	ibia County Building P	
	rmit Expires One Year From the Date	
APPLICANT ALBERT E. MONIZ  ADDRESS 455 SW CHALET TE	PHONE RRACE FT. WHITE	386.454.0855 FL 32038
OWNER ALBERT E. MONIZ	PHONE	386.454.0855
ADDRESS 455 SW CHALET TE		FL 32038
CONTRACTOR ALBERT E. MONIZ, O		
	US 27,TL TO C-138,TR TO CHALET,TR AND	
PLACE	· · · · · · · · · · · · · · · · · · ·	
TYPE DEVELOPMENT SFD/UTILITY	ESTIMATED COST OF C	CONSTRUCTION 177950.00
HEATED FLOOR AREA 3559.00	TOTAL AREA 5733.00	HEIGHT STORIES 2
FOUNDATION CONC WA	ALLS FRAMED ROOF PITCH 12'1	2 FLOOR CONC
LAND USE & ZONING A-3	MA	X. HEIGHT 35
Minimum Set Back Requirments: STREE	T-FRONT 30.00 REAR	25.00 SIDE 25.00
NO. EX.D.U. 0 FLOOD ZONE	E X DEVELOPMENT PE	RMIT NO.
PARCEL ID 19-7S-17-10025-114	SUBDIVISION	
LOT BLOCK PHASE	UNIT TO	TAL ACRES 10.01
	( ODD »	A & Monis
Culvert Permit No. Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
EXISTING 06-0386-N	<del></del>	JTH N
Driveway Connection Septic Tank Number	er LU & Zoning checked by Ap	oproved for Issuance New Resident
COMMENTS: NOC ON FILE. 1 FOOT ABO	VE ROAD.	
COMMENTS: NOC ON FILE. 1 FOOT ABO	VE ROAD.	Check # or Cash 9484
		Chock ii of Cash
FOR B	BUILDING & ZONING DEPARTMEN	T ONLY (footer/Slab)
FOR B	BUILDING & ZONING DEPARTMENT Foundation	T ONLY (footer/Slab)  Monolithic
Temporary Power date/app. by	Foundation date/app. by	TONLY (footer/Slab)  Monolithic date/app. by
Temporary Power  date/app. by  Under slab rough-in plumbing	BUILDING & ZONING DEPARTMENT Foundation	T ONLY (footer/Slab)  Monolithic
Temporary Power  date/app. by  Under slab rough-in plumbing  date/a	Foundation date/app. by	TONLY (footer/Slab)  Monolithic date/app. by  Sheathing/Nailing date/app. by  od floor
Temporary Power  date/app. by  Under slab rough-in plumbing  date/a  Framing  date/app. by  Electrical rough-in	Foundation  Slab  app. by  Rough-in plumbing above slab and below woo	Monolithic
Temporary Power  date/app. by  Under slab rough-in plumbing  date/a	Foundation  Slab  app. by  Rough-in plumbing above slab and below wood  Heat & Air Duct	TONLY (footer/Slab)  Monolithic date/app. by  Sheathing/Nailing date/app. by  od floor date/app. by  Peri. beam (Lintel)
Temporary Power  date/app. by  Under slab rough-in plumbing  date/a  Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power	Foundation  Slab  app. by  Rough-in plumbing above slab and below woo	TONLY (footer/Slab)  Monolithic date/app. by  Sheathing/Nailing date/app. by  od floor date/app. by
Temporary Power  date/app. by  Under slab rough-in plumbing  date/a  Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power  date/app. by	Foundation  Foundation    date/app. by   Slab   app. by   date/app. by   Rough-in plumbing above slab and below wood   Heat & Air Duct   date/app. by   C.O. Final   date/app. by	TONLY  (footer/Slab)  Monolithic  date/app. by  Sheathing/Nailing  date/app. by  od floor  date/app. by  Peri. beam (Lintel)  date/app. by  Culvert  date/app. by
Temporary Power  date/app. by  Under slab rough-in plumbing  date/a  Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power	Foundation  Foundation    date/app. by   Slab   app. by   date/app. by   Rough-in plumbing above slab and below wood   Heat & Air Duct   date/app. by   C.O. Final   date/app. by	TONLY (footer/Slab) Monolithic
FOR B Temporary Power  date/app. by  Under slab rough-in plumbing  date/a Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power  date/app. by  M/H tie downs, blocking, electricity and plumbing	Foundation  Foundation    date/app. by     Slab     app. by   date/app. by     Rough-in plumbing above slab and below wood    Heat & Air Duct     date/app. by     C.O. Final     date/app. by     Pump pole   Utility Poles     Utility Poles     Utility Poles     Concept     Concept	TONLY  (footer/Slab)  Monolithic  date/app. by  Sheathing/Nailing  date/app. by  od floor  date/app. by  Peri. beam (Lintel)  date/app. by  Culvert  date/app. by  Pool  date/app. by
FOR B Temporary Power  date/app. by  Under slab rough-in plumbing  date/a Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power  date/app. by  M/H tie downs, blocking, electricity and plumbing  Reconnection  date/app. by  M/H Pole	Foundation  Foundation    date/app. by     Slab     app. by   date/app. by     Rough-in plumbing above slab and below wood    Heat & Air Duct     date/app. by     C.O. Final     date/app. by     date/app. by	TONLY (footer/Slab) Monolithic
FOR B Temporary Power  date/app. by  Under slab rough-in plumbing  date/a Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power  date/app. by  M/H tie downs, blocking, electricity and plumbing  Reconnection  date/app. by	Foundation  Foundation    date/app. by     Slab     app. by   date/app. by     Rough-in plumbing above slab and below wood    Heat & Air Duct     date/app. by     C.O. Final     date/app. by     Pump pole   Utility Pole     date/app. by     Utility Pole   Utility Pole     date/app. by	TONLY  (footer/Slab)  Monolithic  date/app. by  Sheathing/Nailing  date/app. by  od floor  date/app. by  Peri. beam (Lintel)  Culvert  date/app. by  Pool  date/app. by  look  date/app. by  date/app. by
FOR B Temporary Power  date/app. by  Under slab rough-in plumbing  date/a Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power  date/app. by  M/H tie downs, blocking, electricity and plumbing  Reconnection  date/app. by  M/H Pole	Foundation  Foundation    date/app. by     Slab     app. by   date/app. by     Rough-in plumbing above slab and below wood    Heat & Air Duct     date/app. by     C.O. Final     date/app. by     Pump pole   Utility Poly     date/app. by     ravel Trailer	TONLY  (footer/Slab)  Monolithic  date/app. by  Sheathing/Nailing  date/app. by  od floor  date/app. by  Peri. beam (Lintel)  Culvert  date/app. by  Pool  date/app. by  Pool  date/app. by  Re-roof
Temporary Power  date/app. by  Under slab rough-in plumbing  date/a  Framing  date/app. by  Electrical rough-in  date/app. by  Permanent power  date/app. by  M/H tie downs, blocking, electricity and plumbing  Reconnection  date/app. by  M/H Pole  date/app. by  BUILDING PERMIT FEE \$ 890.00	Foundation  Foundation    date/app. by     Slab     app. by   date/app. by     Rough-in plumbing above slab and below wood    Heat & Air Duct     date/app. by     C.O. Final     date/app. by     Pump pole   Utility Polyage     date/app. by     ravel Trailer     date/app. by	TONLY  (footer/Slab)  Monolithic  date/app. by  Sheathing/Nailing  date/app. by  od floor  date/app. by  Peri. beam (Lintel)  date/app. by  Culvert  date/app. by  Pool  date/app. by  Re-roof  date/app. by  SURCHARGE FEE \$ 28.66
Temporary Power    date/app. by	Foundation  Foundation    date/app. by     Slab     app. by   date/app. by     Rough-in plumbing above slab and below work    Heat & Air Duct     date/app. by     C.O. Final     date/app. by     Pump pole   Utility Poly     ravel Trailer     date/app. by     CERTIFICATION FEE \$ 28.66	TONLY  (footer/Slab)  Monolithic  date/app. by  Sheathing/Nailing  date/app. by  Peri. beam (Lintel)  Culvert  date/app. by  Pool  date/app. by  Pool  date/app. by  Surcharge FEE \$ 28.66

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

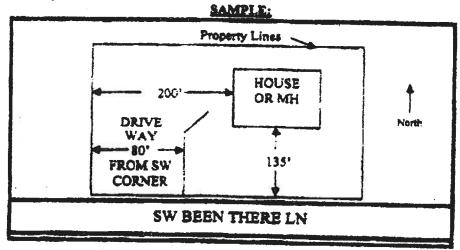
"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

## This Permit Must Be Prominently Posted on Premises During Construction

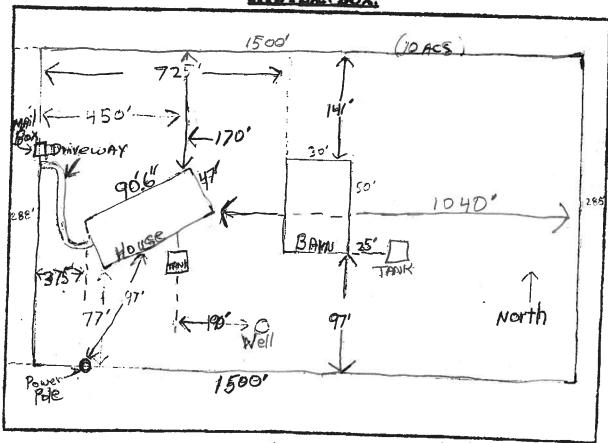
PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

Columbia County Building Po	ermit Application	<b>Hevised 9-23-0</b>
For Office Use Only Application # 0608-10 Date R	eceived <u>\$118 06</u> By Permit	24936
Application Approved by - Zoning Official BLK Date	30,08.6 Plans Examiner of THE	Date 8-3/-06
Flood Zone Development Permit Zoning		
Comments		agory
	e xisting	hiell
All I on it		
Applicants Name Hibert E. Moniz	Phone 386	454-0855
Address 455 S. W. Chalet Terr.	Ft. White, FL 3	2038
Owners Name Albert E. Moniz 911 Address 455 S. W. Chalet Terr	Phone 386)	454-0855
911 Address 435 S. W. Chalet Terr.	Ft. White, FL 3	2038
Contractors Name Wher / Builder	Phone	
Address Same as above		1
Fee Simple Owner Name & Address SAME AS AB	OVE.	
Bonding Co. Name & Address		
Architect/Engineer Name & Address John Yencho	5559 S. W. CR 313, Tre	nton, FL 32693
Mortgage Lenders Name & Address None		
Circle the correct power company - FL Power & Light - Cic	ry Elec Suwannee Valley Elec	- Progressive Energ
Property ID Number <u>R 100 25 - 114</u> /9 - 75 - 17	Estimated Cost of Construction	
Subdivision Name Ly N-Dee Dairy	104 114 m	. /
Driving Directions U.S. 27 TSO. to CR 138, TA3	8 W. to CHALEY Terr.	Chalet Fern.
No. to 455. 8th on Hight	TR	And the second of the second
Type of Construction FRAME SFD	Number of Existing Dwellings on Pro	
Total Acreage <u>10.01</u> Lot Size Do you need a - <u>Cu</u>	vert Permit or Culvert Walver or H	ave an Existing Driv
Actual Distance of Structure from Property Lines - Front 375	Ft. Side 179t Side 1706	+ Dary MUTOST
Total Building Height Number of Stories	Heated Floor Area 355959 Re	oof Pitch 12×12
Application is hereby made to obtain a permit to do work and	TOTAL 157	
instantation has commenced brior to the issuance of a Delling a	installations as indicated. I certify that all work be performed to me	at no work or let the standards of
an laws regulating construction in this jurisdiction.		
OWNERS AFFIDAVIT: I hereby certify that all the foregoing inficompliance with all applicable laws and regulating construction	ormation is accurate and all work wi	Il be done in
WARNING TO OWNER, VOUR EARLINE TO SECOND A MONTH		TIN VOIL BAVING
TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU IN LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE	TEND TO OBTAIN FINANCING, CON	ISULT WITH YOUR
AM AE m	OF COMMERCEMENT.	
Owner Builder or Agent (Including Contractor)		
€ stage-dates	Contractor Signature	
STATE OF FLORIDA COUNTY OF COLUMBIA	Contractors License Number Competency Card Number	
Sworn to (or affirmed) and subscribed before me	NOTARY STAMP/SEAL	************
this 3 day of Quesust 20 06	MY COM	BURMEISTER & MISSION # DDS36652 &
Personally known or Produced Identification	free XXXXIII	Notice Discount Assoc Co.
	Notary Signature	•••••

- 1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
- 2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
- 3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
- 4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).



### SITE PLAN BOX:



Page 2 of 2 455 S. W.Chalet Terr. Ft. Write F1. 32038 Albert + Pat Moniz 386454-0855

F.ONNIE BRANNON, CFC COLUMBIA COUNTY TAX COLLECTOR 2005 REAL ESTATE 01290230000 NOTICE OF AD VALOREM TAXES AND NON-AC VALOREM ASSESSMENTS ACCOUNT NUMBER ASSESSED VALUE EXEMPTIONS TAXABLE VALUE MILLIGE CODE ESCROW CD R10025-114 63,912 33,912 003

0011457 01 AV 0.278 \*\*AUTO T1 0 0810 32038-123 նահարկինում հետևոնների հետևոնների հետևոնների MONIZ ALBERT E SR & PATRICIA A 455 SW CHALET TERR FORT WHITE FL 32038-2557

SEE INSERT FOR IMPORTANT INFO AND TELEPHONE NUMBERS WWW.COLUMBIATAXCOLLECTOR.COM

QU12705

R

19-7S-17 4800/4800 1).01 Acres
COMM NW COR, RUN W 1111.14 FT
TO SW R/W ABANDONED SCL RR,
SE ALONG R/W TO ITS INTERS
WITH W R/W BOB-CAT LANE, RUN S
3438.08 FT FOR POB, CONT S

<i>(</i>		
	AD VALOR	EM TAXES
FAKING AUTHORITY	MILLAGE RATE (DOLL)	ARS PER \$1,000 OF TAXABLE VALUE
Troot Boyling of Cookill Cokkill 19910MERS	8.7260	557.70
S002 COLUMBIA COUNTY SCHOOL BOARD DISCRETIONARY	7000	37:.70
LOCAL	.7600 5.1950	48.57
CAPITAL OUTLAY	2.0000	332.02
W SR SUWANNEE RIVER WATER MGT DIST	.4914	127,83
HLSH SHANDS AT LAKE SHORE	1.7500	31.41
IIDA INDUSTRIAL DEVELOPEMENT AUTH	.1380	8.82
		V
		la). Acar
		0. 00./0- 25
		Pd. de. 156- 29-25
		1 26 11
		T re

TOTAL VILLAGE 19.0604 ADVILORENT MES \$1.318.20	
NON-AD VALOREM ASSESSMENTS RATE	(S) (E)
FFIR FIRE ASSESSMENTS	(C)

PAY ONLY ONE AMOUNT IN YELLOW SHADED AREA NON-AD VALOREM ASSESSMENTS \$67.99 Sae reverse side for COMBINED TAXES AND ASSESSMENTS PAY ONLY \$1,286.19 important information. ONE AMOUNT Mar 31 IF PAID BY Feb 28 Nov 30 Dec 31 Jan 31 F PAID PLEASE PAY 1,234.74 1,260.47 1,247.60 1,.86.19

		1	
- * *	285'	STATE OF ELOSIE	
		STATE OF FLORIDA DEPARTMENT OF HEALTH	
	APPLICATION FOR	PNSITE SEWAGE DISPOSAL SYSTEM CONST	RUCTION PERMIT
		Permit Application	Number 06-0386
• •		PART II - SITEPLAN	
Scale: 1 inch	= 50 feet	TOTAL BOUT SOLUTION	
	00 /eet.		
			L D1
	NORTH	65	17/
	Molert	em .	
	0-1	WELL 1 62' 8M	
	BARN WELL	1513 H2DI	<u>k</u>
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Notes:	// 386	L 2 of 10 ACRES	
Site Plan subm	thad but O 1	77-0	
oite Pian subm Pian Approved	MITTO	Not Approved	MASTER CONTRACTOR
By M	· · · · · · · · · · · · · · · · · · ·	Not Approved	Date 4/24/06
	· · · · · · · · · · · · · · · · · · ·	Columbia	County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

### DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$25,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

	() Two-Family Residence () Other
() New Construction	() Addition, Alteration, Modification or other Improvement
NEW C	ONSTRUCTION OR IMPROVEMENT
for exemption from contractor ficens	n, have been advised of the above disclosure statement ing as an owner/builder. I agree to comply with all requirements 39.103(7) allowing this exception for the construction permitted by tumber
Ollet & M Signature	ong 8/18/06 Date
I hereby certify that the above listed Florida Statutes ss 489.103(7).	FOR BUILDING USE ONLY owner/builder has been notified of the disclosure statement in

Building Official/Representative\_\_\_\_

# NOTICE OF COMMENCEMENT FORM COLUMBIA COUNTY, FLORIDA

# \*\*\* THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION \*\*\*

TH	IE UNDERSIGNED basebu alua	SEEMAS OFFICE BEFORE YOUR FIRST INSPECTION.***
wi	th Chapter 713, Florida Statutes, the follow	improvement will be made to certain real property, and in accordance ing information is provided in this Notice of Commencement.
Ta	x Parcel ID Number <u>R10025-114</u>	
		PERMIT NUMBER 06 08-7
1.	Description of property: (legal description	of the property and street address or 911 address)
	COMM NW COR, RUN W 1111 14 FT	4555 s. W. Chalet Terr
	TO SW R/W ABANDONED SCL RR, SE ALONG R/W TO ITS INTERS	433 3 W Chalet Terr
	TILL VY DIVY KOR CATLAND S 9	Fort White FL 32038
	288.21 FT W 1513.00 FT W	
	288.26 FT, E 1510.40 FT TO POB	
2.	General description of improvement:	HOME DWEILING
		MONIZ, 455 S. W. Chalet Terrace,
٠.	M. White Fl 32020	1.10N12, 453 S. W. Chalet Terrace,
4.	Name & Address of Fee Simple Owner (if	other than owner):
		•
5.	Contractor Name Habert E MON	Phone Number
	Address 435 Sw Chalet	lerrace Hulbuta El 22120
8.	ONITY HOUSE RAINE /VU//C_	
	Address	Inch good cooper
	· """ - "" - " - " - " - " - " - " - " -	UL P HOUR ET CAGAS OF LAI
7.	Lender Name None	P:1511
	Address	The state of the s
8.	Persons within the State of Florida designated as provided by seed as provided by	ated by the Owner upon whom notices or other documents may be
	ved as provided by section 718.13 (1)(a) 7;	Florida Statutes:
	Address	Phone Number
9.	in addition to himself/herself the owner de	
		eive a copy of the Lienor's Notice as provided in Section 713.13 (1) -
10.	Expiration date of the Notice of Commence	ement (the expiration date is 1 (one) year from the date of recording,
	(Unless a different data is appointed)	ement (the expiration date is 1 (one) year from the date of recording,
		oment (the expiration date is 1 (one) year from the date of recording,
101	ICF AS DED CHARTER SAS SI	
he	owner must sign the notice of commencer	
	and a manufaction	ent and no one else may be permitted to sign in his/her stead.
	<b>A B R B</b>	
	Court & m.	Swom to (or affirmed) and subscribed before day of
100	Signature of Course	NOTABY STANDARD
,	C.R. armia of OMUSI	NOTARY STANDARY NOEL BURMEISTER  NOEL BURMEISTER  MY COMMISSION # DD536652  EXPIRES: June 22, 2010

Signature of Notary

**ALBERT MONIZ** 

24936 commed

Project Name:

Address:

City, State:

MONIZ

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Builder:

compliance with Section 553.908

**BUILDING OFFICIAL:** \_\_\_\_

Florida Statutes.

Permitting Office:

Permit Number:

Owner:	Jurisdiction Number: 22/000
Climate Zone: North	
New construction or existing	12. Cooling systems
2. Single family or multi-family Single family	a. Central Unit Cap: 28.8 kBtu/hr
3. Number of units, if multi-family	SEER: 15.00
4. Number of Bedrooms 2	b. Central Unit Cap: 28.6 kBtu/hr
5. Is this a worst case? Yes	SEER: 13.00
6. Conditioned floor area (ft²) 3559 ft²	c. N/A
7. Glass type 1 and area: (Label reqd. by 13-104.4.5 if not default)	_
a. U-factor: Description Area	13. Heating systems
(or Single or Double DEFAULT) 7a. (Dble Default) 412.0 ft <sup>2</sup>	a. Electric Heat Pump Cap: 28.6 kBtu/hr
b. SHGC:	HSPF: 8.50
(or Clear or Tint DEFAULT) 7b. (Clear) 412.0 ft <sup>2</sup>	b. Electric Heat Pump Cap: 28.8 kBtu/hr
8. Floor types	HSPF: 8.50
a. Raised Wood, Adjacent R=11.0, 1656.0ft <sup>2</sup>	c. N/A
b. N/A	
c. N/A	14. Hot water systems
9. Wall types	a. LP Gas Cap: 50.0 gallons
a. Frame, Wood, Exterior R=19.0, 2152.0 ft <sup>2</sup>	EF: 0.66
b. Frame, Wood, Adjacent R=13.0, 195.0 ft <sup>2</sup>	b. N/A
c. N/A	_
d. N/A	c. Conservation credits
e. N/A	(HR-Heat recovery, Solar
10. Ceiling types	DHP-Dedicated heat pump)
a. Under Attic R=30.0, 1656.0 ft <sup>2</sup>	15. HVAC credits
b. Under Attic R=19.0, 660.0 ft <sup>2</sup>	(CF-Ceiling fan, CV-Cross ventilation,
c. N/A	HF-Whole house fan,
11. Ducts	PT-Programmable Thermostat,
a. Sup: Unc. Ret: Unc. AH: Attic Sup. R=6.0, 224.0 ft	MZ-C-Multizone cooling,
b. Sup: Unc. Ret: Unc. AH: Outdoors Sup. R=6.0, 188.0 ft	MZ-H-Multizone heating)
Total as built	points: 35451
Glass/Floor Area: 0.12 Total as-built	points: 39451 points: 39256 PASS
Total baco	50816. 00200
I hereby certify that the plans and specifications covered by	Review of the plans and
this calculation are in compliance with the Florida Energy	specifications covered by this
Code. SUNCOAST INSULATIONS	calculation indicates compliance
PREPARED BY: , November 1, 1999	with the Florida Energy Code.
DATE: 8/9/00 (362) 472-6895	Before construction is completed
Pax (382) 472-2033	this building will be inspected for

I hereby certify that this building, as designed, is in

compliance with the Florida Energy Code.

OWNER/AGENT: \_\_\_\_\_

DATE: \_\_

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: PERMIT#:	
ADDRESS.,,,	

	BASE					AS-	BUI	LT				
GLASS TYPES .18 X Condition Floor Are		SPM =	Points	Type/SC	Ove Omt	erhang Len		Area X	SP	м×	SOF	= Points
.18 3559.0	0	20.04	12838.0	Double, Clear	W	2.0	6.0	220.0	38.	52	0.85	7198.9
				Double, Clear	S	2.0	6.0	8.0	35.		0.78	222.7
Ĭ				Double, Clear	N	2.0	6.0	70.0	19.		0.90	1209.7
				Double, Clear	E	2.0	6.0	114.0	42.	06	0.85	4066.5
				As-Built Total:				412.0				12697.8
WALL TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	X	SPN	1 =	Points
Adjacent	195.0	0.70	136.5	Frame, Wood, Exterior			19.0	2152.0		0.90		1936.8
Exterior	2152.0	1.70	3658.4	Frame, Wood, Adjacent,			13.0	195.0		0.60		117.0
Base Total:	2347.0		3794.9	As-Built Total:				2347.0				2053.8
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	Х	SPN	/1 =	Points
Adjacent	20.0	2.40	48.0	Exterior Insulated				44.0		4.10		180.4
Exterior	44.0	6.10	268.4	Adjacent Insulated				20.0		1.60		32.0
Base Total:	64.0		316.4	As-Built Total:				64.0				212.4
CEILING TYPES	S Area X	BSPM	= Points	Туре	1	R-Valu	ie /	Area X	SPM	X S	CM =	Points
Under Attic	1656.0	1.73	2864.9	Under Attic			30.0	1656.0	1.73	X 1.00		2864.9
1				Under Attic			19.0	660.0	2.34	X 1.00		1544.4
Base Total:	1656.0		2864.9	As-Built Total:				2316.0				4409.3
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	X	SPN	1 =	Points
Slab	0.0(p)	0.0	0.0	Raised Wood, Adjacent			11.0	1656.0		0.70		1159.2
Raised	1656.0	-3.99	-6607.4									
Base Total:			-6607.A	As-Built Total:			,	1656.0				1159.2
INFILTRATION	Area X	BSPM	= Points					Area	X	SPN	A =	Points
	3559.0	10.21	36337.4					3559.	.0	10.2	1	36337.4

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

		· · · · · · · · · · · · · · · · · · ·
ADDRESS:,,,	PERMIT #:	•

BASE			AS-BUILT				
Summer Bas	se Points:	49544.2	Summer As-Built Points: 56869.8				
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)				
			(sys 1: Central Unit 28800 btuh ,SEER/EFF(15.0) Ducts:Unc(S),Unc(R),Att(AH),R6.0(INS)				
·			56870 0.50 (1.09 x 1.147 x 1.11) 0.228 1.000 8676.6				
ł .			(sys 2: Central Unit 28600 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Out(AH),R6.0(INS)				
49544.2	0.4266	21135.5	56870       0.50 (1.09 x 1.147 x 1.02)       0.263 1.000 9941.9         56869.8       1.00 1.336 0.244 1.000 18523.5				

# **WINTER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS:,,,
PERMIT #:

	BASE					AS-	BUI	LT				
GLASS TYPES .18 X Condition Floor Are		WPM =	Points	Type/SC	Ove Omt	erhang Len		Area X	w	РМ Х	WOI	= = Point
.18 3559.	0	12.74	8161.5	Double, Clear	W	2.0	6.0	220.0	20	).73	1.04	4755.0
				Double, Clear	S	2.0	6.0	8.0		3.30	1.26	133.9
				Double, Clear	N	2.0	6.0	70.0		1.58	1.00	1728.5
				Double, Clear	Ε	2.0	6.0	114.0	18	3.79	1.06	2272.1
				As-Built Total:				412.0				8889.6
WALL TYPES	Area X	BWPM	= Points	Туре		R-\	/alue	Area	Х	WPM	=	Points
Adjacent	195.0	3.60	702.0	Frame, Wood, Exterior			19.0	2152.0		2.20		4734.4
Exterior	2152.0	3.70	7962.4	Frame, Wood, Adjacent			13.0	195.0		3.30		643.5
Base Total:	2347.0		8664.4	As-Built Total:				2347.0				5377.9
DOOR TYPES	Area X	BWPM	= Points	Туре				Area	X	WPM	=	Points
Adjacent	20.0	11.50	230.0	Exterior Insulated				44.0		8.40		369.6
Exterior	44.0	12.30	541.2	Adjacent Insulated				20.0		8.00		160.0
Base Total:	64.0		771.2	As-Built Total:				64.0				529.6
CEILING TYPES	SArea X	BWPM	= Points	Туре	R-	Value	Ar	ea X W	PM	x wc	M =	Points
Under Attic	1656.0	2.05	3394.8	Under Attic			30.0	1656.0	2.05	5 X 1.00		3394.8
				Under Attic			19.0	660.0	2.70	X 1.00		1782.0
Base Total:	1656.0		3394.8	As-Built Total:				2316.0				5176.8
FLOOR TYPES	Area X	BWPM	= Points	Туре		R-\	/alue	Area	X	WPM	=	Points
Slab	0.0(p)	0.0	0.0	Raised Wood, Adjacent			11.0	1656.0		3.60		5961.6
Raised	1656.0	0.96	1589.8									1200
Base Total:			1589.8	As-Built Total:				1656.0				5961.6
INFILTRATION	Area X	BWPM	= Points					Area	X	WPM	=	Points
	3559.0	-0.59	-2099.8					3559.	0	-0.59		-2099.8

# **WINTER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT #:

BASE			AS-BUILT					
Winter Base	Points:	20481.8	Winter As-Built Points:	23835.6				
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Heating r Points				
20481.8	0.6274	12850.3	(sys 1: Electric Heat Pump 28600 btuh ,EFF(8.5) Ducts:Unc(S),Unc(R),Att(/ 23835.6	6467.9				

# **WATER HEATING & CODE COMPLIANCE STATUS**

Residential Whole Building Performance Method A - Details

ADDRESS:,,,
PERMIT #:

	ASE					AS-BUIL	.T			
WATER HEA Number of Bedrooms	TING	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank X Ratio	Multiplier X	Credit Multipli	
2	-	2635.00	5270.0	50.0	0.66	2	1.00	1973.45	1.00	3946.9
				As-Built To	tal:					3946.9

	CODE COMPLIANCE STATUS												
BASE					AS-BUILT								
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
21136		12850		5270		39256	18523		12981		3947		35451

**PASS** 



# **Code Compliance Checklist**

# Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT #:

### 6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall;	
		foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility	
		penetrations; between wall panels & top/bottom plates; between walls and floor.	
		EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends	
		from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.	1
		EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed	
ł		to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases,	
		soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate;	
		attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is	
		installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a	
		sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from	
		conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA,	
		have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit	
		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools	
		must have a pump timer. Gas spa & pool heaters must have a minimum thermal	
		efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically	
		attached, sealed, insulated, and installed in accordance with the criteria of Section 610.	
		Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.	
		Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

### ESTIMATED ENERGY PERFORMANCE SCORE\* = 84.4

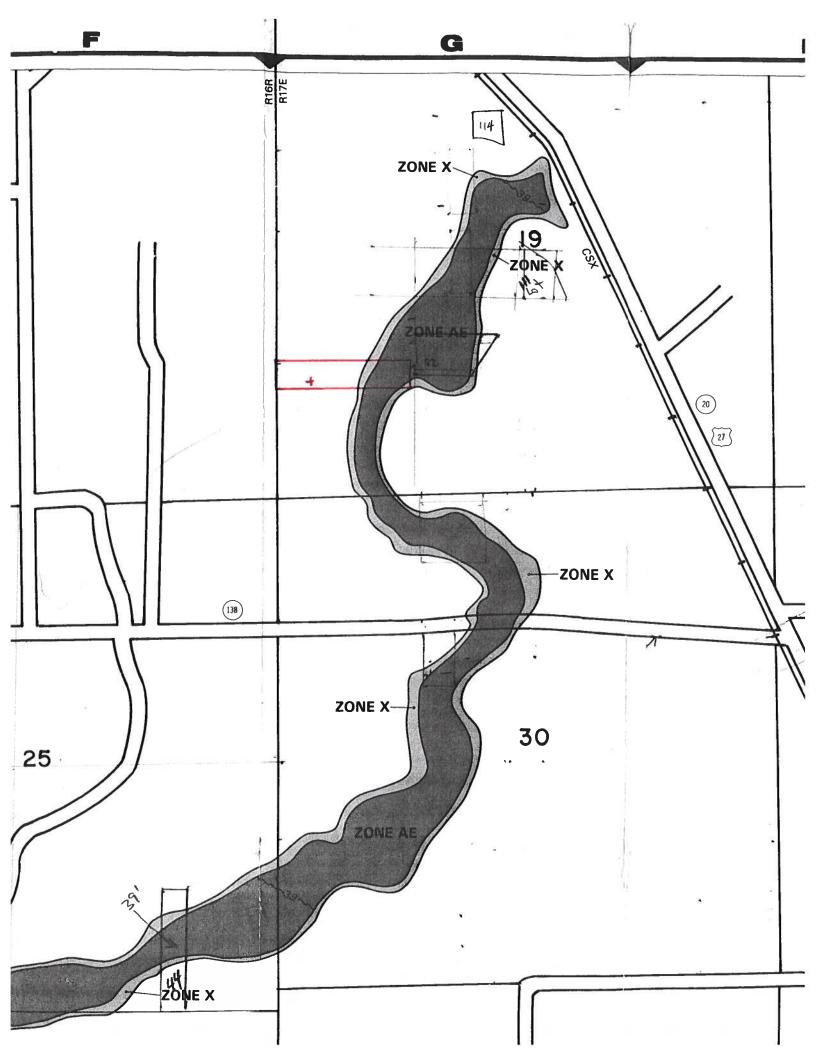
The higher the score, the more efficient the home.

i.			,,,,			
1.	New construction or existing	New	= 13	2. Cooling systems		
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 28.8 kBtu/hr	
3.	Number of units, if multi-family	1	_		SEER: 15.00	
4.	Number of Bedrooms	2		b. Central Unit	Cap: 28.6 kBtu/hr	
5.	Is this a worst case?	Yes			SEER: 13.00	
6.	Conditioned floor area (ft²)	3559 ft²		c. N/A		
7.	Glass type 1 and area: (Label reqd.	by 13-104.4.5 if not default)				
a.	U-factor:	Description Area	13	3. Heating systems		
	(or Single or Double DEFAULT)	7a. (Dble Default) 412.0 ft <sup>2</sup>		a. Electric Heat Pump	Cap: 28.6 kBtu/hr	_
b.	SHGC:				HSPF: 8.50	_
	(or Clear or Tint DEFAULT)	7b. (Clear) 412.0 ft <sup>2</sup>		b. Electric Heat Pump	Cap: 28.8 kBtu/hr	
8.	Floor types				HSPF: 8.50	
a.	Raised Wood, Adjacent	$R=11.0, 1656.0 ft^2$		c. N/A		-
b.	N/A					-
C.	N/A		1	4. Hot water systems		
	Wall types			a. LP Gas	Cap: 50.0 gallons	
	Frame, Wood, Exterior	R=19.0, 2152.0 ft <sup>2</sup>			EF: 0.66	
b.	Frame, Wood, Adjacent	$R=13.0, 195.0  ft^2$		b. N/A		_
	N/A					-
	N/A		-	c. Conservation credits		_
	N/A			(HR-Heat recovery, Solar		
	Ceiling types			DHP-Dedicated heat pump)		
a.	Under Attic	R=30.0, 1656.0 ft <sup>2</sup>	1:	5. HVAC credits		_
	Under Attic	R=19.0, 660.0 ft <sup>2</sup>	_	(CF-Ceiling fan, CV-Cross ventilation,		
	N/A		-	HF-Whole house fan,		
	Ducts			PT-Programmable Thermostat,		
	Sup: Unc. Ret: Unc. AH: Attic	Sup. R=6.0, 224.0 ft		MZ-C-Multizone cooling,		
b.	Sup: Unc. Ret: Unc. AH: Outdoor	s Sup. R=6.0, 188.0 ft	-	MZ-H-Multizone heating)		
I ce	rtify that this home has compl	ied with the Florida Energ	v Efficie	ency Code For Building		
	istruction through the above e				OF THE STATE	à
	his home before final inspection					
	ed on installed Code complian		Display	Cuta vini oo oompiotoa		5
	-		Data			
Bui	lder Signature:		Date: _		10 Line	

\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStd<sup>M</sup> designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

City/FL Zip:

Address of New Home:





From: The Columbia County Building & Zoning Department

Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529

Reference to a building permit application Number: 0608–70

Owner/Builder Albert E. Moniz Property ID# 19-7s-17-10025-114

On the date of August 24, 2006 application 0608-70 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0608-70 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

- **1.** Please show the total height of the structure from the established grade to the highest roof ridge.
- 2. The garage area shall meet the following FRC requirements.

- A. Sections R309.1 openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.
- **B.** This section (R309.1) will also apply to the door from the garage to the HVAC equipment room.
- R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.
- 3. Please provide a drawing of all the staircases detailing the requirements of the FRC- R311.5.3 Stair treads and risers. (See attached section FRC-R311.5.3)
- **4.** Please verify that in each second story bedroom one window will comply with section FBC-2004 Section R310.1.1 Minimum opening area: All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m2). The minimum net clear opening

- height shall be 24 inches (610 mm): R310.1.3 Minimum opening width.

  The minimum net clear opening width shall be 20 inches (508 mm).
- R408.3 Access shall be provided to all under-floor spaces. Access openings through the floor shall be a minimum of 18 inches by 24 inches (457 mm by 610 mm). Openings through a perimeter wall shall be 16 inches by 24 inches (407 mm by 610 mm). When any portion of the through wall access is below grade, an areaway of not less than 16 inches by 24 inches (407 mm by 610 mm) shall be provided. The bottom of the areaway shall be below the threshold of the access opening. Through wall access openings shall not be located under a door to the residence. See M1305.1.4 for access requirements where mechanical equipment is located under floors.
- Openings for under-floor ventilation: The minimum net area of ventilation openings shall not be less than 1 square foot (0.0929 m2) for each 150 square feet (100 m2) of under-floor space area. One such ventilating opening shall be within 3 feet (914 mm) of each corner of the building. Ventilation openings shall be covered for their height and width with any of the following materials provided that the least dimension of the covering shall not exceed ¼ inch (6.4 mm):
  - 1. Perforated sheet metal plates not less than 0.070 inch (1.8 mm) thick.
  - 2. Expanded sheet metal plates not less than 0.047 inch (1.2 mm) thick.

- 3. Cast iron grills or grating.
- 4. Extruded load-bearing brick vents.
- 5. Hardware cloth of 0.035 inch (0.89 mm) wire or heavier.
- Corrosion-resistant wire mesh, with the least dimension being 1/8 inch(3.2 mm).
- 7. On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
- 8. Show on the electrical plans as required by the National Electrical Code article 210.12 that all branch circuits supplying outlets installed within a bedroom shall be protected by Arc-Fault interrupter devices.
- **9.** Please show compliance the FRC -2004 sections R313.1 Smoke alarms. Smoke alarms shall be installed in the following locations:
  - 1. In each sleeping room.
  - 2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.
- 3. On each additional story of the dwelling, including basements but not including crawl spaces and uninhabitable attics. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a

smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.

When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit. The alarm shall be clearly audible in all bedrooms over background noise levels with

All smoke alarms shall be listed and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

all intervening doors closed.

Joe Haltiwanger

Columbia County
Plan Examiner

Sept. 1, 2007

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit #000024936, located at 455 S.W. Chalet Terr., Ft. White, Fl.

Thank you for your consideration to this request.

Sincerely Yours,

Dec. 1, 2007

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit #000024936, located at 455 S.W. Chalet Terr., Ft. White, Fl.

Thank you for your consideration to this request.

Sincerely Yours,

March 1, 2008

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Ollet E. Moniz

May 29, 2008

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Sept. 1, 2008

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Dec. 1st, 2008

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

February 27, 2009

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

May 28, 2009

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Aug. 28, 2009

Columbia County Bldg. Dept. 135 N.E. Hernando Ave., Su.B-21 Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,



# **COLUMBIA COUNTY, FLORIDA**

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Building permit No. 000024936

Parcel Number 19-7S-17-10025-114

Use Classification SFD/UTILITY

Permit Holder ALBERT E. MONIZ, OWNER BLDR.

Owner of Building ALBERT E. MONIZ

318.67

Total:

Waste: 184.25

Fire:

Location: 455 SW CHALET TERR., FT. WHITE, FL

Date: 11/16/2009



**POST IN A CONSPICUOUS PLACE** Business Places Only Pex Pipe Pg. #2

Florida Building Code, 2004 Complete Collection

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.4 Water service pipe.

### P2904.4 Water service pipe.

Water service pipe shall conform to NSF 61 and shall conform to one of the standards listed in Table P2904.4.1. Water service pipe or tubing, installed underground and outside of the structure, shall have a minimum working pressure rating of 160 psi at 73°F (1100 kPa at 23°C).

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.4 Water service pipe. / P2904.4.1 Water service installation.

### P2904.4.1 Water service installation.

Trenching, pipe installation and backfilling shall be in accordance with Section P2604. Water-service pipe is permitted to be located in the same trench with a building sewer provided such sewer is constructed of materials listed for underground use within a building in Section P3002.1. If the building sewer is not constructed of materials listed in Section P3002.1, the water-service pipe shall be separated from the building sewer by a minimum of 5 feet (1524 mm), measured horizontally, of undisturbed or compacted earth or placed on a solid ledge at least 12 inches (305 mm) above and to one side of the highest point in the sewer line.

Exception: The required separation distance shall not apply where a water service pipe crosses a sewer pipe, provided the water service pipe is sleeved to at least 5 feet (1524 mm), horizontally from the sewer pipe centerline, on both sides of such crossing with pipe materials listed in Tables P2904.4.1, P3002.1 or P3002.2.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.4 Water service pipe. / P2904.4.1 Water service installation. / TABLE P2904.4.1 WATER SERVICE PIPE

# TABLE P2904.4.1 WATER SERVICE PIPE

MATERIAL	STANDARD
Acrylonitrile butadiene styrene (ABS) plastic pipe	ASTM D 1527; ASTM D 2282
Asbestos-cement pipe	ASTM C 296
Brass pipe	ASTM B 43
Chlorinated polyvinyl chloride (CPVC) plastic pipe	ASTM D 2846; ASTM F 441; ASTM F 442; CSA B137.6
Copper or copper-alloy pipe	ASTM B 42; ASTM B 302
Copper or copper-alloy tubing (Type K, WK, L, WL, M or WM)	ASTM B 75; ASTM B 88; ASTM B 251; ASTM B 447

MATERIAL	STANDARD
Cross-linked polyethylene/aluminum/cross-linked polyethylene (PEX-AL-PEX) pipe	ASTM F 1281; CSA B137.10
Cross-linked polyethylene (PEX) plastic tubing	ASTM F 876; ASTM F 877; CSA B137.5
Ductile iron water pipe	AWWA C151; AWWA C115
Galvanized steel pipe	ASTM A 53
Polybutylene (PB) plastic pipe and tubing	ASTM D 2662; ASTM D 2666; ASTM D 3309; CSA B13
Polyethylene/aluminum/polyethylene (PE-AL-PE) pipe	ASTM F 1282; CSA B137.9M
Polyethylene (PE) plastic pipe	ASTM D 2239; CSA-B137.1
Polyethylene (PE) plastic tubing	ASTM D 2737; CSA B137.1
Polyvinyl chloride (PVC) plastic pipe	ASTM D 1785; ASTM D 2241; ASTM D 2672; CSA B13
Stainless steel (Type 304/304L) pipe	ASTM A312; ASTM A778
Stainless steel (Type 316/316L) pipe	ASTM A312; ASTM A778

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.5 Water-distribution pipe.

### P2904.5 Water-distribution pipe.

Water-distribution piping within dwelling units shall conform to NSF 61 and shall conform to one of the standards listed in Table P2904.5. All hot-water-distribution pipe and tubing shall have a minimum pressure rating of 100 psi at 180°F (689 kPa at 82°C).

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.5 Water-distribution pipe. / TABLE P2904.5 WATER DISTRIBUTION PIPE

# TABLE P2904.5 WATER DISTRIBUTION PIPE

MATERIAL	STANDARD
Brass pipe	ASTM B 43
Chlorinated polyvinyl chloride (CPVC) plastic pipe and tubing	ASTM D 2846; ASTM F 441; ASTM F 442; CSA
Copper or copper-alloy pipe	ASTM B 42; ASTM B 302
Copper or copper-alloy tubing (Type K, WK, L, WL, M or WM)	ASTM B 75; ASTM B 88; ASTM B 251; ASTM B 447
Cross-linked polyethylene (PEX) plastic tubing	ASTM F 877; CSA B137.5
Cross-linked polyethylene/aluminum/cross-linked polyethylene (PEX-AL-PEX) pipe	ASTM F 1281; CSAB137.10
Galvanized steel pipe	ASTM A 53
Polybutylene (PB) plastic pipe and tubing	ASTM D 3309; CSA B137.8
Polyethylene/aluminum/polyethylene (PE-AL-PE) composite pipe	ASTM F 1282
Stainless steel (Type 304/304L) pipe	ASTM A312; ASTM A778

MATERIAL	STANDARD
Stainless steel (Type 316/316L) pipe	ASTM A312; ASTM A778

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.5 Water-distribution pipe. / P2904.5.1 Under concrete slabs.

### P2904.5.1 Under concrete slabs.

Inaccessible water distribution piping under slabs shall be copper water tube minimum Type M, brass, ductile iron pressure pipe, cross-linked polyethylene/aluminum/cross-linked polyethylene (PEX-AL-PEX) pressure pipe, chlorinated polyvinyl chloride (CPVC) or polybutylene (PB) or cross-linked polyethylene (PEX) plastic pipe or tubing—all to be installed with approved fittings or bends. The minimum pressure rating for plastic pipe or tubing installed under slabs shall be 100 psi at 180°F (689 kPa at 82°C).

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.6 Fittings.

### P2904.6 Fittings.

Pipe fittings shall be approved for installation with the piping material installed, and shall conform to the respective pipe standards listed in Table P2904.6. Pipe fittings utilized in the water supply system shall also conform to NSF 61.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.6 Fittings. / TABLE P2904.6 PIPE-FITTINGS

### TABLE P2904.6 PIPE FITTINGS

Acrylonitrile butadiene styrene (ABS) plastic	ASTM D 2468
Brass	ASTM F1974
Cast-iron	ASME B16.4; ASME B16.12
Chlorinated polyvinyl chloride (CPVC) plastic	ASTM F 437; ASTM F 438; ASTM F 439
Copper or copper alloy	ASME B16.15; ASME B16.18; ASME B16.22; ASME B16.23; ASME B16.26; ASME B16.29
Fittings for cross-linked polyethylene (PEX) plastic tubing	ASTM F 1807; ASTM F 1960; ASTM F 2080
Gray iron and ductile iron	AWWA C110; AWWA C153
Malleable iron	ASME B16.3
Polyethylene (PE) plastic	ASTM D 2609
Polyvinyl chloride (PVC) plastic	ASTM D 2464; ASTM D 2466; ASTM D 2467; CSA B137.2

Stainless steel (Type 304/304L) pipe	ASTM A312; ASTM A778
Stainless steel (Type 316/316L) pipe	ASTM A312; ASTM A778
Steel	ASME B16.9; ASME B16.11; ASME B16.28

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints.

### P2904.9 Plastic pipe joints.

Joints in plastic piping shall be made with approved fittings by solvent cementing, heat fusion, corrosion-resistant metal clamps with insert fittings or compression connections. Flared joints for polyethylene pipe are permitted in accordance with Section P2904.3.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing.

### P2904.9.1 Solvent cementing.

Solvent-cemented joints shall comply with Sections P2904.9.1.1 through P2904.9.1.3.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.1 ABS plastic pipe.

### P2904.9.1.1 ABS plastic pipe.

Solvent cement for ABS plastic pipe conforming to ASTM D 2235 shall be applied to all joint surfaces.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.2 CPVC plastic pipe.

### P2904.9.1.2 CPVC plastic pipe.

Solvent cement for CPVC plastic pipe conforming to ASTM F 493 shall be applied to all joint surfaces. The joint shall be made while the cement is wet, and in accordance with ASTM D 2846 or ASTM F 493. Solvent-cement joints shall be permitted above or below ground.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS /

P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.3 PVC plastic pipe.

### P2904.9.1.3 PVC plastic pipe.

A primer complying with ASTM F 656 shall be applied to all PVC solvent cemented joints. Solvent cement for PVC plastic pipe conforming to ASTM D 2564 shall be applied to all joint surfaces.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.4 Cross-linked polyethylene plastic (PEX).

### P2904.9.1.4 Cross-linked polyethylene plastic (PEX).

Joints between cross-linked polyethylene plastic tubing or fittings shall comply with Section P2904.9.1.4.1 or Section P2904.9.1.4.2.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.4 Cross-linked polyethylene plastic (PEX). / P2904.9.1.4.1 Flared joints.

### P2904.9.1.4.1 Flared joints.

Flared pipe ends shall be made by a tool designed for that operation.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.4 Cross-linked polyethylene plastic (PEX). / P2904.9.1.4.2 Mechanical joints.

### P2904.9.1.4.2 Mechanical joints.

Mechanical joints shall be installed in accordance with the manufacturer's instructions. Fittings for cross-linked polyethylene (PEX) plastic tubing as described in ASTM F 1807, ASTM F 1960 and ASTM F 2080 shall be installed in accordance with the manufacturer's instructions.



5602 N.W. 13th STREET GAINESVILLE, FLORIDA 32653-2198

#24836

PO. BOX 5875 GAINESVILLE, FLORIDA 32627-5875

> PHONE (352) 373-3642 FAX (352) 373-9037

# CERTIFICATE OF PROTECTIVE TREATMENT

Biterthin	Date:	11-30-06 Time:	5	1
Bitest T Bitest T O6B # Gallons	Site Location:	15 /		
Biteat T . 0602 # Gallons	Area Treated:	Porches		
G600 # Gallons	Product Used:	Riter I Chemical Used: Bit Pathrin	es in	
Applicator:	% Concentration:	. 06 % # Gallons Used: 330		7.4
	Applicator:	Jora		



5602 N.W. 13th STREET GAINESVILLE, FLORIDA 32653-2198

#24936

P.O. BOX 5875 GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642 FAX (352) 373-9037

Builder:	Albert Manie			
Date:	108-06	Time:	AM	PM
Site Location:	455 SW Chalet	5+		
Area Treated:	Tostoha		77 0 0	
Product Used:	BiterIT	Chemical Used:	Bitathrin	
% Concentration:	. O6 % # Gallons Used:	. Used:	300	
Applicator:	LIPS.			



5602 N.W. 13th STREET GAINESVILLE, FLORIDA 32653-2198

# 24936

P.O. BOX 5875 GAINESVILLE, FLORIDA 32627-5875

	PHONE (352) 373-364 FAX (352) 373-9037
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PPO.	PVC	
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	berg lass	ICYNEY + Spider

# Albert & Patricia Moniz HVAC Load Calculations

for

Albert & Patricia Moniz 455 Chalet Terr Ft White FI 32038



RHVAC RESIDENTIAL HVAC LOADS

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Prepared By:
Chuck Fischer
North Central Florida Air Conditioning
P.O Box 700
High Springs FI 32655-0700
386-454-4767
Thursday, May 25, 2006



Elite Software Development, Inc. Albert & Patricia Moniz Page 2

# Project Report

General Project Information

C:\Documents and Settings\Heat\My Documents\Projects\AutoLoad MJ8.rhy Project Filename:

**Project Title:** 

Albert & Patricia Moniz

Designed By: **Project Date:** 

**Chuck Fischer** 

Client Name:

May 26,2006 Albert & Patricia Moniz

Client Address:

455 Chalet Terr

**Client City:** 

Ft White FI 32038

Client Phone:

386-454-0855

**Client Comment:** 

Company Name:

North Central Florida Air Conditioning

Company Representative: Company Address:

**Chuck Fischer** P.O Box 700

Company City:

High Springs FI 32655-0700

Company Phone:

386-454-4767

Company Fax:

386-454-4854

**Company Comment:** 

Bedroom 2,3 & 4 R/A are 10x10x8 Master bedroom R/A is 12x12x9 Main R/A is 20x20x18

Design Data

Reference City:

Gainesville, Florida

Daily Temperature Range:

Medium

Latitude:

29 Degrees 152 ft.

Elevation:

Altitude Factor:

0.995

Elevation Sensible Adj. Factor:

1.000

Elevation Total Adj. Factor:

Elevation Heating Adj. Factor:

1.000 1.000

Elevation Heating Adj. Factor:

1.000

Winter:	
Summer:	

Outdoor	Outdoor	Indoor	Indoor	Grains
Dry Bulb	Wet Bulb	Rel.Hum	Dry Bulb	<b>Difference</b>
31	0	0	68	0
93	77	50	75	50

Indoor

Check Flaures

Total Building Supply CFM:	1,814	CFM Per Square ft.:
Square ft. of Room Area:	3,033	Square ft. Per Ton:
Volume (ft³) of Cond. Space:	27,585	Air Turnover Rate (per hour):

Eulding Loads

Total Heating Required With Outside Air:	43,205		43.205	
Total Sensible Gain:	39,683	Btuh	84	%
Total Latent Gain:	7,374	Btuh	16	%

3.92 Tons (Based On Sensible + Latent) Total Cooling Required With Outside Air: 47.057 Btuh

4.35 Tons (Based On 75% Sensible Capacity)

1.194 698 7.9

Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary.

Be sure to select a unit that meets both sensible and latent loads.

Rhyac - Residential & Light Commer North Central Florida A/C Inc High Springs, FL 32643	cial HVAC Loads		9				rs Development, Inc. Albert & Patricia Moniz Page 3
Miscellaneous Report							
System 1 Main Floor	Outdoor		Outdoor		Indoor	Indoor	
Input Data	Dry Bulb	MEN MAN	Vet Bulb		Rel.Hum	<b>Dry Buil</b>	
Winter: Summer:	- 31 93		0 77		50 50	68 75	
							eranno arti il suo facilità i con con il monocco di con
System 2 Bonas Room Input Data	Outdoo Dry Bull		Outdoor Wet Bulb		Indoor Rel.Hum		
Winter:	3.	the life in the second second second	O CASE DOID	MUNICIPAL PROPERTY.	30	the state of the s	
Summer:	9:		77		50	-	
Duct Sizing Inputs							
Main T	<u>runk</u>		Runouts		15.00 16.00 19.00 10.00	TO STATE OF	
Calculate:	Yes		Yes				
Use Schedule:	No		No				
	300		0.01000				
	1000 in.wg./100 ft.		0.1000				
Minimum Velocity:	650 ft./min			ft./min			
Maximum Velocity:	900 ft./min			ft./min	1		
Minimum Height:	0 in.		0	****			
Maximum Height:	0 in.		0	in.			
Outside Air Data							
1 64 4	Winter	•		mmer			
Infiltration:	0.900 AC/			0.400			
Volume of Conditioned Space:	X 27585 Cu.			27585			
	24,827 Cu.t X_0.0167	R./NF		.0167	Cu.ft./hr		
Total Building Infiltration:	414 CFM	A	<u> </u>	184	CEM		
Total Building Ventilation:	0 CFM				CFM		
Total Dullding Vollmanori.	0 011	<b>,</b>		U	OI IVI		
System 1							
Infiltration & Ventilation Sensible	Gain Multiplier:	19.69	= (1.10)	K 0.995	X 18.00	Summer Temp.	Difference)
Infiltration & Ventilation Latent G	ain Multiplier:	33.85				Grains Difference	
Infiltration & Ventilation Sensible	Loss Multiplier:	40.48	= (1.10)	K 0.995	X 37.00	Winter Temp. Di	fference)
<u> </u>							
—System 2—		40.00	,, ,,,		W 46 55		
Infiltration & Ventilation Sensible		19.69	•			Summer Temp.	
Infiltration & Ventilation Latent G	•	33.85	•			Grains Difference	•

Infiltration & Ventilation Sensible Loss Multiplier:

40.48 = (1.10 X 0.995 X 37.00 Winter Temp. Difference)

Rhvac - Residential & Light North Central Florida A/C Inc High Springe, FL 32643	Commercial:	IVAC Londs					Elite Seftw	are Develop Albert & Pat	
Load Preview Re	eport								
Scope L			ke li						Duo Size
Building 3,92 Net Tons,					21 MBH Hea				
Building	3,033	39,683	7,374	47,057	43,205	564	1,814	1,814	
System 1:2:13 Net Ton		ammended.			34 MBH He				
System 1	1,656	21,985	3,594	25,579	24,341	318	1,005	1,005	14x14
Zone 1	1,656	21,985	3,594	25,579	24,341	318	1,005	1,005	
1-Bath 1	126	1,620	380	2,000	2,368	31	74	74	1-5
2-Dining Room	190	2,180	425	2,605	2,701	35	100	100	1-6
3-Kitchen	204	3,716	521	4,237	1,966	26	170	170	1-8
4-Nook	168	2,399	224	2,623	2,551	33	110	110	1-6
5-Gathering Room	272	3,840	730	4,570	5,594	73	176	176	1-8
6-Parior	213	3,235	732	3,967	4,763	62	148	148	1-1
7-Foyer	129	1,704	170	1,874	1,489	19	78	78	1-4
8-Pantry	49	101	0	101	112	1	5	5	1-2
9-Hall	112	983	90	1,073	816	11	45	45	1-4
10-Back Stairs	76	1,056	224	1,280	1,300	17	48	48	1-4
11-Front Stairs	70	636	0	636	160	2	29	29	1-3
12-Closet	47	512	98	610	521	7	23	23	1-3
System 2-1-79 Net Tons	1.97 Rec	ommended i	Tons 700	ft. <sup>2</sup> /Ton. 18	.86 MBH He	atino			
System 2	1,377	17,698	3,780	21,478	18,864	246	809	809	13x13
Zone 1	1,377	17,698	3,780	21,478	18,864	246	809	809	
13-Master Bedroom	333	4,493	1,243	5,736	. 5,555	73	205	205	2-6
14-Master W.I.C	80	1,057	334	1,391	1,656	22	48	48	1-4
15-Master Bath	120	1,224	210	1,434	1,362	18	56	56	1-4
16-Bath 3	72	893	148	1,041	898	12	41	41	1-
17-Bedroom 2	178	2,672	538	3,210	2,266	30	122	122	1-7
18-Bedroom 3	175	3,127	712	3,839	3,264	43	143	143	1-3
19-Hall	160	1,885	315	2,200	2,040	27	86	86	1-4
20-Laundry	35	563	0	563	79	-1	26	26	1-
21-Closet	48	191	76	267	444	6	9	9	1-2
22-Open To Below	176	1,590	204	1,794	1,300	17	73	73	1-

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Rhyac - Residential & Light Commercial HVAC Loads North Central Florida A/C Inc High Springs, FL :32643	Elite Software Development, In Albert & Patricia Mon Page				
Total Building Summary Loads					
Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Tota Gair
A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	135	2,746	0	2,558	2,55
A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	103.5	2,104	0	1,540	1,54
A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 50% coverage	12.5	255	0	168	16
0C-m: Glazing-French door, double pane low-e glass (e = 0.40), metal frame no break, ground reflectance = 0.32	35.4	1,870	0	1,228	1,22
B-v-o: Glazing-Double pane low-e (e = 0.60), fixed sash, vinyl frame, ground reflectance = 0.32	31.3	625	0	905	90
A-b-o: Glazing-Double pane low-e (e = 0.60), operable window, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	10.9	339	0	294	29
A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, external shade screen coefficient of 0.45 and 100% coverage	11.1	226	0	268	26
1P: Door-Polyurethane Core	53	570	0	447	44
2E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	2152.8	4,379	0	1,857	1,85
2E-4bw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, brick finish, wood studs	195.1	397	0	146	14
6C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	3033.3	3,590	0	4,171	4,17
9C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	3033.3	2,151	0	1,047	1,04
Subtotals for structure: People: Equipment:	5	19,252	0 1,150 0	14,629 1,500 1,200	14,62 2,65 1,20
Lighting:	3555			12,123	12,12
Ductwork:		7,203	0	6,612	6,61
Infiltration: Winter CFM: 414, Summer CFM: 184		16,750	6,224	3,619	9,84
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	
		42 205	7 274	၁၈ စေ၁	47

Check Figures			
Total Building Supply CFM:	1,814	CFM Per Square ft.:	1.194
Square ft. of Room Area:	3,033	Square ft. Per Ton:	698
Volume (ft³) of Cond. Space:	27,585	Air Turnover Rate (per hour):	7.9

43,205

**Total Building Load Totals:** 

39,683

7,374

47,057

Rhyac - Residential & Light Commercial HVAC Loads
North Central Florida Arc Inc.

High Springs, FL 32643



Elite Software Development, Inc. Albert & Patricia Moniz Page 6

# Total Building Summary Loads (cont'd)

Building Loads

Total Heating Required With Outside Air:

43,205 Btuh

43.205 MBH

Total Sensible Gain:

39,683 Btuh

84 %

**Total Latent Gain:** 

7,374 Btuh

16 %

47,057 Btuh

Total Cooling Required With Outside Air:

3.92 Tons (Based On Sensible + Latent)

4.35 Tons (Based On 75% Sensible Capacity)

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Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary.

Be sure to select a unit that meets both sensible and latent loads.

High Springs, FL 32643



Elite Software Development, Inc. Albert & Patricia Moniz Page 7

System 1 Main Floor Summary Loads (Avera
--

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable	21.8	444	0	391	391
window, vinyl frame, ground reflectance = 0.23,					
outdoor insect screen with 100% coverage, light color					
blinds at 45° with 25% coverage, external shade					
screen coefficient of 0.45 and 100% coverage	400 5	0.404	_		
A-v-o: Glazing-Double pane low-e (e = 0.60), operable	103.5	2,104	0	1,540	1,540
window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color					
blinds at 45° with 25% coverage, external shade					
screen coefficient of 0.45 and 100% coverage					
A-v-o: Glazing-Double pane low-e (e = 0.60), operable	12.5	255	0	168	168
window, vinyl frame, ground reflectance = 0.32,	12.0	200	J	100	100
outdoor insect screen with 100% coverage, light color					
blinds at 45° with 25% coverage, external shade					
screen coefficient of 0.45 and 50% coverage					
0C-m: Glazing-French door, double pane low-e glass (e	35.4	1,870	0	1,228	1,228
= 0.40), metal frame no break, ground reflectance =					
0.32					
2B-v-o: Glazing-Double pane low-e (e = 0.60), fixed sash,	31.3	625	0	905	905
vinyl frame, ground reflectance = 0.32		570	•	4.47	4.47
11P: Door-Polyurethane Core	53	570	0	447	447
2E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity,	1095.2	2,228	0	944	944
R-4 board insulation, siding finish, wood studs 16C-30: Roof/Ceiling-Under attic or knee wall, Vented	1656.2	1,961	0	2,278	2,278
Attic, No Radiant Barrier, White or Light Color	1030.2	1,901	U	2,210	2,210
Shingles, Any Wood Shake, Light Metal, Tar and					
Gravel or Membrane, R-30 insulation					
19C-11sp: Floor-Over enclosed unconditioned crawl	1656.2	1,175	0	572	572
space, R-11 insulation on exposed walls, sealed crawl		.,	_		
space, passive, R-11 blanket					
Subtotals for structure:		11,232	0	8,473	8,473
People:	1	,	230	300	530
Equipment:	-		0	1,200	1,200
Lighting:	1875			6,394	6,394
Ductwork:		4,058	0	3,663	3,663
Infiltration: Winter CFM: 224, Summer CFM: 99		9,051	3,364	1,955	5,319
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
System 1 Main Floor Load Totals:		24,341	3,594	21,985	25,579

Check Figures			
Supply CFM:	1,005	CFM Per Square ft.:	0.607
Square ft. of Room Area:	1,656	Square ft. Per Ton:	696
Volume (ft³) of Cond. Space:	14,906	Air Turnover Rate (per hour):	4.0

System Loads					197
Total Heating Required With Outside Air:	24,341	Btuh	24.341	MBH	
Total Sensible Gain:	21,985	Btuh	86	%	
Total Latent Gain:	3,594	Btuh	14	%	

2.13 Tons (Based On Sensible + Latent) Total Cooling Required With Outside Air: 25,579 Btuh

2.38 Tons (Based On 77% Sensible Capacity)

Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary.

