIC Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:50 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-8 13-5-8 | 15-11-6 | 2-0-8 3-8-2 3-7-14 2-5-14 2-5-14 3-7-14 3-8-2 3-7-10 5x7 = Scale = 1:66.2 3x6 3x6 = 18 <sup>5</sup> 17 3x6 [] 3x6 || 2x4 || 7×8 / 7x8 8 00 12 3 4-10-12-0-0 B1 B2 6x8 || 16 15 13 12 3x8 || 10x12 = 8x9 = 3x8 || 8x9 = 3-7-10 7-3-12 3-8-2 12-3-8 Plate Offsets (X,Y): [3:0-4-0,0-4-8], [9:0-4-0,0-4-8], [13:0-3-8,0-5-12], [15:0-3-8,0-5-12] LOADING (psf) SPACING CSI DEFL GRIP 3-0-0 I/defl PLATES (loc) L/d TCIL 20.0 Plates Increase 1.25 TC 0.97 Vert(LL) -0.29 13-15 >999 360 MT20 244/190 TCDL 7.0 BC Lumber Increase 1.25 0.66 Vert(TL) -0.53 13-15 >602 240 Rep Stress Incr BCLL 0.0 NO WB 0.46 Horz(TL) 0.02 10 n/a n/a BCDL Code FBC2007/TPI2002 0.11 13-15 5.0 (Matrix) Wind(LL) >999 240 Weight: 526 lb LUMBER BRACING TOP CHORD 2 X 6 SYP No.1D TOP CHORD 2-0-0 oc purlins (5-7-1 max.) BOT CHORD 2 X 12 SYP No.2 (Switched from sheeted: Spacing > 2-0-0). WEBS 2 X 4 SYP No.3 \*Except\* BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. W3: 2 X 4 SYP No.2 JOINTS 1 Brace at Jt(s): 6 No 34909

ROSTATE OF

FLORIDA

ONAL

INTERIOR OF

TOTAL WEDGE Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3 REACTIONS (lb/size) 2=4170/0-3-8, 10=2841/0-3-8 Max Horz 2=-549(LC 3) Max Uplift 2=-204(LC 5), 10=-418(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-5109/41, 3-4=-4121/175, 4-18=-2795/295, 5-18=-2508/299, 5-6=0/954, 6-7=0/910, TOP CHORD 7-8=-2710/302, 8-9=-3935/188, 9-10=-3534/177 0 BOT CHORD 2-16=-99/3799, 15-16=-99/3803, 14-15=0/2838, 13-14=0/2838, 12-13=-8/2684, Ш 10-12=-9/2695 HOW 5-17=-3983/242, 7-17=-3983/242, 4-15=0/1697, 8-13=0/1951, 3-16=-239/542, WEBS 9-12=-1113/265, 3-15=-1423/280, 9-13=-327/526 NOTES (15-16)1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Bottom chords connected as follows: 2 X 12 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 7) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-17, 7-17; Wall dead load (5.0 psf) on member(s).4-15, 8-13 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15 9) All bearings are assumed to be SYP No.2 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2 and 418 lb uplift at joint 10. 11) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss. 12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 13) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails Continued on page 2 May 24,2010

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Qty Job Truss Type Ply Huber Res, / Roof Truss ATTIC ✓ Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:50 2010 Page 2 Builders FrstSource, Lake City, FL 32055 NOTES (15-16) 14) Attic room checked for L/360 deflection. 15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 2-15=-115(B=-100), 13-15=-165, 10-13=-15, 1-2=-81, 5-18=-96, 5-6=-81, 6-7=-81, 7-8=-96, 8-11=-81, 5-7=-15 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-247-to-4=-182, 4=-197-to-18=-173 2) IBC BC Live: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 2-15=-65(B=-20), 10-15=-45, 1-2=-21, 5-18=-36, 5-6=-21, 6-7=-21, 7-8=-36, 8-11=-21, 5-7=-15 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-139-to-4=-74, 4=-89-to-18=-65 3) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 2-15=-29(B=-20), 13-15=-27, 10-13=-9, 1-2=29, 5-18=-24, 5-6=-15, 6-7=42, 7-8=33, 8-10=42, 10-11=26, 5-7=-9 Horz: 1-2=-42, 2-6=3, 6-10=54, 10-11=38 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-78-to-4=-13, 4=-22-to-18=2 4) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 2-15=-29(B=-20), 13-15=-27, 10-13=-9, 1-2=26, 5-18=33, 5-6=42, 6-7=-15, 7-8=-24, 8-10=-15, 10-11=29, 5-7=-9 Horz: 1-2=-38, 2-6=-54, 6-10=-3, 10-11=42 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-21-to-4=44, 4=35-to-18=59 5) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 2-15=-29(B=-20), 13-15=-27, 10-13=-9, 1-2=91, 5-18=44, 5-6=53, 6-7=29, 7-8=20, 8-10=29, 10-11=15, 5-7=-9 Horz: 1-2=-104, 2-6=-66, 6-10=42, 10-11=28 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-0-to-4=65, 4=56-to-18=80 6) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 2-15=-29(B=-20), 13-15=-27, 10-13=-9, 1-2=15, 5-18=20, 5-6=29, 6-7=53, 7-8=44, 8-10=53, 10-11=91, 5-7=-9 No 34899

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INTERIOR OF Horz: 1-2=-28, 2-6=-42, 6-10=66, 10-11=104 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-24-to-4=41, 4=32-to-18=56 7) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 2-15=-29(B=-20), 13-15=-27, 10-13=-9, 1-2=91, 5-18=44, 5-6=53, 6-7=29, 7-8=20, 8-10=29, 10-11=15, 5-7=-9 Horz: 1-2=-104, 2-6=-66, 6-10=42, 10-11=28 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-0-to-4=65, 4=56-to-18=80 8) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 2-15=-29(B=-20), 13-15=-27, 10-13=-9, 1-2=15, 5-18=20, 5-6=29, 6-7=53, 7-8=44, 8-10=53, 10-11=91, 5-7=-9 Horz: 1-2=-28, 2-6=-42, 6-10=66, 10-11=104 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-24-to-4=41, 4=32-to-18=56 9) Attic Floor: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 2-15=-115(B=-100), 13-15=-165, 10-13=-15, 1-2=-21, 5-18=-36, 5-6=-21, 6-7=-21, 7-8=-36, 8-11=-21, 5-7=-15 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-139-to-4=-74, 4=-89-to-18=-65 10) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 2-15=-115(B=-100), 13-15=-165, 10-13=-15, 1-2=-81, 5-18=-96, 5-6=-81, 6-7=-21, 7-8=-36, 8-11=-21, 5-7=-15 Drag: 4-15=-15, 8-13=-15 Trapezoidal Loads (plf) Vert: 2=-247-to-4=-182, 4=-197-to-18=-173 11) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 2-15=-115(B=-100), 13-15=-165, 10-13=-15, 1-2=-21, 5-18=-36, 5-6=-21, 6-7=-81, 7-8=-96, 8-11=-81, 5-7=-15 Drag: 4-15=-15, 8-13=-15

May 24,2010

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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milak connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to inset stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control. storage, delivery, erection and bracing, consult — ANSI/IPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information—available from truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719,

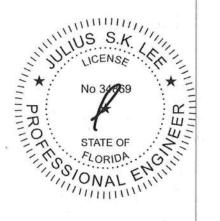
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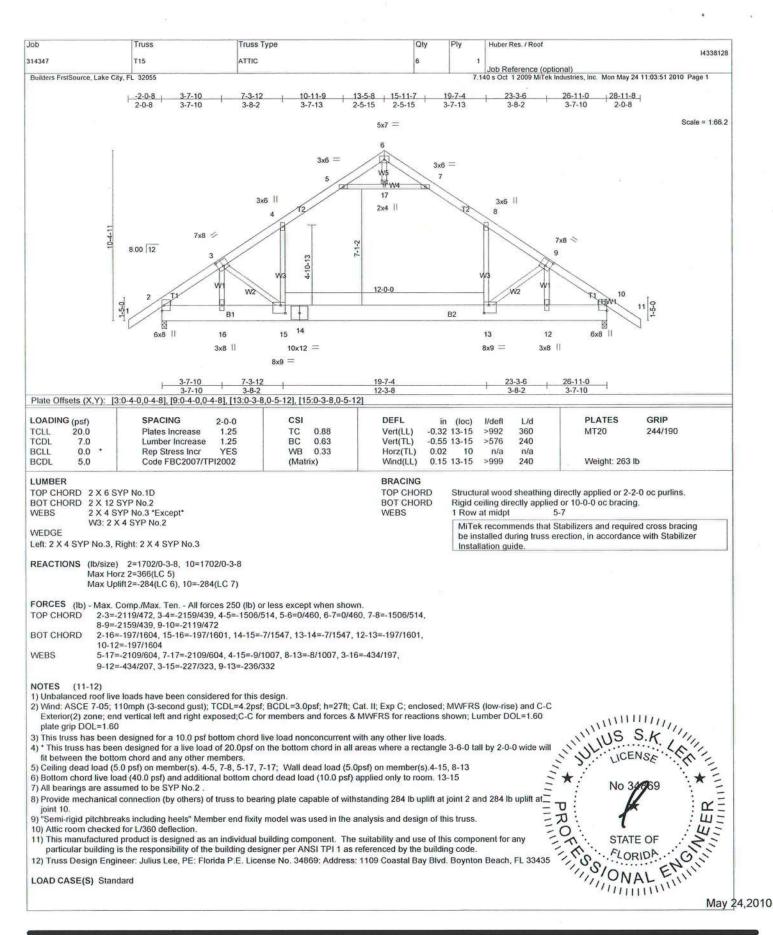
Job	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	200 50 500
314347	T14	ATTIC	2	2	Job Reference (optional)	14338127
Builders FrstSource, Lake City, F	L 32055				10 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:50 2010 Pag	je 3

LOAD CASE(S) Standard

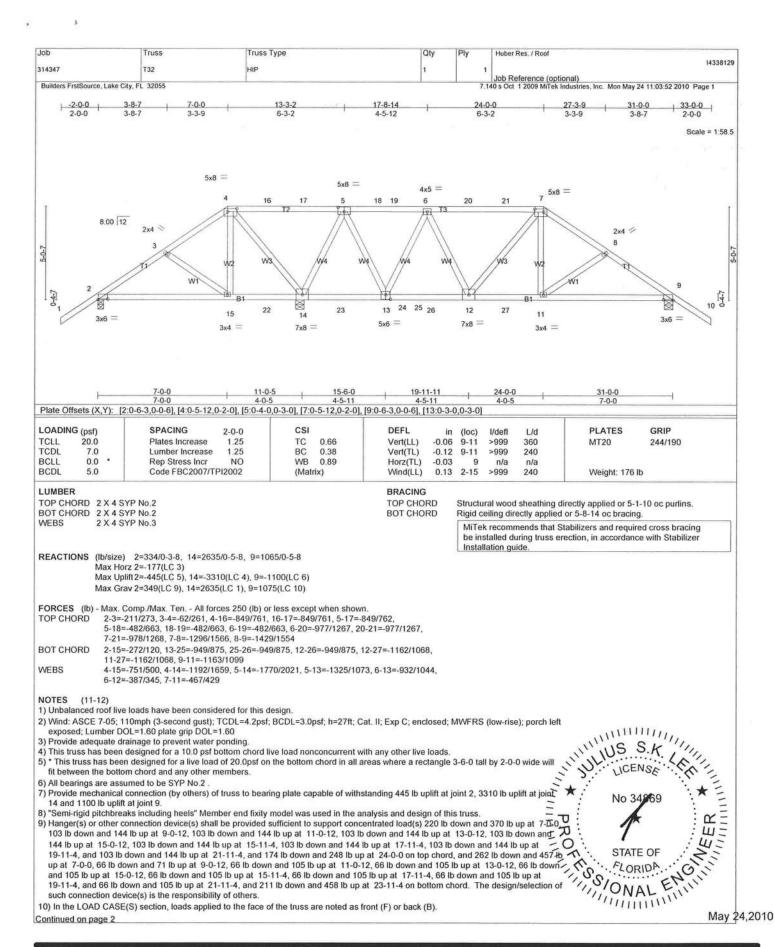
Trapezoidal Loads (plf) Vert: 2=-187-to-4=-122, 4=-137-to-18=-113



May 24,2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-74-73 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, DS8-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.

	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	Edition Comments
147	T32	HIP	1	1		1433812
lers FrstSource, Lake Ci	tv. FL 32055	1949		7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon	May 24 11:03:52 2010 Page 2
ouilding designer p Fruss Design Engir D CASE(S) Stand egular: Lumber Inc niform Loads (plf) Vert: 1-4=-5 oncentrated Loads Vert: 4=-220	er ANSI TPI 1 as referencer: Julius Lee, PE: Fl dard drease=1.25, Plate Incre 4, 4-7=-54, 7-10=-54, 2 ((lb) 0(B) 5=-103(B) 7=-134(		1109 Coastal Bay Bl	vd. Boynton	Beach, FL 33435	
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Job Truss Truss Type Qty Huber Res. / Roof 14338130 314347 T33 Job Reference (optional) 0 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:52 2010 Page 1 Builders FrstSource, Lake City, FL 32055 -2-0-0 4-5-14 9-0-0 15-6-0 22-0-0 26-6-2 31-0-0 6-6-0 6-6-0 4-6-2 4-5-14 2-0-0 Scale = 1:58.5 5x6 3x8 = 4x5 = 5 8.00 12 2x4 \\ 2x4 = 0-4-7 13 14 12 11 10 3x5 / 3x5 5x6 = 5x6 = 3x8 = 10-10-12 5-2-15 15-6-0 22-0-0 31-0-0 Plate Offsets (X,Y): [2:0-1-15,0-1-8], [4:0-3-12,0-2-0], [6:0-2-12,0-2-0], [8:0-1-15,0-1-8], [11:0-3-0,0-3-0] LOADING (psf) SPACING CSI **PLATES** GRIP 2-0-0 (loc) I/defl L/d 20.0 Plates Increase TC TCLL 1.25 0.40 Vert(LL) -0.158-10 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.39 Vert(TL) -0.25 >936 240 8-10 BCLL 0.0 Rep Stress Incr YES WB 0.93 0.01 Horz(TL) n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.05 2-13 >999 240 Weight: 177 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 6-0-0 oc bracing. 2 X 4 SYP No.3 WEBS MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 2=301/0-3-8, 12=1290/0-5-8, 8=698/0-5-8 Max Horz 2=228(LC 5) Max Uplift 2=-397(LC 6), 12=-772(LC 5), 8=-361(LC 7) Max Grav 2=333(LC 10), 12=1290(LC 1), 8=715(LC 11) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-190/374, 3-4=-100/456, 4-5=-112/317, 5-6=-453/440, 6-7=-615/431, 7-8=-821/517 2-13=-273/169, 12-13=-154/349, 11-12=-59/275, 11-14=-59/275, 10-14=-59/275, BOT CHORD 8-10=-190/613 3-13=-200/285, 4-13=-679/272, 4-12=-548/647, 5-12=-909/616, 5-10=-182/272, WEBS 7-10=-190/267 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C ALLIULIUS S.L. Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf. 6) All bearings are assumed to be SYP No.2 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 397 lb uplift at joint 2, 772 lb uplift at joint 42 and 361 lb uplift at joint 8. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 30FE THO NONAL HOLA LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

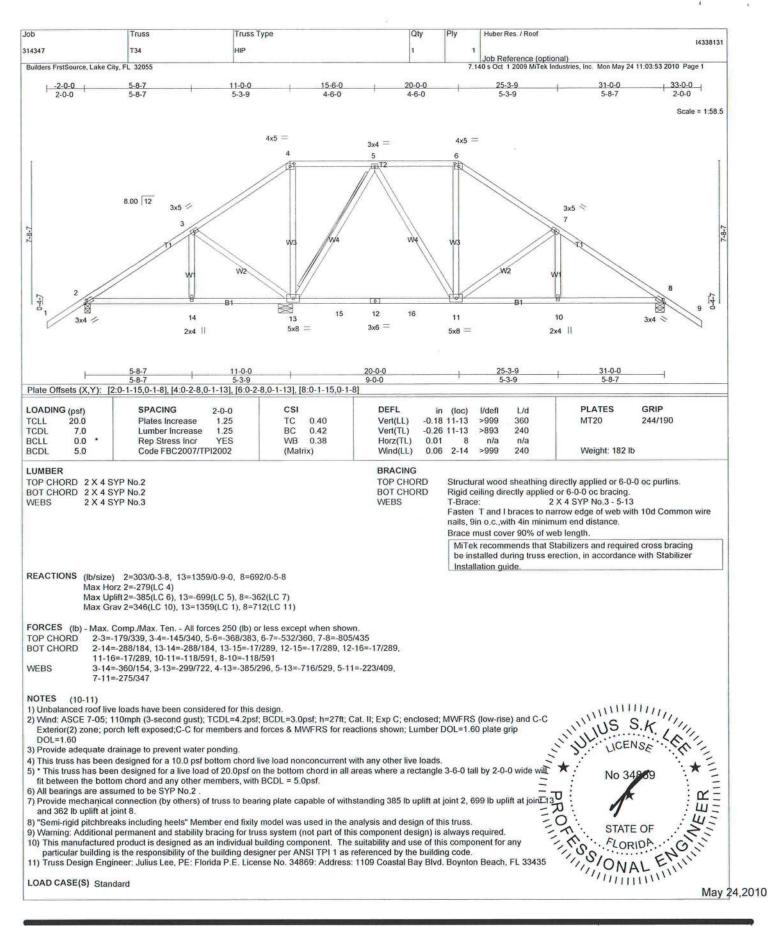
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

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ANSI/ITEL ADSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Madison, WI 53719.

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

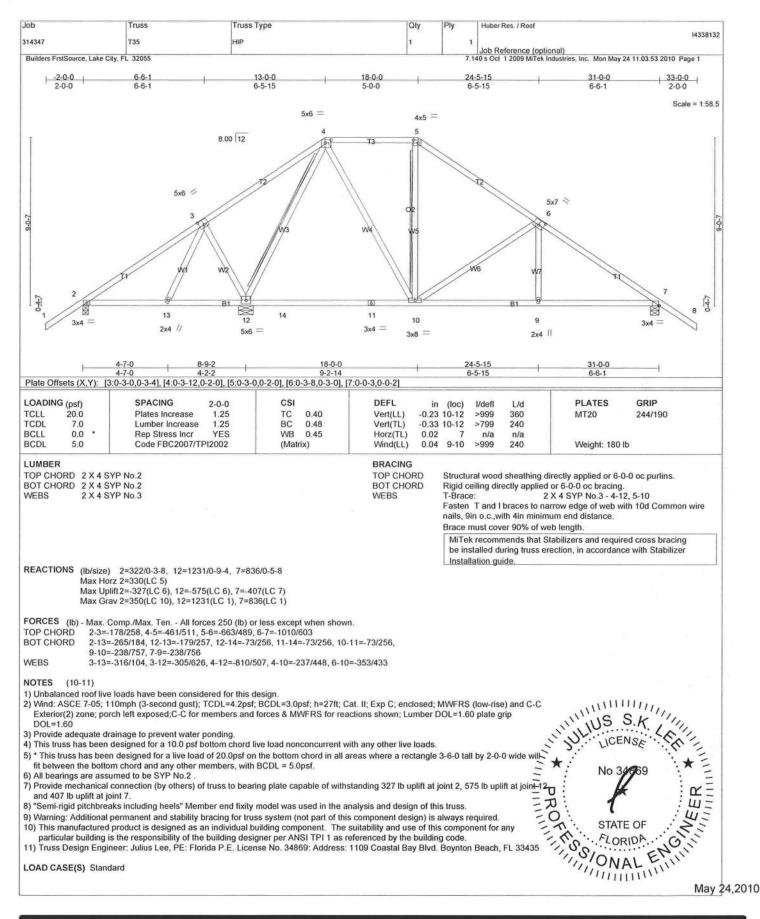
May 24,2010



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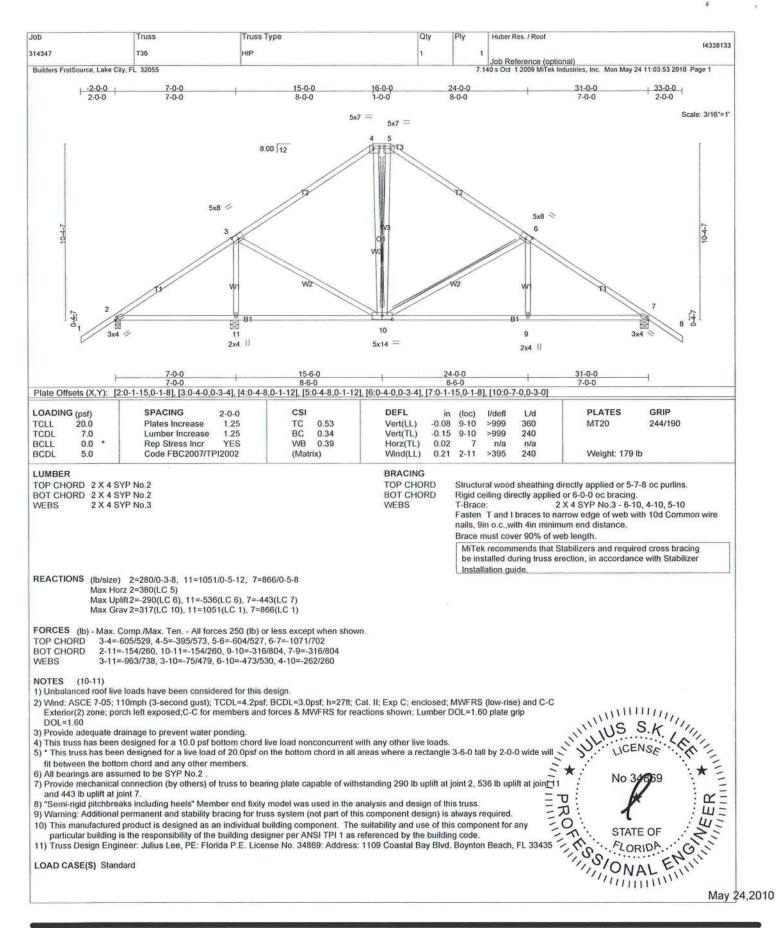
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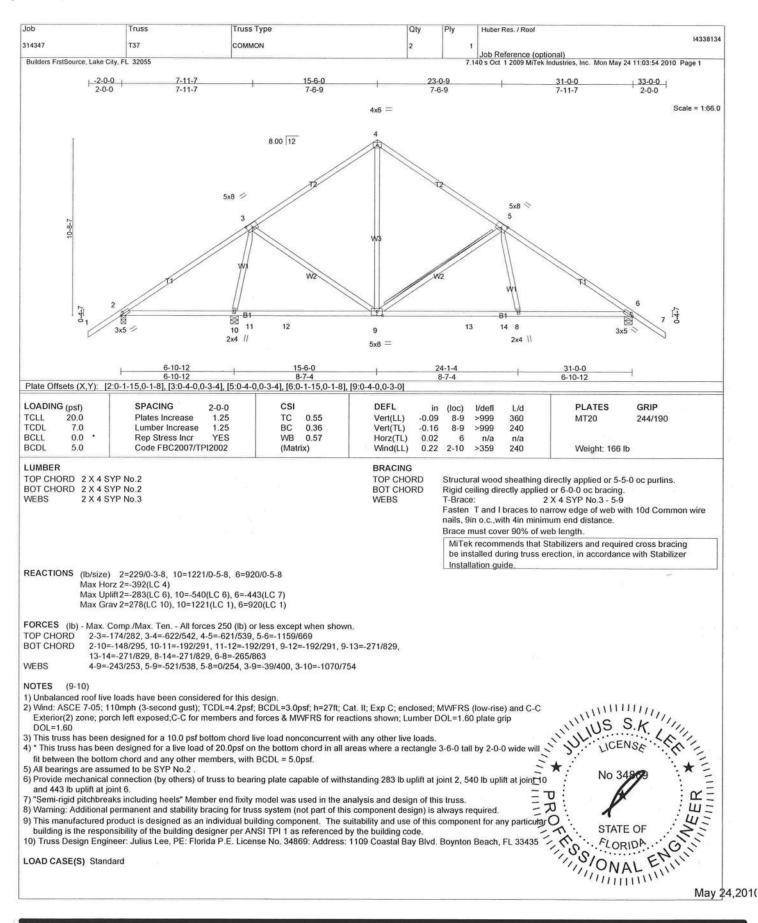
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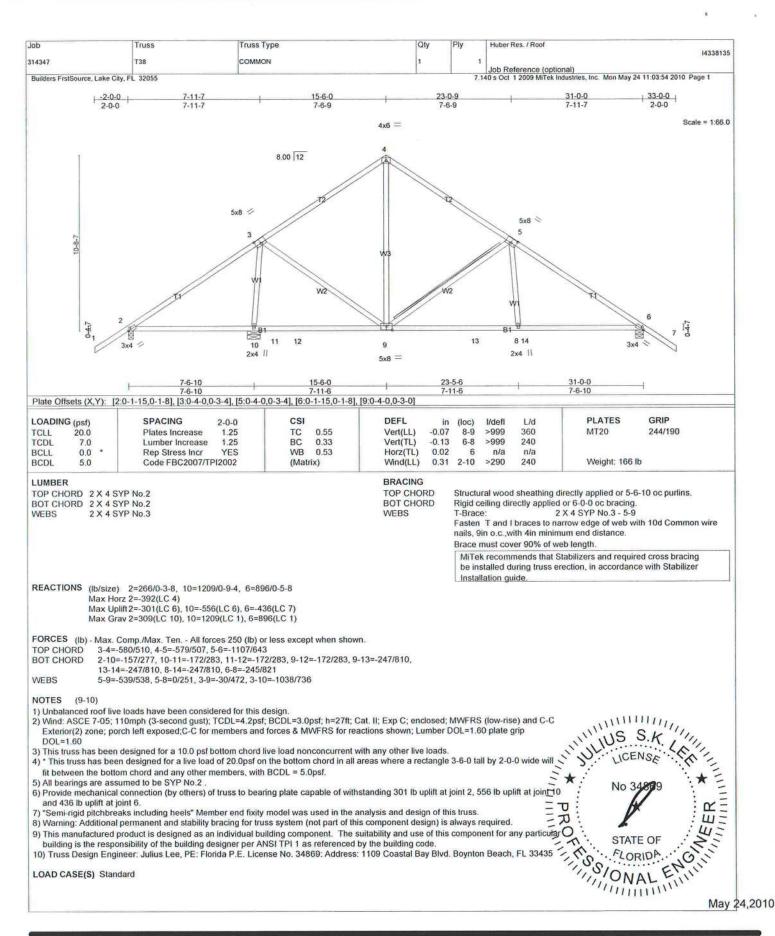
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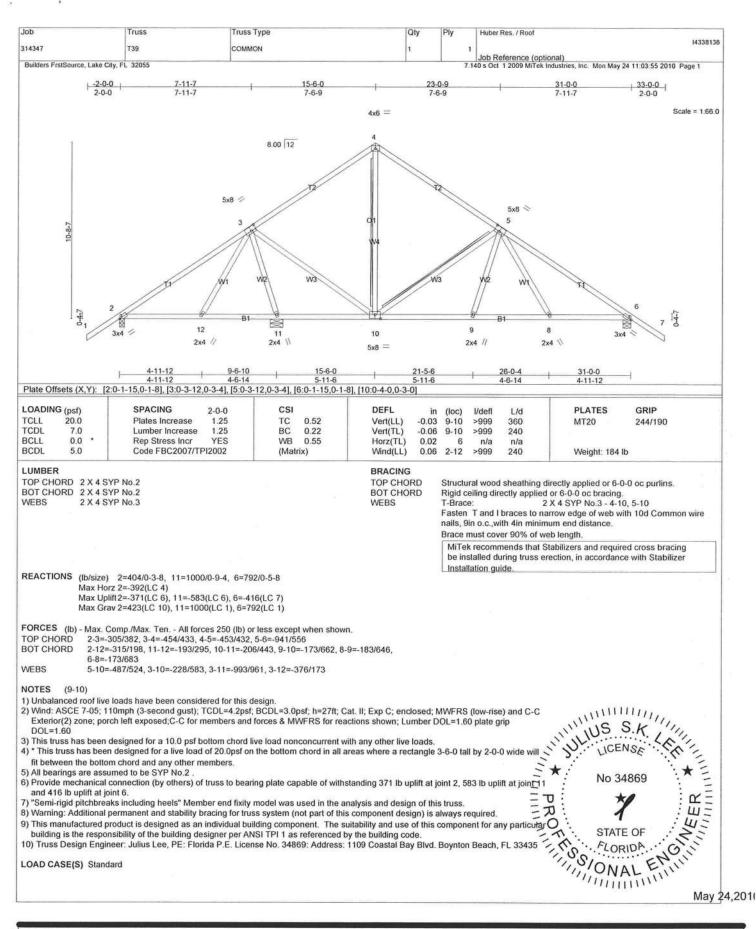
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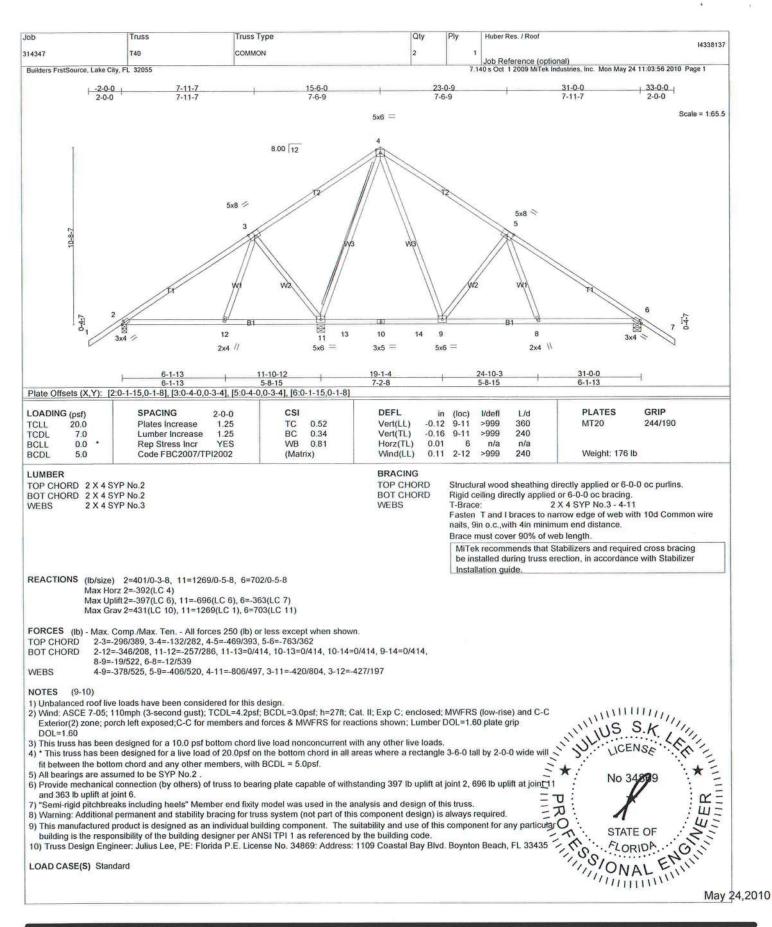
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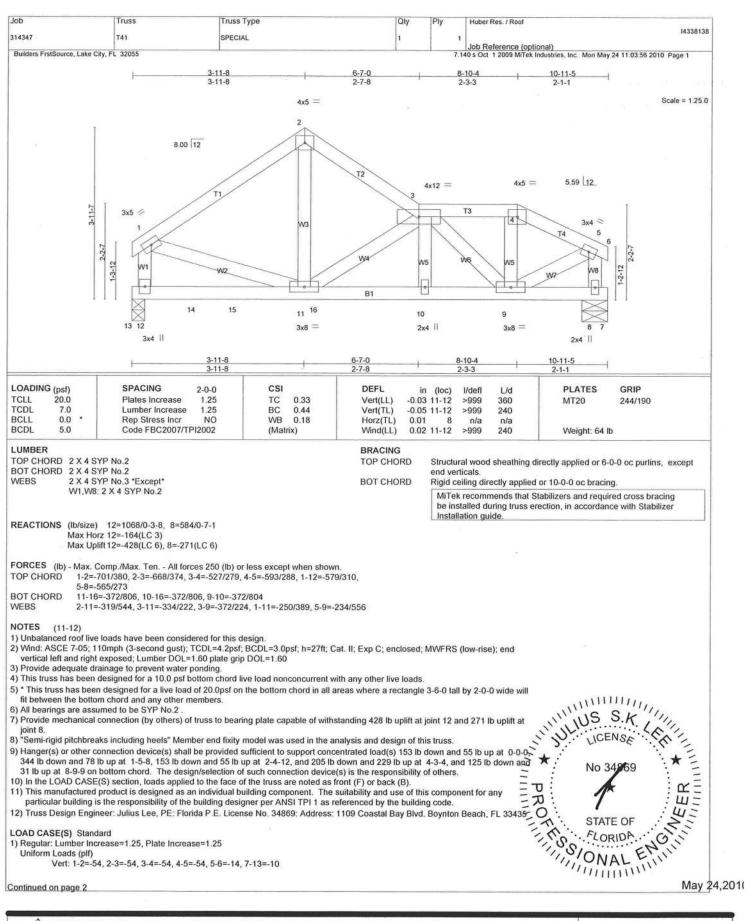
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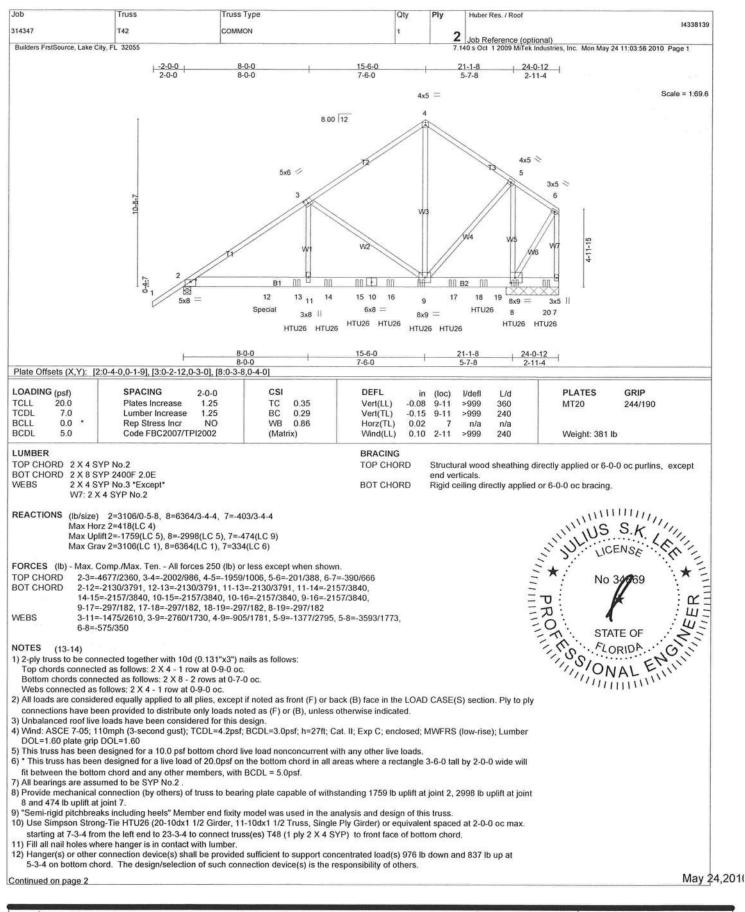
Safety Information available from Truss Plate Institute, 583 D'Onafrio Drive, Madison, WI 53719.

