DATE 09/01/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024936

APPLICANT ALBERT E. MONIZ PHONE 386.454.0855

ADDRESS 455 SW CHALET TERRACE FT. WHITE FL 32038

OWNER ALBERT E. MONIZ PHONE 386.454.0855

ADDRESS 455 SW CHALET TERRACE FT. WHITE FL 32038

CONTRACTOR ALBERT E. MONIZ, OWNER BLDR. PHONE 385.454.0855

LOCATION OF PROPERTY 47-S TO US 27,TL TO C-138,TR TO CHALET,TR AND IT'S THE 8TH PLACE ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 177950.00

HEATED FLOOR AREA 3559.00 TOTAL AREA 5733.00 HEIGHT ____ STORIES 2

FOUNDATION CONC WALLS FRAMED ROOF PITCH 12'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO. ____

PARCEL ID 19-7S-17-10025-114 SUBDIVISION ____

LOT ____ BLOCK ____ PHASE ____ UNIT ____ TOTAL ACRES 10.01

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
			Albert E Moniz

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	06-0386-N	BLK	JTH	N

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD.

Check # or Cash 9484

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by Foundation ____ date/app. by Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by Slab ____ date/app. by Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by Heat & Air Duct ____ date/app. by Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by C.O. Final ____ date/app. by Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by Pool ____ date/app. by

Reconnection ____ date/app. by Pump pole ____ date/app. by Utility Pole ____ date/app. by

M/H Pole ____ date/app. by Travel Trailer ____ date/app. by Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 890.00 CERTIFICATION FEE $ 28.66 SURCHARGE FEE $ 28.66

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ ____

FLOOD DEVELOPMENT FEE $ ____ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ ____ **TOTAL FEE** 1022.32

INSPECTORS OFFICE CDD CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-0

For Office Use Only Application # 0608-70 Date Received 8/18/06 By GP Permit # 24936

Application Approved by - Zoning Official BLK Date 30.08.06 Plans Examiner OK JTH Date 8-31-06

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments #

existing well

Applicants Name Albert E. Moniz Phone 386 454-0855

Address 455 S.W. Chalet Terr., Ft. White, FL 32038

Owners Name Albert E. Moniz Phone 386) 454-0855

911 Address 455 S.W. Chalet Terr, Ft. White, FL 32038

Contractors Name Owner/Builder Phone

Address Same as above

Fee Simple Owner Name & Address SAME AS ABOVE

Bonding Co. Name & Address

Architect/Engineer Name & Address John Yencho, 5559 S.W. CR 313, Trenton, FL 32693

Mortgage Lenders Name & Address None

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number R10025-114 19-7S-17 Estimated Cost of Construction 287,000

Subdivision Name Lyn-Dee Dairy Lot 14 Block Unit Phase

Driving Directions 475 U.S. 27 So. to CR 138, TR 138 W. to CHALET Terr., Chalet Terr., No. to 455. 8th on right TR

Type of Construction FRAME SFD Number of Existing Dwellings on Property 0

Total Acreage 10.01 Lot Size Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Driv

Actual Distance of Structure from Property Lines - Front 375 ft. Side 774 ft. Side 170 ft. Rear 1046 ft.

Total Building Height Number of Stories 2 Heated Floor Area 3559 sq ft TOTAL 5733 Roof Pitch 12X12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Albert E Moniz

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 3 day of August 2006.

Personally known ✓ or Produced Identification

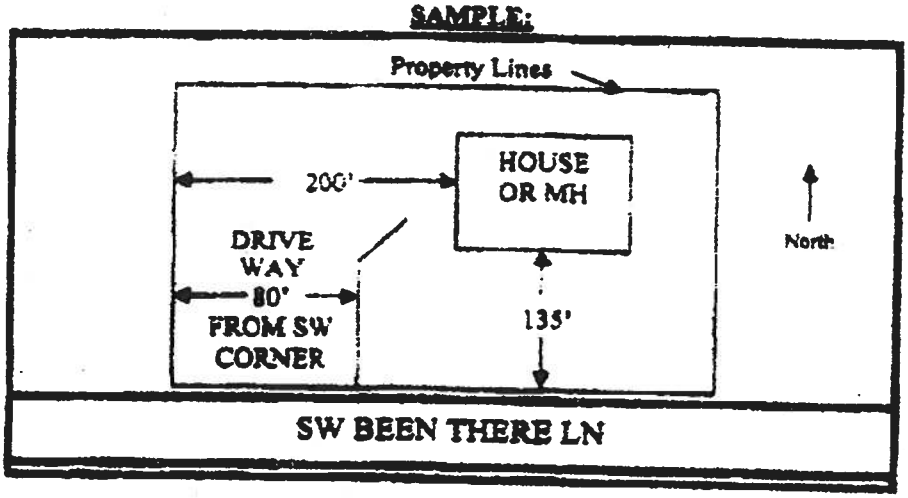
Notary Signature

Contractor Signature
Contractors License Number
Competency Card Number
NOTARY STAMP/SEAL