Be sure to select a unit that meets both sensible and latent loads.

Rhvac - Residential & Light Commercial HVAC Loads North Central Florida A/C Inc High Springs, FL 32843	9		Elitie S	oftware Develo Albert & Pr	pment, Inc. stricia Moniz Page 8
System 2 Bonas Room Summary Loads (	Average	Method)	2)		
Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Tota Gain
A-v-o: Glazing-Double pane low-e (e = 0.60), eperable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	113.1	2,302	0	2,167	2,167
A-b-o: Glazing-Double pane low-e (e = 0.60), operable window, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	10.9	339	0	294	294
A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, external shade screen coefficient of 0.45 and 100% coverage	11.1	226	0	268	268
2E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	1057.7	2,151	0	913	913
2E-4bw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, brick finish, wood studs	195.1	397	0	146	146
6C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1377.1	1,629	0	1,893	1,893
9C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	1377.1	976	0	475	479
Subtotals for structure:		8,020	0	6,156	6,15
People:	4		920 0	1,200 0	2,12
Equipment: Lighting:	1680		U	5,729	5,72
Ductwork:		3,145	0	2,949	2,94
nfiltration: Winter CFM: 190, Summer CFM: 85		7,699	2,860	1,664	4,52
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0 700	0	04.47
System 2 Bonas Room Load Totals:		18,864	3,780	17,698	21,47
Check Figures					
Supply CFM: 809		/ Per Square f			587 700
Square ft. of Room Area: 1,377  Volume (ft³) of Cond. Space: 12,679	•	are ft. Per Ton Furnover Rate			700 3.8
System Loads	, wi				
Total Heating Required With Outside Air: 18,864 Bt	uh 18.	.864 MBH			
Total Sensible Gain: 17,698 Bt		82 %			
Total Latent Gain: 3,780 Bt		18 %			
Total Cooling Required With Outside Air: 21,478 Bt		1.79 Tons (Ba			

1.97 Tons (Based On 75% Sensible Capacity)

Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary. Be sure to select a unit that meets both sensible and latent loads.

Rhyac - Residential & Light Commercial HVAC Loads North Central Florida A/C Inc High Springs, FL 32643				Elite Software Development, Inc. Albert & Patricia Moniz Page 9			
System 1, Zone 1 Summary Loads (Aver	age Metho	od)					
component .		Sen	<b>La</b> r	Sen	Total		
Description  A-v-o: Glazing-Double pane low-e (e = 0.60); operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	Quan 21.8	444	- Gain 0	<b>Gain</b> 391	<b>Gain</b> 391		
A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	103.5	2,104	0	1,540	1,540		
A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 50% coverage	12.5	255	0	168	168		
OC-m: Glazing-French door, double pane low-e glass (e = 0.40), metal frame no break, ground reflectance = 0.32	35.4	1,870	0	1,228	1,228		
B-v-o: Glazing-Double pane low-e (e = 0.60), fixed sash, vinyl frame, ground reflectance = 0.32	31.3	625	0	905	905		
1P: Door-Polyurethane Core	53	570	0	447	447		
2E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	1095.2	2,228	0	944	944		
6C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1656.2	1,961	0	2,278	2,278		
9C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	1656.2	1,175	0	572	572		
Subtotals for structure:		11,232	0	8,473	8,473		
People:	1	·	230	300	530		
Equipment:			0	1,200	1,200		
Lighting:	1875			6,394	6,394		
Ductwork:		4,058	0	3,663	3,663		
Infiltration: Winter CFM: 224, Summer CFM: 99	W	9,051	3,364	1,955	5,319		
System 1, Zone 1 Load Totals:		24,341	3,594	21,985	25,579		

Supply CFM: Square ft. of Room Area: Volume (ft³) of Cond. Space:	1,005 1,656 14,906		Square	r Square ft.: ft. Per Ton: over Rate (per hour):	0.607 696 4.0
Zone Loads					
Total Heating Required:	•	41 Btuh	24.341	WRH	
Total Sensible Gain:	21,9	85 Btuh	86	%	
Total Latent Gain:	3,5	94 Btuh	14	%	
Total Cooling Required:	25,5	79 Btuh	2.13	Tons (Based On Sensible	+ Latent)
•	·	((*))	2.38	Tons (Based On 77% Ser	sible Capacity)

Notes
Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary. Be sure to select a unit that meets both sensible and latent loads.



Elits Software Development, Inc. Albert & Patricia Moniz Page 10

System 2, Zone 1 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Galii	Sen Gain	Total Gain
2A-v-o: Glazing-Double pane low-e (e = 0.60),-operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	113.1	2,302	0	2,167	2,167
2A-b-o: Glazing-Double pane low-e (e = 0.60), operable window, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	10.9	339	0	294	294
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, external shade screen coefficient of 0.45 and 100% coverage	11.1	226	0	268	268
I2E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	1057.7	2,151	0	913	913
2E-4bw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, brick finish, wood studs	195.1	397	0	146	146
I6C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1377.1	1,629	0	1,893	1,893
9C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	1377.1	976	0	475	475
Subtotals for structure:	4	8,020	0	6,156	6,156
People: Equipment:	4		920 0	1,200 0	2,120 0
Lighting:	1680		_	5,729	5,729
Ductwork: Infiltration: Winter CFM: 190, Summer CFM: 85		3,145 7,699	0 2,860	2,949 1,664	2,949 4,524
irmitiation. Valida Crivi. 180, Suffither Crivi. 65		18,864	3,780	17,698	21,478

Check Figures			
Supply CFM:	809	CFM Per Square ft.:	0.587
Square ft. of Room Area:	1,377	Square ft. Per Ton:	700
Volume (ft³) of Cond. Space:	12,679	Air Turnover Rate (per hour):	3.8

Zone Loads		THE LETTER		
Total Heating Required:	18,864	Btuh	18.864	MBH
Total Sensible Gain:	17,698	Btuh	82	%
Total Latent Gain:	3,780	Btuh	18	%
Total Cooling Required:	21,478	Btuh	1.79	Tons (Based On Sensible + Latent)
•			1.97	Tons (Based On 75% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary.

Be sure to select a unit that meets both sensible and latent loads.

North Central Florida A/C h	ht Commercial HVAC (Cads) C
High Springs, FL 52543	**************************************
	Rhvac - Residential & Lig North Central Florida A/C in High Springs FL 32843



Elite Software Development, Inc. Albeit & Patricia Moniz Page 11

# System 1 Room Load Summary

	Room Name							eri Glija Listi Siuli	Clg W Nom CFM	Sys CFM
Zo	ne 1—									
1	Bath 1	126	2,368	31	1-5	543	1,620	380	74	74
2	Dining Room	190	2,701	35	1-6	508	2,180	425	100	100
3	Kitchen	204	1,966	26	1-8	487	3,716	521	170	170
4	Nook	168	2,551	33	1-6	559	2,399	224	110	110
5	Gathering Room	272	5,594	73	1-8	503	3,840	730	176	176
6	Parior	213	4,763	62	1-7	553	3,235	732	148	148
7	Foyer	129	1,489	19	1-5	571	1,704	170	78	78
8	Pantry	49	112	1	1-2	212	101	0	5	5
9	Hall	112	816	11	1-4	515	983	90	45	45
10	Back Stairs	76	1,300	17	1-4	553	1,056	224	48	48
11	Front Stairs	70	160	2	1-3	592	636	0	29	29
12	Closet	47	521	7	1-3	476	512	98	23	23
	System 1 total	1,656	24,341	318			21,985	3,594	1,005	1,005

System 1 Main Trunk Size:

Velocity:

14x14 in. 819 ft./min

Loss per 100 ft.:

0.089 in.wg

	y de independent			1. 1. 45 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	
	· 以及所謂於你們們的問題的人們就是一個問題的。	Sensible/Latent	Sensible	Latent	Total
Net Required:	<b>Tons</b> 2.13	86% / 14%	<b>Btuh</b> 21,985	3,594	25,579
Recommended:	2.38	77% / 23%	21,985	6,567	28,552
Actual:	2.40	78% / 22%	22,350	6,450	28,800

THE CHARGE	24	CONT.	PATE.	2.85	557	-	2.77	2.23	d
Ea		200.00	~~	·	10.0	800	•	•	n
3 204 9 1	t i		œ	3 I II	503		25 (		

Type: Model: Brand: Efficiency: Sound: Capacity: Sensible Capacity:

Latent Capacity:

Air Cooled Condensor RHF30C2\*-CAPF042B2+TXV-MBE1600\*\*-1 Amana 8.5 HFSP 28.600 n/a n/a

**Heating System** 

Cooling System
Air Cooled Condensor
RHF30C2\*CHPF042B2+TXVMBE1600\*\*-1
Amana
15 SEER

28,800
22,350 Btuh

6,450 Btuh

2.5			
107 - 1 100 HOUSE 100 PO 12	tadget Ja. beded, t Jed . dt., Jäl. Jä., Ja.,	A Lectobological Section 2 Laborated Section 2	ok es a . a . a . a . an andread house house entre se
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PROPERTY	11: 2 - 2 - 2 - 3 - 1 - 2 - 1	• 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	og a la l
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	gs, FL 32643	kabbubabuba canabaracabar	**************************************
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Eilte Software Development, Inc. Albert & Patricia Moniz Page 12

# System 2 Room Load Summary

			Hig	His	Rui	Run		- Clg	Glg	Air
22.16	Room	Area	Sens	Nom	Duct	Duct	Sens	Lat	Nom	Sys
	Name	is z	Bryk	Wale Indiana	Size	Vel	- Buh	Bluh	CFM	CFM
Zo	ne 1									
13	Master Bedroom	3 <b>3</b> 3	5,555	73	2-6	523	4,493	1,243	205	205
14	Master W.I.C	<b>8</b> 0	1,656	22	1-4	554	1,057	334	48	48
15	Master Bath	120	1,362	18	1-4	641	1,224	210	56	56
16	Bath 3	72	898	12	1-4	468	893	148	41	41
17	Bedroom 2	178	2,266	30	1-7	457	2,672	538	122	122
18	Bedroom 3	175	3,264	43	1-7	535	3,127	712	143	143
19	Hall	160	2,040	27	1-5	632	1,885	315	86	86
20	Laundry	35	79	1	1-3	524	563	0	26	26
21	Closet	48	444	6	1-2	400	191	76	9	9
22	Open To Below	176	1,300	17	1-5	533	1,590	204	73	73
	System 2 total	1,377	18,864	246			17,698	3,780	809	809

System 2 Main Trunk Size:

13x13 in.

Velocity:

757 ft./min

Loss per 100 ft.:

0.083 in.wg

COOLITES SEGMENTALES							
	Clooling	Sensible/Latent	Sensible Buh	Latent Blub	Total Bluh		
Net Required:	1.79	82% / 18%	17,698	3,780	21,478		
Recommended:	1.97	75% / 25%	17,698	5,899	23,597		
Actual:	2.38	78% / 22%	22,350	6,250	28,600		

l**≡្**ហេស្រីលីពីរដែន

	Heating System	Cooling System
Type:	Air Cooled Condensor	Air Cooled Condensor
Model:	RHF30C2*-CHPF042B2+TXV-	RHF30C2*-CHPF042B2+TXV-
	MBE1600**-1	MBE1600**-1
Brand:	Amana	Amana
Efficiency:	8.5 HFPF	13 Seer

Efficiency: Sound: Capacity:

28.800

28.600

Sensible Capacity: Latent Capacity:

•••

n/a n/a

22,350 Btuh 6,250 Btuh

¢	0	b) Wood frame wall
	25	1. All materials making up wall
		2. Size and species of studs
		3. Sheathing size, type and nailing schedule
		4. Headers sized
		5. Gable end showing balloon framing detail or gable truss and wall
		hinge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with wind resistance rating)
		8. Fire resistant construction (if applicable)
		9. Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)
		11. Slab on grade
		a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
		b. Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports
		12. Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity
0		c. Crawl space (if applicable)
	D	c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)
		Floor Framing System:
0/		a) Floor truss package including layout and details, signed and sealed by Florida
		Registered Professional Engineer
0/		b) Floor joist size and spacing
	0	c) Girder size and spacing
		d) Attachment of joist to girder
		e) Wind load requirements where applicable
		Plumbing Fixture layout
6/	0	Electrical layout including:
12/		a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
ur m	0	b) Ceiling fans
		c) Smoke detectors
		<ul> <li>d) Service panel and sub-panel size and location(s)</li> <li>e) Meter location with type of service entrance (overhead or underground)</li> </ul>
	Ö	f) Appliances and HVAC equipment
n//	٥	g) Arc Fault Circuits (AFCI) in bedrooms
0//	0	h) Exhaust fans in bathroom
* /	_	HVAC information
0/		a) Energy Calculations (dimensions shall match plans)
.0/	<u></u>	b) Manual J sizing equipment or equivalent computation
9/	ū	c) Gas System Type (LP or Natural) Location and BTU demand of equipment
G	0	Disclosure Statement for Owner Builders
Ď /	Ð	*** Notice Of Commencement Required Before Any Inspections Will Be Done
Q/	0	Private Potable Water

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)



- d) Location, size and height above roof of chimneys.
- e) Location and size of skylights
- f) Building height
- e) Number of stories

### Floor Plan including:

- a) Rooms labeled and dimensioned.
- b) Shear walls identified.
- c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
- d) Show safety glazing of glass, where required by code.
- e) Identify egress windows in bedrooms, and size.
- f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
- g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
- h) Must show and identify accessibility requirements (accessible bathroom) Foundation Plan including:
- a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel.

### Roof System:

- a) Truss package including:
  - 1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  - 2. Roof assembly (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
  - 1. Rafter size, species and spacing
  - 2. Attachment to wall and uplift
  - 3. Ridge beam sized and valley framing and support details
  - 4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

### Wall Sections including:

- a) Masonry wall
  - 1. All materials making up wall
  - 2. Block size and mortar type with size and spacing of reinforcement
  - 3. Lintel, tie-beam sizes and reinforcement
  - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  - All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
  - 6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  - 7. Fire resistant construction (if required)
  - 8. Fireproofing requirements
  - 9. Shoe type of termite treatment (termiticide or alternative method)
  - 10. Slab on grade
    - a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  - 11. Indicate where pressure treated wood will be placed
  - 12. Provide insulation R value for the following:

### HODGE AND INCOME OF LOW TOATION STILL

ocation: 4558. W. CHALET TERR, Project Name: MONIZ PRS.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at <a href="https://www.floridabuilding.org">www.floridabuilding.org</a>

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS		FIBRICIAN WITE LOOK	
√1. Swinging	MASONITE	SIDE-HINGED FIBERGLASS DOORS	FL 4608.1
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
√ 4. Double Hung	MI WINDOWSA!	DORS-CELLULAR PUC TRIPLE Mulled	14. 4.175. 19 4173.15ª
5. Fixed		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	PL 686.1
6. Awning			11- 9010-1
7. Pass -through			
8. Projected	17		
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			The second second second second second
1. Siding		HADY PLANK	
2. Soffits			
3. EIFS		Hady Plank	
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other		211160116	
D. ROOFING PRODUCTS	EIR	Blue BOARD (INside)	
✓ 1. Asphalt Shingles	RIKED	LAMINATED ASPHALT/FIBERGIASS	F11476.1
2. Underlayments			
3. Roofing Fasteners			
	~		
4. Non-structural Metal	MI		
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys	8		
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shak	es		
12. Roofing Slate			

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

### ALL REQUIREMENTS ARE SUBJECT TO CHANGE EFFECTIVE OCTOBER 1, 2005

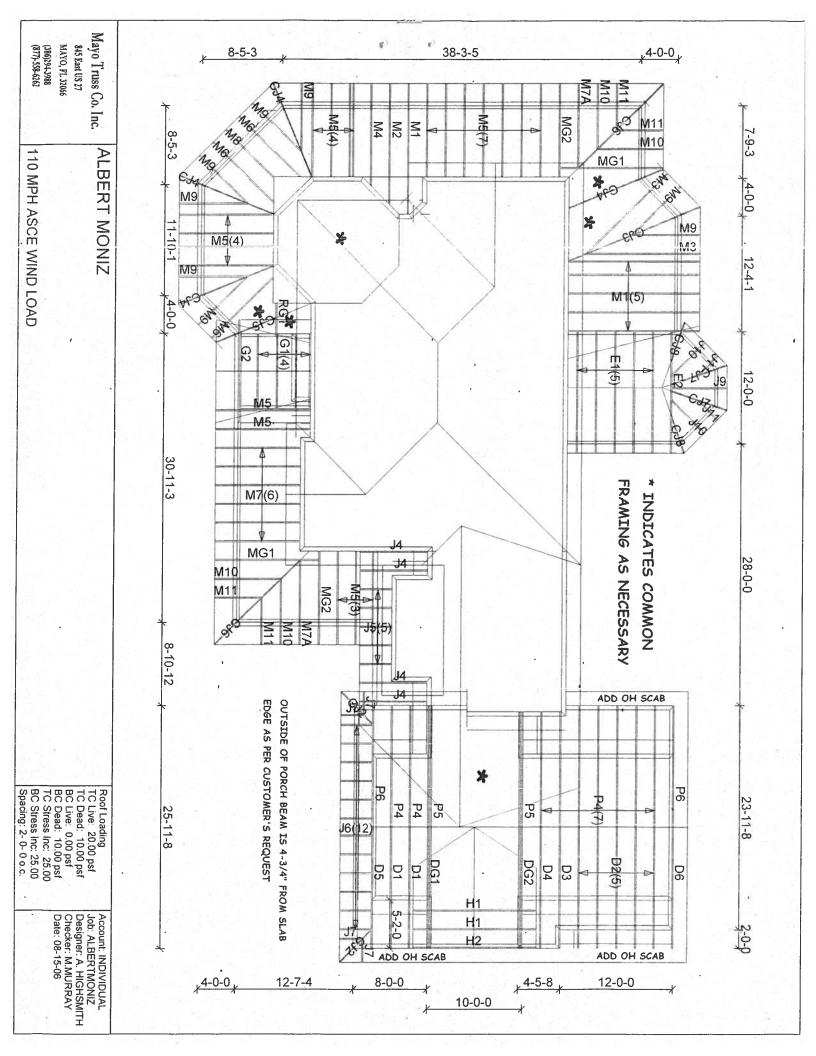
ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

## APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL	REQUIREME	ENTS: Two (2) complete sets of plans containing the following:
Applicant	Plans Examine	
	D	All drawings must be clear, concise and drawn to scale ("Optional"
		details that are not used shall be marked void or crossed off). Square
		footage of different areas shall be shown on plans.
	0	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
0	0	Site Plan including:
		2) Dimensions of lot
		b) Dimensions of building set backs
		c) Location of all other buildings on lot, well and septic tank if
		applicable, and all utility easements.
		d) Provide a full legal description of property.
0		Wind-load Engineering Summary, calculations and any details required
		Plans of specifications must state compliance with FRC Section 1600
		The following information must be shown as per section 1603.1.4 FBC
		a. Basic wind speed (3-second gust), miles per hour (km/hr).
		<ol> <li>Wind importance factor, Iw. and building classification from Table</li> </ol>
		1604.5 or Table 6-1, ASCE 7 and building classification in Table
		i-i, asce /.
		c. Wind exposure, if more than one wind exposure is utilized, the
		WIRE EXPOSURE and applicable wind direction shall be indicated
		d. The applicable enclosure classifications and if designed with
		ASCE /, internal pressure coefficient
		e. Components and Cladding. The design wind pressures in terms of
		psi (KIV/III-) to be used for the design of exterior component and
		cladding materials not specifally designed by the registered design professional.
.//		Elevations including:
	0	a) All sides
4/		b) Roof pitch
<i>M</i>	D	c) Overhang dimensions and detail with attic ventilation



Permit Number:	Lot Number:	= 1% 11
Miscellaneous:	Address:	
The information in this box is for admi	inistrative purposes only and is not part of	the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: ALBERTMONIZ - ALBERT MONIZ

ROBBINS P.O. Box 280055
Tampa, FL 33682-0055
Phone: (813) 972-1135

**Engineering Index Sheet** 

Index Page 1 of 1

Job Number Date T06080605 08/07/2006 FBC - 2004 Chapter 16 and 23

**Specification Quantity** 

46

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Sofware - Online Plus)

Notes: Refer to individual truss design drawings for special loading conditions.

Index Page 1 of 1

T.C. Live T.C Dead B.C Live B.C. Dead

MWFRS

Enclosed

Standard Loading:

ANSI/ASCE 7-02 Wind Speed - 110 MPH Mean Roof Ht. - 15 FT Exposure Catergory - B Occupancy Factor - 1.00

20 psf 10 psf 0 psf 10 psf 40 psf

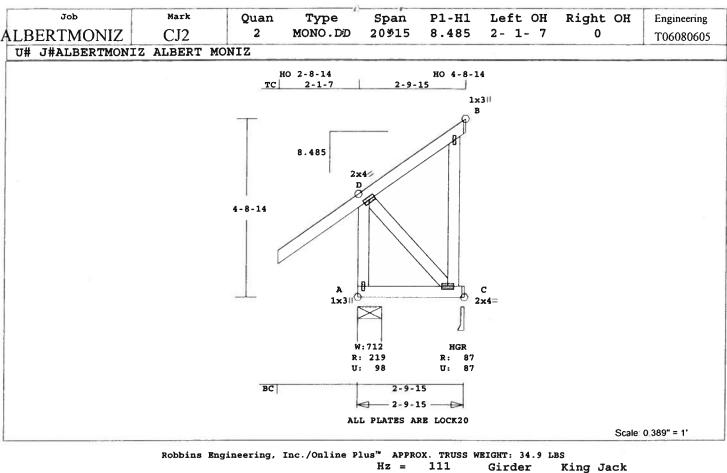
Date Mark	Date Mark	Date Mark	Date Mark
1 08/07/06 CJ2	2 08/07/06 CJ3	3   08/07/06   CJ4	4 08/07/06 CJ5
5   08/07/06   CJ6	6 08/07/06 CJ7	7 08/07/06 CJ8	8   08/07/06   D1
9 08/07/06 D2	10 08/07/06 D3	11 08/07/06 D4	12 08/07/06 D5
13  08/07/06   D6	14 08/07/06 DG1	15 08/07/06 DG2	16   08/07/06   E1
17 08/07/06 E2	18   08/07/06   G1	19 08/07/06 G2	20 08/07/06 H1
21   08/07/06   H2	22   08/07/06   J4	23 08/07/06 J5	24   08/07/06   J6
25   08/07/06   J7	26   08/07/06   J9	27 08/07/06 J10	28 08/07/06 J11
29   08/07/06   M1	30   08/07/06   M2	31   08/07/06   M3	32   08/07/06   M4
33 08/07/06 M5	34 08/07/06 M6	35   08/07/06   M7	36   08/07/06   M7A
37 08/07/06 M8	38 08/07/06 M9	39 08/07/06 M10	40 08/07/06 M11
41 08/07/06 MG1	42 08/07/06 MG2	43   08/07/06   P4	44   08/07/06   P5
45 08/07/06 P6	46 08/07/06 RG1		

Mangle

Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006



Hz =111 Online Plus -- Version 19.0.034 Membr CSI P Lbs Axl-CSI-Bnd RUN DATE: 04-AUG-06 -----Top Chords-----D -B 0.01 47 T 0.00 0.01 CSI -Size- ----Lumber---------Bottom Chords-----0.01 SP-#2 A -C 0.01 87 T TC 2x 4 SP-#2 BC 0.01 2x 4 ------Webs------WB 0.03 2x 4 SP-#2 A -D 0.01 81 C WindLd D -C 0.01 82 T Brace truss as follows: C -B 0.03 27 T WindLd To o.c. From TC Cont. 0- 0- 0 2- 9-15 TL Defl 0.00" in A -C L/999 0- 0- 0 2- 9-15 LL Defl 0.00" in A -C L/999 BC Cont. Shear // Grain in D -B 0.01 Loading Live Dead (psf) 10.0 Plates for each ply each face. TC 20.0 PLATING CONFORMS TO TPI. BC 0.0 10.0 40.0 REPORT: NER 691 20.0 20.0 Total 24.0" ROBBINS ENGINEERING, INC. Spacing Lumber Duration Factor 1.25 BASED ON SP LUMBER Plate Duration Factor 1.25 USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area TC Fb=1.00 Fc=1.00 Ft=1.00 Plate - RHS 20 Ga, Gross Area BC Fb=1.00 Fc=1.00 Ft=1.00 Jt Type Plt Size X Y Load Case # 1 Girder Loading D LOCK 2.0x 4.0 Ctr Ctr 0.75 Lumber Duration Factor 1.25 В LOCK 1.0x 3.0 Ctr Ctr 0.75 Plate Duration Factor 1.25 Α LOCK 1.0x 3.0 Ctr Ctr 0.75 2.0x 4.0 Ctr Ctr 0.75 plf - Live Dead From To LOCK 2.8' TC V 40 20 0.01

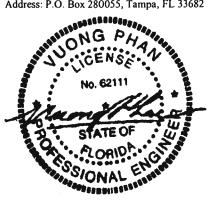
> REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004

Loading TC and BC Setback 2-0-0 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Use properly rated hangers for loads framing into girder truss. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 81 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License # 62111 Address: P.O. Box 280055, Tampa, FL 33682



1-8

-106

1-8

0.0'

0.0'

0.0'

Lbs In-Sx In-Sx

7-12

Hz =

3 - 8

2.8'

2.8'

2.8'

BC V

TC V

BC V

Plus

A

C

٥

0

0

-40

-26

Lbs

219

87

20

-20

-13

-20

-13

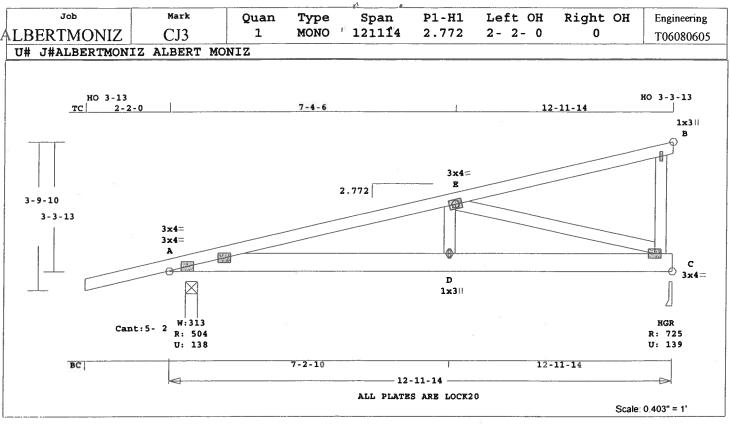
5 Wind Load Case(s)

React Uplft Size Req'd

98

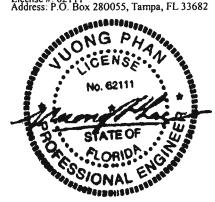
87

1 UBC LL Load Case(s)



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 83.5 LBS Membr CSI P Lbs Axl-CSI-Bnd Girder King Jack -----Top Chords-----Loading TC and BC Online Plus -- Version 19.0.034 A -E 0.49 1178 C 0.01 0.48 Setback 12- 0- 0 RUN DATE: 04-AUG-06 E -B 0.53 43 C 0.00 0.53 OH Loading -----Bottom Chords-----Soffit psf 2.0 ----Lumber----A -D 0.24 1163 T 0.15 0.09 CSI -Size-Design checked for 10 psf non-2x 4 SP-#2 D -C 0.26 1163 T 0.15 0.11 TC 0.53 concurrent LL on BC. 0.26 2x 6 SP-#2 BC Use properly rated hangers for 0.47 2x 4 SP-#2 D-E 0.04 301 T loads framing into girder E -C 0.47 1212 C truss. Brace truss as follows: C -B 0.03 263 C WindLd Wind Loads - ANSI / ASCE 7-02 o.c. From To Truss is designed as a Main TL Defl -0.06" in A -D L/999 LL Defl -0.03" in A -D L/999 0- 0- 0 12-11-14 TC Cont. Wind-Force Resistance System. 0- 0- 0 12-11-14 BC Cont. Wind Speed: 110 mph Shear // Grain in E -B 0.35 Mean Roof Height: 15-0 Loading (psf) Dead Live Exposure Category: В TC 20.0 10.0 Plates for each ply each face. Occupancy Factor BC 0.0 10.0 PLATING CONFORMS TO TPI. Building Type: Enclosed Total 20.0 20.0 40.0 REPORT: NER 691 Zone location: Exterior 24.0" ROBBINS ENGINEERING, INC. 5.0 psf Spacing TC Dead Load : Lumber Duration Factor BASED ON SP LUMBER 1.25 BC Dead Load : 5.0 psf Plate Duration Factor 1.25 USING GROSS AREA TEST. 1212 Lbs Max comp. force TC Fb=1.00 Fc=1.00 Ft=1.00 BC Fb=1.00 Fc=1.00 Ft=1.00 Plate - LOCK 20 Ga, Gross Area Quality Control Factor 1.25 Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y A LOCK 3.0x 4.0 Ctr Ctr 0.46 Load Case # 1 Girder Loading Lumber Duration Factor LOCK 3.0x 4.0 Ctr Ctr 0.12 1.25 LOCK 3.0x 4.0 Ctr Ctr 0.65 Plate Duration Factor 1.25 Ė plf - Live Dead From To В LOCK 1.0x 3.0 Ctr Ctr 0.75 TC V 40 20 0.0' 13.0' D LOCK 1.0x 3.0 Ctr Ctr 0.75 Truss Design Engineer: Vuong Phan BC V 3.0x 4.0 Ctr Ctr 0.77 0 20 0.0' 13.0' C LOCK TC V -40 -20 0.01

License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



Plus 5 Wind Load Case(s) 1 UBC LL Load Case(s)

22

-20

22

0.01

44

0

O

BC V

Size Req'd Jt React Uplft Lbs In-Sx In-Sx Lbs 139 3-13 1-8 505 Hz =56 C 725 139 3 - 8 1-8 Hz =108

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

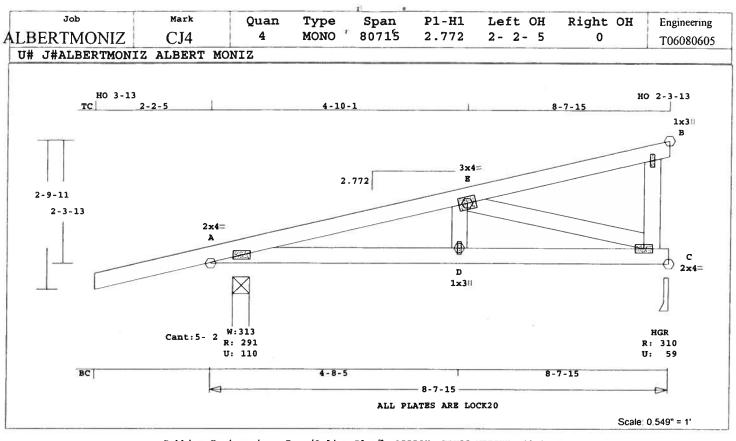
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004

13.0'

13.0'



Robbins Engineering, Inc./Online Plus® APPROX. TRUSS WEIGHT: 48.9 LBS
Membr CSI P Lbs Axl-CSI-Bnd Loading T Loading TC and BC ------Top Chords-----Setback 8- 0- 0 Online Plus -- Version 19.0.034 A -E 0.10 446 C 0.00 0.10 OH Loading 23 C 0.00 0.16 RUN DATE: 04-AUG-06 E -B 0.16 Soffit psf 2.0 -----Bottom Chords-----Design checked for 10 psf non-CSI -Size- ----Lumber----A -D 0.10 439 T 0.05 0.05 concurrent LL on BC. 0.16 TC 2x 4 SP-#2 D -C 0.14 439 T 0.08 0.06 Use properly rated hangers for BC 0.14 2x 4 SP-#2 ------Webs---loads framing into girder WB 0.08 2x 4 SP-#2 D -E 0.01 119 T truss. E -C 0.08 459 C Wind Loads - ANSI / ASCE 7-02 Brace truss as follows: C -B 0.01 118 C WindLd Truss is designed as a Main To O.C. From Wind-Force Resistance System. TL Defl -0.02" in D -C L/999 LL Defl -0.01" in D -C L/999 8- 7-15 0- 0- 0 TC Cont. Wind Speed: BC Cont. 0- 0- 0 8- 7-15 Mean Roof Height: Shear // Grain in E -B Exposure Category: 0.14 Loading Live Dead (psf) Occupancy Factor : 1.00 Plates for each ply each face. TC 20.0 10.0 Building Type: Enclosed 10.0 PLATING CONFORMS TO TPI. BC 0.0 Zone location: Exterior 20.0 40.0 REPORT: NER 691 Total 20.0 TC Dead Load : ROBBINS ENGINEERING, INC. 24.0" Spacing BC Dead Load : Lumber Duration Factor 1.25 BASED ON SP LUMBER Max comp. force Plate Duration Factor 1.25 USING GROSS AREA TEST. Quality Control Factor 1.25 TC Fb=1.00 Fc=1.00 Ft=1.00 Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI BC Fb=1.00 Fc=1.00 Ft=1.00 Load Case # 1 Girder Loading LOCK 2.0x 4.0 Ctr Ctr 0.91 Lumber Duration Factor 1.25 3.0x 4.0 Ctr Ctr 0.40 LOCK Plate Duration Factor 1.25 В LOCK 1.0x 3.0 Ctr Ctr 0.75 plf - Live Dead From To D LOCK 1.0x 3.0 Ctr Ctr 0.75 TC V 8.7' 20 C 2.0x 4.0 Ctr Ctr 0.79 40 0.0' LOCK BC V 0 20 0.01 8.7' 0.0' TC V -20 -40 14 7 8.7' REVIEWED BY: Robbins Engineering, Inc. BC V 0 -20 0.0 0 8.7' PO Box 280055 Tampa, FL 33682 5 Wind Load Case(s) REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682

110 mph

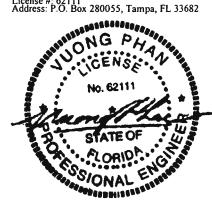
5.0 psf

5.0 psf

459 Lbs

В

15-0



Girder King Jack

1-8

-22

1-8

55

Robbins Engineering Inc /Online Plus™ © 1996-2006 Version 19 0 034 Engineering - Portrait 8/4/2006 8 53 39 PM Page 1

NOTES:

FBC2004

Plus

Lbs

292

311

Jt

A

1 UBC LL Load Case(s)

React Uplft Size Req'd

110

60

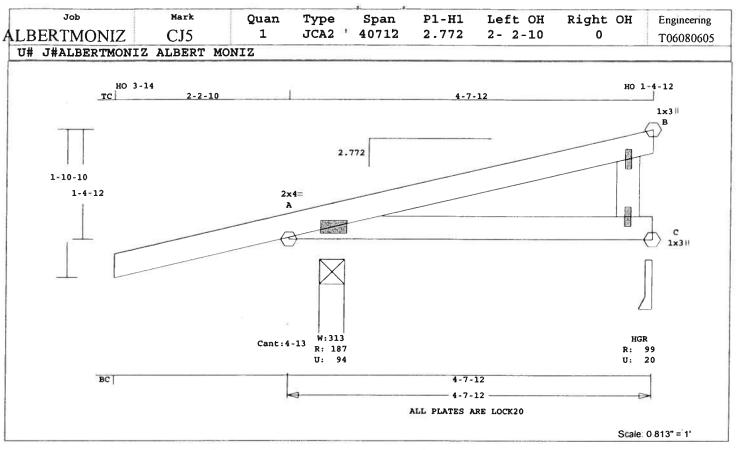
Lbs In-Sx In-Sx

3-13

Hz =

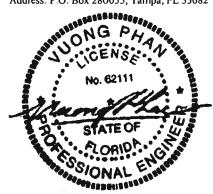
3 - 8

Hz =



Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 23.7 LBS Hz =22 OH Loading Soffit psf 2.0 Online Plus -- Version 19.0.034 Membr CSI P Lbs Axl-CSI-Bnd Design checked for 10 psf non-RUN DATE: 04-AUG-06 -----Top Chords----concurrent LL on BC. A -B 0.07 10 C 0.00 0.07 Use properly rated hangers for -----Bottom Chords-----CSI -Size- ----Lumber---loads framing into girder TC 0.07 2x 4 SP-#2 A -C 0.05 13 T 0.00 0.05 truss. BC 0.05 2x 4SP-#2 ------Webs-----Wind Loads - ANSI / ASCE 7-02 C -B 0.00 WB 0.00 2x 4SP-#2 59 C WindLd Truss is designed as a Main Wind-Force Resistance System. TL Defl -0.01" in A -C L/999 Brace truss as follows: Wind Speed: 110 mph 0.00" in A -C o.c. From To LL Defl L/999 Mean Roof Height: 15-0 Shear // Grain in A -B TC Cont. 0 - 0 - 04- 7-12 0.06 Exposure Category: В Occupancy Factor : 1.00 BC Cont. 0- 0- 0 4- 7-12 Plates for each ply each face. Building Type: Enclosed Loading Live Dead (psf) PLATING CONFORMS TO TPI. Zone location: Exterior TC 20.0 10.0 REPORT: NER 691 TC Dead Load : 5.0 psf BC ROBBINS ENGINEERING, INC. 0.0 10.0 BC Dead Load : 5.0 psf 40.0 BASED ON SP LUMBER Total 20.0 20.0 Max comp. force 59 Lbs USING GROSS AREA TEST. 24.0" Spacing Quality Control Factor 1.25 Lumber Duration Factor 1.25 Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI Plate Duration Factor 1.25 TC Fb=1.00 Fc=1.00 Ft=1.00 Jt Type BC Fb=1.00 Fc=1.00 Ft=1.00 2.0x 4.0 Ctr Ctr 0.81 A LOCK LOCK 1.0x 3.0 Ctr Ctr 0.75 R С LOCK 1.0x 3.0 Ctr Ctr 0.75 Load Case # 1 Girder Loading Lumber Duration Factor 1.25 Plate Duration Factor 1.25 Truss Design Engineer: Vuong Phan plf - Live Dead From REVIEWED BY: To 4.6' TC V 40 20 0.0' Robbins Engineering, Inc.

License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

1-8

- 9

BC V

TC V

BC V

Plus

Plus

Jt

A

C

0

٥

0

187

-40 -9 20

-20

- 5

-20

- 5

5 Wind Load Case(s)

React Uplft Size Req'd

95

20

1 UBC LL Load Case(s)

0.01

0.0

0.01

Lbs In-Sx In-Sx

3-13

Hz =

4.6'

4.6'

4.6'

PO Box 280055

NOTES:

Girder

FBC2004

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL

King Jack

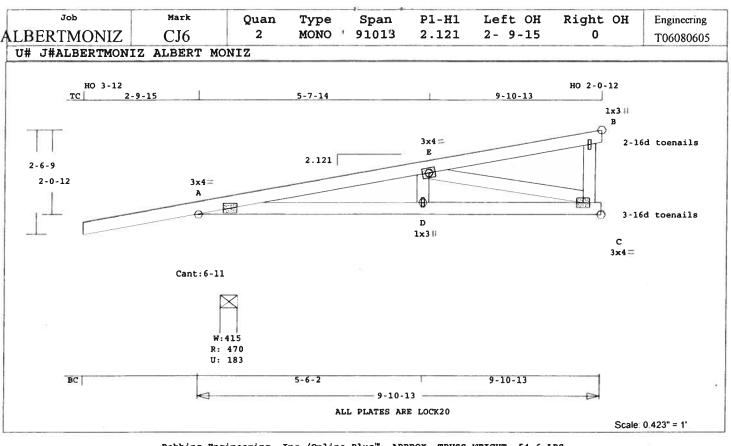
NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Trusses Manufactured by: Mayo Truss Co. Inc.

Analysis Conforms To:

Loading TC and BC

Setback 4- 0- 0



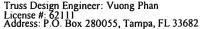
Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 54.6 LBS Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----Online Plus -- Version 19.0.034 A -E 0.19 1019 C 0.00 0.19 RUN DATE: 04-AUG-06 E-B 0.33 41 T 0.00 0.33 -----Bottom Chords-----CSI -Size- ----Lumber----A -D 0.23 1009 T 0.18 0.05 TC 0.33 2x 4 SP-#2 D -C 0.31 1009 T 0.18 0.13 BC 0.31 2x 4 SP-#2 -Webs - -D -E 0.03 WB 0.23  $2 \times 4$ SP-#2 216 T E -C 0.23 1039 C Brace truss as follows: C -B 0.00 0 T WindLd To o.c. From TL Defl -0.06" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in E -B 0.26 9-10-13 TC Cont. 0- 0- 0 0- 0- 0 9-10-13 BC Cont. Loading Live Dead (psf) 10.0 TC 20.0 Plates for each ply each face. PLATING CONFORMS TO TPI. BC 0.0 10.0 Total 20.0 20.0 40.0 REPORT: NER 691 ROBBINS ENGINEERING, INC. 24.0" Spacing Lumber Duration Factor BASED ON SP LUMBER 1.25 USING GROSS AREA TEST. Plate Duration Factor 1.25 Plate - LOCK 20 Ga, Gross Area TC Fb=1.00 Fc=1.00 Ft=1.00 Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI BC Fb=1.00 Fc=1.00 Ft=1.00 Jt Type Load Case # 1 Girder Loading LOCK 3.0x 4.0 Ctr Ctr 0.74 Lumber Duration Factor 1.25 E LOCK 3.0x 4.0 Ctr Ctr 0.55 Plate Duration Factor 1.25 LOCK 1.0x 3.0 Ctr Ctr 0.75 В Dead From To LOCK 1.0x 3.0 Ctr Ctr 0.75 plf -Live D 9.91 TC V 40 20 0.0 C LOCK 3.0x 4.0 Ctr Ctr 0.62 BC V 20 0.0 9.91 n TC V -40 -20 0.0' 9.91 REVIEWED BY: 45 22 BC V 0 -20 0.01 Robbins Engineering, Inc. 9.91 PO Box 280055 22 Tampa, FL 33682 5 Wind Load Case(s) REFER TO ROBBINS ENG. GENERAL Plus 1 UBC LL Load Case(s) NOTES AND SYMBOLS SHEET FOR Plus

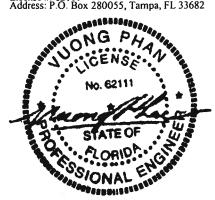
ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 Girder King Jack Loading TC and BC Setback 7- 0- 0 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Use properly rated hangers for loads framing into girder truss. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : BC Dead Load : 5.0 psf 1039 Lbs Max comp. force Quality Control Factor 1.25





1-8

1 - 8

1-8

44

React Uplft Size Req'd

184

28

111

Lbs In-Sx In-Sx

4-15

Hz =

1-8

1-8

Hz

Jt

A

C

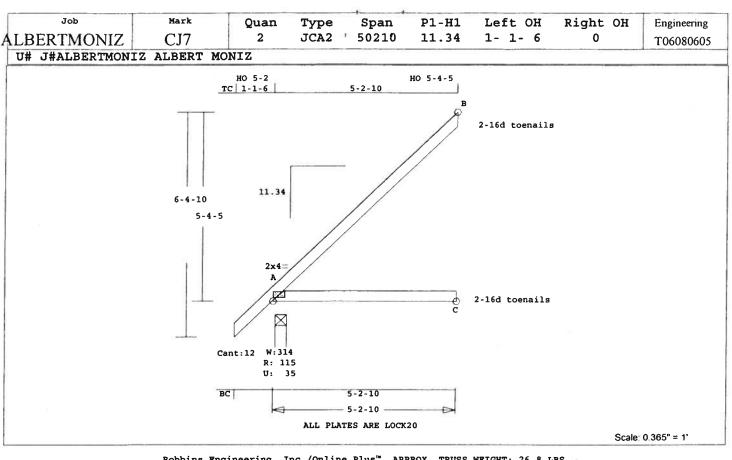
В

Lbs

471

339

232



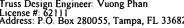
Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 26.8 LBS Hz =32 Setback 4-11- 4 OH Loading Online Plus -- Version 19.0.034 Membr CSI P Lbs Ax1-CSI-Bnd Soffit psf 2.0 RUN DATE: 04-AUG-06 -----Top Chords-----Design checked for 10 psf non-A -B 0.10 37 C 0.00 0.10 concurrent LL on BC. CSI -Size- ----Lumber---------Bottom Chords-----Use properly rated hangers for A -C 0.06 0.10 2x 4 SP-#2 O T 0.00 0.06 loads framing into girder BC 0.06 2x 4 SP-#2 truss. TL Defl -0.02" in A -C L/999 Wind Loads - ANSI / ASCE 7-02 LL Defl -0.01" in A -C L/999 Brace truss as follows: Truss is designed as a Main Shear // Grain in A -B O.C. From To 0.05 Wind-Force Resistance System. 5- 2-10 TC Cont. 0- 0- 0 Wind Speed: 110 mph 0- 0- 0 5- 2-10 Plates for each ply each face. BC Cont. Mean Roof Height: 15-0 PLATING CONFORMS TO TPI. Exposure Category: Loading (psf) REPORT: NER 691 Live Dead Occupancy Factor : 1.00 TC 20.0 10.0 ROBBINS ENGINEERING, INC. Building Type: Enclosed BC 0.0 10.0 BASED ON SP LUMBER Zone location: Exterior 40.0 USING GROSS AREA TEST. 5.0 psf Total 20.0 20.0 TC Dead Load: Spacing 24.0" Plate - LOCK 20 Ga, Gross Area BC Dead Load : 5.0 psf Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI 1.25 Lumber Duration Factor Max comp. force 37 Lbs Plate Duration Factor 1.25 Quality Control Factor 1.25 TC Fb=1.00 Fc=1.00 Ft=1.00 BC Fb=1.00 Fc=1.00 Ft=1.00 A LOCK 2.0x 4.0 0.3 0.3 0.59 Load Case # 1 Girder Loading REVIEWED BY: Robbins Engineering, Inc. Lumber Duration Factor 1.25 PO Box 280055 Plate Duration Factor 1.25 Tampa, FL 33682 Dead From To plf - Live Truss Design Engineer: Vuong Phan 20 5.2' TC V 40 0.0 License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682 BC V 0 20 0.0' 5.2' REFER TO ROBBINS ENG. GENERAL TC V -40 -20 0.0' NOTES AND SYMBOLS SHEET FOR -7 5.2' -14

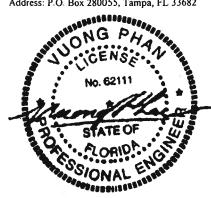
ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 Girder King Jack Loading TC and BC





1-8

47

1-8

1-8

5.2'

BC V

Plus

Jt

C

B

0

O

Lbs

42

59

-20

-7

5 Wind Load Case(s)

React Uplft Size Reg'd

0

46

1 UBC LL Load Case(s)

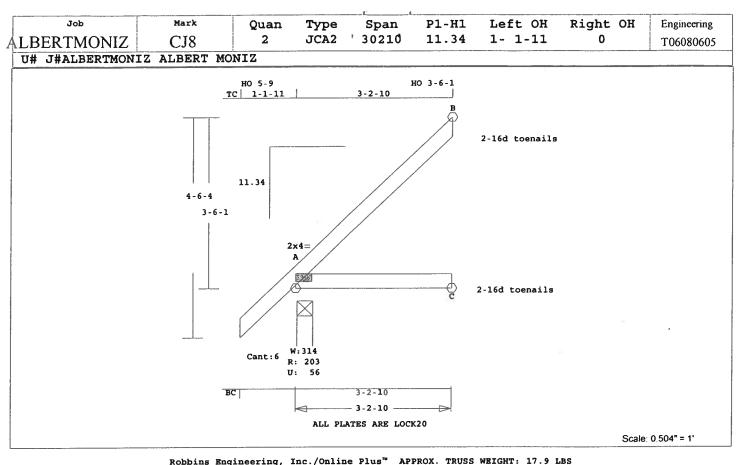
0.0

Lbs In-Sx In-Sx 3-14

Hz =

1-8

1-8



Online Plus -- Version 19.0.034 TL Defl -0.01" in A -C L/999 RUN DATE: 04-AUG-06

CSI -Size- ----Lumber----2x 4 SP-#2 0.10 0.07 2x 4 SP-#2

Brace truss as follows: O.C. From

To 3- 2-10 0-0-0 TC Cont. 0-0-0 3- 2-10 BC Cont.

Live Dead (psf) Loading 20.0 TC 10.0 BC 0.0 10.0 Total 20.0 20.0 40.0 Spacing 24.0" 1.25 Lumber Duration Factor Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

5 Wind Load Case(s) 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd Lbs In-Sx In-Sx Lbs 3-14 1-8 203 57 Hz =91 C 59 O 1 - 8 В 1 - 8 1-8 Ηz

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----54 C 0.00 0.10 A -B 0.10 ------Bottom Chords------

0.00" in A -C LL Defl L/999 Shear // Grain in A -B 0.09

A -C 0.07

0 T

0.00

0.07

Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Max comp. force Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI A LOCK 2.0x 4.0 0.5 0.5 0.61

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

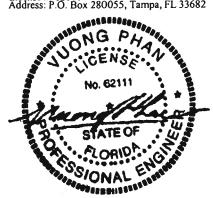
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

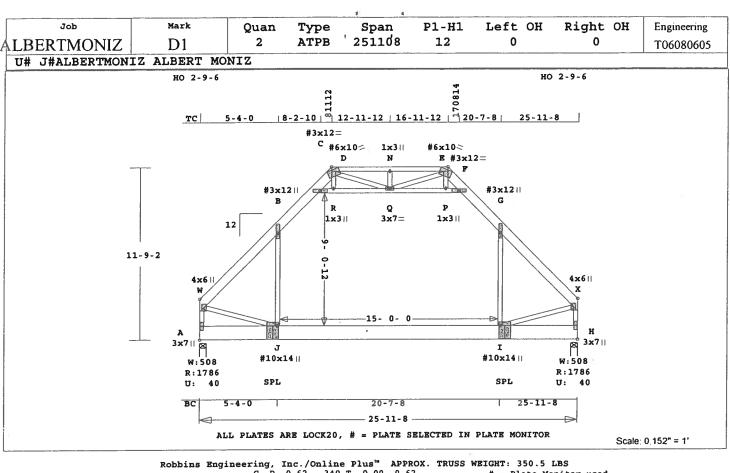
NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf non-

concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf 54 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #, 62111 Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006



Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 350.5 LBS
C -D 0.62 340 T 0.00 0.62 # = Plate Monit 340 T 0.00 0.62 318 T 0.00 0.26 # = Plate Monitor used D-N 0.26 Online Plus -- Version 19.0.034 N-E 0.26 318 T 0.00 0.26 REVIEWED BY: 340 T E-F 0.62 0.00 0.62 Robbins Engineering, Inc. RUN DATE: 04-AUG-06 1149 C 1785 C PO Box 280055 0.00 Tampa, FL 33682 G -X 0.58 0.00 0.58 CSI -Size-----Lumber------Bottom Chords----TC 0.62 2x 8 SP-#2 EX D -E 2x 4 SP-#2 A -J 0.56 267 T 0.00 0.56 REFER TO ROBBINS ENG. GENERAL 0.80 2x12 SP-#2 J-I 0.80 1178 T 0.11 0.69 NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS. 267 T 0.00 WB 0.29 2x 4 0.16 2x 4 SP-#2 I-H 0.56 0.56 -Webs----SP-#2 ACT 0.16 1972 C WindLd 1265 T A -W 0.23 NOTES: W -J 0.29 Trusses Manufactured by: J -B 0.18 971 T Mayo Truss Co. Inc. Brace truss as follows: ō.c. From To 0-0-0 8-11-12 I -G I -X 971 T 1265 T Analysis Conforms To: FBC2004 0.18 0.29 TC Cont. TC 2- 0- 0 8-11-12 16-11-12 TC Cont. 16-11-12 25-11-8 1972 C WindLd Design checked for 10 psf non-C -R 0.16 1675 C 0.16 0.00 R -Q 0.08 1677 C 0.02 0.06 Q -P 0.08 1677 C 0.02 0.06 P -F 0.16 1675 C 0.16 0.00 concurrent LL on BC.
NOTE: USER MODIFIED PLATES BC Cont. 0- 0- 0 25-11- 8 This design may have plates (psf) Q -P P -F Dead selected through a plate TC BC 20.0 10.0 monitor. 10.0 --Attic Webs (Top)-----Wind Loads - ANSI / ASCE 7-02 0.0 Truss is designed as a Main Wind-Porce Resistance System 20.0 20.0 40.0 R -D 0.01 52 T 576 T D -0 0.13 Spacing 24.0" Q -N 0.03 299 C Wind Speed: Lumber Duration Factor 1.00 Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Plate Duration Factor 1.00 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10 -E 0.13 576 T - E 0.01 Building Type: Enclosed Zone location: Exterior TL Defl -0.44" in J -I LL Defl -0.32" in J -I L/685 Load Case # 1 Attic Loading 1.00 Shear // Grain in B -C Lumber Duration Factor 0.45 TC Dead Load : 5.0 psf Plate Duration Factor 1.00 To BC Dead Load : 5.0 psf Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. Unbalanced Loads Checked plf - Live TC V 40 Dead From 26.0 BC V TC V TC V 0 20 0.0' 26.0 Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682 5.5 8.1 10 17.9 BASED ON SP LUMBER USING GROSS AREA TEST. 20.51 5.51 10 10 BC V 80 20.5 USING GROSS AREA 1857.

Plate - LOCK 20 Ga, Gross Area

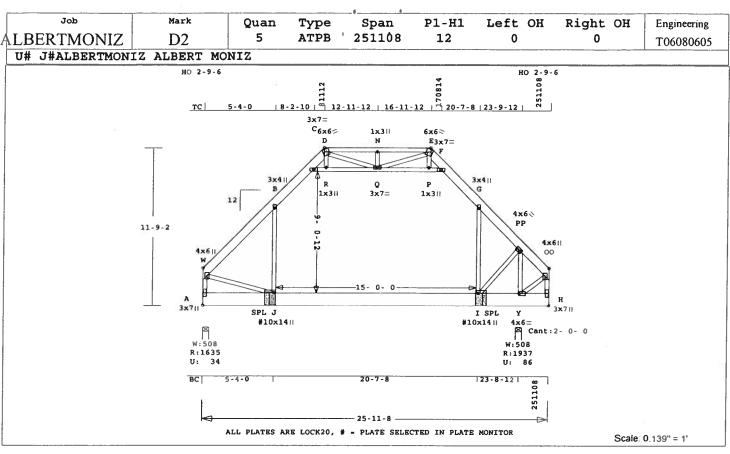
Plate - RHS 20 Ga, Gross Area

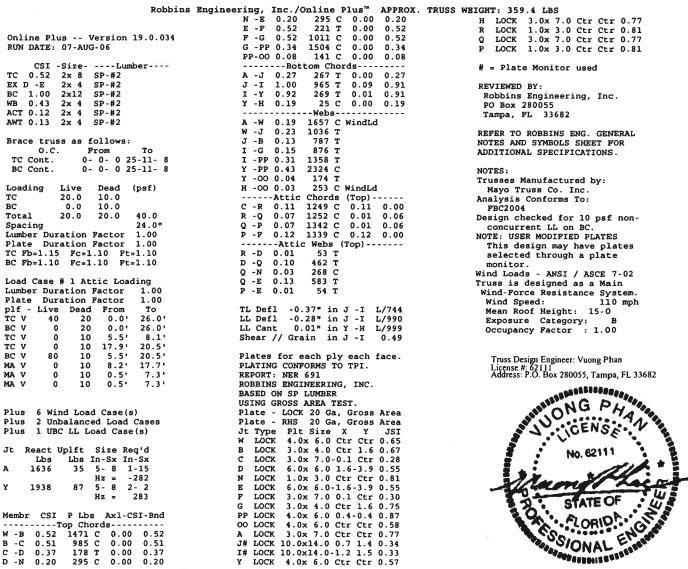
Jt Type Plt Size X Y JSI

W LOCK 4.0x 6.0 Ctr Ctr 0.77 AND CENSON MA V 8.2 17.7 0 MA NONG PHAN MA V 10 0.5 7.31 B# LOCK C# LOCK 3.0x12.0 Ctr 1.6 0.26 3.0x12.0-0.1 Ctr 0.37 6 Wind Load Case(s) Plus D# LOCK 2 Unbalanced Load Cases Plus 1 UBC LL Load Case(s) N LOCK 1.0x 3.0 Ctr Ctr 0.81 6.0x10.0-0.3-1.3 0.57 Plus F# LOCK 3.0x12.0 0.1 Ctr 0.37 React Uplft Size Req'd 3.0x12.0 Ctr 1.6 0.26 4.0x 6.0 Ctr Ctr 0.77 3.0x 7.0 Ctr Ctr 0.77 G# LOCK Lbs In-Sx In-Sx 5-8 2-2 Hz = -282 1787 40 LOCK J# LOCK 10.0x14.0 Ctr 1.8 0.42 I# LOCK 10.0x14.0 Ctr 1.8 0.42 1787 40 5-8 2-2 283 Hz = LOCK 3.0x 7.0 Ctr Ctr 0.77 Membr CSI P Lbs Axl-CSI-Bnd R LOCK 1.0x 3.0 Ctr Ctr 0.81 3.0x 7.0 Ctr Ctr 0.77 SOO ON A PROPERTY LOCK W -B 0.58 1785 C 0.00 0.58 B -C 0.62 1149 C 0.00 0.62 1.0x 3.0 Ctr Ctr 0.81

Job	Mark	Quan	Туре	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D1	2	ATPB '	251108	12	0	0	T06080605
U# J#ALBERTMONI	Z ALBERT MO	NIZ						

Load Factors = 1.00 and 0.00 Max comp. force 1972 Lbs Quality Control Factor 1.25





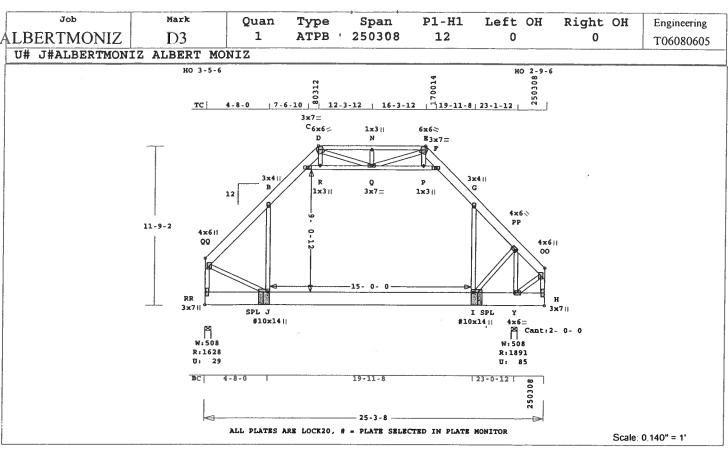
LOCK 10.0x14.0-1.2 1.5 0.33

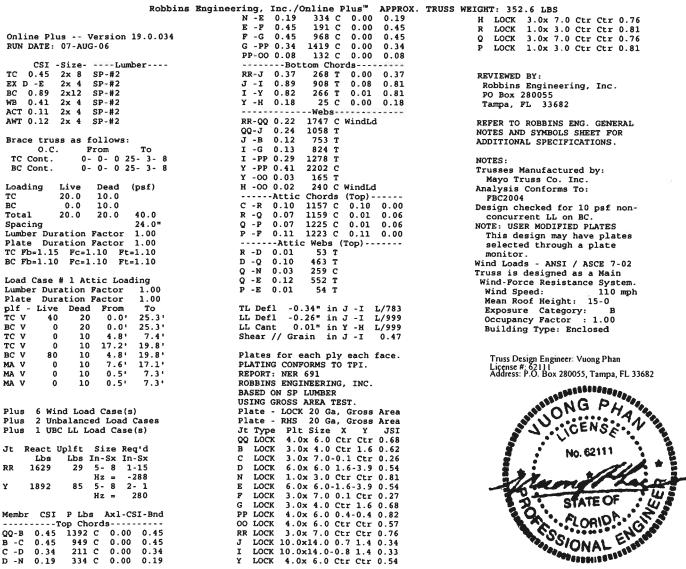
Y LOCK 4.0x 6.0 Ctr Ctr 0.57

295 C 0.00 0.20

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D2	5	ATPB	251108	12	0	0	T06080605
U# J#ALBERTMONIZ ALBERT MONIZ								

Building Type: Enclosed
Zone location: Exterior
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 2324 Lbs
Quality Control Factor 1.25





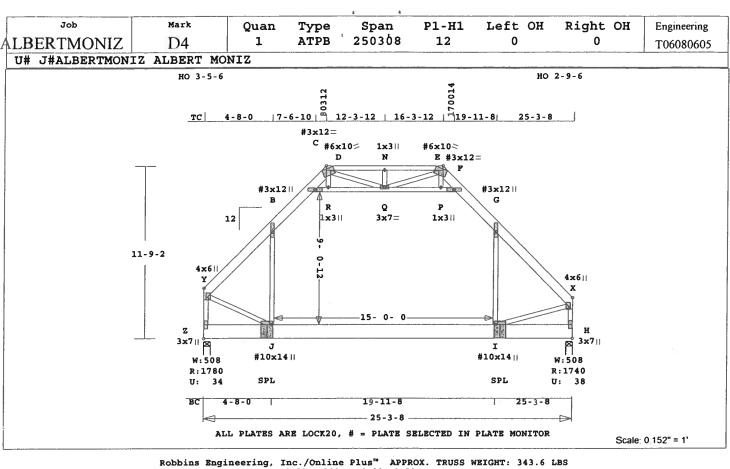
Y LOCK 4.0x 6.0 Ctr Ctr 0.54

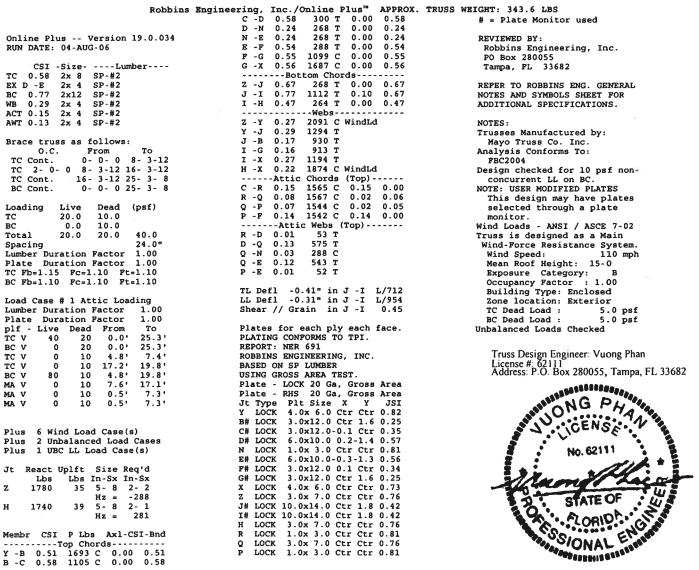
0.19

334 C 0.00 0.19

Job	Mark	Quan	Туре	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D3	1	ATPB	250308	12	0	0	T06080605
U# J#ALBERTMONI	Z ALBERT MO	DNIZ		M. 				

Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 2202 Lbs
Quality Control Factor 1.25



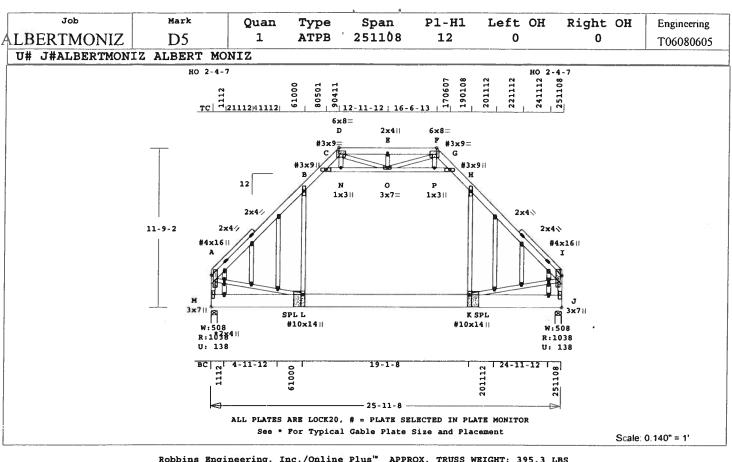


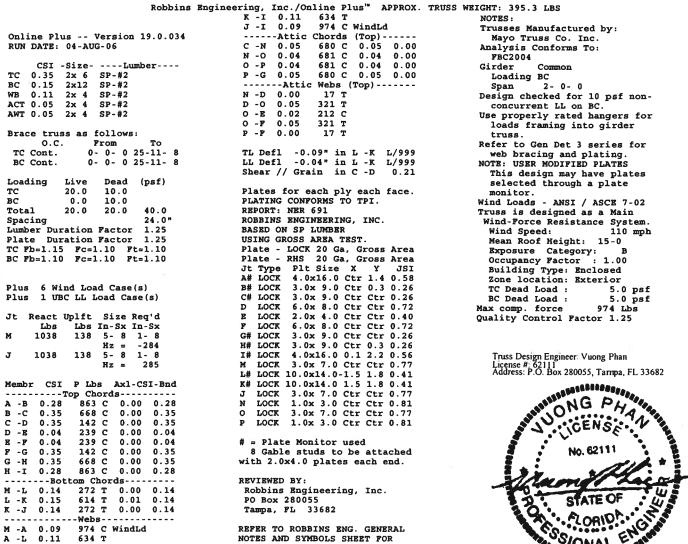
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Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D4	1	ATPB	250308	12	0	0	T06080605
U# J#ALBERTMONI	Z ALBERT M	ONIZ						

Load Factors = 1.00 and 0.00 Max comp. force 2091 Lbs Quality Control Factor 1.25



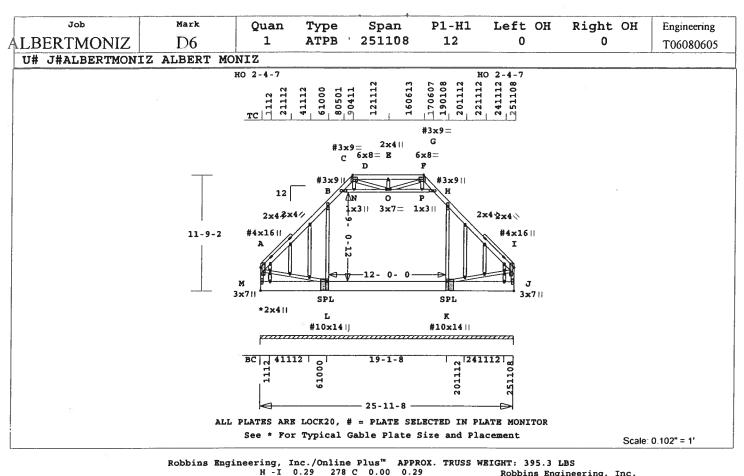


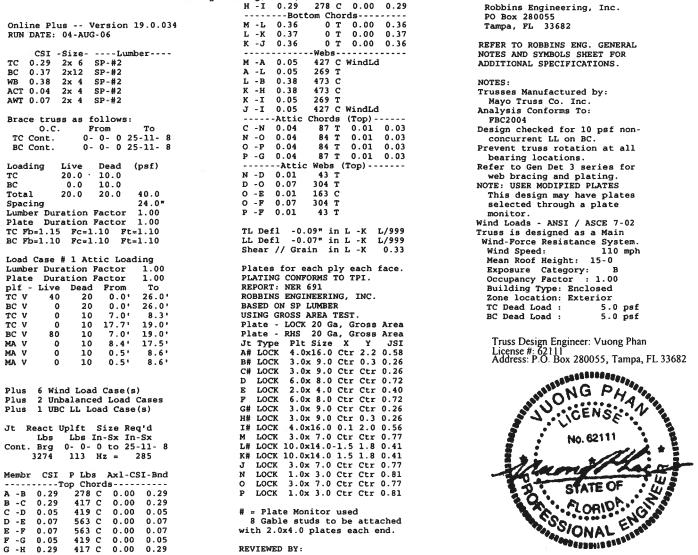
ADDITIONAL SPECIFICATIONS.

-B 0.06

-H 0.06

298 T

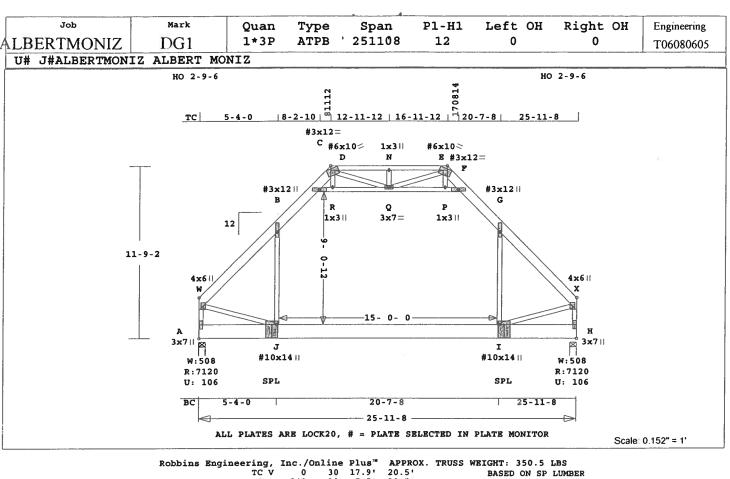


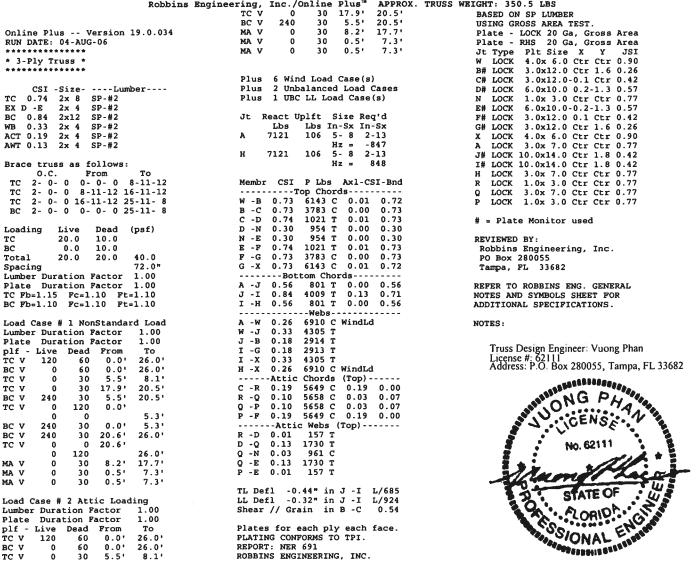


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Job	Mark	Quan	Туре	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D6	1	ATPB	251108	12	0	0	T06080605
U# J#ALBERTMONIZ ALBERT MONIZ								

Unbalanced Loads Checked Load Factors = 1.00 and 0.00 Max comp. force 563 Lbs Quality Control Factor 1.25

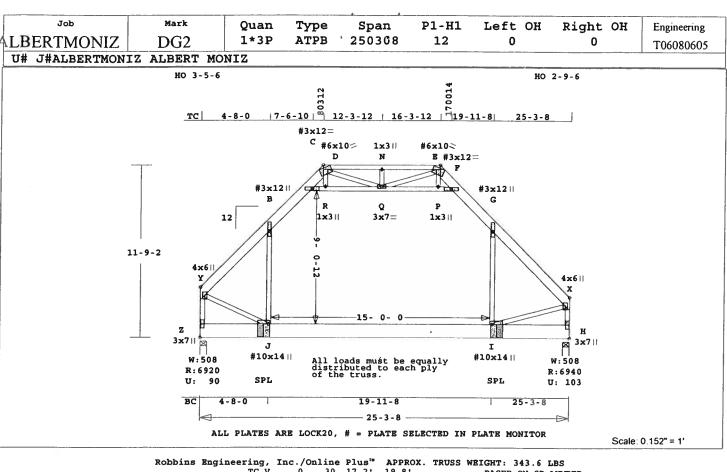


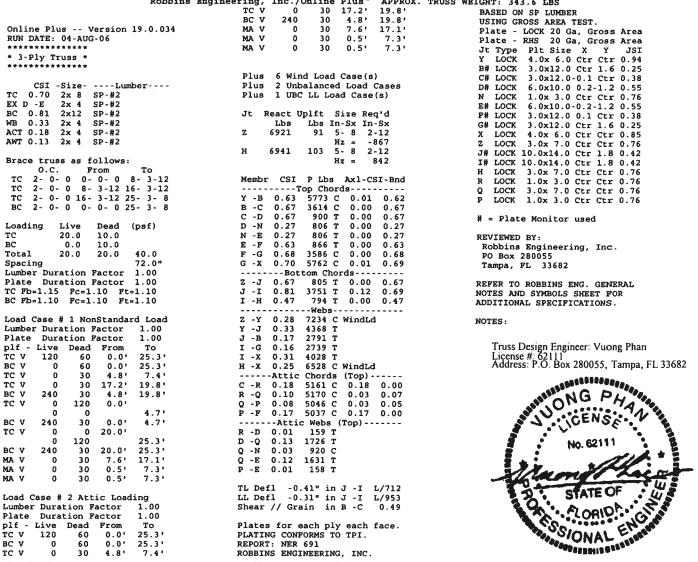


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Job	Mark	Quan	Туре	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	DG1	1*3P	ATPB	251108	12	0	0	T06080605
U# J#ALBERTMONIZ ALBERT MONIZ								

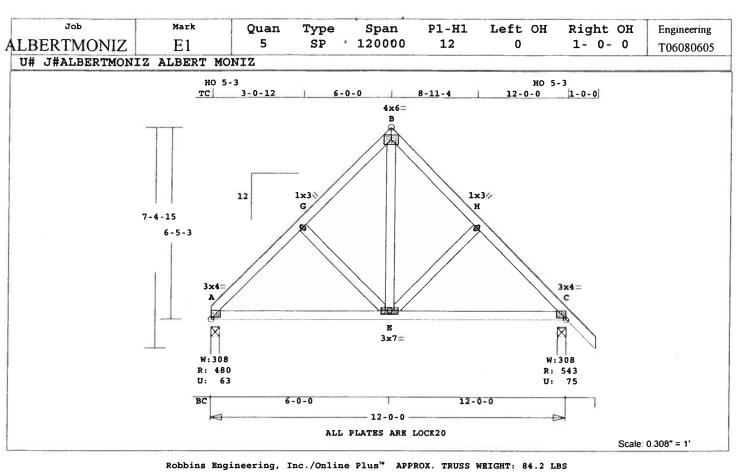
Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 3 COMPLETE TRUSSES REQUIRED. Fasten together in staggered pattern. (1/2" bolts -OR-SDS4.5 screws -OR- 16d nails as each layer is applied.) ----Spacing (In)----Nails Screws Bolts Rows TC 1 12 BC 3 12 0 20 24 WB 1 8 8 No bolts in 2x4s or smaller. Provide connection to bearing for 848 Lbs Horiz Reaction Design checked for 10 psf nonconcurrent LL on BC. Prevent truss rotation at all bearing locations. NOTE: USER MODIFIED PLATES This design may have plates selected through a plate monitor. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. wind Speed: 110 Mean Roof Height: 15-0 Wind Speed: 110 mph Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf 5.0 psf TC Dead Load : BC Dead Load : Unbalanced Loads Checked Load Factors = 1.00 and 0.00 Max comp. force 6910 Lbs Quality Control Factor 1.25





Job	Mark	Quan	Туре	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	DG2	1*3P	ATPB	250308	12	0	0	T06080605
U# J#ALBERTMONIZ ALBERT MONIZ								

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 3 COMPLETE TRUSSES REQUIRED. Pasten together in staggered pattern. (1/2" bolts -OR-SDS4.5 screws -OR- 16d nails as each layer is applied.) ----Spacing (In) ----Rows Nails Screws Bolts TC 1 12 BC 3 12 20 0 WB 1 8 8 No bolts in 2x4s or smaller. Provide connection to bearing for 868 Lbs Horiz Reaction Design checked for 10 psf nonconcurrent LL on BC. Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES This design may have plates selected through a plate monitor. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf 5.0 psf TC Dead Load : BC Dead Load : Unbalanced Loads Checked Load Factors = 1.00 and 0.00 Max comp. force 7234 Lbs Quality Control Factor 1.25



344 C 0.00 0.07 NOTES: B -H 0.07 H -C 0.07 464 C 0.00 0.07 Online Plus -- Version 19.0.034 ------Bottom Chords-----RUN DATE: 04-AUG-06 A -E 0.23 332 T 0.03 0.20 E -C 0.23 332 T 0.03 0.20 CSI -Size- ----Lumber----------Webs-----TC 0.07 2x 4 SP-#2 G -E 0.03 129 C BC 0.23 2x 4 SP-#2 E -B 0.05 328 T 0.05 2x 4SP-#2 E -H 0.03 129 C Brace truss as follows: TL Defl -0.04" in A -E L/999 From To LL Defl -0.02" in A -E L/999 Shear // Grain in A -E TC Cont. 0- 0- 0 12- 0- 0 0.13 BC Cont. 0- 0- 0 12- 0- 0 Plates for each ply each face. PLATING CONFORMS TO TPI. Loading Live Dead (psf) TC 20.0 10.0 REPORT: NER 691 ROBBINS ENGINEERING, INC. BC 0.0 10.0 Total 20.0 20.0 40.0 BASED ON SP LUMBER 24.0" Spacing USING GROSS AREA TEST. Lumber Duration Factor 1.25 Plate - LOCK 20 Ga, Gross Area Plate Duration Factor 1.25 Plate - RHS 20 Ga, Gross Area TC Fb=1.15 Fc=1.10 Ft=1.10 Jt Type Plt Size X Y JSI BC Fb=1.10 Fc=1.10 Ft=1.10 A LOCK 3.0x 4.0 0.4 0.3 0.58 G LOCK 1.0x 3.0 Ctr Ctr 0.75 B LOCK 4.0x 6.0 Ctr-0.1 0.41 Plus 4 Wind Load Case(s) Н LOCK 1.0x 3.0 Ctr Ctr 0.75 1 UBC LL Load Case(s) Plus C LOCK 3.0x 4.0-0.4 0.3 0.58 LOCK 3.0x 7.0 Ctr Ctr 0.37 React Uplft Size Req'd Lbs Lbs In-Sx In-Sx 480 3-81-8 REVIEWED BY: Hz = -135 Robbins Engineering, Inc. C 544 75 3-8 1-8

PO Box 280055 Tampa, FL 33682

Membr CSI P Lbs Ax1-CSI-Bnd REFER TO ROBBINS ENG. GENERAL -----Top Chords-----A -G 0.07 464 C 0.00 0.07 344 C 0.00 0.07 0.07

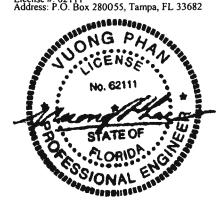
G-B

Hz =

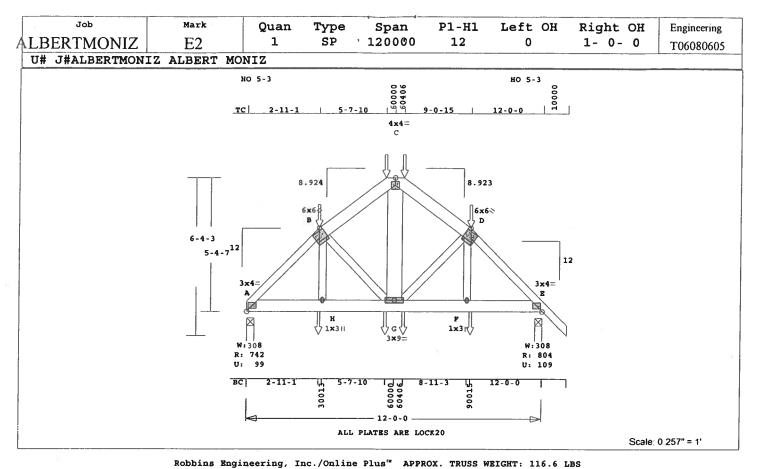
NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 464 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



135



Lbs Lbs In-Sx In-Sx 742 99 3 - 8 1-8 Online Plus -- Version 19.0.034 Hz =-110 RUN DATE: 04-AUG-06 B 805 110 3-8 1-8 Hz =111 CSI -Size- ----Lumber----TC 0.08 2x 4 SP-#2 Membr CSI P Lbs Axl-CSI-Bnd EX B -C SP-#2 2x 6 -----Top Chords----EX C -D 2x 6 0.07 SP-#2 A -B 827 C 0.00 0.07 SP-#2 BC 0.12 0.00 2x 6 B -C 0.05 604 C 0.05 WB 0.02 2x 4 SP-#2 C -D 0.05 603 C 0.00 0.05 EX G -C 2x 8 SP-#2 D -E 0.08 825 C 0.00 ---Bottom Chords-Brace truss as follows: A -H 0.11 589 T 0.07 O.C. From To H-G 0.12 587 T 0.07 0.05 0- 0- 0 12- 0- 0 TC Cont. 0.12 587 T 0.07 G-F 0.05 BC Cont. 0- 0- 0 12- 0- 0 F-E 0.11 589 T 0.07 0.04 Webs - -Loading Dead 0.01 Live (psf) H -B 115 T TC 20.0 10.0 B-G 0.02 127 C BC 0.0 10.0 G -C 0.04 405 T Total 20.0 20.0 40.0 G -D 0.02 127 C 24.0" 0.01 112 T Spacing Lumber Duration Factor 1.25 TL Defl -0.02" in G -F L/999 LL Defl -0.01" in G -F L/999 Shear // Grain in H -G 0.09 Plate Duration Factor 1.25 TC Fb=1.00 Fc=1.00 Ft=1.00 BC Fb=1.00 Fc=1.00 Ft=1.00 Load Case # 1 Standard Loading Plates for each ply each face. Lumber Duration Factor PLATING CONFORMS TO TPI. 1.25 Plate Duration Factor 1.25 REPORT: NER 691 plf - Live Dead From To ROBBINS ENGINEERING, INC. TC V 40 20 0.0 12.0' BASED ON SP LUMBER BC V 0 20 0.0' 12.0 USING GROSS AREA TEST. TC 15 Plate - LOCK 20 Ga, Gross Area 5.6' 6.4 Plate - RHS 20 Ga, Gross Area 15 5.6 6.4 Jt Type ВC 30 30 2.91 Plt Size X CL-LB Y JSI 3.0x 4.0 Ctr Ctr 0.60 TC v 41 41 2.91 CL-LB LOCK BC v 30 9.1' LOCK 6.0x 6.0 0.8 0.6 0.56 30 CL-LB В TC 41 41 9.1 CL-LB C LOCK 4.0x 4.0 Ctr Ctr 0.65 BC V 21 21 5.6' CL-LB D LOCK 6.0x 6.0-0.8 0.6 0.56 TC ν 30 30 5.61 CL-LB B LOCK 3.0x 4.0 Ctr Ctr 0.61 BC V 21 21 1.0x 3.0 Ctr Ctr 0.75 CL-LB LOCK TC V 30 30 CL-LB G LOCK 3.0x 9.0 Ctr Ctr 0.51 LOCK 1.0x 3.0 Ctr Ctr 0.75

6 Wind Load Case(s) Plus 1 UBC LL Load Case(s) Plus

Jt React Uplft Size Req'd

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

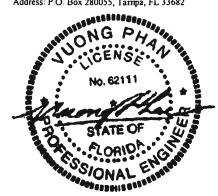
Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading

Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf

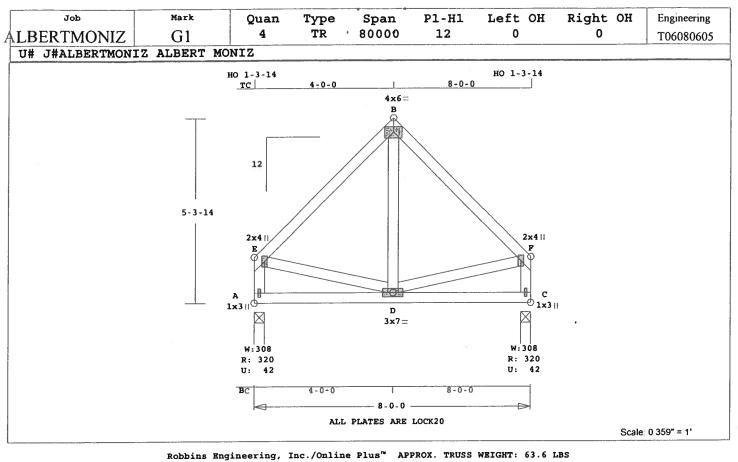
Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tarmpa, FL 33682

Quality Control Factor 1.25

827 Lbs



Max comp. force



-----Bottom Chords-----114 T 0.00 0.10 A -D 0.10 Online Plus -- Version 19.0.034 D -C 0.10 114 T 0.00 0.10 ------Webs-----RUN DATE: 04-AUG-06 287 C WindLd A-E 0.02 E -D 0.02 146 T CSI -Size- ----Lumber----0.14 2x 4 SP-#2 D -B 0.01 127 T TC 2x 4 SP-#2 D-F 0.02 146 T BC 0.10 287 C WindLd 0.02 2x 4SP-#2 C -F 0.02 TL Defl -0.01" in D -C L/999 Brace truss as follows: LL Defl -0.01" in D -C L/999 O.C. From To Shear // Grain in E -B 8- 0- 0 0.11 TC Cont. 0- 0- 0 8- 0- 0 BC Cont. 0- 0- 0 Plates for each ply each face. PLATING CONFORMS TO TPI. Loading Live Dead (psf) 20.0 10.0 REPORT: NER 691 TC ROBBINS ENGINEERING, INC. 10.0 BC 0.0 40.0 20.0 20.0 BASED ON SP LUMBER Total 24.0" USING GROSS AREA TEST. Spacing Plate - LOCK 20 Ga, Gross Area Lumber Duration Factor 1.25 Plate - RHS 20 Ga, Gross Area Plate Duration Factor 1.25 Jt Type Plt Size X Y JST TC Fb=1.15 Fc=1.10 Ft=1.10 2.0x 4.0 Ctr Ctr 0.76 E LOCK BC Fb=1.10 Fc=1.10 Ft=1.10 LOCK 4.0x 6.0 Ctr-0.1 0.36 В F LOCK 2.0x 4.0 Ctr Ctr 0.76

Α

D

LOCK LOCK

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

C LOCK 1.0x 3.0 Ctr Ctr 0.75

1.0x 3.0 Ctr Ctr 0.75

3.0x 7.0 Ctr Ctr 0.52

Membr CSI P Lbs Ax1-CSI-Bnd REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00

Building Type: Enclosed

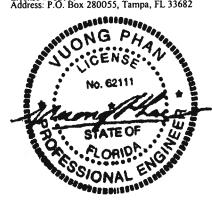
Zone location: Exterior

TC Dead Load :

5.0 psf BC Dead Load : Max comp. force 287 Lbs Quality Control Factor 1.25

5.0 psf

Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



0.14

Plus 4 Wind Load Case(s)

Lbs

320

320

0.14

E-B 0.14

B -F

Jt

Α

C

Plus 1 UBC LL Load Case(s)

React Uplft Size Req'd

43

------Top Chords-----

188 C

Lbs In-Sx In-Sx

Hz =

3 - 8

Hz =

3-8 1-8

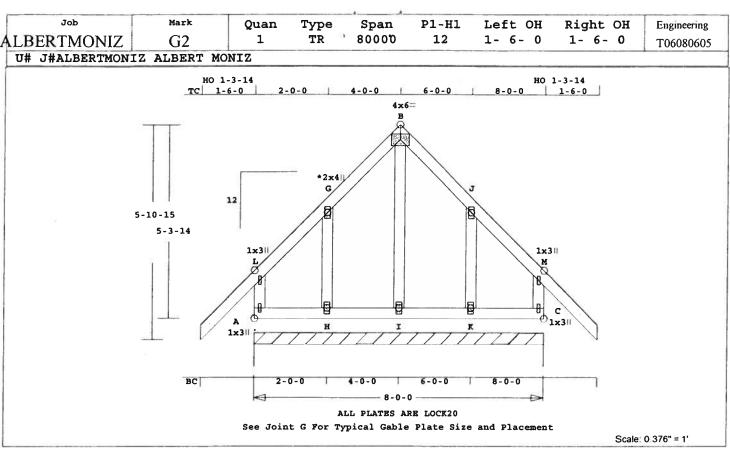
188 C 0.00 0.14

0.00

-122

1-8

123



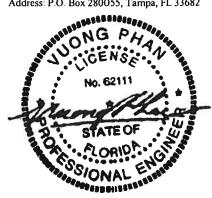
Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 68.2 LBS K -C 0.04 0 T 0.00 0.04 -Webs--75 C Online Plus -- Version 19.0.034 A -L 0.05 0.00 0.05 0.05 RUN DATE: 04-AUG-06 C -M 75 C 0.00 0.05 -----Gable Webs-----CSI -Size- ----Lumber----H -G 0.03 124 C 0.01 0.02 TC 0.03 2x 4 SP-#2 I -B 0.04 134 C BC 0.04 2x 4 SP-#2 K -J 0.03 124 C 0.01 0.02 WB 0.05 2x 4 SP-#2 0.00" in K -C L/999 GW 0.04 2x 4SP-#2 TL Defl 0.00" in K -C LL Defl L/999 Brace truss as follows: Shear // Grain in L -G 0.05 To O.C. From 0- 0- 0 8- 0- 0 TC Cont. Plates for each ply each face. 0- 0- 0 8- 0- 0 PLATING CONFORMS TO TPI. BC Cont. REPORT: NER 691 ROBBINS ENGINEERING, INC. Loading Live Dead (psf) TC 20.0 10.0 BASED ON SP LUMBER USING GROSS AREA TEST. BC 0.0 10.0 20.0 20.0 40.0 Plate - LOCK 20 Ga, Gross Area Total Plate - RHS 20 Ga, Gross Area Spacing 24.0" Lumber Duration Factor 1.25 Jt Type Plt Size X Y JSI Plate Duration Factor 1.25 LOCK 1.0x 3.0 Ctr Ctr 0.75 L 2.0x 4.0 Ctr Ctr 0.00 TC Fb=1.15 Fc=1.10 Ft=1.10 G LOCK 4.0x 6.0 Ctr-0.1 0.36 BC Fb=1.10 Fc=1.10 Ft=1.10 LOCK LOCK 2.0x 4.0 Ctr Ctr 0.00 J LOCK 1.0x 3.0 Ctr Ctr 0.75 M 4 Wind Load Case(s) LOCK 1.0x 3.0 Ctr Ctr 0.75 Plus Plus 1 UBC LL Load Case(s) Н LOCK 2.0x 4.0 Ctr Ctr 0.00 I LOCK 2.0x 4.0 Ctr Ctr 0.00 React Uplft Size Req'd ĸ LOCK 2.0x 4.0 Ctr Ctr 0.00 Lbs Lbs In-Sx In-Sx LOCK 1.0x 3.0 Ctr Ctr 0.75 0- 0- 0 to 8- 0- 0 Cont. Brg 119 Hz = 120 832 REVIEWED BY: Membr CSI P Lbs Axl-CSI-Bnd Robbins Engineering, Inc. ------Top Chords-----PO Box 280055 L -G 0.03 52 T 0.00 0.03 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by:

Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Refer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf 5.0 psf BC Dead Load : Max comp. force 134 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



0.03

0.03

0.03

0.04

0.02

0.02

0.00

0.00

0.00

0.00

0.00

0.00

125 T

125 T

---Bottom Chords---

52 T

0 T

0 T

0 T

G -B

J-M

A -H

H -I

I-K

B -J 0.03

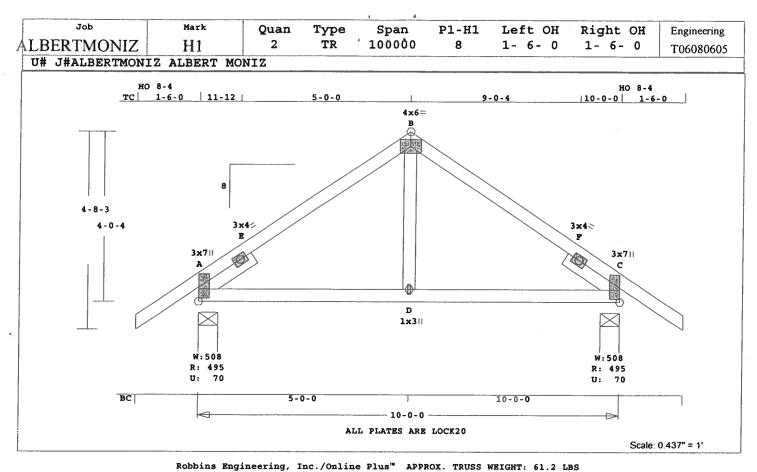
0.03

0.03

0.04

0.02

0.02



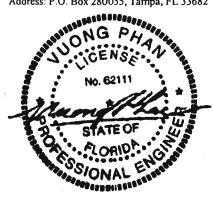
D -C 0.16 291 T 0.03 0.13

Design checked for 10 psf non-

Online Plus -- Version 19.0.034 D -B 0.03 201 T RUN DATE: 04-AUG-06 -----Sliders-A -E 0.02 273 C CSI -Size- ----Lumber----F -C 0.02 273 C TC 0.12 SP-#2 2x 4 BC 0.16 2x 4 SP-#2 -0.02" in A -D TL Defl L/999 LL Defl -0.01" in A -D WB 0.03 2x 4 SP-#2 L/999 Shear // Grain in E -B 0.02 2x 4SP-#2 Brace truss as follows: Plates for each ply each face. 0.C. From To PLATING CONFORMS TO TPI. 0- 0- 0 10- 0- 0 TC Cont. REPORT: NER 691 BC Cont. 0- 0- 0 10- 0- 0 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER Loading Live Dead (psf) USING GROSS AREA TEST. TC 20.0 10.0 Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area BC 0.0 10.0 40.0 Plt Size X Y Total 20.0 20.0 Jt Type JSI Spacing 24.0" A LOCK 3.0x 7.0 1.5 0.6 0.55 Lumber Duration Factor 1.25 LOCK 3.0x 4.0 Ctr Ctr 0.50 E Plate Duration Factor 1.25 LOCK 4.0x 6.0 Ctr Ctr 0.40 В TC Fb=1.15 Fc=1.10 Ft=1.10 F LOCK 3.0x 4.0 Ctr Ctr 0.50 BC Fb=1.10 Fc=1.10 Ft=1.10 C LOCK 3.0x 7.0-1.5 0.6 0.55 LOCK 1.0x 3.0 Ctr Ctr 0.75 Plus 6 Wind Load Case(s) Plus 1 UBC LL Load Case(s) REVIEWED BY: Robbins Engineering, Inc. React Uplft Size Req'd Jt PO Box 280055 Tampa, FL 33682 Lbs Lbs In-Sx In-Sx 5-81-8 496 70 -66 Hz =REFER TO ROBBINS ENG. GENERAL C 70 5-8 NOTES AND SYMBOLS SHEET FOR 496 1-8 Hz = 67 ADDITIONAL SPECIFICATIONS. Membr CSI P Lbs Axl-CSI-Bnd NOTES: ------Top Chords-----Trusses Manufactured by: 0.00 0.05 A -E 0.05 217 C Mayo Truss Co. Inc. 0.00 0.12 345 C Analysis Conforms To: 345 C 0.00 0.12 В - F 0.12 FBC2004 OH Loading F -C 0.05 217 C 0.00 0.05 --Bottom Chords----Soffit psf 2.0

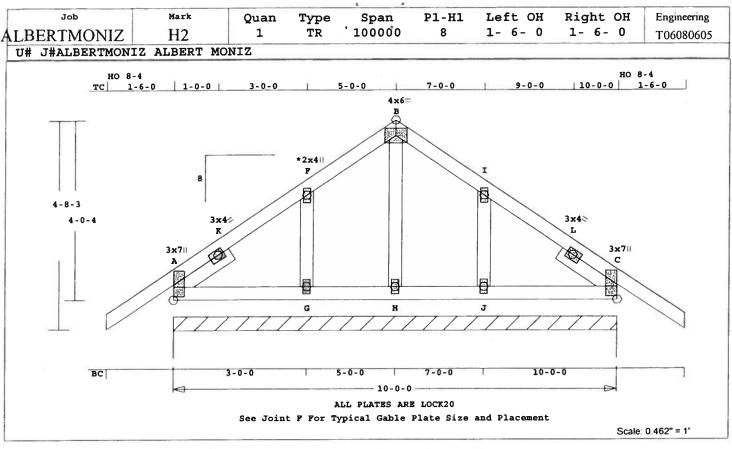
concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 345 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



291 T 0.03 0.13

A -D 0.16



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.3 LBS 0 T G -H 0.03 0.00 0.03 NOTES: 0.00 0.03 H -J 0.03 0 T Trusses Manufactured by: Online Plus -- Version 19.0.034 J -C 0.03 5 T 0.00 0.03 Mayo Truss Co. Inc. ------Gable Webs----Analysis Conforms To: RUN DATE: 04-AUG-06 G -F 0.01 145 C FBC2004 CSI -Size- ----Lumber----H-B 0.01 84 C OH Loading 2x 4 SP-#2 J -I 0.01 145 C Soffit psf 2.0 TC 0.05 2x 4 SP-#2 -----Sliders-----Design checked for 10 psf non-BC 0.03 A -K 0.00 SP-#2 86 C concurrent LL on BC. 2 x 4 GW 0.01 0.00 2x 4SP-#2 L -C 0.00 86 C Refer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02 0.00" in A -G L/999 TL Defl Brace truss as follows: To LL Defl 0.00" in A -G L/999 Truss is designed as a Main O.C. From 0- 0- 0 10- 0- 0 Shear // Grain in K -F TC Cont. Wind-Force Resistance System. 0- 0- 0 10- 0- 0 Wind Speed: 110 mph BC Cont. Plates for each ply each face. Mean Roof Height: 15-0 PLATING CONFORMS TO TPI. Loading Live Dead (psf) Exposure Category: В REPORT: NER 691 Occupancy Factor : 1.00 Building Type: Enclosed 20.0 10.0 TC ROBBINS ENGINEERING, INC. BC 0.0 10.0 40.0 BASED ON SP LUMBER Zone location: Exterior Total 20.0 20.0 24.0" USING GROSS AREA TEST. 5.0 psf TC Dead Load : Spacing Lumber Duration Factor 1.25 Plate - LOCK 20 Ga, Gross Area BC Dead Load : 5.0 psf Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI 1.25 Max comp. force 145 Lbs Plate Duration Factor Quality Control Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 LOCK 3.0x 7.0 1.5 0.6 0.55 BC Fb=1.10 Fc=1.10 Ft=1.10 3.0x 4.0 Ctr Ctr 0.50 LOCK LOCK 2.0x 4.0 Ctr Ctr 0.00 В LOCK 4.0x 6.0 Ctr Ctr 0.40 Plus 6 Wind Load Case(s) 2.0x 4.0 Ctr Ctr 0.00 Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682 Plus 1 UBC LL Load Case(s) I LOCK LOCK 3.0x 4.0 Ctr Ctr 0.50 BURNES PARTE LOCK 3.0x 7.0-1.5 0.6 0.55 C React Uplft Size Req'd JUONG Lbs Lbs In-Sx In-Sx G LOCK 2.0x 4.0 Ctr Ctr 0.00 2.0x 4.0 Ctr Ctr 0.00 0- 0- 0 to 10- 0-0 LOCK Н Cont. Brg 992 140 Hz =67 J LOCK 2.0x 4.0 Ctr Ctr 0.00 Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----REVIEWED BY: 84 C 0.00 0.01 Robbins Engineering, Inc. 0.01

0.05

0.05

0.05

0.05

0.01

0.03

PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

K-F

F-B

I -L

L -C

0.05

0.05

0.05

0.01

B -I 0.05

A -G 0.03

118 C

133 C

133 C 118 C

84 C

-----Bottom Chords---

0.00

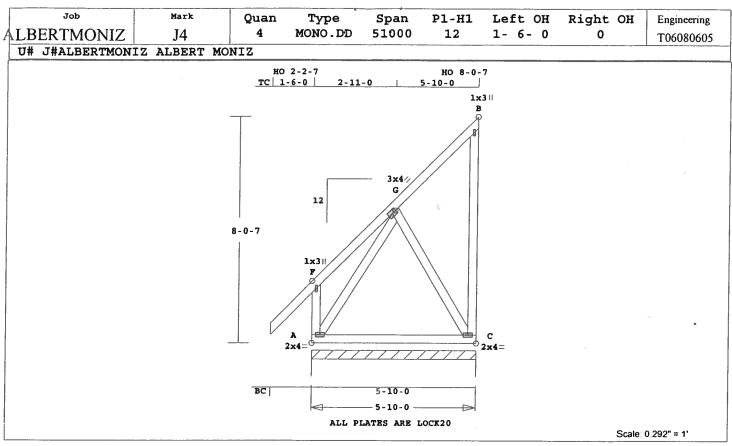
0.00

0.00

0.00

0.00

5 T 0.00



Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 67.6 LBS ---------Webs-----OH Loading A-F 0.01 80 T WindLd Soffit psf 2.0 Online Plus -- Version 19.0.034 A -G 0.08 236 C Design checked for 10 psf non-RUN DATE: 04-AUG-06 G -C 0.04 149 T concurrent LL on BC. C -B 0.11 91 T WindLd Wind Loads - ANSI / ASCE 7-02 CSI -Size- ----Lumber----Truss is designed as a Main TC 0.10 2x 4 SP-#2 TL Defl -0.10" in A -C L/712 Wind-Force Resistance System. BC 0.24 2x 4 SP-#2 LL Defl -0.05" in A -C L/999 Wind Speed: 110 mph Shear // Grain in A -C 0.11 2x 4SP-#2 Mean Roof Height: 15-0 Exposure Category: В Brace truss as follows: Plates for each ply each face. Occupancy Factor : 1.00 O.C. From To PLATING CONFORMS TO TPI. Building Type: Enclosed TC Cont. 0- 0- 0 5-10- 0 REPORT: NER 691 Zone location: Exterior ROBBINS ENGINEERING, INC. BC Cont. 0-0-0 5-10- 0 TC Dead Load : 5.0 psf BASED ON SP LUMBER BC Dead Load : 5.0 psf Loading Live Dead (psf) USING GROSS AREA TEST. Max comp. force 236 Lbs TC 20.0 10.0 Plate - LOCK 20 Ga, Gross Area Quality Control Factor 1.25 BC 0.0 10.0 Plate - RHS 20 Ga, Gross Area Total 20.0 20.0 40.0 Jt Type Plt Size X Y 1.0x 3.0 Ctr Ctr 0.75 Spacing 24.0" F LOCK G LOCK Lumber Duration Factor 1.25 3.0x 4.0 Ctr Ctr 0.55 Plate Duration Factor 1.25 B LOCK 1.0x 3.0 Ctr Ctr 0.75 TC Fb=1.15 Fc=1.10 Ft=1.10 A LOCK 2.0x 4.0 Ctr Ctr 0.75 BC Fb=1.10 Fc=1.10 Ft=1.10 C LOCK 2.0x 4.0 Ctr Ctr 0.75 Truss Design Engineer: Vuong Phan License # 62111 Address: P.O. Box 280055, Tampa, FL 33682 Plus 4 Wind Load Case(s) REVIEWED BY:

PO Box 280055

NOTES:

FBC2004

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL

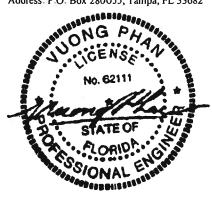
NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Trusses Manufactured by:

Mayo Truss Co. Inc.

Analysis Conforms To:

Robbins Engineering, Inc.



0.00 0.09

266

Plus 1 UBC LL Load Case(s)

Lbs

563

Cont. Brg

F-G 0.10

G-B 0.09

A -C 0.24

React Uplft Size Req'd

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----

-----Bottom Chords-----

113 C

79 Hz =

Lbs In-Sx In-Sx

0- 0- 0 to 5-10- 0

81 T 0.01 0.09

0 T 0.00 0.24

Job Mark Quan Type Span P1-H1 Left OH Right OH Engineering 5 MONO.DD 20000 J5 12 1-6-0 ALBERTMONIZ 0 T06080605 U# J#ALBERTMONIZ ALBERT MONIZ HO 2-2-7 HO 4-2-7 1-6-0 2-0-0 1x3 II 12 2×4 || 4-2-7 C 1x31  $\oplus_{2\times 4}=$ BC 2-0-0 - 2-0-0 -ALL PLATES ARE LOCK20

Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 28.0 LBS

A -D 0.01 131 C WindLd D -C 0.01 127 T

Online Plus -- Version 19.0.034 C -B 0.02 60 C WindLd

RUN DATE: 04-AUG-06

CSI -Size- ---Lumber---TC 0.03 2x 4 SP-#2
BC 0.02 2x 4 SP-#2
WB 0.02 2x 4 SP-#2

Brace truss as follows:

O.C. From To

TC Cont. 0-0-0 2-0-0

BC Cont. 0-0-0 2-0-0

Loading Live Dead (psf) TC 20.0 10.0 BC 0.0 10.0 20.0 Total 40.0 20.0 Spacing 24.0" Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd

Lbs Lbs In-Sx In-Sx

Cont. Brg 0- 0- 0 to 2- 0- 0

256 38 Hz = 129

TL Defl 0.00" in A -C L/999 LL Defl 0.00" in A -C L/999 Shear // Grain in D -B 0.04

Plates for each ply each face.

PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER 2- 0- 0 USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI D LOCK 2.0x 4.0 Ctr Ctr 0.75 В LOCK 1.0x 3.0 Ctr Ctr 0.75 Α LOCK 1.0x 3.0 Ctr Ctr 0.75 LOCK 2.0x 4.0 Ctr Ctr 0.75

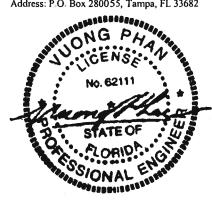
> REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

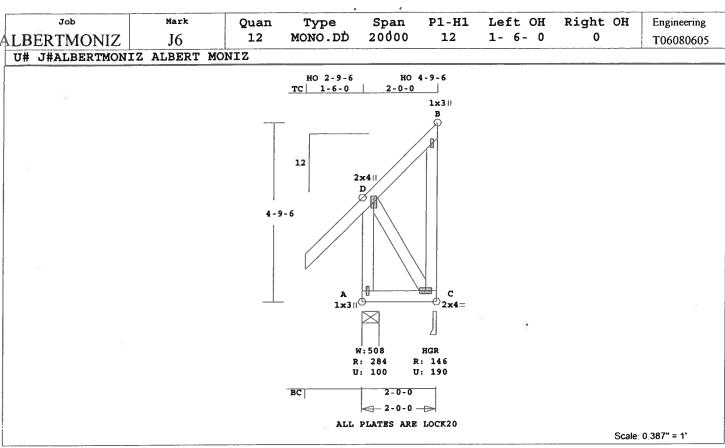
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
 Mayo Truss Co. Inc.
Analysis Conforms To:
 FBC2004
OH Loading
 Soffit psf 2.0
Design checked for 10 psf non-

concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 131 Lbs Quality Control Factor 1.25

Scale: 0.482" = 1'





Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 31.1 LBS 96 T 0.00 0.02 Soffit psf 2.0 A -C 0.02 -Webs-----Design checked for 10 psf non-178 C WindLd Online Plus -- Version 19.0.034 A -D 0.01 concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 RUN DATE: 04-AUG-06 D -C 0.02 167 T C -B 0.03 61 T WindLd Truss is designed as a Main CSI -Size- ----Lumber----Wind-Force Resistance System. TL Defl 0.00" in A -C L/999 0.03 2x 4 SP-#2 Wind Speed: 0.00" in A -C L/999 0.02 2x 4 SP-#2 LL Defl Mean Roof Height: 15-0 0.03 2x 4 SP-#2 Shear // Grain in D -B 0.04 Exposure Category: В Occupancy Factor : 1.00 Plates for each ply each face. Brace truss as follows: Building Type: Enclosed PLATING CONFORMS TO TPI. To Zone location: Exterior O.C. From 0-0-0 2- 0- 0 REPORT: NER 691 TC Dead Load : 5.0 psf TC Cont. 0-0-0 2- 0- 0 ROBBINS ENGINEERING, INC. BC Dead Load : 5.0 psf BC Cont. BASED ON SP LUMBER Max comp. force 178 Lbs USING GROSS AREA TEST. Dead (psf) Quality Control Factor 1.25 Live Loading TC 20.0 10.0 Plate - LOCK 20 Ga, Gross Area 0.0 10.0 Plate - RHS 20 Ga, Gross Area BC Jt Type Plt Size X Total 20.0 20.0 40.0 Y JSI 2.0x 4.0 Ctr Ctr 0.75 Spacing 24.0" D LOCK 1.0x 3.0 Ctr Ctr 0.75 LOCK 1.25 Lumber Duration Factor В

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Plate Duration Factor 1.25

TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Jt React Uplft Size Req'd

Lbs Lbs In-Sx In-Sx
A 285 101 5-8 1-8

Hz = -115
C 147 190 3-8 1-8

Hz = 148

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords----D-B 0.03 71 C 0.00 0.03
-----Bottom Chords-----

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

C LOCK 2.0x 4.0 Ctr Ctr 0.75

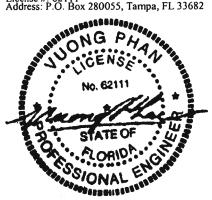
1.0x 3.0 Ctr Ctr 0.75

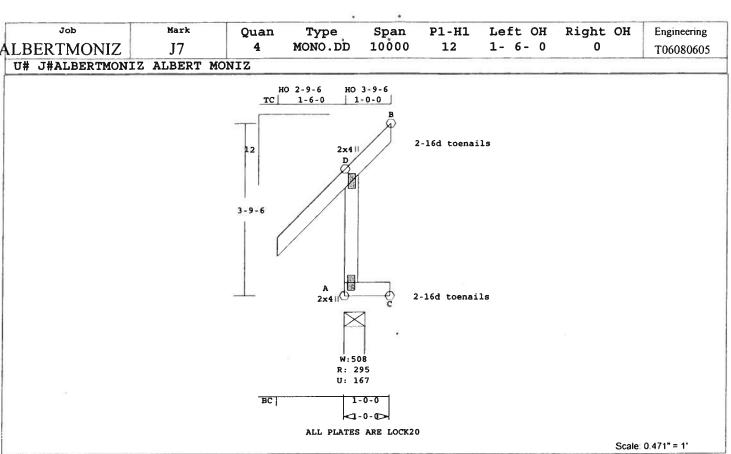
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

LOCK

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading





Robbins Engineering, Inc./Online Plus" APPROX. TRUSS WEIGHT: 13.6 LBS -----Bottom Chords-----FBC2004 A -C 0.12 0 T 0.00 0.12 OH Loading Online Plus -- Version 19.0.034 -------Webs-----Soffit psf 2.0 A -D 0.13 109 T 0.00 0.13 RUN DATE: 04-AUG-06 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 CSI -Size- ----Lumber----TL Defl 0.00" in A -C L/999 0.00" in A -C L/999  $0.08 \ 2x \ 4 \ SP-\#2$ LL Defl TC Truss is designed as a Main 0.12 2x 4 SP-#2 Shear // Grain in A -C Wind-Force Resistance System. 0.13 2x 4 Wind Speed: 110 mph Plates for each ply each face. Mean Roof Height: 15-0 PLATING CONFORMS TO TPI. Brace truss as follows: Exposure Category: REPORT: NER 691 o.c. From To Occupancy Factor : 1.00 ROBBINS ENGINEERING, INC. TC Cont. 0-0-0 1-0-0 Building Type: Enclosed 0- 0- 0 1- 0- 0 BASED ON SP LUMBER BC Cont. Zone location: Exterior USING GROSS AREA TEST. TC Dead Load : 5.0 psf (psf) Plate - LOCK 20 Ga, Gross Area 5.0 psf Live BC Dead Load : Loading Dead TC 20.0 10.0 Plate - RHS 20 Ga, Gross Area 87 Lbs Max comp. force Jt Type Plt Size X Y JSI BC 0.0 10.0 Quality Control Factor 1.25 20.0 40.0 D LOCK 2.0x 4.0 Ctr Ctr 0.38 Total 20.0 A LOCK 2.0x 4.0 Ctr Ctr 0.38 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25

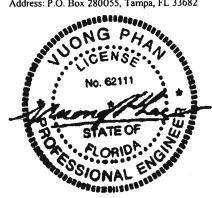
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

86

1- 8 1- 8

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

1 UBC LL Load Case(s)

React Uplft Size Req'd

167

102

109

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

Lbs In-Sx In-Sx

Hz =

1 - 8

1-8

Hz =

5-8 1-8

111 T 0.00 0.08

Plus 4 Wind Load Case(s)

Lbs

295

43

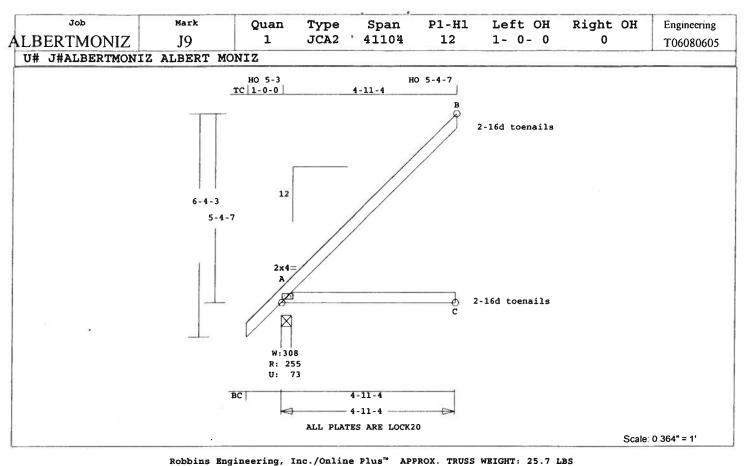
99

D-B 0.08

A

C

В



Online Plus -- Version 19.0.034 TL Defl -0.05" in A -C L/999
RUN DATE: 04-AUG-06 LL Defl -0.02" in A -C L/999

A -C 0.18

Shear // Grain in A -B 0.14

O T 0.00 0.18

CSI -Size- ----Lumber---TC 0.24 2x 4 SP-#2
BC 0.18 2x 4 SP-#2

Brace truss as follows:
 O.C. From To
TC Cont. 0- 0- 0 4-11- 4
BC Cont. 0- 0- 0 4-11- 4

Loading Live Dead (psf) 10.0 TC 20.0 BC 0.0 10.0 20.0 20.0 40.0 Total 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

BC Fb=1.10 Fc=1.10 Ft=1.10

React Uplft Size Req'd Jŧ Lbs Lbs In-Sx In-Sx 256 3-81-8 Hz =149 С 92 0 1-8 1-8 В 126 104 1-8 1-8 Hz =101

 Membr
 CSI
 P Lbs
 Ax1-CSI-Bnd

 -----Top
 Chords----- 

 A -B
 0.24
 90 C
 0.00
 0.24

 -----Bottom
 Chords-----

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 0.4 0.3 0.58

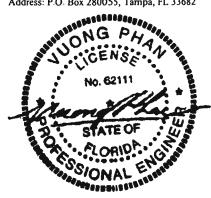
REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

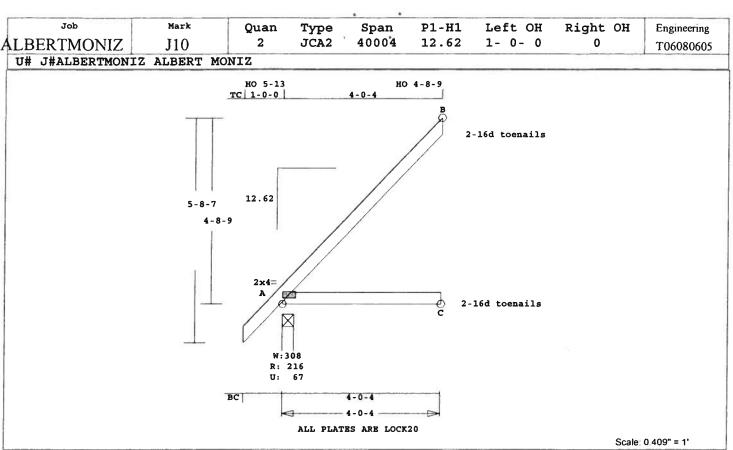
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
 Mayo Truss Co. Inc.
Analysis Conforms To:
 FBC2004
OH Loading
 Soffit psf 2.0
Design checked for 10 psf non-

concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 90 Lbs Quality Control Factor 1.25





Online Plus -- Version 19.0.034 TL Defl -0.02" in A -C L/999 RUN DATE: 04-AUG-06

CSI -Size- ----Lumber----TC 0.16 2x 4 SP-#2 BC 0.12 2x 4 SP-#2

Brace truss as follows: To O.C. From TC Cont. 0- 0- 0 4-0-4 BC Cont. 0-0-0 4-0-4

(psf) Loading Live Dead TC 20.0 10.0 ВC 0.0 10.0 Total 20.0 20.0 40.0 Spacing 24.0" Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s) Plus 1 UBC LL Load Case(s)

React Uplft Size Req'd Lbs In-Sx In-Sx Lbs 3-81-8 Hz =128 75 2 1 - 8 C 1-8 B 103 91 1-8 1-8 Hz =87

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----A -B 0.16 79 C 0.00 0.16 -----Bottom Chords-----

LL Defl -0.01" in A -C L/999 Shear // Grain in A -B 0.11

0 T 0.00 0.12

A -C 0.12

Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Max comp. force Plate - RHS 20 Ga, Gross Area Quality Control Factor 1.25 Jt Type Plt Size X Y JSI A LOCK 2.0x 4.0 0.7 0.7 0.69

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

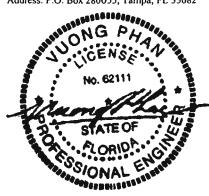
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

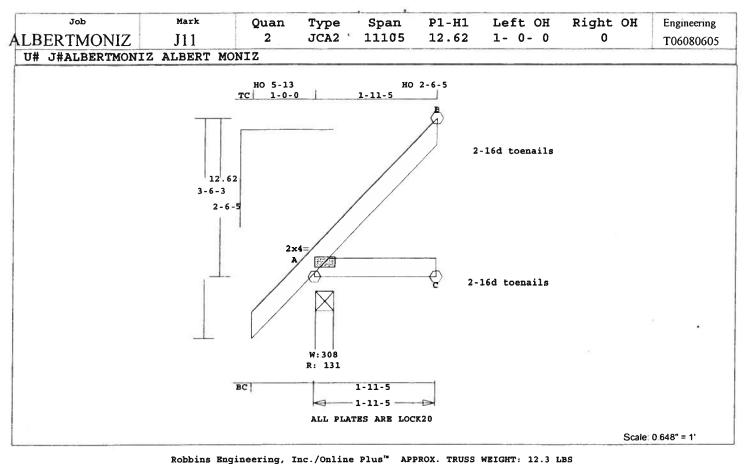
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf non-

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.9 LBS

concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf 79 Lbs





Online Plus -- Version 19.0.034 TL Defl RUN DATE: 04-AUG-06 LL Defl

CSI -Size- ----Lumber---TC 0.03 2x 4 SP-#2
BC 0.02 2x 4 SP-#2

Brace truss as follows:
O.C. From

O.C. From To

TC Cont. 0- 0- 0 1-11- 5

BC Cont. 0- 0- 0 1-11- 5

Dead (psf) Loading Live TC 20.0 10.0 0.0 10.0 BC 20.0 20.0 40.0 Total Spacing 24.0" Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd Lbs Lbs In-Sx In-Sx 3-8 1-8 131 63 Hz =50 1-8 1-8 55 В C 35 1-8 1-8 Hz =

TL Defl 0.00" in A -C L/999 LL Defl 0.00" in A -C L/999 Shear // Grain in A -B 0.04

42 T 0.00 0.02

A -C 0.02

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 0.7 0.7 0.69

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc.

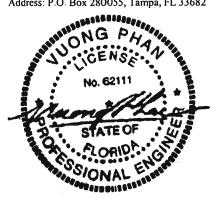
Analysis Conforms To: FBC2004

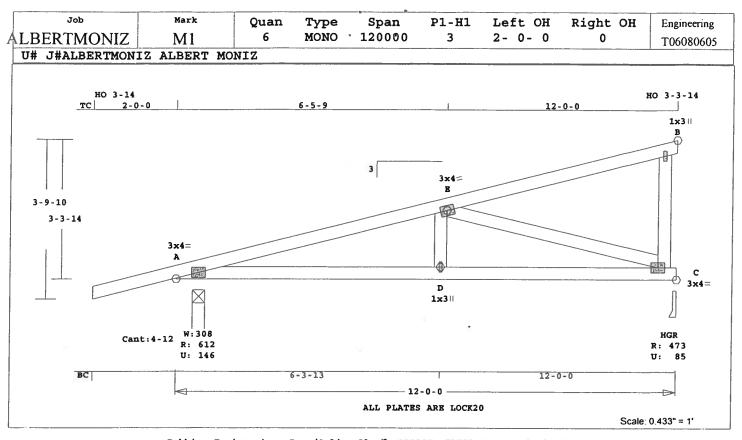
OH Loading
Soffit psf 2.0

Design checked for 10 psf non-

concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load: 5.0 psf Max comp. force 43 Lbs

Quality Control Factor 1.25

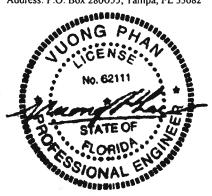




Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 67.7 LBS A -D 0.28 943 T 0.09 0.19 943 T D -C 0.28 0.09 0.19 Online Plus -- Version 19.0.034 ------Webs - - -RUN DATE: 04-AUG-06 D-E 0.03 255 T E -C 0.38 984 C C -B 0.01 CSI -Size- ----Lumber----136 C WindLd TC 0.33 2x 4 SP-#2 TL Defl -0.07" in A -D L/999 BC 0.28 2x 4 SP-#2 LL Defl -0.03" in A -D L/999 WR 0.38 2x 4 SP-#2 Shear // Grain in E -B 0.24 Brace truss as follows: o.c. From To Plates for each ply each face. TC Cont. 0- 0- 0 12- 0- 0 PLATING CONFORMS TO TPI. BC Cont. 0- 0- 0 12- 0- 0 REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER Loading Live Dead (psf) TC 20.0 10.0 USING GROSS AREA TEST. BC 0.0 10.0 Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Total 20.0 20.0 40.0 Spacing 24.0" Jt Type Plt Size X Y Lumber Duration Factor 1.25 3.0x 4.0 Ctr Ctr 0.73 A LOCK Plate Duration Factor 1.25 LOCK 3.0x 4.0 Ctr Ctr 0.53 TC Fb=1.15 Fc=1.10 Ft=1.10 B LOCK 1.0x 3.0 Ctr Ctr 0.75 BC Fb=1.10 Fc=1.10 Ft=1.10 LOCK D 1.0x 3.0 Ctr Ctr 0.75 C LOCK 3.0x 4.0 Ctr Ctr 0.63 Plus 5 Wind Load Case(s) 1 UBC LL Load Case(s) Plus REVIEWED BY: Robbins Engineering, Inc. React Uplft Size Req'd PO Box 280055 Lbs Lbs In-Sx In-Sx Tampa, FL 33682 613 146 3-8 1-8 Hz =45 REFER TO ROBBINS ENG. GENERAL C 473 85 3 - 8 1 - 8 NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS. Hz =105 Membr CSI P Lbs Axl-CSI-Bnd NOTES: -----Top Chords-----Trusses Manufactured by: 0.00 0.33 A -E 0.33 965 C Mayo Truss Co. Inc.

OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 984 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



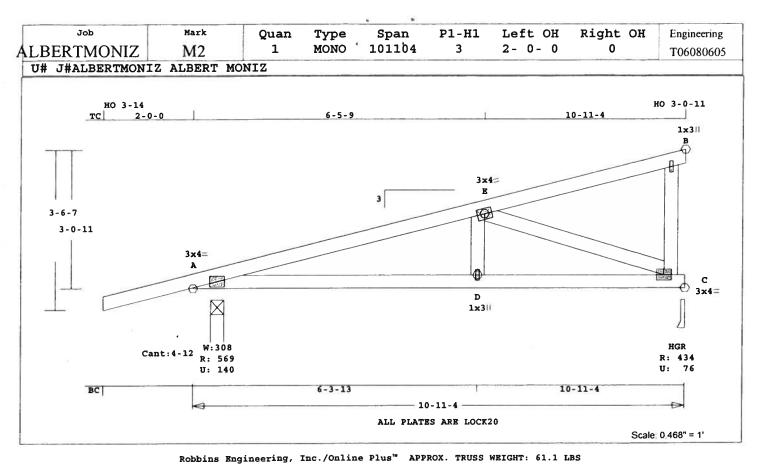
Analysis Conforms To:

FBC2004

34 C 0.00 0.33

-----Bottom Chords-----

0.33



766 T 0.12 A -D 0.26 0.14 OH Loading D-C 0.24 766 T 0.08 0.16 Online Plus -- Version 19.0.034 -------Webs - -D -E 0.03 RUN DATE: 04-AUG-06 232 T E -C 0.21 817 C CSI -Size- ----Lumber----C -B 0.01 102 C WindLd 0.28 2x 4 SP-#2 2x 4 TL Defl -0.07" in A -D L/999 0.26 SP-#2 LL Defl -0.03" in A -D L/999 0.21 2x 4 SP-#2 Shear // Grain in A -E 0.23 Brace truss as follows: Plates for each ply each face. 0.C. From To PLATING CONFORMS TO TPI. 0- 0- 0 10-11- 4 TC Cont. 0- 0- 0 10-11- 4 REPORT: NER 691 BC Cont. ROBBINS ENGINEERING, INC. BASED ON SP LUMBER Loading Live Dead (psf) USING GROSS AREA TEST. TC 20.0 10.0 BC 0.0 10.0 Plate - LOCK 20 Ga, Gross Area 20.0 20.0 40.0 Plate - RHS 20 Ga, Gross Area Total Jt Type Plt Size X Spacing 24.0" Y 3.0x 4.0 Ctr Ctr 0.71 Lumber Duration Factor 1.25 A LOCK 3.0x 4.0 Ctr Ctr 0.44 Plate Duration Factor 1.25 LOCK LOCK 1.0x 3.0 Ctr Ctr 0.75 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10 LOCK 1.0x 3.0 Ctr Ctr 0.75 C LOCK 3.0x 4.0 Ctr Ctr 0.57 5 Wind Load Case(s) Plus

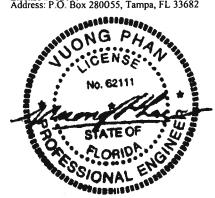
REVIEWED BY:

PO Box 280055

Tampa, FL 33682

Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 817 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----784 C 0.00 0.28 A -E 0.28 29 C 0.00 0.28

-----Bottom Chords-----

1 UBC LL Load Case(s)

React Uplft Size Req'd

140

77

Lbs

569

434

E -B 0.28

C

Lbs In-Sx In-Sx

3 - 8

Hz =

3 - 8

Ηz

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004

Robbins Engineering, Inc.

REFER TO ROBBINS ENG. GENERAL

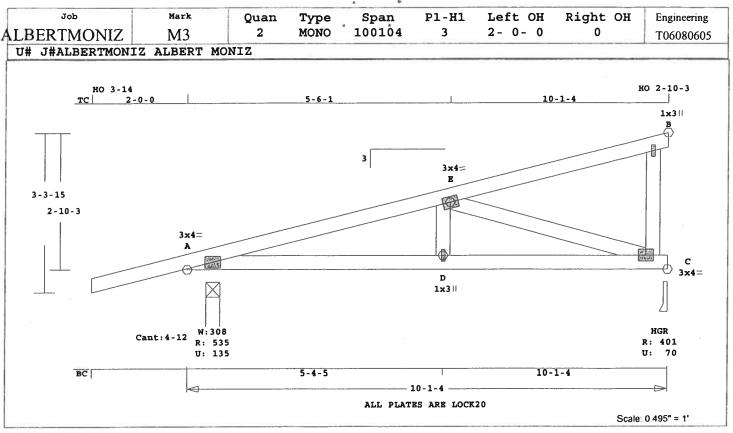
NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

1 - 8

1-8

41



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 57.2 LBS A -D 0.20 751 T 0.12 0.08 OH Loading 751 T D -C 0.19 0.07 0.12 Soffit psf 2.0 Online Plus -- Version 19.0.034 -Webs - -Design checked for 10 psf non-D -E 0.03 207 T RUN DATE: 04-AUG-06 concurrent LL on BC. E-C 0.21 786 C Wind Loads - ANSI / ASCE 7-02 CSI -Size- ----Lumber----C -B 0.01 115 C WindLd Truss is designed as a Main 2x 4 SP-#2 Wind-Force Resistance System. TC 0.21 2x 4 TL Defl -0.04" in A -D L/999 Wind Speed: 110 mph BC 0.20 SP-#2 0.21 2x 4 SP-#2 LL Defl -0.02" in A -D L/999 WB Mean Roof Height: 15-0 Shear // Grain in E -B 0.20 Exposure Category: Brace truss as follows: Occupancy Factor : 1.00 To Plates for each ply each face. 0.C. From Building Type: Enclosed 0- 0- 0 10- 1- 4 PLATING CONFORMS TO TPI. Zone location: Exterior TC Cont. BC Cont. 0- 0- 0 10- 1- 4 REPORT: NER 691 TC Dead Load : 5.0 psf ROBBINS ENGINEERING, INC. BC Dead Load : 5.0 psf BASED ON SP LUMBER 786 Lbs Loading Live Dead (psf) Max comp. force 10.0 USING GROSS AREA TEST. 20.0 TC Quality Control Factor 1.25 0.0 10.0 Plate - LOCK 20 Ga, Gross Area BC Total 20.0 20.0 40.0 Plate - RHS 20 Ga, Gross Area 24.0" Jt Type Plt Size X Spacing Y Lumber Duration Factor 1.25 LOCK 3.0x 4.0 Ctr Ctr 0.69 Plate Duration Factor 1.25 E LOCK 3.0x 4.0 Ctr Ctr 0.42 TC Fb=1.15 Fc=1.10 Ft=1.10 В LOCK 1.0x 3.0 Ctr Ctr 0.75 1.0x 3.0 Ctr Ctr 0.75 BC Fb=1.10 Fc=1.10 Ft=1.10 LOCK D LOCK 3.0x 4.0 Ctr Ctr 0.54 Truss Design Engineer: Vuong Phan Plus 5 Wind Load Case(s) 1 UBC LL Load Case(s) REVIEWED BY:

License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004

Robbins Engineering, Inc.

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

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Tampa, FL 33682

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1-8

1-8

38

87

Plus

Jt

C

React Uplft Size Reg'd

135

71

Membr CSI P Lbs Axl-CSI-Bnd

------Top Chords-----

-----Bottom Chords-----

401

0.21

A -E 0.21

E -B

Lbs In-Sx In-Sx

3 - 8

Hz =

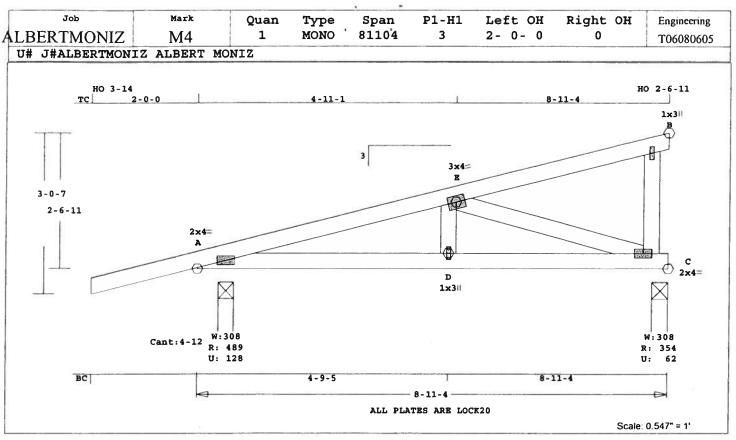
3 - 8

Hz =

769 C 0.00 0.21

28 C 0.00 0.21

Date Sealed: 8/7/2006



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 50.7 LBS A -D 0.17 635 T 0.10 0.07 OH Loading D -C 0.15 635 T 0.10 0.05 Soffit psf 2.0 Online Plus -- Version 19.0.034 -------Webs----Design checked for 10 psf non-RUN DATE: 04-AUG-06 D-E 0.02 178 T concurrent LL on BC. E -C 0.13 667 C Wind Loads - ANSI / ASCE 7-02 CSI -Size- ----Lumber----C -B 0.01 102 C WindLd Truss is designed as a Main SP-#2 0.15 2x 4 Wind-Force Resistance System. 0.17 2x 4 SP-#2 TL Defl -0.03" in A -D L/999 Wind Speed: 110 mph LL Defl -0.01" in A -D WB 0.13 2x 4 SP-#2 L/999 Mean Roof Height: 15-0 Shear // Grain in E -B 0.17 Exposure Category: Brace truss as follows: Occupancy Factor : 1.00 0.C. From To Plates for each ply each face. Building Type: Enclosed TC Cont. 0-0-0 8-11- 4 PLATING CONFORMS TO TPI. Zone location: Exterior BC Cont. 0- 0- 0 REPORT: NER 691 8-11- 4 TC Dead Load : 5.0 psf ROBBINS ENGINEERING, INC. BC Dead Load : 5.0 psf Loading Live Dead BASED ON SP LUMBER (psf) Max comp. force 667 Lbs TC 20.0 10.0 USING GROSS AREA TEST. Quality Control Factor 1.25 BC 0.0 10.0 Plate - LOCK 20 Ga, Gross Area Total 20.0 20.0 40.0 Plate - RHS 20 Ga, Gross Area Spacing 24.0" Jt Type Plt Size X Y Lumber Duration Factor 1.25 A LOCK 2.0x 4.0 Ctr Ctr 0.89 Plate Duration Factor 1.25 R LOCK 3.0x 4.0 Ctr Ctr 0.40 TC Fb=1.15 Fc=1.10 Ft=1.10 В LOCK 1.0x 3.0 Ctr Ctr 0.75 BC Fb=1.10 Fc=1.10 Ft=1.10 D LOCK 1.0x 3.0 Ctr Ctr 0.75

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

 Jt
 React Uplft
 Size
 Req'd

 Lbs
 Lbs
 In-Sx
 In-Sx

 A
 489
 128
 3-8
 1-8

 Hz
 =
 34

 C
 354
 63
 3-8
 1-8

 Hz
 =
 77

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----

A -E 0.15 650 C 0.00 0.15 E -B 0.15 24 C 0.00 0.15 -----Bottom Chords----- REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

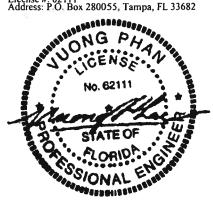
2.0x 4.0 Ctr Ctr 0.78

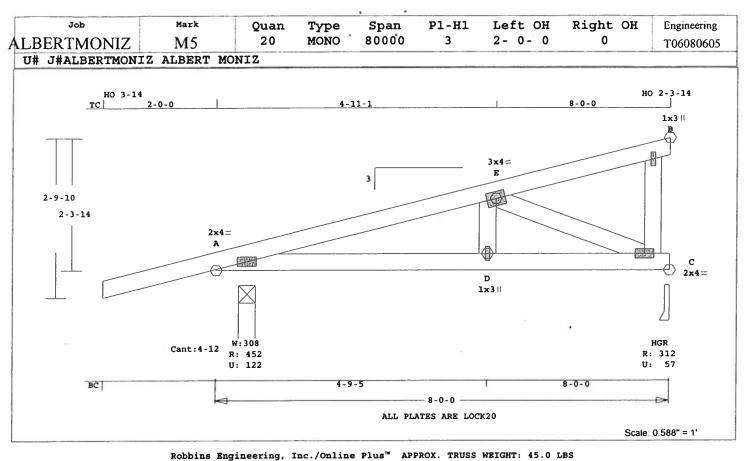
LOCK

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:

Mayo Truss Co. Inc. Analysis Conforms To: FBC2004



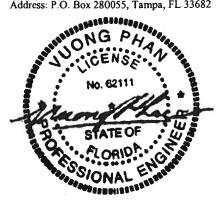


-----Bottom Chords-----A -D 0.16 497 T 0.08 0.08 Online Plus -- Version 19.0.034 D -C 0.12 497 T 0.05 0.07 RUN DATE: 04-AUG-06 -Webs-----D-E 0.02 161 T 0.07 E -C 538 C CSI -Size- ----Lumber----C -B 0.00 72 C WindLd TC 0.13 2x 4 SP-#2 BC 0.16 2x 4 SP-#2 TL Defl -0.02" in A -D L/999 2x 4 SP-#2 0.07 LL Defl -0.01" in A -D L/999 Brace truss as follows: Shear // Grain in A -E o.c. From To 0-0-0 8- 0- 0 Plates for each ply each face. TC Cont. 0- 0- 0 8- 0- 0 PLATING CONFORMS TO TPI. BC Cont. REPORT: NER 691 ROBBINS ENGINEERING, INC. Loading Live Dead (psf) BASED ON SP LUMBER TC 20.0 10.0 BC 0.0 10.0 USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area 40.0 Total 20.0 20.0 24.0" Plate - RHS 20 Ga, Gross Area Spacing Jt Type Plt Size X Lumber Duration Factor 1.25 Plate Duration Factor 1.25 A LOCK 2.0x 4.0 Ctr Ctr 0.87 TC Fb=1.15 Fc=1.10 Ft=1.10 E LOCK 3.0x 4.0 Ctr Ctr 0.39 B LOCK 1.0x 3.0 Ctr Ctr 0.75 BC Fb=1.10 Fc=1.10 Ft=1.10 D LOCK 1.0x 3.0 Ctr Ctr 0.75 C LOCK 2.0x 4.0 Ctr Ctr 0.76 Plus 5 Wind Load Case(s) Plus 1 UBC LL Load Case(s) REVIEWED BY: Jt React Uplft Size Req'd Robbins Engineering, Inc. PO Box 280055 Lbs In-Sx In-Sx Lbs

FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 538 Lbs Quality Control Factor 1.25

Analysis Conforms To:

Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



Membr CSI P Lbs Axl-CSI-Bnd

58

453

313

A

C

3 - 8

Hz =

3 - 8

Hz =

A -E 0.13 508 C 0.00 0.13 E -B 0.13 20 C 0.00 0.13 ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:

Mayo Truss Co. Inc.

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR

Tampa, FL 33682

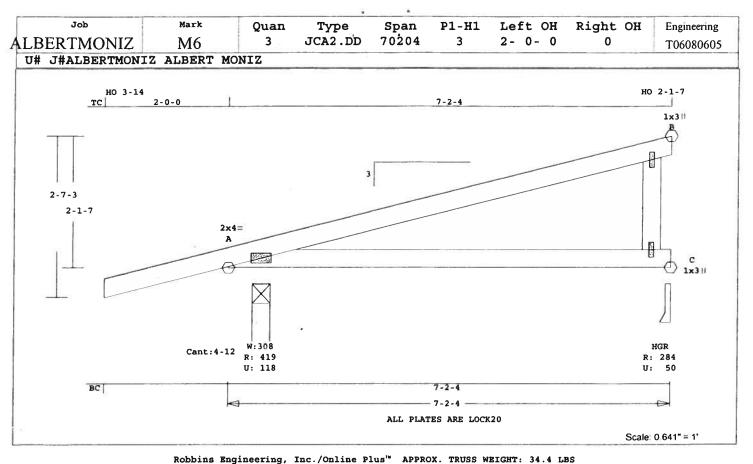
Date Sealed: 8/7/2006

1-8

1-8

31

68



A -C 0.33 47 T 0.00 0.33 Wind Loads - ANSI / ASCE 7-02 Online Plus -- Version 19.0.034 C -B 0.01 182 C WindLd RUN DATE: 04-AUG-06 TL Defl -0.16" in A -C L/479 LL Defl -0.06" in A -C L/999 CSI -Size- ----Lumber----Shear // Grain in A -B TC 0.45 2x 4 SP-#2 0.22 0.33 2x 4 SP-#2 BC 0.01 2x 4 SP-#2 Plates for each ply each face. WB PLATING CONFORMS TO TPI. Brace truss as follows: REPORT: NER 691 o.c. From To ROBBINS ENGINEERING, INC. TC Cont. 0- 0- 0 7-2-4 BASED ON SP LUMBER 0- 0- 0 7-2-4 USING GROSS AREA TEST. BC Cont. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Loading Live Dead (psf) 10.0 Jt Type Plt Size X Y JSI TC 20.0 BC 0.0 10.0 A LOCK 2.0x 4.0 Ctr Ctr 0.85 B LOCK 1.0x 3.0 Ctr Ctr 0.75 Total 20.0 20.0 40.0 Spacing 24.0" C LOCK 1.0x 3.0 Ctr Ctr 0.75

Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 182 Lbs Quality Control Factor 1.25

REVIEWED BY: Robbins Engineering, Inc.

PO Box 280055 Tampa, FL 33682

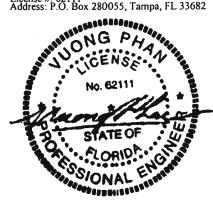
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading

Soffit psf 2.0 26 C 0.00 0.45 Design checked for 10 psf nonconcurrent LL on BC.

Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



28

61

1-8

Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)

Lbs

420

284

A -B 0.45

C

Plus 1 UBC LL Load Case(s)

React Uplft Size Req'd

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

-----Bottom Chords-----

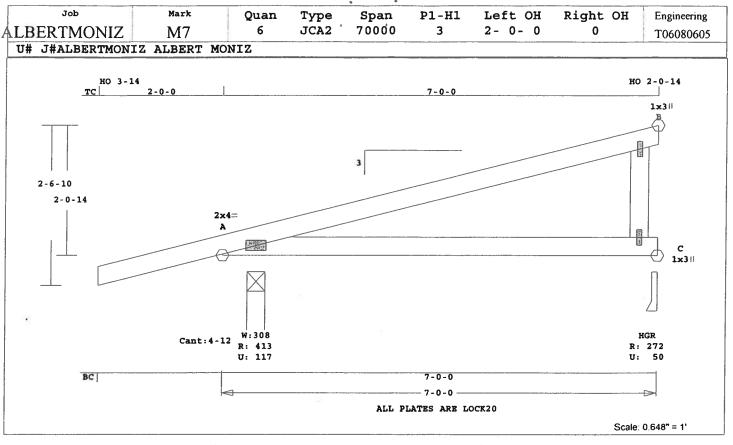
Lbs In-Sx In-Sx

Hz =

3 - 8

Hz =

3-81-8



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.6 LBS A -C 0.32 46 T 0.00 0.32 Wind Loads - ANSI / ASCE 7-02 -Webs-----Online Plus -- Version 19.0.034 C -B 0.01 178 C WindLd RUN DATE: 04-AUG-06 TL Defl -0.14" in A -C L/515 CSI -Size- ---- LL Defl -0.06" in A -C L/999 Shear // Grain in A -B 0.43 2x 4 SP-#2 TC 0.21 0.32 2x 4 SP-#2 BC WB 0.01 2x 4 SP-#2 Plates for each ply each face. PLATING CONFORMS TO TPI. Brace truss as follows: REPORT: NER 691 ROBBINS ENGINEERING, INC. From To

> BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI A LOCK 2.0x 4.0 Ctr Ctr 0.85 1.0x 3.0 Ctr Ctr 0.75 B LOCK C LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

React Uplft Size Req'd Lbs In-Sx In-Sx 413 117 3-8 1-8 Hz =27 51 3-8 273 1-8 Hz =59

0-0-0 7-0-0

0-0-0 7-0-0

(psf)

40.0

24.0"

1.25

Dead

10.0

10.0

20.0

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----25 C 0.00 0.43 A -B 0.43 -----Bottom Chords-----

TC Cont.

BC Cont.

Live

20.0

0.0

20.0

Lumber Duration Factor

Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s) Plus 1 UBC LL Load Case(s)

Loading

TC

BC

C

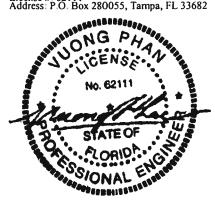
Total

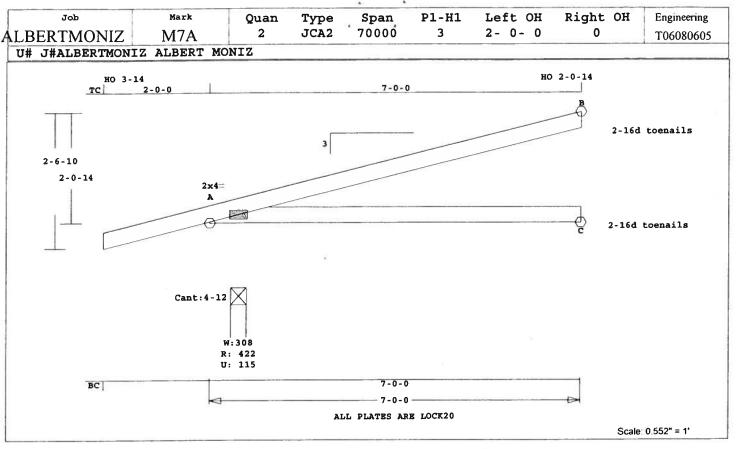
Spacing

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004

OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC.

Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 178 Lbs Quality Control Factor 1.25





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 30.9 LBS A -C 0.34 O T 0.00 0.34

Online Plus -- Version 19.0.034 TL Defl -0.16" in A -C L/459 LL Defl -0.07" in A -C L/999 Shear // Grain in A -B 0.22

CSI -Size- ----Lumber----2x 4 SP-#2 0.46 0.34 2x 4 SP-#2

RUN DATE: 04-AUG-06

Brace truss as follows: o.c. From To 7- 0- 0 0 - 0 - 0TC Cont. 0- 0- 0 7-0-0 BC Cont.

Loading Live Dead (psf) 10.0 TC 20.0 0.0 10.0 BC 20.0 20.0 40.0 Total 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

5 Wind Load Case(s) 1 UBC LL Load Case(s) Plus

React Uplft Size Req'd Jt Lbs In-Sx In-Sx Lbs 115 3-8 1-8 422 56 Hz =0 1-81-8 C 121 В 176 64 1-8 1-8 Hz =

Membr CSI P Lbs Ax1-CSI-Bnd -----Top Chords-----22 C 0.00 0.46 A -B 0.46 -----Bottom Chords-----

Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Max comp. force Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.85

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

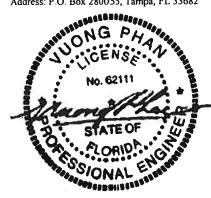
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

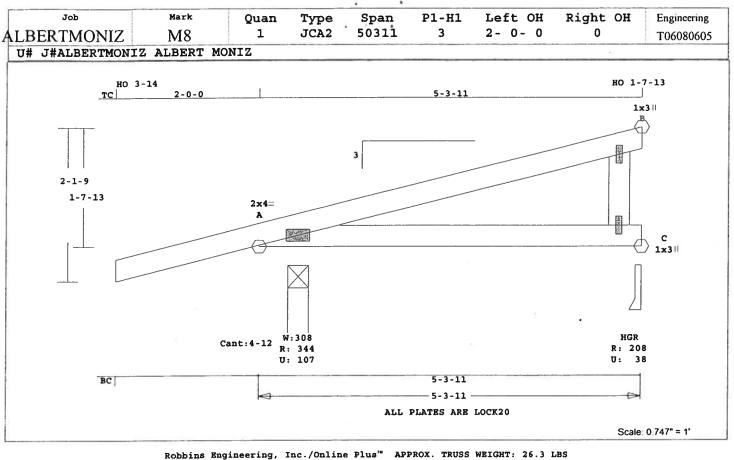
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf non-

concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf 22 Lbs

Quality Control Factor 1.25





Online Plus -- Version 19.0.034 C -B 0.01 RUN DATE: 04-AUG-06 CSI -Size- ----Lumber----TC 0.22 2x 4 SP-#2 0.17 2x 4 SP-#2 BC WB 0.01 2x 4 SP-#2 Plates for each ply each face. Brace truss as follows: O.C. From To TC Cont. 0- 0- 0 5- 3-11 BC Cont. 0- 0- 0 Loading Live Dead (psf) 20.0 10.0 TC BC 0.0 10.0 20.0 20.0 40.0 Total 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s) Plus 1 UBC LL Load Case(s)

BC Fb=1.10 Fc=1.10 Ft=1.10

React Uplft Size Req'd Lbs Lbs In-Sx In-Sx 3-8 1-8 A 345 107 Hz =21 1-8 C 209 38 3 - 8 43 Hz =

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----18 C 0.00 0.22 A -B 0.22 -----Bottom Chords-----

TL Defl -0.04" in A -C L/999 LL Defl -0.02" in A -C L/999 Shear // Grain in A -B 0.15

130 C WindLd

PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. 5- 3-11 BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI A LOCK 2.0x 4.0 Ctr Ctr 0.80 B LOCK 1.0x 3.0 Ctr Ctr 0.75 C LOCK 1.0x 3.0 Ctr Ctr 0.75

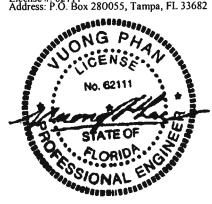
> REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

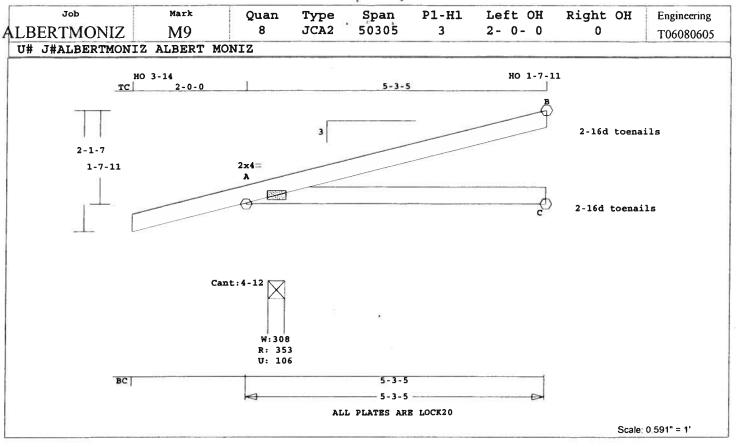
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004

OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC.

A -C 0.17 33 T 0.00 0.17 Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 130 Lbs Quality Control Factor 1.25





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.3 LBS A -C 0.19 0 T 0.00 0.19

Online Plus -- Version 19.0.034 TL Defl -0.05" in A -C L/999LL Defl -0.02" in A -C L/999 RUN DATE: 04-AUG-06

CSI -Size- ----Lumber----

TC 0.24 2x 4 SP-#2 0.19 2x 4 SP-#2

Brace truss as follows: To o.c. From 5- 3- 5 TC Cont. 0- 0- 0 0- 0- 0 5- 3- 5 BC Cont.

Live (psf) Loading Dead 20.0 10.0 TC BC 0.0 10.0 20.0 20.0 40.0 Total Spacing 24.0" Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s) 1 UBC LL Load Case(s)

React Uplft Size Req'd Jt Lbs In-Sx In-Sx 354 106 3-8 1-8 Hz =42 1-8 C 89 0 1-8 1-8 1- 8 В 46 128 Hz =27

Membr CSI P Lbs Axl-CSI-Bnd ------Top Chords-----A -B 0.24 17 T 0.00 0.24 -----Bottom Chords----- Shear // Grain in A -B 0.16 Plates for each ply each face. PLATING CONFORMS TO TPI.

REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Max comp. force Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI A LOCK 2.0x 4.0 Ctr Ctr 0.81

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading

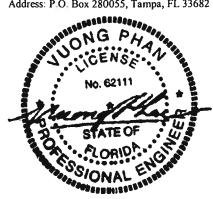
Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC.

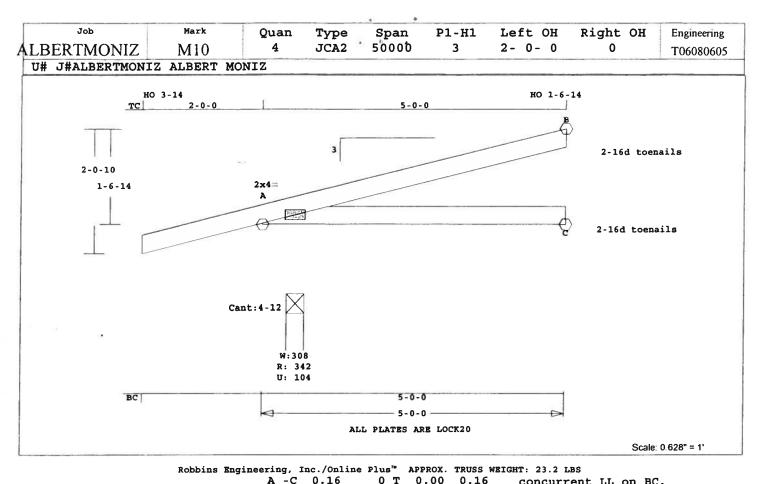
Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph

Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed

Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf

16 Lbs Quality Control Factor 1.25





A -C 0.16 0 T 0.00 0.16 Win Online Plus -- Version 19.0.034 TL Defl -0.04" in A -C L/999 Tru

LL Defl -0.02" in A -C L/999 Shear // Grain in A -B 0.15

CSI -Size- ----Lumber---TC 0.21 2x 4 SP-#2
BC 0.16 2x 4 SP-#2

RUN DATE: 04-AUG-06

Dead (psf) Loading Live TC 20.0 10.0 0.0 10.0 BC Total 20.0 20.0 40.0 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

React Uplft Size Req'd Jt Lbs Lbs In-Sx In-Sx 105 3-8 1-8 343 Hz =40 0 1-8 1-8 C 83 В 121 44 1-8 1-8 Hz =

Membr CSI P Lbs Axl-CSI-Bnd
-----Top Chords----A -B 0.21 16 T 0.00 0.21
-----Bottom Chords-----

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORT: NER 691

ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER

USING GROSS AREA TEST.

Plate - LOCK 20 Ga, Gross Area

Plate - RHS 20 Ga, Gross Area

Jt Type Plt Size X Y JSI

A LOCK 2.0x 4.0 Ctr Ctr 0.80

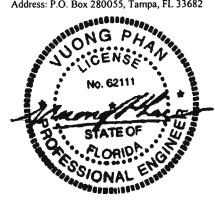
REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

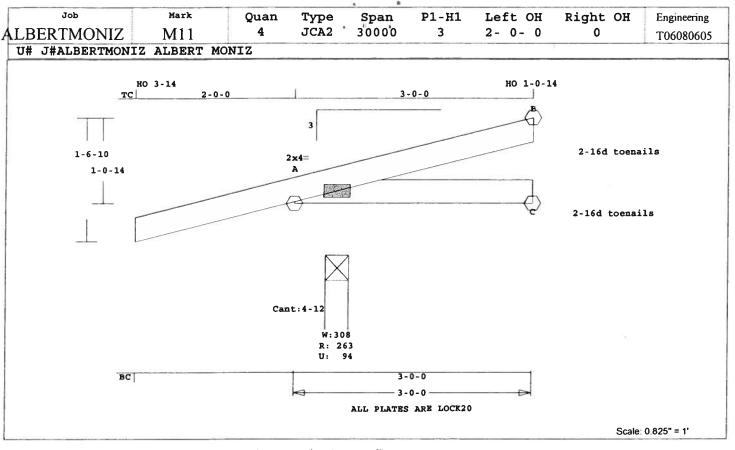
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
 Mayo Truss Co. Inc.
Analysis Conforms To:
 FBC2004
OH Loading
 Soffit psf 2.0
Design checked for 10 psf non-

concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf 15 Lbs Quality Control Factor 1.25





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.5 LBS
A -C 0.05 0 T 0.00 0.05 concurrent LL on BC.

Online Plus -- Version 19.0.034 TL Defl 0.00" in A -C L/999
RUN DATE: 04-AUG-06 LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.08

CSI -Size- ----Lumber---TC 0.06 2x 4 SP-#2
BC 0.05 2x 4 SP-#2

Brace truss as follows:
 O.C. From To
TC Cont. 0-0-0 3-0-0
BC Cont. 0-0-0 3-0-0

Loading Live Dead (psf) TC 20.0 10.0 0.0 10.0 BC 20.0 20.0 40.0 Total Spacing 24.0" Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd Lbs Lbs In-Sx In-Sx 3-81-8 264 94 Hz = 23 1-8 C 45 0 1-8 В 65 23 1-8 1-8 Hz =

 Membr
 CSI
 P Lbs
 Ax1-CSI-Bnd

 -----Top
 Chords----- 

 A -B
 0.06
 10 T
 0.00
 0.06

 -----Bottom
 Chords-----

Shear // Grain in A -B 0.08

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A LOCK 2.0x 4.0 Ctr Ctr 0.79

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

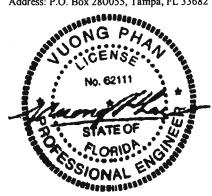
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
 Mayo Truss Co. Inc.
Analysis Conforms To:
 FBC2004
OH Loading
 Soffit psf 2.0
Design checked for 10 psf non-

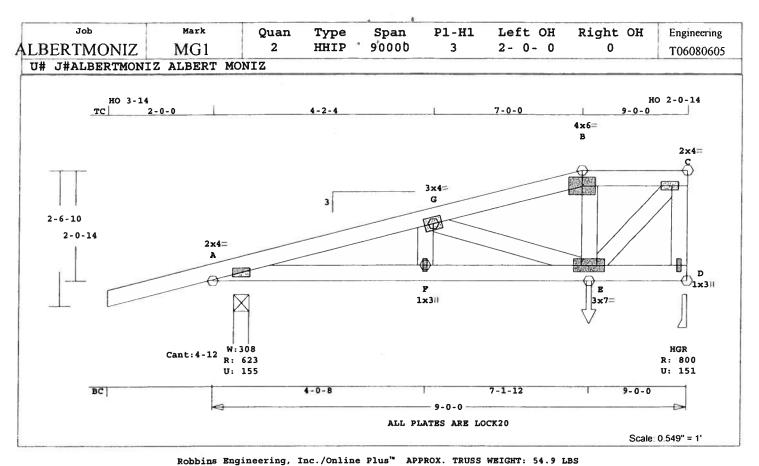
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main
Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior

Zone location: Exterior
TC Dead Load: 5.0 psf
BC Dead Load: 5.0 psf
Max comp. force 7 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006



Membr CSI P Lbs Axl-CSI-Bnd NOTES: ------Top Chords-----1143 C 0.01 0.09 Online Plus -- Version 19.0.034 A -G 0.10 RUN DATE: 04-AUG-06 G -B 0.09 767 C 0.00 0.09 B -C 0.06 744 C 0.06 0.00 CSI -Size- ----Lumber----Bottom Chords-----A-F 0.26 1107 T 0.20 0.06 TC 0.10 2x 4 SP-#2 1107 T BC 2x 4 SP-#2 F-E 0.26 0.20 0.06 0.26 49 T 0.19 2x 4SP-#2 E -D 0.03 0.00 0.03 WB -Webs - -Brace truss as follows: F-G 0.01 114 T To G-E 0.05 382 C O.C. From TC Cont. 0-0-0 9-0-0 61 T E -B 0.00 0- 0- 0 9-0-0 E -C 1076 T BC Cont. 0.19 0.07 D -C 793 C WindLd Loading Live (psf) Dead TL Defl -0.04" in F -E LL Defl -0.02" in F -E 20.0 10.0 L/999 TC L/999 BC 0.0 10.0 Shear // Grain in A -G 20.0 40.0 Total 20.0 0.12 Spacing 24.0" Lumber Duration Factor 1.25 Plates for each ply each face. PLATING CONFORMS TO TPI. Plate Duration Factor 1.25 REPORT: NER 691 TC Fb=1.00 Fc=1.00 Ft=1.00 ROBBINS ENGINEERING, INC. BC Fb=1.00 Fc=1.00 Ft=1.00 BASED ON SP LUMBER USING GROSS AREA TEST. Load Case # 1 Girder Loading 1.25 Lumber Duration Factor Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Plate Duration Factor 1.25 Plt Size X JSI plf -Live Dead From To Jt Type Y TC V 20 0.0 9.0' LOCK 2.0x 4.0 Ctr Ctr 0.90 40 BC V 0.0 9.01 3.0x 4.0 Ctr Ctr 0.40 0 20 LOCK TC V 50 25 7.01 8.0' В LOCK 4.0x 6.0 Ctr Ctr 0.65 2.0x 4.0-0.5 Ctr 0.82 TC V -40 -20 8.01 9.0' C LOCK BC V 25 7.1' 8.0 F LOCK 1.0x 3.0 Ctr Ctr 0.75 0 BC V 0 -20 8.0' 9.0' E LOCK 3.0x 7.0 Ctr Ctr 0.78 BC V 280 280 7.1' CL-LB LOCK 1.0x 3.0 Ctr Ctr 0.75

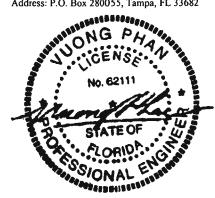
Plus 6 Wind Load Case(s) Plus 1 UBC LL Load Case(s)

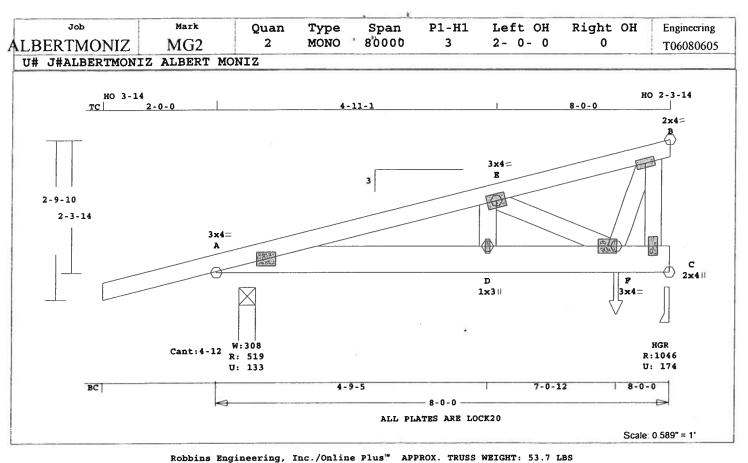
React Uplft Size Req'd Jt Lbs In-Sx In-Sx Lbs 1-8 3 - 8 624 155 Hz =28 3 - 8 801 151 1-8 Hz =62

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 Girder Half Hip Framing King Jacks Open Faced Jack Setback 7- 0- 0 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 1143 Lbs Quality Control Factor 1.25





A -E 0.10 765 C 0.00 0.10 E -B 0.10 297 C 0.00 0.10 ---Bottom Chords----Online Plus -- Version 19.0.034 -----0.19 746 T 0.09 0.10 RUN DATE: 04-AUG-06 A -D D-F 0.17 746 T 0.09 0.08 51 T CSI -Size- ----Lumber----F -C 0.08 0.00 0.08 0.10 2x 4 SP-#2 Webs - -TC SP-#2 D-E 0.02 176 T BC 0.19 2x 6 SP-#2 E-F 0.05 514 C WB 0.17 2x 4 F-B 920 T 0.17 Brace truss as follows: C -B 0.08 883 C WindLd From To 0.C. -0.02" in A -D L/999 8- 0- 0 TC Cont. 0 - 0 - 0TL Defl 8- 0- 0 LL Defl -0.01" in A -D BC Cont. 0-0-0 L/999 Shear // Grain in A -E 0.13 Loading Live Dead (psf) 20.0 10.0 Plates for each ply each face. TC BC 0.0 10.0 PLATING CONFORMS TO TPI. 20.0 20.0 40.0 REPORT: NER 691 Total ROBBINS ENGINEERING, INC. Spacing 24.0" Lumber Duration Factor BASED ON SP LUMBER 1.25 USING GROSS AREA TEST. Plate Duration Factor 1.25 TC Fb=1.00 Fc=1.00 Ft=1.00 Plate - LOCK 20 Ga, Gross Area BC Fb=1.00 Fc=1.00 Ft=1.00 Plate - RHS 20 Ga, Gross Area Plt Size X Jt Type Y JSI Load Case # 1 Standard Loading Α LOCK 3.0x 4.0 Ctr Ctr 0.91 3.0x 4.0 Ctr Ctr 0.39 Lumber Duration Factor 1.25 E LOCK Plate Duration Factor 1.25 LOCK 2.0x 4.0 Ctr Ctr 0.98 To plf - Live Dead From D LOCK 1.0x 3.0 Ctr Ctr 0.75 TC V 40 20 0.0 8.01 F LOCK 3.0x 4.0 Ctr Ctr 0.75 BC V 0.0' 8.0' LOCK 2.0x 4.0 Ctr Ctr 0.42 20 0 BC V 400 400 7.1' CL-LB

Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 883 Lbs Quality Control Factor 1.25

Design checked for 10 psf non-

Wind Loads - ANSI / ASCE 7-02

Truss is designed as a Main

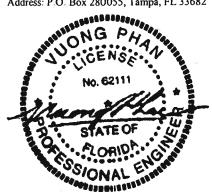
concurrent LL on BC.

FBC2004

Soffit psf 2.0

OH Loading

Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



REVIEWED BY:

PO Box 280055 Tampa, FL 33682

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To:

Robbins Engineering, Inc.

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Membr CSI P Lbs Axl-CSI-Bnd ------Top Chords------

5 Wind Load Case(s)

React Uplft Size Req'd

134

1 UBC LL Load Case(s)

Lbs In-Sx In-Sx

3 - 8

Hz =

3-8

Hz =

Plus

Lbs 519

1046

Jt

C

Date Sealed: 8/7/2006

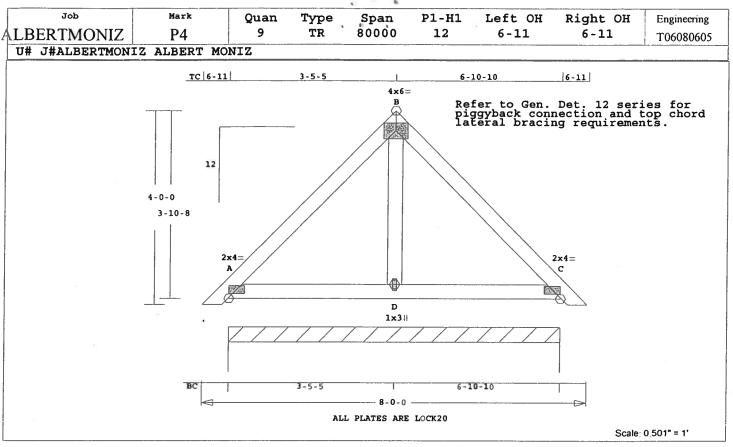
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1- 8

1-8

31

66



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.4 LBS D -C 0.07 0 T 0.00 0.07 Online Plus -- Version 19.0.034 D -B 0.00 59 C RUN DATE: 04-AUG-06 0.00" in D -C L/999 TL Defl LL Defl 0.00" in D -C L/999 CSI -Size- ----Lumber----TC 0.07 2x 4 SP-#2 Shear // Grain in A -B 0.08 BC 0.07 2x 4 SP-#2 Plates for each ply each face. 0.00 2x 4SP-#2 PLATING CONFORMS TO TPI.

Brace truss as follows: REPORT: NER 691 ROBBINS ENGINEERING, INC. O.C. From To 8- 0- 0 BASED ON SP LUMBER TC Cont. 0- 0- 0 8- 0- 0 0- 0- 0 USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Max comp. force Plate - RHS 20 Ga, Gross Area Live Dead (psf) 20.0 10.0 Jt Type Plt Size X Y JSI 0.0 10.0 A LOCK 2.0x 4.0 0.4 0.3 0.62 LOCK 4.0x 6.0 Ctr-0.1 0.35 20.0 20.0 40.0 C LOCK 2.0x 4.0-0.4 0.3 0.62 24.0"

BC Cont. Loading TC BC Total Spacing Lumber Duration Factor 1.25 D LOCK 1.0x 3.0 Ctr Ctr 0.75 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10

Robbins Engineering, Inc. PO Box 280055 Plus 4 Wind Load Case(s) Tampa, FL 33682 Plus 1 UBC LL Load Case(s)

> REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

Cont. Brg 0- 0- 0 to 6-10-10 86 Hz = 622 76 Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----

Lbs In-Sx In-Sx

BC Fb=1.10 Fc=1.10 Ft=1.10

Jt React Uplft Size Req'd

Lbs

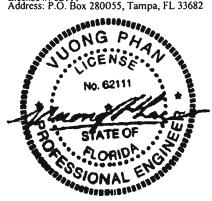
A -B 0.07 151 C 0.00 0.07 B -C 0.07 151 C 0.00 0.07 -----Bottom Chords-----O T 0.00 0.07 NOTES:

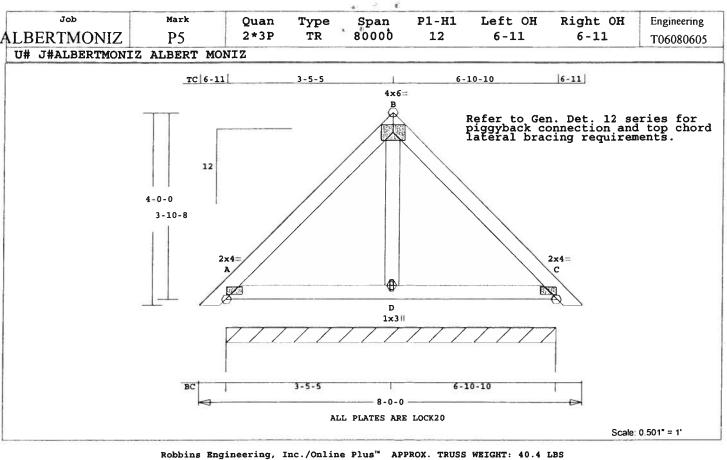
REVIEWED BY:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonRefer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior

concurrent LL on BC.

TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf 151 Lbs Quality Control Factor 1.25





CSI -Size- ----Lumber---TC 0.07 2x 4 SP-#2
BC 0.07 2x 4 SP-#2
WB 0.00 2x 4 SP-#2

Brace truss as follows:

To o.c. From 2- 0- 0 0-0-0 8- 0- 0 2-0-0 0-0-0 8- 0- 0 Loading Live Dead (psf) 20.0 TC 10.0 BC 0.0 10.0 40.0 Total 20.0 20.0 Spacing 72.0" Lumber Duration Factor 1.25 Plate Duration Factor 1.25

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Jt React Uplft Size Req'd
 Lbs Lbs In-Sx In-Sx
Cont. Brg 0- 0- 0 to 6-10-10
 1867 258 Hz = 229

TL Defl 0.00" in D -C L/999 LL Defl 0.00" in D -C L/999 Shear // Grain in A -B 0.08

178 C

D -B 0.00

Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI A LOCK 2.0x 4.0 0.4 0.3 0.62 В LOCK 4.0x 6.0 Ctr-0.1 0.35 2.0x 4.0-0.4 0.3 0.62 C LOCK LOCK 1.0x 3.0 Ctr Ctr 0.75

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

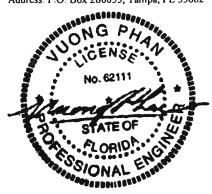
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

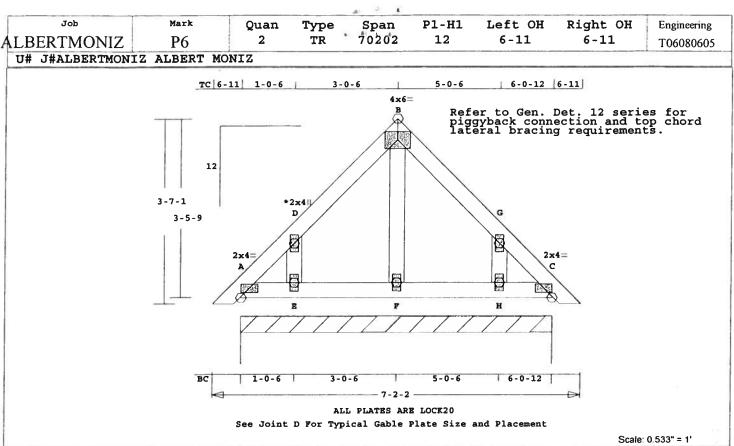
NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 3 COMPLETE TRUSSES REQUIRED. Fasten together in staggered pattern. (1/2" bolts -OR-SDS4.5 screws -OR- 16d nails as each layer is applied.) ---- Spacing (In) ---Nails Screws Bolts Rows TC 1 12 24 0 0 BC 1 12 24 WB 1 8

No bolts in 2x4s or smaller. OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Refer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : BC Dead Load : 5.0 psf Max comp. force 455 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #. 62111 Address: P.O. Box 280055, Tampa, FL 33682



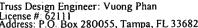


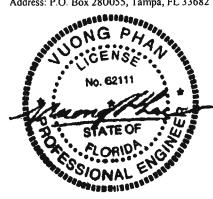
Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 39.3 LBS ------Gable Webs----concurrent LL on BC. E -D 0.01 99 C Refer to Gen Det 3 series for Online Plus -- Version 19.0.034 F -B 0.00 61 C web bracing and plating. RUN DATE: 04-AUG-06 H-G 0.01 99 C Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main 0.00" in F -H L/999 CSI -Size- ----Lumber----TL Defl Wind-Force Resistance System. 0.00" in F -H L/999 0.02 2x 4 SP-#2 LL Defl Wind Speed: 110 mph Shear // Grain in D -B 0.05 BC 0.02 2x 4 SP-#2 Mean Roof Height: 15-0 0.01 2x 4 SP-#2 Exposure Category: Plates for each ply each face. Occupancy Factor : 1.00 Building Type: Enclosed Brace truss as follows: PLATING CONFORMS TO TPI. O.C. From To REPORT: NER 691 Zone location: Exterior ROBBINS ENGINEERING, INC. 7- 2- 2 5.0 psf TC Cont. 0-0-0 TC Dead Load : BC Cont. 0- 0- 0 7- 2- 2 BASED ON SP LUMBER BC Dead Load : 5.0 psf USING GROSS AREA TEST. Max comp. force 99 Lbs Plate - LOCK 20 Ga, Gross Area Live Dead (psf) Quality Control Factor 1.25 Loading Plate - RHS 20 Ga, Gross Area TC 20.0 10.0 ВÇ 0.0 10.0 Jt Type Plt Size X Y A LOCK D LOCK 20.0 40.0 2.0x 4.0 0.4 0.3 0.60 Total 20.0 2.0x 4.0 Ctr Ctr 0.00 Spacing 24.0" 1.25 Lumber Duration Factor LOCK 4.0x 6.0 Ctr-0.1 0.34 LOCK 2.0x 4.0 Ctr Ctr 0.00 Plate Duration Factor 1.25 G LOCK 2.0x 4.0-0.4 0.3 0.60 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10 2.0x 4.0 Ctr Ctr 0.00 E LOCK LOCK 2.0x 4.0 Ctr Ctr 0.00 LOCK 2.0x 4.0 Ctr Ctr 0.00 Plus 4 Wind Load Case(s) Plus 1 UBC LL Load Case(s) Truss Design Engineer: Vuong Phan REVIEWED BY: License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682 React Uplft Size Req'd Robbins Engineering, Inc. Lbs In-Sx In-Sx

PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf non-





Date Sealed: 8/7/2006

67

0.02

0.02

0.02

0.01

0.02

0.02

0.01

0- 0- 0 to 6- 0-12

60 C 0.00 0.02

0.00

0.00

0.00

0.00

0.00

0.00

0.00

77 Hz =

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

43 C

43 C

60 C

1 T

0 T

0 T

1 T

-----Bottom Chords----

Cont. Brg

A -D

D-B

B-G

G -C

A-E

E-F

H -C

-H

F

556

0.02

0.02

0.02

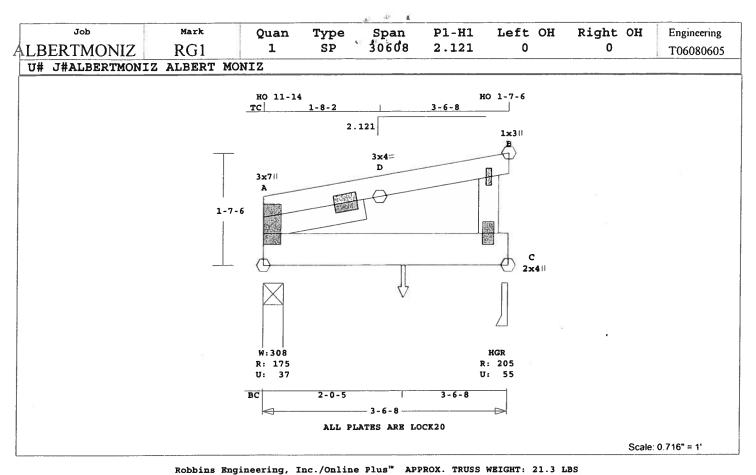
0.02

0.01

0.02

0.02

0.01



Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----Online Plus -- Version 19.0.034 A -D 0.06 43 C 0.00 0.06 RUN DATE: 04-AUG-06 D-B 0.09 12 C 0.00 0.09 -----Bottom Chords-----20 T 0.00 0.11 CSI -Size- ----Lumber----A -C 0.11 Webs-----TC 2x 4 SP-#2 0.09 C -B 0.00 85 C WindLd BC 0.11 2x 6 SP-#2 WB 0.00 2x 4 SP-#2 -----Sliders-----A -D 0.00 SL 0.00 2x 4SP-#2 66 C Brace truss as follows: TL Defl 0.00" in A -C L/999 0.00" in A -C To LL Defl L/999 0.C. From Shear // Grain in D -B TC Cont. 0-0-0 3-6-8 0.09 0- 0- 0 BC Cont. Plates for each ply each face. PLATING CONFORMS TO TPI. Loading Live Dead (psf) TC 20.0 10.0 REPORT: NER 691 ROBBINS ENGINEERING, INC. 10.0 BC 0.0 40.0 BASED ON SP LUMBER Total 20.0 20.0 24.0" USING GROSS AREA TEST. Spacing Lumber Duration Factor Plate - LOCK 20 Ga, Gross Area 1.25 Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI 1.25 Plate Duration Factor TC Fb=1.00 Fc=1.00 Ft=1.00 3.0x 7.0 1.5 0.1 0.93 BC Fb=1.00 Fc=1.00 Ft=1.00 LOCK LOCK 3.0x 4.0 Ctr Ctr 0.50 D Load Case # 1 Standard Loading В LOCK 1.0x 3.0 Ctr Ctr 0.75 Lumber Duration Factor LOCK 2.0x 4.0 Ctr Ctr 0.38 1.25 Plate Duration Factor 1.25 plf - Live Dead From To 3.5 TC V 40 20 0.0 REVIEWED BY:

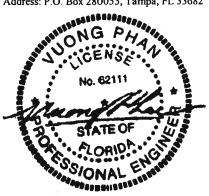
Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading

Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf 5.0 psf BC Dead Load : Max comp. force 85 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Vuong Phan License #: 62111 Address: P.O. Box 280055, Tampa, FL 33682



30

1-8

-13

1-8

BC V

BC V

Plus

C

0

50

Lbs

175

206

20

50

5 Wind Load Case(s)

1 UBC LL Load Case(s)

React Uplft Size Req'd

38

56

0.01

2.0'

Lbs In-Sx In-Sx 3 - 8

Hz =

3 - 8

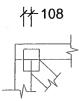
Hz =

3.5'

CL-LB

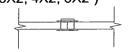
# ROBBINS ENG. GENERAL NOTES & SYMBOLS

### PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

### FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)



(W) = Wide Face Plate (N) = Narrow Face Plate

### LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.



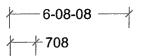
### PLATE SIZE AND ORIENTATION



The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

### **DIMENSIONS**

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



W = Actual Bearing Width (IN-SX) R = Reaction (lbs.)

U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with "National Design Specifications for Wood Construction" (AF & PA)," National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

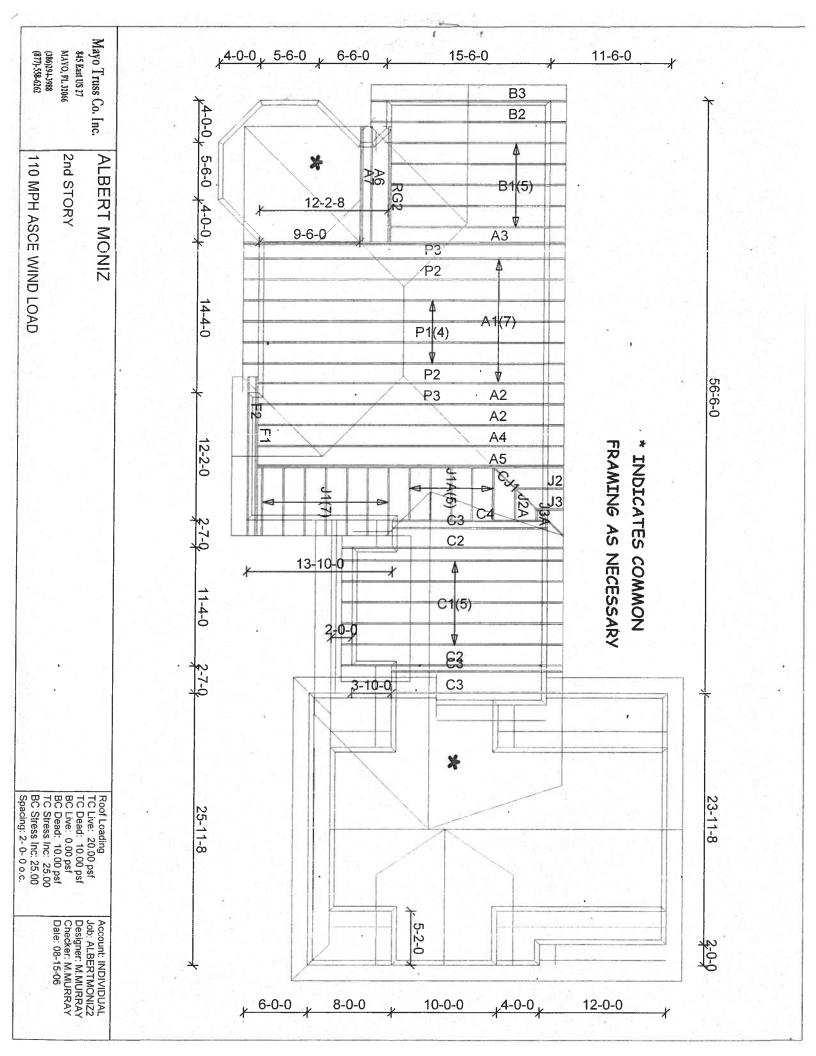
Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.



6904 Parke East Blvd. Tampa, FI 33610-4115 Tel: 813-972-1135 Fax: 813-971-6117

www.robbinseng.com



Permit Number:	Lot Number:	
Miscellaneous:	Address:	
The information in this box is for admi	nistrative purposes only and is not part of the engineering	ng review.

Truss Fabricator: Mayo Truss Company, Inc.

Job Reference: ALBERTMONIZ2 - ALBERT MONEZ - 2nd STORY

ROBBINS	
ENGINEERING,	INC.

Job Number

T06080606

P.O. Box 280055 Tampa, FL 33682-0055 Phone: (813) 972-1135

# **Engineering Index Sheet**

Index Page 1 of 1

Date 08/07/2006 FBC - 2004 Chapter 16 and 23

Specification Quantity

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specification Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Sofware - Online Ptus)

Notes: Refer to individual truss design drawings for special loading conditions.