ob	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	143381
14347	T41	SPECIAL	1	1	Job Reference (optional)	
Builders FrstSource, Lake City, F	FL 32055	— <del>  </del>		7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon May 24	11:03:56 2010 Page 2
LOAD CASE(S) Standard Concentrated Loads (lb Vert: 13=-153(l		5=-153(B) 16=-205(B)				
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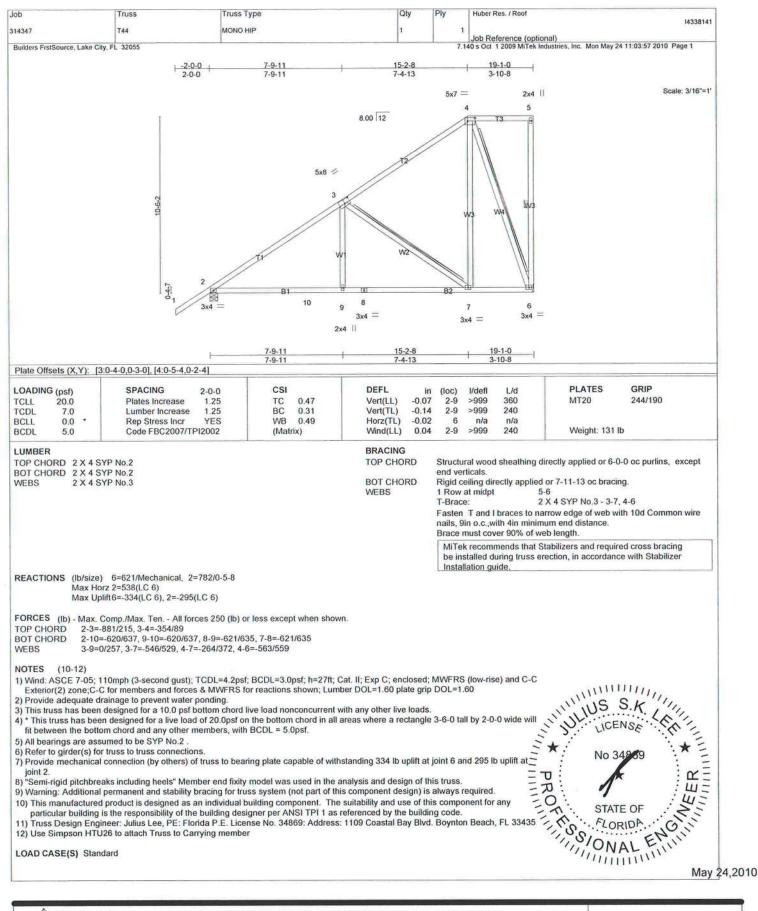
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ilders FrstSource, Lake City, F	FL 32055			7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon May	24 11:03:56 2010 Page 2
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May 24,2010

Job Truss Truss Type Qty Huber Res. / Roof 14338140 314347 T43 COMMON Job Reference (optional) 0 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:03:57 2010 Page 1 Builders FrstSource Lake City FL 32055 -2-0-0 2-0-0 19-1-0 3-7-0 7-6-9 Scale: 3/16"=1" 5x6 = 8.00 12 2x4 | 5x8 / 10 8 7 11 3x4 = 3x5 19-1-0 Plate Offsets (X,Y): [2:0-1-15,0-1-8], [3:0-4-0,0-3-4] LOADING (psf) SPACING CSI DEFL PLATES GRIP (loc) I/defl L/d TC BC TCLL 20.0 Plates Increase 1.25 0.52 Vert(LL) -0.32 6-8 >699 360 MT20 244/190 TCDL 1.25 7.0 Lumber Increase 0.74 Vert(TL) -0.446-8 >513 240 BCLL 0.0 Rep Stress Incr 0.36 WB Horz(TL) 0.01 6 n/a n/a BCDL 50 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.06 2-8 >999 240 Weight: 119 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 5-10-8 oc purlins, except BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 \*Except\* BOT CHORD Rigid ceiling directly applied or 8-2-11 oc bracing. W4: 2 X 4 SYP No.2 WEBS 4-8, 5-6, 4-6 1 Row at midpt MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide REACTIONS (lb/size) 6=801/Mechanical, 2=843/0-5-8 Max Horz 2=482(LC 6) Max Uplift 6=-311(LC 6), 2=-323(LC 6) FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-980/326, 3-4=-853/520 BOT CHORD 2-9=-585/719, 9-10=-585/719, 8-10=-585/719 3-8=-388/560, 4-8=-541/805, 4-6=-607/468 WEBS NOTES (9-11)1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 STATISTICS S.K 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf. 5) All bearings are assumed to be SYP No.2 Refer to girder(s) for truss to truss connections. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 6 and 323 lb uplift at 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. No 34869 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33436 ROTTOS/ONAL 11) Use Simpson HTU26 to attach Truss to Carrying member ONE LOAD CASE(S) Standard May 24,201

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

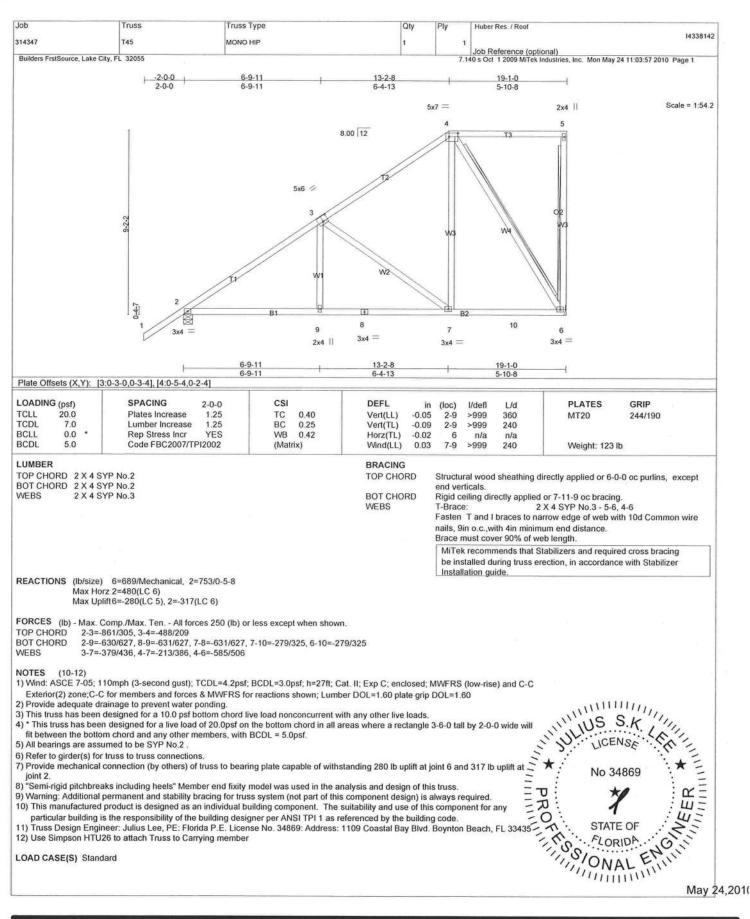
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/ITI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onotrio Drive, Madison. WI 53719.



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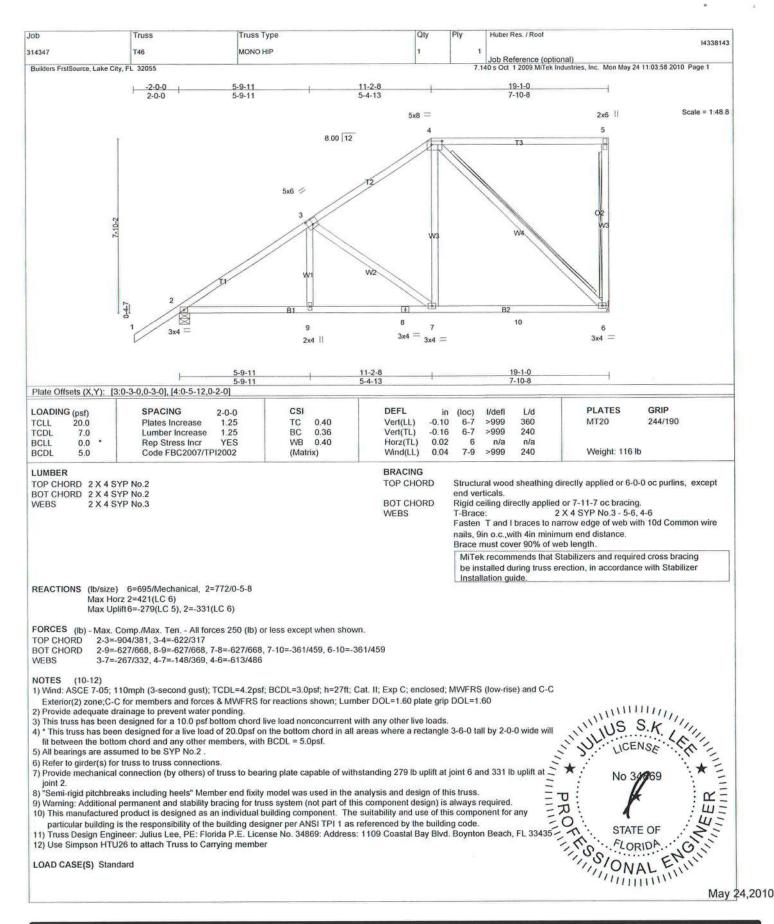
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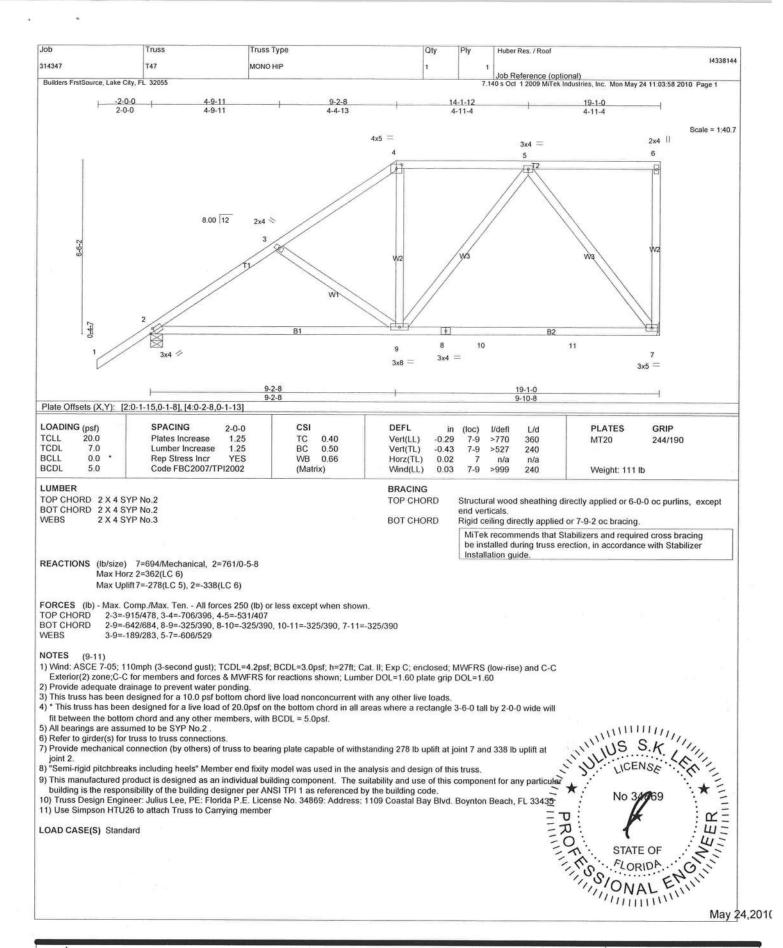
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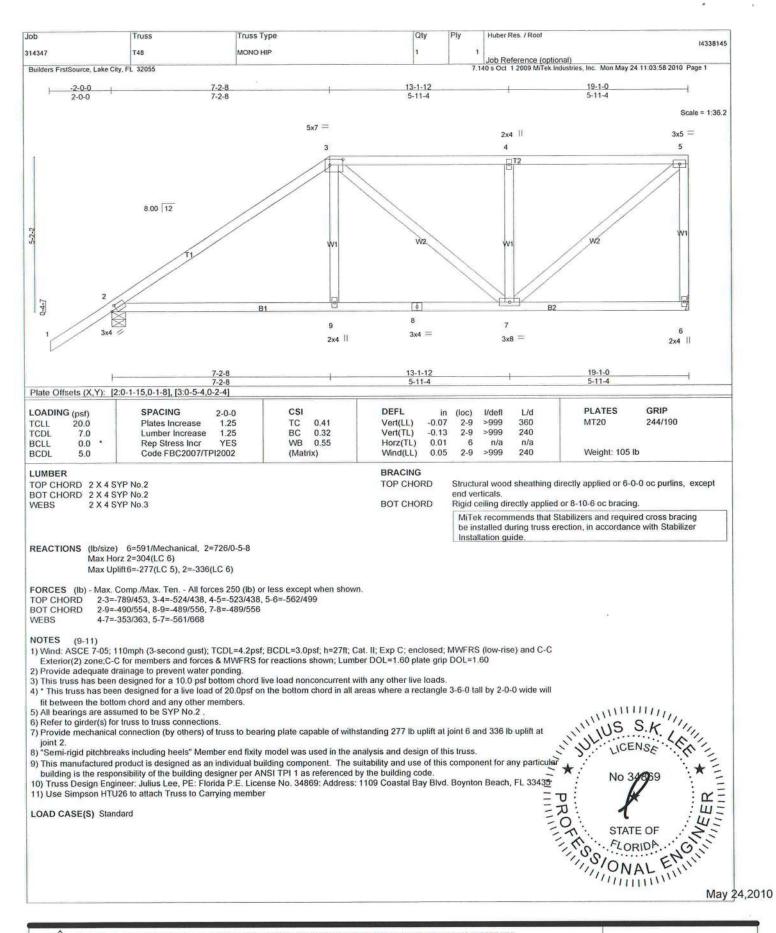
ANSI/IPII Quality Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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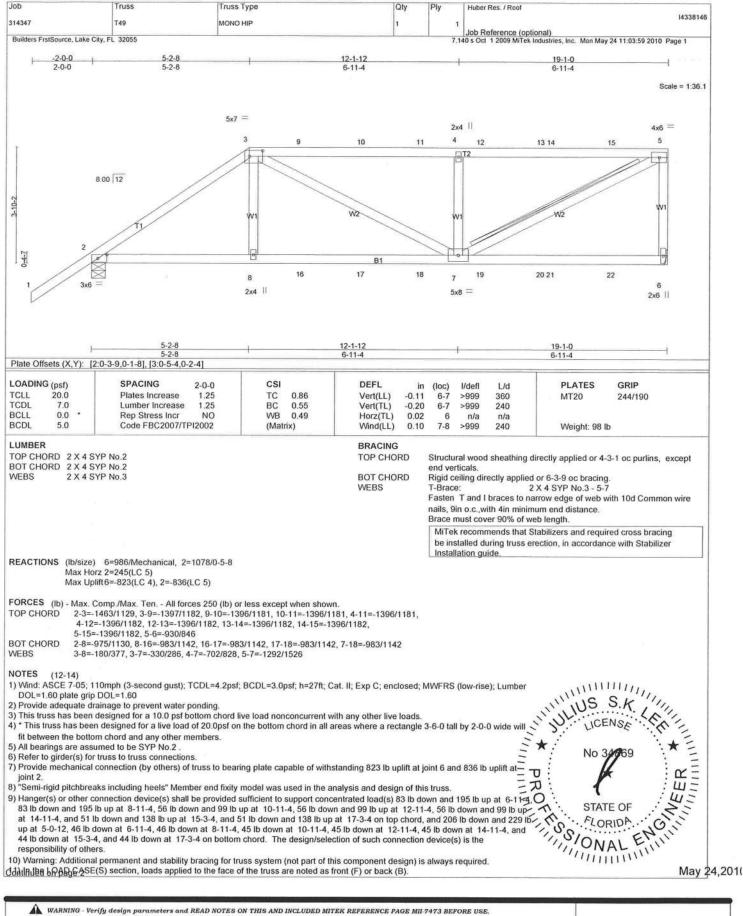
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	W	TT	Ohr	Dhr	Huber Res. / Roof	
Job	Truss	Truss Type	City	riy	Hubel Res. / Roof	14338146
314347	T49	MONO HIP	1	1	Job Reference (optional)	1070710015

Builders FrstSource, Lake City, FL 32055

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- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

14) Use Simpson HTU26 to attach Truss to Carrying member

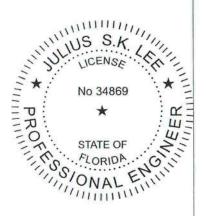
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 2-6=-10

Concentrated Loads (lb)

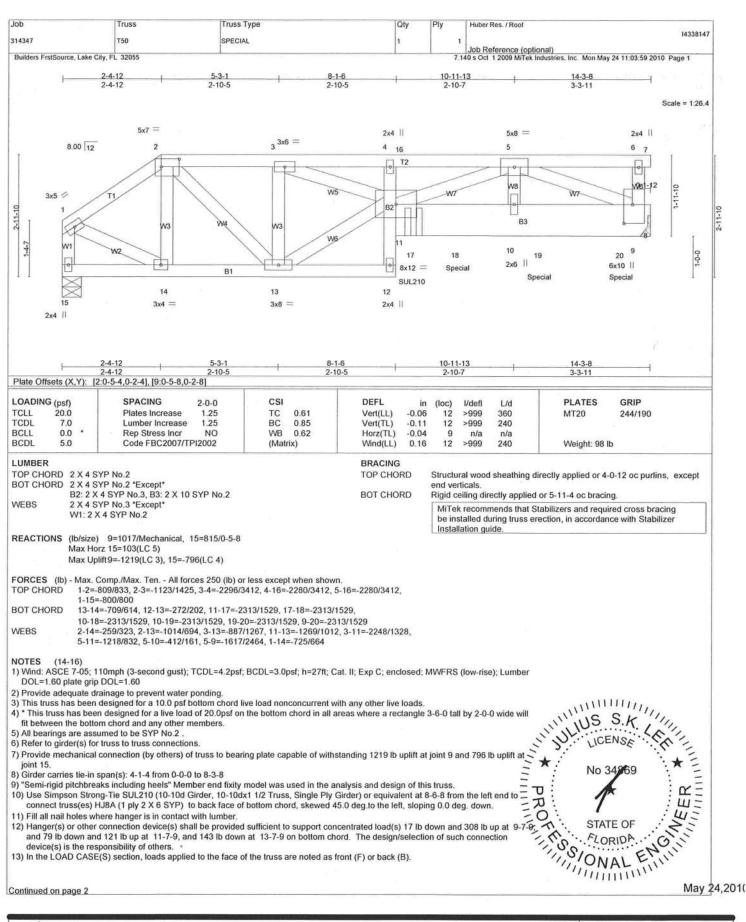
Vert: 8=-206(B) 9=-83(B) 10=-83(B) 11=-56(B) 12=-56(B) 13=-56(B) 14=-51(B) 15=-51(B) 16=-15(B) 17=-15(B) 18=-15(B) 19=-15(B) 20=-15(B) 21=-15(B) 2





May 24,2010

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is to used only upon parameters shown, and is for an individual building component. Applicability of design parameters and respective to use of the use of the



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	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	MANAGEMENT
47	T50	SPECIAL	1	1		143381
ders FrstSource, Lak	1000000				Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon Ma	w 24 11-03-59 2010 Page 2
ouilding designe Fruss Design Er Jse Simpson H D CASE(S) St egular: Lumber niform Loads (p Vert: 1-2: oncentrated Load	er per ANSI TPI 1 as refe ngineer: Julius Lee, PE: GUS28 to attach Truss I andard Increase=1.25, Plate In- if) =-83(B=-29), 2-16=-83(B	crease=1.25 3=-29), 6-16=-54, 6-7=-14, 12-15=-10, 8-1	1109 Coastal Bay Blvo			sponsibility of the
					PROPERTY N	TATE OF LORIDA CO

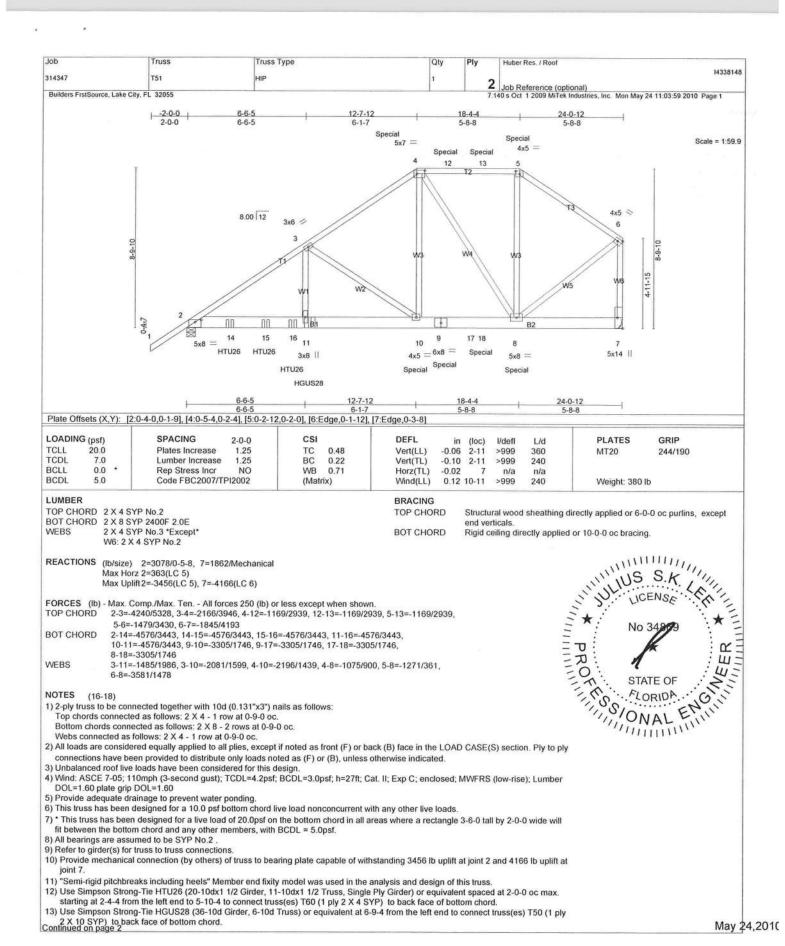
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May 24,2010

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ANSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onotrio Drive, Madison, WI 53719.



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Job	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	14338148
314347	T51	HIP	1	2	Job Reference (notional)	14330140

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## NOTES (16-18)

14) Fill all nail holes where hanger is in contact with lumber.

- 15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 221 lb down and 378 lb up at 12-7-12, 79 lb down and 169 lb up at 14-3-8, and 79 lb down and 169 lb up at 16-3-8, and 196 lb down and 399 lb up at 18-4-4 on top chord, and 196 lb down and 738 lb up at 12-7-12, 56 lb down and 452 lb up at 16-3-8, and 222 lb down and 2407 lb up at 18-3-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 16) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 17) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 18) Use Simpson HGUS28-2 to attach Truss to Carrying member

## LOAD CASE(S) Standard

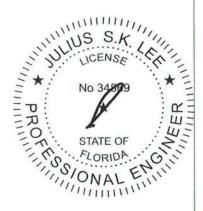
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

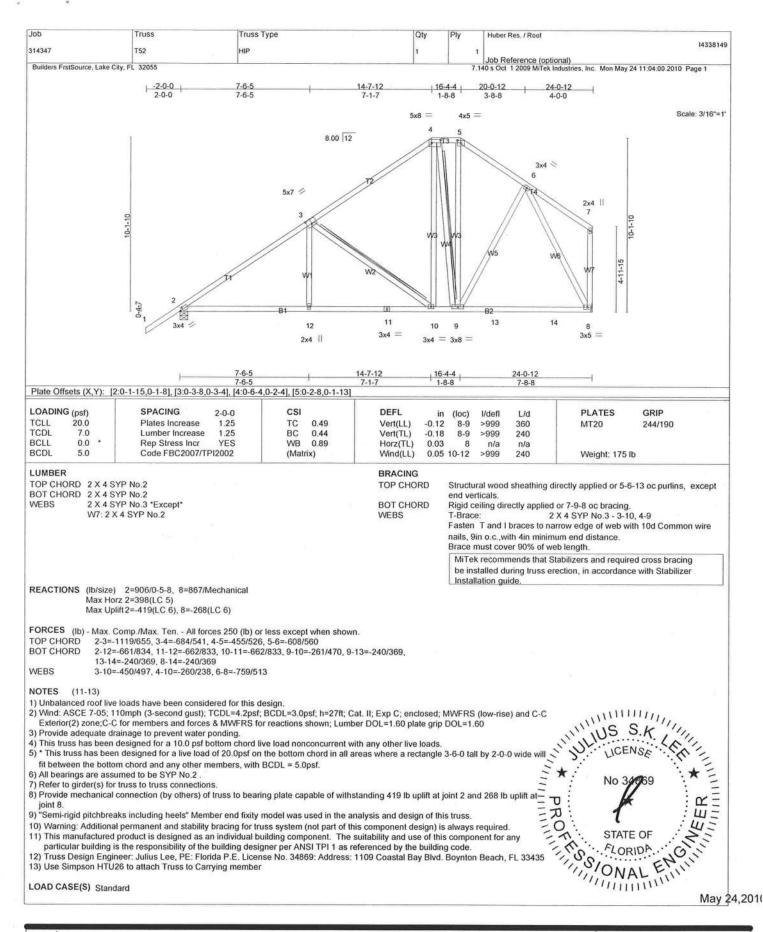
Vert: 1-4=-54, 4-5=-54, 5-6=-54, 2-10=-10, 10-17=-50, 7-17=-10

Concentrated Loads (lb)

Vert: 5=-196(B) 9=-19(B) 11=-1007(B) 10=-196(B) 4=-181(B) 8=-111(B) 12=-79(B) 13=-79(B) 14=-428(B) 15=-438(B) 16=-438(B) 18=-19(B)



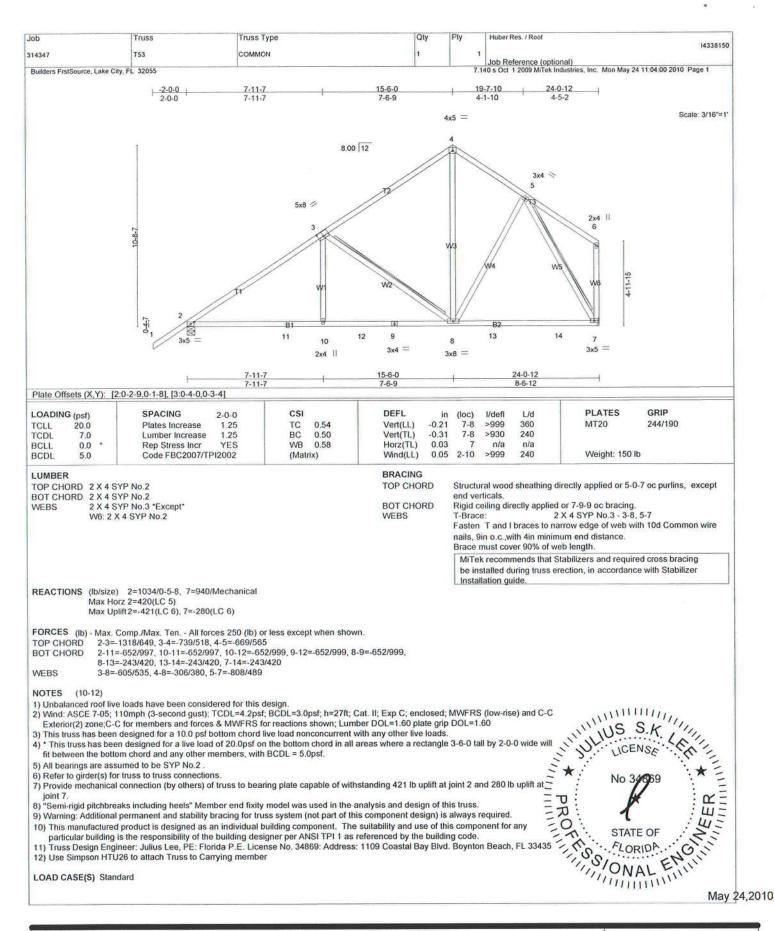
May 24,2010



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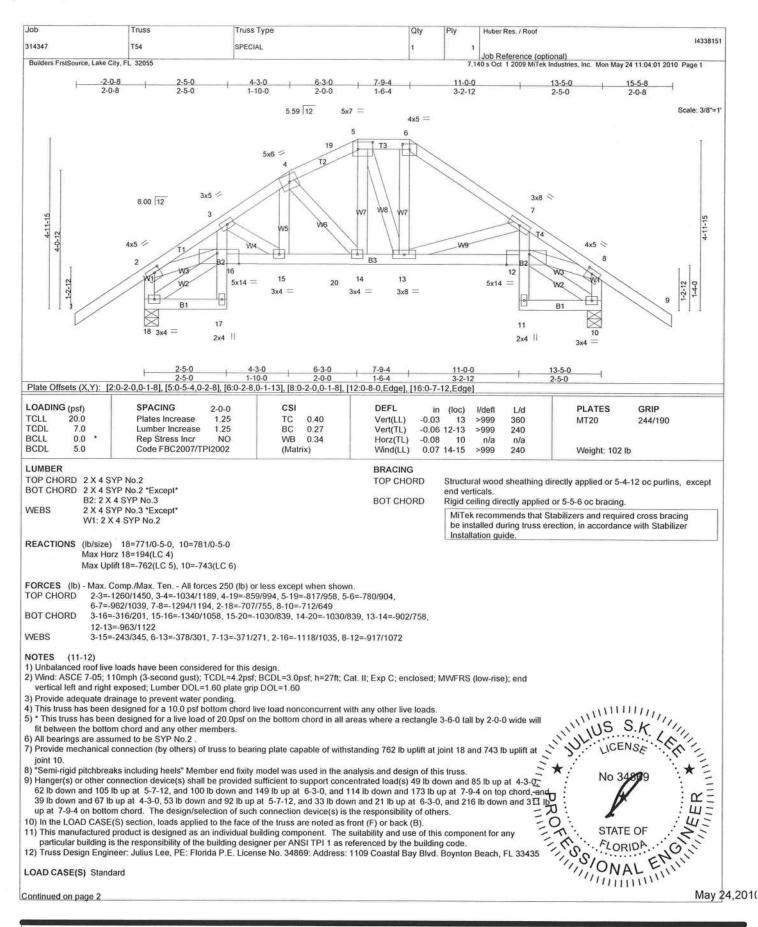


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Job	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	14220151
314347	T54	SPECIAL	1	1	Job Reference (optional)	14338151

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LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 4-5=-54, 5-6=-54, 6-8=-54, 8-9=-54, 17-18=-10, 12-16=-10, 10-11=-10 Concentrated Loads (lb)

Vert: 5=-60(F) 6=-74(F) 15=-33(F) 14=-11(F) 4=-9(F) 13=-216(F) 19=-22(F) 20=-51(F)

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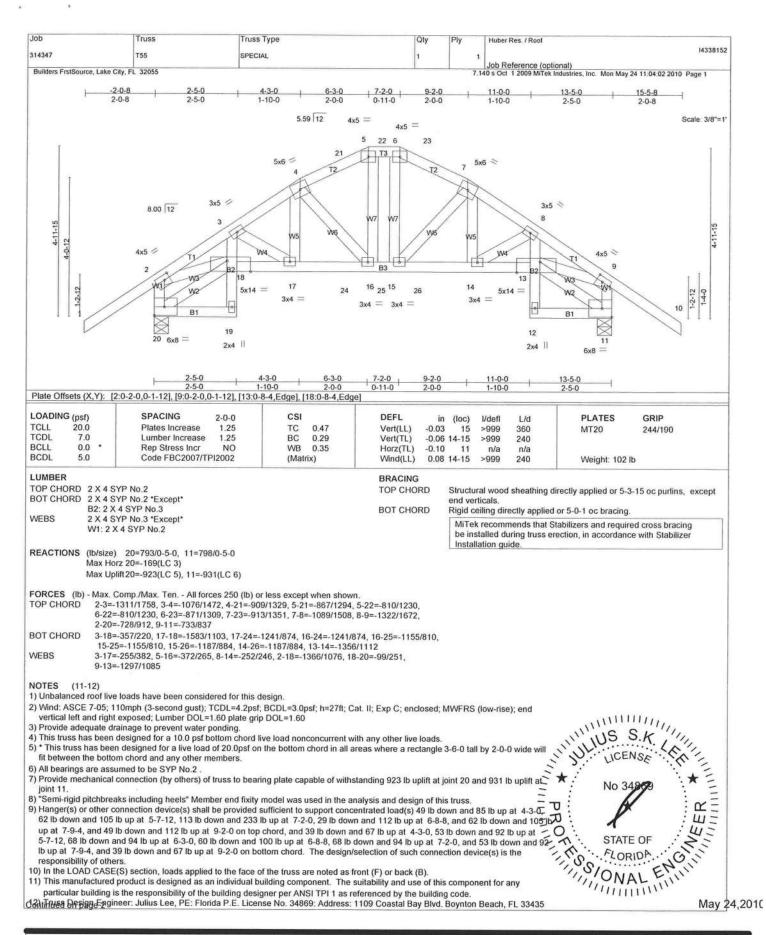
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Job	Truss	Truss Type	Qty	Ply	Huber Res. / Roof	338152
314347	T55	SPECIAL	1	1	Job Reference (optional)	000102

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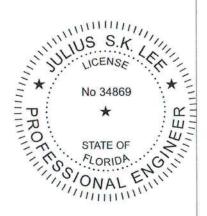
7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:04:02 2010 Page 2

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 4-5=-54, 5-6=-54, 6-7=-54, 7-9=-54, 9-10=-54, 19-20=-10, 13-18=-10, 11-12=-10

Concentrated Loads (lb)
Vert: 6=-73(B) 7=-9(B) 17=-33(B) 16=-63(B) 4=-9(B) 15=-63(B) 14=-33(B) 21=-22(B) 22=-29(B) 23=-22(B) 24=-51(B) 25=-57(B) 26=-51(B)

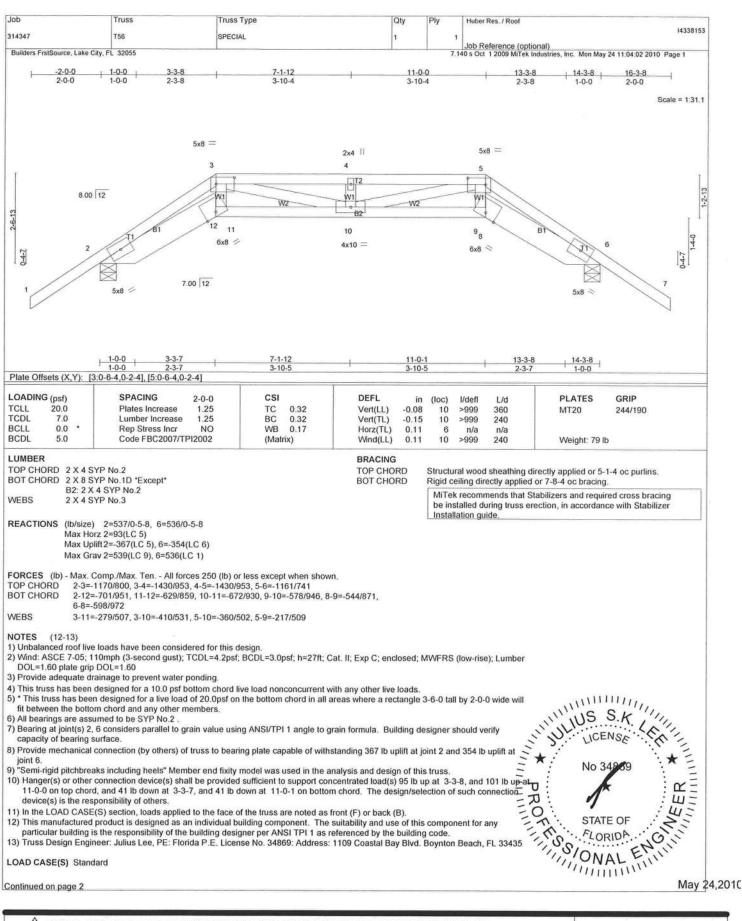




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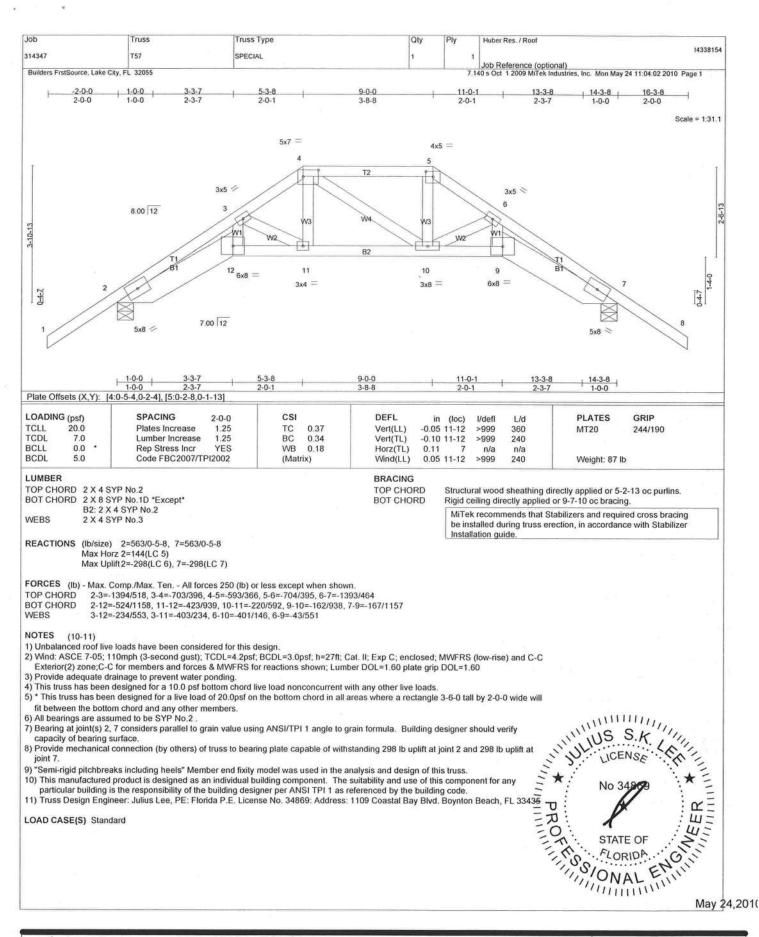
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T56 SPECIAL 1 1 Job Reference (optional) Source, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:04:02 2010 Page 2  SE(S) Standard : Lumber Increase=1.25, Plate Increase=1.25 Loads (plf) Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-12=-10, 8-12=-10, 6-8=-10 trated Loads (lb)	17 T56 SPECIAL 1 1 Job Reference (optional)		Truss	Truss Type	Qty	Ply Hube	er Res. / Roof	
Job Reference (optional)   7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 11:04:02 2010 Page 2	Job Reference (optional)	347	Lange Control	100000000000000000000000000000000000000				14338
SE(S) Standard : Lumber Increase=1.25, Plate Increase=1.25 : Loads (plf) Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-12=-10, 8-12=-10 trated Loads (lb)	AD CASE(S) Standard  Regular: Lumber Increase=1.25, Plate Increase=1.25  Inform Loads (plf)  Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-12=-10, 8-12=-10  Concentrated Loads (lb)					Job I	Reference (optional)	Mon May 24 11:04:02 2010 Page 2
: Lumber Increase=1.25, Plate Increase=1.25 Loads (plf) Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-12=-10, 8-12=-10, 6-8=-10 trated Loads (lb)	Regular: Lumber Increase=1.25, Plate Increase=1.25 Iniform Loads (plf) Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-12=-10, 8-12=-10, 6-8=-10 Concentrated Loads (lb)							
		egular: Lumber Incre niform Loads (plf) Vert: 1-3=-54 oncentrated Loads (	ease=1.25, Plate Increa: , 3-5=-54, 5-7=-54, 2-12 lb)	=-10, 8-12=-10, 6-8=-10				
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May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

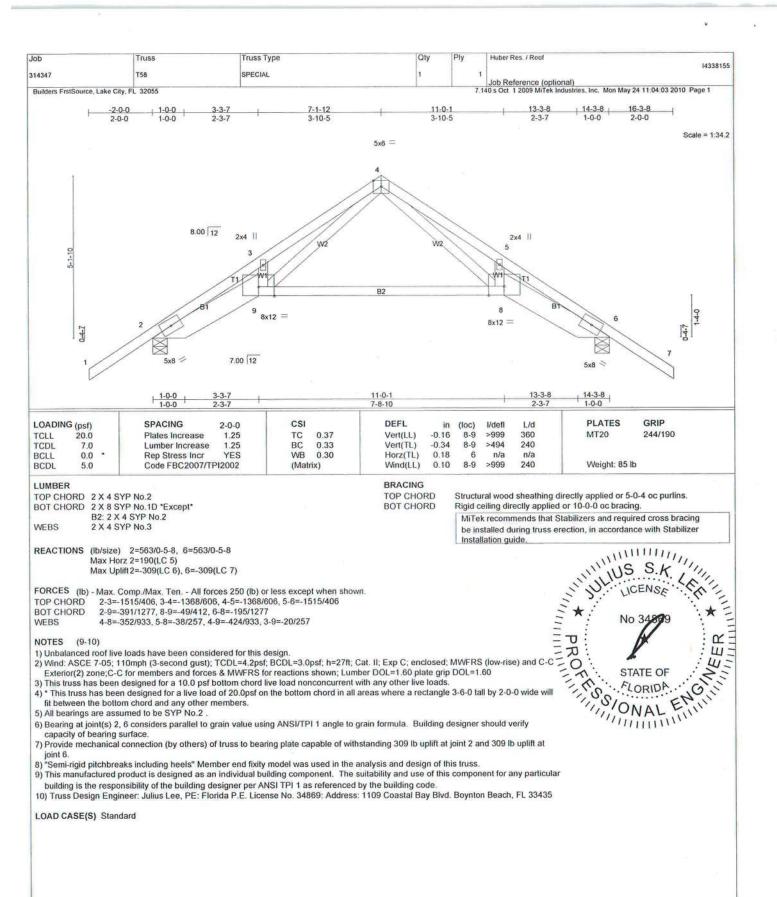
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designers. Reacing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TPI Quality Criteria, DSB-89 and 8CS11 Building Component Safety Information.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

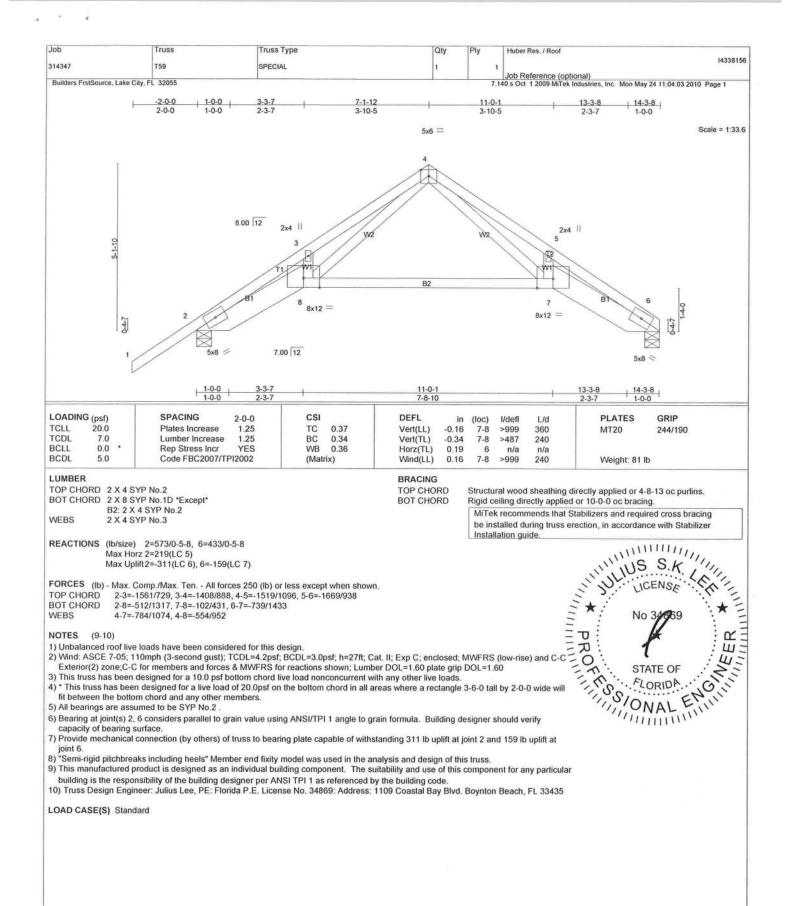
Applicability of design paramenters and proper incarporation of component is responsibility of building designer - not truss designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult. ANSI/ITI Qualify Criteria, DSB-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



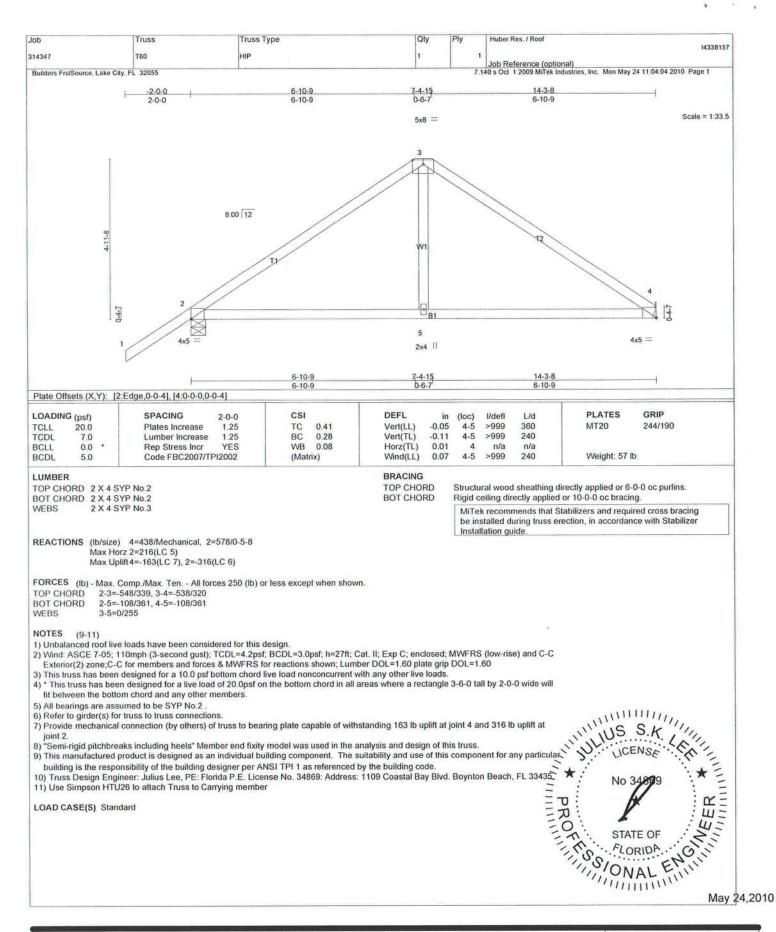
May 24,2010

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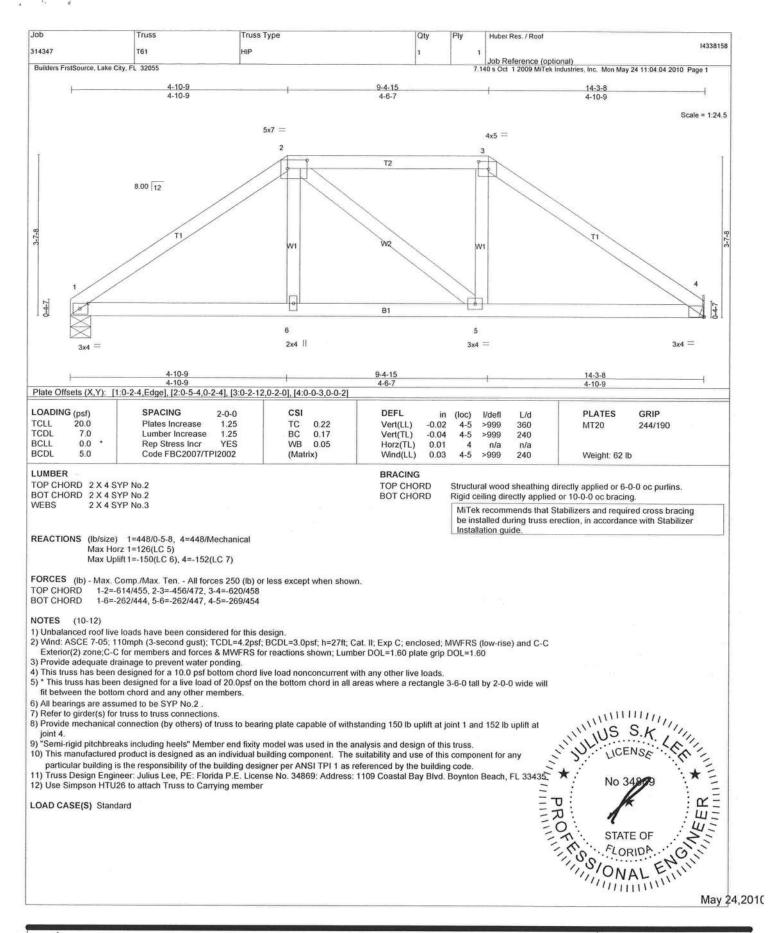
May 24,2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

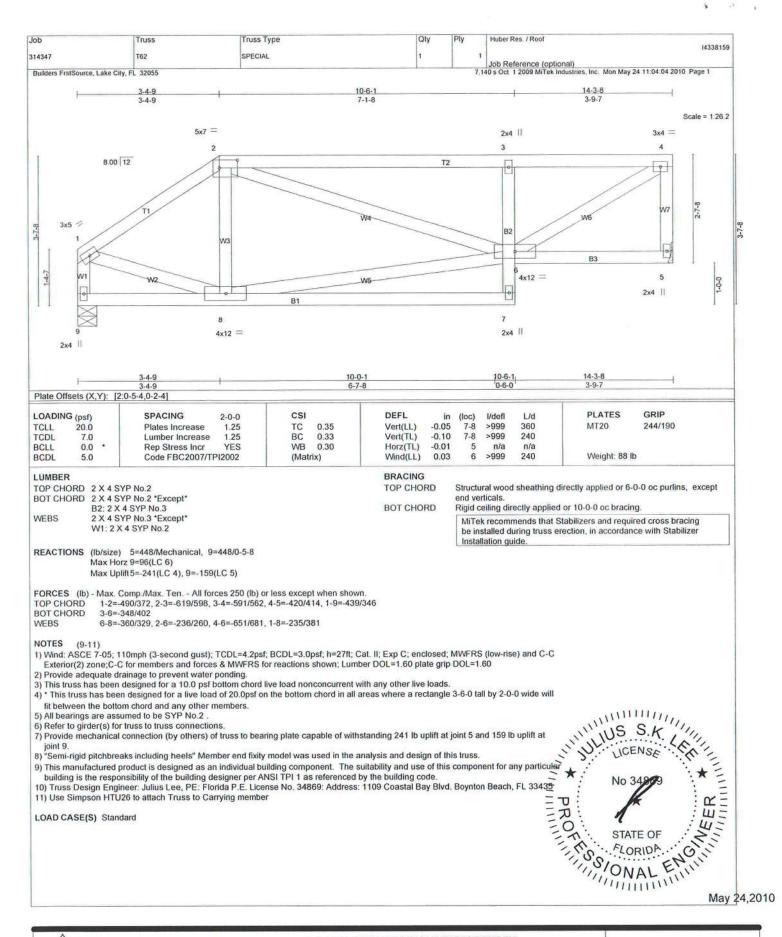
Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, quality control, storage, delivery, erection and bracing, consult MAIS/IPII Quality Criteria, DSB-89 and BCSI1 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only wilth MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Riacing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/IPI Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

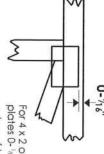
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## Symbols

# PLATE LOCATION AND ORIENTATION



and fully embed teeth. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss Center plate on joint unless x, y offsets are indicated.



plates 0- 1/16" from outside edge of truss. For  $4 \times 2$  orientation, locate

This symbol indicates the required direction of slots in connector plates.

00

0

Cn

Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 × 4

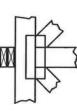
to slots. Second dimension is width measured perpendicular the length parallel to slots. The first dimension is the plate

# LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

## BEARING



(supports) occur. Icons vary but number where bearings occur. reaction section indicates joint

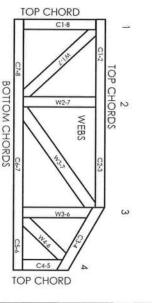
#### ANSI/TPII: Industry Standards:

Plate Connected Wood Truss Construction. National Design Specification for Metal Design Standard for Bracing.

Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling, Building Component Safety Information

# Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A

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## Boynton, FL 33435 Julius Lee 1109 Coastal Bay Blvd.

# **General Safety Notes**

## Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves bracing should be considered. may require bracing, or atternative T, I, or Eliminator

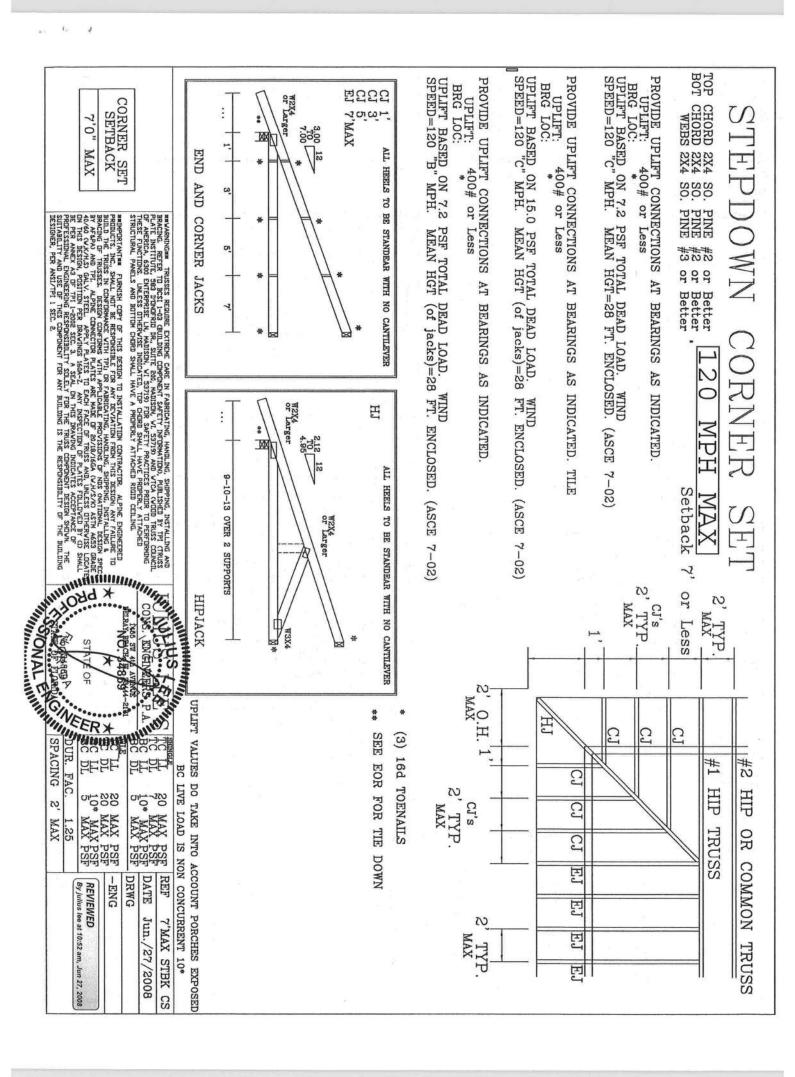
N

- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building
- Cut members to bear tightly against each other

Çn

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- 16. Do not cut or after truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI I Quality Criteria.