Index Page 1 of 1

Standard Loading: T.C. Live 20 psf T.C Dead 10 psf B.C Live 0 psf B.C. Dead 10 psf

ANSVASCE 7-02 Wind Speed - 110 MPH Mean Roof Ht. - 15 FT Exposure Catergory - B Occupancy Factor - 1.00 MWFRS

Total

**Enclosed** 

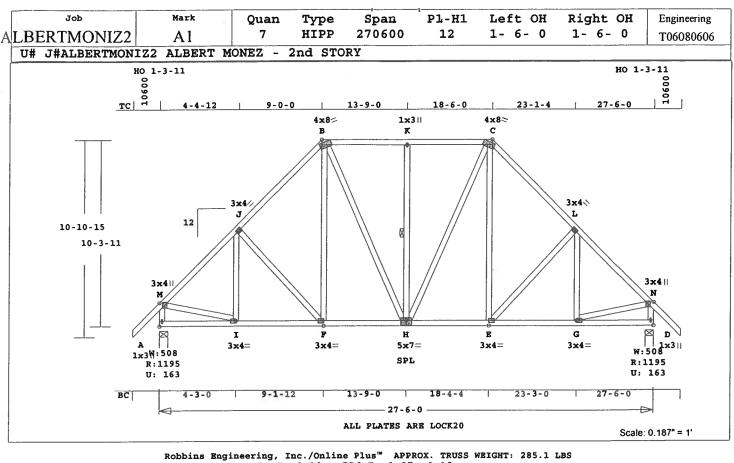
20 psf 10 psf 0 psf 10 psf 10 psf 40 psf

	Date Mark	Date Mark	Date Mark	Date Mark
1_	08/07/06 A1	2 08/07/06 A2	3   08/07/06   A3	4 08/07/06 A4
5	08/07/06 A5	6 08/07/06 A6	7 08/07/06 A7	8 [08/07/06] B1
9	08/07/06 B2	10 08/07/06 B3	11   08/07/06   C1	12 08/07/06 C2
13	08/07/06 C3	14   08/07/06   C4	15  08/07/06  CJ1	16 08/07/06 F1
17	08/07/06 F2	18 08/07/06 J1	19   08/07/06   J1A	20 [08/07/06] J2
21	08/07/06 J2A	22 [08/07/06] J3	23   08/07/06   J3A	24 08/07/06 P1
25	08/07/06 P2	26  08/07/06   P3	27   08/07/06   RG2	

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, Ft 33682



Date Sealed: 8/7/2006



I -F F -H 756 T 0.07 0.12 0.19 0.06 662 T 0.17 0.11 0.06 662 T Online Plus -- Version 19.0.034 H-E 0.17 0.11 RUN DATE: 04-AUG-06 E -G 0.19 756 T 0.07 0.12 G -D 0.12 250 T 0.00 0.12 -Webs-----CSI -Size-----Lumber----A -M 0.10 1064 C WindLd 0.22 2x 4 SP-#2 M -I 783 T 0.19 2x 4 SP-#2 0.14 BC SP-#2 I -J 0.03 105 C WB 0.18 2x 4 J-F 0.09 149 259 Brace truss as follows: F -B 0.07 B-H 0.18 221 T To o.c. From 0-0-0 9-0-0 311 C 1 Br H-K 0.10 TC Cont. 9- 0- 0 18- 6- 0 H -C 221 T TC 2-0-0 0.18 18- 6- 0 27- 6- 0 E -C 259 TC Cont. 0.07 Т BC Cont. 0- 0- 0 27- 6- 0 E -L 0.09 149 C WB 1 rows CLB on H -K G-L 0.03 105 C Attach CLB with (2)-10d nails G -N 0.14 783 T D-N 0.10 1064 C WindLd at each web. TL Defl -0.05" in F -H LL Defl -0.02" in I -F L/999 Loading Dead (psf) Live TC 20.0 10.0 L/999 Shear // Grain in B -K 0.20 ВC 0.0 10.0 40.0 20.0 Total 20.0 Plates for each ply each face. PLATING CONFORMS TO TPI. 24.0 Spacing 1.25 Lumber Duration Factor REPORT: NER 691 Plate Duration Factor 1.25 ROBBINS ENGINEERING, INC. TC Fb=1.15 Fc=1.10 Ft=1.10 BASED ON SP LUMBER USING GROSS AREA TEST. BC Fb=1.10 Fc=1.10 Ft=1.10 Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Plus 6 Wind Load Case(s) Plus 1 UBC LL Load Case(s) Jt Type Plt Size X Y JSI 3.0x 4.0 Ctr Ctr 0.79 LOCK М React Uplft Size Req'd LOCK 3.0x 4.0 Ctr Ctr 0.84 Lbs In-Sx In-Sx LOCK 4.0x 8.0 2.5-3.4 0.72 Lbs 5-8 1-8 1.0x 3.0 Ctr Ctr 0.81 LOCK 1196 163 4.0x 8.0-2.5-3.4 0.72 -258 C LOCK Hz = LOCK 3.0x 4.0 Ctr Ctr 0.84 מ 1196 163 5-8 1 - 8 L LOCK 3.0x 4.0 Ctr Ctr 0.79 259 N Hz =1.0x 3.0 Ctr Ctr 0.81 Α LOCK 3.0x 4.0 Ctr Ctr 0.81 LOCK Membr CSI P Lbs Axl-CSI-Bnd Τ F 3.0x 4.0 Ctr Ctr 0.59 -----Top Chords-----LOCK 5.0x 7.0 Ctr-0.5 0.66 0.20 1050 C 0.00 0.20 н LOCK 0.20 944 C 0.00 0.20 E LOCK 3.0x 4.0 Ctr Ctr 0.59 - B 754 C 0.00 0.22 G LOCK 3.0x 4.0 Ctr Ctr 0.81 0.22 -C 0.22 754 C 0.00 0.22 D LOCK 1.0x 3.0 Ctr Ctr 0.81 C -L 0.20 944 C 0.00 0.20

Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

PO Box 280055

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed : 1.00 Zone location: Exterior 5.0 psf TC Dead Load : 5.0 psf BC Dead Load : 1064 Lbs Max comp. force Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



REVIEWED BY:

Robbins Engineering, Inc.

0.20

1050 C

--Bottom Chords-

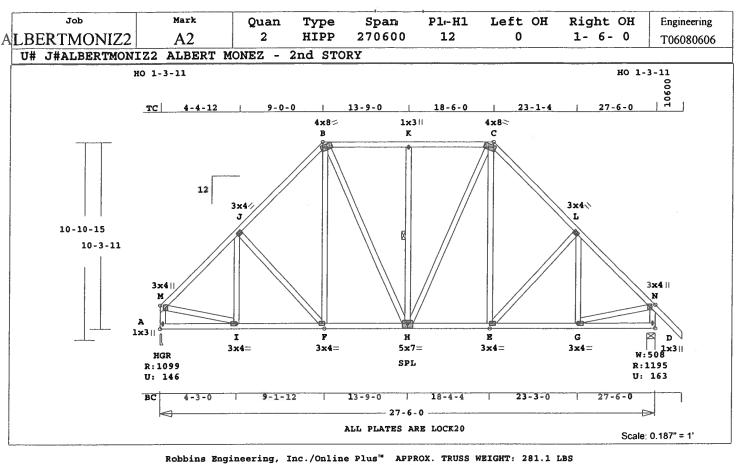
0.00

250 T 0.00 0.12

L -N

0.20

A -I 0.12



```
0.07 0.12
                                     I-F
                                                   756 T
                                            0.19
                                     F-H
                                            0.17
                                                          0.11
                                                   662 T
                                                                 0.06
                                                   662 T
Online Plus -- Version 19.0.034
                                     H-E
                                            0.17
                                                          0.11
                                                                 0.06
                                                   756 T
RUN DATE: 04-AUG-06
                                     E-G
                                            0.19
                                                          0.07
                                                                 0.12
                                     G -D
                                            0.12
                                                   250 T
                                                          0.00
                                                                 0.12
     CSI -Size- ----Lumber----
                                                   -Webs--
                                                  1064 C WindLd
TC
    0.22
         2x 4 SP-#2
                                     A -M
                                            0.10
    0.19
          2x 4
                                                   783 T
               SP-#2
                                     M -I
                                            0.14
BC
          2x 4
                SP-#2
                                     I-J
                                            0.03
                                                   105 C
    0.18
                                     J-F
                                            0.09
                                                   149 C
                                     F-B
                                                   259
                                                       Т
                                            0.07
Brace truss as follows:
                                     В -Н
                                                   221 T
                          To
                                           0.18
      o.c.
               From
              0- 0- 0 9- 0- 0
9- 0- 0 18- 6- 0
                                     H-K
                                            0.10
                                                   311 C
                                                                 1 Br
 TC Cont.
                                     H-C
                                                   221 T
 TC
     2- 0- 0
                                            0.18
 TC Cont.
             18- 6- 0 27- 6- 0
                                     E -C
                                                   259 T
                                            0.07
              0- 0- 0 27- 6- 0
                                     E-L
                                            0.09
                                                   149 C
 BC Cont.
 WB 1 rows CLB on H -K
                                     G-L
                                            0.03
                                                   105 C
Attach CLB with (2)-10d nails
                                     G-N
                                            0.14
                                                   783 T
  at each web.
                                     D -N
                                            0.10
                                                  1064 C WindLd
                                     TL Defl -0.05" in F -H L/999
LL Defl -0.02" in I -F L/999
Loading
          Live
                 Dead
                        (psf)
TC
          20.0
                  10.0
                                     Shear // Grain in B -K
           0.0
                  10.0
BC
                  20.0
                         40.0
          20.0
Total
                         24.0"
                                     Plates for each ply each face.
Spacing
Lumber Duration Factor
                                     PLATING CONFORMS TO TPI.
                         1.25
                                     REPORT: NER 691
                         1.25
Plate Duration Factor
TC Fb=1.15 Fc=1.10 Ft=1.10
                                     ROBBINS ENGINEERING, INC.
                                     BASED ON SP LUMBER
BC Fb=1.10 Fc=1.10 Ft=1.10
                                     USING GROSS AREA TEST.
                                     Plate - LOCK 20 Ga, Gross Area
Plus 6 Wind Load Case(s)
                                     Plate - RHS 20 Ga, Gross Area
     1 UBC LL Load Case(s)
                                     Jt Type
                                              Plt Size X
                                                            Y
                                                                JSI
                                       LOCK
                                              3.0x 4.0 Ctr Ctr 0.79
                                        LOCK
                                               3.0x 4.0 Ctr Ctr 0.84
    React Uplft Size Req'd
                                               4.0x 8.0 2.5-3.4 0.72
      Lbs
            Lbs In-Sx In-Sx
                                        LOCK
                                              1.0x 3.0 Ctr Ctr 0.81
     1100
            147
                 3-8 1-8
                                        LOCK
A
                        -258
                                        LOCK
                                               4.0x 8.0-2.5-3.4 0.72
                 Hz =
                 5-8 1-8
                                        LOCK
                                               3.0x 4.0 Ctr Ctr 0.84
D
     1196
            163
                                              3.0x 4.0 Ctr Ctr 0.79
                         259
                                     N
                                        LOCK
                 Hz =
                                              1.0x 3.0 Ctr Ctr 0.81
                                        LOCK
                                               3.0x 4.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-Bnd
                                     Ι
                                        LOCK
  -----Top Chords-----
                                     F
                                        LOCK
                                               3.0x 4.0 Ctr Ctr 0.59
                         0.20
                                               5.0x 7.0 Ctr-0.5 0.66
      0.20
            1050 C 0.00
                                     H
                                        LOCK
      0.20
             944 C
                    0.00
                          0.20
                                     Ε
                                        LOCK
                                              3.0x 4.0 Ctr Ctr 0.59
             754 C
                    0.00
                          0.22
                                     G
                                        LOCK
                                               3.0x 4.0 Ctr Ctr 0.81
      0.22
             754 C
                    0.00
                          0.22
                                     D
                                        LOCK
                                              1.0x 3.0 Ctr Ctr 0.81
K-C
             944 C
                    0.00
                           0.20
C -L
      0.20
```

ADDITIONAL SPECIFICATIONS. NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor: 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : BC Dead Load : 5.0 psf 1064 Lbs

REFER TO ROBBINS ENG. GENERAL

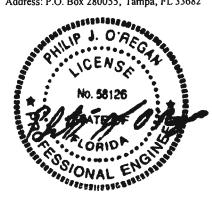
NOTES AND SYMBOLS SHEET FOR

PO Box 280055

Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682

Quality Control Factor 1.25



Max comp. force

REVIEWED BY:

Robbins Engineering, Inc.

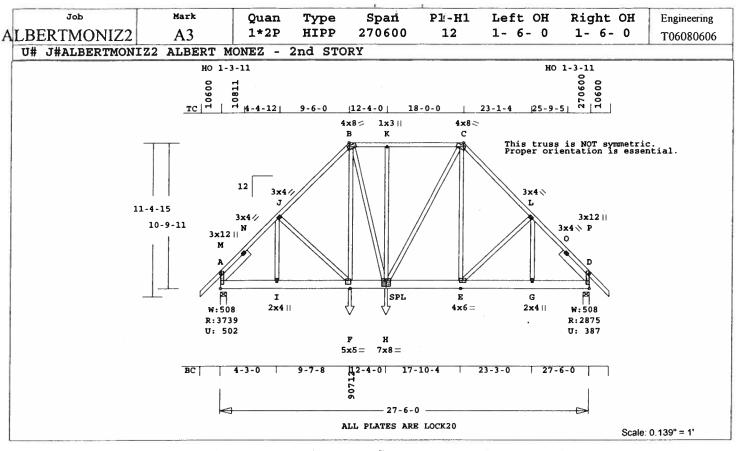
1050 C 0.00

250 T 0.00 0.12

--Bottom Chords-

0.20

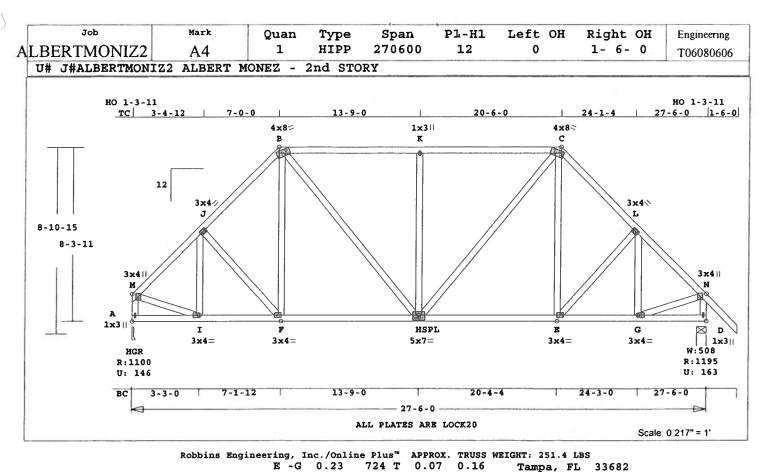
A -I 0.12



		Scale. 0. 139 - 1
Pobhins	Bigineering, Inc./Online Plus™ APPROX.	TRIISS WRIGHT: 337 3 LBS
KODJIII	O -P 0.27 1864 C 0.00 0.27	NOTES:
	Bottom Chords	Trusses Manufactured by:
Online Plus Version 19.0.034	M -I 0.63 2698 T 0.15 0.48	Mayo Truss Co. Inc.
RUN DATE: 04-AUG-06	I -F 0.29 2698 T 0.15 0.14	Analysis Conforms To:
******	F -H 0.21 2964 T 0.16 0.05	FBC2004
* 2-Ply Truss *	H -E 0.15 2209 T 0.12 0.03	2 COMPLETE TRUSSES REQUIRED.
******	E -G 0.23 2029 T 0.11 0.12	Fasten together in staggered
	G -P 0.47 2029 T 0.11 0.36	pattern. (1/2" bolts -OR-
CSI -SizeLumber	Webs	SDS3 screws -OR- 10d nails
TC 0.41 2x 4 SP-#2	I -J 0.03 510 C	as each layer is applied.)
BC 0.63 2x 8 SP-#2	J-F 0.03 421 T	Spacing (In)
WB 0.18 2x 4 SP-#2	F-B 0.18 2025 T	Rows Nails Screws Bolts
SL 0.05 2x 6 SP-#2	В -Н 0.07 772 Т	TC 1 12 24 0
	н -к 0.05 342 С	BC 2 12 24 0
Brace truss as follows:	H -C 0.18 2013 T	WB 1 8 8
O.C. From To	E -C 0.01 134 T	Plus clusters of nails where
TC Cont. 0- 0- 0 27- 6- 0	E-L 0.02 280 T	shown.
BC Cont. 0- 0- 0 27- 6- 0	G -L 0.02 406 C	OH Loading
tandiam time think (-is)		Soffit psf 2.0
Loading Live Dead (psf) TC 20.0 10.0	M -N 0.05 1627 C O -P 0.03 1220 C	Design checked for 10 psf non-
BC 0.0 10.0	0 -P 0.03 1220 C	concurrent LL on BC.
Total 20.0 20.0 40.0	TL Defl -0.11" in F -H L/999	Prevent truss rotation at all
Spacing 24.0"	LL Defl -0.06" in F -H L/999	bearing locations. Wind Loads - ANSI / ASCE 7-02
Lumber Duration Factor 1.25	Shear // Grain in A -I 0.17	Truss is designed as a Main
Plate Duration Factor 1.25	phedi // didin in k -1 0.1/	Wind-Porce Resistance System.
TC Fb=1.00 Fc=1.00 Ft=1.00	Plates for each ply each face.	Wind Speed: 110 mph
BC Fb=1.00 Fc=1.00 Ft=1.00	PLATING CONFORMS TO TPI.	Mean Roof Height: 15-0
20 12-1:00 10-1:00 10-1:00	REPORT: NER 691	Exposure Category: B
Load Case # 1 Standard Loading	ROBBINS ENGINEERING, INC.	Occupancy Factor : 1.00
Lumber Duration Factor 1.25	BASED ON SP LUMBER	Building Type: Enclosed
Plate Duration Factor 1.25	USING GROSS AREA TEST.	Zone location: Exterior
plf - Live Dead From To	Plate - LOCK 20 Ga, Gross Area	TC Dead Load : 5.0 psf
TC V 40 20 0.0' 27.5'	Plate - RHS 20 Ga, Gross Area	BC Dead Load : 5.0 psf
BC V 0 20 0.0' 27.5'	Jt Type Plt Size X Y JSI	-
BC V 75 75 9.6' 12.3'	M LOCK 3.0x12.0 1.5 0.6 0.68	Truck Design Engineer Philip I O'Dagen
BC V 973 973 9.6' CL-LB	N LOCK 3.0x 4.0 Ctr Ctr 0.73	Truss Design Engineer: Philip J. O'Regan License #: 58126
BC V 937 937 12.3' CL-LB	J LOCK 3.0x 4.0 Ctr Ctr 0.84	Address: D.O. Doy 200055 Tampa El 33682
	B LOCK 4.0x 8.0 2.5-3.4 0.72	
	K LOCK 1.0x 3.0 Ctr Ctr 0.79	-441111144-
Plus 6 Wind Load Case(s)	C LOCK 4.0x 8.0-2.5-3.4 0.91	
Plus 1 UBC LL Load Case(s)	L LOCK 3.0x 4.0 Ctr Ctr 0.84	AND IN URA
	O LOCK 3.0x 4.0 Ctr Ctr 0.73	Address: P.O. Box 280033, Talipa, TE 33002
Jt React Uplft Size Req'd	P LOCK 3.0x12.0-1.5 0.6 0.68	S Q O CHAIN
Lbs Lbs In-Sx In-Sx	I LOCK 2.0x 4.0 Ctr Ctr 0.40	\$ 10ENS
A 3740 502 5-8 2-3	F LOCK 5.0x 5.0 Ctr-1.1 0.68 H LOCK 7.0x 8.0 Ctr-2.0 0.62	
Hz = -239 D 2876 387 5- 8 1-11	E LOCK 4.0x 6.0 Ctr Ctr 0.40	No ease
2876 387 5-8 1-11 Hz = 240	G LOCK 2.0x 4.0 Ctr Ctr 0.40	No. 58126
HZ = 210	9 BOCK 2.0X 1.0 CEI CEI 0.40	
Membr CSI P Lbs Axl-CSI-Bnd		TILL MANYORE A CHAIN
Top Chords	REVIEWED BY:	
M -N 0.39 2467 C 0.01 0.38	Robbins Engineering, Inc.	3/0/4//2 // 2008
N -J 0.41 3951 C 0.03 0.38	PO Box 280055	OAID CONAL ENGINEER
7 -B 0.32 4163 C 0.03 0.29	Tampa, FL 33682	
3 -K 0.13 3156 C 0.01 0.12	Jampa, La Droom	A OCO
,	DESIGN WAS DARRING THAT COMMENT	WYONAL EVAN
C-C 0.16 3156 C 0.02 0.14	KEPER TO RUBBINS ENG. GENERAL	AV AIAME . '44.
K -C 0.16 3156 C 0.02 0.14 C -L 0.23 3089 C 0.01 0.22	REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR	ORIDA ORIDA

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A3	1*2P	HIPP	270600	12	1- 6- 0	1- 6- 0	T06080606
U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY								

Max comp. force 4163 Lbs Quality Control Factor 1.25



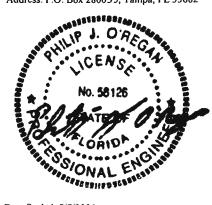
	E-G 0.23 724 T 0.07 0.16
	E-G 0.23 724 T 0.07 0.16 G-D 0.05 199 T 0.00 0.05
Online Plus Version 19.0.034	Webs
RUN DATE: 04-AUG-06	A -M 0.10 1069 C WindLd
	M -I 0.14 768 T
CSI -Size Lumber	I -J 0.04 194 C
TC 0.50 2x 4 SP-#2	J-F 0 03 102 T
BC 0.30 2x 4 SP_#2	F-R 0.03 215 T
WR 0.36 2v 4 CD-#2	R_H 0 15 388 T
WB 0.30 2X % SF-W2	H -K 0.36 456 C
Online Plus Version 19.0.034 RUN DATE: 04-AUG-06  CSI -SizeLumber TC 0.50 2x 4 SP-#2 BC 0.32 2x 4 SP-#2 WB 0.36 2x 4 SP-#2  Brace truss as follows:	H -C 0.15 388 T
O.C. From To	E -C 0.03 215 T
TC Cont. 0-0-027-6-0	E-L 0.03 102 T
BC Cont. 0- 0- 0 27- 6- 0	G-L 0.04 194 C
	G-N 0.14 768 T
Loading Live Dead (psf)	D -N 0.10 1069 C WindLd
TC 20.0 10.0	
BC 0.0 10.0	TL Defl -0.09" in F -H L/999
Total 20.0 20.0 40.0	LL Defl -0.04" in F -H L/999
Spacing 24.0"	Shear // Grain in B -K 0.29
Lumber Duration Factor 1.25	,,
Plate Duration Factor 1.25	Plates for each ply each face.
TC Fb=1.15 Fc=1.10 Ft=1.10	PLATING CONFORMS TO TPI.
BC Fb=1.10 Fc=1.10 Ft=1.10	REPORT: NER 691
20 12-1.10 10-1.10 10-1.10	ROBBINS ENGINEERING, INC.
	BASED ON SP LIMBER
Plus 6 Wind Load Case(s)	HIGING CDOGG ADRA TROT
Plus 1 IRC II. Load Case(s)	Diate - LOCK 20 Ga Gross Area
FIRS I OBC DD DORG Case(S)	Plate DUCK 20 Ga, Gross Area
The Donat Halft Circ Boald	The Time Dir Gire V V 191
Jt React Uplft Size Req'd	Jt Type Plt Size X Y JSI
Jt React Uplft Size Req'd Lbs Lbs In-Sx In-Sx	Jt Type Plt Size X Y JSI M LOCK 3.0x 4.0 Ctr Ctr 0.79
Jt React Uplft Size Req'd	Jt Type Plt Size X Y JSI M LOCK 3.0x 4.0 Ctr Ctr 0.79 J LOCK 3.0x 4.0 Ctr Ctr 0.84
Jt React Uplft Size Req'd  Lbs Lbs In-Sx In-Sx A 1100 147 3-8 1-8  Hz = -207	Jt Type Plt Size X Y JSI M LOCK 3.0x 4.0 Ctr Ctr 0.79 J LOCK 3.0x 4.0 Ctr Ctr 0.84 B LOCK 4.0x 8.0 2.5-3.4 0.72
Jt React Uplft Size Req'd Lbs Lbs In-Sx In-Sx A 1100 147 3-8 1-8 Hz = -207 D 1196 163 5-8 1-8	Jt Type Plt Size X Y JSI M LOCK 3.0x 4.0 Ctr Ctr 0.79 J LOCK 3.0x 4.0 Ctr Ctr 0.84 B LOCK 4.0x 8.0 2.5-3.4 0.72 K LOCK 1.0x 3.0 Ctr Ctr 0.81
Jt     React Uplft     Size     Req'd       Lbs     Lbs     In-Sx     In-Sx       A     1100     147     3-8     1-8       Hz     = -207       D     1196     163     5-8     1-8       Hz     = 208	### Type Plt Size X Y JSI  M LOCK 3.0x 4.0 Ctr Ctr 0.79  J LOCK 3.0x 4.0 Ctr Ctr 0.84  B LOCK 4.0x 8.0 2.5-3.4 0.72  K LOCK 1.0x 3.0 Ctr Ctr 0.81  C LOCK 4.0x 8.0-2.5-3.4 0.72
Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15  Fc=1.10  Ft=1.10 BC Fb=1.10  Fc=1.10  Ft=1.10  Plus 6 Wind Load Case(s) Plus 1 UBC LL Load Case(s)  Jt React Uplft Size Req'd	Jt Type Plt Size X Y JSI M LOCK 3.0x 4.0 Ctr Ctr 0.79 J LOCK 3.0x 4.0 Ctr Ctr 0.84 B LOCK 4.0x 8.0 2.5-3.4 0.72 K LOCK 1.0x 3.0 Ctr Ctr 0.81 C LOCK 4.0x 8.0-2.5-3.4 0.72 L LOCK 3.0x 4.0 Ctr Ctr 0.84
Jt React Uplft Size Req'd Lbs Lbs In-Sx In-Sx A 1100 147 3-8 1-8 Hz = -207 D 1196 163 5-8 1-8 Hz = 208  Membr CSI P Lbs Axl-CSI-Bnd	Jt Type Plt Size X Y JSI M LOCK 3.0x 4.0 Ctr Ctr 0.79 J LOCK 3.0x 4.0 Ctr Ctr 0.84 B LOCK 4.0x 8.0 2.5-3.4 0.72 K LOCK 1.0x 3.0 Ctr Ctr 0.81 C LOCK 4.0x 8.0-2.5-3.4 0.72 L LOCK 3.0x 4.0 Ctr Ctr 0.84 N LOCK 3.0x 4.0 Ctr Ctr 0.79
Jt React Uplft Size Req'd  Lbs Lbs In-Sx In-Sx A 1100 147 3-8 1-8  Hz = -207 D 1196 163 5-8 1-8  Hz = 208  Membr CSI P Lbs Axl-CSI-Bnd	Jt Type Plt Size X Y JSI M LOCK 3.0x 4.0 Ctr Ctr 0.79 J LOCK 3.0x 4.0 Ctr Ctr 0.84 B LOCK 4.0x 8.0 2.5-3.4 0.72 K LOCK 1.0x 3.0 Ctr Ctr 0.81 C LOCK 4.0x 8.0-2.5-3.4 0.72 L LOCK 3.0x 4.0 Ctr Ctr 0.84 N LOCK 3.0x 4.0 Ctr Ctr 0.84 N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-BndTop Chords M -I 0 10 1014 C 0 02 0 08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-BndTop Chords M -I 0 10 1014 C 0 02 0 08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-BndTop Chords M -I 0 10 1014 C 0 02 0 08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-BndTop Chords M -I 0 10 1014 C 0 02 0 08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-BndTop Chords M -I 0 10 1014 C 0 02 0 08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr         CSI         P Lbs         Axl-CSI-Bnd          Top         Chords           M -J         0.10         1014 C         0.02         0.08           J -B         0.18         1009 C         0.00         0.18           B -K         0.50         968 C         0.00         0.50           C -C         0.50         968 C         0.00         0.50           C -L         0.18         1009 C         0.00         0.18           L -N         0.10         1014 C         0.02         0.08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-BndTop Chords M -J 0.10 1014 C 0.02 0.08 J -B 0.18 1009 C 0.00 0.18 B -K 0.50 968 C 0.00 0.50 K -C 0.50 968 C 0.00 0.50 C -L 0.18 1009 C 0.00 0.18 L -N 0.10 1014 C 0.02 0.08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81 I LOCK 3.0x 4.0 Ctr Ctr 0.59 F LOCK 3.0x 4.0 Ctr Ctr 0.59 H LOCK 5.0x 7.0 Ctr-0.5 0.66 E LOCK 3.0x 4.0 Ctr Ctr 0.59 G LOCK 3.0x 4.0 Ctr Ctr 0.59 D LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr CSI P Lbs Axl-CSI-BndTop Chords M -J 0.10 1014 C 0.02 0.08 J -B 0.18 1009 C 0.00 0.18 B -K 0.50 968 C 0.00 0.50 K -C 0.50 968 C 0.00 0.50 C -L 0.18 1009 C 0.00 0.18 L -N 0.10 1014 C 0.02 0.08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81 I LOCK 3.0x 4.0 Ctr Ctr 0.59 F LOCK 3.0x 4.0 Ctr Ctr 0.59 H LOCK 5.0x 7.0 Ctr-0.5 0.66 E LOCK 3.0x 4.0 Ctr Ctr 0.59 G LOCK 3.0x 4.0 Ctr Ctr 0.59 D LOCK 1.0x 3.0 Ctr Ctr 0.81
Membr         CSI         P Lbs         Axl-CSI-Bnd          Top         Chords           M -J         0.10         1014 C         0.02         0.08           J -B         0.18         1009 C         0.00         0.18           B -K         0.50         968 C         0.00         0.50           C -C         0.50         968 C         0.00         0.50           C -L         0.18         1009 C         0.00         0.18           L -N         0.10         1014 C         0.02         0.08	N LOCK 3.0x 4.0 Ctr Ctr 0.79 A LOCK 1.0x 3.0 Ctr Ctr 0.81 I LOCK 3.0x 4.0 Ctr Ctr 0.59 F LOCK 3.0x 4.0 Ctr Ctr 0.59 H LOCK 5.0x 7.0 Ctr-0.5 0.66 E LOCK 3.0x 4.0 Ctr Ctr 0.59 G LOCK 3.0x 4.0 Ctr Ctr 0.59 D LOCK 1.0x 3.0 Ctr Ctr 0.81

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf nonconcurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph 15-0 Mean Roof Height: Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 1069 Lbs Quality Control Factor 1.25

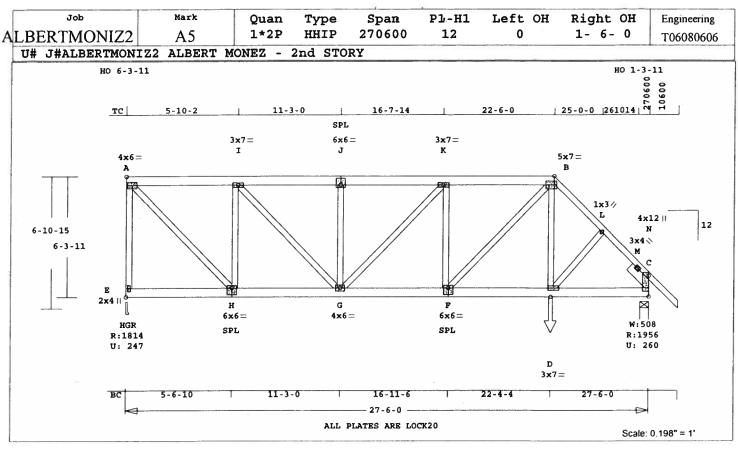
> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



PO Box 280055

720 T 0.07 0.25

H -E 0.32



Pohhing	Engineering, Inc./Online Plus™ APPROX.	TRIES WETCHT. 279 0 1.85
KODDING	B -L 0.18 1890 C 0.00 0.18	NOTES:
	L -M 0.38 1999 C 0.00 0.38	Trusses Manufactured by:
Online Plus Version 19.0.034	M -N 0.38 936 C 0.00 0.38	Mayo Truss Co. Inc.
RUN DATE: 04-AUG-06	Bottom Chords	Analysis Conforms To:
***********	E -H 0.08 146 T 0.00 0.08	PBC2004
* 2-Ply Truss *	H -G 0.14 1523 T 0.10 0.04	Girder Half Hip
**********	G -F 0.21 2249 T 0.15 0.06	Framing King Jacks
	F -D 0.18 1394 T 0.09 0.09	Jack Open Faced
CSI -SizeLumber	D-N 0.39 1264 T 0.08 0.31	Setback 5- 0- 0
TC 0.15 2x 6 SP-#2		2 COMPLETE TRUSSES REQUIRED.
EX B -N 2x 4 SP-#2	E -A 0.11 1760 C WindLd	
BC 0.39 2x 6 SP-#2	A -H 0.20 2213 T	Fasten together in staggered
	H -I 0.09 1387 C	pattern. (1/2" bolts -OR-
	I -G 0.09 1025 T	SDS3 screws -OR- 10d nails
SL 0.03 2x 6 SP-#2	G -J 0.03 555 C	as each layer is applied.)
B 6 6.33		Spacing (In)
Brace truss as follows:	*	Rows Nails Screws Bolts
O.C. From To		TC 1 12 24 0
TC Cont. 0- 0- 0 27- 6- 0	F-B 0.11 1225 T	BC 2 12 24 0
BC Cont. 0- 0- 0 27- 6- 0	D-B 0.01 206 T	WB 1 8 8
	D -L 0.01 204 T	OH Loading
Loading Live Dead (psf)	Sliders	Soffit psf 2.0
TC 20.0 10.0	M -N 0.03 1259 C	Design checked for 10 psf non-
BC 0.0 10.0		concurrent LL on BC.
Total 20.0 20.0 40.0	TL Defl -0.07" in F -D L/999	Wind Loads - ANSI / ASCE 7-02
Spacing 24.0"	LL Defl -0.04" in G -F L/999	Trușs is designed as a Main
Lumber Duration Factor 1.25	Shear // Grain in M -N 0.22	Wind-Force Resistance System.
Plate Duration Factor 1.25		Wind Speed: 110 mph
TC Fb=1.00 Fc=1.00 Ft=1.00	Plates for each ply each face.	Mean Roof Height: 15-0
BC Fb=1.00 Fc=1.00 Ft=1.00	PLATING CONFORMS TO TPI.	Exposure Category: B
	REPORT: NER 691	Occupancy Factor : 1.00
Load Case # 1 Girder Loading	ROBBINS ENGINEERING, INC.	Building Type: Enclosed
Lumber Duration Factor 1.25	BASED ON SP LUMBER	Zone location: Exterior
Plate Duration Factor 1.25	USING GROSS AREA TEST.	TC Dead Load : 5.0 psf
plf - Live Dead From To	Plate - LOCK 20 Ga, Gross Area	BC Dead Load : 5.0 psf
TC V 40 20 0.0' 27.5'	Plate - RHS 20 Ga, Gross Area	
BC V 0 20 0.0' 27.5'	Jt Type Plt Size X Y JSI	Truss Design Engineer: Philip J. O'Regan
TC V 30 15 1.0' 22.5'	A LOCK 4.0x 6.0 Ctr Ctr 0.76	License #: 58126
TC V -40 -20 0.0' 1.0'	I LOCK 3.0x 7.0 Ctr Ctr 0.43	
BC V 0 15 1.0' 22.4'	J LOCK 6.0x 6.0 Ctr 1.2 0.53	7.00.000.1.0.200.20003, 74pa, 7.2.000
BC V 0 -20 0.0' 1.0'	K LOCK 3.0x 7.0 Ctr Ctr 0.43	
BC V 133 133 22.4' CL-LB	B LOCK 5.0x 7.0 Ctr Ctr 0.90	44/14
	L LOCK 1.0x 3.0 Ctr Ctr 0.79	A JO J. O'RA"
	M LOCK 3.0x 4.0 Ctr Ctr 0.73	60.5
Plus 6 Wind Load Case(s)	N LOCK 4.0x12.0-2.0 1.0 0.96	
Plus 1 UBC LL Load Case(s)	E LOCK 2.0x 4.0 Ctr Ctr 0.65	CENCA
	H LOCK 6.0x 6.0 Ctr-1.2 0.53	Address: P.O. Box 280055, 1ampa, FL 33082
Jt React Uplft Size Req'd	G LOCK 4.0x 6.0 Ctr Ctr 0.52	
Lbs Lbs In-Sx In-Sx	F LOCK 6.0x 6.0 Ctr-1.2 0.53	No. 58126
E 1814 248 3-8 1-8	D LOCK 3.0x 7.0 Ctr Ctr 0.44	
Hz = -194		
C 1956 260 5-8 1-8		" Y'// SAATEST // YEAR
Hz = 119	REVIEWED BY:	
	Pobbine Engineering Inc	3107415.11 - 2188

Tampa, FL 33682

Membr CSI P Lbs Ax1-CSI-Bnd

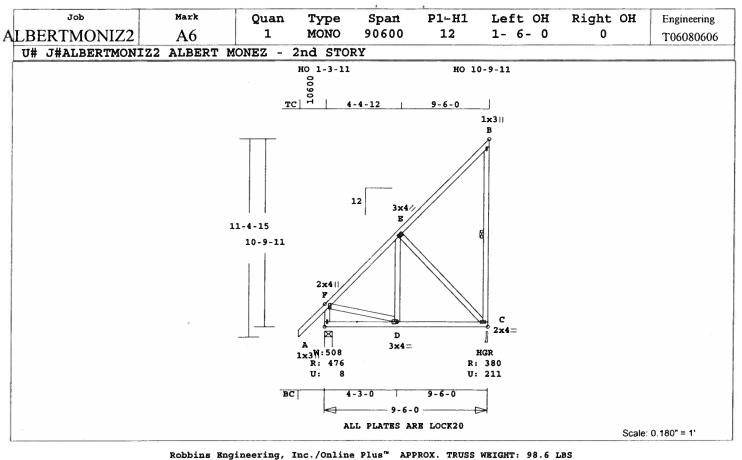
Robbins Engineering, Inc. PO Box 280055

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

AND ONAL ENTE

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A5	1*2P	HHIP	270600	12	0	1- 6- 0	T06080606
U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY								

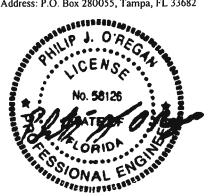
Max comp. force 2249 Lbs Quality Control Factor 1.25



```
D -C 0.18
                                             Webs-----
Online Plus -- Version 19.0.034 A -F
                                      0.03
                                              348 C WindLd
RUN DATE: 04-AUG-06
                                F -D
                                      0.03
                                              208 T
                                D -E
                                      0.02
                                              189 T
                                              287 C
     CSI -Size- ----Lumber----
                                E -C
                                      0.18
         2x 4 SP-#2
                                C -B
                                      0.22
                                             143 T WindLd 1 Br
TC
    0.29
          2x 4
                SP-#2
BC
    0.18
                                TL Defl -0.04" in D -C L/999
         2x 4
               SP-#2
   0.22
                                LL Defl -0.02" in D -C L/999
                                Shear // Grain in E -B
Brace truss as follows:
      o.c.
               From
                         To
 TC Cont.
              0- 0- 0
                      9- 6- 0
                                Plates for each ply each face.
              0- 0- 0 9- 6- 0
                                PLATING CONFORMS TO TPI.
 BC Cont.
 WB 1 rows CLB on C -B
                                REPORT: NER 691
Attach CLB with (2)-10d nails
                                ROBBINS ENGINEERING, INC.
  at each web.
                                BASED ON SP LUMBER
                                USING GROSS AREA TEST.
Loading
          Live
                 Dead
                       (psf)
                                Plate - LOCK 20 Ga, Gross Area
                                Plate - RHS 20 Ga, Gross Area
TC
          20.0
                 10.0
                                Jt Type Plt Size X Y
                 10.0
BC
           0.0
Total
          20.0
                 20.0
                        40.0
                                F
                                   LOCK
                                         2.0x 4.0 Ctr Ctr 0.80
                        24.0"
                                        3.0x 4.0 Ctr Ctr 0.56
                                Ε
                                   LOCK
Spacing
Lumber Duration Factor
                        1.25
                                В
                                   LOCK
                                        1.0x 3.0 Ctr Ctr 0.75
Plate Duration Factor
                        1.25
                                Α
                                   LOCK
                                         1.0x 3.0 Ctr Ctr 0.75
                                         3.0x 4.0 Ctr Ctr 0.54
                                   LOCK
TC Fb=1.15 Fc=1.10 Ft=1.10
                                D
BC Fb=1.10 Fc=1.10 Ft=1.10
                                C LOCK
                                         2.0x 4.0 Ctr Ctr 0.80
Plus 4 Wind Load Case(s)
                                REVIEWED BY:
Plus
    1 UBC LL Load Case(s)
                                 Robbins Engineering, Inc.
```

202 T 0.02 0.16 Provide connection to bearing for 368 Lbs Horiz Reaction Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 348 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----257 C 0.00 0.29 173 C 0.00 0.29 -----Bottom Chords-----212 T 0.00 0.16

-220

369

React Uplft Size Req'd

8

212

Lbs In-Sx In-Sx

Hz =3-8 1-8

Hz =

5-8 1-8

Jt

Α

C

Lbs

380

F-E 0.29

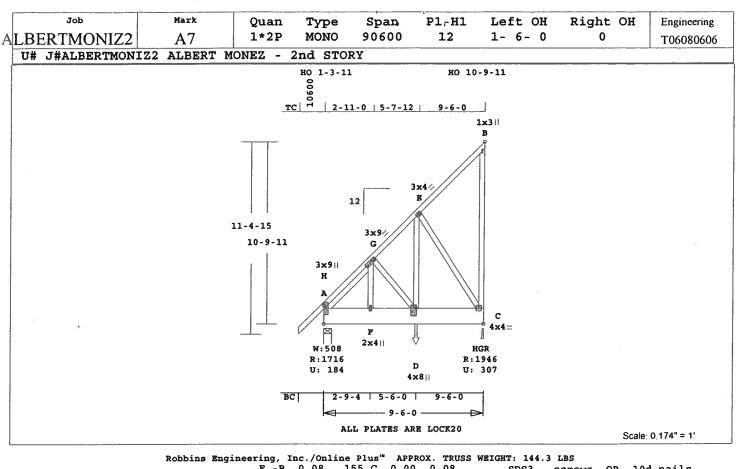
E-B 0.29

A -D 0.16

PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0



Online Plus -- Version 19.0.034 RUN DATE: 04-AUG-06 \* 2-Ply Truss \* CSI -Size- ----Lumber----0.08 2x 4 SP-#2 ВC 0.14 2x12 SP-#2 WB 0.19 2x 4 SP-#2 0.04 2x 4 Brace truss as follows: To 0.C. From TC Cont. 0- 0- 0 9- 6- 0 0- 0- 0 9-6-0 BC Cont. (psf) Live Dead Loading TC 20.0 10.0 ВÇ 0.0 10.0 Total 20.0 20.0 40.0 Spacing 24.0 Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.00 Fc=1.00 Ft=1.00 BC Fb=1.00 Fc=1.00 Ft=1.00 Load Case # 1 Standard Loading Lumber Duration Factor 1.25 Plate Duration Factor 1.25 Dead plf - Live From To 9.51 TC V 20 0.0 40 BC V 0 20 0.0' 9.5' BC v 43 43 0.01 BC V 1000 1000 5.5' CL-LB Plus 4 Wind Load Case(s) 1 UBC LL Load Case(s) Plus React Uplft Size Req'd Jt Lbs In-Sx In-Sx Lbs

E -B 0.08 155 C 0.00 0.08 155 C 0.00 0.08 H -F 0.07 998 T 0.04 0.03 -D 0.14 998 T 0.04 0.10 913 T D -C 0.13 0.03 0.10 Webs--F-G 0.02 333 T G -D 0.00 135 C 2175 T D-E 0.19 E-C 0.15 1716 C C -B 0.08 126 T WindLd ---Sliders-H-G 0.04 TL Defl -0.02" in F -D L/999 LL Defl -0.01" in F -D L/999 Shear // Grain in D -C 0.07 Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI H LOCK 3.0x 9.0 1.0-0.1 0.81 LOCK 3.0x 9.0 Ctr Ctr 0.53 LOCK 3.0x 4.0-0.4-0.4 0.76 E LOCK 1.0x 3.0 Ctr Ctr 0.75 В 2.0x 4.0 Ctr Ctr 0.38 LOCK D LOCK 4.0x 8.0 Ctr-1.6 0.56 4.0x 4.0 Ctr Ctr 0.83 LOCK

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered
pattern. (1/2" bolts -OR-

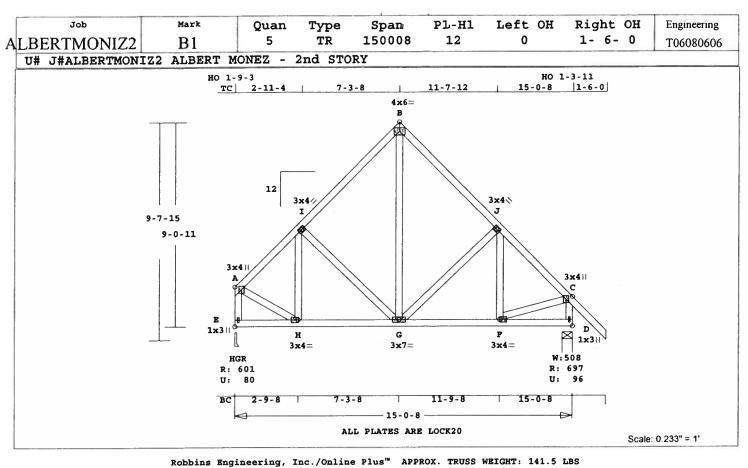
SDS3 screws -OR- 10d nails as each layer is applied.) ----Spacing (In)--Screws Bolts Nails Rows TC 1 12 24 0 12 BC 24 0 WB 8 8 Plus clusters of nails where shown. OH Loading Soffit psf 2.0 Provide connection to bearing for 346 Lbs Horiz Reaction Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : BC Dead Load : 5.0 psf 1716 Lbs Max comp. force Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

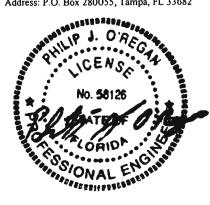
Robbins Engineering, Inc /Online Plus ™ © 1996-2006 Version 19 0 034 Engineering - Portrait 8/4/2006 8 55 06 PM Page 1



370 T 0.03 0.11 ADDITIONAL SPECIFICATIONS. 199 T 0.00 0.08 G -F 0.14 F -D 0.08 Online Plus -- Version 19.0.034 ------Webs-----580 C WindLd RUN DATE: 04-AUG-06 E -A 0.05 A -H 0.07 382 T 117 C CSI -Size- ----Lumber--H -I 0.02 115 T 2x 4 SP-#2 I -G 0.04 0.16 G-B 0.10 306 T BC 0.14 2x 4 SP-#2 147 C G -J 0.06 WB 0.10 2x 4SP-#2 F -J 0.01 66 T F -C 0.07 393 T Brace truss as follows: From To D -C 0.05 575 C WindLd o.c. 0- 0- 0 15- 0- 8 TC Cont. TL Defl -0.02" in H -G L/999 LL Defl -0.01" in H -G L/999 BC Cont. 0- 0- 0 15- 0- 8 Shear // Grain in I -B Loading Live Dead (psf) TC 20.0 10.0 10.0 Plates for each ply each face. BC 0.0 PLATING CONFORMS TO TPI. Total 20.0 20.0 40.0 REPORT: NER 691 Spacing 24.0" ROBBINS ENGINEERING, INC. Lumber Duration Factor 1.25 Plate Duration Factor 1.25 BASED ON SP LUMBER USING GROSS AREA TEST. TC Fb=1.15 Fc=1.10 Ft=1.10 Plate - LOCK 20 Ga, Gross Area BC Fb=1.10 Fc=1.10 Ft=1.10 Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI 3.0x 4.0 Ctr Ctr 0.61 4 Wind Load Case(s) A LOCK Plus 3.0x 4.0 Ctr Ctr 0.65 Ι LOCK 1 UBC LL Load Case(s) В LOCK 4.0x 6.0 Ctr-0.1 0.44 LOCK 3.0x 4.0 Ctr Ctr 0.65 React Uplft Size Req'd J Jt Lbs Lbs In-Sx In-Sx C LOCK 3.0x 4.0 Ctr Ctr 0.61 3 - 8 1 - 8 E LOCK 1.0x 3.0 Ctr Ctr 0.75 602 80 E 3.0x 4.0 Ctr Ctr 0.46 LOCK Hz =-214 н 3.0x 7.0 Ctr Ctr 0.40 698 5-81-8 G LOCK D 3.0x 4.0 Ctr Ctr 0.63 F LOCK 208 Hz =LOCK 1.0x 3.0 Ctr Ctr 0.75 D Membr CSI P Lbs Axl-CSI-Bnd ----Top Chords-----443 C 0.00 0.15 REVIEWED BY: 0.15 A-I 0.15 Robbins Engineering, Inc. 375 C 0.00 0.15 I-B PO Box 280055 376 C 0.00 0.16 B -J 0.16 503 C 0.00 Tampa, FL 33682 J -C 0.16 0.16 -----Bottom Chords----203 T 0.00 0.07 REFER TO ROBBINS ENG. GENERAL 0.07 E-H

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. 110 mph Wind Speed: Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : BC Dead Load : 5.0 psf Max comp. force 580 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



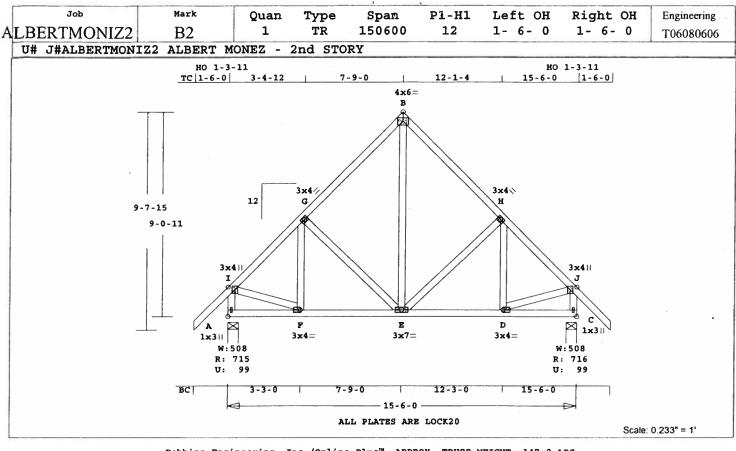
0.11

0.03

0.14

329 T

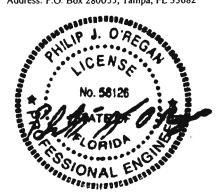
NOTES AND SYMBOLS SHEET FOR



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 147.2 LBS 384 T 0.04 0.11 ADDITIONAL SPECIFICATIONS. E -D 0.15 201 T 0.00 0.08 D -C 0.08 Online Plus -- Version 19.0.034 -------Webs-----RUN DATE: 04-AUG-06 A -I 0.05 593 C WindLd I -F 407 T 0.07 F-G 0.01 CSI -Size- ----Lumber----62 T TC 0.16 2x 4 SP-#2 G-E 0.06 144 C BC 0.15 2x 4 SP-#2 E -B 0.11 330 T 0.11 2x 4 SP-#2 E-H 0.06 144 C D -H 0.01 62 T 407 T Brace truss as follows: D-J 0.07 O.C. From To C -J 0.05 593 C WindLd 0- 0- 0 15- 6- 0 TC Cont. 0- 0- 0 15- 6- 0 TL Defl -0.02" in F -E L/999 BC Cont. LL Defl -0.01" in F -E L/999 Shear // Grain in G -B Loading Live Dead (psf) 0.12 10.0 TC 20.0 BC 0.0 10.0 Plates for each ply each face. PLATING CONFORMS TO TPI. Total 20.0 20.0 40.0 Spacing 24.0" REPORT: NER 691 ROBBINS ENGINEERING, INC. Lumber Duration Factor 1.25 1.25 Plate Duration Factor BASED ON SP LUMBER TC Fb=1.15 Fc=1.10 Ft=1.10 USING GROSS AREA TEST. BC Fb=1.10 Fc=1.10 Ft=1.10 Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Plt Size X Y Jt Type 4 Wind Load Case(s) Plus LOCK 3.0x 4.0 Ctr Ctr 0.62 Т 1 UBC LL Load Case(s) G LOCK 3.0x 4.0 Ctr Ctr 0.66 Plus 4.0x 6.0 Ctr-0.1 0.44 LOCK В React Uplft Size Req'd н LOCK 3.0x 4.0 Ctr Ctr 0.66 Lbs Lbs In-Sx In-Sx J LOCK 3.0x 4.0 Ctr Ctr 0.62 5-8 1-8 1.0x 3.0 Ctr Ctr 0.75 A 716 99 A LOCK -209 Hz =LOCK 3.0x 4.0 Ctr Ctr 0.64 C 99 5-8 1-8 LOCK 716 E 3.0x 7.0 Ctr Ctr 0.40 Hz =210 D LOCK 3.0x 4.0 Ctr Ctr 0.64 1.0x 3.0 Ctr Ctr 0.75 LOCK Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----REVIEWED BY: I -G 0.16 522 C 0.00 0.16 G -B 0.16 398 C 0.00 0.16 Robbins Engineering, Inc. 398 C 0.00 0.16 522 C 0.00 0.16 PO Box 280055 B -H 0.16 Н 0.16 Tampa, FL 33682

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 593 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



0.11

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

---Bottom Chords-----

384 T

201 T 0.00 0.08

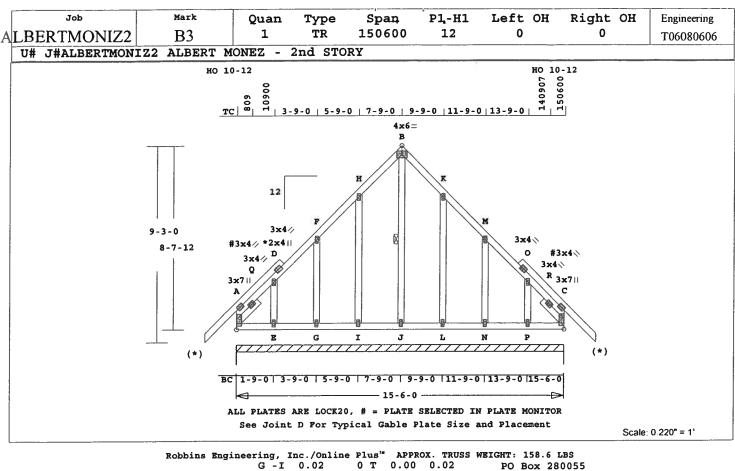
0.04

A -F

- E

0.08

0.15



G -I I -J 0.02 0 T 0.00 0.02 0.02 0 T 0.00 0.02 Online Plus -- Version 19.0.034 J-L 0.02 0 T 0.00 0.02 0 T RUN DATE: 04-AUG-06 L - N 0.02 0.00 0.02 - P 0.02 0 T 0.00 0.02 ----Lumber----P -C 0.05 3 Т 0.00 0.05 ----Gable Webs----(\*) TC 0.04 2x 4 SP-#2 E -D 0.05 2x 4 SP-#2 0.01 125 T BC 123 C 0.06 2x 4 SP-#2 G-F 0.03 122 C 166 C I-H SP-#2 0.00 2x 4 0.06 1 Br J-B 0.03 122 Brace truss as follows: L -K 0.06 To 123 TC Cont. 0- 0- 0 15- 6- 0 BC Cont. 0- 0- 0 15- 6- 0 WB 1 rows CLB on J -B N -M 0.03 o.c. From 125 P -0 0.01 Т ------Sliders-A -Q R -C 0.00 105 C Attach CLB with (2)-10d nails 0.00 105 C at each web. 0.00" in E -G 0.00" in E -G L/999 TL Defl LL Defl L/999 Dead Live (psf) Loading Shear // Grain in Q -D 20.0 10.0 TC 0.0 10.0 BC Plates for each ply each face. PLATING CONFORMS TO TPI. 20.0 40.0 20.0 Total 24.0" Spacing REPORT: NER 691 Lumber Duration Factor 1.25 ROBBINS ENGINEERING, INC. Plate Duration Factor TC Fb=1.15 Fc=1.10 Ft=1.10 BASED ON SP LUMBER BC Fb=1.10 Fc=1.10USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Plt Size X Y JSI 3.0x 7.0 1.5 0.7 0.96 3.0x 4.0 Ctr Ctr 0.50 Plus 4 Wind Load Case(s) Jt Type Plus 1 UBC LL Load Case(s) LOCK LOCK React Uplft Size Req'd 2.0x 4.0 Ctr Ctr 0.00 LOCK 2.0x 4.0 Ctr Ctr 0.00 Lbs In-Sx In-Sx F LOCK Lbs 0- 0- 0 to 15- 6- 0 165 Hz = 178 н LOCK 2.0x 4.0 Ctr Ctr 0.00 Brg 4.0x 6.0 Ctr-0.1 0.44 1240 В LOCK 2.0x 4.0 Ctr Ctr 0.00 2.0x 4.0 Ctr Ctr 0.00 LOCK Membr CSI P Lbs Axl-CSI-Bnd M LOCK 0.04 75 C 0.00 0.04 168 C 0.00 2.0x 4.0 Ctr Ctr 0.00 LOCK 0 0.04 R LOCK 3.0x 4.0 Ctr Ctr 0.50 3.0x 7.0-1.5 0.7 0.96 C LOCK 0.04 Q -D 0.04 0.00 0.03 E LOCK 2.0x 4.0 Ctr Ctr 0.00 89 - F 0.03 C T 0.00 LOCK 2.0x 4.0 Ctr Ctr 0.00 66 0.03 G F - H 0.03 0.00 I LOCK 2.0x 4.0 Ctr Ctr 0.00 0.03 н 148 - B 0.03 2.0x 4.0 Ctr Ctr 0.00 148 Т 0.00 0.03 J LOCK В - K 0.03 2.0x 4.0 Ctr Ctr 0.00 66 0.00 0.03 LOCK L ~ M 0.03 2.0x 4.0 Ctr Ctr 0.00 89 C 0.00 0.03 N LOCK M -0 0.03 2.0x 4.0 Ctr Ctr 0.00 168 C 0.00 0.04 LOCK 0 - R 0.04 75 C 0.04 0.00 0.04 R -C

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 Design checked for 10 psf nonconcurrent LL on BC. Refer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed:
Mean Roof Height: 15-0
Exposure Category: B
Factor: 1.00 Wind Speed: 110 mph Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : BC Dead Load : 5.0 psf

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

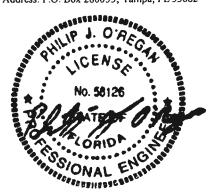
ADDITIONAL SPECIFICATIONS.

Tampa, FL 33682

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682

Quality Control Factor 1.25

168 Lbs



Max comp. force

REVIEWED BY:

Robbins Engineering, Inc.