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( END, MAX WEB 16 O.C. O.C. O.C. GABLE VERTICAL SPF SPF SPF DFI DFI DFL SP SP SP H ASCE NAOHB STANDARD STANDARD STANDARD STANDARD STANDARD GRADE TANDARD STUD STUD STUD STUD BRACE 7-02: GABLE TEUBS NO 10 130 SPF #/#2, DF-L #2,
SPF #/#2, DR BETTER
DIAGONAL BRACE,
SINGLE OR DOUBLE
CUY (AS SELWA) AT GROUP A Ξ MPH 1X4 °L" BRACE + UPPER END. GROUP B WIND GROUP A GROUP B (1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• SPEED REPER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH 15 THE PROPER 18" 18 GROUP A MEAN PX4 CONTINUOUS GROUP B 10' 2" 0 #EN OR BETTER HEIGHT, SNEVER B (1) 2X6 "L" BRACE \* CONS. 12 4 4 5 10 3 GROUP A 12 5 12 5 13 8 DELEAN HEACH, FL SE444-2161 No: 34869 STATE OF FLORIDA 1 ENGINEERS LEI ENCLOSED, GROUP B 13 5" 12 8" 10 10 1 5 5 5 P.A. (2) ZXB GROUP A 20 N 12 12' 11" r! MAX MAX HRACE GROUP 11 13 11 8 13' 11' 12, 13 **D** 1.00, TOT. SPACING 8 1 E ATTACH EACH "L" HEACE WITH 10A NAIDS.

# POR (1) "L" HEACE, SPACE NAIDS AT 2" O.C.

# POR (2) "L" REACES AND A" O.C. HETMEN ZONES.

## POR (2) "L" REACES: SPACE NAIDS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. HETMEN ZONES. CABLE END SUPPORTS LOAD FROM 4' 0" T. BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. PROVIDE UPLIET CONNECTIONS FUR 136 FLF OVER CONTINUOUS BEARING (6 PSF PC DEAD LOAD). LIVE LOAD DEPLECTION CHIERIA IS L/240. DOUGLAS FIR-LARCH
#3
STUD
STANDARD PLYWOOD OVERHANG. BRACING GROUP SPECIES \$1 / #2 STANDARD VERTICAL LENGIN

IESS THAN 4. D', BUT

LESS THAN 11 B'

GREATER THAN 11 B' EXPOSURE GABLE TRUSS DETAIL SOUTHERN PINE + REFER TO COMMON THUSS DESIGN FOR PEAK SPLICE, AND HEST FLATES. 60 24.0 CABLE VERTICAL PSF DATE REF DRWG GROUP GROUP H & BIR HEM-PIR 0 WITEN SID CABLE 16 E HI DOUGLAS FIR-LARCH PLATE SIZES 11/26/09 ä STUD SUPERIORS ASCB7-02-CAB13015 A: 1X4 OR EX3 HIZK-PIR AND STANDARD NOTES 2.5X4 XX STANDARD GRADES:

GABLE VERTICAL ASCE BRACE 7-02: 130 (1) 1X4 °L" BRACE • (1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• MPH WIND SPEED, 30, MEAN HEIGHT, ENCLOSED, (1) 2X8 "L" BRACE • (2) ZXB H. \_ BRACE II 1.00, EXPOSURE 0

GROUP

A:

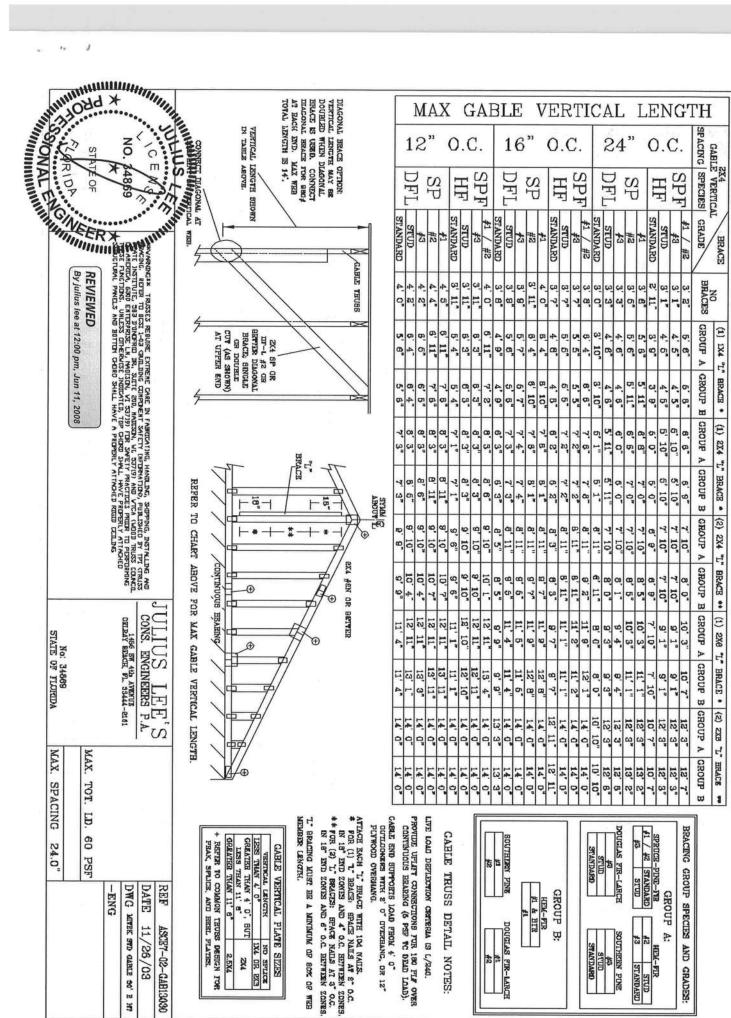
AND

GRADES:

SOUTHERN PINE #3 12

STANDARD

STANDARD



11/26/03 ASCE7-02-CAB13030

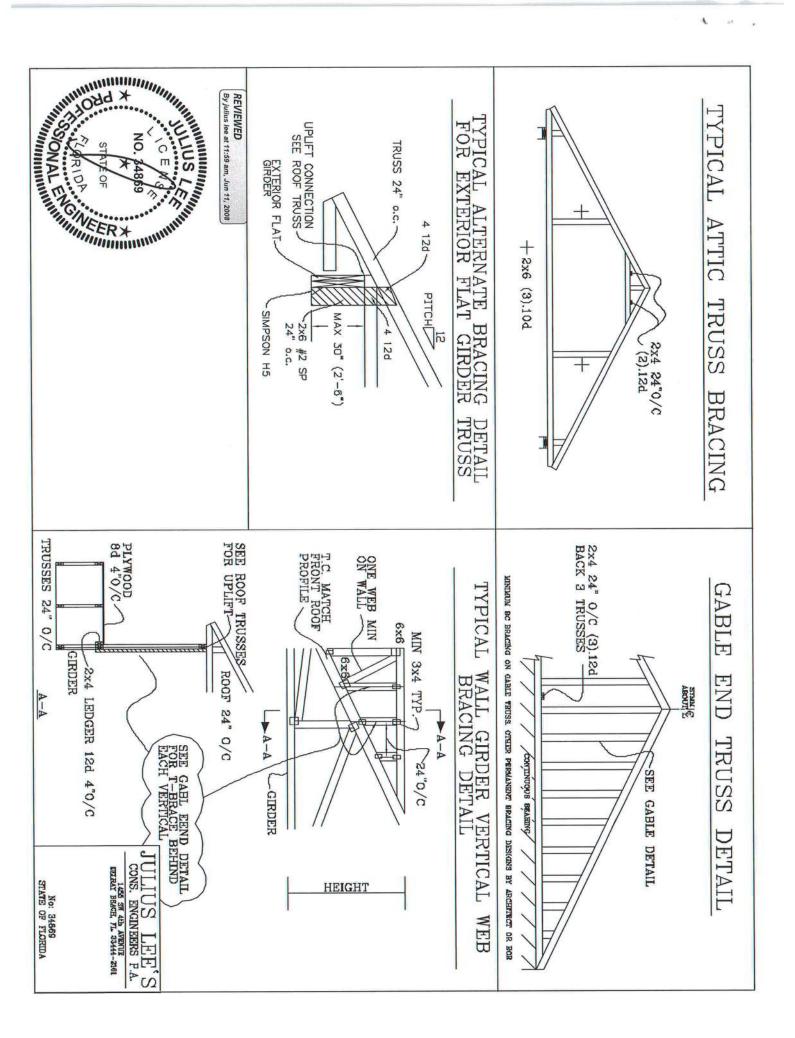
NO SPLICE

2.5X4

22

#1 & BIR GROUP B:

DOUGLAS FIR-LARCH



# PIGGYBACK DETAIL

SPACE PIGGYBACK VERTICALS AT 4' OC MAX. REFER TO SEALED DESIGN FOR DASHED PLATES.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO BUGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS:
110 MPH WIND, 30 MEAN HGT, ASCE 7-02, CLOSED BLDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT L EXP C, WIND TO DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (5,\*) FLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. 110 MPH WIND, 30' MBAN HOT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TO DL-5 PSF WIND BC DL-5 PSF

#2 OR BETTER

LOCATION IS ACCEPTABLE

Ш

D-SPLICE

20' FLAT TOP CHORD MAX SPAN

TYPE Ħ H H 0 > 1.5X3 AXB OR 3X6 TRULOX AT 4' OC, HOTATED VEHTICALLY **5**X4 **4X8** 284 30 2.5X4 1.5X4 SPANS **5X6** 5X8 34 Ą 1.5X4 2.6X4 900 **6X**6 æ 5 1.6X4 **BX**6 **BX**6 3X6 52

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C. WIND TC DL=6 PSF, WIND HC DL=6 PSF ATTACH THULOX PLATES WITH (6) 0.120" X 1.575" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR THULOX

INFORMATION.

	10' TO 14'	7'9" TO 10'	o' To 7'9"	WEB LEN	
	14	10'	.6,	GIB	
* PICCYBACK SPRCIAL PLATE	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMHER, OR HETTER, AND 80% LENGTH OF WEH MEMHER. ATTACH WITH 164 NAILS AT 4" OC.	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR HETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.	NO BRACING	REQUIRED BRACING	WEB BRACING CHART

)	FABRICA (4) 0.11 PIGGYB SPACE	
)	TEETH TO ATTON. ATT 20" X 1.375 ACK SPECIA 4" OC OR I	* F
۰	THE PIGGY TACH TO SUI 5" NAILS PER AL PLATE TO LESS.	PIGGYBACK SPECIAL PLATE
۵	PORTU FACE EACH	PECIA
۰	PER TRUS	PLAT
۰	TIME USS W PLY. FACE	
۵	APPLY APPLY	
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		Y ATTACHED RIGHT CELLING.	ALL HAVE PROPERLY ATTACHED	THANDLING, SHIPPING, INSTALLING AND TRICKS OF INCIDENTIAL CHEEN BY THE CHRUSS OF INCIDENTIAL CHEEN CONTROL OF INCIDENTIAL CHEEN CH		SACK SPECIAL PLATE.
STATE OF FLORIDA			DEARAY BEACH, FL. 33444, 2161	CONS. ENGINEERS P.A.	N, HH I NIII IIII	THIS DRAW
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	1.25 DUR. FAC.	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	NG REPLACES DRAWINGS
		-ENG JL	DRWGMITEK STD PIGGY	DATE 09/12/07	REF PIGGYBACK	THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

## VALLEY TRUSS DETAIL

HOP CHORD CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR 2X4 SP #3 OR BETTER. BETTER

- ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE)
- \* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: FHC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 16' MEAN HEIGHT, ENCLOSED ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENC BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF. (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FOR

UNIESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VALLEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

ENGINEERS' SEALED

DESIGN

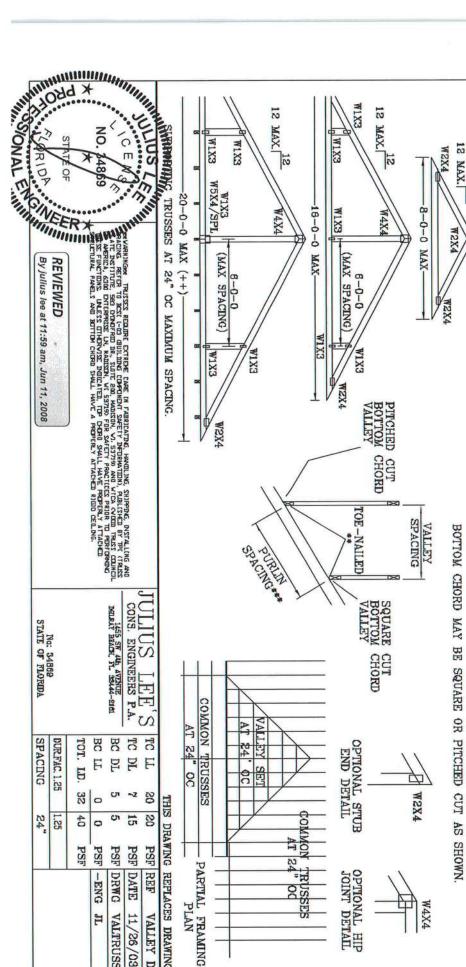
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES

CUT FROM 2X6 OR LARGER AS REQ'D

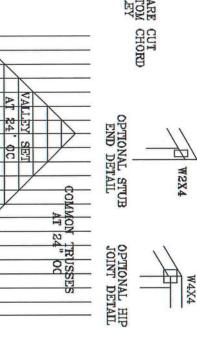
4-0-0

MAX

NOT EXCEED 12'0" BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



\*\*\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.



THIS DRAWING REPLACES DRAWING A105

CONS. ENGINEERS P.A. DELINAY BEACH, I'L 33444-2161 No: 34869 STATE OF FLORIDA BC LE TC DL BC DL SPACING TC DUR.FAC. 1.25 TOT. LD E 32 80 0 20 1.25 40 24 0 PSF PSF REF PSF DRWG PSF DATE -ENG VALTRUSS1103 11/26/09 VALLEY DETAIL

## TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF	SOUTH	SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH	HEM-FIR	-FIR		SPRUCE PINE
TOE-NAILS	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	ES	ES 1 PLY
N	187#	256#	181#	234#	156#	203#	3#	3# 154#
ω	296#	383#	271#	351#	234#	ယ္အ	304#	04# 230#
4	394#	511#	361#	468#	312#	4.	406#	06# 307#
Ó	493#	639#	452#	585#	390#	rn.	507#	384#

NO. 44869

NO. 44869 1/8" JACK 30° GIRDER OPTIONAL (2) PLY 30°-60° JACK ALTERNATIVE CONDITION OPTIONAL GIRDER (2) PLY

SIHL
DRAWING
REPLACES
DRAWING
784040

			D RIGID CEILING	VTCA (WOOD TRUSS COUNCE.		<u> </u>
STATE OF FLORIDA	No: 34889			DELRAY BEACH, FL SO444-2161	CONS. ENGINEERS P.A.	S, HH'I SNI'INI
SPA	DUF	TOT	BC LL	BC DL	TC	TC LL
SPACING	DUR. FAC.	TOT. LD.	F 	DL	DE	LL
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL	DRWG	DATE	PSF REF
			IL	CNTONAIL1103	09/12/07	TOE-NAIL

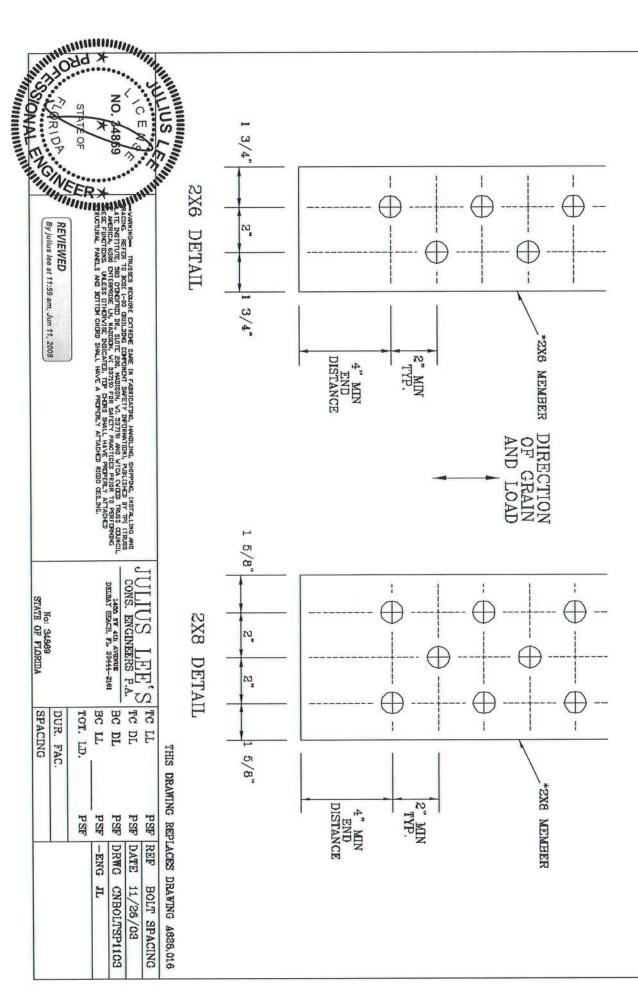
## DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW. APPLIED

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



CONS.

DELRAY SEACH, FL 33444-2161

BC LL

PSF

-ENG

T

BC TC

PSF PSF

DRWG DATE

CNBOLTSP1103 11/26/03

No: 34869 STATE OF FLORIDA

SPACING DUR. FAC. TOT. LD.

# TRULOX CONNECTION DETAIL

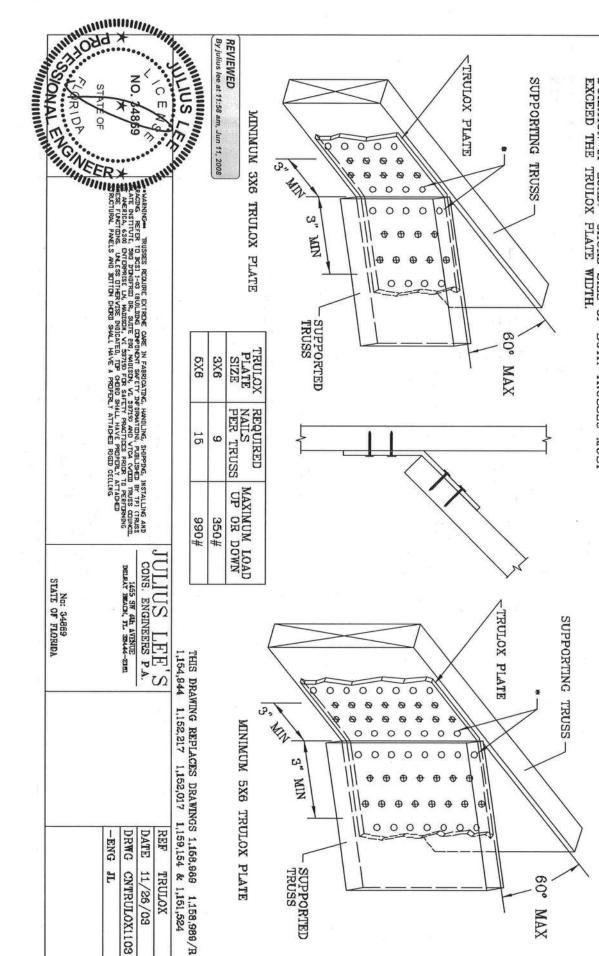
II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (+).

\* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS—FIR OR HEM—FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE—FINE—FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



#### MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

#### Maximum Uniform Load Applied to Either Outside Member (PLF)

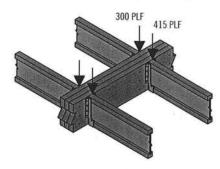
					C	onnector Pattern		
Connector Type	Number of Rows	Connector On-Center Spacing	Assembly A	Assembly B	Assembly C	Assembly D	Assembly E  1  2  1  2  1  2  1  1  1  1  1  1  1	Assembly F
			1-134" 31/2" 2-ply	51/4" 3-ply	1¼" 3½" 5¼" 2-ply	134" 3½" 134"  7" 3-ply	7" 2-ply	-   -   134"   7"   4-ply
10d (0.128" x 3")	2	12"	370	280	280	245		- 17 15 to 12 - 13
Nail®	3	12"	555	415	415	370	REPUBLISHED TO THE	CHARLES OF THE PARTY.
1/11/4/007		24"	505	380	520	465	860	340
1/2" A307 hrough Bolts(2)(4)	2	19.2"	635	475	655	580	1,075	425
modgii botta		16"	760	570	785	695	1,290	505
		24"	680	510	510	455		Cara Constitution
SDS 1/4" x 31/2"(4)	2	19.2"	850	640	640	565		4 1352
		16"	1,020	765	765	680		
		24"				455	465	455
SDS 1/4" x 6"(3)(4)	2	19.2"				565	580	565
		16"				680	695	680
		24"	480	360	360	320		
USP WS35 (4)	2	19.2"	600	450	450	400		
		16°	715	540	540	480	HE MARKET THE SECOND	
		24"				350	525	350
USP WS6 (3)(4)	2	19.2"				440	660	440
		16"				525	790	525
33/8"		24"	635	475	475	425		
TrussLok(4)	2	19.2"	795	595	595	530		
		16"	955	715	715	635		
5"		24"		500	500	445	480	445
TrussLok(4)	2	19.2"		625	625	555	600	555
		16"		750	750	665	725	665
63/4"		24"	STATISTICS OF SERVICE			445	620	445
TrussLok(4)	2	19.2"		CANCEN STREET	THE RESERVE TO SERVE THE RESERVE THE RE	555	770	555
	SHEET SHEET	16"				665	925	665

Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

#### **General Notes**

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
   Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
  of the members (to minimize rotation).
- · Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

#### **Uniform Load Design Example**



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

#### Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.

<sup>(2)</sup> Washers required. Bolt holes to be %6" maximum.

<sup>(3) 6&</sup>quot; SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

<sup>(4) 24&</sup>quot; on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

#### MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

#### Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

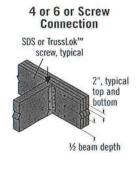
			高级长沙鸡	Ci	onnector Pattern	10 m	
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
Connector Type	Number of Connectors	2° 1 2° 1 34°	134"	11/4" 31/2"	134" 31½" 134"	1 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	To appropriate the second seco
		31/i" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
	6	1,110	835	835	740		
Od (0.128" x 3")	12	2,225	1,670	1,670	1,485		CREATING THE
Nail	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		Miles State State
SDS Screws	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)
" x 31/2" or WS35	6	2,870	2,150 (4)	2,150	1,915	2,785(2)	2,110(2)
" x 6" or WS6(1)	8	3,825	2,870 (4)	2,870	2,550	3,715(2)	2,810(2)
2368 av EU	4	2,545	1,910 (4)	1,910	1,695	1,925(3)	1,775(3)
3³/8" or 5" TrussLok™	6	3,815	2,860 (4)	2,860	2,545	2,890(3)	2,665(3)
	8	5,090	3,815 (4)	3,815	3,390	3,855(3)	3,550(3)

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

- (2) 6" long screws required.
- (3) 5" long screws required.
- (4) 3½" and 35%" long screws must be installed on both sides.

#### Connections

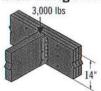


#### 8 Screw Connection SDS or TrussLok™ screw, typical 2" Equal spacing 2"

# Nail Connection 10d (0.128" x 3") nails, typical. Stagger to prevent splitting. 8"-10" 2" spacing, typical 1½" minimum spacing, typical

#### There must be an equal number of nails on each side of the connection

#### Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3¾" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

#### MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

#### 13/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3¾" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed
- on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

#### 31/2" Wide Pieces

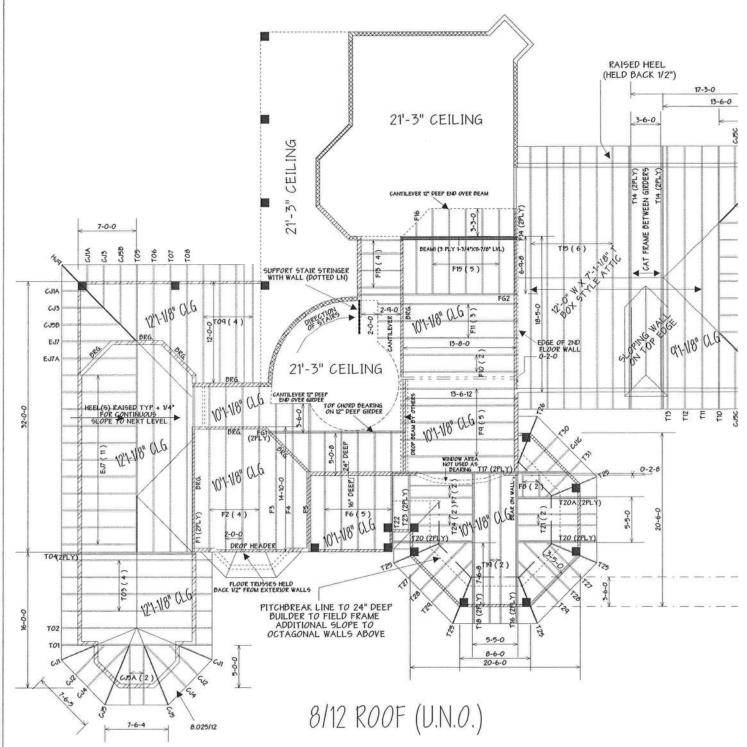
- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

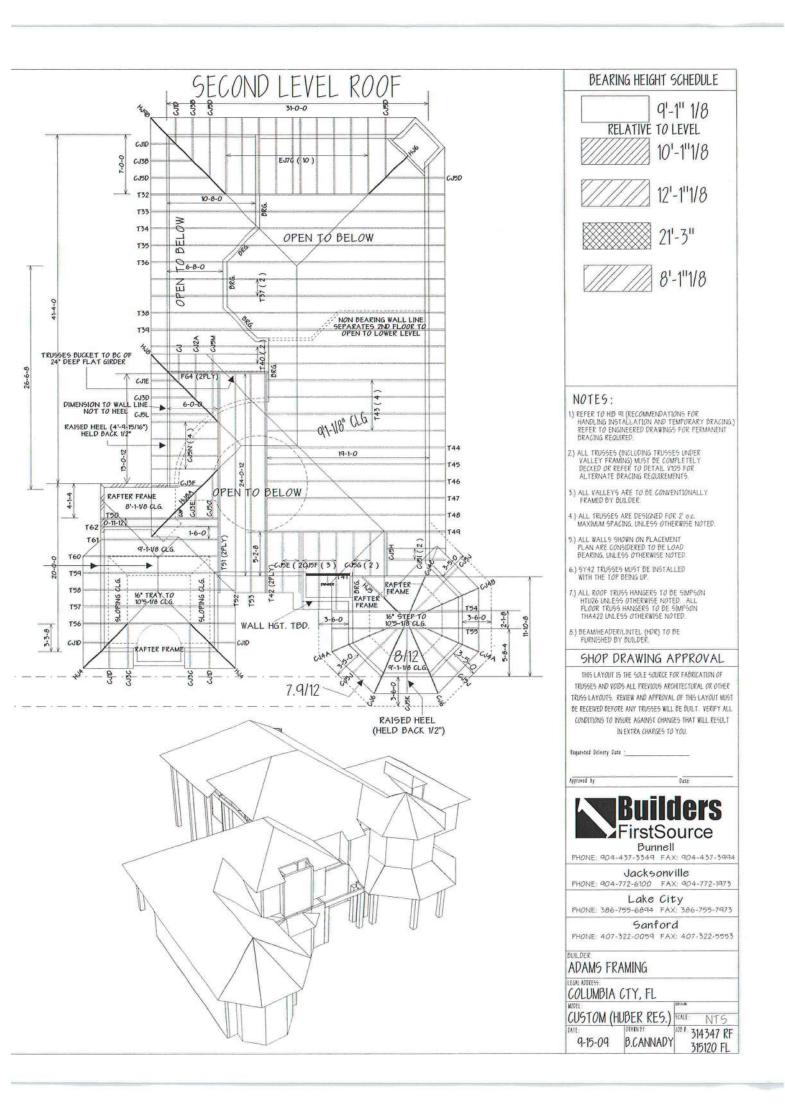


#### FIRST LEVEL ROOF AND FLOOR



1 QUANT. 3 PLY 1-3/4" X 11-7/8" LVL SUPPLI ALL OTHER HEADERS/BEAMS/VALLEY MAT. SUPPLIED BY OTHERS

FLOOR TRUSSES 24" DEEP 24" O.C.



#### **Julius Lee**

RE: 315120 - HUBER RES. / FLOOR

#### 1109 Coastal Bay Blvd. **Boynton Beach, FL 33435**

Site Information:

Project Customer: ADAM'S FRAMING Project Name: 315120 Model: HUBER RES.

Lot/Block:

Subdivision:

Address: 412 SW HIGHPOINT GLENN

City: COLUMBIA CTY

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: ADAM R. PAPKA

License #: CBC1253409

Address: P.O. BOX 1921

City: LAKE CITY,

State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002

Design Program: MiTek 20/20 7.1

Wind Code: ASCE 7-05 Wind Speed: 110 mph

Floor Load: N/A psf

Roof Load: 32.0 psf

This package includes 35 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	14337625	CJ1C	5/24/010	18	14337642	FG2	5/24/010
2	14337626	F1	5/24/010	19	14337643	T16	5/24/010
3	14337627	F10	5/24/010	20	14337644	T17	5/24/010
4	14337628	F11	5/24/010	21	14337645	T18	5/24/010
5	14337629	F13	5/24/010	22	14337646	T19	5/24/010
6	14337630	F14	5/24/010	23	14337647	T20	5/24/010
7	14337631	F15	5/24/010	24	14337648	T20A	5/24/010
8	14337632	F16	5/24/010	25	14337649	T21	5/24/010
9	14337633	F2	5/24/010	26	14337650	T22	5/24/010
10	14337634	F3	5/24/010	27	14337651	T23	5/24/010
11	14337635	F4	5/24/010	28	14337652	T24	5/24/010
12	14337636	F5	5/24/010	29	14337653	T25	5/24/010
13	14337637	F6	5/24/010	30	14337654	T26	5/24/010
14	14337638	F7	5/24/010	31	14337655	T27	5/24/010
15	14337639	F8	5/24/010	32	14337656	T28	5/24/010
16	14337640	F9	5/24/010	33	14337657	T29	5/24/010
17	14337641	FG1	5/24/010	34	14337658	T30	5/24/010

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

ONAL May 24,2010

1 of 4

Julius Lee

RE: 315120 - HUBER RES. / FLOOR

Site Information:

Project Customer: ADAM'S FRAMING Project Name: 315120 Model: HUBER RES.

Subdivision:

Lot/Block: Address: 412 SW HIGHPOINT GLENN

City: COLUMBIA CTY

State: FL

No.	Seal#	Truss Name	Date
35	14337659	T31	5/24/010

Job Truss Truss Type Qty HUBER RES. / FLOOR 14337625 315120 CJ1C JACK Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 08:42:19 2010 Page 1 Builders FrstSource, Lake City, FL 32055 2-0-0 Scale = 1:15.0 8.00 12 0-4-11 LOADING (psf) SPACING CSI DEFL PLATES GRIP 2-0-0 I/defl 1 /d TCLL 20.0 Plates Increase 1.25 TC 0.39 Vert(LL) -0.00 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1 25 BC 0.04 Vert(TL) -0.00 2 >999 240 BCLL 0.0 Rep Stress Incr WB YES 0.00 Horz(TL) 0.00 n/a n/a BCDL 5.0 Code FBC2007/TPI2002 (Matrix) Wind(LL) 0.00 >999 240 Weight: 10 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 1-9-2 oc purlins.

BOT CHORD

REACTIONS (lb/size) 2=229/0-3-8, 4=9/Mechanical, 3=-18/Mechanical Max Horz 2=190(LC 6) Max Uplift 2=-343(LC 6), 4=-30(LC 4), 3=-18(LC 1)

Max Grav 2=229(LC 1), 4=26(LC 2), 3=62(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2 X 4 SYP No.2

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No.2

5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 343 lb uplift at joint 2, 30 lb uplift at joint 4 and 18 lb uplift at joint 3.

"Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

STATE OF

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to lavers stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobstaction, qualify control, storage, delivery, erection and bracing. AMSI/TPI Quality Citleria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

OTHERS 2 X 4 SYP No.3

REACTIONS All bearings 14-9-8 except (jt=length) 14=0-5-0.

(lb) - Max Uplift All uplift 100 lb or less at joint(s) except 16=-166(LC 1)

Max Grav All reactions 250 lb or less at joint(s) 26, 14, 25, 24, 23, 22, 21, 20, 19, 18, 17, 16 except 15=664(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 12-15=-588/0

2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply
  connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
  5) Gable studs spaced at 1-4-0 oc.

6) All bearings are assumed to be SYP No.2 .

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 166 lb uplift at joint 16.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

  9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular
- building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34969

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May 24,2010

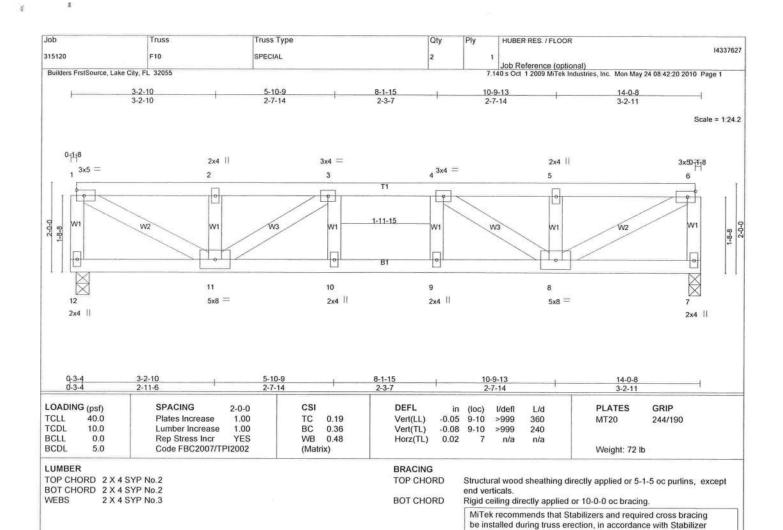
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

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AMSI/TEL Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI, 53719.



Installation guide.

REACTIONS (lb/size) 12=756/0-3-8, 7=756/0-3-0

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-12=-729/0, 1-2=-1085/0, 2-3=-1085/0, 3-4=-1492/0, 4-5=-1086/0, 5-6=-1086/0,

6-7=-729/0

BOT CHORD 10-11=0/1492, 9-10=0/1492, 8-9=0/1492

WEBS 1-11=0/1201, 2-11=-307/0, 3-11=-483/0, 4-8=-483/0, 5-8=-307/0, 6-8=0/1201

NOTES (4-5)

1) Provide adequate drainage to prevent water ponding.

2) All bearings are assumed to be SYP No.2

3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

 This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34869

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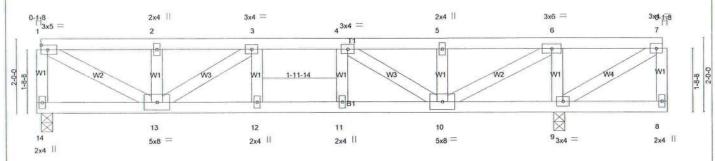
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May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fluss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, qualify control, storage, delivery, erection and bracing, consult. AMSI/ITI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madison. WI 53719.



0-3-4 0-3-4	3-2-10 5-10-9 2-11-6 2-7-15		8-1-15 10-9-13 2-3-6 2-7-14		13-10-11 3-0-14			16-10-0 2-11-5			
LOADING (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Incre		TC	0.21	Vert(LL)	-0.05	12	>999	360	MT20	244/190
TCDL 10.0	Lumber Incr	ease 1.00	BC	0.35	Vert(TL)	-0.08	12	>999	240		
BCLL 0.0	Rep Stress	Incr YES	WB	0.48	Horz(TL)	0.02	9	n/a	n/a	schedungended (Chephine	
BCDL 5.0	Code FBC2	007/TPI2002	(Matri	x)	100000000000000000000000000000000000000					Weight: 86 lb	)

LUMBER TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No 3 WEBS

BRACING

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 5-4-3 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 9-10.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=725/0-3-8, 9=1095/0-3-8

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-14=-699/0, 1-2=-1033/0, 2-3=-1033/0, 3-4=-1367/0, 4-5=-874/0, 5-6=-874/0

BOT CHORD 12-13=0/1367, 11-12=0/1367, 10-11=0/1367 WEBS

1-13=0/1143, 2-13=-317/0, 3-13=-397/0, 4-10=-586/0, 5-10=-263/0, 6-10=0/1193,

6-9=-927/0

NOTES

1) Provide adequate drainage to prevent water ponding.
2) All bearings are assumed to be SYP No.2.

 3) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 4) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

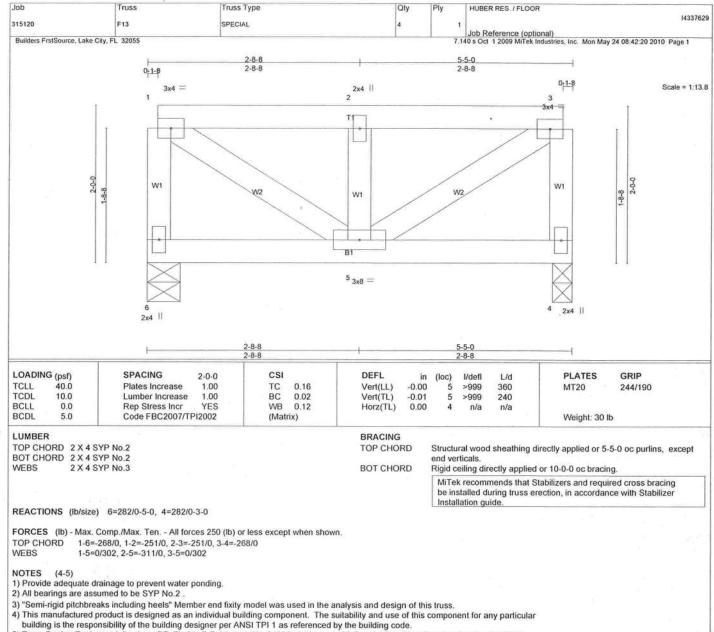
LOAD CASE(S) Standard

ONEF

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fluss designer. Bracing shown is for lateral support of individual use members only. Additional temporary bracing to instability of unique construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labitication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, DS8-89 and BCS11 Building Component Salety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34069

No 34069

STATE OF

FLORIDA

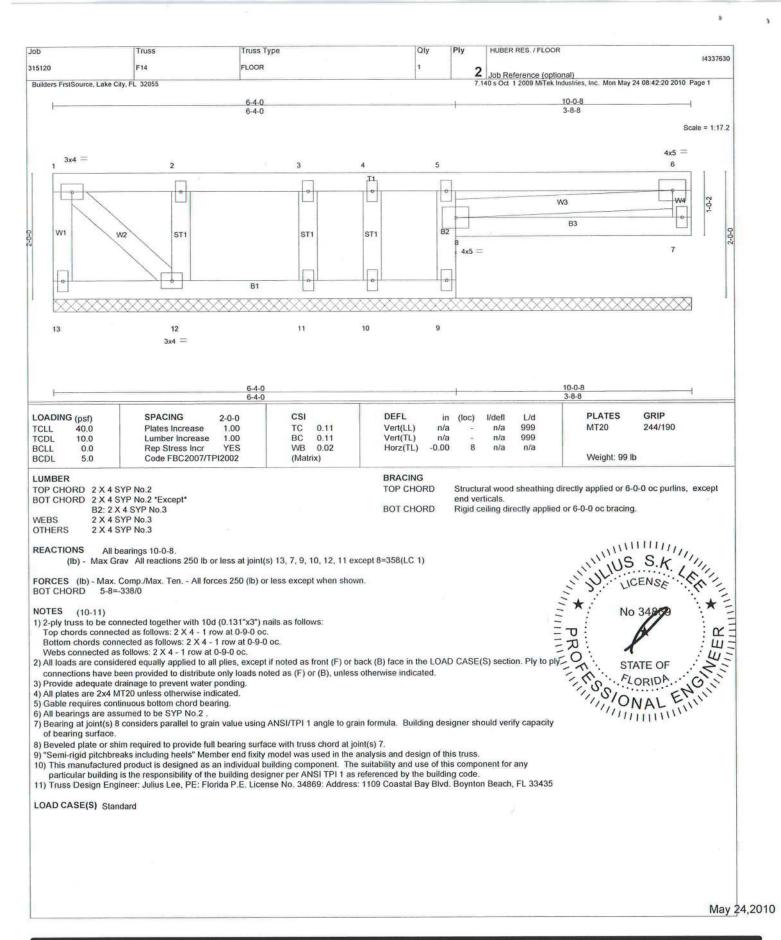
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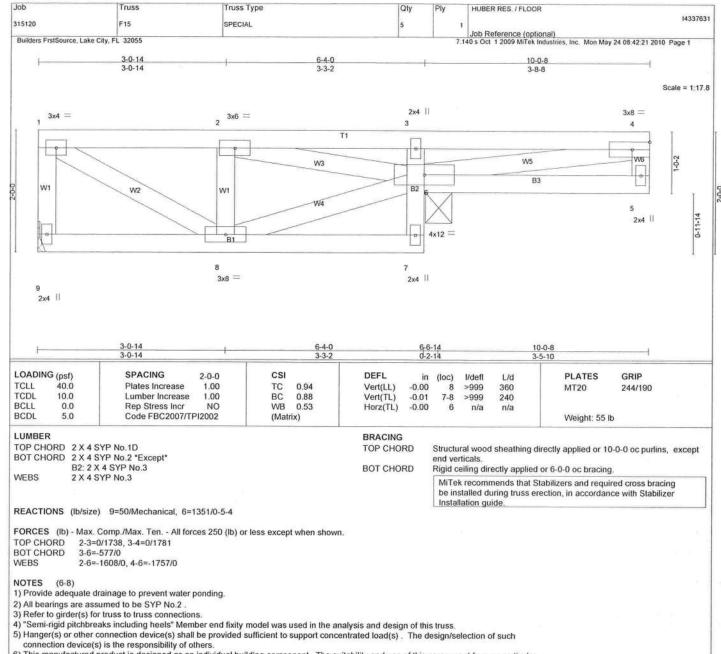
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Miflek connectors. This design is based only upon parameters shown, and is for an individual building component. 
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6) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Regular: Lumber Increase=1.00, Plate Increase=1.00
 Uniform Loads (plf)

Vert: 1-3=-100, 3-4=-140, 7-9=-10, 5-6=-10 Concentrated Loads (lb)

Vert: 4=-181

No 34899

No 34899

STATE OF

FLORIDA

May

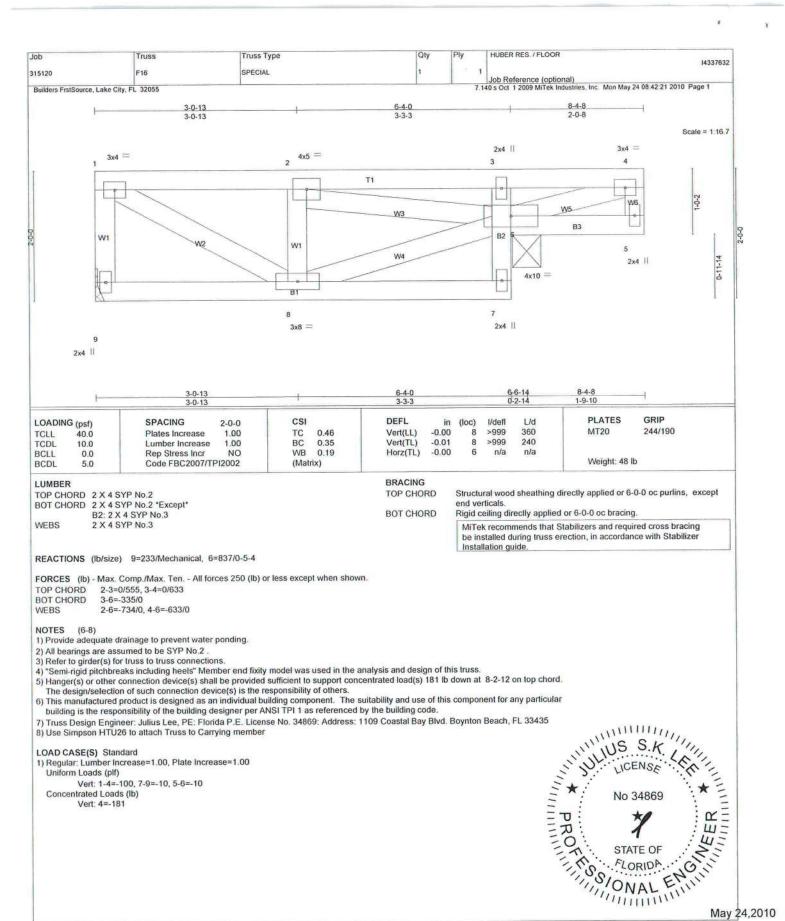
May 24,2010

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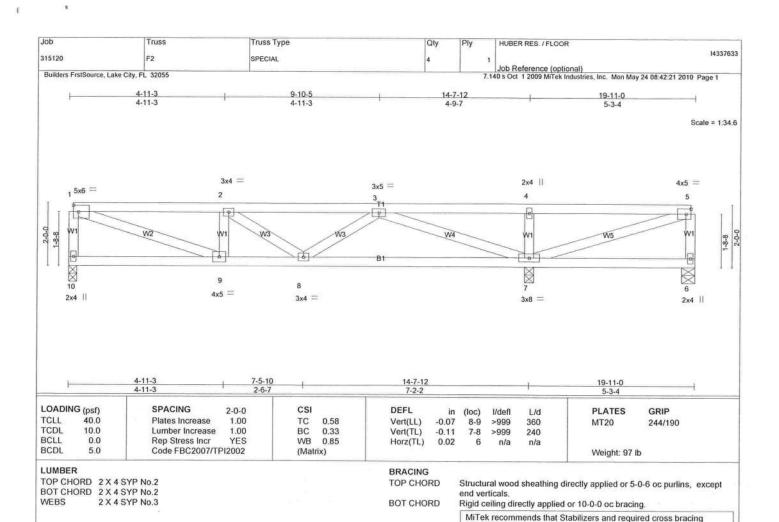
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REACTIONS (lb/size) 10=703/0-3-0, 6=14/0-5-0, 7=1442/0-3-8

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-10=-668/0, 1-2=-1340/0, 2-3=-1215/0, 3-4=0/603, 4-5=0/603

BOT CHORD 8-9=0/1340, 7-8=0/1026

1-9=0/1350, 3-7=-1729/0, 4-7=-563/0, 5-7=-682/0, 2-9=-439/0 WEBS

NOTES (4-5)

1) Provide adequate drainage to prevent water ponding.

All bearings are assumed to be SYP No.2.
 "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

4) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34899

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be installed during truss erection, in accordance with Stabilizer

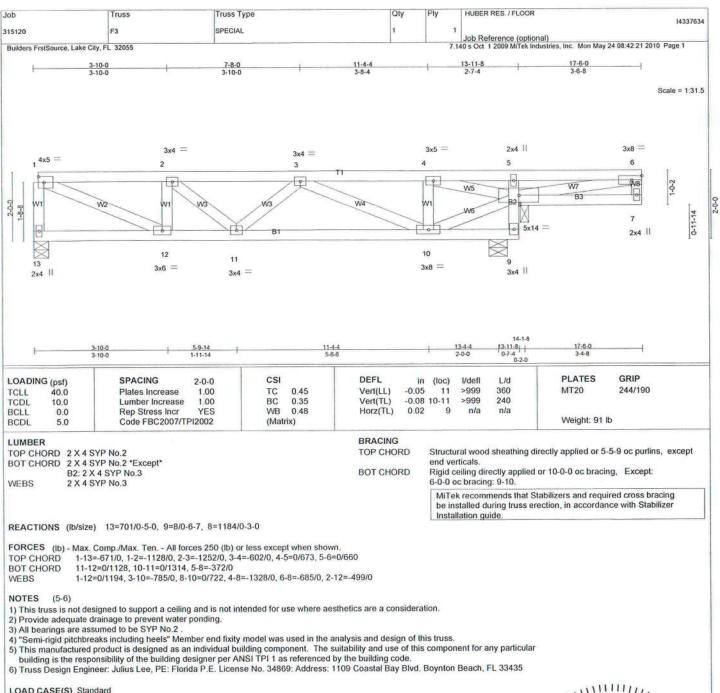
Installation guide

May 24,2010

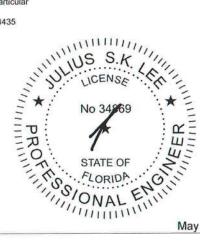
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LOAD CASE(S) Standard



May 24,2010

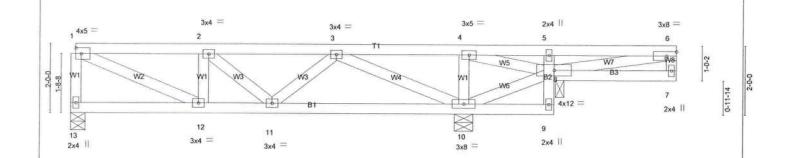
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available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Truss Type Qty HUBER RES. / FLOOR 14337635 315120 F4 SPECIAL Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 08:42:21 2010 Page 1 Builders FrstSource, Lake City, FL 32055 3-10-0 7-8-0 11-4-4 13-11-8 17-6-0 3-10-0 3-10-0 3-8-4 Scale = 1:31.5



-	3-10-0 3-10-0	1	5-9-14 1-11-14			11-4-4 5-6-6		-	13-11-8 2-7-4	14-1-8 0-2-0	17-6-0 3-4-8	
LOADING (psi TCLL 40. TCDL 10. BCLL 0.	0	SPACING Plates Increase Lumber Increase Rep Stress Incr	2-0-0 1.00 1.00 YES	CSI TC BC WB	0.40 0.31 0.41	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.02 -0.04 0.01	(loc) 11 11 10	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 5.	0	Code FBC2007/TF	P12002	(Matr	ix)					01000	Weight: 91 I	b

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

B2: 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except

end verticals

Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS (lb/size) 13=513/0-5-0, 8=334/0-3-0, 10=1046/0-6-7

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

BOT CHORD

1-13=-486/0, 1-2=-733/0, 2-3=-627/0, 3-4=0/595, 4-5=0/746, 5-6=0/674

11-12=0/733, 10-11=0/488, 5-8=-336/0

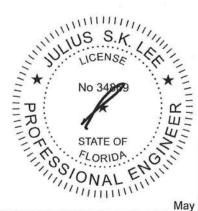
WEBS

1-12=0/765, 3-10=-1194/0, 4-10=-265/0, 8-10=-558/0, 6-8=-702/0, 2-12=-309/0

## NOTES (4-5)

- 1) Provide adequate drainage to prevent water ponding.
- 2) All bearings are assumed to be SYP No.2
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
   This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
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LOAD CASE(S) Standard

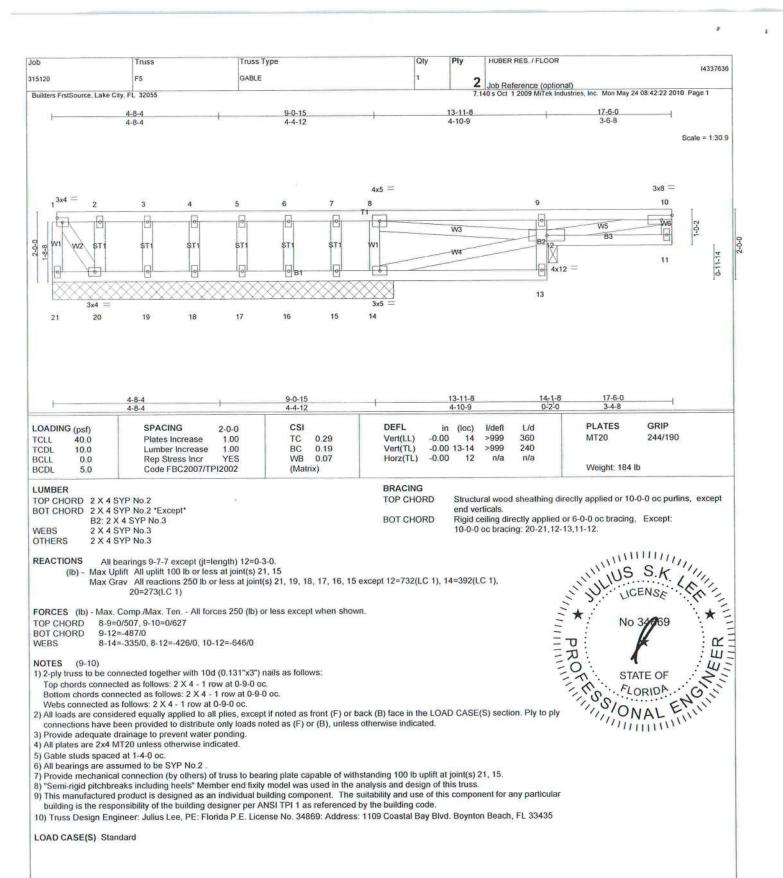


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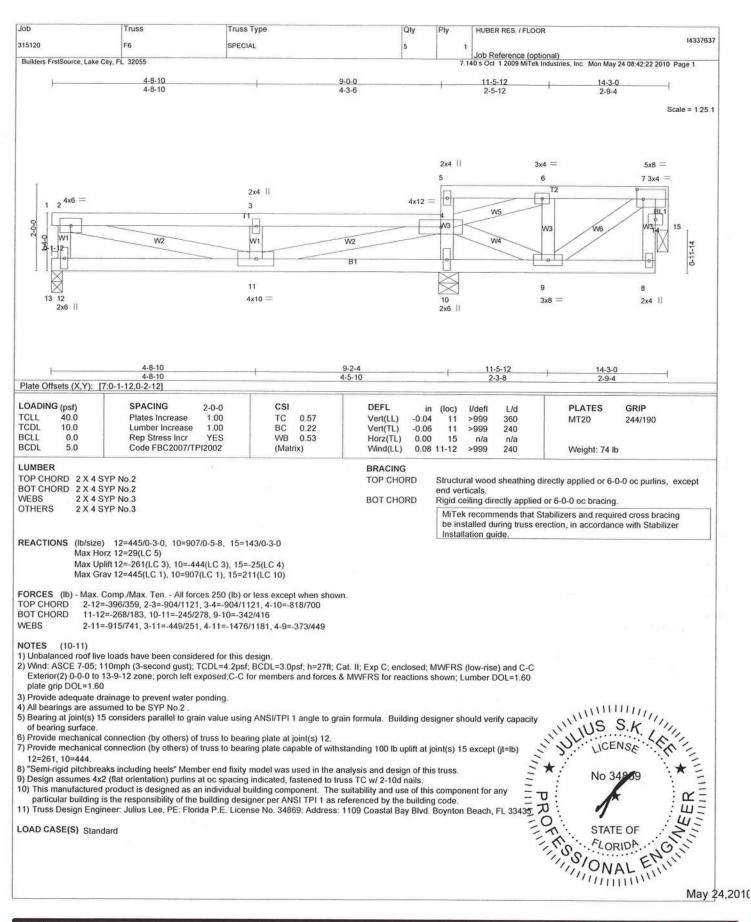
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May 24,2010

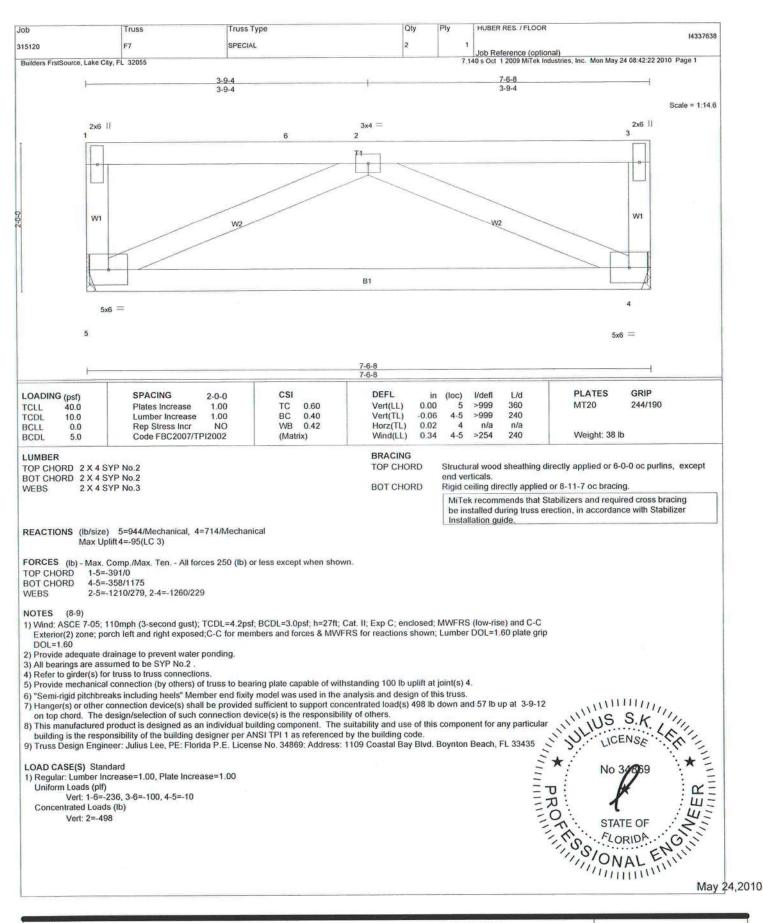
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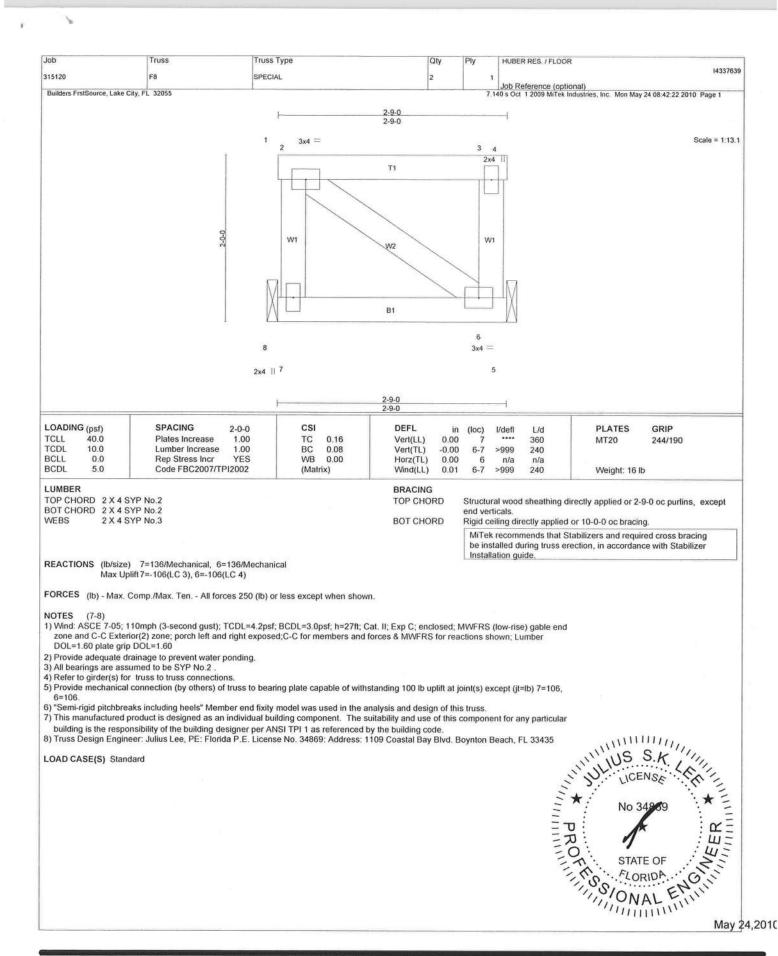