--Bottom Chords----

3 T

0.00

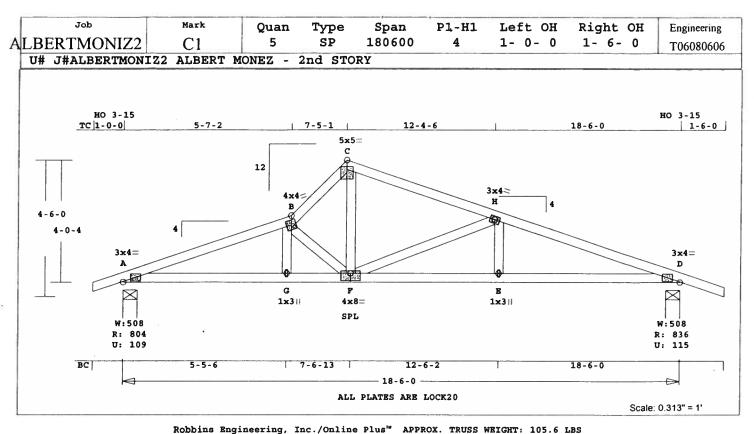
0 T 0.00 0.02

-E

E-G

0.05

0.02



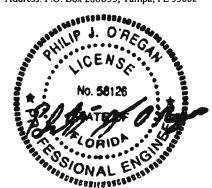
-----Bottom Chords-----

A-G 0.38 1549 T 0.26 0.12 Online Plus -- Version 19.0.034 G -F 0.29 1546 T 0.25 0.04 RUN DATE: 04-AUG-06 F-E 0.33 1523 T 0.25 0.08 E -D 0.38 1523 T 0.25 0.13 CSI -Size- ----Lumber-----Webs - -2x 4 0.28 SP-#2 G -B 0.01 128 T 0.38 2x 4 SP-#2 0.10 849 C B-F 0.24 2x 4 SP-#2 F -C 0.16 867 T F-H 0.24 687 C E -H 0.03 Brace truss as follows: 226 T 0.C. From To 0- 0- 0 18- 6- 0 TL Defl -0.13" in F -E L/999 TC Cont. LL Defl -0.06" in F -E L/999 0- 0- 0 18- 6- 0 BC Cont. Shear // Grain in H -D 0.22 Loading Live Dead (psf) TC 20.0 10.0 Plates for each ply each face. ВC 0.0 10.0 PLATING CONFORMS TO TPI. Total 20.0 20.0 40.0 REPORT: NER 691 Spacing 24.0" ROBBINS ENGINEERING, INC. Lumber Duration Factor 1.25 BASED ON SP LUMBER Plate Duration Factor USING GROSS AREA TEST. 1.25 Plate - LOCK 20 Ga, Gross Area TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10 Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y 3.0x 4.0 Ctr Ctr 0.80 LOCK Plus 6 Wind Load Case(s) LOCK 4.0x 4.0 Ctr Ctr 0.83 Plus 1 UBC LL Load Case(s) C LOCK 5.0x 5.0 Ctr-1.3 0.86 H LOCK 3.0x 4.0 Ctr Ctr 0.52 3.0x 4.0 Ctr Ctr 0.80 React Uplft Size Req'd LOCK Jt D G LOCK Lbs Lbs In-Sx In-Sx 1.0x 3.0 Ctr Ctr 0.81 804 110 5- 8 1-8 F LOCK 4.0x 8.0 Ctr-1.0 0.67 Α -64 LOCK 1.0x 3.0 Ctr Ctr 0.81 Hz = מ 836 115 5- 8 1-8 Hz =48

ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 1628 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



REVIEWED BY: Membr CSI P Lbs Axl-CSI-Bnd

Robbins Engineering, Inc. PO Box 280055

Tampa, FL 33682

1628 C B -C 0.20 1166 C 0.00 0.20 C -H 0.27 943 C 0.00 0.27

-----Top Chords-----

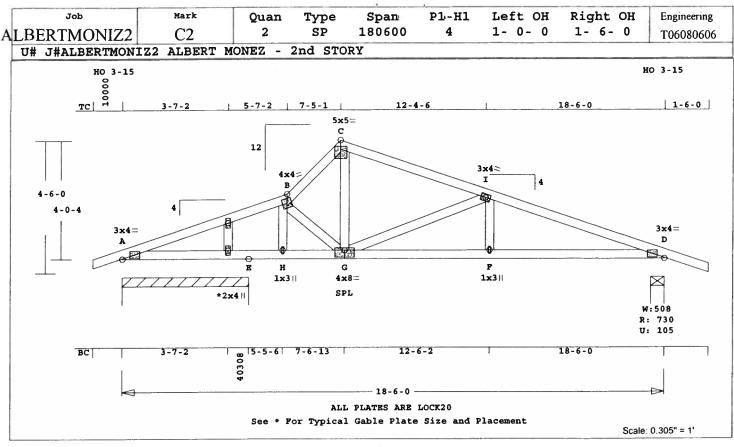
A -B

0.20

REFER TO ROBBINS ENG. GENERAL H -D NOTES AND SYMBOLS SHEET FOR 0.28 1599 C 0.01 0.27

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0.02 0.18



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 107.4 LBS 705 T 0.11 0.31 Trusses Manufactured by: 1226 T 0.20 0.09 Mayo Truss Co. Inc. H-G 0.42 1226 T G-F 0.29 Mayo Truss Co. Inc. 0.34 1226 T 0.20 0.14 Analysis Conforms To: Online Plus -- Version 19.0.034 F -D RUN DATE: 04-AUG-06 -Webs-----FBC2004 386 C OH Loading H -B 0.03 CSI -Size- ----Lumber----B-G 0.01 158 C Soffit psf 2.0 2x 4 SP-#2 G -C 0.09 531 T Design checked for 10 psf non-0.30 G-I 0.24 696 C SP-#2 2x 4concurrent LL on BC. BC 0.49 0.24 2x 4 SP-#2 F-I 0.03 225 T Refer to Gen Det 3 series for web bracing and plating. TL Defl -0.09" in F -D L/999 LL Defl -0.04" in F -D L/999 Shear // Grain in E -H 0.43 Wind Loads - ANSI / ASCE 7-02 Brace truss as follows: To From Truss is designed as a Main O.C. 0- 0- 0 18- 6- 0 Wind-Force Resistance System. TC Cont. BC Cont. 0- 0- 0 18- 6- 0 Wind Speed: Mean Roof Height: 15-0 Plates for each ply each face. Loading Live Dead (psf) PLATING CONFORMS TO TPI. Exposure Category: Occupancy Factor : 1.00 REPORT: NER 691 TC 10.0 20.0 ROBBINS ENGINEERING, INC. BC 0.0 10.0 Building Type: Enclosed 20.0 20.0 40.0 BASED ON SP LUMBER Zone location: Exterior Total USING GROSS AREA TEST. 24.0" TC Dead Load : Spacing Lumber Duration Factor 1.25 Plate - LOCK 20 Ga, Gross Area BC Dead Load : Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI 1284 Lbs Plate Duration Factor 1.25 Max comp. force TC Fb=1.15 Fc=1.10 Ft=1.10 Quality Control Factor 1.25 BC Fb=1.10 Fc=1.10 Ft=1.10 LOCK 3.0x 4.0 Ctr Ctr 0.80 4.0x 4.0 Ctr Ctr 0.83 LOCK В LOCK 5.0x 5.0 Ctr-1.3 0.86 3.0x 4.0 Ctr Ctr 0.52 LOCK 6 Wind Load Case(s) Plus Ι LOCK 3.0x 4.0 Ctr Ctr 0.80 1 UBC LL Load Case(s) D Truss Design Engineer: Philip J. O'Regan LOCK 1.0x 3.0 Ctr Ctr 0.81 н License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682 React Uplft Size Req'd G LOCK 4.0x 8.0 Ctr-1.0 0.67 APPROVED D. O. A. Lbs In-Sx In-Sx LOCK 1.0x 3.0 Ctr Ctr 0.81 Lbs Cont. Brg 0- 0- 0 to 4-3-8 PHILIP J. O. R.C. 909 119 Hz =65 1 Gable studs to be attached 5-8 1-8 with 2.0x4.0 plates each end. n 731 106 CENS Hz = REVIEWED BY: Membr CSI P Lbs Axl-CSI-Bnd Robbins Engineering, Inc. ------Top Chords-----PO Box 280055 726 C 0.00 0.20 0.20 Tampa, FL 33682 B -C 0.14 782 C 0.00 0.14 0.00 0.29 REFER TO ROBBINS ENG. GENERAL C -I 0.29 622 C AND SIONAL ENTER 1284 C NOTES AND SYMBOLS SHEET FOR 0.30 0.01 0.29 I -D -----Bottom Chords-----ADDITIONAL SPECIFICATIONS. 0.00 0.38

110 mph

5.0 psf

5.0 psf

B

0.38

NOTES:

0.11

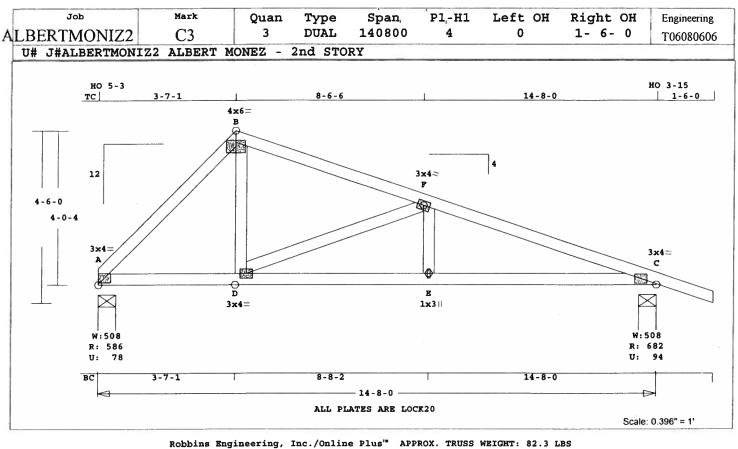
A -E

E -H

0.38

0.49

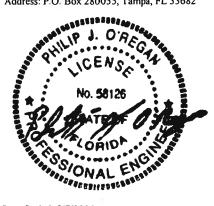
121 T 685 T



-----Bottom Chords-----A -D 0.12 435 T 0.07 0.05 Online Plus -- Version 19.0.034 D -E 0.28 1092 T 0.18 0.10 RUN DATE: 04-AUG-06 E -C 0.32 1092 T 0.18 0.14 -Webs--D-B 0.06 358 T CSI -Size- ----Lumber----SP-#2 D-F 0.24 710 C TC 0.29 2x 4 2x 4 SP-#2 E-F 0.03 231 T 0.32 0.24 2x 4 SP-#2 TL Defl -0.08" in E -C L/999 LL Defl -0.03" in E -C L/999 Brace truss as follows: Shear // Grain in F -C O.C. To 0.22 From 0- 0- 0 14- 8- 0 TC Cont. 0- 0- 0 14- 8- 0 BC Cont. Plates for each ply each face. PLATING CONFORMS TO TPI. Loading Live Dead REPORT: NER 691 (psf) TC 20.0 10.0 ROBBINS ENGINEERING, INC. BC 0.0 10.0 BASED ON SP LUMBER USING GROSS AREA TEST. Total 20.0 20.0 40.0 24.0" Plate - LOCK 20 Ga, Gross Area Spacing 1.25 Plate - RHS 20 Ga, Gross Area Lumber Duration Factor Plate Duration Factor 1.25 Jt Type Plt Size X Y TC Fb=1.15 Fc=1.10 Ft=1.10 LOCK 3.0x 4.0 0.4 0.3 0.62 Α BC Fb=1.10 Fc=1.10 Ft=1.10 В LOCK 4.0x 6.0 Ctr-1.3 0.95 LOCK 3.0x 4.0 Ctr Ctr 0.48 С LOCK 3.0x 4.0 Ctr Ctr 0.74 5 Wind Load Case(s) D LOCK 3.0x 4.0 Ctr Ctr 0.64 Plus 1 UBC LL Load Case(s) E LOCK 1.0x 3.0 Ctr Ctr 0.75 React Uplft Size Req'd Jt Lbs In-Sx In-Sx REVIEWED BY: Lbs Robbins Engineering, Inc. Α 587 78 5 - 8 1-8 -80 PO Box 280055 Hz =C 683 95 5-8 1-8 Tampa, FL 33682 Hz =49 REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR Membr CSI P Lbs Axl-CSI-Bnd ADDITIONAL SPECIFICATIONS. -----Top Chords-----A -B 0.11 612 C 0.00 0.11 0.00 0.29 B-F 0.29 468 C NOTES:

Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf 5.0 psf BC Dead Load : Max comp. force 1143 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682

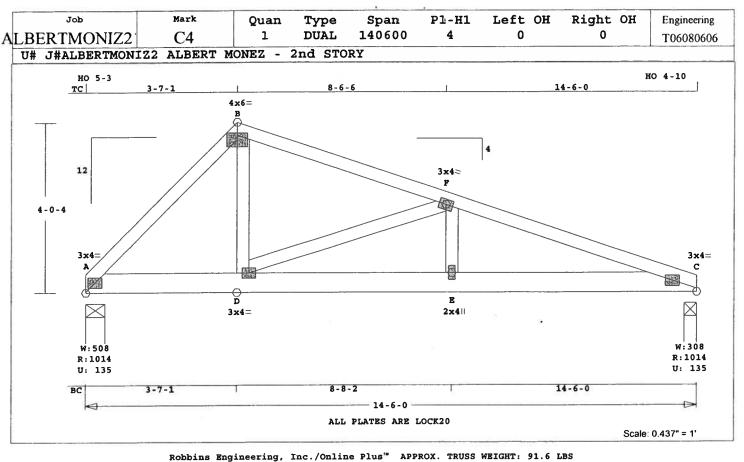


Trusses Manufactured by:

1143 C 0.00 0.29

F -C

0.29



A -B 0.21 1077 C 0.00 0.21 B -F 0.29 827 C 0.00 0.29 Analysis Conforms To: FBC2004 1987 C 0.02 0.29 Online Plus -- Version 19.0.034 F -C 0.31 Common Girder ---Bottom Chords-----RUN DATE: 04-AUG-06 Loading BC 5- 0- 0 0.19 777 T 0.10 0.09 A -D Span 1894 T 0.25 0.14 CSI -Size- ----Lumber----D-E 0.39 Design checked for 10 psf non-0.31 2x 4 SP-#2 1894 T 0.25 0.27 E -C 0.52 concurrent LL on BC. TC -Webs-----0.52 2x 6 SP-#2 ----Use properly rated hangers for SP-#2 D-B 0.14 771 T loads framing into girder 0.41 2x 4 1200 C D-F 0.41 truss. Brace truss as follows: E-F 0.08 478 T Wind Loads - ANSI / ASCE 7-02 To Truss is designed as a Main O.C. From TL Defl -0.09" in E -C LL Defl -0.05" in E -C L/999 TC Cont. 0- 0- 0 14- 6- 0 Wind-Force Resistance System. 0- 0- 0 14- 6- 0 L/999 Wind Speed: BC Cont. Shear // Grain in B -F Mean Roof Height: 15-0 0.21 Exposure Category: Dead Loading Live (psf) Occupancy Factor : 1.00 Plates for each ply each face. TC 20.0 10.0 Building Type: Enclosed BC 0.0 10.0 PLATING CONFORMS TO TPI. REPORT: NER 691 40.0 Zone location: Exterior Total 20.0 20.0 24.0" ROBBINS ENGINEERING, INC. TC Dead Load : Spacing Lumber Duration Factor BASED ON SP LUMBER BC Dead Load : 1.25 USING GROSS AREA TEST. Plate Duration Factor 1.25 Max comp. force TC Fb=1.00 Fc=1.00 Ft=1.00 BC Fb=1.00 Fc=1.00 Ft=1.00 Plate - LOCK 20 Ga, Gross Area Quality Control Factor 1.25 Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI A LOCK 3.0x 4.0 Ctr Ctr 0.61 Load Case # 1 Girder Loading Lumber Duration Factor 1.25 В LOCK 4.0x 6.0 Ctr-1.3 0.94 Plate Duration Factor 1.25 F LOCK 3.0x 4.0 Ctr Ctr 0.65

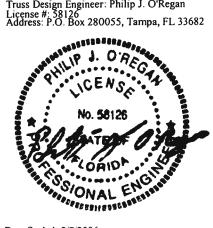
Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682

110 mph

5.0 psf

5.0 psf

1987 Lbs



5 Wind Load Case(s) Plus 1 UBC LL Load Case(s) Plus

20

50

plf - Live Dead From

40

30

TC V

BC V

React Uplft Size Req'd Jt. Lbs Lbs In-Sx In-Sx 1015 135 5-8 1-8 A -78 Hz = 3 - 8 1-8 1015 135 Hz =48

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

3.0x 4.0 Ctr Ctr 0.84

3.0x 4.0 Ctr Ctr 0.82

2.0x 4.0 Ctr Ctr 0.51

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc.

To

14.51

14.5'

0.0'

0.0'

C

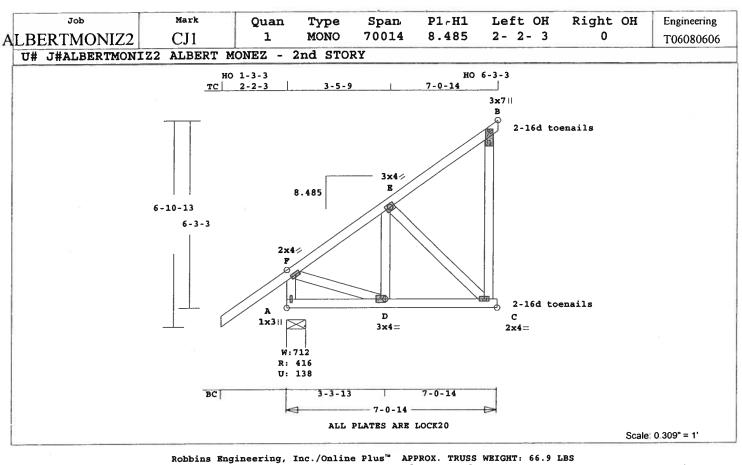
D

R

LOCK

LOCK

LOCK



Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----Online Plus -- Version 19.0.034 F-E 0.14 186 C 0.00 0.14 0.00 0.16 E -B 0.16 169 T RUN DATE: 04-AUG-06 B -B 0.00 9 C ---Bottom Chords-----CSI -Size- ----Lumber----2x 4 SP-#2 A -D 0.07 105 T 0.00 0.07 TC 0.16 2x 4 SP-#2 D -C 0.08 163 T 0.01 0.07 0.08 0.06 2x 4 SP-#2 Webs-----WB A -F 0.02 249 C WindLd Brace truss as follows: 0.03 F -D 172 T D-E 0.01 121 T O.C. From To TC Cont. 0- 0- 0 7- 0-14 E -C 0.06 225 C 7- 0-14 C -B 0.06 0 T WindLd BC Cont. 0- 0- 0 TL Defl -0.01" in D -C L/999 Live Dead (psf) Loading 0.00" in D -C L/999 10.0 LL Defl TC 20.0 Shear // Grain in E -B BC 0.0 10.0 0.16 Total 20.0 20.0 40.0 Plates for each ply each face. PLATING CONFORMS TO TPI. 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 REPORT: NER 691 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Load Case # 2 NonStandard Load Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI Lumber Duration Factor 1.25 Jt Type Plate Duration Factor 1.25 plf -F LOCK 2.0x 4.0 Ctr Ctr 0.75 Live Dead From To 3.0x 4.0 Ctr Ctr 0.46 TC V 0 0.01 7.1' LOCK 0 0.01 7.1' 3.0x 7.0 Ctr-0.3 0.36 LOCK BC V 0 0 В TC 0 O 0.0' LOCK 1.0x 3.0 Ctr Ctr 0.75 v 79 40 7.1' LOCK 3.0x 4.0 Ctr Ctr 0.38 0 0.0' LOCK 2.0x 4.0 Ctr Ctr 0.75 0 BC V n 40 7.1'

Plus 5 Wind Load Case(s) 1 UBC LL Load Case(s)

React Uplft Size Req'd Jt Lbs Lbs In-Sx In-Sx 7-12 416 138 1-8 A -117 Hz =1-8 C 209 50 1- 8 1-8 1-8 В 176 Hz = 205

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

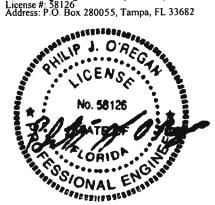
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

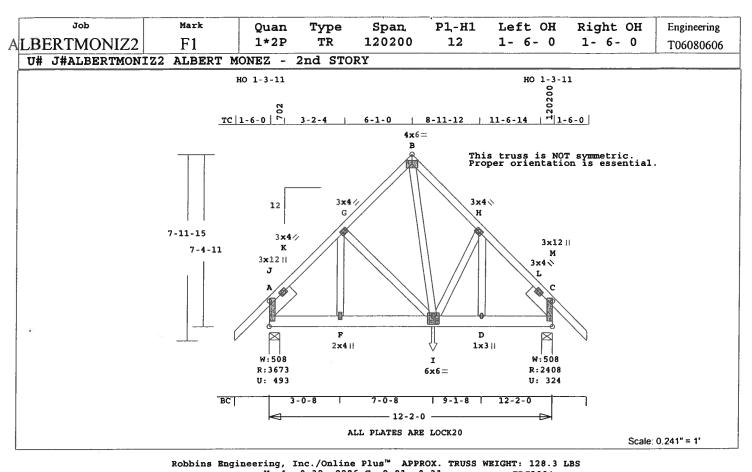
For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC.

Max gap between edge of brg and end vertical is 1/2". Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : BC Dead Load : 5.0 psf 249 Lbs Max comp. force Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682





H -L 0.32 2286 C 0.01 0.31 L -M 0.31 1500 C 0.00 0.31 -----Bottom Chords-----Online Plus -- Version 19.0.034 RUN DATE: 04-AUG-06 0.81 2027 T 0.13 0.48 2027 T 0.13 J-F 0.68 P -T 0.48 0.35 1527 T 0.10 \* 2-Ply Truss \* I -D 0.19 D-M 0.59 1527 T 0.10 0.49 -Webs-----F -G 0.12 1407 T ----Lumber---0.35 2x 4 SP-#2 G-I 0.05 870 C B -I 0.27 BC 0.81 2x 6 SP-#2 2936 T 2x 4 SP-#2 I-H 0.06 703 T WB 0.27 0.03 2x 6 SP-#2 D -H 0.04 787 C -----Sliders-----J -K 0.03 1001 C Brace truss as follows: O.C. From To cont. 0-0-0 12-2-0 cont. 0-0-0 12-2-0 L -M 0.02 960 C TC Cont. TL Defl -0.08" in F -I L/999 LL Defl -0.04" in F -I L/999 BC Cont. Loading Live Dead (psf) Shear // Grain in A -F 0.48 TC 20.0 10.0 Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 ВC 0.0 10.0 40.0 Total 20.0 20.0 24.0 Spacing 1.25 ROBBINS ENGINEERING, INC. Lumber Duration Factor Plate Duration Factor 1.25 BASED ON SP LUMBER TC Fb=1.00 Fc=1.00 Ft=1.00 BC Fb=1.00 Fc=1.00 Ft=1.00 USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI J LOCK 3.0x12.0 1.5 0.7 0.67 Load Case # 1 Standard Loading JSI Lumber Duration Factor 1.25 Duration Factor 1.25 LOCK 3.0x 4.0 Ctr Ctr 0.53 To 12.2 3.0x 4.0 Ctr Ctr 0.66 4.0x 6.0 Ctr-0.1 0.81 - Live Dead From LOCK TC V 40 20 0.0 В LOCK 0.0' 12.2' 3.0x 4.0 Ctr Ctr 0.60 20 Н LOCK 0 3.0x 4.0 Ctr Ctr 0.53 BC 255 255 0.0' 6.1' LOCK 907 CL-LB LOCK 3.0x12.0-1.5 0.7 0.67 907 7.01 LOCK 2.0x 4.0 Ctr Ctr 0.76 LOCK 6.0x 6.0 Ctr-1.4 0.57 LOCK 1.0x 3.0 Ctr Ctr 0.75 т LOCK Plus 4 Wind Load Case(s) 1 UBC LL Load Case(s) Plus REVIEWED BY: React Uplft Size Req'd Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682 Lbs In-Sx In-Sx 3674 493 5-8 2-3 Hz = -137 5-8 1-8 325 C 2408 REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR Membr CSI P Lbs Axl-CSI-Bnd ADDITIONAL SPECIFICATIONS. -----Top Chords-----J -K 0.35 2178 C 0.01 0.34 К -G 0.35 2999 C 0.01 0.34 NOTES: Trusses Manufactured by: 0.01 1938 Mayo Truss Co. Inc. 0.15

FBC2004 2 COMPLETE TRUSSES REQUIRED. Fasten together in staggered pattern. (1/2" bolts -OR-SDS3 screws -OR- 10d nails as each layer is applied.) ----Spacing (In)---Rows Nails Screws Bolts TC 1 BC 2 12 24 12 24 0 WB 1 8 8 Plus clusters of nails where shown. OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Prevent truss rotation at all bearing locations. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System 110 mph Wind Speed: Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf 5.0 psf BC Dead Load : Max comp. force 2999 Lb Quality Control Factor 1.25 2999 Lbs

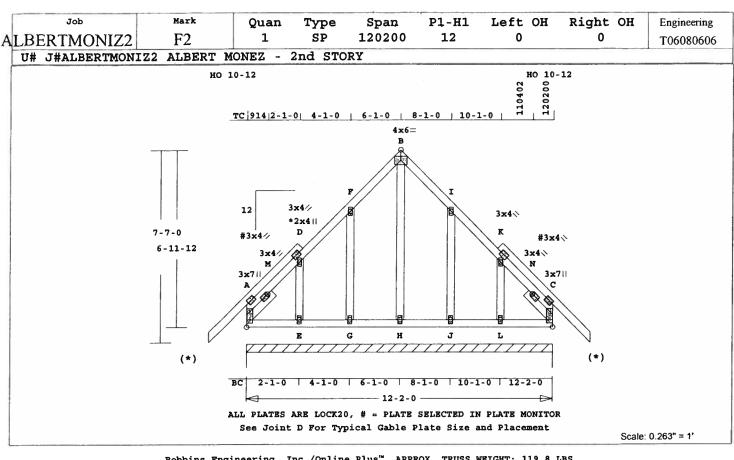
Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



2528 C

0.01

Analysis Conforms To:



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 119.8 LBS G -H 0.02 0 T 0.00 0.02 0.02 H -J 0 T 0.00 0.02 Online Plus -- Version 19.0.034 0.02 0 T 0.00 0.02 J-L 0.04 2 Т 0.00 0.04 RUN DATE: 04-AUG-06 L -C -----Gable Webs---------E -D 0.01 121 T CSI -Size- ----Lumber----2x 4 SP-#2 0.03 G-F 0.03 123 C (\*) TC 0.08 H -B 152 C BC 0.04 2x 4 SP-#2 GW 0.08 2x 4 SP-#2 J-I 0.03 123 C 0.00 2x 4 SP-#2 L -K 0.01 121 T -----Sliders-A -M 0.00 63 C Brace truss as follows: o.c. From To N -C 0.00 63 C TC Cont. 0- 0- 0 12- 2- 0 0- 0- 0 12- 2- 0 TL Defl 0.00" in A -E L/999 BC Cont. LL Defl 0.00" in A -E L/999 Shear // Grain in M -D 0.05 Loading Live Dead (psf) TC 20.0 10.0 0.0 10.0 Plates for each ply each face. BC PLATING CONFORMS TO TPI. 40.0 20.0 Total 20.0 Spacing 24.0" REPORT: NER 691 ROBBINS ENGINEERING, INC. Lumber Duration Factor 1.25 BASED ON SP LUMBER Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10 USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y 3.0x 7.0 1.5 0.7 0.88 Plus 4 Wind Load Case(s) LOCK LOCK 3.0x 4.0 Ctr Ctr 0.50 Plus 1 UBC LL Load Case(s) M D LOCK 2.0x 4.0 Ctr Ctr 0.00 LOCK 2.0x 4.0 Ctr Ctr 0.00 React Uplft Size Req'd 4.0x 6.0 Ctr-0.1 0.41 Lbs In-Sx In-Sx В LOCK Lbs 0- 0- 0 to 12- 2- 0 LOCK 2.0x 4.0 Ctr Ctr 0.00 I Cont. Brg LOCK 2.0x 4.0 Ctr Ctr 0.00 973 130 Hz =138 K N LOCK 3.0x 4.0 Ctr Ctr 0.50 Membr CSI P Lbs Axl-CSI-Bnd С LOCK 3.0x 7.0-1.5 0.7 0.88 -----Top Chords-----2.0x 4.0 Ctr Ctr 0.00 E LOCK 2.0x 4.0 Ctr Ctr 0.00 57 C 0.00 0.03 A -M 0.03 G LOCK M -D 0.03 109 C 0.00 0.03 н LOCK 2.0x 4.0 Ctr Ctr 0.00 D 0.03 57 T 0.00 0.03 J LOCK 2.0x 4.0 Ctr Ctr 0.00 -F 2.0x 4.0 Ctr Ctr 0.00 -B 0.00 0.03 LOCK F 138 T 0.03 В - I 0.03 138 T 0.00 0.03 I-K 57 T 0.00 0.03 0.03 REVIEWED BY: 0.03 109 C 0.00 0.03 K-N Robbins Engineering, Inc. 57 C 0.00 N-C 0.03 0.03 PO Box 280055 ---Bottom Chords----A-E 0.04 2 T 0.00 0.04 Tampa, FL 33682 0 T 0.00 0.02 E-G 0.02

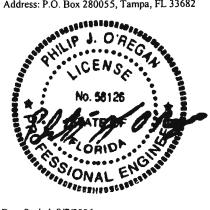
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

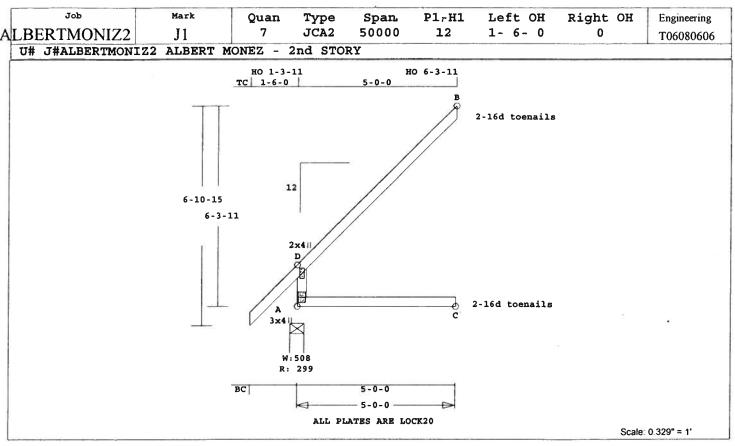
NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 Design checked for 10 psf nonconcurrent LL on BC. Refer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf 5.0 psf BC Dead Load : Max comp. force 152 Lbs

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682

Quality Control Factor 1.25





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 29.1 LBS -----Bottom Chords-----A -C 0.26 119 T 0.00 0.26 Online Plus -- Version 19.0.034 ------Webs-----RUN DATE: 04-AUG-06 A -D 0.28 164 C 0.00 0.28 CSI -Size- ----Lumber----TL Defl -0.05" in A -C L/999 LL Defl -0.02" in A -C L/999 2x 4 SP-#2 0.26 Shear // Grain in D -B 0.26 2x 4 SP-#2 0.14 WB 0.28 2x 4 SP-#2 Plates for each ply each face.

To

5- 0- 0

5- 0- 0

(psf)

40.0

24.0"

PLATING CONFORMS TO TPI. REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y JSI D LOCK 2.0x 4.0 Ctr Ctr 0.38 A LOCK 3.0x 4.0 Ctr Ctr 0.38

REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. 103 C 0.00 0.26 Analysis Conforms To:

FBC2004 OH Loading Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: В Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf BC Dead Load : 5.0 psf Max comp. force 164 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----D-B 0.26

123

Brace truss as follows:

Live

20.0

20.0

0.0

Plus 4 Wind Load Case(s)

1 UBC LL Load Case(s)

React Uplft Size Req'd

Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10

From

0-0-0

0-0-0

Dead

10.0

10.0

20.0

Fc=1.10 Ft=1.10

Lbs In-Sx In-Sx

Hz =

Hz =

1-8

5-8 1-8

1-8 1-8

172

119

1-8

O.C.

TC Cont.

BC Cont.

Loading

TC

BC

Total

Plus

C

В

Spacing

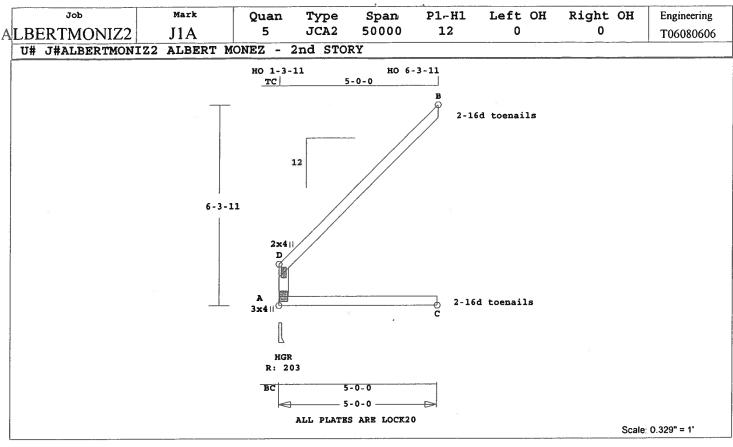
BC Fb=1.10

Lhs

300

131

Date Sealed: 8/7/2006



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 25.0 LBS -----Bottom Chords-----FBC2004 A -C 0.26 119 T 0.00 0.26 Design checked for 10 psf non-Online Plus -- Version 19.0.034 -------Webs----concurrent LL on BC. A -D 0.28 164 C 0.00 0.28 Wind Loads - ANSI / ASCE 7-02 RUN DATE: 04-AUG-06 Truss is designed as a Main CSI -Size- ----Lumber----TL Defl -0.05" in A -C L/999 Wind-Force Resistance System. 0.26 2x 4 SP-#2 LL Defl -0.02" in A -C L/999 Wind Speed: 110 mph Shear // Grain in D -B BC 0.26 2x 4 SP-#2 0.14 Mean Roof Height: 15-0 SP-#2 Exposure Category: WB 0.28 2x 4 В Plates for each ply each face. Occupancy Factor : 1.00 Brace truss as follows: PLATING CONFORMS TO TPI. Building Type: Enclosed REPORT: NER 691 O.C. From To Zone location: Exterior 0- 0- 0 5- 0- 0 ROBBINS ENGINEERING, INC. TC Dead Load : 5.0 psf TC Cont. BC Cont. 0- 0- 0 5- 0- 0 BASED ON SP LUMBER BC Dead Load : 5.0 psf USING GROSS AREA TEST. Max comp. force 164 Lbs Loading Live Dead (psf) Plate - LOCK 20 Ga, Gross Area Quality Control Factor 1.25 Plate - RHS 20 Ga, Gross Area TC 20.0 10.0 Jt Type Plt Size X Y JSI BC 0.0 10.0 D LOCK 2.0x 4.0 Ctr Ctr 0.38 40.0 Total 20.0 20.0 A LOCK 3.0x 4.0 Ctr Ctr 0.38 24.0" Spacing 1.25 Lumber Duration Factor Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 REVIEWED BY: Robbins Engineering, Inc. BC Fb=1.10 Fc=1.10 Ft=1.10 PO Box 280055 Truss Design Engineer: Philip J. O'Regan Tampa, FL 33682 Address: P.O. Box 280055, Tampa, FL 33682 Plus 4 Wind Load Case(s) REFER TO ROBBINS ENG. GENERAL 1 UBC LL Load Case(s) Plus NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

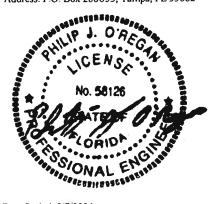
For proper installation of

toe-nails, refer to the 2001

(NDS) for Wood Construction

Trusses Manufactured by: Mayo Truss Co. Inc.

National Design Specification



Date Sealed: 8/7/2006

NOTES:

103 C 0.00 0.26 Analysis Conforms To:

React Uplft Size Req'd

0

13

123

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

Lbs In-Sx In-Sx

Hz =

1-8

Hz =

1-8

3 - 8 1 - 8

172

119

1-8

1-8

Lbs

204

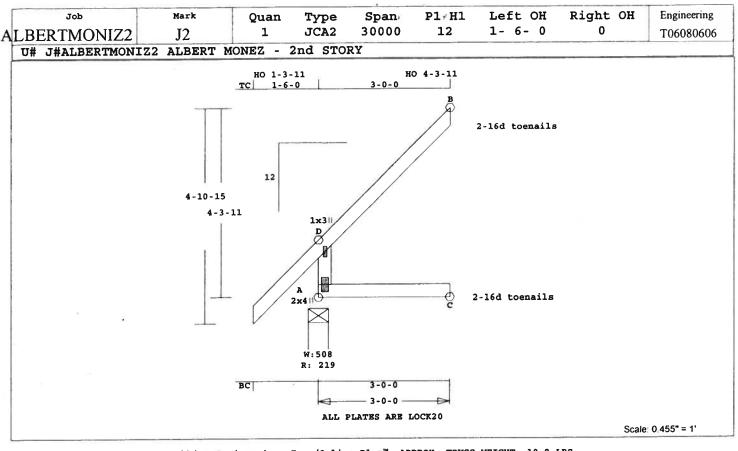
94

131

D-B 0.26

A

C



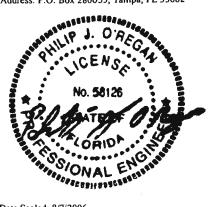
Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 19.9 LBS -----Bottom Chords-----FBC2004 A -C 0.12 77 T 0.00 0.12 OH Loading Online Plus -- Version 19.0.034 -------Webs-----Soffit psf 2.0 A -D 0.13 98 C 0.00 0.13 Design checked for 10 psf non-RUN DATE: 04-AUG-06 concurrent LL on BC. Wind Loads - ANSI / ASCE 7-02 CSI -Size- ----Lumber----TL Defl -0.01" in A -C L/999 SP-#2 LL Defl 0.00" in A -C L/999 Truss is designed as a Main 0.09 2x 4 TC Shear // Grain in D -B BC 0.12 2x 4 SP-#2 0.08 Wind-Force Resistance System. 110 mph Wind Speed: 0.13 2x 4SP-#2 Plates for each ply each face. Mean Roof Height: 15-0 PLATING CONFORMS TO TPI. Exposure Category: Brace truss as follows: REPORT: NER 691 Occupancy Factor : 1.00 To o.c. From 0-0-0 3-0-0 ROBBINS ENGINEERING, INC. Building Type: Enclosed TC Cont. 0- 0- 0 3- 0- 0 BASED ON SP LUMBER Zone location: Exterior BC Cont. USING GROSS AREA TEST. TC Dead Load : 5.0 psf 5.0 psf Live Dead (psf) Plate - LOCK 20 Ga, Gross Area BC Dead Load : Loading Plate - RHS 20 Ga, Gross Area Max comp. force 98 Lbs 10.0 TC 20.0 Jt Type Plt Size X Y JSI Quality Control Factor 1.25 10.0 BC 0.0 D LOCK 1.0x 3.0 Ctr Ctr 0.75 20.0 20.0 40.0 Total A LOCK 2.0x 4.0 Ctr Ctr 0.38 24.0" Spacing 1.25 Lumber Duration Factor Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 REVIEWED BY: Robbins Engineering, Inc. BC Fb=1.10 Fc=1.10 Ft=1.10 PO Box 280055 Truss Design Engineer: Philip J. O'Regan Tampa, FL 33682 License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682 Plus 4 Wind Load Case(s)

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

#### NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To:



111

78

1-8

1-8

66 C 0.00 0.09

1 UBC LL Load Case(s)

Lbs In-Sx In-Sx

Hz =

1-8

Hz =

81 1-8

0 5-8 1-8

React Uplft Size Req'd

16

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

Lbs

220

55

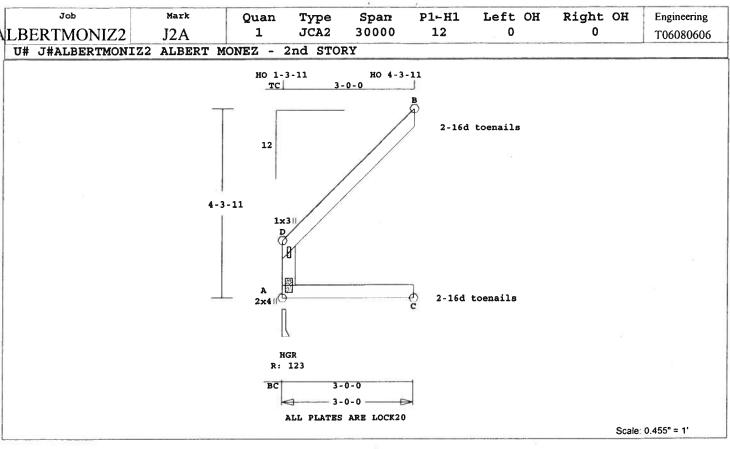
77

D -B 0.09

Α

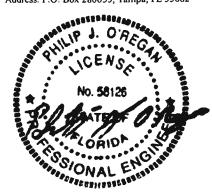
С

В



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.9 LBS -----Bottom Chords-----FBC2004 77 T 0.00 0.12 A -C 0.12 Design checked for 10 psf non-Online Plus -- Version 19.0.034 -------Webs-----concurrent LL on BC. 98 C 0.00 0.13 RUN DATE: 04-AUG-06 A -D 0.13 Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main CSI -Size- ----Lumber----TL Defl -0.01" in A -C L/999 Wind-Force Resistance System. TC 0.09 2x 4 SP-#2 LL Defl 0.00" in A -C L/999 Wind Speed: 110 mph Shear // Grain in D -B BC 0.12 2x 4 SP-#2 0.08 Mean Roof Height: 15-0 SP-#2 2x 4 Exposure Category: WB 0.13 Plates for each ply each face. Occupancy Factor : 1.00 Brace truss as follows: PLATING CONFORMS TO TPI. Building Type: Enclosed To REPORT: NER 691 O.C. From Zone location: Exterior 0- 0- 0 3- 0- 0 ROBBINS ENGINEERING, INC. TC Cont. TC Dead Load : 5.0 psf BC Cont. 0- 0- 0 3- 0- 0 BASED ON SP LUMBER BC Dead Load : 5.0 psf USING GROSS AREA TEST. Max comp. force 98 Lbs Loading Live Dead (psf) Plate - LOCK 20 Ga, Gross Area Quality Control Factor 1.25 Plate - RHS 20 Ga, Gross Area TC 20.0 10.0 Jt Type Plt Size X Y JSI 10.0 BC 0.0 D LOCK 1.0x 3.0 Ctr Ctr 0.75 20.0 20.0 40.0 Total 24.0" A LOCK 2.0x 4.0 Ctr Ctr 0.38 Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 REVIEWED BY: BC Fb=1.10 Fc=1.10 Ft=1.10 Robbins Engineering, Inc. PO Box 280055 Truss Design Engineer: Philip J. O'Regan Tampa, FL 33682 Plus 4 Wind Load Case(s) Plus 1 UBC LL Load Case(s) REFER TO ROBBINS ENG. GENERAL

License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



Membr CSI P Lbs Axl-CSI-Bnd

React Uplft Size Req'd

0

Lbs In-Sx In-Sx

Hz =

1-8

Hz =

1-8

3-8 1-8

Lbs

124

55

77

A

C

В

-----Top Chords-----D-B 0.09 66 C 0.00 0.09

81

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To:

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

For proper installation of

toe-nails, refer to the 2001

(NDS) for Wood Construction

National Design Specification

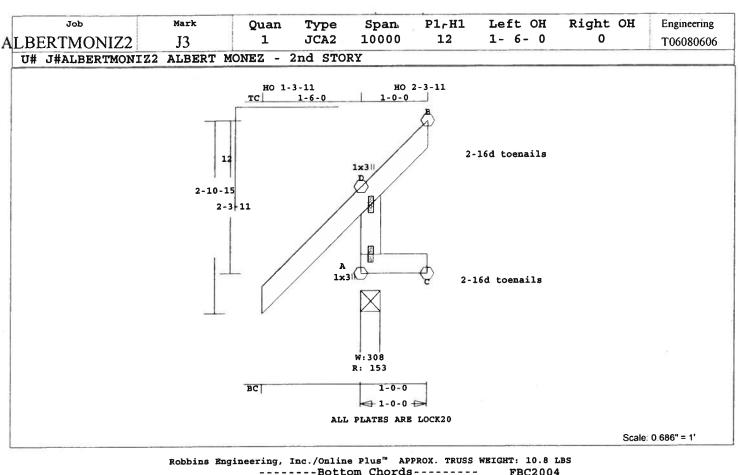
111

78

NOTES:

1-8

1-8



-----Bottom Chords-----FBC2004 A -C 0.03 35 T 0.00 0.03 OH Loading Online Plus -- Version 19.0.034 -------Webs-----Soffit psf 2.0 A -D 0.03 33 C 0.00 0.03 RUN DATE: 04-AUG-06 Design checked for 10 psf nonconcurrent LL on BC. 0.00" in A -C L/999 Wind Loads - ANSI / ASCE 7-02 CSI -Size- ----Lumber----TL Defl 2x 4 SP-#2 0.00" in A -C L/999 TC 0.02 LL Defl Truss is designed as a Main Shear // Grain in D -B Wind-Force Resistance System. 0.03 BC 0.03 2x 4 SP-#2 110 mph SP-#2 Wind Speed: 2x 4 WB 0.03 Plates for each ply each face. Mean Roof Height: 15-0 PLATING CONFORMS TO TPI. Brace truss as follows: Exposure Category: REPORT: NER 691 To Occupancy Factor : 1.00 0.C. From 0- 0- 0 1- 0- 0 ROBBINS ENGINEERING, INC. Building Type: Enclosed TC Cont. Zone location: Exterior BC Cont. 0- 0- 0 1- 0- 0 BASED ON SP LUMBER USING GROSS AREA TEST. TC Dead Load : 5.0 psf Plate - LOCK 20 Ga, Gross Area 5.0 psf BC Dead Load : Loading Live Dead (psf) Plate - RHS 20 Ga, Gross Area Max comp. force 33 Lbs 10.0 TC 20.0 Jt Type Plt Size X Y JSI 10.0 Quality Control Factor 1.25 0.0 BC 20.0 40.0 D LOCK 1.0x 3.0 Ctr Ctr 0.75 20.0 Total A LOCK 1.0x 3.0 Ctr Ctr 0.75 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 REVIEWED BY: BC Fb=1.10 Fc=1.10 Ft=1.10 Robbins Engineering, Inc. PO Box 280055 Truss Design Engineer: Philip J. O'Regan Tampa, FL 33682 Plus 4 Wind Load Case(s)

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To:

License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



32 C 0.00 0.02

1-8

1-8

1-8

50

36

Plus 1 UBC LL Load Case(s)

Lbs

154

15

22

D -B 0.02

A

C

В

React Uplft Size Req'd

0

42

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----

Lbs In-Sx In-Sx

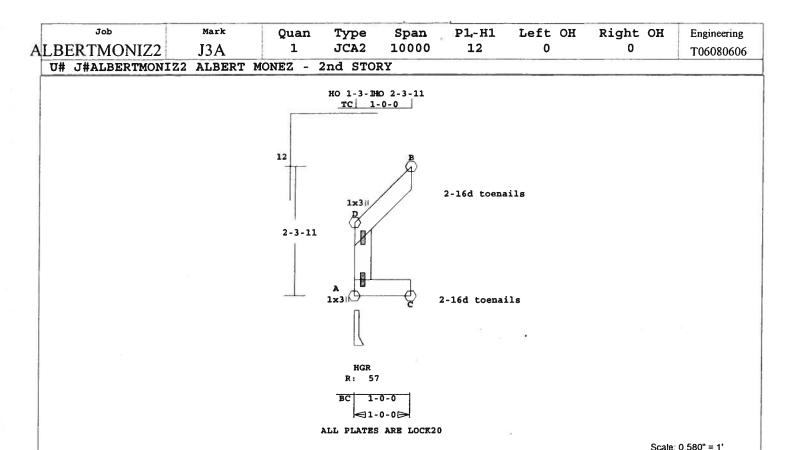
3 - 8

Hz =

1- 8

Hz =

1-8



Robbins Engineering, Inc./Online Plus APPROX. TRUSS WEIGHT: 6.8 LBS -----Bottom Chords-----FBC2004 A -C 0.03 35 T 0.00 0.03

RUN DATE: 04-AUG-06 A -D 0.03

CSI -Size- ----Lumber----TL Defl SP-#2 LL Defl TC 0.02 2x 4 0.03 2x 4 SP-#2 0.03 2x 4 SP-#2

Brace truss as follows: To O.C. From 0- 0- 0 1- 0- 0 TC Cont. BC Cont. 0- 0- 0 1- 0- 0

(psf) Loading Live Dead 20.0 10.0 TC BC 0.0 10.0 Total 20.0 20.0 40.0 24.0" Spacing Lumber Duration Factor 1.25 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s) Plus 1 UBC LL Load Case(s)

React Uplft Size Req'd Lbs In-Sx In-Sx Lbs 3-81-8 58 A 50 Hz =C 15 1-81-8 Hz =36 22 42 1-8 1-8 В

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----D-B 0.02

0.00" in A -C L/999 0.00" in A -C L/999 0.03 Shear // Grain in D -B

Plates for each ply each face.

PLATING CONFORMS TO TPI.

REPORT: NER 691 ROBBINS ENGINEERING, INC. BASED ON SP LUMBER USING GROSS AREA TEST. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y 1.0x 3.0 Ctr Ctr 0.75 D LOCK A LOCK 1.0x 3.0 Ctr Ctr 0.75

Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

REVIEWED BY:

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

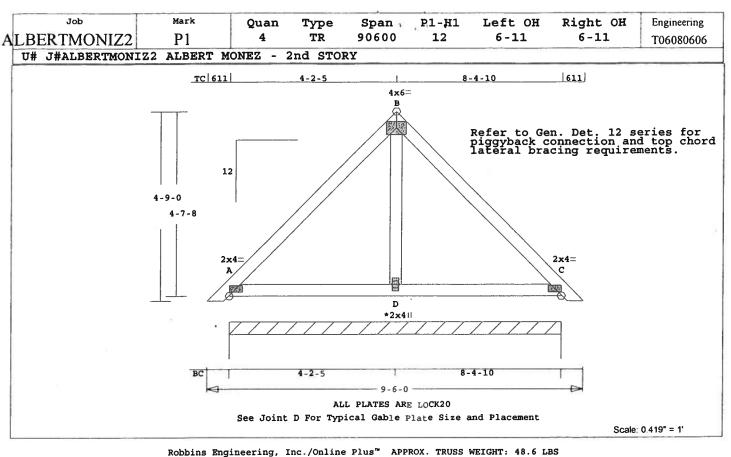
NOTES:

Trusses Manufactured by: Mayo Truss Co. Inc. 32 C 0.00 0.02 Analysis Conforms To:

Design checked for 10 psf nonconcurrent LL on BC. 33 C 0.00 0.03 Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load: BC Dead Load : 5.0 psf Max comp. force 33 Lbs Quality Control Factor 1.25

> Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682





D -C 0.11 0 T 0.00 0.11
------Gable Webs----Online Plus -- Version 19.0.034 D -B 0.02 93 C
RUN DATE: 04-AUG-06
TL Defl -0.01" in A -D L/999
CSI -Size- ----Lumber---- LL Defl 0.00" in A -D L/999

0.11 2x 4 SP-#2

Lumber Duration Factor 1.25

Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10

4 Wind Load Case(s)

React Uplft Size Req'd

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----

212 C

-----Bottom Chords-----

0- 0- 0 to

102 Hz =

Lbs In-Sx In-Sx

212 C 0.00 0.11

0 T 0.00 0.11

Plus 1 UBC LL Load Case(s)

Lbs

742

Cont. Brq

A -B 0.11

B -C 0.11

A -D 0.11

SP-#2

 $0.11 \ 2x \ 4$ 

TC

Spacing

0.02 2x 4 SP-#2 Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 Brace truss as follows: ROBBINS ENGINEERING, INC. O.C. From TO 0-0-0 9- 6- 0 BASED ON SP LUMBER TC Cont. 0- 0- 0 9-6-0 USING GROSS AREA TEST. BC Cont. Plate - LOCK 20 Ga, Gross Area Plate - RHS 20 Ga, Gross Area Loading Live Dead (psf) Jt Type Plt Size X Y JSI 20.0 10.0 TC BC 0.0 10.0 A LOCK 2.0x 4.0 0.4 0.3 0.64 20.0 20.0 40.0 B LOCK 4.0x 6.0 Ctr-0.1 0.37 Total

24.0"

D LOCK 2.0x 4.0 Ctr Ctr 0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055

Tampa, FL 33682

C LOCK 2.0x 4.0-0.4 0.3 0.64

Shear // Grain in A -B

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-

Refer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02 Truss is designed as a Main Wind-Force Resistance System. Wind Speed: 110 mph Mean Roof Height: 15-0 В Exposure Category: Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior 5.0 psf TC Dead Load : 5.0 psf BC Dead Load : Max comp. force 212 Lbs Quality Control Factor 1.25

concurrent LL on BC.

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



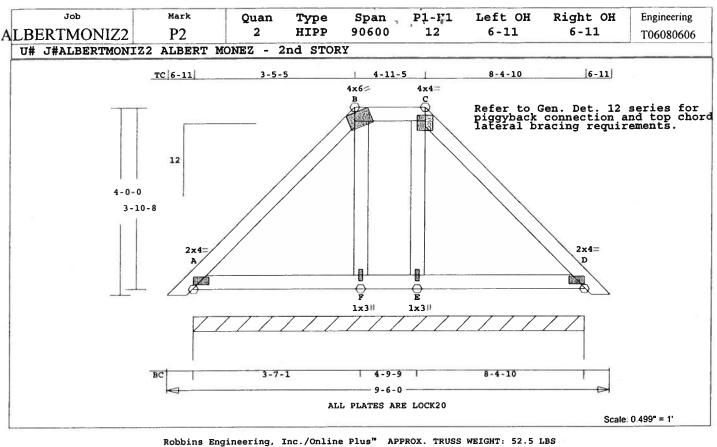
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0.00 0.11

8- 4-10

94

Date Sealed: 8/7/2006



A -F 0 T 0.05 0.00 0.05 Analysis Conforms To: F-E 0.05 0 T 0.00 0.05 FBC2004 Online Plus -- Version 19.0.034 E -D 0.05 0 T 0.00 0.05 OH Loading RUN DATE: 04-AUG-06 Soffit psf 2.0 Webs--Design checked for 10 psf non-F-B 0.00 64 T CSI -Size- ----Lumber----E -C 0.00 64 T concurrent LL on BC. 2x 4 SP-#2 Refer to Gen Det 3 series for 0.08 TL Defl -0.01" in E -D L/999 BC 0.05 2x 4 SP-#2 web bracing and plating. 0.00" in E -D L/999 0.00 2x 4 SP-#2 LL Defl Wind Loads - ANSI / ASCE 7-02 Shear // Grain in A -B 0.09 Truss is designed as a Main Brace truss as follows: Wind-Force Resistance System. Plates for each ply each face. Wind Speed: 110 mph 0.C. From To PLATING CONFORMS TO TPI. 9-6-0 TC Cont. 0-0-0 Mean Roof Height: 15-0 0-0-0 9-6-0 REPORT: NER 691 Exposure Category: B BC Cont. ROBBINS ENGINEERING, INC. Occupancy Factor : 1.00 Loading Live Dead (psf) BASED ON SP LUMBER Building Type: Enclosed USING GROSS AREA TEST. 20.0 10.0 Zone location: Exterior TC Plate - LOCK 20 Ga, Gross Area 0.0 10.0 TC Dead Load : 5.0 psf BC 20.0 40.0 Plate - RHS 20 Ga, Gross Area BC Dead Load : 5.0 psf 20.0 Total Plt Size X Y JSI Spacing 24.0" Jt Type Max comp. force 228 Lbs Lumber Duration Factor 1.25 A LOCK 2.0x 4.0 0.4 0.3 0.64 Quality Control Factor 1.25

4.0x 6.0 1.2-3.0 0.37

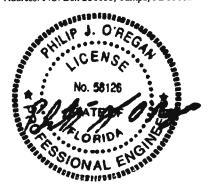
4.0x 4.0 Ctr-0.5 0.86

2.0x 4.0-0.4 0.3 0.64

1.0x 3.0 Ctr Ctr 0.75

1.0x 3.0 Ctr Ctr 0.75

Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

B LOCK

R LOCK

C

D LOCK

F

LOCK

LOCK

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc.

A -B 0.08 228 C 0.00 0.08 B -C 0.03 156 C 0.00 0.08 C -D 0.08 228 C 0.00 0.08

-----Bottom Chords-----

Membr CSI P Lbs Ax1-CSI-Bnd

Plate Duration Factor

Plus 6 Wind Load Case(s)

Lbs

Cont. Brg 742

Plus 1 UBC LL Load Case(s)

React Uplft Size Req'd

102 Hz =

Lbs In-Sx In-Sx

0- 0- 0 to 8- 4-10

TC Fb=1.15 Fc=1.10 Ft=1.10

BC Fb=1.10 Fc=1.10 Ft=1.10

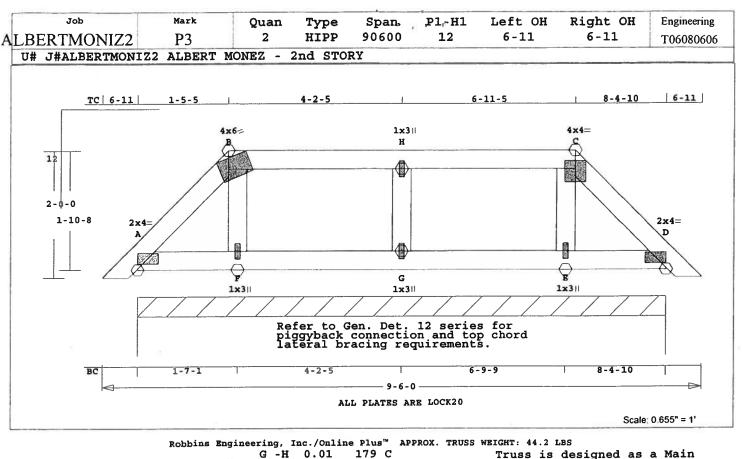
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1.25

78

Date Sealed: 8/7/2006



G -H 0.01 179 C E -C 0.00 78 C Online Plus -- Version 19.0.034 0.00" in F -G L/999 RUN DATE: 04-AUG-06 TL Defl LL Defl 0.00" in F -G Shear // Grain in B -H L/999 CSI -Size- ----Lumber----0.11 TC 0.07 2x 4 SP-#2 Plates for each ply each face. BC 0.03 2x 4 SP-#2 2x 4 SP-#2 PLATING CONFORMS TO TPI. 0.01 REPORT: NER 691 Brace truss as follows: ROBBINS ENGINEERING, INC. BASED ON SP LUMBER To O.C. From 9- 6- 0 USING GROSS AREA TEST. TC Cont. 0- 0- 0 0-0-0 9-6-0 Plate - LOCK 20 Ga, Gross Area BC Cont. Plate - RHS 20 Ga, Gross Area Jt Type Plt Size X Y Loading Live Dead (psf) A LOCK 2.0x 4.0 0.4 0.3 0.64 10.0 TC 20.0 BC 0.0 10.0 B LOCK 4.0x 6.0 1.2-3.0 0.37 H LOCK 1.0x 3.0 Ctr Ctr 0.75 C LOCK 4.0x 4.0 Ctr-0.5 0.86 40.0 20.0 20.0 Total 24.0" Spacing Lumber Duration Factor 1.25 D LOCK 2.0x 4.0-0.4 0.3 0.64 F LOCK 1.0x 3.0 Ctr Ctr 0.75 Plate Duration Factor 1.25 TC Fb=1.15 Fc=1.10 Ft=1.10 BC Fb=1.10 Fc=1.10 Ft=1.10 LOCK G 1.0x 3.0 Ctr Ctr 0.75 E LOCK 1.0x 3.0 Ctr Ctr 0.75

Truss is designed as a Main Wind-Force Resistance System. 110 mph Wind Speed: Mean Roof Height: 15-0 Exposure Category: B Occupancy Factor : 1.00 Building Type: Enclosed Zone location: Exterior TC Dead Load : 5.0 psf 5.0 psf BC Dead Load : Max comp. force 179 Lbs Quality Control Factor 1.25

Plus 6 Wind Load Case(s) Plus 1 UBC LL Load Case(s)

Jt React Uplft Size Req'd Lbs Lbs In-Sx In-Sx Cont. Brg 0- 0- 0 to 8- 4-10 742 102 Hz = 31

Membr CSI P Lbs Ax1-CSI-Bnd -----Top Chords-----A -B 0.00 52 C B -H 0.07 39 T 0.00 0.07 39 T 0.00 0.07 H -C 0.07 C -D 0.00 52 C -----Bottom Chords-----0 T 0.00 0.02 A-F 0.02 0 T 0.00 0.03 F-G 0.03 0 T 0.00 0.03 0 T 0.00 0.02 G -E 0.03 E -D 0.02

------Webs-----

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F-B 0.00 78 C

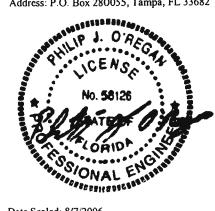
REVIEWED BY: Robbins Engineering, Inc. PO Box 280055 Tampa, FL 33682

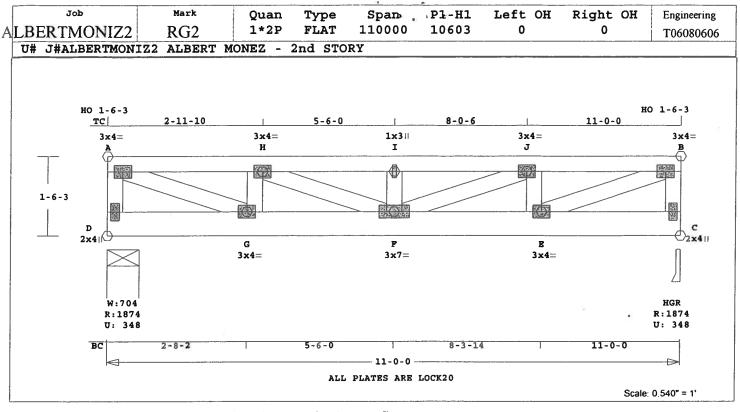
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES: Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 OH Loading

Soffit psf 2.0 Design checked for 10 psf nonconcurrent LL on BC. Refer to Gen Det 3 series for web bracing and plating. Wind Loads - ANSI / ASCE 7-02

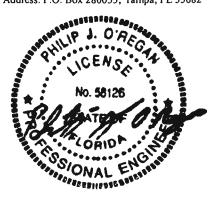
Truss Design Engineer: Philip J. O'Regan License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 78.1 LBS F -E 0.37 3179 T 0.21 0.16 E -C 0.13 24 T 0.00 0.13 2 COMPLETE TRUSSES REQUIRED. Fasten together in staggered pattern. (1/2" bolts -OR-Online Plus -- Version 19.0.034 -Webs----1448 C WindLd SDS3 screws -OR- 10d nails RUN DATE: 04-AUG-06 D-A 0.06 A -G 0.31 3455 T as each layer is applied.) \* 2-Ply Truss \* G -H 0.02 574 C ----Spacing (In) ---0.09 1044 H-F Rows Nails Screws Bolts -I 0.00 141 TC 1 12 24 0 CSI -Size- ----Lumber-----J 0.09 1044 T BC 2 12 24 0 0.11 2x 4 SP-#2 E -J 574 C TC 0.02 WR Я 1 R SP-#2 3455 Design checked for 10 psf non-BC 0.37 2x 6 E -B 0.31 Т 1448 C WindLd C -B 0.06 WB 0.31 2x 4 SP-#2 concurrent LL on BC. Provide drainage to prevent TL Defl -0.08" in G -F LL Defl -0.04" in G -F Shear // Grain in G -F L/999 Brace truss as follows: water ponding. To o.c. From L/999 Use properly rated hangers for 0- 0- 0 11- 0- 0 TC Cont. 0.17 loads framing into girder 0- 0- 0 11- 0- 0 BC Cont. truss. Plates for each ply each face. PLATING CONFORMS TO TPI. REPORT: NER 691 This truss must be installed Loading Live Dead (psf) as shown. It cannot be TC 20.0 10.0 installed upside-down. BC 0.0 10.0 ROBBINS ENGINEERING, INC. Wind Loads - ANSI / ASCE 7-02 BASED ON SP LUMBER Total 20.0 20.0 40.0 Truss is designed as a Main USING GROSS AREA TEST. 24.0" Spacing Wind-Force Resistance System. Plate - LOCK 20 Ga, Gross Area Lumber Duration Factor 1.25 Wind Speed: 110 mph Plate - RHS 20 Ga, Gross Area Plate Duration Factor 1.25 Mean Roof Height: 15-0 Plt Size X Y Exposure Category: Occupancy Factor : TC Fb=1.00 Fc=1.00 Ft=1.00 Jt Type JSI 3.0x 4.0 Ctr Ctr 0.97 : 1.00 BC Fb=1.00 Fc=1.00 Ft=1.00 LOCK 3.0x 4.0 Ctr Ctr 0.41 Building Type: Enclosed Zone location: Exterior H LOCK Load Case # 1 Girder Loading LOCK 1.0x 3.0 Ctr Ctr 0.75 Lumber Duration Factor 1.25 J LOCK 3.0x 4.0 Ctr Ctr 0.41 TC Dead Load : 5.0 psf Plate Duration Factor 1.25 В LOCK 3.0x 4.0 Ctr Ctr 0.97 BC Dead Load : 5.0 psf plf - Live Dead From To D LOCK 2.0x 4.0 Ctr Ctr 0.45 Max comp. force 4140 Lbs TC V 40 20 0.0' 11.0' G LOCK 3.0x 4.0 Ctr Ctr 0.97 Quality Control Factor 1.25 BC V 130 150 0.0' 11.0' F LOCK 3.0x 7.0 Ctr Ctr 0.42 E LOCK 3.0x 4.0 Ctr Ctr 0.97 Truss Design Engineer: Philip J. O'Regan LOCK 2.0x 4.0 Ctr Ctr 0.45 Plus 6 Wind Load Case(s) 1 UBC LL Load Case(s)

License #: 58126 Address: P.O. Box 280055, Tampa, FL 33682



Span

REVIEWED BY:

PO Box 280055

Tampa, FL 33682

Trusses Manufactured by: Mayo Truss Co. Inc. Analysis Conforms To: FBC2004 Girder Common Loading BC

15- 0- 8

Robbins Engineering, Inc.

REFER TO ROBBINS ENG. GENERAL

NOTES AND SYMBOLS SHEET FOR

ADDITIONAL SPECIFICATIONS.

0.08

React Uplft

348

348

Membr CSI P Lbs Axl-CSI-Bnd -----Top Chords-----

4140 C

4140 C

3179 C

--Bottom Chords----

24 T

Lbs

1875

1875

0.08

0.11

0.11

0.13

0.37

0.08

D

C

H -I

D-G

-J

Size Req'd

7-41-8

-32

1- 8

0.03 0.08

0.02 0.06

Lbs In-Sx In-Sx

Hz =

3-8

Hz =

3179 C 0.02 0.06

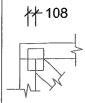
0.03

0.00

3179 T 0.21 0.16

# ROBBINS ENG. GENERAL NOTES & SYMBOLS

### PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

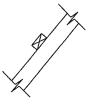
## FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)



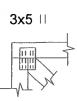
(W) = Wide Face Plate(N) = Narrow Face Plate

# LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.



#### PLATE SIZE AND ORIENTATION

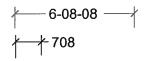


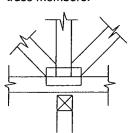
Trussed Rafters.

The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

#### **DIMENSIONS**

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX) R = Reaction (lbs.) U = Uplift (lbs.)

#### **BEARING**

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with "National Design Specifications for Wood Construction" (AF & PA )," National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.



6904 Parke East Blvd. Tampa, FI 33610-4115 Tel: 813-972-1135 Fax: 813-971-6117

www.robbinseng.com

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BUILDING DESIGN.

2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE
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3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.

4. SHIM ALL BEARINGS FOR FULL CONTACT.

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To not cut, notich, drill or after LP LVL and CTR. LP I-Joists except as shown in published material from LP any use of LP LVL and CTR. LP I-Joists commany to the limits set forth hereon equals any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness. \*A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP LVL and CTR, LP I-Joist Specifications OAD FROM TO FT-IN-SX FT-IN-SX 64 PLF 00-00-00 01-11-00 24 PLF 00-00-00 01-11-00 LP is a registered trademark of Louisiana-Pacific Corporation and connections for LP LVL and CTR, LP i-Joists to be specific applications, mails driven parellel to glue lines shall be spaced a minimum of 4" for 10d OTHER LOAD CASES ۳ \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* БAO 1 LPI 20Plus DEPTH 11.875 " WEB: 0.375" FLANGE 1.50 X 2.50 0.90 3d1 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wats 800.999.9105 DWG SHEET LP Engineered Wood Products N.Y. CITY DEAD LOAD TOTAL LOAD DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: SPACING = DESIGN CRITERIA : WISCONSIN CODE COMPLIANCES S3-23 3 of 42 0608-049 90/1/90 REPORT # ESR-1305 200405-W 12724-R MEA 101-00-E SBCCI гг // 55 WSI: 0.01 VSI: 0.05 480 240 2006.1 354 354

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Handling & Erection  Temporary and permanent bracing for holding component The purits and for resisting siteral lorges shall be designed and omega installed by defent. No leads are to be applied to the component until after all the framing and fastening are completed. An orine shall leads greater than design based code in the applied to the component.  Design Criteria  The design and material specified are in substantial based and definication installed and in Statement's with component to the specified are in substantial with the latest revisions of NDS and ATC.  The design and material specified are in substantial substantial lead definication installed sequences of the property of the prope	SUPPORT REACTIONS (LBS):  HAXINUM B E A R I N G N U M B E R  LOAN 612 612  DOWN 612 612  UPLIFT  HANINUM DEFLECTIONS  CALCULATED ALLOWABLE  LIVE LOAD 0.10*  TOTAL LOAD 0.24*  0.34"  1000	NOTE: 2006.1 Allowable Stress Design  1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERRIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO NICORPORATE THIS COMPONENT INTO THE BUILDING DESIGN. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE 1. ASHIM ALL BEARINGS FOR FULL CONTACT. 3. VERIFY DIMENSIONS BEFORE CUTTING LET TO SIZE. 6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY. 7. PROVIDE COMPRESSION EOGE BRACING AT APT O.C. OR LESS. AN
Miscelaneous information  The use of this component shall be specified by the designer of the complete structure. Obtain at the necessary code completes structure. Obtain at the necessary code completes structure before using this instructions from the design offensis listed above does not may be calculated to component. If the design offensis listed above does not may be calculated on component. If the design offensis listed above does not may be calculated and seasons of the survey of the seasons. The Armond is the drawing based on data provided by the customer. UP LV, and CTR, UP I-jobst on made without camboe and will defined under load. Wood in direct contact with concrete must be profesced as required by code. Confinensis steril support is essured (wait, floor beam, etc.) LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal afficial to be considered an Engineering document.	11.875 D CROSS SECTION	NOTE: LOADS SHOWN ARE FOR INPUT LOAD TO NOTE: LOADS SHOWN ARE FOR INPUT LOAD FOR PATTERN LIVE LOADING ARE CHECKED A (DIMENSIONS MEASURED FROM LEFT END OF TRIBUTION SOURCE TYPE TOP/SIDE LOADING WARE FLOOR LIVE TOP FORM FLOOR DEAD TOP WANING NOTES:  WANING NOTES:
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LP Engineered Wood Products auritoos SBCCI 2001 2706 Highway 421 North Withington, NC 28401 Local 910,782,9878 National Wats 800,999,9105  DWG # 0608-049  SHEET # 6 of 42	ALE	DESIGN CRITERIA: WS: 0.63  LIVE LOAD

File: L'Engineering\Woode 2008/August\0808-048\6036-15.SPX	p. Inspection. This drawing must have an Architects or Engineer's seal affixed to be considered an Engineering document.	Handing & Erection    Handing & Erection   Hiscoellaneous Information   The use of \$15 component state the specified by the designer of the plants and for resisting lateral forces shall be designed and component until after all the framings and fastening are component until after all the framings and fastening are component until after all the framings and fastening are component. If the design orders lated above does not meet local building component. If the design orders lated above does not meet local building to component, and the specified to the component. If the design or design and fastening are period to the component. If the design and fastening are period to the component. If the design and fastening are period to the component. If the design and fastening are period to the component. If the design and fastening are period to the component and component. If the design and fastening are period to the component and component and component. If the design are period to the component and component and component and component and component. If the design are period to the component and component and component and component. If the design are period to the component and	LCULARED ALLOWABLE  0.03" 0.20"  0.04" 0.39"	SUPPORT REACTIONS (LBS):  MAXIMUM B E A R I N G N U M B E R  DOWN 367 367  UPLIFT  WIN BEARING SIZES (IN-SX)  MAXIMUM DEFICITIONS  MAXIMUM DEFICITIONS	6. THIS IP IS 10 BE USED AS A FLOOR JOIS I ONLY. JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. 7. PROVIDE COMPRESSION EDGE BRACING AT JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. 88° O.C. OR LESS. ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.  ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.	2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE  THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS.  LATERAL STABILITY.  USE OF THIS DESIGN FOR ANYTHING OTHER THAN L-P LVL OR L-P I-JOISTS IS  STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW  4. SHIM ALL BEARINGS FOR FULL CONTACT.  BY A DESIGN PROFESSIONAL  5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.	PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL UNIFORM FLOOR LIVE TOP RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS UNIFORM FLOOR DEAD TOP OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.	FOR PATTERN LIVE LOADING ARE CHECKED A COMENSIONS MEASURED FROM LEFT END OF TRIBUTION SOURCE TYPE TOP/SIDE LO	
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8 of 42	cific Corporation.	LP to a registered tradement of Louisiana-Pacific Corporation.	on, ins crawing must have an Alciment a or Englance a seal amed exidered an Engineering document.	Dead load deflection includes adjustment factor for creep. Inspects to be co
0608-049	TO THE INSTALLING CONTRACTOR DWG #	• A COPY OF THIS DRAWING IS TO BE GIVE	crets must be protected as required by code. Continuous lateral is assumed (wall, floor beam, etc.). LP does not provide on-site is assumed (wall, floor beam, etc.).	
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WISCONSIN 200405-W COMC 12724-R N.Y. CITY MEA 101-00-E		VENT CRUSHING OF THE LPI THE PROJECT ENGINEER, UPPORT STRUCTURE FOR THIS NS.	MINIMUM BEJANING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPY JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.	6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY. 7. PROVIDE COMPRESSION EDGE BRACING AT 48" O.C. OR LESS.
CODE COMPLIANCES:  REPORT #  ICC-ES ESR-1305		HIS DOCUMENT REQUIRES REVIEW	USE OF THIS DESIGN FOR ANY MINIOUS TIMEN THAN LIFTCH ON LIFTCH PROHIBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL	LATERAL STABILITY.  3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.  4. SHIM ALL BEARINGS FOR FULL CONTACT.  5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
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4. SHIM ALL BEARINGS FOR FULL CONTACT.

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6. THISLP IS TO BE USED AS A FLOOR JOIST ONLY.

7. PROVIDE COMPRESSION EDGE BRACING AT 3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE DOWN MIN BEARING SIZES (IN-SX) ding & Erection LOAD 2006.1 Allowable Stress Design MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE AD 0.25" 0.37" AD 0.14" AD 0.35" 0.74" NUMBER The use of this component shall be specified by the d complete structure. Obtain all the necessary code correct structures. Obtain all the necessary code corrections from the designer of the complete struct component. If the design critical lated above does in total code requirements, do not use this design. Where this code requirements, do not use this design, where the safe conductive structure designs its approved as show the season on data provided by the outstoner. LP LVL min. Miscellaneous Information DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD UNI FORM UNI FORM THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNED TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS. NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REOURED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) FLOOR DEAD CROSS SECTION gg gg LOAD TABLE Do not cut, acitch, drill or after LP LVI, and CTR, LP Llosts except as shown in
published material from LP any use of LP LVIL and CTR, LP Licities contrary to the
limits set forth hereout, negates any express warranty of the product and LP disclaims
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NER ICBO L.A. City WISCONSIN N.Y. CITY CCMC		L-P ENGINEERED WOOD PRODUCTS. N LP LVL OR LP I-JOISTS IS HIS DOCUMENT REQUIRES REVIEW	THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL	6. VERIFY DIMENSIONS BEFORE CUTTING LP TO SIZE.  6. THIS LP IS TO BE USED AS A FLOOR JOIST ONLY.  7. PROVIDE COMPRESSION EDGE BRACING AT  EACH END OF COMPONENT.
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49	DWG # 0608-049	TO THE DISTALLING CONTRACTOR	for a particular use.	ed contact ous lateral de on-site	
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1 1 1 1 55 PSF	TOTAL LOAD	FLANGE 1.50 X 2.60	CASE (1). OTHER LOAD CASES AS REQUIRED. SPAN OR CANTILEVER.)	NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD CASES FOR PATTERN LYE LOADING ARE CHECKED AS REQUIRED.  (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)	THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERIFICATION OF LOADING, DEFLECTION
vs:		1 LPI 20Plus DEPTH 11.876"	ABLE		OTE: 2006.1 Allowable Stress Design
us: 072		Mayo iruss	FL Mayo		lonez Res. / Joist-3

File: L:Engineering\Woode 2006\August\0608-0496036-6.SPX	No loads are to be applied to the face all the harming and fastering are than design loads thine shall loads greater than design loads component.  Somponent are the face of	Handling & Erection  Temporary and permanent bracing for holding component  The use of plumb and for resisting lateral forces shall be designed and complete a	SUPPORT REACTIONS (LBS): MAXIMUM B E A R I N G N U M B E R MAXIMUM 638 638 UPLIFT  MIN BEARING SIZES (IN-SX)  J- 8 J- 8  MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE LIVE LOAD 0.12" TOTAL LOAD 0.28" 0.35" TOTAL LOAD 0.28" 0.70"	NOTE: LOAD-IN ANOMERO BUSINESS DESIGNED TO SUPPORT ONLY 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL, BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OF ARCHITECT, I DISCLAIM ALL INTERPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY. 3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES. 4. SHIM ALL BEABNOS FOR FULL CONTACT, 5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE 6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY. 7. PROVIDE COMPRESSION EDGE BRACING AT 39" O.C. OR LESS.  AN
	of this straight of the straig	of the approval and	11.875 CROSS SECTION	NOTE: LOADS SHOWN ARE FOR INPUT LOAD; FOR PATTERN LIVE LOADING ARE CHECKED A (DIMENSIONS MEASURED FROM LEFT END OF FORM FLOOR LIVE TOP FORM FLOOR LIVE TOP FORM FLOOR DEAD TOP FORM FLOOR DEAD TOP FORM FLOOR DEAD TOP RINING NOTES: US COMPONENT DESIGN IS SPECIFICALLY FOR LE EOF THIS DESIGN FOR AMYTHING OTHER THAN RICTLY PROHIBITED. ANY MODIFICATION OF THIS A DESIGNED, IT IS THE RESPONSIBILITY OF FI CHITECT OR DESIGNER TO VERIFY THAT THE SUIS IST AS DESIGNED. OF SUPPORTING THE REACTIONS CHORLE OF SUPPORTING THE REACTIONS CHORLES OF HAME CHORLES OF THE PROMISE OF THE PR
LP is a registered trademark of Louisiana-Pacific Corporation.	as shown in southery to the and LP disclaims billing and fitness		THIS DRAWNIG IS NOT TO SCALE	AD CASE I.  AD CASE I.  AD CASE I.  AD CASE II.  AND CASE II.  OF SPAN OR CANTILEVER.)  LOAD FROM TO LOAD LDF  TT-IN-SX ET-IN-SX  64 PLF 00-00-00 14-06-00  24 PLF 00-00-00 14-06-00  24 PLF 00-00-00 14-06-00  75 PLANGETER WOOD PRODUCTS.  AN LP IVI. OR LP LADISTS 15  THIS DOCUMENT REQUIRES REVIEW  REVENT CRUSHING OF THE LPI  THE PROJECT ENGINEER.  SUPPORT STRUCTURE FOR THIS  ONS.  HANGERS.
	Local   910.762.9878   National Wats   800.999.9105	LP Engineered Wood Products outlook SBCCI 2008.1 2708 Highway 421 North Wilmington, NC 28401	ME***	DESIGN CRITERIA: WS: 0.46  LIVE LOAD

A CONNECT DOUBLE LPI USING 2X6 + 58° OSB
FILLERS FULL LENGTH OF JOIST: FOR JOISTS
FHALERS FULL LENGTH OF JOIST: FOR JOISTS
FHAT ARE TOP LOADED ONLY FILLERS NEED ONLY
BE LOCATED AT BACH SUPPORT AND AT 8' ON
CENTER MAX FILLERS MUST BE AT LEAST 4' LONG.
TOP LOADS MUST BE APPLIED TO BOTH HOISTS.
B. PROVIDE 4' LONG FILLERS AT ALL
CONCENTRATED LOADS. CENTER FILLER ON LOAD
C. FASTEN FILLERS THROUGH LPI WEBS WITH
2 ROWS OF 84(2-1/27) MULLS AT 12' OC FROM
BOTH SIDES STAGGER ROWS AND CLINCH
WHERE POSSIBLE. LATERAL STABILITY.

LATERAL STABILITY.

3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.

4. SHIM ALL BEARINGS FOR FULL CONTACT.

5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.

6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.

7. PROVIDE COMPRESSION EDGE BRACING AT Monez Res. / Joist-6 ID. MAXIMUM SIDE-APPLIED LOADS FOR STANDARD
DURATION: UNIFORM LOAD = 250 PLF;
CONCENTRATED LOAD = 1500 LBS. LOADS CAN BE
DOUBLED FOR NAILS AT 6" OC. TRIPLED FOR
NAILS AT 4" OC. ETC., AND MAY BE ADJUSTED
FOR OTHER LOAD DURATIONS. BUILDING DESIGN.

2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE The design and material s conformity with the latest Dead toad deflection inclu SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G
1 2 NOTE FOR DOUBLE LPI 37" O.C. OR LESS. UPLIFT ndling & Erection MIN BEARING SIZES (IN-SX) porary and permanent bracing for holding component b and for resisting internal forces shall be designed and lad by others. No loads are to be expliced to the concent until after all the framing and instending are 1174 2006.1 Allowable Stress Design SHEATHING TO BE NAILED TO FLANGES OF MAXIMUM DEFLECTIONS
CALCULATED ALLOWABLE
AD 0.13" 0.36"
AD 0.28"
AD 0.32" 0.72" 1928 UMBE The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approximately Miscellaneous Information UNI FORM
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CONCENTRATED MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN L-P LVL OR L-P I-JOISTS IS STBCTLY PROMUNTED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. WARNING NOTES: ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS. PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY. IOIST IS CAPABLE OF SUPPORTING THE REACTIONS. NOTE: LOADS SHOWN ARE FOR INPUT (DAD CASE (1), OTHER LOAD OF FOR PAITERN LIVE LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) FLOOR FLOOR FLOOR FLOOR CROSS SECTION inco-approval and \* Continen nails driven parallel to gave time areas or providing his and \* for 8d.

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an LOAD TABLE LOAD 300 800 800 800 LP LVL and CTR, LP I-Joist Specifications \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications common nalls driven parallel to give fines shall be speced a minimum of 4" for 10d AD FROM TO L
FT-IN-SX FT-IN-SX
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40 PLF 00-00-00 14-10-00
15 PLF 00-00-00 14-10-00
90 LBS 12-06-00 MINBRG10 LBS 12-06-00 MINBRG-LP is a registered trademark of Louisiana-Pacific Corporation OTHER LOAD CASES MINBRG-3.50\* MINBRG-3.50\* Mayo Truss \*\* THIS DRAWING IS NOT TO SCALE \*\*\* 2 LPI 20Plus DEPTH 11.878"
NEB: 0.375"
FLANGE 150 X 2.60
DESIGN CONSISTS OF 2 - PLIES FASTENED
TOGETHER (REFER TO NOTES). 0.90 0.90 0.90 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wats 800.989.9105 SHEET DWG LP Engineered Wood Products WISCONSIN CCMC N.Y. CITY DEAD TOTAL DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: SPACING = DESIGN CRITERIA : CODE COMPLIANCES : S3-22 13 of 42 0608-049 LOAD LOAD 08/10/08 12724-R MEA 101-00-E . . . SECCI \$5 \$5 MSI: 0.61 VSI: 0.71 2008.1 PSF PSF

File: L:\Engineering\Woode 2005\August0508-049\S036-8.SPX	Interports and permanent bracks for holding component.  The upon and for resisting lateral locas shall be designed and installed by others. No bads are to be applied to the installed by others. No bads are to be applied to the component until after all the framing and fastering are compeled. At no time shall loads greater than design loads component. If this design criteria issed above does not meet local building compeled. At no time shall loads greater than design loads provided to the component. Design Criteria  Design Criteria  The design and material specified are in substantial conformity with the altest revisions of NDS and Aff.C.*  Dead load deflection includes adjustment sector for creep, load deflection includes adjustment sector for creep, to be considered as Engineer's seal althout to component as filteria.	MAXIMOM DEFLECTIONS LIVE LOAD 0.01" 0.13" *DEAD LOAD 0.00" 0.26"	SUPPORT REACTIONS (LBS):  MAXIMUM B E A R I N G N U M B E R  DOWN 246 246  UPLIFT  MIN BEARING SIZES (IN-SX)  3-8 3-8			8. VERITY DMENSIONS BEFORE CUTTING INTO SIZE 8. THIS LIP IS TO BE USED AS A FLOOR JOIST ONLY. 7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.	2 PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY. 3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES. 4. SHIM ALL BEARINGS FOR FULL CONTACT.	PROJECT ENGINEER OF ARCHITECT. LUSICALM ALL UNI PROJECT ENGINEER OF ARCHITECT. LUSICATIONS UNI RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS UNI OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE RIJI DING DESIGN	OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISI BRACING, AND OTHER LATERAL BRACING THAT IS BRACING THAT IS BRACING THAT IS A THAT IS A THAT IS BRACING THAT IS A THAT IS A THAT IS A THAT IS BRACING THAT IS A THAT IS A THAT IS A THAT IS BRACING THAT IS A THAT IS A THAT IS A THAT IS A THAT IS BRACING THAT IS A THAT IS A THAT IS A THAT IS BRACING THAT IS A THAT IS A THAT IS A THAT IS BRACING THAT IS A	NOTE:  1. THIS COMPONENT IS DESIGNED TO SUPPORT ONL THE VERTICAL LOADS SHOWN AS DETERMINED BY	yist-8
LP is a nigistaned trademark of Louisians-Pacific Corporation	Miscelaneous information  The use of this component shall be specified by the designer of the complete structure. Obtain at the necessary code completes structure before using this component. If the designer of the complete structure before using this component. If the designer of the competent is the design criteria listed showe does not meet local building component. If the design criteria listed showe does not meet local building component. If the design criteria listed showed does not meet local building component. If the design criteria listed showed does not meet local building component in the design showed in this drawing based on the structural design is approved as shown in this drawing based on eat the product and CRR, LP I-Joists and ST (or 8d. of the product and showed day the customer. LP LV. and CRR, LP I-Joists on limits at both hereon, negates any suppress warranty of the product and showed on the product of the product and the product and the product of the product and the pro		11.875  A 2.500  CROSS SECTION	<b>—</b>	ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.	MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.	THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LPLYL, OR LP L-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL	UNIFORM FLOOR LIVE TOP 64 PLF 00-00-00 05-07-00 UNIFORM FLOOR DEAD TOP 24 PLF 00-00-00 05-07-00 WARNING NOTES:	TON TAI LERN LIVE LOALING ARE CHECKED A (DIMENSIONS MEASURED FROM LEFT END OF TRIBUTION SOURCE TYPE TOP/SIDE LC		\rangle \rang
	LP LVL and CTR, LP I-Joist Specifications  LP Engineered Wood Products  *Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.  *Common rails driven parallel to give lines shall be specide a minimum of * for 100  *Bid 3" for 88, and 3" for 88, and 3" for 89, and 5" for 89, and 5" for 100  *Innits set forth hereon negatics any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and filness for a periodar use.  *A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR  *A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR  *A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR  SHEET # 14	5- 7- 0 THIS DRAWING IS NOT TO SCALE ***	8				175.	0.90	LOAD LDF	1 LPI 20Plus DEPTH 11.876" WEB: 0.375" FLANGE 1.50 X 2.80	Mayo Truss
	Products 0810006 SBCCI 2001 105 105 106 108-049				3	ICC-ES ESR-1305 WISCONSIN 200405-W CCMC 12724-R N.Y. CITY MEA 101-00-E	Ŀ	DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: L / 480 L / 240		CRITERIA: WS:	

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Mayo Truss

The design and material specified are in audistantial conformity with the steet revisions of NDS and ATTC irreport Deal toad defrection includes adjustment factor for creep. In the conformation is instantaneous.	Handling & Erection  Temporary and permanent bracing for hotding component  The use plumb and for resisting interest forces shall be designed and competent installed by others. We beats are to be applied to the component until after all the framing and festatining are competed. At no time shall beads greater than design leads onder on the applied to the component.  Design Criteria	SUPPORT REACTIONS (LBS):  MAXIMUM B E A R I N G N U H B E R  DOWN (69 469  UPLIFT  HIN BEARING SIZES (IN-SX)  3-8 3-8  HAXIMUM DEFLECTIONS  CALCULATED ALLOWABLE  LIVE LOAD 0.04*  *DEAD LOAD 0.04*  TOTAL LOAD 0.09*  0.51"	NOTE:  2006 1 Allowable Stress Design  NOTE:  1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY  THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERHCATION OF LOADING, DEFLECTION  LIMITATIONS, FRAMING METHODS. WIND AND SEISMC  BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECTI. DISCLAIM ALL  PROJECT BROINGER OR ARCHITECTI. DISCLAIM ALL  UNITED OCCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE  BUILDING DESIGN.  2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE  USED TO NOTE OF THE LATERAL STABILITY.  3. DO NOT CUT, NOTICH OR DRILL LONTACT.  5. VERIFY DIMENSIONS BEFORE CUTTING LET TO SIZE.  6. THIS LIPI IS TO BE USED AS A FLOOR JOIST ONLY.  7. PROVIDE COMPRESSION EDGE BRACING AT  ARCHITECTURES.  AND  AND  AND  AND  AND  AND  THE VERTICAL COMPONENT OF THE PROVIDE COMPRESSION EDGE BRACING AT  AND  AND  AND  THE VERTICAL COMPONENT OF SIZE.  AND  AND  AND  AND  THE VERTICAL COMPONENT OF SIZE.  AND  AND  AND  AND  THE VERTICAL COMPONENT ONLY.  AND  AND  AND  AND  THE VERTICAL COMPONENT ONLY.  AND  AND  AND  AND  AND  THE VERTICAL COMPONENT ONLY.  AND  AND  AND  AND  AND  THE VERTICAL COMPONENT ONLY.  AND  AND  AND  AND  AND  AND  AND  AN
will conclude must be purchase as requires by cook continuous returns support is assured foral, floor bearn, etc.). If does not provide on-title inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	Miscellaneous Information  IT The use of this component shall be specified by the designer of the modern complete structure. Obtain as the necessary code compleance approval and complete structure. Obtain as the necessary code compleance approval and complete structure before using this component. If the design order is lated above does not ment local building. Designed of requirements, to not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing made without camber and will deflice under load. Wood in direct contact made without camber and will deflice under load. Wood in direct contact made without camber and will deflice under load. Wood in direct contact made without camber and will deflice under load. Wood in direct contact made without camber and will deflice under load. Wood in direct contact made without camber and will deflice under load. Wood in direct contact made without camber and will deflice under load. Wood in direct contact made without camber and will deflice under load.	11.875  CROSS SECTION	NOTE: LOADS SHOWN ARE FOR INPUTED IN THE TOP ATTENLUME LOADING ARE CHE (DMENSIONS MEASURED FROM LEFT TOP/SI TRIBUTION SOURCE TYPE TOP/SI FORM FLOOR LIVE TOP FORM FLOOR DEAD TOP RANING NOTES: 19 COMPONENT DESIGN IS SPECIFICALL. 19 COMPONENT DESIGN IS SPECIFICALL. 20 OF THIS DESIGN FOR ANYTHING OTHE A DESIGN PROPESSIONAL A DESIGN PROPESSIONAL SIT AS DESIGNED IT IS THE RESPONSIBILIST AS DESIGNED IT IS THE RESPONSIBILIST AS DESIGNED IT IS THE RESPONSIBILIST AS DESIGNED TO SUPPORTING THERE IST IS CAPABLE OF SUPPORTING THERE ICHOR LPI JOIST SECURELY TO BEARING
* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademan of Louisiana-Pacific Corporation.	LP LVL and CTR, LP I-loist Specifications  *Supports and connections for LP LVL and CTR, LP I-loists to be specific applications.  *Common nais deview parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 6d.  *Do not cut, noich, drill or eller LP LVL and CTR, LP I-loists except as shown in published material from LP any use of LP LVL and CTR, LP I-loists contany to the limits set forth hereon, negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties or merchantability and fitness for a particular use.	THIS DRAWING IS NOT TO SCALE	1 LP1 20Pius WEB: 0.375° FLANGE 1.50  OAD LDF 1.00 0.90
SHEET #	LP Engineered Wood Products 2705 Highway 421 North Wilmington, NC 28401 Local National Wats 800,999,9105 0608-049	TO SCALE ***	THE TOTAL LOAD DESIGN CRITES TOTAL LOAD DEFLECTION CLIVE LOAD DE TOTAL LOAD DE COMPLIA LOAD DE COMPLIA LOAD DE MISCONSIN COMPLIA N.Y. CITY
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LATERAL STABILITY,
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
6. VERIFY DIMENSIONS BEFORE CUTTING SUPPORT REACTIONS (LBS): ATTACH THE TWO PLIES WITH 3 ROWS OF 18d (3-12") NAILS AT 8" OC. STAGGER ROWS.
NAILS CAN BE DRIVEN FROM ONE FACE OR HALF ROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DAMETER OF D.131". 16d SINKERS (3-1/4") MAY BE. THIS LP LVL IS TO BE USED AS A THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
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PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. otal load deflection is instantaneous. MIN BEARING SIZES (IN-SX) ndling & Erection rary and permanent bracing for holding component and for resisting literal forces shall be designed and of by others. No loads are to be applied to the tent until after all the framming and festiening are ted. At no time shall loads greater than design loads total. BUT HALF MUST BE DRIVEN FROM 2359 2006.1 Allowable Stress Design RESTRAINT AT SUPPORTS TO ENSURE MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE AD 0.02" 0.16" AD 0.02" 0.32" 7023 2359 NUMBER THIS COMPONENT DESIGN IS SPECIFICALLY FOR 1-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN 1-P LVL OR 1-P 1-JOISTS IS STRICTLY PROMBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. WARNING NOTES: ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LIVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. ARCHITECT OR DESIGNED TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE I), OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) 11.875 CROSS SECTION LOAD TABLE LOAD LOAD FROM TO I

67-IN-SX FT-IN-SX
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233 PLF 00-00-00 13-00-00
12 PLF 00-00-00 13-00-00 and 3" for 6d.

Do not out, notich, drill or alter LP LVL, and CTR, LP I-loiests except as shown in published material from LP any use of LP LVL, and CTR, LP I-loiests contrary to the limits at forth hereon, negates any express wernarity of the product and LP discisions all implied wernarity including the implied wernarities of merchantability and fitness for a particular use. LP LVL and CTR, LP I-Joist Specifications A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered tradement of Louisiana-Pacific Corporation and connections for LP LVL and CTR, LP i-bists to be specific applications nails driven perallel to give lines shall be speced a minimum of 4" for 10d 6- 6- 0 H LOAD \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 2 BEAMS 1.76 X 11.875 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED
TOGETHER (REFER TO NOTES). 0.90 1DE 13- 0- 0 -2706 Highway 421 North Wilmington, NC 28401 Local 910.762,9878 National Wats 800.999.9105 DWG LP Engineered Wood Products ٩ 6-0 TOTAL LOAD ICBO
L.A. City
CCMC
WISCONSIN
NER
N.Y. CITY CODE COMPLIANCES:
REPORT #
ICBO ER-5004 DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA FLR RIGHT SPAN CARR. : 0608-049 16 of 42 90/01/80 NEA 97-94-E NR 1214D 200124-W 622 SBCCI 16.00 FT 15.00 FT ۲۲ // 555 WSI: 0.24 VSI: 0.44 240 2008,1 PSF

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ent breaking for holding compone lateral forces shall be designed to be published to the families are to be applied to the hall loads greater than design it benefit as the state of the st	Handling & Erection Mis	SUPPORT REACTIONS (LES):  MAXINUM B E A R I N G N U M B E R  DOWN 3304 11008 3314  UPLIFT  MIN BEARING SIZES (IN-SX) 3-0 4-4 3-0  MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE LIVE LOAD 0.02" 0.20"  *DEAD LOAD 0.02" 0.39"	EACH FACE TO THE THE THE THE THE EQUALLY CONCENTRATED TO ALL PLIES, ADDITIONAL FASTENERS MAY BE REQUIRED.	ATTACH THE TWO PLIES WITH 3 ROWS OF 18d (3-12") NAILS AT 8" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DAMETER OF 0.131". 18d SINKERS (3-1/4") MAY BE USED, BUT MALF MUST BE DRIVEN FROM	3. DO NOT CUT, NOTCH OR DRILL LP LVL. 4. SHIM ALL BEARINGS FOR FULL CONTACT. 5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE. 6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY. 7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.	1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISED BY ALMAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL WARYS REQUIRED IS THAT MAY BE USED TO INCORPOPATE THIS COMPONENT INTO THE BUILDING DESIGN.  2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE CONTENTS TO ENSURE CONTE	NOTE: 2008.1 Allowable Stress Design
The use of this component shall be specified by the designer of the compilet structure, Obtain all the necessary code compilence approval and Compilet structure. Obtain all the necessary code compilence approval and Compilet of the component. If the design create lasted above does not meet local butiding code negativements, do not use this design, When this drawing is speed and senied, the structural design is approved as shown in this drawing based on data provided by the customer. LP VN and CTR, LP Hotes are small within connecte must be protected as required by code. Continuous lateral support is assemed (veil, flood beam, etc.), LP dees not provide on-site inspection. This drawing must have an Auchitact's or Engineer's seal shoot.	Miscellaneous information	11.875 1.750 CROSS SECTION		MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.  ANCHOR LP LYL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.	WARNING NOTES: THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP LIOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.	NALY   NOTE: LOADS SHOWN ARE FOR INDUT LOAD CASE (1), OTHER LOAD CASES	
Supports and connections for LP LVL and CTR, LP I-loids to be specific applications of Common nalls driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.  To not out, note, ordit or after LP LVL and CTR, LP I-loidst except as shown in provisited marietist from LP any use of LP LVL and CTR, LP -loidst centrary to the limits set from hereon, regalets any suppress warmany of the product and LP obtaclaries all implied warmanies including the implied warmanies of merchanisability and fitness for a particular use.  1" A COPY OF THIS DRAWNING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered teadement of Louisians-Pecific Corporation.	LP LVL and CTR, LP i-Joist Specifications	3- 0- 0 THIS DRAWING IS NOT TO SCALE ***		REVENT CRUSHING OF THE LP LVL OF THE PROJECT ENGINEER, SUPPORT STRUCTURE FOR THIS ONS. ARINGS OR HANGERS.	TO ENSURE LATERAL STABILITY.	ADCASE (1). OTHER LOAD CASES  DESIGN CONSISTS OF 2 - PLIES FASTENED DAS RECOVERD  LOAD FROM TO LOAD LDF FT-IN-SX FT-IN-SX 600 PLF 00-00-00 16-00-00 160 PLF 00-00-00 16-00-00 122 PLF 00-00-00 16-00-00 124 PLF 00-00-00 16-00-00 125 PLF 00-00-00 16-00-00 120 PLF 00-00-00 16-00-00 120 PLF 00-00-00 16-00-00 120 PLF 00-00-16-00-00 120 PLF 00-00-00 16-00-00 120	LOAD TABLE 2 BEAMS 1.75 X 11.87
2706 Highway 421 North Williamington NC 28401 Local 910.762.9878 National Wats 800.999.9105  DWG # 06 SHEET # 17	LP Engineered Wood	8- 0- 0				- PLIES FASTENED	LP LVL2650Fb-1.9E
0608-049					CODE COMPLIANCES: REPORT # ICBO ER-5004 L.A. City RR 25167 CCHC 11518-R MISCONSIN 200124-W NER 622	DEAD LOAD - TOTAL LOAD - FLR LEFT SPAN CARR. : FLR RIGHT SPAN CARR. : DEFLECTION CRITERIA : LIVE LOAD DEFL: TOTAL LOAD DEFL:	DESIGN CRITERIA :
	1 8000			(1)		FF 442 514	MSI: 0.48 VSI: 0.74

PSF

ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-127) NAILS AT 8° OC. STAGSER ROWS.
NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DAMETER OF 0.131". 18d SINKERS (3-147) MAY BE USED, BUT HALF MLST BE DRIVEN FROM 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERTICATION OF LOADING, DEFLECTION
LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC
BRACING, AND OTHER LATERAL BRACING THAT IS
ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE
PROJECT ENGINEER OR ARCHITECT; DISCLAIM ALL
WIND RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS
OR OTHER DOCUMENTS THAT MAY DE USED TO
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UNITED STEELS OF THE STEELS OF THE COMPONENT INTO THE
UNITED STEELS OF THE STEELS OF THE COMPONENT INTO THE
UNITED STEELS OF THE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING 6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY. BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE DOWN 3321 SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G
1 2 EACH END OF COMPONENT. MIN BEARING SIZES (IN-SX) PROVIDE COMPRESSION EDGE BRACING AT indling & Eraction porary and permanent bracing for holding component to and for resisting interest forces shall be designed and used by others. No loads are to be applied to the poorsi until ster at the framing and festering are 2006.1 Allowable Stress Design MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE AD 0.04" 0.20" AD 0.06" 0.39" 7854 dified are in substantial risions of NDS and AITC. \* adjustment factor for creep. 5889 NUMBER The use of this component shall be specified by the designer of the complete structure. Obtain at the necessary code completers a provise and instructions from the designers of the complete structure before using his component. If the design crisis is lated above does not need the balliding code requirements, do not use this design. When this drawing is eighed and sealed, the structural design is approved as shown in this drawing based on date provided by the outcomer. If DV, Lin CCTRL ID Lipids are Miscellaneous Information UNIFORM
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UNIFORM DISTRIBUTION SOURCE TYPE TOP/SIDE MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNED TO VERIETY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP L-JOISTS IS STRUCTLY PROMIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. WARNING NOTES: ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS. NOTE: LOADS SHOWN ARE FOR NIPUT LOAD CASE (1), OTHER LOAD I FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) FLOOR FLOOR FLOOR FLOOR FLOOR FLOOR BEAM 11.875 LIVE LIVE DEAD LIVE DEAD DEAD DEAD MEAD SIDE SIDE SIDE SIDE SIDE SIDE LOAD TABLE LOAD 300 280 120 120 113 105 125 \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR Do not cut, notch, drill or stier LP LVL and CTR, LP I-Joints except as shown in published material from LP any use of LP LVL and CTR, LP I-Joints contrary to the limits set forth hereon-negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. LP LVL and CTR, LP I-Joist Specifications LP is a registered tradement of Louisians-Pacific Corporation D FROM TC 114-SX FT-IN-SX FT-I nd connections for LP LVL and CTR, LP i-loists to be specific application ails driven parallel to give lines shall be spaced a minimum of  $\mathbf{A}^a$  for 10d as OTHER LOAD CASES 8- 0- 0 LOAD \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 2 BEAMS 1.75 X 11.875 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 30.1 24- 0- 0 | DWG 2706 Highway 421 North Wilmington, NC 28401 SHEET Local 910,762,9878 National Wats 800,999,9105 LP Engineered Wood Products ٣ 0-0 NER N.Y. CITY HUD DEAD TOTAL COMC ICBO DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA CODE COMPLIANCES : FLR LEFT SPAN CARR. FLR RIGHT SPAN CARR. ISCONSIN 0608-049 8 of 42 city TOAD TOAD 08/10/08 MEA 97-94-E MR 1214D ER-5004 RR 25167 11518-R 200124-W 622 SBCCI : 15.00 FT : 0.00 FT L / 480 L / 240 55 55 WSI: 0.33

2008.1

CONTRINENT   CONTRIBUTED   C	on. The drawing must have an Architect's or Engineer's seal store residened an Engineering document.	d load deflection includes adjustment factor for creep. Inspedi
COUNTRY   COUN	n data provided by the customer. Un LYL and CIX, Uniquess an thou camber and will defined under load. Wood in direct contact larets must be prefeted as required by code. Continuous litteral is assumed (walf, floor beam, etc.), LP does not provide on-site.	ified are in substantial
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LOAD LOF  LOAD LOF  LOAD LOF  LOAD LOF  THA LEFT SPAN CARR. :  FILA RIGHT SPAN CARR. :  FILA LEFT SPAN CARR. :  FILA RIGHT SPAN CARR. :  FILA LEFT SPAN CARR. :  FILA LEFT SPAN CARR. :  FILA LEFT SPAN CARR. :  FILA RIGHT SPAN CARR. :  FILA RI	CACOD DECETOR	
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LOAD   LDF	16.000	
LOAD   LDF	<i>∤</i> -	ARING 2
LOAD   LDF	_	PORT REACTIONS (LBS):
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LOAD LDF  LOAD LDF  1.00  1.00  1.00  1.25  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  1.25  0.90  0.90  0.90  1.25  0.90	ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS	
LOAD LDF  LOAD LDF  1.00  1.00  1.00  1.25  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  0.90  1.25  0.90  0.90  0.90  0.90  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  0.90  0.90  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.25  0.90  1.26  EXEPORT # EX	PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.	DISTRIBUTED TO ALL PUES. ADDITIONAL FASTENERS MAY BE REQUIRED.
DEAD LOAD	MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.	FROM EACH FACE. NAIL'S MAY BE COMMON ON BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 164 SINIKERS (3-147) MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.
DEAD LOAD	PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.	1/2") NAILS AT 12" OC. STAGGER ROWS.
LOAD LDF TOTAL LOAD TOTAL LOAD TOTAL LOAD LOFT SET RIGHT SET SET RIGHT SET SET SET SET SET SET SET SET SET SE	WARNING NOTES:  THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP L-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL	THAS LP IVA, IS TO BE USED AS A FLOOR BEAM ONLY. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. ATTACH THE TWO PUES WITH 3 ROWS OF 18d
LOAD LOF  LOAD LOF  TOTAL LOAD  LOAD  LIVE LOAD DEFL:  TOTAL LOAD DEFL:  TOTAL LOAD DEFL:	FORM BEAM WEIGHT IVE CENTRATED FLOOR LIVE CENTRATED FLOOR DEAD	IERCYL SHABILITY.  NOTICUT, NOTICH OR DRILL LP LVL.  RIN ALL BEARINGS FOR FULL CONTACT.  RIPY DIMENSIONS BEFORE CUTTING  LVL TO SIZE.
LOAD LOF  I LOAD LOF  I CAR TOTAL LAND  TOTAL LAND  TOTAL SPAN CARR. :  OF LR RIGHT SPAN CARR. :  OF 1.25	FORM	OR OTHER DOCUMENTS THAT MAY BE USED TO UNIOCRED PAINT THIS COMPONENT INTO THE UNIUBULIONIC DESIGN.  2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE UNIUBULIONIC DESIGN.
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Z BEAMS 1.75 X 18.000 LP LVL3650FD-1.9E DESIGN CRITERIA : DESIGN CONSISTS OF 2 - PLIES FASTENED DESIGN CRITERIA : TOGETHER (REFER TO NOTES).  LIVE LOAD		ES COMPONENT IS DESIGNED TO SUPPORT ONLY IE VERTICAL LOADS SHOWN AS DETERMINED BY

August/0608-049/6	The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * upper in a substantial based on the latest revisions of NDS and AITC. * upper in a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and AITC. * upper is a conformity with the latest revisions of NDS and		Temporary and perimanest bracing for holding component. The use purch and for resisting intendit forces shall be designed and competing installed by others. No facts are to be applied to the component until after all the framming and fastering are component. In time shall beast greater thin mit design beast component. In time shall beast greater thin mit design beast code exc.	Handling & Erection Miscell	LOAD 0.06" 0.40"	ALCULATED ALLOWABLE				'민	DOWN 652 2858 UPLIFT 78	SUPPORT REACTIONS (LBS): MAXIMUM B E A R I N G N U M B E R	77	7				DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.	USED, BUT HALF MUST BE DRIVEN FROM  EACH FACE	FROM EACH FACE, NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DAMETER OF 0 131-164 NAVERS (3-144") MAY BE	ATTACH THE TWO PLIES WITH 3 ROWS OF TOU (3-12") NAILS ATTA" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF	EACH END OF COMPONENT.	6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY. 7. PROVIDE COMPRESSION EDGE BRACING AT	4. SHIM ALL BEARINGS FOR FULL CONTACT.  5. VERIFY DIMENSIONS BEFORE CUTTING  I P I M TO SIZE	BUILDING DESIGN.  2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE CO.  LATERAL STABILITY.  CO.  CATERAL STABILITY.  CO.	RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE	PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL	THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIN THE PROPERTY OF THE PROPE	NOTE: 2008,1 Allowable Stress Design  1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY	Monez Res. / Beam-E (MB right hand side)
SPX	ta assumed (well, floor beam, etc.). LP does not provide on-site on. This drawing must have an Architect's or Engineer's seal affixed ansidered an Engineer's seal affixed assidered an Engineering document.	led, the structural design is approved as shown in this trawing in data provided by the customer. LP LVL and CTR, LP Hybrists are thout camber and will deflect under load. Wood in direct contact to the ground of the structure of the continuous lateral provide must be professed as required by code. Continuous lateral	if The use of this component shall be specified by the designed of the and complete structure. Octain all the necessary code completemes approval and complete structure. Octain all the necessary code complete structure before using this component. If the design orderia listed above does not near local building. O pasts code requirements, do not use this design. When this drawing is signed pasts code requirements, do not use this design.	Miscellaneous Information	0.40"	CALCULATED ALLOWABLE 0.05" 0.20"	T CANTITUER	CROSS SECTION			16.000	<i>}</i> —						ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS	PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.	ARCHITECT OR DESIGNER TO VERIFY THAT THE S BEAM IS CAPABLE OF SUPPORTING THE REACTIO	MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL. BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.	PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.	USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP I-JOISTS IS STRICTLY PROHIBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL	WARNING NOTES: THIS COMPONENT DESIGN IS SPECIFICALLY FOR	UNIFORM BEAM WEIGHT CONCENTRATED FLOOR LIVE TOP CONCENTRATED FLOOR DEAD TOP	FORM FLOOR LIVE	FORM ROOF LIVE TOP	FOR PATTERN LIVE LOADING ARE CHECKED (DIMENSIONS MEASURED FROM LEFT END (		
LP is a registered trademark of Louisana-Pacific Corporation.	1 * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR	afflut set forth reven neglets any express warranty or the pro- all implied warranties lectuating the implied warranties of merch- for a particular use.	Supports and connectors for UP by and UPs, UP houses to the special Deprice of Common nells driven parallel let glue here shall be special antiferrior of 4° for 10 and 5° for 8d. and 6°	LP LVL and CTR, LP I-Joist Specifications	*** THIS DRAV	12- 5- 0		100 mg		×.								RINGS OR HANGERS.	S INDICATED BY NEGATIVE REACTIONS. CT ARCHITECT, ENGINEER, OR DESIGNER.	NS.	EVENT CRUSHING OF THE LP LVL	O ENSURE LATERAL STABILITY.	IN LP LVL OR LP 1-JOISTS IS HIS DOCUMENT REQUIRES REVIEW	L-P ENGINEERED WOOD PRODUCTS.	LBS 16-04-00 MINBRG=2.50" 1 LBS 16-04-00 MINBRG=2.50" 0	80 PLF 00-00-00 16-05-00 0.90	FT-IN-SX FT-IN-SX PLF 12-05-00 1.	TOAN I	2 BEAN DESIGN	
- 1	SHEET #	2	aptract approach.  2/05 highway 4.2 i North infram of 4" for 10d  Willington, NC 28401  Volumiquo, NC 28401  Local  910,782,9876  National Wats 800,999,9105		THIS DRAWING IS NOT TO SCALE ***	16- 5- 0	-		-	.8			18 10	, , , , ,	カンド	10)	1777)	17.72.			;	N.	NEC C.	10 00				time or so social.	AS 1.76 X 16.000 LP LVL2860Fb-1.9E DE: CONSISTS OF 2 - PLIES FASTENED IT	'
	20 of 42	0608-049		ducts ourpos SBCCI 2008.1						2	22			Jours !									L.A. City RR 25167 CCMC 11518-R WISCONSIN 200124-W NER 622	F	t	DEFLECTION CRITERIA : L / 480	FIR LEFT SPAN CARR. : 2.00 FT FIR RIGHT SPAN CARR. : 2.00 FT	DEAD LOAD = 15 PSF TOTAL LOAD = 55 PSF	N CRITERIA : VS:	ISM

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1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERTICAL DO FLOADING, DETLECTION
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PROJECT ENGINEER OR ACCHITECT, I DISCLAIM ALL
URE PROJECT ENGINEER OR ACCHITECT, I DISCLAIM ALL
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ONLY DESIGNED. 3. DO NOT CUT, NOTICH OR DRILL LP LVL.
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5. VERILY DIMENSIONS BEFORE CUTTING
LP LVL TO SIZE. BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE 8. THIS LP LVALIS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G ATTACH THE TWO PLIES WITH 3 ROWS OF 18d (3-1/2) NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131" 16d SINKERS (3-1/4") MAY BE The design and material specified are in substantial ordiormity with the latest revisions of NDS and ATC. \*
back boad deflection includes adjustment factor for creep-total load deflection is instantaneous. DOWN 5573 LATERAL STABILITY CONCENTRATED LOADS MUST BE EQUALLY INSTRIBUTED TO ALL PLIES. ADDITIONAL INSTENERS MAY BE REQUIRED. MIN BEARING SIZES (IN-SX) nding & Erection owary and permanent bracing for holding component b and for resisting letteral forces shall be designed and led by others. No loads are to be applied to the orient until after all the framing and festening are ED, BUT HALF MUST BE DRIVEN FROM 10AB MAXIMOM DEFLECTIONS
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AD -0.02" 0.33"
AD 0.03" 0.65" 2112 UMBE LEFT CANTILEVER
E CALCULATED ALLOWABLE C.
0.31"
0.20"
-0.01" if The use of this component shall be specified by the designed complete structure. Order in the measure yorde complete instructions from the designers of the complete structure component from the designers of the complete structure component. If the design criteria listed above does not a state order requirements, do not use this design. When this of and sasked, the structural design is approved as aboven the following provided by the outsiners. If VL, and C or the provident of the outsiners. If VL, and C or the provident of the outsiners. If VL, and C or the provident of the outsiners. If VL, and C or the provident of the outsiners in VL or the C or the provident of the outsiners. If VL, and C or the provident of the outsiners in VL or the control of the outsiners. If VL or the C or the outsiners is the outsiners in VL or the outsiners in VL or the outsiners. If VL or the outsiners is the outsiners in VL or the outsiners in VL or the outsiners in VL or the outsiners. If VL or the outsiners is the outsiners in VL or the outsiners in VL o Miscellaneous Information UNIFORM
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CONCENTRATED UNIFORM DISTRIBUTION THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP L-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUISHING OF THE LP LIV. BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY. WARNING NOTES: NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD I FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED; (DMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) criteria listed above does not meet local building it use this dealign. When this drawing is signed dealign is spooved as shown in this drawing by the customer. LP LVL and CTR, LP I-plaists are will deflect under load. Wood in direct contact will deflect under load. Wood in direct contact be specified by the designer of the SOURCE TYPE TOP/SIDE WALL
FLOOR
FLOOR
ROOF
ROOF
FLOOR
FLOOR
FLOOR
FLOOR
FLOOR
FLOOR RIGHT CANTILEVER
CALCULATED ALLOWABLE
0.02"
0.12"
-0.01"
0.23" 16.000 CROSS SECTION TOP TOP LOAD TABLE LOAD and 3" for 6d.

Do not cut, notich, drill or allier LP LVL, and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL, and CTR, LP I-Joists contrary to the junities art forth hereon regales any outgress warranty of the product and LP disclaims all implied warranties including the implied warranties of marchantability and fitness. \*Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. 
\*Common nalls driven parallel to glue lines shall be spaced a minimum of 4" for 10d LP LVL and CTR, LP LJoist Specifications A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR 15 15 LP is a registered trademark of Louisiana-Pacific Corporation OTHER LOAD CASES MINBRG=2.50\*
MINBRG=2.50\*
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MINBRG=2.50\* LOAD \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). ΙĐΕ 16-8-0 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.8676 National Wats 800.989.9105 DWG SHEET LP Engineered Wood Products CODE COMPLIANCES:
REPORT #
ICBO ER-5004
L.A. City RR 25167
CCHC 11518-R
WISCONSIN 200124-W
NER 622 WISCONSIN NER N.Y. CITY DEAD TOTAL DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA FLR LEFT SPAN CARR. FLR RIGHT SPAN CARR. 2- 4- 0 21 of 42 0608-049 , 1040 1401 08/10/08 MEA 97-94-E MR 1214D SBCCI 0.00 FI rr '' 55 55 MSI: 0.14 VSI: 0.43 480 240 2008.1 PSF

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The design Amenical specified are in substantial with on conformity with the letest revisions of NDS and ATC.  Deed load deflection includes egiptement factor for creep. Inspect Total load deflection is instantaneous.	manent bracing for holding component strange letters forces shall be designed and it. No leads are to be applied to the force that the funding are first all the funding and flattering are line shall leads greater than design loads component.	TOTAL LOAD 0.02" 0.37"	CAN CAN	sx)	SUPPORT REACTIONS (LBS):		DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOPE EGGE OF 1P LIV., SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/27) NALLS AT 127 CO. STAGGER ROWS. (3-1/27) NALLS AT 127 CO. STAGGER ROWS. NALLS AT 127 CO. STAGGER ROWS. NALLS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NALLS MAY BE COMMON OR BOX NALLS WITH A MUNIMUM SHANK DUMBETER OF 0.131". 19d SINKERS (3-1/4") MAY BE USED. BUT HALF MUST BE DRIVEN FROM EACH FACE.	C. INS. LY LYL IS IO BE USED AS A COMBINATION ROOF AND PLOOR BEAM ONLY. 7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.	A SHIM ALL BEARINGS FOR FULL CONTACT.  4. SHIM ALL BEARINGS FOR FULL CONTACT.  6. VERIFY DIMENSIONS BEFORE CUTTING  UP LIV. TO SIZE.	BRACING, AND OTHER LATERAL BRACING THAT IS ALWAY'S REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS UNITED THE OCCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE W	NOTE: 2008.1 Allowable Stress Design  1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY  1. THE YERTICAL (JOADS SHOWN AS DETERMINED BY OTHERS, VERRIFICATION OF LOADING, DETER ECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISM	Monez Res. / Beam-F (at Stairwell)
made without cambor end will delect under bate. Wood in orrect contract with concrete must be protected as required by code, Continuous lideral support is assumed (wall, floor beam, etc.). UP does not provide on-site impedion. This drawing must have an Archiact's or Engineer's seal afood to be considered an Engineering document.	Miscalianeous Information  The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code completes approved and instructions from the designers of the complete structure before using the component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing it signed and sealed, the associated leading to approved as a shown in this drawing is separed.			CROSS SECTION	16.000	<u>.                                    </u>	ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS	MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYLBEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERRY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.	THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.	TREBUTION SOURCE TIPE 107/31PE 107/31PE 107/51PE	LOAD NOTE: LOADS SHOWN ARE FOR INPUT LOAD FOR PATTERN LIVE LOADING ARE CHECKE (DIMENSIONS MEASURED FROM LEFT END	
*A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INST  LP is a registered tradement of Louisiana-Pacific Corporation.	LP LVL and CTR, LP I-Joist Specifications  *Supports and connections for LP LVL and CTR, LP I-Joists to I  *Common nails driven parallel to glue lines shall be apaced a m and "I for 8d.  *Do net out, notch, drill or alter LP LVL and CTR, LP I-Joists ax published material from LP any use of LP LVL and CTR, LP I-  fanits set from hereon-organise any express warranty of the po- fanits set from hereon-organise any express warranty of the parallel parallel and the parallel paralle	7.7	7- 7- 0	Q			O BEARINGS OR HANGERS.	EVENT CRUSHING OF THE LP LVL F THE PROJECT ENGINEER. SUPPORT STRUCTURE FOR THIS NS.	L-P ENGINEERED WOOD PRODUCTS. N LP LVL OR LP I-JOISTS IS HIS DOCUMENT REQUIRES REVIEW	320 PLF 00-00-00 07-07-00 0.91 16 PLF 00-00-00 07-07-00 0.91	2 BEAN DESIGN TOGETH	FL Mayo Truss
DWG # SHEET #	LP Engineered Wood Products be specific applications.  2706 Highway 421 North Warnington, NC 28401 Local 910,782,9878 Local 910,782,9878 National Wats 800,998,9105 harnability and finess	TO SCALE ***		ē						3330	\$ 1.75 X 16.000 LP LVL2650Fb-1.9E CONSISTS OF 2 - FILES FASTEMED R (REFER TO NOTES).	
0608-049 22 of 42	Products deviated SECCI 2008.1					Mul	CCMC 115187 CCMC 115187 MISCONSIN 200124-W NER 622 N.Y. CITY MEA 97-94-E HUD MR 1214D	CODE COMPLIANCES: REPORT # TOBO C::	DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: L / 240	ROOF LIVE LOAD = 20 PSF ROOF DEAD LOAD = 20 PSF ROOF TOTAL LOAD = 40 PSF FLR LEFT SPAN CARR. : 0.00 FT ROOF LEFT SPAN CARR. : 20.00 FT ROOF RIGHT SPAN CARR. : 28.00 FT	PESIGN CRITERIA: WS: 0.13  FLOOR LIVE LOAD = 40 PSE  FLOOR TOTAL LOAD = 15 PSE  FLOOR TOTAL LOAD = 55 PSE	

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERFECATION OF LOADING, DETECTION
LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC
BRACING, AND OTHER LATERAL BRACING THAT IS
ALWAY'S REQUIRED IS THE RESPONSIBILITY OF THE
PROJECT ENGINEER OR ARCHITECT, LOISCLAIM ALL UNI
RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS
OR OTHER DOCUMENTS THAT MAY BE USED TO
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DIN CORPORATE THIS COMPONENT INTO THE
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ON THE PROJE LATERAL STABILITY,
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
6. VERIFY DIMENSIONS BEFORE CUTTING BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LP LVL. TO SIZE.

8. THIS LP LVL IS TO BE USED AS A
COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT
EACH END OF COMPONENT. MAXIMUM B E A R I N G ATTACH THE TWO PLIES WITH 3 ROWS OF 184 (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF ROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DAMETER OF 0.131". 184 SINKERS (3-1/4") MAY BE FASTENERS MAY BE REQUIRED he design and material specified are in substantial onformity with the latest revisions of NDS and AITC. UPLIFT CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL pad load deflection includes adjustm pai load deflection is instantaneous. MIN BEARING SIZES (IN-SX) ndling & Erection ED, BUT HALF MUST BE DRIVEN FROM orany and permanent bracing for holding component so and for resisting lateral forces shall be designed and feed by others. No loads are to be applied to the conent until after all the framing and tastening are keted. At no time shall loads greater than design loads 1385 2006,1 Allowable Stress Design MAXIMUM DEFLECTIONS
CALCULATED ALLOWABLE
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-0.00" 0.30" w The use of this component shall be specified by the designer of the complete studium. Obtain all the necessary code completes approved and instructions from the designers of the complete structure before using his component. If the design offer listed above does not meet leval building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this trawing based on data provided by the customer. If Lfv. and CTR, Lft. In-lists are Miscellaneous Information UNIFORM UNIFORM UNIFORM UNIFORM UNIFORM UNIFORM DISTRIBUTION SOURCE TYPE TOP/SIDE CONCENTRATED THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP I-JOISTS IS STRUTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LV. BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNED TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS. PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY. WARNING NOTES: NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD I FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) ROOF
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ROOF 16.000 CROSS SECTION oved as shown in this drawing r. LP LVL and CTR, LP Hoists are sider load. Wood in direct contact TOP LOAD TABLE IOAD Do not out, notch, drill or after LP LVL and CTR, LP L-lotets except as shown in published material from LP any use of LP LVL, and CTR, LP L-losets contrary to the limits set forth hereon-negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantishilly and fitness for a particular use. \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP LVL and CTR, LP I-Joist Specifications Supports and connections for LP LVL and CTR, LP i-loists to be specific applications.