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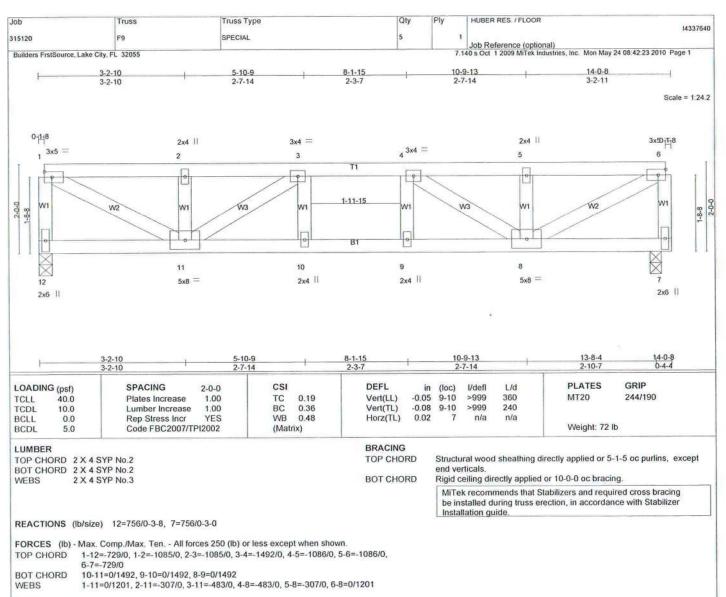
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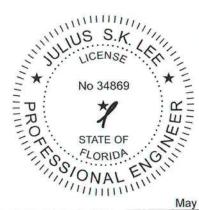
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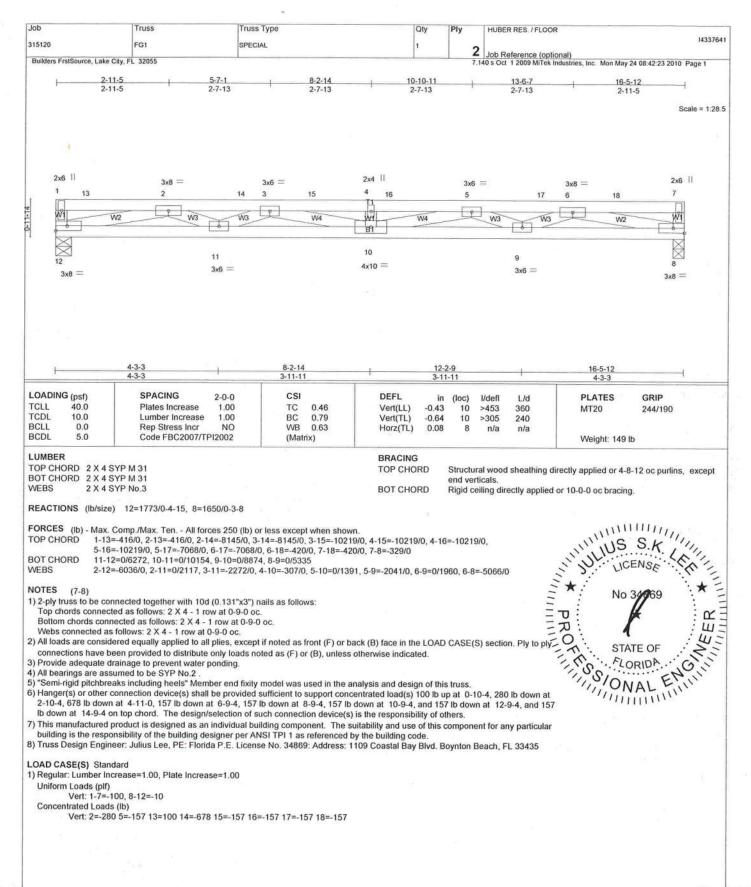
LOAD CASE(S) Standard



May 24,2010

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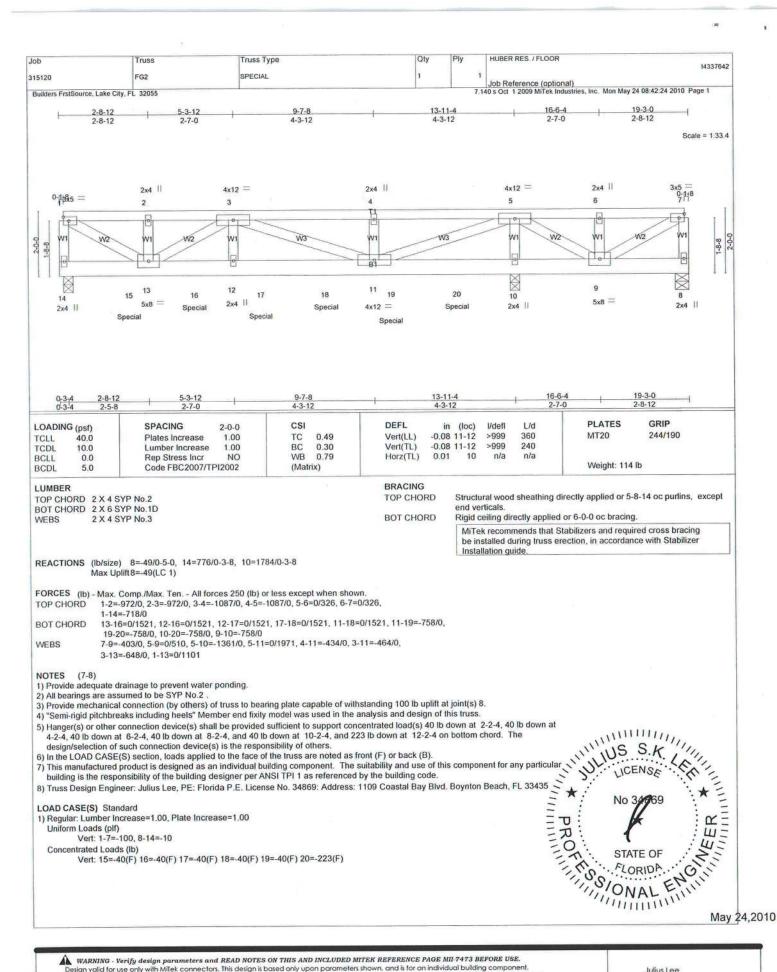
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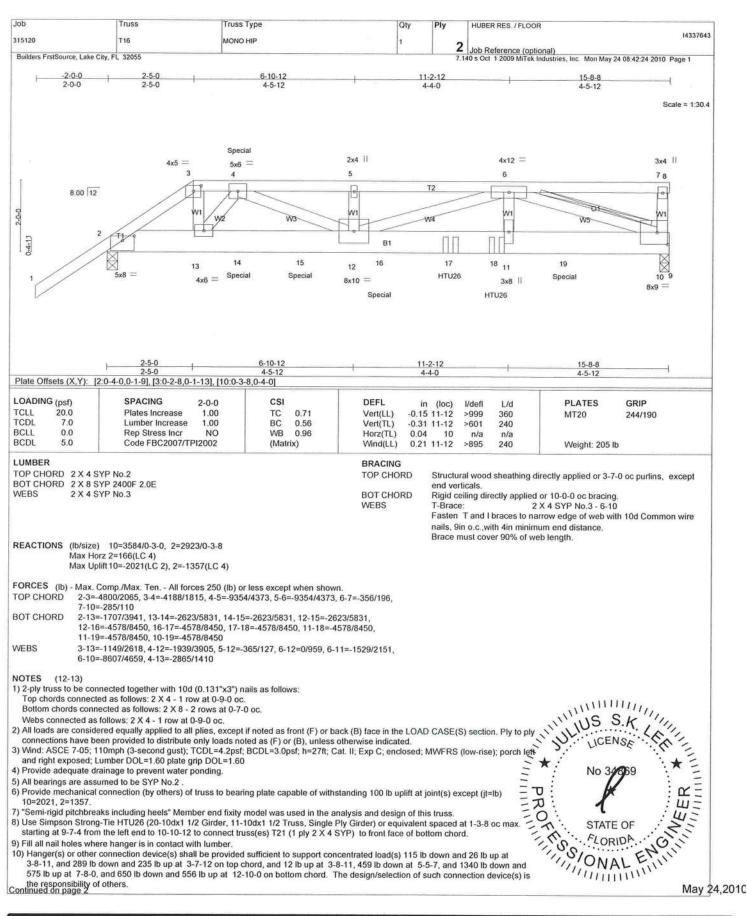


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ob	Truss	Truss Type	Qty	Ply	HUBER RES. / FLOOR	14337643
15120	T16	MONO HIP	1	2	Joh Reference (antional)	1403/043
Builders FrstSource, Lake City	, FL 32055			7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon Mi	ay 24 08:42:24 2010 Page 2
<ol> <li>This manufactured p building designer pe</li> </ol>	product is designed as a or ANSI TPI 1 as referer	bracing for truss system (not part of this in individual building component. The sized by the building code. inda P.E. License No. 34869; Address: 1	uitability and use of t	his compon	ent for any particular building is the re	esponsibility of the
Concentrated Loads	ease=1.00, Plate Increa , 3-4=-54, 4-7=-109, 7- (lb)		(F) 19=-650(F)			
					,,,,,	AUTHI),
					* HILLIAM	S S.K. LICENSE No 34869
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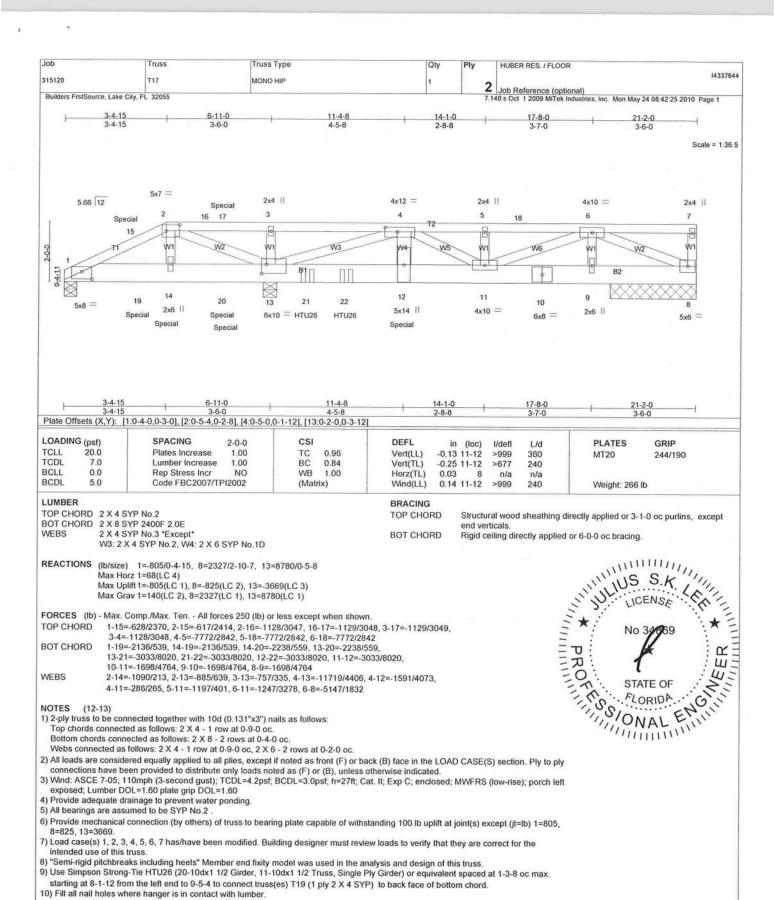
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available from Truss Plate Institute, SS3 D'Onotrio Drive, Madison, WI S3719.

Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

May 24,2010



Continued on page 2

May 24,2010

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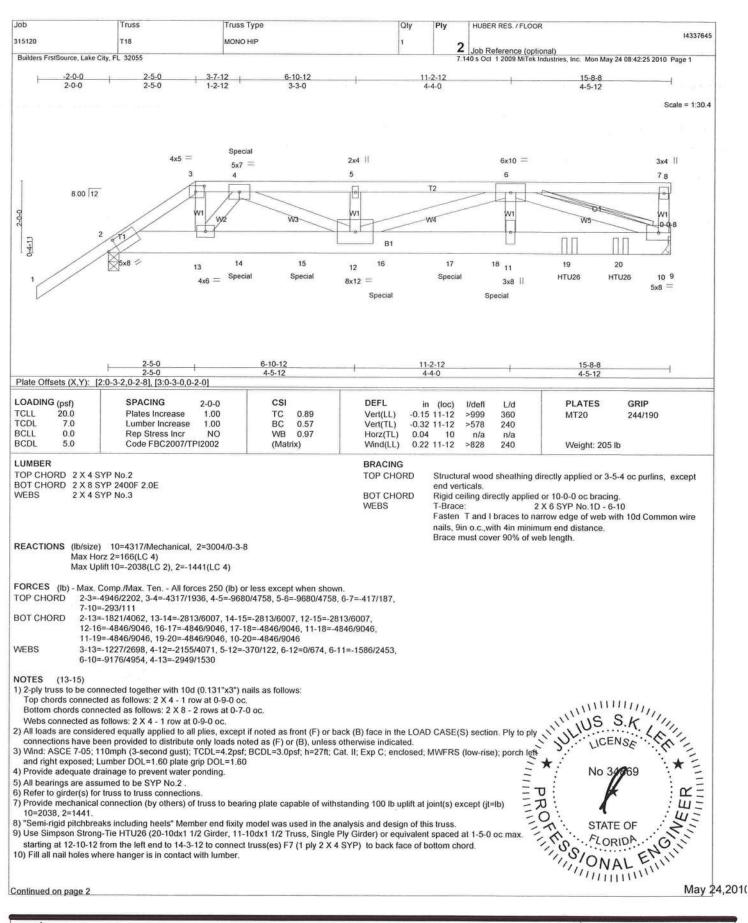
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	Truss	Truss Type	Qty	Ply	HUBER RES. / FLOOR	143376
20	T17	MONO HIP	1	2	Inh Deference (actions)	143370
ders FrstSource, Lake City,	FL 32055			7.1	Job Reference (optional) 40 s Oct 1 2009 MiTek Industries, Inc. Mon	May 24 08:42:25 2010 Page 2
Ib up at 4-8-12 and 1	014 lb down and 417 lb	up at 15-2-12, and 68 lb down a	t 15-2-12 on top chord, a	nd 36 lb up	19 lb up at 2-5-14, 24 lb up at 5-3 eat 2-5-14, 126 lb down and 112 lb oottom chord. The design/selection	up at 3-5-12, 34 lb down
device(s) is the respo This manufactured pr building designer per	nsibility of others. oduct is designed as an i ANSI TPI 1 as reference	ndividual building component. T	he suitability and use of the	nis compon	ent for any particular building is the	
AD CASE(S) Standar						
Vert: 1-2=-54, Concentrated Loads (II		=-10 ) 16=-530 17=24(F) 18=-1082 1	9-1/E) 20- 161/E- 34 B:	126) 21=	1216(R) 22=-1216(R)	
MWFRS Wind Left: Lu Uniform Loads (plf)	mber Increase=1.60, Pla	te Increase=1.60	9-1(F) 20101(F34, B-	-120) 21	1210(0) 221210(0)	
Horz: 1-2=-20 Drag: 2-3=-0	!-16=36, 7-16=33, 1-13=	20, 0-13=-6				
		16=244 17=1(F) 18=333 19=36(I late Increase=1.60	F) 20=206(F=93, B=112) 2	21=480(B)	22=480(B)	
Jniform Loads (plf)	2-16=36, 7-16=33, 1-13=					
Drag: 2-3=-0 Concentrated Loads (I	0) 0) 12-1722/B) 15- 16/E	) 16=244 17=1(F) 18=333 19=36	(E) 20=206(E=93 R=112)	21=480/B	) 22=480(B)	
MWFRS 1st Wind Para Uniform Loads (plf)	allel: Lumber Increase=1	.60, Plate Increase=1.60	(1 ) 20-200(1 -00, B-112)	21 100(5)	, 22 100(0)	
Horz: 1-2=-44 Drag: 2-3=-0	2-16=19, 7-16=17, 1-13=	20, 0-13=-0				
MWFRS 2nd Wind Pa	3) 12=1733(B) 15=-28(F	) 16=260 17=17(F) 18=349 19=3 1.60, Plate Increase=1.60	86(F) 20=206(F=93, B=11)	2) 21=480(8	B) 22=480(B)	
Horz: 1-2=-28	2-16=19, 7-16=17, 1-13=	28, 8-13=-6				
	B) 12=1733(B) 15=-11(F	) 16=260 17=17(F) 18=349 19=3	86(F) 20=206(F=93, B=11	2) 21=480(1	B) 22=480(B)	
Uniform Loads (plf)	allel: Lumber Increase=1 2-16=19, 7-16=17, 1-13=	.60, Plate Increase=1.60 28, 8-13=-6			11111	JS S.K
Horz: 1-2=-44 Drag: 2-3=-0 Concentrated Loads (I	b)				Sill Sill	LICENSE
		) 16=260 17=17(F) 18=349 19=3 .60, Plate Increase=1.60	86(F) 20=206(F=93, B=11	2) 21=480(		No 34969
Horz: 1-2=-28 Drag: 2-3=-0		28, 8-13=-6			o RO	1
Concentrated Loads ( Vert: 14=112(	b) B) 12=1733(B) 15=-11(F	) 16=260 17=17(F) 18=349 19=3	36(F) 20=206(F=93, B=11	2) 21=480(	B) 22=480(B)	FLORIDA ON STATE OF
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May 24,2010



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Job	Truss	Truss Type	Qty		HUBER RES. / FLOOR	1433764
315120	T18	MONO HIP	1	2	Job Reference (optional)	1433704

Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 08:42:25 2010 Page 2

NOTES

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 115 lb down and 26 lb up at 3-8-11, and 289 lb down and 235 lb up at 3-7-12 on top chord, and 12 lb up at 3-8-11, 459 lb down at 5-5-7, 1340 lb down and 575 lb up at 7-8-0, and 974 lb down and 821 lb up at 9-7-4, and 974 lb down and 821 lb up at 10-10-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 15) Use Simpson HGUS28-2 to attach Truss to Carrying member

LOAD CASE(S) Standard

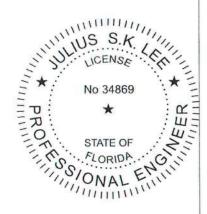
1) Regular: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-7=-109, 7-8=-69, 2-9=-10

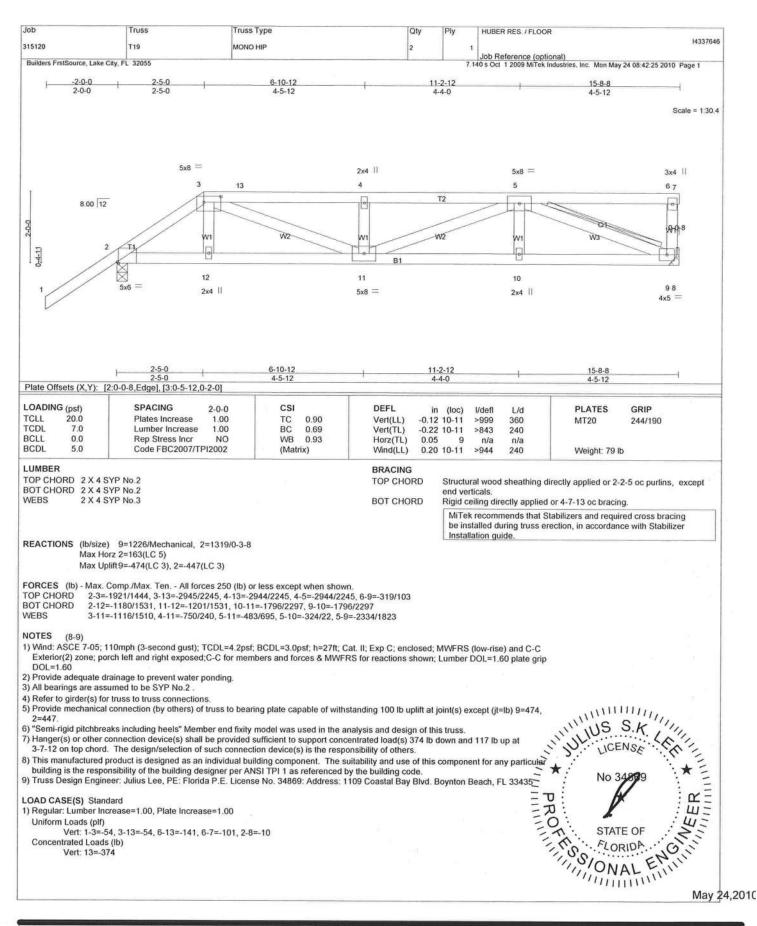
Concentrated Loads (lb)

Vert: 4=-404(B=-115) 14=5(B) 15=-459(B) 16=-1340(B) 17=-974(B) 18=-974(B) 19=-704(B) 20=-704(B)





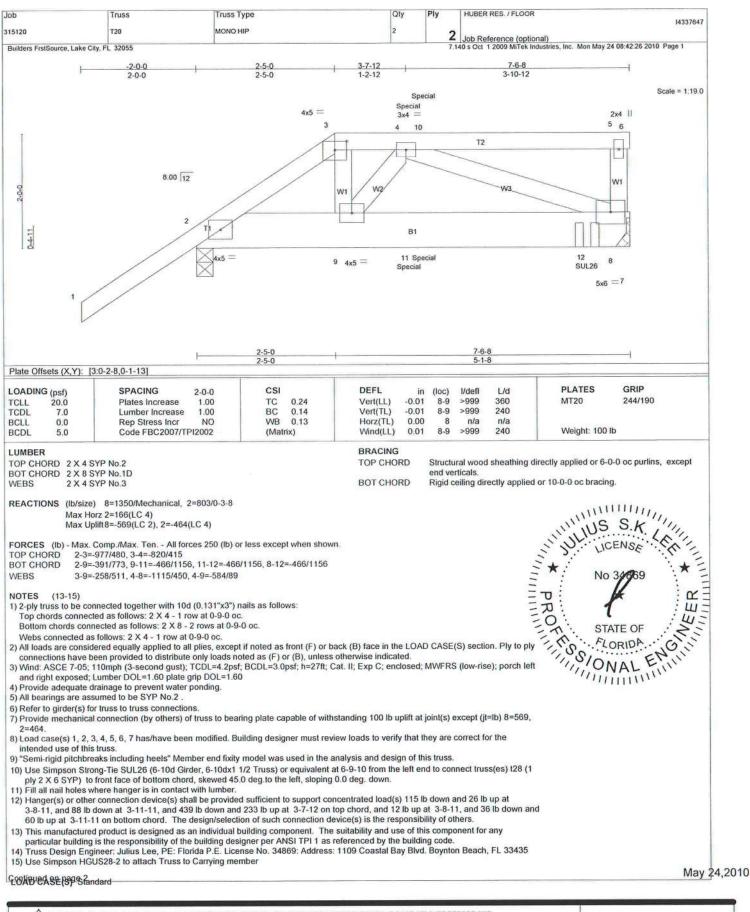
May 24,2010



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Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

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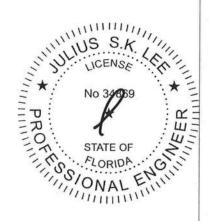
Job Truss Truss Type T20 315120 MONO HIP Builders FrstSource, Lake City, FL 32055 LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 1-3=-54, 3-4=-54, 4-5=-109, 5-6=-69, 2-7=-10 Concentrated Loads (lb) Vert: 4=-554(B=-115) 10=-69(B) 11=-30(B) 12=-703(F) 2) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=20, 2-3=-10, 3-4=43, 4-5=37, 5-6=7, 2-8=34, 7-8=-6 Horz: 1-2=-28, 2-3=2 Drag: 3-4=-1 Concentrated Loads (lb) Vert: 4=235(B=2) 10=-88(B) 11=66(B) 12=255(F) 3) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=13, 2-3=22, 3-4=36, 4-5=29, 5-6=20, 2-8=28, 7-8=-6 Horz: 1-2=-22, 2-3=-31 Drag: 3-4=-1 Concentrated Loads (lb) Vert: 4=177(B=10) 10=-88(B) 11=72(B) 12=261(F) 4) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=61, 2-3=36, 3-4=19, 4-5=13, 5-6=4, 2-8=28, 7-8=-6 Horz: 1-2=-69, 2-3=-44 Drag: 3-4=-0 Concentrated Loads (lb) Vert: 4=193(B=26) 10=-88(B) 11=72(B) 12=261(F) 5) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=10, 2-3=19, 3-4=19, 4-5=13, 5-6=20, 2-8=28, 7-8=-6 Horz: 1-2=-19, 2-3=-28 Drag: 3-4=-0 Concentrated Loads (lb) Vert: 4=117(B=26) 10=-88(B) 11=72(B) 12=261(F) 6) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=61, 2-3=36, 3-4=19, 4-5=13, 5-6=4, 2-8=28, 7-8=-6 Horz: 1-2=-69, 2-3=-44 Drag: 3-4=-0 Concentrated Loads (lb)
Vert: 4=193(B=26) 10=-88(B) 11=72(B) 12=261(F)
7) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=10, 2-3=19, 3-4=19, 4-5=13, 5-6=20, 2-8=28, 7-8=-6 Horz: 1-2=-19, 2-3=-28 Drag: 3-4=-0 Concentrated Loads (lb) Vert: 4=117(B=26) 10=-88(B) 11=72(B) 12=261(F)

Qty

Ply

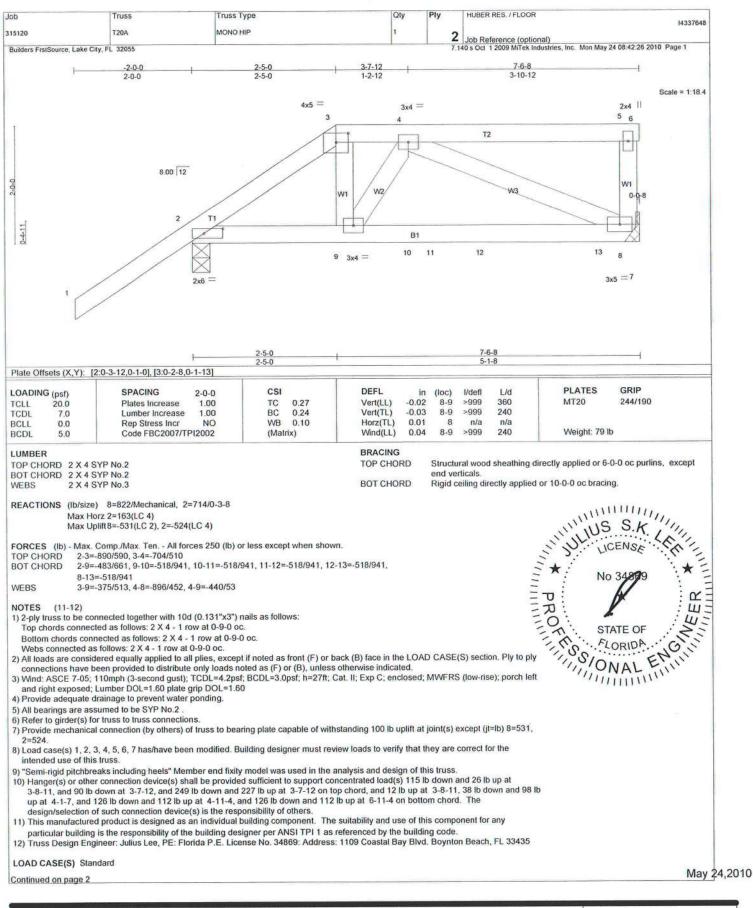
HUBER RES. / FLOOR

∠ Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 08:42:26 2010 Page 2



May 24,2010

14337647



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Job Truss Truss Type 315120 T20A MONO HIP Builders FrstSource, Lake City, FL 32055 LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 1-3=-54, 3-4=-54, 4-5=-109, 5-6=-69, 2-7=-10 Concentrated Loads (lb)

Vert: 4=-454(F=-115) 10=5(F) 11=-38(F) 12=-126(F) 13=-126(F)

2) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=20, 2-3=-10, 3-4=43, 4-5=37, 5-6=7, 2-8=34, 7-8=-6 Horz: 1-2=-28, 2-3=2 Drag: 3-4=-1 Concentrated Loads (lb) Vert: 4=116(F=2) 10=12(F) 11=98(F) 12=112(F) 13=112(F) 3) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=13, 2-3=22, 3-4=36, 4-5=29, 5-6=20, 2-8=28, 7-8=-6 Horz: 1-2=-22, 2-3=-31 Drag: 3-4=-1 Concentrated Loads (lb) Vert: 4=131(F=10) 10=12(F) 11=98(F) 12=112(F) 13=112(F) 4) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=61, 2-3=36, 3-4=19, 4-5=13, 5-6=4, 2-8=28, 7-8=-6 Horz: 1-2=-69, 2-3=-44 Drag: 3-4=-0 Concentrated Loads (lb) Vert: 4=164(F=26) 10=12(F) 11=98(F) 12=112(F) 13=112(F) 5) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=10, 2-3=19, 3-4=19, 4-5=13, 5-6=20, 2-8=28, 7-8=-6 Horz: 1-2=-19, 2-3=-28 Drag: 3-4=-0 Concentrated Loads (lb) Vert: 4=164(F=26) 10=12(F) 11=98(F) 12=112(F) 13=112(F) 6) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=61, 2-3=36, 3-4=19, 4-5=13, 5-6=4, 2-8=28, 7-8=-6 Horz: 1-2=-69, 2-3=-44 Drag: 3-4=-0 Concentrated Loads (lb) Vert: 4=164(F=26) 10=12(F) 11=98(F) 12=112(F) 13=112(F) 7) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=10, 2-3=19, 3-4=19, 4-5=13, 5-6=20, 2-8=28, 7-8=-6 Horz: 1-2=-19, 2-3=-28 Drag: 3-4=-0 Concentrated Loads (lb) Vert: 4=164(F=26) 10=12(F) 11=98(F) 12=112(F) 13=112(F)

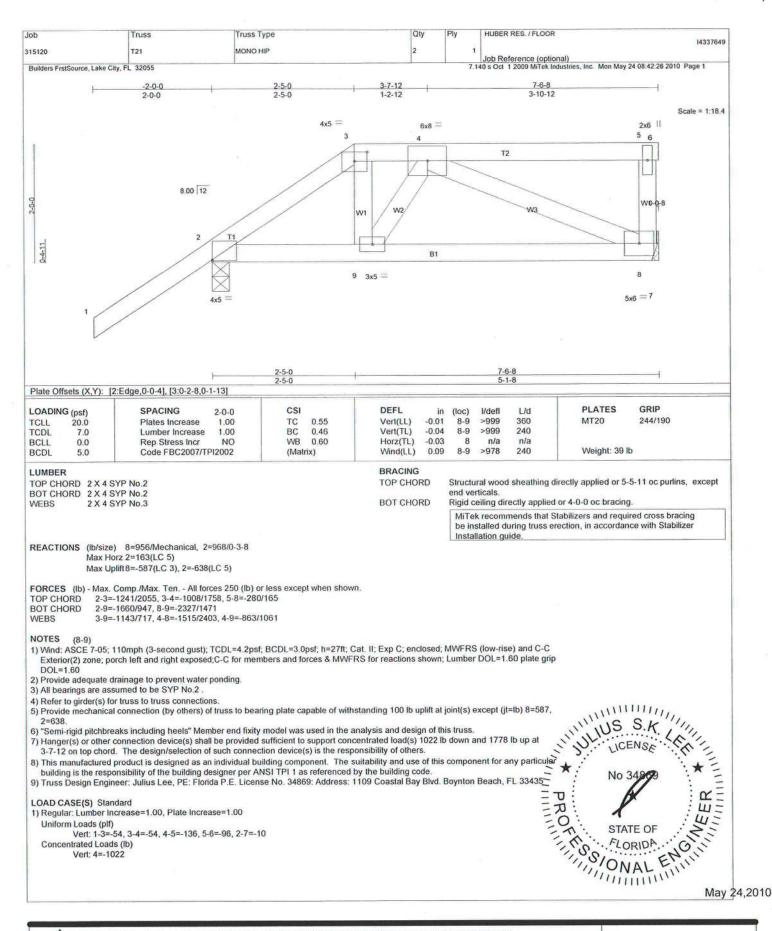
Qty

Ply

HUBER RES. / FLOOR

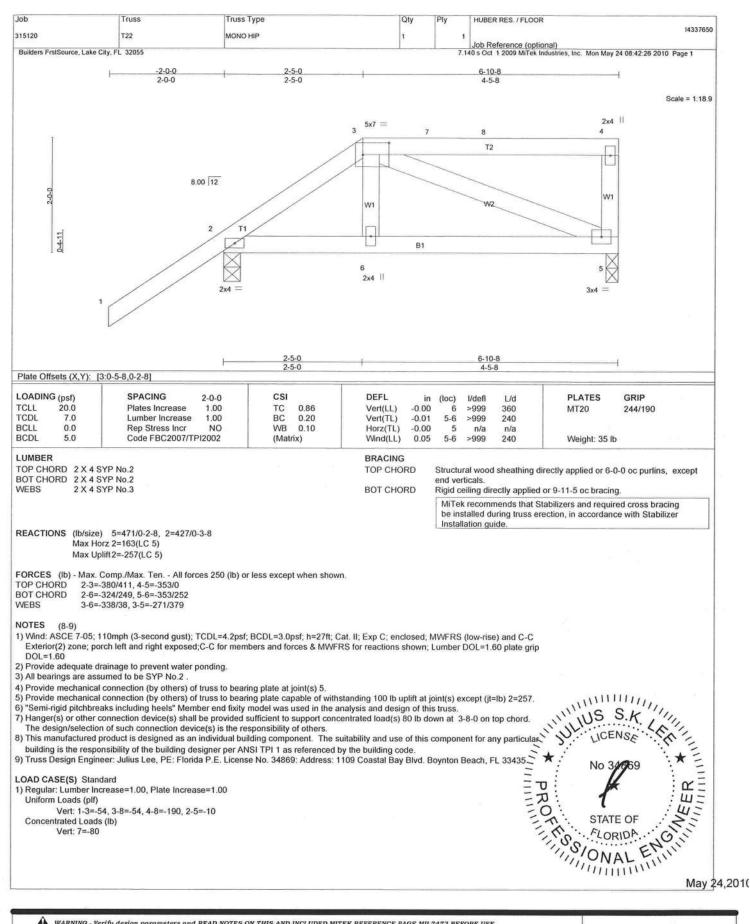
Z Job Reference (optional)
7.140 s Oct 1 2009 MiTek Industries, Inc. Mon May 24 08:42:26 2010 Page 2

May 24,2010



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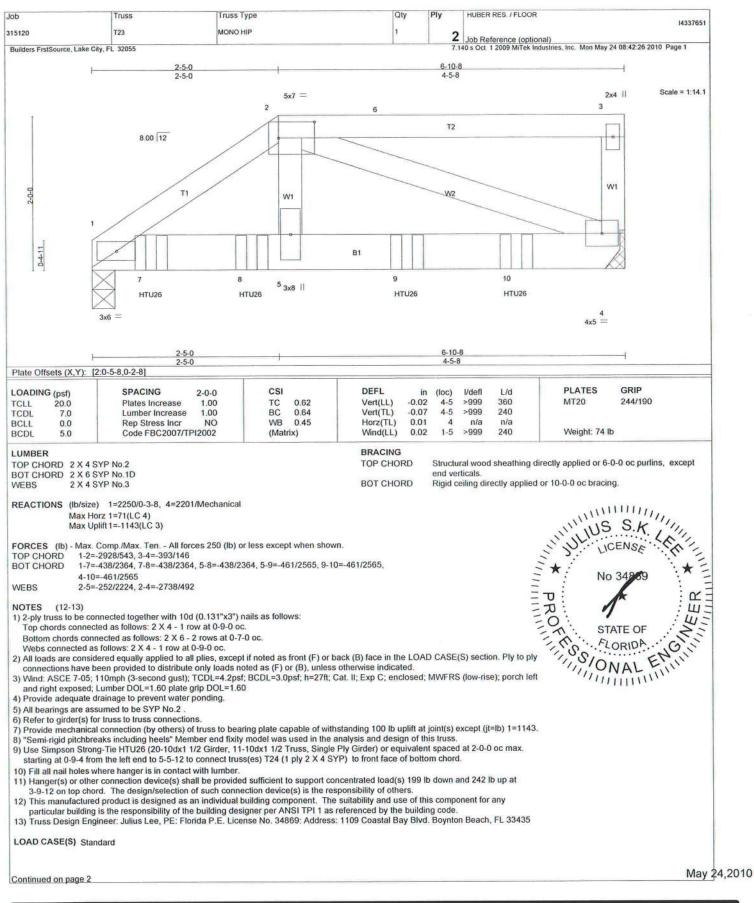
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. 
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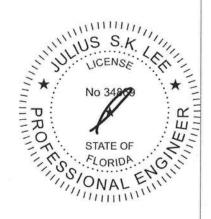
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Job	Truss	Truss Type	Qty	Ply	HUBER RES. / FLOOR	
315120	T23	MONO HIP	1			14337651
	120	MONO THE		2	Job Reference (optional)	
Builders ErstSource Lake City E	32055				0 - Out 1 2000 MT-1 Industrial Inc. May 14 - 24 05 12 05 0515 5	

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 1-2=-54, 2-6=-54, 3-6=-164, 1-4=-10 Concentrated Loads (lb)

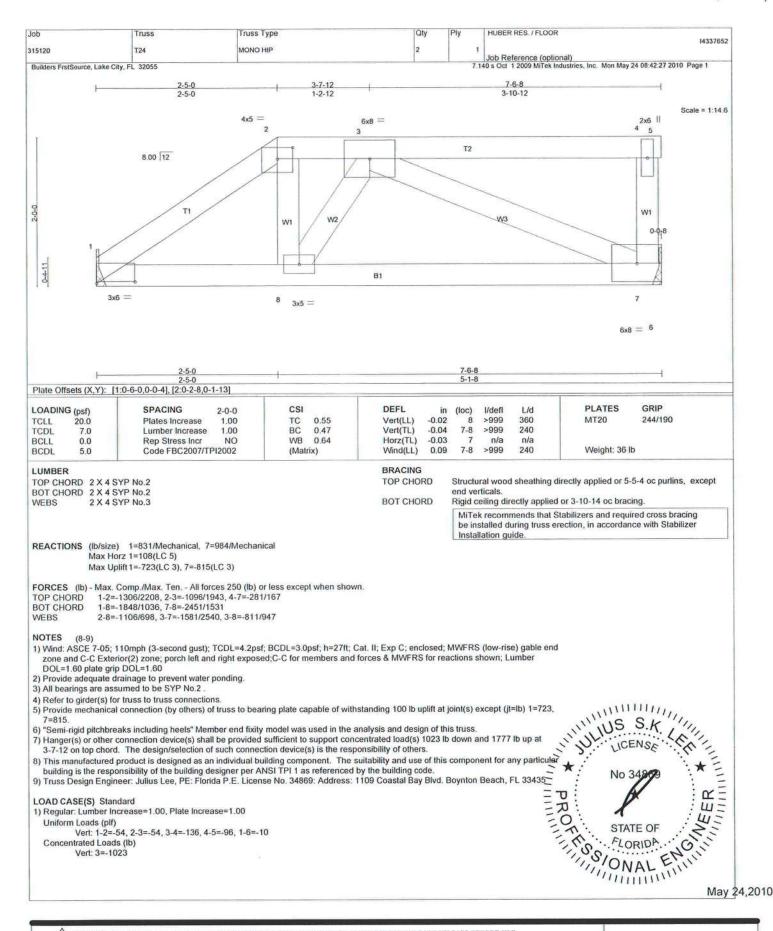
Vert: 6=-199 7=-821(F) 8=-821(F) 9=-934(F) 10=-934(F)



May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

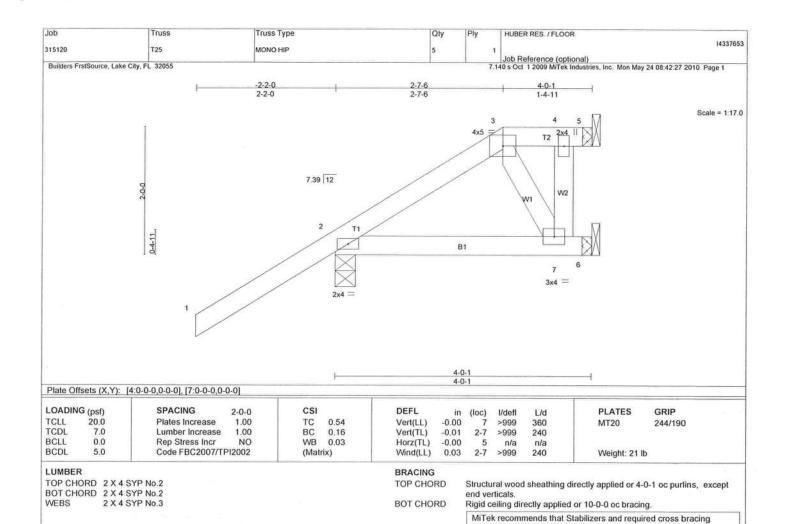
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REACTIONS (lb/size) 5=169/Mechanical, 2=285/0-3-13, 6=5/Mechanical Max Horz 2=162(LC 5)

Max Uplift 5=-43(LC 4), 2=-300(LC 5) Max Grav 5=169(LC 1), 2=285(LC 1), 6=49(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 4-7=-384/58

NOTES (8-9)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.
 All bearings are assumed to be SYP No.2.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-6=-10 Concentrated Loads (lb)

Vert: 5=-91

The design/selection of such connection device(s) is the responsibility of others.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Pay Device.

be installed during truss erection, in accordance with Stabilizer

Installation guide.

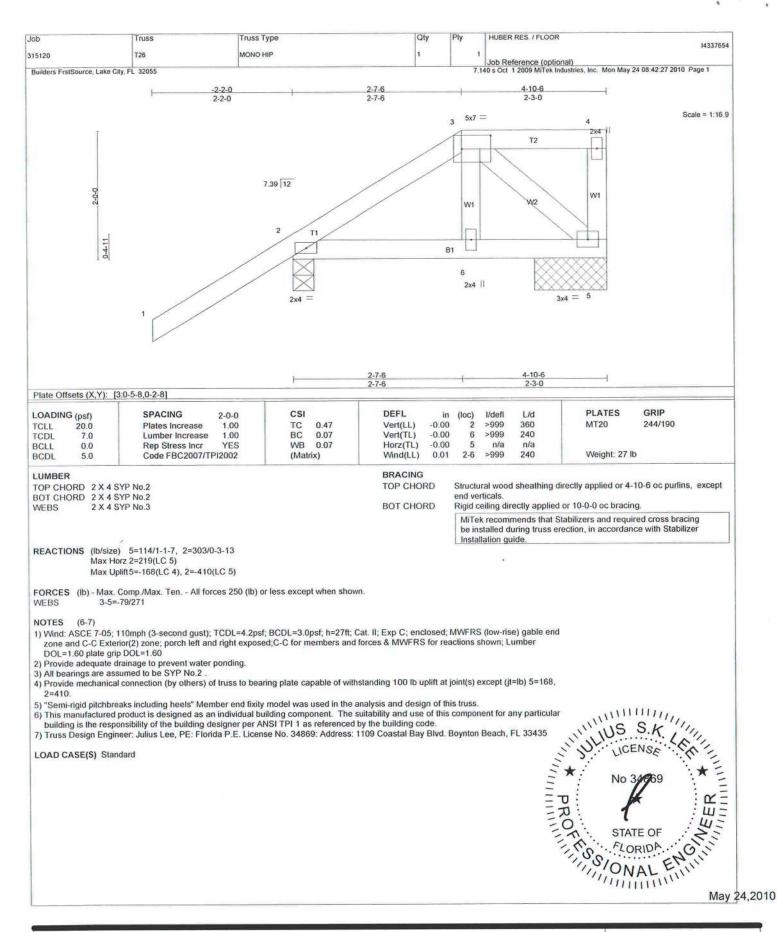
May 24,2010

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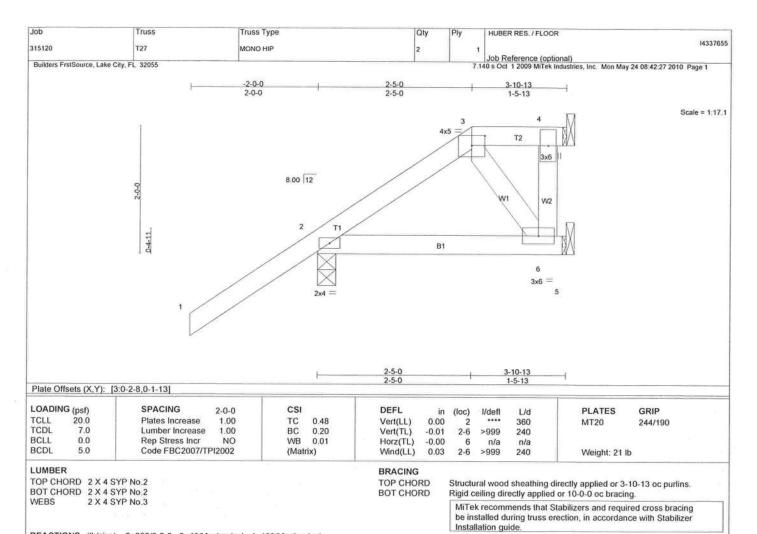
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AMSI/PIV Quality Citeria, DSB-89 and BCS11 Building Component available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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REACTIONS (lb/size) 2=262/0-3-8, 6=46/Mechanical, 4=123/Mechanical Max Horz 2=220(LC 5)

Max Uplift 2=-359(LC 5), 6=-86(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## (10-11)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (it=lb) 2=359
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 91 lb down at 3-7-5 on top chord. The design/selection of such connection device(s) is the responsibility of others.

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (pt)

Vert: 1-3=-54, 3-4=-54, 2-5=-10

Concentrated Loads (lb)

Vert: 4=-91

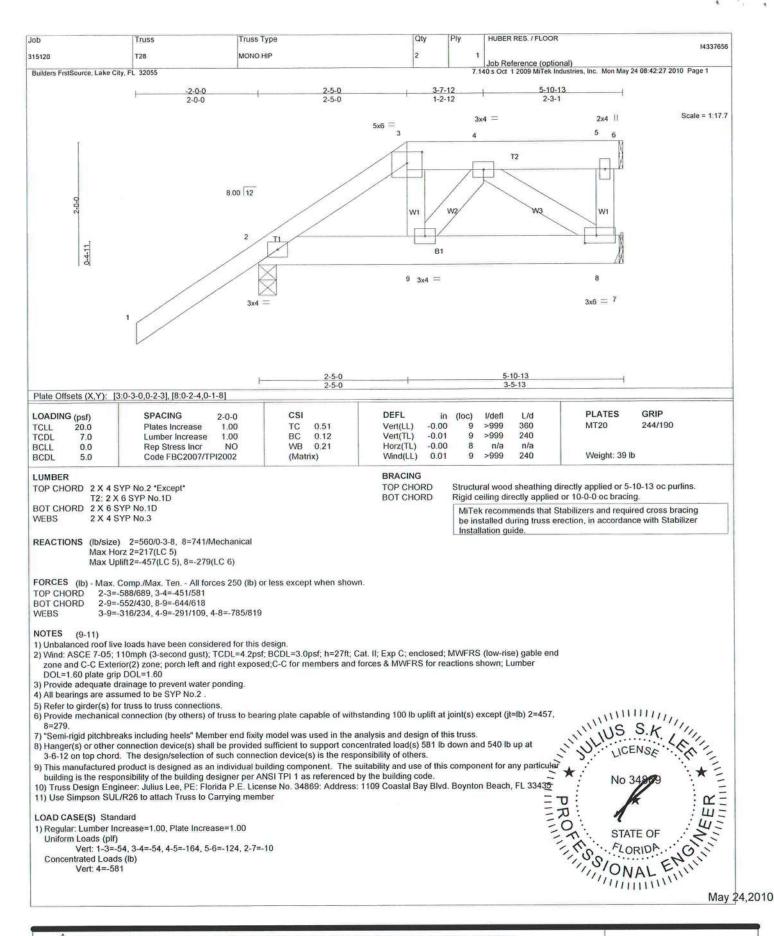
Concentrated Loads (lb)

Vert: 4=-91

May 24,2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

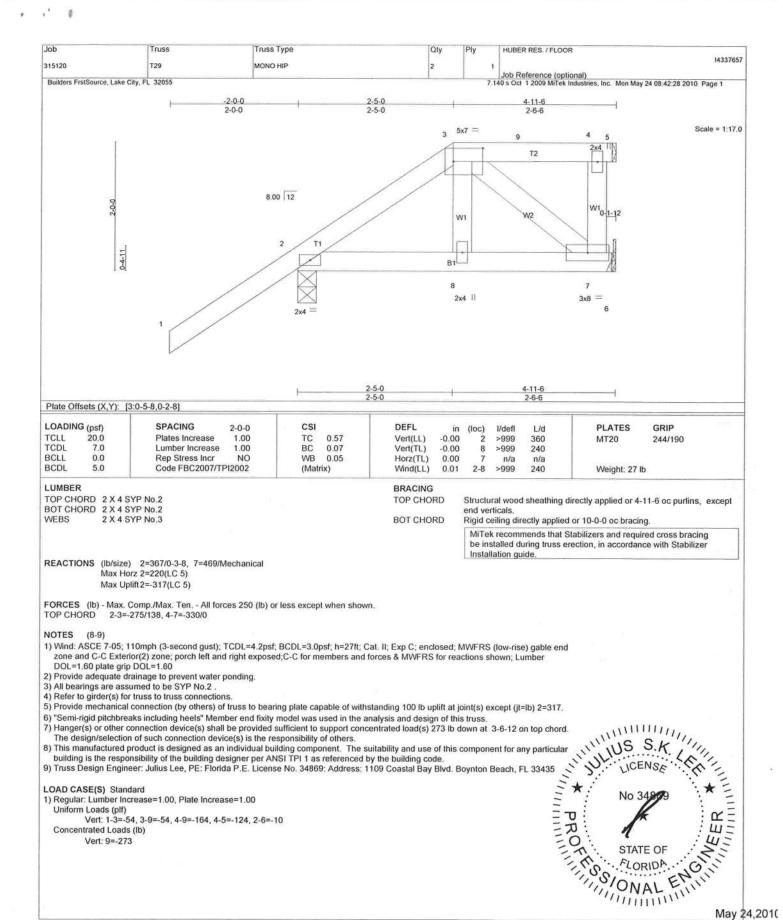
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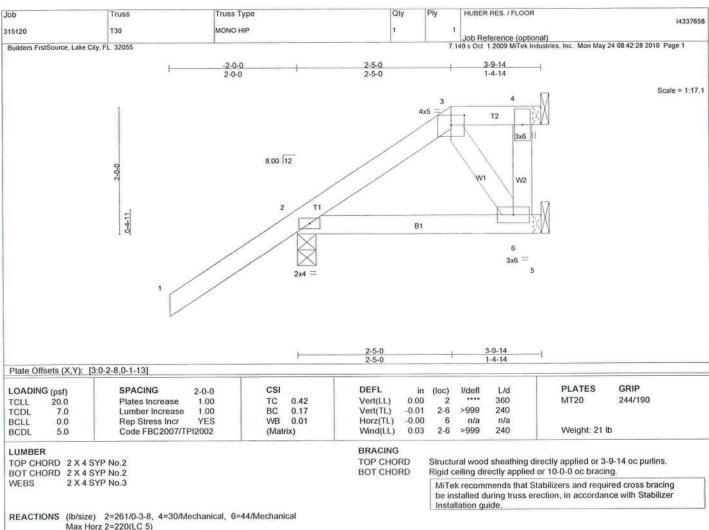
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Julius Lee 1109 Coastal Bay Blvd. Boynton, FL 33435

May 24,2010



Max Uplift 2=-358(LC 5), 4=-37(LC 3), 6=-85(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 .
   Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 6 except (jt=lb) 2=358.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular

building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

No 34899

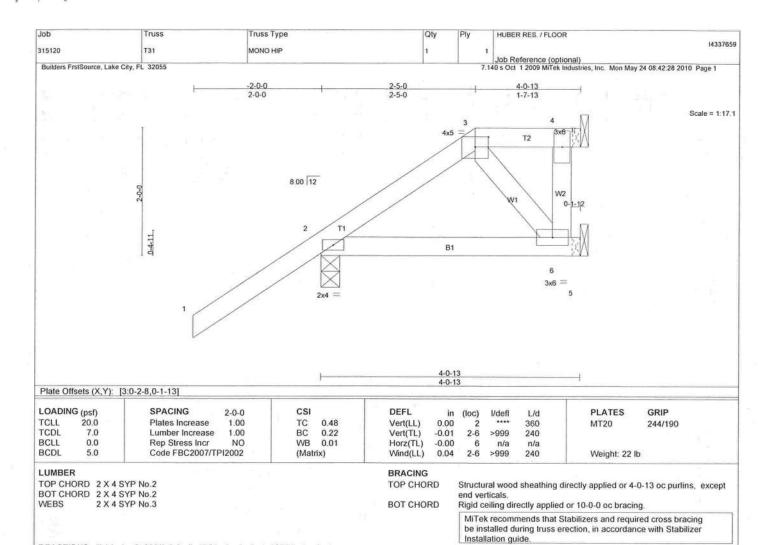
STATE OF

CONSTRUCTION OF LORIDA

May 24,2010

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milles connectors. This design is based only upon parameters shown, and is for an individual building component. 
Applicability of design parameters and proper incorporation of component is responsibility of building designer—not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/IPI Quality Criteria, DSB-89 and BCS11 Building Component Safely Information. available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

1109 Coastal Bay Blvd. Boynton, FL 33435



REACTIONS (lb/size) 2=266/0-3-8, 6=48/Mechanical, 4=127/Mechanical Max Horz 2=220(LC 5) Max Uplift 2=-363(LC 5), 6=-90(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=27ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
   All bearings are assumed to be SYP No.2.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 2=363.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
  7) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

## LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-54, 2-5=-10

Concentrated Loads (lb) Vert: 4=-90

The design/selection of such connection device(s) is the responsibility of others.

9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435. SIN STATE OF ONAL

May 24,2010

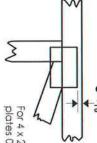
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual bullding component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insert stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, qualify control storage, delivery, erection and bracing, consult. ANS/ITI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison. WI 53719.

## Symbols

# PLATE LOCATION AND ORIENTATION



offsets are indicated. and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths. Center plate on joint unless x, y



edge of truss. For 4 x 2 orientation, locate plates 0- 'nd" from outside

This symbol indicates the required direction of slots in connector plates.

Plate location details available in MiTek 20/20 software or upon request.

## PLATE SIZE

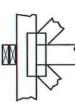
to slots. Second dimension is width measured perpendicular the length parallel to slots The first dimension is the plate

## LATERAL BRACING LOCATION



if indicated. output. Use T, I or Eliminator bracing by text in the bracing section of the Indicated by symbol shown and/or

## BEARING



Indicates location where bearings number where bearings occur reaction section indicates joint (supports) occur. Icans vary but

## ANSI/TPII: Industry Standards:

National Design Specification for Metal Design Standard for Bracing. Plate Connected Wood Truss Construction.

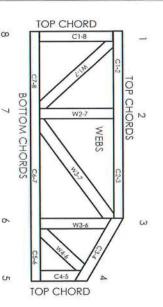
Installing & Bracing of Metal Plate Building Component Safety Information, Guide to Good Practice for Handling,

Connected Wood Trusses

DSB-89:

# Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A

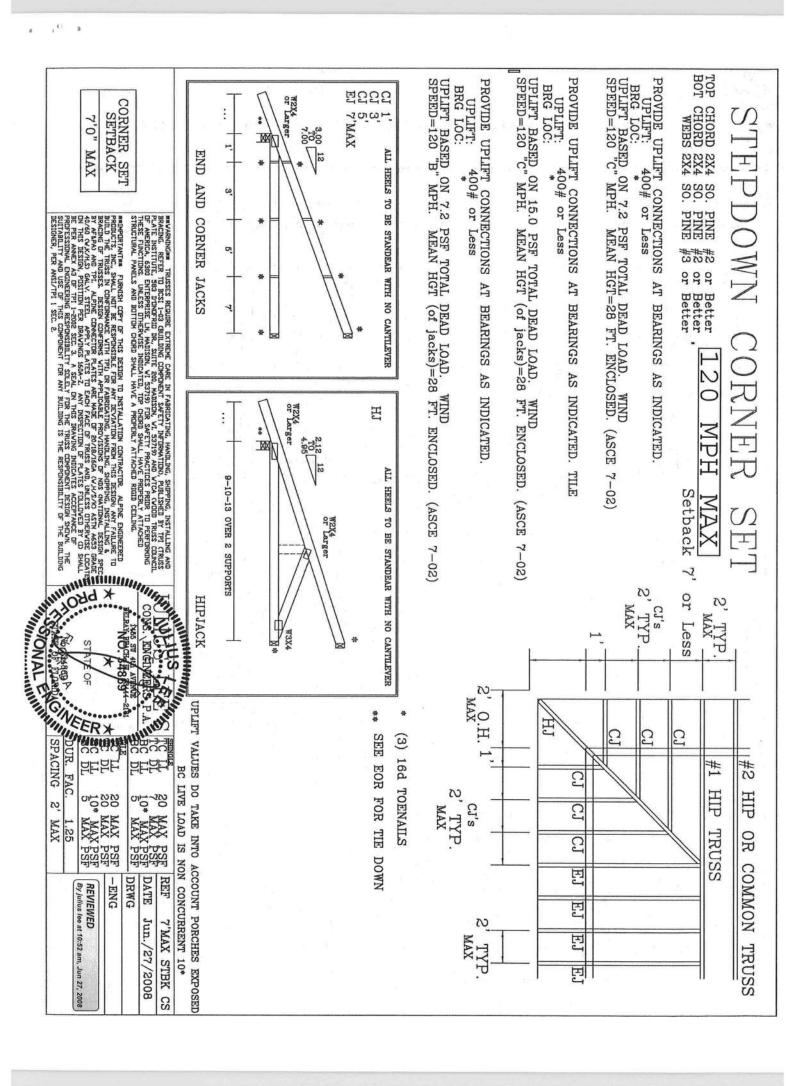
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## Boynton, FL 33435 Julius Lee 1109 Coastal Bay Blvd.