Common neils driven parallel to give lines shall be speced a minimum of 4" for 10d. AD FROM TO LOAD

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58 LBS 08-00-00 MINBRG=2.50" LP is a registered trademark of Louisiana-Pacific Corporation nells driven perallel to glue lines shell be OTHER LOAD CASES 끄 \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 7-0-0-2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). Ŷ 1.25 0.90 1.00 0.90 0.90 1.25 0 2706 Highway 421 North Wilmington, NC 28401 Local 910.762,9878 National Wats 800.989,9105 SHEET DWG LP Engineered Wood Products ٩ FLOOR LIVE FLOOR DEAD FLOOR TOTAL NER N.Y. CITY HUD DESIGN CRITERIA : CODE COMPLIANCES : DEFLECTION CRITERIA : LIVE LOAD DEFL: TOTAL LOAD DEFL: FIR RIGHT SPAN CARR. ROOF LEFT SPAN CARR. ROOF RIGHT SPAN CARR. WISCONS IN 23 of 42 0608-049 city 469 90/10/06 REPORT # ER-5004 RR 25167 11518-R 200124-W 622 MEA 97-94-E MR 1214D TOAD TOAD SBCCI 2.00 FT 6.00 FT 8.00 FT 20 20 15 WSI: 0.05 20061 PSF PSF

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LP Enginees 2706 Highway Wilmington, No. Local National Wats National Wats R SHEET #	O TO SCALE ""	PEAMS 1.75 X 16.000 LP LVL2660Fb-1.9E DESIGN CONSISTS OF 2 - PLISS FASTENED TOGETHER (REFER TO NOTES).  LDF 1.00 0.90 0.90
red Wood Products eviave SBCCI 421 North 570,7821 9878 900,792,9105  0608-049  24 of 42		DESIGN CRITERIA:  LIVE LOAD LOAD LOAD TOTAL LOAD LOAD TOTAL LOAD LIVE LOAD TOTAL LOAD LOAD LOAD LOAD LOAD LOAD LOAD LOAD

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-127) NAILS AT 12" OC. STAGGER ROWS.
NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (2-147) MAY BE USED, BUT HALF MUST BE DRIVEN FROM NOTE: Monez Res. / Beam-H 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G DO NOT CUT, NOTCH OR DRILL LP LVL.
SHIM ALL BEARINGS FOR FULL CONTACT.
VERIFY DIMENSIONS BEFORE CUTTING
LP LVL. TO SIZE. PROJECT ENGINEER OR ARCHITECT, IDSCLAIM ALL UNIFORM RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS UNIFORM OR OTHER DOCUMENTS THAT MAY BE USED TO UNIFORM MICORPORATE THIS COMPONENT INTO THE EACH END OF COMPONENT. PROVIDE COMPRESSION EDGE BRACING AT andling & Erection MIN BEARING SIZES (IN-SX) THIS LP LVL IS TO BE USED AS A LWAYS REQUIRED IS THE RESPONSIBILITY OF THE onery and permanent bracing for holding component bend for resisting lateral forces shall be designed and led by others. No loads are to be applied to the sonerst until after all the framing and fastening are 1235 006.1 Allowable Stress Design MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE AD 0.00" 0.13" AD 0.00" 0.26" AD 0.00" 0.26" 1235 NUMBER Miscellaneous Information DISTRIBUTION SCURCE TYPE TOP/SIDE LOAD THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANTHINIG OTHER THAN LP LYL OR LP LOUISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNED TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS. NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1) OTHER LOAD FOR PAITERN LUTE LOADING ARE CHECKED AS REQUIRED; COMBENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) ent shall be specified by the designer of the FLOOR FLOOR BEAM 16.000 CROSS SECTION LIVE DEAD WEIGHT SIDE LOAD TABLE 10AD FROM TO 1
FT-IN-SX FT-IN-SX
320 PLF 00-00-00 05-05-00
120 PLF 00-00-00 05-05-00
16 PLF 00-00-00 05-05-00 \*Do not cal, notch, drill or altor UP LVL, and CTR, UP I-belots except as shown in published material from UP any use of UP LVL, and CTR, UP I-belots contany to the limits set forth hereon, negates any acyress warranty of the product and UP disclaims all implied warranties including the implied warranties of merchantability and fitness. LP LVL and CTR, LP I-Joist Specifications \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation OTHER LOAD CASES its driven persitel to glue lines shall be speced a mi 四 \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* and CTR, LP Noists to be specific applications.
Ilines shall be speced a minimum of 4° for 10d Mayo Truss LOAD 2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 0.90 LDF D S S 2706 Highway 421 North Wilmington, NC 28401 SHEET Local 910.762,9878 National Wats 800.999,9105 LP Engineered Wood Products CODE COMPLIANCES:

ICBO ER-5004

A. City RR 25167

C. 11518

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TY MEA 97-94-E
MR 1214D LIVE LOAD DEAD LOAD TOTAL LOAD DEFLECTION CRITERIA : LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA: FLR LEFT SPAN CARR. : 25 of 42 0608-049 08/10/06 SBCCI 16.00 FT 15 55 MSI: 0.05 480 240 2006.1 PSF PSF

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERHICATION OF LOADING, DEFLECTION
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UNI 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT. NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING 6. THIS LP LVL IS TO BE USED AS A DOWN SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G
1 2 landling & Erection NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DUMETER OF 0.131" 16d SINKERS (3-1/4") MAY BE ATTACH THE TWO PLIES WITH 3 ROWS OF 18d EACH END OF COMPONENT. MIN BEARING SIZES (IN-SX)
3- 0 3- 0 NCORPORATE THIS COMPONENT INTO THE JUILDING DESIGN. design and material specified are in substantial formity with the latest revisions of NDS and AITC. • diead deflection includes adjustment factor for creep. porary and permanent bracing for holding component to and for resisting lateral forces shall be designed and fixed by others. No leads are to be applied to the powerf until after all the framing and fastishing are pleased. At no time shall loads greater than design loads BUT HALF MUST BE DRIVEN FROM LOAD 1080 2006.1 Allowable Stress Design COMPRESSION EDGE BRACING AT MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE AD 0.00" 0.15" AD 0.00" 0.29" 1080 NUMBER The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code completes a sprayed in complete structure. Obtain all the necessary code completes a provide that instructure before using this component. If the designer of the complete structure before using this component. If the design criteria itsed above does not meet local building code requirements, do not use this design. When this drawing is algored and sealed, the structural design is approved as a provided by the outsioner. LP I/N. and CTR, LP lights are made without camber and will defined under load. Wood in direct contact with concrete must be protected as required by code. Continuous isteral with concrete must be protected as required by code. Continuous isteral Miscellaneous information DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD UNI FORM UNI FORM THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP LADISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNET TO VERBY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD OF FOR PATTERN LIVEL LOADING ARE CHECKED AS REQUIRED FOR PATTERN LIVEL LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) FLOOR FLOOR BEAM CROSS SECTION DEAD WEIGHT SIDE LOAD TABLE LOAD FROM TO I
FT-IN-SX FT-IN-SX
247 FLF 00-00-00 06-01-00
93 FLF 00-00-00 06-01-00
16 FLF 00-00-00 06-01-00 Supports and connections for UP LVL, and CTR, UP i-loists to be specific applications, of Common naits driven parallel to glue lines shall be specied an inhimum of 4 for 100 and 5 for 68.
Do not cut, notch, drill or eller LP LVL, and CTR, UP i-loists except as shown in published material from P any use of UP LVL and CTR, LP i-loists contany to the limits are if forth hereon, neglets any express warming of the product and LP declaims all incipled warmindes including line implied warming of the product and LP declaims. LP LVL and CTR, LP I-Joist Specifications A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation OTHER LOAD CASES 7 \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* LOAD 2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 0.90 TOF 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wets 800.999.9105 DWG SHEET LP Engineered Wood Products CODE COMPLIANCES:

REPORT #

ICBO ER-5004

L.A. City RR 25167 DEAD LOAD
TOTAL LOAD ICBO L.A. City CCMC WISCONSIN NER N.Y. CITY DEFLECTION CRITERIA : LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA: FLR LEFT SPAN CARR. : 26 of 42 0608-049 08/10/08 11518-R 200124-W MEA 97-94-E MR 1214D SBCCI : 0.00 FT : 12.33 FT rr // WSI 0.05 240 2008.1 PSF PSF PSF

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	27 of 42	SHEET # 27		LP is a registered trademark of Louisiana-Pacific Corporation.	nsidered an Engineering document.	Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.
	0608-049	DWG # UG			ncrete must be protected as required by code. Continuous lateral is assumed (wall, floor beam, etc.). LP does not provide on-site	The design and material specified are in substantiat conformity with the latest revisions of NDS and AITC. * support
<del></del>		Wirnington, NC 28401 Local 910, 782 8978 910, 782 8978 National Wats 800,999,9105		Common natis enven parasist to give lines sales de speede a minimum of 4" for 6d, and 3" for 6d.  Do not out, notch, drist or eiter LP L/L and CTR, LP i-foldes except as showing the published material from LP any use of LP L/L and CTR, LP i-loists contrary to the limits set forth hereon,negates any express warmany of the product and LP disclaims all implied warranties industring the implied warranties of marchamability and fitness for a periodizin use.	and complete structure, cottain sit the necessary code complaince approval and complete structure before using this a complete complete structure before using this a complete complete structure before using this design above does not retire the stated above to be derived to support the structure of the stated above to be derived to support the structure of design is approved as aboven in the drawing based on data provided by the outstorner. LP LNE and CTR, LP i-joists are a made without cambor and state dideled under load. Whose is direct contact of the c	plumb and for resisting lateral forces shall be designed and competen resulted by others. No loads are to be applied to the component until after all the framing and testening are code re ampleted. At no time shall loads greater than design loads gode re be applied to the component.  Design Criteria made w
2006.1	08/10/06 SBCCI	LP Engineered Wood Products 2706 Highway 421 North		LP LVL and CTR, LP I-Joist Specifications * Supports and connections for LP LVL and CTR.	Miscellaneous information The use of this component shall be specified by the designer of the	Handling & Erection Miscell Temporary and permanent bracing for holding component. The use
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		HUB.		RINGS OR HANGERS.	ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS	EDGE BLOCKING OF CALL CALL CALL CALL
	ISIN 200124-W 622 622 97-94-F	WISCONSIN NER N.Y. CITY		UPPORT STRUCTURE FOR THIS	ARCHITECT OR DESIGNER TO VERIFY THAT THE SUBSAM IS CAPABLE OF SUPPORTING THE REACTION	7, MEMBER MUST BE SIDE LOADED TO ENSURE ADEQUATE STABILITY, PROVIDE COMPRESSION
		L.A. C		EVENT CRUSHING OF THE LP LVL	MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.	B. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
	COMPLIANCES : REPORT #	CODE CC		HIS DOCUMENT REQUIRES REVIEW	USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP IJOISTS IS STRUCTLY PROHIBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.	3, DO NOT CUT, NOTCH OR DRILL LP LVL. 4. SHIM ALL BEARINGS FOR FULL CONTACT. 5. VERIFY DIMENSIONS BEFORE CUTTING BY
	t			L-P ENGINEERED WOOD PRODUCTS	THIS COMPONENT DESIGN IS SPECIFICALLY FOR I	BUILDING DESIGN. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE 1 ATERAL STARII MY
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NIFORM ROOF LIVE TOP 350 PLF 00-00-00 12-00-00 INIFORM ROOF DEAD TOP 350 PLF 00-00-00 12-00-00 INIFORM FLOOR LIVE SIDE 280 PLF 00-00-00 12-00-00 INIFORM FLOOR LIVE SIDE 280 PLF 00-00-00 12-00-00 INIFORM FLOOR DEAD TOP 80 PLF 00-00-00 12-00-00 INIFORM BEAM WEIGHT TOP 16 PLF 00-00-00 12-00-00 INIFORM BEAM WEIGHT TOP 16 PLF 00-00-00 12-00-00 INIFORM BEAM WEIGHT TOP 16 PLF 00-00-00 12-00-00 INIFORM BEAM WEIGHT TOP ENGINEERED WOOD PRODUCTS.  WARNING NOTES:  THIS COMPONENT DESIGN IS SPECIFICALLY FOR LP ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRUCTLY PROHIBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. ARCHITECT OR DESIGNED TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. REACTIONS. PROVIDE ANCHORAGE FOR UPCLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNEER.	MITEORM ROOF DEAD TOP JSO PLE 00-00-00 12-00-00 0.90 0.90 0.90 0.90 0.90 0.90 0.9
IN-SX	. ;

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DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LYL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 3 ROWS OF 184 (3-12") NALLS AT 12" OC. STAGGER ROWS. NALLS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NALLS MAY BE COMMON OR BOX NALLS WITH A MINIMUM SHANK DIAMETER OF 0.131". 164 SINKERS (3-14") MAY BE 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERTICATION OF LOADING, DETECTION
LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC
BRACING, AND OTHER LATERAL BRACING THAT IS
ALWAYS RECQUIRED IS THE RESPONSIBILITY OF THE
PROJECT ENGINEER OR ARCHITECT I DISCLAIM ALL UNI
RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS
OR OTHER DOCUMENTS THAT MAY BE USED TO
UNICORPORATE THIS COMPONENT INTO THE
DISCUSSION. LATROMINE NOTCH OR DRILL LP LVL 3. DO NOT CUT, NOTCH OR DRILL LP LVL 4. SHIM ALL BEARINGS FOR FULL CONTACT. 5. VERIFY DIMENSIONS BEFORE CUTTING E. THIS LIP LYL. TO SIZE.

6. THIS LIP LYL IS TO BE USED AS A
COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROYIDE COMPRESSION EDGE BRACING AT
EACH END OF COMPONENT. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE MAXIMUM B E A R I N G end load deflection includes adjustin Xall load deflection is instantaneous. TAILIAU MIN BEARING SIZES (IN-SX) ndling & Erection design and material specified are in substantial formity with the latest revisions of NDS and AITC. • d load deflection includes adjustment factor for creep. porary and permanent bracing for heiding component be and for resisting isleral forces shall be designed and little by others. No loads are to be applied to the gonerd until after all the framing and issistating are picted. At no time shall loads graster than design loads BUT HALF MUST BE DRIVEN FROM 2006.1 Allowable Stress Design MAXIMOM DEFLECTIONS
CALCULATED ALLOWABLE
AD 0.00" 0.06"
AD 0.00" 0.13" 214 NUMBER Miscellaneous information

The use of this component shall be specified by the designer of the completes structure. Octain all the necessary code compliance approval and "Common nails driven parallel to give iness shall be "synuctions from the designers of the completes structure before using this instructions from the designers of the completes structure before using this code provides as the structure before using this code provides as the structure before using this code provides and sealed, the structural design is approved as shown in this drawing and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data provided by the customer. If LVL and CTR, LP Hoists are based on data pr UNI FORM
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UNI FORM DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JUISTS IS STRICTLY PROHBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. ANCHOR LP LYL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS WARNING NOTES: NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD C FOR PAITERU LUYE LOADING ARE CHECKED AS REQUIRED ( DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) FLOOR BEAM 16.000 CROSS SECTION DEAD LIVE WEIGHT DEAD ğ ig g LOAD TABLE LOAD FROM TO I 80 PLF 00-00-00 02-10-00 40 PLF 00-00-00 02-10-00 16 PLF 00-00-00 02-10-00 15 PLF 00-00-00 02-10-00 \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louislana-Pacific Corporation OTHER LOAD CASES THIS DRAWING IS NOT TO SCALE \*\*\* g Q 2 BEAMS 1.75 X 16.000 LPLVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 0.90 IDF 2706 Highway 421 North Wilmington, NC 28401 Local 910,762,9878 National Wats 800,999,9105 DWG SHEET LP Engineered Wood Products ROOF LIVE LOAD = ROOF DEAD LOAD = ROOF TOTAL LOAD = FIR LEFT SPAN CARR. FIR RIGHT SPAN CARR. ROOF LEFT SPAN CARR. FLOOR LIVE FLOOR DEAD FLOOR TOTAL DESIGN CRITERIA : CODE COMPLIANCES : REPORT DEFLECTION CRITERIA : LIVE LOAD DEFL: TOTAL LOAD DEFL: WISCONSIN NOOF RIGHT SPAN CARR. 0608-049 29 of 42 CITY City 90.05/80 ER-5004 RR 25167 11518-R 200124-W 10A0 40A0 40A0 MEA 97-94-E MR 1214D SBCCI 20 PSF 20 PSF 40 PSF 0.00 FT 2.00 FT : 0.00 FT нн // WS: 0.00 2006.1 PSF

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Handling & Erection  Temporary and permanent bracing for holding component installed by others. No bacts are to be applied to the component until after all the framing and fastering are completed. An online shall be also greater than despin loads code require the applied to the component.  Design Criteria  The design can malerial specified are in substantial conformity with the litest revisions of NDS and Artic.*  Treal load deflection is instantaneous.  Miscolaria  Miscolaria  Miscolaria  Installation  Miscolaria  Installation  Miscolaria  Installation  Miscolaria  Miscolaria  Installation  Miscolaria  Insta	SUPPORT REACTIONS (LBS):  MAXIMUM B E A R I N G N U M B E R  DOWN 713 1673 107  UPLIFT 83  MIN BEARING SIZES (IN-SX) 3-0 3-8 3-0  MAXIMUM DEFLECTIONS  CALCULATED ALLOWABLE LIVE LOAD 0.00" 0.13"  *DEAD LOAD 0.00" 0.13"  TOTAL LOAD 0.00" 0.27"	8. THIS LP LIVE IS TO BE USED AS A COMBINATION ROOF AND FLOOR BEAM ONLY. 7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENTS DESIGN ASSUMES COMPONENTS CAPPLED TO TOP EDGE OF LP LIVL, SUCH THAT LOAD IS DISTRIBUTED BEQUALLY TO EACH PLY. ATTACH THE TWO PLES WITH 3 ROWS OF 164 (3-127) NALLS AT 12 OC. STAGGER ROWS. NALLS CAN BE DRIVEN FROM ONE FACE OR NALS FROM EACH FACE. INALS MAY BE COMMON OR BOX NALS WITH A MINIMUM SHANK DIAMETER OF 0.131". 164 SINKERS (3-1/47) MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FAGE CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLES. ADDITIONAL FASTENERS MAY BE REQUIRED CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLES. ADDITIONAL	NOTE: 2006.1 Allowable Stress Design  1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS VERHICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIREDS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE UNITERAL STABILITY. 3. DO NOT CUT, NOTCH OR DRILL LP LVL. 4. SHIM ALL BEARNIOS FOR FULL CONTACT. 5. VERRY DIMENSIONS BEFORE CUTTING WILLIAM TO THE CONTACT.  4. SHIM ALL BEARNIOS FOR FULL CONTACT.  5. VERRY DIMENSIONS BEFORE CUTTING WILLIAM TO THE CONTACT.  WILLIAM TO THE CONTACT.
Miscellaneous information The use of this component shall be specified by the designer of the complete standars. Dobtain all the necessary code compliance approval and component. If the designers of the complete standars may be designered to the complete standars have been provided and seeded, in the design criteria lasted above does not meet local building code requirements, do not use all this desay, where his drawing a signed and seeded, the structural design is approved as shown in this drawing a signed made seeded, the structural design is approved as shown in this drawing based on dails provided by the customer, Le FLV, and CTR, L.P. I-plots are made without camber and will defined under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-the inspection. This drawing must have an Architect's or Engineer's seal shoot to be considered an Engineering document.	16.000  16.000  1.750  CROSS SECTION	THIS COMPONENT DESIGN IS SPECIFICALLY FOR L.P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LJOISTS IS STRUCTLY PROPUBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.  PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.  MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECTOR DESIGNER TO VERIFY THAT THE SUFPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.  PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS, ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER, ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER, ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.  ANCHOR LP LVL ROOFFILOOR BEAM SECURELY TO BEARINGS OR HANGERS.	NOTE: LOADS SHOWN ARE FOR INPUT FOR PATTERN LIVE COADING ARE CHE (DIMENSIONS MEASURED FROM LEFT) TRIBUTION SOURCE TYPE TOP/SI FORM ROOF LIVE TOP FORM FILOR LIVE TOP FORM FILOR LIVE TOP FORM FILOR LIVE TOP FORM FILOR DEAD TOP DEAD TOP CENTRATED ROOF LIVE TOP CENTRATED ROOF LIVE TOP CENTRATED ROOF DEAD TOP CENTRATED ROOF DEAD TOP CENTRATED ROOF DEAD TOP
LP LVL and CTR. LP I-Joist Specifications  Supports and connections for LP LVL and CTR, LP I-Jobsts to be specific applications.  Common rulat driven parallel to glue lines shall be specid a minimum of 4" for 10d and 3" for 6d.  On not cut, notich, drill or allor LP LVL and CTR, LP I-Joists except as abown in published material from LP and use of LP LVL, and CTR, LP I-Joists confravy to the limits set forth hereon, registes any superass warmarily of the product and LP discialnis et all implied versentibes holding the implied warmariles not form the product and LP discialnis et for a particular use.  10 A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisians-Pacific Corporation.	5- 6- 0 9- 3- 0 THIS DRAWING IS NOT TO SCALE ***	R L-P ENGINEERED WOOD PRODUCTS. MAN LP LVL OR LP I-JOISTS IS THIS DOCUMENT REQUIRES REVIEW  TO ENSURE LATERAL STABILITY.  REVENT CRUSHING OF THE LP LVL OF THE PROJECT ENGINEER, SUPPORT STRUCTURE FOR THIS ON. TO BEARINGS OR HANGERS.	2 BEAMS 1.75 X 16.000
LP Engineered Wood Products 2706 Highway 421 North Wilmington, NC 28401 Local 910.782,9878 National Wats 800.999.9105  DWG # 06  SHEET # 30			LP LVL2650Fb-1, 9E - PLIES FASTENED DTES):
0608-049		CODE COMPLIANCES: REPORT # ICBO ER-5004 L.A. City RR 25167 CCMC 11518-R MISCONSIN 200124-H NER 622 N.Y. CITY MEA 97-94-E HUD MR 1214D	PLOOR LIVE LOAD = FLOOR DEAD LOAD = FLOOR DEAD LOAD = FLOOR TOTAL LOAD = ROOF DEAD LOAD ROOF TOTAL LOAD ROOF TOTAL LOAD ROOF EARTH SEAN CARR. FLATER SEAN CARR. ROOF RIGHT SPAN CARR. ROOF RIGHT SPAN CARR. LOAD ROOF RIGHT SPAN CARR. LOEFLECTION CRITERIA : LIVE LOAD DEFL:
SBCCI 2008.1		, , , , , , , , , , , , , , , , , , ,	VSE 0.03 VSE 0.05 40 PSE 15 PSE 55 PSE 20 PSE 20 PSE 20 PSE 20.00 FT 20.00 FT 20.00 FT 1 / 480 L / 480

PSF PSF

LOAD IS DISTRIBUTED EQUALLY TO EACH PLY.
ATTACH THE TWO PLIES WITH 3 ROWS OF 16d
(3-1/27) NAILS, AT 12" OC. STAGGER ROWS.
NAILS CAN BE DRIVEN FROM ONE FACE OR HALF
FROM EACH FACE. NAILS MAY BE COMMON OR
BOX NAILS WITH A MINUMUM SHANK DIAMETER
OF 0.131". 16d SINKERS (3-1/4") MAY BE 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERHECATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT; DISCLAM ALL UNIT RESPONSIBILITY FOR ALL PLAYS, SPECIFICATIONS OR OTHER DOCUMENT THAT MAY DE USED TO UNIT OR OTHER D LATERAL STABLITY.

3. DO NOT CUT, NOTCH OR DRILL LP LVL.

4. SHIM ALL BEARINGS FOR FULL CONTACT.

5. VERIFY DIMENSIONS BEFORE CUTTING SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G
1 2 6. THIS LP LVL IS TO BE USED AS A 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE DOWN PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. indling & Erection HIN BEARING SIZES (IN-SX) xary and permanent bracing for holding component and for resisting intensi forces shall be designed and sor existing intensi forces shall be designed and so by others. No loads are to be applied to the world until after all the training and itseening are treed. At no time shall leads greater than design leads sign and material specified are in substantial nity with the latest revisions of NDS and AFTC. \* sad deflection includes adjustment factor for creep. BUT HALF MUST BE DRIVEN FROM 4621 2006.1 Allowable Stress Design MAXIMUM DEFLECTIONS
CALCULATED ALLOWABLE
AD 0.10" 0.26"
AD 0.06"
AD 0.15" 0.52" 4622 NUMBER The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approximationations from the designers of the complete structure before using component. If the design criteria listed above does not meet local by some requirements, do not use that the design. When this drawing is sign and seasied, the structural design is approved as shown in this drawing and seasied, the structural design is approved as shown in this drawing the same of the structural design is approved as shown in this drawing and seasied, the structural design is approved as shown in this drawing. Miscellaneous Information UNI FORM UNI FORM DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANTTHING OTHER THAN LP LVL OR LP L-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUISHING OF THE LP LYL. BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. ARCHITECT OR DESIGNET TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS. NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (), OTHER LOAD CASES FOR PATTERN LUXE LOADING ARE CHECKED AS REQUIRED (COMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) FLOOR FLOOR BEAM LIVE DEAD NEIGHT i i LOAD TABLE COAD FROM TO 1
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and of the 10-bosts continuy to the published material from LP any use of LP LVL and OTR, LP Lobists continuy to the published warrantees of the product and LP discalains all implied warrantees including the implied warrantees of menchanubility and fitness. \*A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP LVL and CTR, LP I-Joist Specifications alls driven perallel to give lines shall be spaced a mi for LP LVL and CTR, LP I-Joists to be specific applications, sitel to give lines shall be spaced a minimum of 4" for 10d LOAD THIS DRAWING IS NOT TO SCALE \*\*\* 2 BEAMS 1.75 X 14.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 0.90 0.90 JUL 2708 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wats 800.998.9105 DWG SHEET LP Engineered Wood Products NER N.Y. CITY HUD LIVE LOAD DEAD LOAD TOTAL LOAD OEDI CODE COMPLIANCES DEFLECTION CRITERIA : LIVE LOAD DEFL: TOTAL LOAD DEFL: FLR LEFT SPAN CARR. : 15.00 FT FLR RIGHT SPAN CARR. : 16.00 FT DESIGN CRITERIA : ISCONSIN 31 of 42 0608-049 City 08/10/06 MEA 97-94-E MR 1214D ER-5004 RR 25167 11518-R 200124-W REPORT SBCCI I / 480 I / 240 MSI: 0.47 VSI: 0.37

LP is a registered trademark of Louisiana-Pacific Corporation

2006.1

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Dead load deflection includes adjustment factor for creep. In Yotal load deflection is instantaneous.	Design Criteria The design and material specified are in substantial conformity with the latest revisions of NDS and ATC.	manent bracing for holding compone isting leteral forces shall be designed it. No loads are to be applied to the fler all the framing and fastening are three shall loads greater than design to component.	Handling & Erection	HAXIHOM DEFLICTIONS CALCULATED ALLOWABLE LIVE LOAD 0.25" 0.41" *DEAD LOAD 0.20" 0.82"	DOWN 11933 10523 UPLIFT	SUPPORT REACTIONS (LBS):					CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.	COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131", 16d SINKERS	BEAM AND ATTACH THE THRD PLY WITH 3 ROWS OF 164 (3-1/2") MAILS AT 12" OC TO THE UNLAWLED SIDE OF THE FREST TWO PLIES. STAGGER ROWS. NAILS MAY BE	ATTACH TWO PLES WITH 3 ROWS OF 18d  (3-12") NAILS AT 12" OC. FROM  ONE SACE ONLY STACCES BOWS ELIB	DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED FOUNTLY TO FACH PLY.	7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.	3. DO NOT CUT, NOTCH OR DRILL LP LVL. 4. SHIM ALL BEARNOS FOR FULL CONTACT. 5. VERIFY DIMENSIONS BEFORE CUTTING CONTROL TO SIZE. 6. THIS LP LVL IS TO BE USED AS A	BUILDING DESIGN 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSUR LATERAL STABILITY.	PROJECT ENGINEER OF ARCHITECT. LISCALD RESPONSIBILITY FOR ALL PLANS, SPECIFICATION OF OTHER DOCUMENTS THAT MAY BE USED TO THE TOTAL THAT THE TOTAL THE INCORPORATE THIS COMPONENT INTO THE INCORPORATE THE TOTAL THE INCORPORATE THE INC	BRACING, AND OTHER LATERAL BRACING THA	NOTE: 2008.1 Allowable Stress Design 1. THIS COMPONENT IS DESIGNED TO SUPPORT OTHERS, VERFICAL LOADS SHOWN AS DETECTION OTHERS, DEFFECTION OF LOADING DEFFECT
inspection. This drawing must have an Architect's or Engineer's seal afticed to be considered an Engineering document.	lado without cambor and will defined under load. Wood in direct contact tht concrete must be protected as required by code. Continuous lateral inport is assumed (wall, floor beam, etc.). LP does not provide on-site	The use of this component shall be specified by the designer of the and complete structure. Obtain all the encessary code compliates contained specified in the contained of the complete structure before using this component. If the design critical listed above does not never local building 1. Deads code requirements, do not use this design. When this drawing is signed and saled, the structural design is approved as shown in this drawing the contained of the customer. Let U. H. and CTRI, LP Hotsta are a	Alscellaneous Information	-	CROSS SECTION	19.000	<i>k</i> —						MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL. BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.		THIS COMPONENT DESIGN IS SPECIFICALLY FOR L.P ENGINEERED WOOD PRODUCTS.  USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL, OR L.P I-JOISTS IS  STRUCTLY PROHIBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW  BY A DESIGN PROFESSIONAL.	WARNING NOTES:	FORM ROOF DEAD TOP CENTRATED FLOOR LIVE TOP CENTRATED FLOOR LIVE TOP CENTRATED FLOOR DEAD TOP CENTRATED FLOOR DEAD TOP	FORM WALL DEAD FORM BEAM WEIGHT FORM ROOF LIVE	MALL UNIFORM FLOOR LIVE TOP ONS UNIFORM FLOOR DEAD TOP ONIFORM FLOOR DEAD TOP	ä	NOTE: LOADS SHOWN ARE FOR INPU
A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR  LP is a registered trademark of Louislans-Pacific Corporation.	er a pertoular use.	Supports and connections for LP LVL and CTR, LP I-oldsts to be specific applications.  Common natis driven persisted to glave fines abusine spaceds an minimum of 4° for 10d and 5° for 60.  Do not cut, notch, driff or eller LP LVL and CTR, LP I-oldsts sensest as shown in published material from LP any use of LP LVL and CTR, LP I-oldst contemy to the limits set froith hereon, regards any suppress warmany of the product and LP dischans all implied warmannies in-chaining the implied warmannies of materials.	LVL and CTR, LP I-Joist Specifications	THIS DRAWING IS NOT TO SCALE ***		8						TO BEARINGS OR HANGERS	EVENT CRUSHING OF THE LP LVL F THE PROJECT ENGINEER, SUPPORT STRUCTURE FOR THIS DNS.	TO ENSURE LATERAL STABILITY.	ALP ENGINEERED WOOD PRODUCTS. AM LP LVI, OR LP I-JOISTS IS THIS DOCUMENT REQUIRES REVIEW		PLF 00-00-00 LBS 11-02-00 LBS 01-07-00 LBS 11-02-00 LBS 01-07-00	PLF 01-07-00 11-02-00 0. PLF 00-00-00 16-08-00 0.	16-08-00 01-07-00 16-08-00 01-07-00	LOAD FOR THE CONTRACT LOAD LDF	TLOAD CASE (1), OTHER LOAD CASES TOCKED AS RECUMED CANTUE (FEB.) TO TO THE CANTUE (FEB.) TO TO THE CANTUE (FEB.) TO TO THE CANTUE (FEB.)
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32 of 42	0608-049		Products ownwo SBCCI			8			1 Muy					NER 622 N.Y. CITY MEA 97-94-E	ICBO ER-5004 L.A. City RR 25167 CCMC 11518-R WISCONSIN 200124-W	CODE COMPLIANCES : REPORT #	DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL:	ROOF LEFT SPAN CARR.	ROOF DEAD LOAD ROOF TOTAL LOAD FIR LEFT SPAN CARR. :	FLOOR TOTAL LOAD -	DESIGN CRITERIA : FLOOR LIVE LOAD = FLOOR DEAD LOAD =
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1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERFECATION OF LOADING, DETECTION
LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC
BRACING, AND OTHER LATERAL BRACING THAT IS
ALWAY'S REQUIRED IS THE RESPONSIBILITY OF THE
PROJECT ENGINEER OR ARCHITECT, I DISCLAIM ALL
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RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS
OR OTHER DOCUMENTS THAT MAY BE USED TO
ONLOGRAPORATE THIS COMPONENT INTO THE
BUILDING THESEN DUILDING DESIGN.

2 PROVIDE RESTRAINT AT SUPPORTS TO ENSURE
LATERAL STABLITY.

3. DO NOT GUT, NOTCH OR DRILL LP LVL.

4. SHIM ALL BEARINGS FOR FULL CONTACT.

5. VERIFY DIMENSIONS BEFORE CUTTING Aonez Res. / Beam-P (at Front of House) 6. THIS LP LVL IS TO BE USED AS A SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G
1 2 DOWN NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER MAKE PROVISION FOR ADEQUATE DRAINAGE.
PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. JESIGN ASSUMES COMPONENTS CARRIED ARE
PAPLIED TO TOP EDGE OF LP LVL, SUCH THAT
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ATTACH THE TWO PLIES WITH 2 ROWS OF 184
3-1(2") NAILS AT 12" OC. STAGGER ROWS. Inding & Erection MIN BEARING SIZES (IN-SX) porary and permanent bracing for hosting component to and for resisting lateral forces shall be designed and liked by others. No loads are to be applied to the ponent until after all the framing and fastening are ponent until after all the framing and fastening are design and malerial specified are in substantial ormity with the latest revisions of NDS and AITC. \* I load deflection includes adjustment factor for creep. MAXIMUM DEFLECTIONS
CALCULATED ALLOWABLE
AD 0.09" 0.39"
AD 0.15" 0.58" 1328 NUMBER If The use of this component shall be specified by the component shall be specified to complete structure. Obtain all the necessary on historications from the designant of the complete component. If the design cateria fisted above component, if the design cateria fisted above and sealed, the structural design is approved a Miscellaneous Information UNI FORM UNI FORM UNI FORM DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP I-JOISTS IS STRICTLY PROHIBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL.
BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER,
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BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. NOTIE: LOADS SHOWN ARE FOR NIPUT LOAD CASE (1), OTHER LOAD (FOR PATTERN LIVEL DADING ARE CHECKED AS REQUIRED FOR PATTERN LIVEL DADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTLEVER.) ROOF BEAM 9.500 CROSS SECTION ied by the designer of the LIVE DEAD WEIGHT ig gg LOAD TABLE LOAD FROM TO I 108 PLF 00-00-00 11-10-00 108 PLF 00-00-00 11-10-00 10 PLF 00-00-00 11-10-00 A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR Oo not cal, noich, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL, and CTR, LP I-Joists contrary to the limits set forth hereon-negates any express verticity of the product and LP disclaims all implied warmarites including the implied warmarites including the implied warmarites of menchantability and fitness for a particular use. LP LVL and CTR, LP I-Joist Specifications LP is a registered tradement of Louisiana-Pacific Corporation. and connections for LP LVL and CTR, LP L-loists to be specific applications, nells driven parallel to give lines shall be spaced a minimum of 4° for 10d OTHER LOAD CASES 7 LOAD \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 2 BEAMS 1.76 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 0.90 **LDF** 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wats 800.989.9105 DWG SHEET LP Engineered Wood Products DEAD LOAD TOTAL LOAD NER N.Y. CITY HUD DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LCAD DEFL: DESIGN CRITERIA : CODE COMPLIANCES : ROOF LEFT SPAN CARR. VISCONSIN 33 of 42 0608-049 City 00/10/06 ER-5004 RR 25167 11518-R 200124-W 622 MEA 97-94-E MR 1214D SBCCI ' ^ 0.0 00 MSI: 0.32 VSI: 0.18 2006.1 98F 98F 77

3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING THIS COMPONENT IS DESIGNED TO SUPPOPATIONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
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RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS
OR OTHER DOCUMENTS THAT MAY BE USED TO
INCORPORATE THIS COMPONENT INTO THE UNIFORM UNIFORM UNIFORM DISTRIBUTION

6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER JESIGN ASSUMES COMPONENTS CARRIED ARE PAPILED TO TOP EDGE OF LP LUL, SUCH THAT (ADAD IS DISTRIBUTED EQUALLY TO EACH PLY. (TTACH THE TWO PULES WITH 2 ROWS OF 184 3-127) NAILS AT 12" OC. STAGGER ROWS.

6 <u>63</u>		NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD FOR PAITERN LIVE LOADING ARE CHECKED AS REGUIRED. (DIMENSIONS MEASURED PROM LEFT END OF SPAN OR CANTILEVER.)	
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	3	RED. RED. R CANTILEVER.)	
ND CASES			

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DEAD LOAD
TOTAL LOAD

20 20

PSF

DESIGN CRITERIA

WSF 0.12 VSF 0.16

SOURCE TYPE LIVE DEAD WEIGHT TOP/SIDE ğğ LOAD LDF

153 153 10 2 PLF 00-00-00 3 PLF 00-00-00 3 PLF 00-00-00 12-05-00 12-05-00 0.90

WARNING NOTES:

ROOF BEAM

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP 1-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUISHING OF THE IP LYL. BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. ARCHITECT OR DESIGNED TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

CODE COMPLIANCES:
REPORT #
ICBO ER-5004
L.A. City RR 25167
CCMC 11518-R
MISCONSIN 200124-W

DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL:

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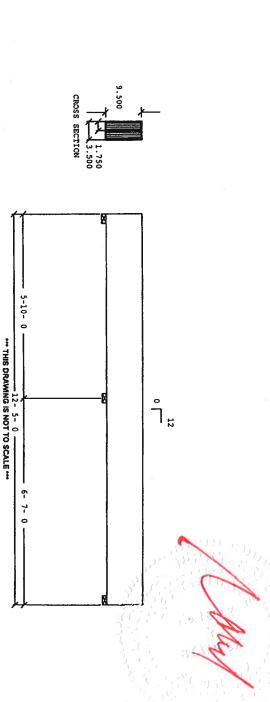
ROOF LEFT SPAN CARR.

.. .. 0.00 FT

ICBO
L.A. City
CCMC
WISCONSIN
NER
N.Y. CITY

MEA 97-94-E MR 1214D

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.



morany and permanent bracing for holding component mb and for resisting lateral forces shall be designed and failed by others. No loads are to be applied to the mponent until after all the framing and festining are ndling & Erection design and material specified are in substantial formity with the latest revisions of KDS and AITC. \* did and deflection includes adjustment factor for creep, all load deflection is instantaneous.

SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G
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NUMBER

UPLIFT DOWN.

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MIN BEARING SIZES (IN-SX)

MAXIMUM DEFLECTIONS CALCULATED ALLOWABLE AD 0.01" 0.27 AD 0.01" 0.32" AD 0.01" 0.32" ALLOWABLE 0.22

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> LP LVL and CTR, LP I-Joist Specifications and connections for LP LVL and CTR, LP I-Joists to be specific applications, nalls driven parallel to give lines shall be spaced a minimum of 4" for 10d

and 3" for 6d.

Do not cut, notich, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Josts contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

\* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louislana-Pacific Corporation

> 2708 Highway 421 North Wilmington, NC 28401 Local 910.762.1 Local 910.762.9878 National Wats 800.999.9105 LP Engineered Wood Products 06/10/06 SBCCI

> > 2008.1

34 of 42 0608-049

DWG

SHEET

SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G N
1 2 636
DOWN 636 636
UPLIFT ---1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
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BRACING, AND OTHER LATERAL BRACING THAT IS
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PROVIDE RESTRAINT AT SUPPORTS TO ENSURE (3-12") NAILS AT 12" OC. STAGGER ROWS.
VAILS CAN BE DRIVEN FROM ONE FACE OR HALF
FROM EACH FACE. NAILS MAY BE COMMON OR
BOX NAILS WITH A MINIMUM SHANK DIAMETER NTTACH THE TWO PLIES WITH 2 ROWS OF 16d MAKE PROVISION FOR ADEQUATE DRAINAGE. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. andling & Erection MIN BEARING SIZES (IN-SX) MAXIMUM DEFLECTIONS
CALCULATED ALLOWABLE
AD 0.01" 0.24"
AD 0.02" 0.36"
AD 0.02" 0.36" holding component hell be designed and applied to the NUMBE Viscellaneous Information UNIFORM UNIFORM UNIFORM DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD ANCHOR LP LYL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNED TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP 1-JOISTS IS STROTLLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. WARNING NOTES. NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD OF FOR PATTERN LUYE LOADING ARE CHECKED AS REGULIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) ROOF BEAN CROSS SECTION LIVE DEAD WEIGHT agi agi LOAD TABLE Do not cut, notch, drill or after LP LVL, and CTR, LP Holists except as shown in
published material from LP any use of LP LVL, and CTR, LP Holists contrary to the
limits set forth hereon, negates any express warrantly of the product and LP disclaims
all implied vernandes including the implied warrantles of merchantability and fitness
for a particular use. \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP LVL and CTR, LP I-Joist Specifications 10 88 88 LP is a registered trademark of Louisiana-Pacific Corporation ID FROM TO 1
FT-IN-SX FT-IN-SX
0 PLF 00-00-00 07-06-00
PLF 00-00-00 07-06-00
PLF 00-00-00 07-06-00 ports and connections for LP LYL and CTR, LP i-joists to be specific applications, minor nails driven parallel to give lines shall be spaced a minimum of 4" for 10d OTHER LOAD CASES THIS DRAWING IS NOT TO SCALE \*\*\* IOAD 2 BEAMS 1.78 X 9.500 LP LVL2850Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED
TOGETHER (REFER TO NOTES). ا ۳ 0.90 LDF 2708 Highway 421 North Wilmington, NC 28401 Local 910.762.8678 National Wats 800.999.9105 DWG SHEET LP Engineered Wood Products ICBO
L.A. City
CCMC
WISCONSIN NER N.Y. CITY HUD DEAD TOTAL DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA CODE COMPLIANCES : ROOF LEFT SPAN CARR. 0608-049 35 of 42 10A0 0A0 0A0 90/01/80 REPORT # ER-5004 RR 25167 11518-R 200124-W MEA 97-94-E MR 1214D SBCCI .. .. L / 360 L / 240 0.00 FT WSI: 0.09 2006.1 PSF PSF PSF

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
OTHERS, VERTICATION OF LOADING, DEFLECTION
LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC
BRACING, AND OTHER LATERAL BRACING THAT IS
ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE
PROJECT ENGINEER OR ARCHITECT I DISCLAIM ALL TRA
RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS
OR OTHER DOCUMENTS THAT MAY BE USED TO
UNCORPORATIE THIS COMPONENT INTO THE 3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING 6. THIS LP LVL IS TO BE USED AS A BUILDING DESIGN.

2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE MAXIMUM BEARING NUMBER
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CALCULATED ALLOWABLE
AD 0.01" 0.24"
AD 0.02" 0.36" ing component be designed and Miscellaneous Information TRAPEZOIDAL UNIFORM DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNED TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS NOTE: LOADS SHOWN ARE FOR NIPUT LOAD CASE (1), OTHER LOAD FOR PAITTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) be undersomed as shown in this drawing the customer. LP LVL and CTR, LP Horists are lift deflect under load. Wood in direct contact cade as required by code. Continuous lateral to the horizon and 1 in accordance. ROOF LIVE DEAD WEIGHT ដូដ្ឋ LOAD TABLE LOAD FROM TO I FT-IN-SX FT-IN-SX 20 PLF 00-00-00 07-06-00 20 PLF 00-00-00 07-06-00 10 PLF 00-00-00 07-06-00 \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP LVL and CTR, LP I-Joist Specifications Do not out, notch, drill or after LP LVL and CTR, LP Libets except as shown in published material from LP any use of LP LVL and CTR, LP Libets contrary to the process at 6 orth hereon-nogates are vegors a wrampy of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness. LP is a registered trademark of Louisians-Pacific Corporation. OTHER LOAD CASES for LP LVL and CTR, LP I-Joists to be specific applications. file to clue lines shall be spaced a minimum of 4° for 10d THIS DRAWING IS NOT TO SCALE \*\*\* 100 PLF 1.25 100 PLF 0.90 0.90 LOAD ຳ<sub>ະ</sub> 2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 10F 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wats 800.999.9105 SHEET DWG LP Engineered Wood Products ICBO
L.A. City
CCMC
WISCONSIN DEAD LOAD
TOTAL LOAD HUD. DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA CODE COMPLIANCES ROOF LEFT SPAN CARR. 36 of 42 0608-049 CITY 08/10/06 REPORT # ER-5004 ER-5167 11518-R 200124-W 622 MEA 97-94-E MR 1214D SBCCI ... rr // 20 40 SIS 360 2006.1 0.00 33 PSF PSF

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERHICATION OF LOADING, DEFECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING TAAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT, I DISCLAIM ALL VIAIS SEPONSIBILITY FOR ALL PLANS, SPECIFICATIONS ON THER DOCUMENTS THAT MAY BE USED TO UNITABLE OF THE PROJECT OF THE PROJET OF THE PROJECT OF THE PROJECT OF THE PROJECT OF THE PROJECT O DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 18d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER 6. THIS LP LIV. IS TO BE USED AS A ROOF BEAM ONLY.

MAKE PROVISION FOR ADEQUATE DRAINAGE.

7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. 3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING
LP LVL TO SIZE. BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G | 1 DOWN ndling & Erection INCORPORATE THIS COMPONENT INTO THE orary and permanent bracing for holding con and for resisting lateral forces shall be dealy and for resisting lateral forces shall be dealy and by others. No loads are to be applied to I whent until after all the framing and fastening BEARING 3- 0 LOAD 2006.1 Allowable Stress Design MAXIMUM DEFLECTIONS
CALCULATED ALLOWABLE
AD 0.03" 0.32"
AD 0.00" 0.47" 2312 ALLOWABLE 0.32" NUMBER ding component
be designed and
piled to the The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compilance appro-Miscellaneous Information UNIFORM UNIFORM UNIFORM DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNET OVERIEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP 1-JOISTS IS STRUCTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. WARNING NOTES NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (), OTHER LOAD CASES FOR PAITERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) iop gor LOAD TABLE LOAD FROM TO L 106 PLF 00-00-00 18-11-00 80 PLF 00-00-00 18-11-00 10 PLF 00-00-00 18-11-00 Supports and connections for IP IVI, and CTR, IP I-kolast to be specific applications. Common sales others parallel to give fires shall be specific an minimum of 4° for 10d. The first of the control of LP LVL and CTR, LP I Joist Specifications \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation 9- 7- 0 LOAD \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 0.90 0.90 301 ٦ ټ 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wats 800.999.9105 DWG LP Engineered Wood Products 9 4-0 LIVE LOAD DEAD LOAD TOTAL LOAD DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA : ROOF LEFT SPAN CARR. ROOF RIGHT SPAN CARR. 37 of 42 0608-049 08/10/06 SBCCI .. .. 0.0 88 WSt 0.18 VSI: 0.16 2006,1 PSF PSF 33

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1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
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RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS
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OR OTHER DOCUMENTS THAT MAY BE USED TO
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NICORPORATE THIS COMPONENT INTO THE LATERAL STABILITY.

3. DO NOT CUT, NOTCH OR DRALL LP LVL.

4. SHIM ALL BEARINGS FOR FULL CONTACT.

5. VERIFY OINENSONS BEFORE CUTTING
LP LVL. TO SIZE. BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE MAXIMUM B E A R I N G 6. THIS LP LVL IS TO BE USED AS A DOWN DESIGN ASSUMES COMPONENTS CAPRIED ARI APPLIED TO TOP EDGE OF IP ILVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 18d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. RAOOF BEAM ONLY.

MAKE PROVISION FOR ADEQUATE DRAINAGE.
PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. TOTAL LOAD VAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER MIN BEARING idling & Erection rary and permanent bracing for holding component and for resisting lateral forces shall be designed and and for resisting lateral forces are to be applied to the nant until after all the framing and fastening are ted. At no time shall loads greater than design heads 2006.1 Allowable Stress Design HALF MUST BE DRIVEN FROM CALCULATED
O.03"
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0.03" C.32" 3- 8 3- 0 2312 678 NUMBER 0.47" The use of this component shall be specified by the designer of the complete shructure. Obtain all the necessary code compilation approval and instructions from the designors of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, so not use this design. When this drawful signed and sealed, the structurel design is approved as shown in this drawful. UNI FORM DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD UNIFORM MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNET TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LYL OR LP L-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. WARNING NOTES: ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. NOTE: LOADS SHOWN ARE FOR NIPUT LOAD CASE (), OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED (OMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) ROOF BEAM 9.500 CROSS SECTION LIVE WEIGHT LOAD TABLE LOAD FROM TO 1

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80 PLF 00-00-00 18-11-00

10 PLF 00-00-00 18-11-00 Supports and connections for LP LVL and CTR, LP IL-losts to be specific applications. of Common nails driven partifiel to glue lines shall be spaced a minimum of 4° for 10d and 3° for 8d.
Do not cut, notch, drill or after LP LVL and CTR, LP IL-losts except as shown in published material from LP any use of LP LVL and CTR, LP IL-losts containty to the limits set forth hereon negates any express warmany of the product and LP discisions all implied warranties including the implied warranties of merchantability and fibress LP LVL and CTR, LP I-Joist Specifications A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisians-Pacific Corporation 9- 7- 0 LOAD D \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 2 BEAMS 1.75 X 8.500 LP LVL2660Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 1.00 0.90 IDF 18-11-0 ٦٣ 2706 Highway 421 North Wilmington, NC 28401 Local 910.762,9878 National Wats 800,988,9105 DWG SHEET LP Engineered Wood Products ۴ 4-0 ICBO
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DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EXCSE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLV. ATTACH THE TWO PUES WITH 2 ROWS OF 18d (2-1/27) NALLS AT 12" OC. STAGGER ROWS.

NALLS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NALLS MAY BE COMMON OR BOOK NALLS WITH A MINUMUM SHANK DAMETER OF 0 131". 16d SINKERS (3-1/4") MAY BE 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
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UNI LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING 6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE TOTAL LOAD MAXIMUN B E A R I N G MAKE PROVISION FOR ADEQUATE DRAINAGE. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. andling & Erection MIN BEARING 3- 0 yary and permanent bracing for holding component and for resisting lateral forces shall be designed and all problems. No loads are to be applied to the ment until after all the framing and fastening are and material specified are in substantial with the latest revisions of NDS and AITC. \*defaction includes adjustment factor for creep. 2006.1 Allowable Stress Design MAXIMOM DEFLECTIONS
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AD 0.01" 0.24"
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ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER. ARCHITECT OR DESIGNED TO VERHEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: ANCHOR LP LYL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. NOTE: LOADS SHOWN ARE FOR NIVITIOAD CASE (), OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REDUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) 9.500 CROSS SECTION LIVE DEAD WEIGHT TOP PQF LOAD TABLE 5 Do not cut, notch, drill or after LP LVL and CTR, LP L-Joists except as shown in
published material from LP any use of LP LVL and CTR, LP L-Joists contrary to the
limits set forth hereon, negates any express warrastly of the product and LP disclaints
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CODE COMPLIANCES:

REPORT #

ICBO RR-5004
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TW MEA 97-94-E

MR 1214D DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: LIVE LOAD DEAD LOAD TOTAL LOAD LIVE ROOF LEFT SPAN CARR. DESIGN CRITERIA : 39 of 42 0608-049 06/10/06 SBCCI 0.0 80 400 WSI: 0.07 2008.1 PSF PSF PSF 33

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DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF IP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-17) NALLS AT 12" OF STAGGER ROWS. WALLS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NALLS MAY 3E COMMON OR BOX NALLS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-14") MAY BE 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY
THE VERTICAL LOADS SHOWN AS DETERMINED BY
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5. VERIFY OINENSIONS BEFORE CUTTING
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BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER,
ARCHITECT OR DESIGNER TO VERIET THAT THE SUPPORT STRUCTURE FOR THIS
BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (), OTHER LOAD CASES FOR PAITERN LIVE LOADING ARE CHECKED AS REQUIRED.

(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) ROOF ROOF BEAM LIVE DEAD DEAD ដូដូដូដូ LOAD TABLE \*Common naise annew parents of the CTR, LP ILloists except as shown in end of the Rd. Policy except as shown in the Do not out, notch, drill or after LP LVI, and CTR, LP ILloists contrary to the published material from LP any use of LP LVI, and CTR, LP ILloists contrary to the imits set forth becon agentar any express warnarity of the product and CP disclaims all implied warranties including the implied warranties of merchantability and fitness. LOAD FROM TO I RT-IN-SX FT-IN-SX FT-IN \* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
 Common nails driven parallel to glue lines shall be spaced a minimum of 4° for 10d LP LVL and CTR, LP I-Joist Specifications LP is a registered trademark of Louisians-Pacific Corporation 7-10- 0 LOAD \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 00 1.00 D PLF 0.90 D PLF 1.25 0.90 2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.8E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). LDE 15- 5- 0 ٦٣ 2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wals 800.989.9105 DWG LP Engineered Wood Products 7- 7- 0 ICBO L.A. City CCMC WISCONSIN DEAD LOAD
TOTAL LOAD DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: CODE COMPLIANCES ROOF LEFT SPAN CARR. : 0.00 FT DESIGN CRITERIA 40 of 42 0608-049 CITY 08/10/06 ER-5004 RR 25167 11518-R 200124-W 622 MEA 97-94-E MR 1214D SBCCI rr '' 20 40 MSI: 0.08 360 240 2008.1 PSF

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DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 18d (2-1/27) NAILS AT 1/2" OC. STAGGER ROWS.

NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DAMETER 3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING 6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE MAXIMUM BEARING MAKE PROVISION FOR ADEQUATE DRAINAGE.
PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT. le; L:\Engineering\Woode 2006\August\0608-049\6038-U.SPX THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERIFICATION OF LOADING, DEFLECTION LUMITATIONS, FRAMING METHODS, WIND AND SESSICE BRACING, AND OTHER LATERAL BRACING THAT IS I ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE MIN BEARING SIZES (IN-SX) CORPORATE THIS COMPONENT INTO THE ROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL ESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS ROTHER DOCUMENTS THAT MAY BE USED TO dling & Erection ray and permanent bracing for holding component ind for resisting lateral forces shall be designed and by others. No leads are to be applied to the earl until after all the framing and fastering are 2006.1 Allowable Stress Design MAXIMUM DEFLECTIONS
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AD C.01" 0.26"
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AD 0.01"
0.02" 0.39" 1330 NUMBER DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD TRAPEZOIDAL TRAPEZOIDAL UNIFORM UNIFORM UNIFORM THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANTHING OTHER THAN LP LYL OR LP LJOISTS IS STRICTLY PROMBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL BEAM AS DESCAVED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. WARNING NOTES: NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (), OTHER LOAD CASES FOR PATTERN LUYE LOADING ARE CHECKED AS REQUIRED. (CIMENSIONS MEASURED FROM LEFT END OF SPANOR CANTILEVER.) ROOF ROOF BEAM CROSS SECTION **2**222 LOAD TABLE and 3" for 6d.

Do not cut, notich, drill or alter LP LVI, and CTR, LP L-loists except as shown in published material from LP any use of LP LVI, and CTR, LP L-loists contany to the justished material from LP any express warrany of the product and LP disclaims all implied warrantes including the implied warrantes of merchantability and fitness LOAD FROM TO L #T-IN-SX FT-IN-SX 67-IN-SX 67-IN A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR Supports and connections for LP LVL and CTR, LP iJoists to be specific application Common nails driven parallel to give lines shall be spaced a minimum of 4" for 10d LP LVL and CTR, LP I-Joist Specifications LP is a registered trademark of Louislana-Pacific Corporation 7-11- 0 IOAD \*\*\* THIS DRAWING IS NOT TO SCALE \*\*\* 00 1.00 0 PLF 0.90 0 PLF 1.25 0.90 2 BEAMS 1.76 X 9.500 LP LVL2850Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). LDF -14-11-0-2706 Highway 421 North Wilmington, NC 28401 Local 910.762.9878 National Wats 800.999.9105 SHEET DWG LP Engineered Wood Products 7-٩ 0 CCMC DEAD TOTAL DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA: CODE COMPLIANCES ROOF LEFT SPAN CARR. \*ISCONSIN 41 of 42 0608-049 CITY City LOAD LOAD 00/10/06 REPORT # ER-5004 RR 25167 11518-R 200124-W 622 MEA 97-94-E MR 1214D SBCCI 0.0 00 å MSI: 0.08 VSI: 0.10 360 2006.1 T I **984** 

File: L.\Engineering\Woode 2006\August\0508-049\6038-V.SPX	and order or includes adjustment factor for creep. Install lead deflection includes adjustment factor for creep. Install lead deflection is installed and adjustment for creep.	arrib and for resisting lakes flores shall be designed and of statisfied by others. No loads are to be applied to the component until after at the framing are omposed. At no time shall loads greater than design loads on applied to the component.	Handling & Erection M	LIVE LOAD 0.01" 0.30"	m   ii	SUPPORT REACTIONS (LBS): MAXIMUM BEARING NUMBER TOTAL 10 20		LOAD IS DISTRIBUTED EQUALLY TO SACH FIVE ATTACH THE TWO PILES WITH 2 ROWS OF 184 (3-1/27) NAILS AT 12" CO. STAGGER ROWS.  NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 164 SINNERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.	NOT BEAM ONLY.  ROOF BEAM ONLY.  MAKE PROVISION FOR ADEQUATE DRAINAGE.  PROVIDE COMPRESSION EDGE BRACING AT  EACH END OF COMPONENTS CARRIED ARI ARBITUTE TO THE COMPONENTS CARRIED ARI ARRIED ARI ARBITUTE TO THE COMPONENTS CARRIED ARI ARRIED ARI ARRIED ARIED ARIE	3. DO NOT CUT, NOTCH OR DRILL LP LVL. 4. SHIM ALL BEARINGS FOR FULL CONTACT. 5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL. TO SIZE.	RESPONSIBILITY FOR ALL PLANS, SPECIFICAN OR OTHER DOCUMENTS THAT MAY BE USED INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN. 2. PROVIDE RESTRANT AT SUPPORTS TO ENSU	THE VERTICAL LOADS SHOWN AS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERHFCATION OF LOADING, DEFLECTION UNITATIONS, FRAMING METHODS, WIND AND SESMIC BRACING, AND OTHER LATERAL BRACING THAT IS DISACKING AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE	NOTE: 2006.1 Allowable Stress Dosign
SVSPX	support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Arothect's or Engineer's soal affixed to be consistent as Forinsearch document.	plants and for resisting lateral forces shall be designed and journeement and the position by the designer of the feathers and for resisting lateral forces shall be designed and journeement will after all the farming and fastening are completed. At no time shall be specified and journeement will after all the farming and fastening are completed. At no time shall loads greater than design loads code requirements, do not use that design. When this drawing its signed be applied to the component. If the design is approved as shown in this drawing beautiful control and the component. If the design is approved as shown in this drawing beautiful control and the component. If the design is approved as shown in this drawing beautiful control and the component. If the design is approved as shown in this drawing beautiful control and the component. If the design is approved as shown in this drawing beautiful control and the component of the control and the component of the control and t	Miscellaneous Information	3	CROSS SECTION	9.500	<b>/</b>	~ 5	MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL. BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROLECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.  ANCHOR LP LYL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.	THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP L-JOISTS IS STRICTLY PROHIBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL		NOTE: LOADS SHOWN ARE FOR INPUFOR PATTERN LIVE LOADING ARE CHE (DIMENSIONS MEASURED FROM LEFT STRIBUTION SOURCE TYPE TOP/SI	
LP is a registered trademark of Louisiana-Pacific Corporation.		Supports and connections for LP LVI, and CTR, LP I-loists to be specific applications of "Common nails driven parallel to glue lines shall be apposed a minimum of 4" for fod and 3" for 8d, of "Do not cut, match, drill or after LP LVI, and CTR, LP I-loists except as shown in published maintenal from LP any use of LP LVI, and CTR, LP I-loists contrary to the limits self forth hereon, regards any exponess warranty of the product and LP disclaims at all implied warranties including the implied warranties of marchantability and fitness for a particular use.	LP LVL and CTR, LP I-Joist Specifications	6- 2- 0		<b>X</b>	0 7 12		PREVENT CRUSHING OF THE LP LVL. OF THE PROJECT ENGINEER, E SUPPORT STRUCTURE FOR THIS TIONS. FARINGS OR HANGERS.	OR L-P ENGINEERED WOOD PRODUCTS. THAN LP LVL OR LP I-JOISTS IS F THIS DOCUMENT REQUIRES REVIEW	00+	× Load	LOAD TABLE 7 BEA
SHEET #			LP Engineered Wood Products	TO SCALE		<b>6</b>					90	DESIGN CONSISTS OF 2 - PLES FASTENED TOGETHER (REFER TO NOTES).	NO 4.75 V 0.550 ID 10.0500 A.75
42 of 42	0608-049	SBCCI							ICBO ER-5004 L.A. City RR 25167 CCMC 11518-R WISCONSIN 200124-W NER 62 N.Y. CITY MEA 97-94-E HUD MR 1214D		ROOF RIGHT SPAN CARK. : 0.00 FT ROOF RIGHT SPAN CARR. : 0.00 FT DEFLECTION CRITERIA : LIVE LOAD DEFL: TOTAL LOAD DEFL: L / 240	40 SI	