# General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Truss bracing must be designed by an engineer, For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T. I. or Eliminator bracing should be considered. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, properly owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



CE NO. 44869

NO. 44869

STATE OF

REVIEWED

By Julius lee at 12:00 pm., Jun 11, 2008

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REPER TO CHAR DIAGONAL BRACE OPTION:
VERTICAL LENGTE MAY BE
DOUBLED WINN DIAGONAL
BRACE IS USSED. CONNECT
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BRACE IS USSED. TOWN SED AT EACH END. MAX WEB
TOTAL LENGTH IS 14\*. **GABLE** VERTICAL LENGTH MAX IN TABLE I SPACING 16" 24" O.C O.C. 12 O.C. GABLE VERTICAL ABOVE. SPECIES SPF SPF SPF DFL SP H SP H SP H HL FL ASCE GRADE STANDARD STANDARD STANDARD STANDARD STANDARD STANDARD EF EF CUIS COLS STUD が表れ / #2 BRACE 7 -02: GABLE TRUBS BRACES NO 130 GROUP A SPF #1/#Z, OR BETTER DIAGONAL BRACE: SINGLE OF DOUBLE MPH 1X4 °L" BRACE . UPPER END. GROUP B WIND 2 8 0 6 GROUP A (1) 2X4 "L" BRACE . SPEED, GROUP B 10 15 THE PROPER 18 GROUP A (2) MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH 2X4 "L" BRACE \*\* CONTINUOUS GROUP B HEIGHT, HEN OR BETTER **( (** (1) 2XB CONS. SNEWS GROUP A 10 12 13' 8° 10 12 10' 3" 12 ō. 10, No: 34869 STATE OF FLORIDA IUS LEE'S "L" BRACE \* ENCLOSED, GROUP B 12' 8" 13 5 5 7 T 12 B 12 12 (2) ZXB GROUP A S 4 MAX. MAX. BRACE GROUP 11 13' 7" 13 14' D° 4.4. (E) 1.00, TOT. SPACING W F ATTACE EACH 'L' BRACE WITH 104 NAIS.

\* FOR (1) 'L' BRACE; SPACE NAIS AT 8" 0.C.

\* FOR (2) 'L' BRACE; AND 4" 0.C. BETWEEN ZONES.

\*\* FOR (2) 'L' BRACE; SPACE NAIS AT 3" 0.C.

IN 18" END ZONES AND 6" 0.C. BETWEEN ZONES. GABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). T. BRACING MUST BE A MINIMUM OF BOX OF WEB LIVE LOAD DEPLECTION CRETERIA IS L/240. MEMBER LENGTH DOUGLAS FIR-LARCH PLYWOOD OVERHANG. BRACING GROUP SPECIES \$1 / #2 STANDARD EXPOSURE VERTICAL LENGTH

IESS THAN 4.0°

GREATER TRAN 4.0°, BUT

LESS THAN 11.0°

GREATER TRAN 11.0° GABLE TRUSS SOUTH YOU PINE 60 PEAK, SPLICE, AND HEEL PLATES. 24.0 STANDARD CABLE VERTICAL PSF DATE REF DRWG MIEK SID GABLE 15 E EI GROUP GROUP HI & BIR HEM-PIR DETAIL 0 PLATE SIZES DOUGLAS FIR-LARCH 11/26/03 Ä ASCE7-02-GAB13015 A: HIM-FIR NO SPLICE STANDARD AND 2.5X4 NOTES: 772 STANDARD GRADES:

NO. 64889 NO. 64 DIAGONAL BEACE OFTON:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
BRACONAL BEACE TOR BROF
AT EACH END. MAX WEB TOTAL LEXIGIN IS 14". MAX **GABLE** VERTICAL LENGTH DI TABLE ABOVE. SPACING SPECIES 16" 24" 12 0 .C. O.C. O.C. GABLE VERTICAL SPF SPF SPF DFL DFL DFL H SP ASCE GONAL AT STANDARD 41 STANDARD STANDARD STANDARD GRADE STANDARD STANDARD 打/提 COLS STUD BRACE 7-02: PARAMATA TRUSSES REBUIRE EXTREME CARE IN FARRICATING, HANDLING, RAPPING, INSTALLING AND CONG. REPR. TO BOST 1-43 GRUILING COMPORENT SAFETY (HOTWAINIDA), PUBLISHED BY TPI CIRLUSS ITE INSTITUTE, 338 INCHORM DAY, SUITE BOJ, MANISAN, NC 5379) AND VITCH KHOOD TRUSS COLVED, MECKUA, 630 ENTERPRISE LY, MIGIGN, VI 53719) FIR SAFETY PRACTICES POTER TO PERFORMING SE FINCTIONS, UNLESS OFFICARICS INTERVIELD TO COLORD SHALL HAVE PROPERTY ATTACHED UND COLORD SHALL HAVE PROPERTY ATTACHED TO COLORD SHALL HAVE PROPERTY ATTACHED TO COLORD SHALL HAVE A PROPERTY ATTACHED TO COLORD SHALL HAVE PROPERTY ATTACHED TO COLORD SHALL HAVE A PROPERTY ATTACHED TO COLORD SHALL HAVE PROPERTY ATTACHED TO COLORD SHALL HAVE PROPERTY ATTACHED TO COLORD SHALL HAVE A PROPERTY ATTACHED TO COLORD SHALL HAV SBURT TRUBS By julius lee at 12:00 pm, Jun 11, 2008 REVIEWED 3. 3 7 BRACES 130 ZK4 SP OR
DIT-L #2 OR
BEFFER DIAGONAL
BRACE; SINGLE
OR DOUBLE GROUP A (1) 1X4 °L" BRACE • (1) 2X4 "L" BRACE • (2) 2X4 "L" BRACE •• (1) 2X6 "L" BRACE • (2) 2XB "L" BRACE •= AT UPPER END MPH GROUP B WIND 10, GROUP A SPEED, BRACE GROUP B REFER TO POOR 30 18 GROUP A CHART ABOVE FOR MAX GABLE VERTICAL LENGTH. MEAN CONLINGOR EX4 #EN OR BETTER GROUP B HEIGHT, **( (** SNEEVER CONS. GROUP A DELEGY BEACH, PL 33444-2161 8 1 10 B' 0\* No: 34868 STATE OF FLORIDA US LEE'S ENCLOSED, GROUP B GROUP A 12 11 10, 10, 12' 3 S MAX MAX GROUP 11 12' 6" 13' 3" 13' 2" 10, 12, TOT. 1.00, SPACING E ATTAGE EACH 'L' BRACE WITH 104 NAIS.

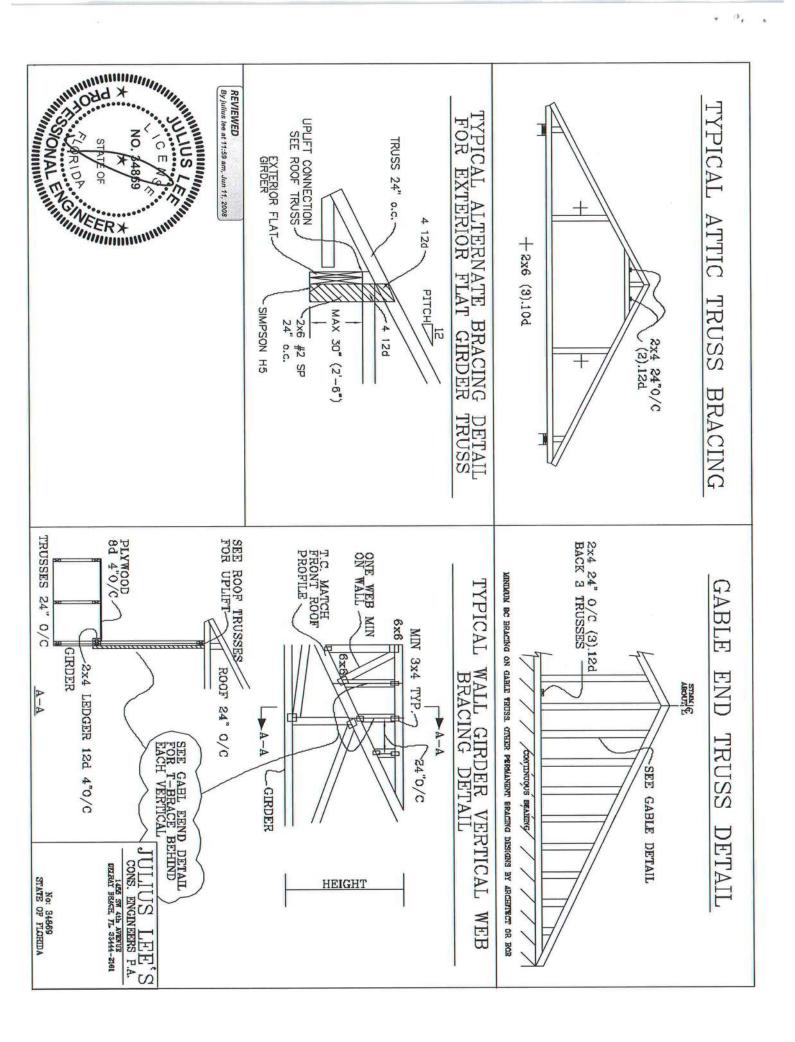
\$ FOR (1) 'L' BRACE; SPACE WALLS AF 2" O.C.

\$ FOR (2) 'L' BRACES; SPACE WALLS AF 2" O.C.

\$ FOR (2) 'L' BRACES; SPACE WALLS AF 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. GABAL END SUPPORTS LOAD FROM 4' 0" DUTLOPHERS WITH 2' 0" DVERHANG, DR 12" PLYWOOD OVERHANG. T' BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGIN. PROVIDE UPLIFT CONNECTIONS FOR 180 FLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CHITERIA IS L/240. DOUGLAS FIR-LARCH SPRUCE-PINE-INB

#3 STUD BRACING GROUP SPECIES AND EXPOSURE CABLE TRUSS DETAIL NOTES: 60 SOUTHING PINE GREATER THAN 4 D BUT GREATER THAN 11' 6" VEHTICAL LENGIK 24.0 PEAK, SPLICE, AND HEEL PLATES. GABLE VERTICAL STANDARD PSF DATE REF DWG MITEK STD GABLE 30' E HI #1 & BIR GROUP GROUP HEM-PIR PLATE SIZES 11/26/03 DOUGLAS FIR-LARCE ASCE7-02-CAB13030 Ŗ UDJS SMD4 NEEHLDOS A: #2 STUD NO SPLICE 2.5X4 STANDARD 200 GRADES:



## PIGGYBACK DETAIL

TYPE

SPANS

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REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, I MI FROM COAST CAT I EXP C. WIND TC DL-5 PSF. WIND BC DL-5 PSF 110 MPH WIND, 30' MEAN HGT, FEG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND HC DL=6 PSF

D

5X6

5XG

834 5X4

OR 3X6 TRULOX AT 4' OC,

C H

1.5X3

1.5X4

1.5X4

1.5X4 BX6

4X8

5X6

**5X8** 

9X9

284

2.5X4

2.6X4

3X6

ATTACH THULOX PLATES WITH (6) 0.120" X 1.575" NAILS, OR EQUAL PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR THULOX

INFORMATION

FRONT FACE (E,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS HOTH FACES ARE SPACED 4' OC MAX. 20' FLAT TOP CHORD MAX SPAN MAX SIZE OF ZXIZ

TYP. W 图 B 英 Ш C-TYP. D-SPLICE

ACCEPTABLE EITHER PLATE

XAX V

O' TO 7'9" 7'9" TO 10' 0 5 1x4 "T" BRACE. SAME GRADE, SPECIES AS I MEMBER. OR BETTER. AND 80% LENGTH OF I MEMBER. ATTACH WITH 8d NAILS AT 4" OC. ZX4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC. NO BRACING WEB BRACING CHART REQUIRED BRACING WEB

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120 % 1.275" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS. ZU

NO. 44869

NO. 44869

NO. 44869

NO. 64869

ULIUS LEE'S CONS. ENGINEERS P.A. 1456 SW 4th AVENUE DELEAY BEAGE, FL. 33444 2161 No: 94866 STATE OF FLORIDA SPACING 1.25 DUR. 55 PSF AT 1.33 DUR. FAC. 47 PSF 1.15 DUR. MAX LOADING 50 PSF AT 25 DUR. FAC 24.0 AT FAC. REF DATE -ENG JL DRWGMITEK STD PIGG 09/12/07 PIGGYBACK

THIS DRAWING

REPLACES DRAWINGS 634,016 634,017 & 847,045

\* PIGGYBACK SPECIAL PLATE 8 1/4"

### VALLEY TRUSS DETAIL

TOP BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 2X4 SP #3 OR BETTER. OR BETTER

- \* ZX3 MAY BE RIPPED FROM A ZX6 (PITCHED OR SQUARE).
- ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, BUILDING, EXP. C. RESIDENTIAL WIND TO DI=5 FEC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 15' MEAN HEICHT, ENCLOSED 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEH, VALLEY WEH, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9"

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY INSTALLATION TRUSS

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

ENGINEERS'

SEALED

DESIGN

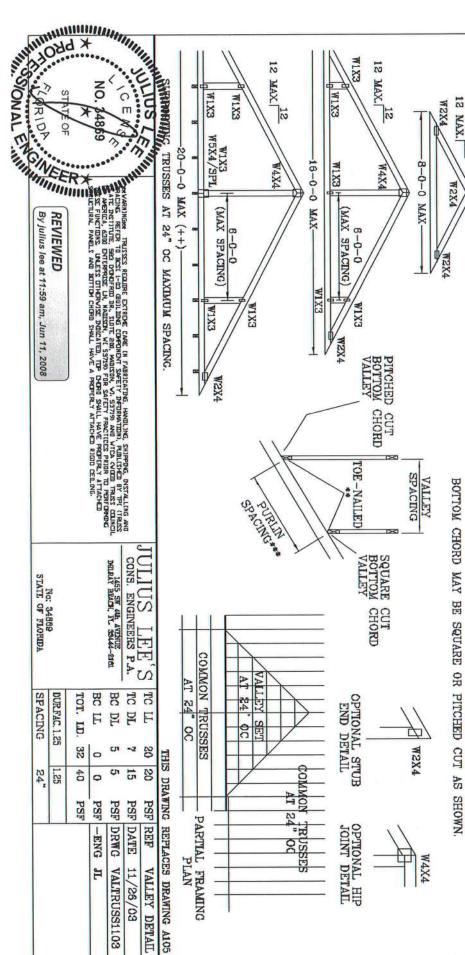
\*\* ++ LARGER SPANS MAY BE NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF BUILT AS LONG AS THE VERTICAL HEIGHT DOES THE TOP CHORD.

NOT EXCEED 12'0"

LARGER AS REQ'D

4-0-0

MAX



THUMAN X PARTY ON A PA

DELEAY BEACH, IL 35444-2101 CONS. ENGINEERS P.A.

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DATE REF

11/26/03 VALLEY DETAIL

DRWG VALTRUSS1103

No: 34869 STATE OF FLORIDA

SPACING DUR.FAC. 1.25 TOT. LD

24

## TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

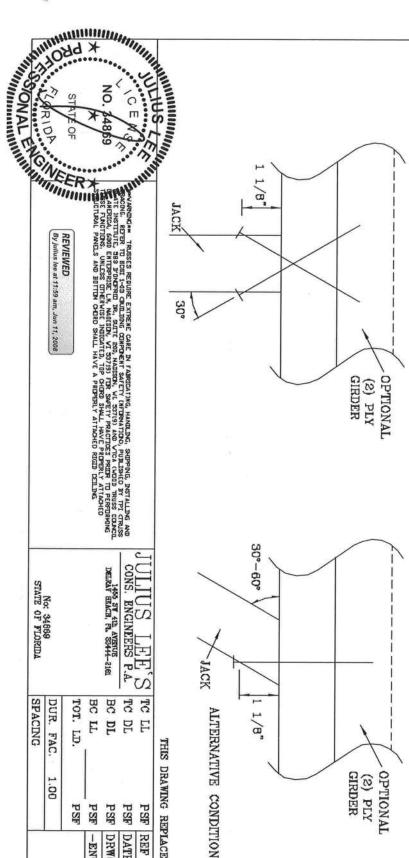
PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

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I	MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS
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ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.	5 48	4 38	3 28	2 19	TOE-NAILS 1	1
MAY BE	493#	394#	296#	197#	1 PLY	SOUTHERN PINE
MULTIPLIE	639#	511#	383#	256#	2 PLIES	
D BY APP	452#	361#	271#	181#	1 PLY	DOUGLAS
ROPRIATE	585#	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH
NOTTAGET	390#	312#	234#	156#	1 PLY	HEM-FIR
TOAT FO	507#	406#	304#	203#	2 PLIES	-FIR
ACTION	384#	307#	230#	154#	1 PLY	SPRUCE
	496#	397#	298#	188#	2 PLIES	SPRUCE PINE FIR



THIS DRAWING REPLACES DRAWING 784040

		SUPPONG, DRTALLING AND PUBLISHED BY I'PT CRAUSS PURCH CRAUSS COLVEN. ES PRODR I'D FEEDBRING PROPERLY ATTRACED PRODRICY ATTRACED O RIGID CELLING.							
STATE OF FLORIDA	No: 34889			DELRAY SEACH, FL. 83444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S			
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL			
	1.00	PSF	PSF	PSF	PSF	PSF			
			-ENG JL	DRWG	DATE	REF			
			IL	CNTONAIL1103	09/12/07	TOE-NAIL			

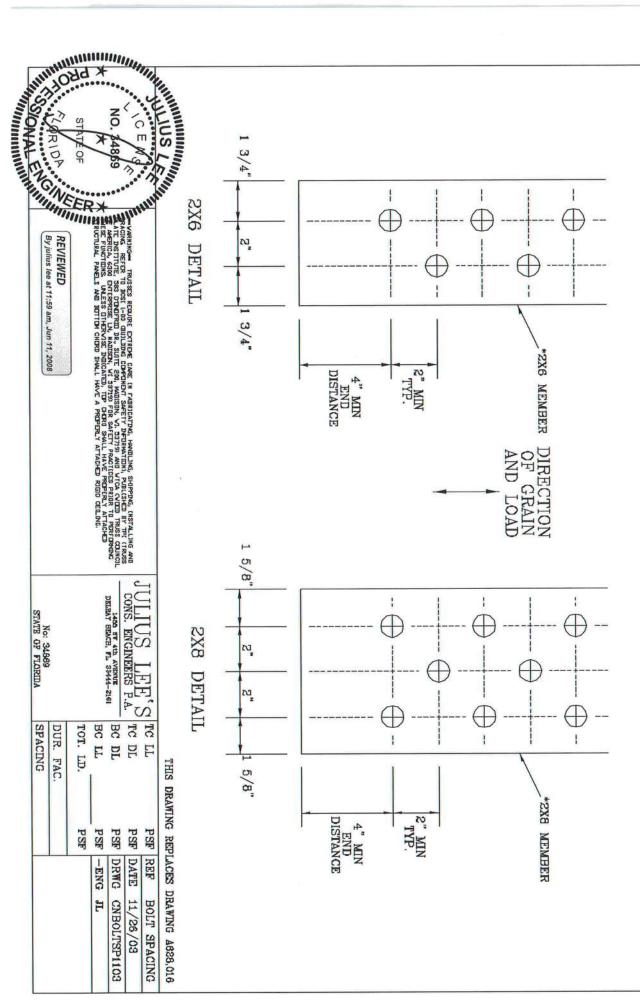
W

# DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER. \* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLIS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



REVIEWED

C

DELRAY SEACH, FL 33444-2161

BC LL

PSF PSF PSF

-ENG

T

DRWG DATE

CNBOLTSP1103 11/26/03

PSF

TOT. LD. DUR. FAC

No: 34869 STATE OF FLORIDA

SPACING

By julius lee at 11:59 am, Jun 11, 2008

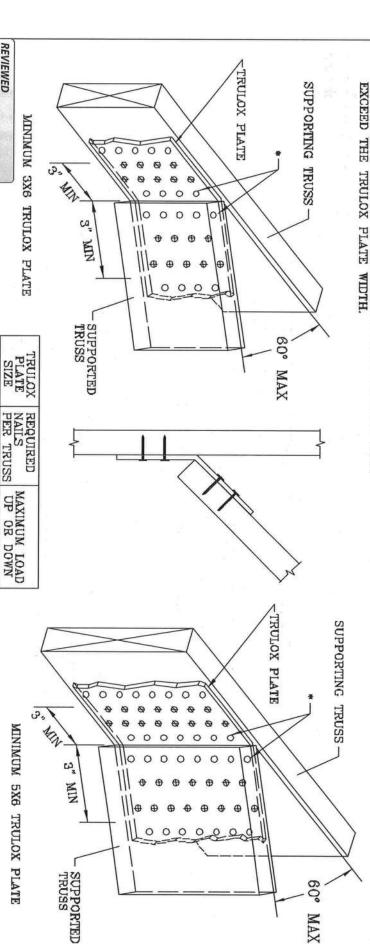
# TRULOX CONNECTION DETAIL

SHOWN (+). 11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST NAILS MAY BE OMITTED FROM THESE ROWS

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING INFORMATION NOT SHOWN. THIS DETAIL FOR LUMBER, PLATES, AND OTHER



NO. 64869

\*\*MANNOS\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND RACING REFER TO 3033 1-00 GUILLING COPOINCHT SAFETY INFORMATION, PUBLISHED BY TRY (TRUSS AND INSTITUTE, 303 DYNOFFEM DR, SUTTE 800, MADIEST, LT 37375 AND VITCA YOUTH TRUSS COUNCIL, RACEILA, 6300 ENTERPRISE LY, MADIESH, VE 39739 FOR SAFETY PRACTICES PRICE TO PERTUNNING (FEEL FOUNTIES). UNLESS OTHERVISE ONLINETE, TO OFURO SALL HAVE PROPERLY ATTACHED RECTURAL PANELS AND SOTTON CHORD SALL HAVE PROPERLY ATTACHED.

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CONS. ENGINEERS P.A. DETRYK BEYCH, IT 201747-STEI

1,154,844

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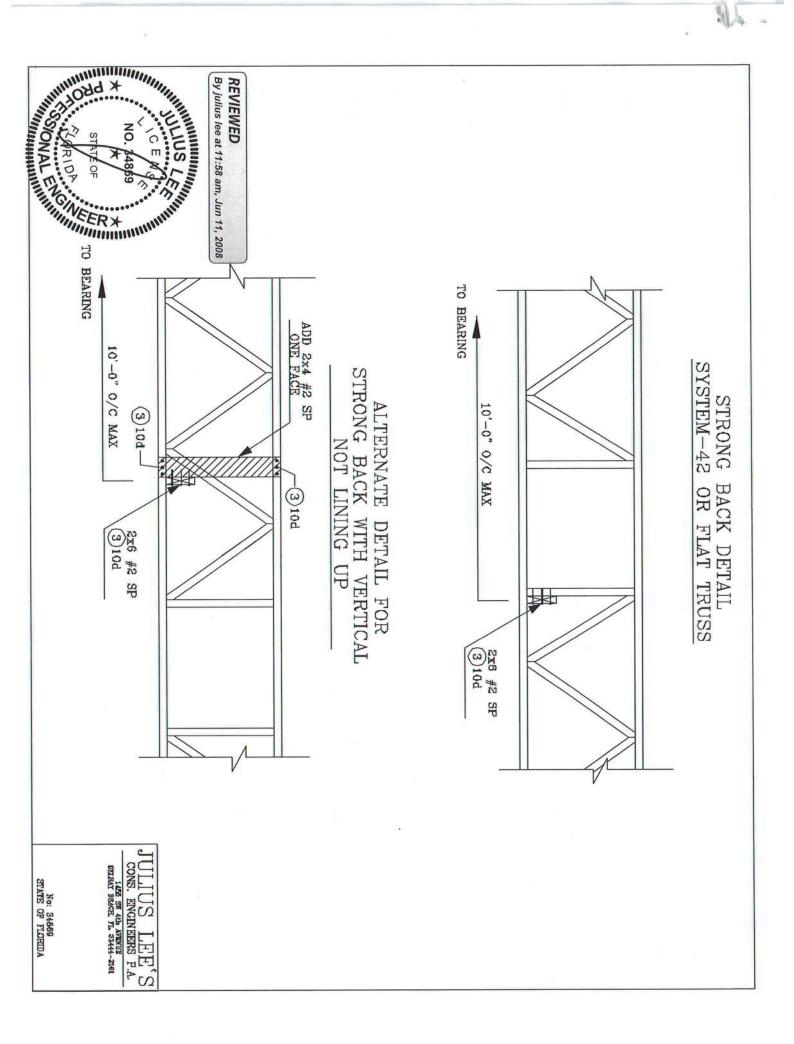
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> CNTRULOX1103 11/26/03 TRULOX

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No: 34869 STATE OF FLORIDA



### MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

### Maximum Uniform Load Applied to Either Outside Member (PLF)

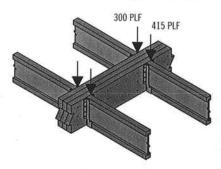
Market			Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
Connector Type	Number of Rows	Connector On-Center Spacing	1 2 1				1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1 2° 1 1 2° 1 2° 1 2° 1 2° 1 2° 1 2° 1
			134	134"	134" 31/2"	13/3" 31/3" 13/4"	31//	1%
			31/2" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3")	2	12"	370	280	280	245		
Nail <sup>(1)</sup>	3	12"	555	415	415	370		MARKET STATE
1/2" A307		24"	505	380	520	465	860	340
Through Bolts(2)(4)	2	19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	510	455		
SDS 1/4" x 31/2"(4)	2	19.2"	850	640	640	565	1945	
		16"	1,020	765	765	680		
		24"		Section 2 Control of the Control of		455	465	455
SDS 1/4" x 6"(3)(4)	2	19.2"		ASSESSED ALCOHOLOGY	MANAGES SE	565	580	565
		16"	400			680	695	680
uen weer (t)		24"	480	360	360	320		
USP WS35 (4)	2	19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 (3)(4)	9	24" 19.2"	Manual San Indiana			350	525	350
USP WSB (*/**)	2	19.2			MENTEROUS EDUCATE	440	660	440
		24"	car	475	475	525	790	525
33/8"	1	19.2"	635	475	475		425 530	
TrussLok(4)	2	19.2	795 955	595	595 715			
		24"	990	715 500	500	635 445	THE RESERVE THE PERSON NAMED IN	AAF
5"	2	19.2"		625	625	555	480	445
TrussLok(4)	4	16"	THE RESERVE OF THE RE	750	750	665	600 725	555 CC5
Constitution of the provided		24"		730	730	445	620	665
63/4"	2	19.2"			HONOVERNI AND END	555	770	445
TrussLok(4)	*	16"		The second second		665	925	555 665

Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

### **General Notes**

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic cells indicate Connector Pattern must be installed on both sides.
   Stagger fasteners on opposite side of beam by ½ the required Connector Spacing.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides
  of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7° require special consideration by the design professional.

### **Uniform Load Design Example**



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

### Alternates

Two rows of 1/2" bolts or SDS 1/4" x 31/2" screws at 19.2" on-center.

<sup>(2)</sup> Washers required. Bolt holes to be %6" maximum.

<sup>(3) 6&</sup>quot; SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

<sup>(4) 24&</sup>quot; on-center bolted and screwed connection values may be doubled for 12" on-center spacing.



### MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

### Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

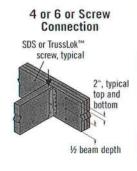
	and the same	Connector Pattern							
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F		
Connector Type	Number of Connectors	2"	134'	11/11 31/11	14" 31/" 14"	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	134		
		31/2" 2-ply	51/4" 3-ply	51/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply		
	6	1,110	835	835	740				
10d (0.128" x 3")	12	2,225	1,670	1,670	1,485	280世 建建设设置的			
Nail	18	3,335	2,505	2,505	2,225				
	24	4,450	3,335	3,335	2,965	THE BROWNING	<b>以至至1000000</b>		
SDS Screws	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)		
1/4" x 31/2" or WS35 1/4" x 6" or WS6(1)	6	2,870	2,150 (4)	2,150	1,915	2,785(2)	2,110(2)		
	8	3,825	2,870 (4)	2,870	2,550	3,715(2)	2,810(2)		
3³/₀" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	1,695	1,925(3)	1,775(3)		
	6	3,815	2,860 (4)	2,860	2,545	2,890(3)	2,665(3)		
	8.	5,090	3,815 (4)	3,815	3,390	3,855(3)	3,550(3)		

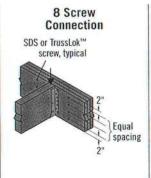
(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

- (2) 6" long screws required.
- (3) 5" long screws required.
- (4) 3½" and 35%" long screws must be installed on both sides.

### Connections





## 10d (0.128" x 3") nails, typical. Stagger to prevent splitting. 8"-10" 2" spacing, typical

**Nail Connection** 

There must be an equal number of nails on each side of the connection

### **Point Load Design Example**



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3¾" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

### MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

11/2"

minimum

spacing,

typical

### 13/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3¾" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed
- on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

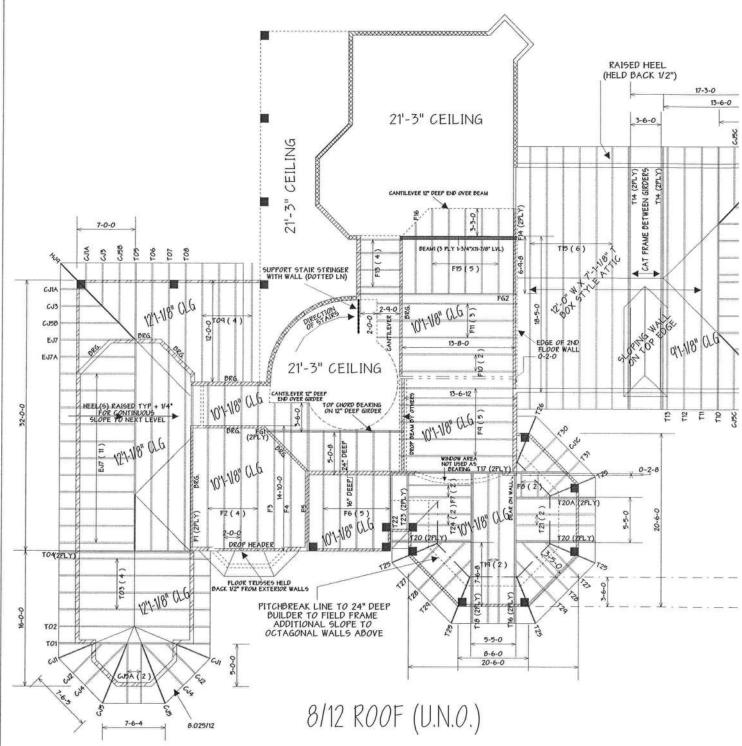
### 31/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.





Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7" FIRST LEVEL ROOF AND FLOOR



1 QUANT. 3 PLY 1-3/4" X 11-7/8" LVL SUPPLIE

ALL OTHER HEADERS/BEAMS/VALLEY MAT. SUPPLIED BY OTHERS

FLOOR TRUSSES 24" DEEP 24" O.C.