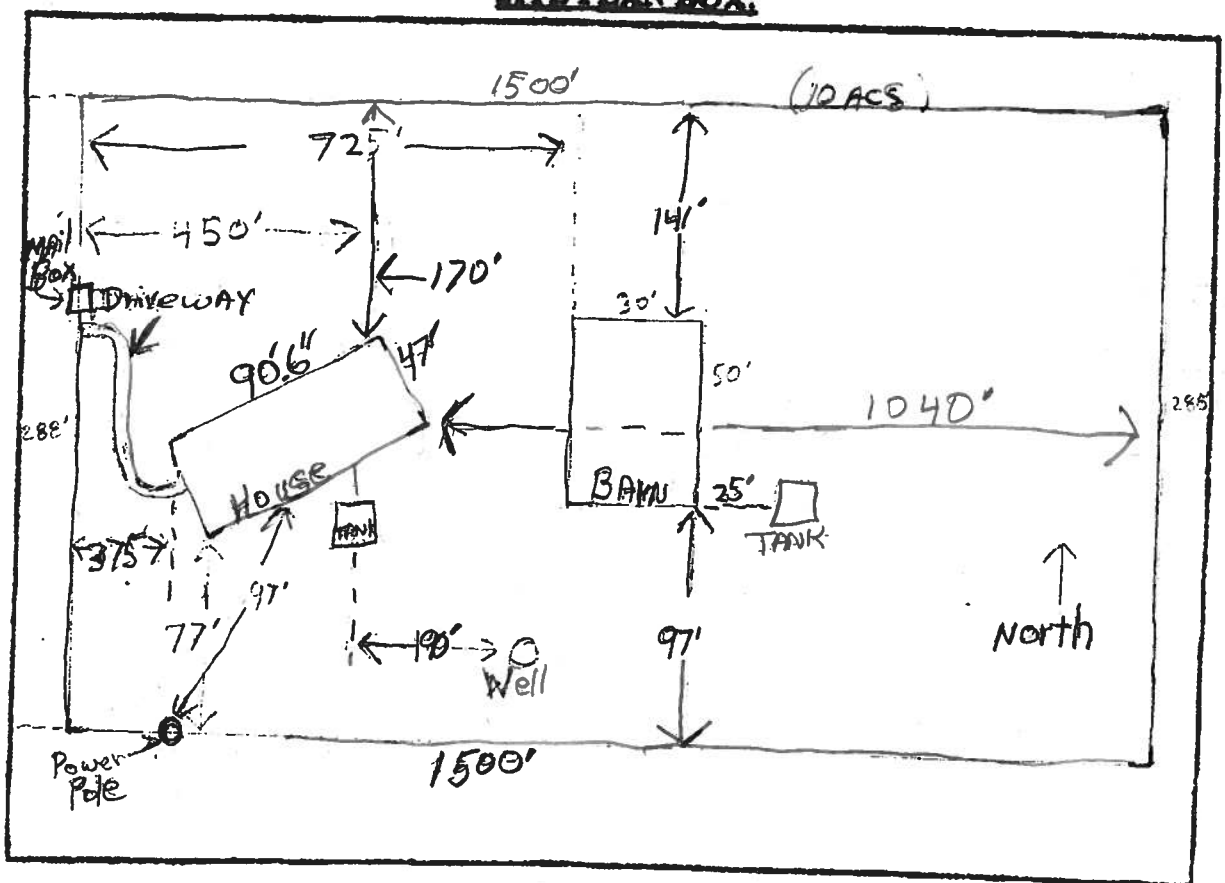
NOEL BURMEISTER
MY COMMISSION # DD536652
EXPIRES: June 22, 2010
1-800-3-NOTARY Fl. Notary Discount Assoc. Co.

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:



455 S.W Chalet Terr. Ft. White Fl. 32038
Albert + Pat Moniz 386-454-0855

RONNIE BRANNON, CFC
COLUMBIA COUNTY TAX COLLECTOR

2005 REAL ESTATE
NOTICE OF AD VALOREM TAXES AND NON-AD VALOREM ASSESSMENTS

01290230000

ACCOUNT NUMBER	ESCROW CD	ASSESSED VALUE	EXEMPTIONS	TAXABLE VALUE	MILLAGE CODE
R10025-114		63,912	0	63,912	003

R

0011457 01 AV 0.278 **AUTO T1 O 0810 32038-123

MONIZ ALBERT E SR & PATRICIA A
455 SW CHALET TERR
FORT WHITE FL 32038-2557

SEE INSERT FOR IMPORTANT INFO
AND TELEPHONE NUMBERS
WWW.COLUMBIATAXCOLLECTOR.COM

19-7S-17 4800/4800 10.01 Acres
COMM NW COR, RUN W 1111.14 FT
TO SW R/W ABANDONED SCL RR,
SE ALONG R/W TO ITS INTERS
WITH W R/W BOB-CAT LANE, RUN S
3438.08 FT FOR POB, CONT S

AD VALOREM TAXES

TAXING AUTHORITY		MILLAGE RATE (DOLLARS PER $1,000 OF TAXABLE VALUE)	TAXES LEVIED
C001	BOARD OF COUNTY COMMISSIONERS	8.7260	557.70
S002	COLUMBIA COUNTY SCHOOL BOARD		
	DISCRETIONARY	.7600	48.57
	LOCAL	5.1950	332.02
	CAPITAL OUTLAY	2.0000	127.83
W SR	SUWANNEE RIVER WATER MGT DIST	.4914	31.41
HLSH	SHANDS AT LAKE SHORE	1.7500	111.85
IIDA	INDUSTRIAL DEVELOPEMENT AUTH	.1380	8.82

Pd. Ck. #9456 - W. Acct 11-24-05

TOTAL MILLAGE	19.0604	AD VALOREM TAXES	$1,218.20

NON-AD VALOREM ASSESSMENTS

LEVYING AUTHORITY	RATE	AMOUNT
FFIR FIRE ASSESSMENTS		67.99

PAY ONLY ONE AMOUNT IN YELLOW SHADED AREA	NON-AD VALOREM ASSESSMENTS	$67.99
COMBINED TAXES AND ASSESSMENTS	$1,286.19	PAY ONLY ONE AMOUNT

See reverse side for important information.

IF PAID BY	Nov 30	Dec 31	Jan 31	Feb 28	Mar 31	IF PAID BY
PLEASE PAY	1,234.74	1,247.60	1,260.47	1,273.33	1,286.19	

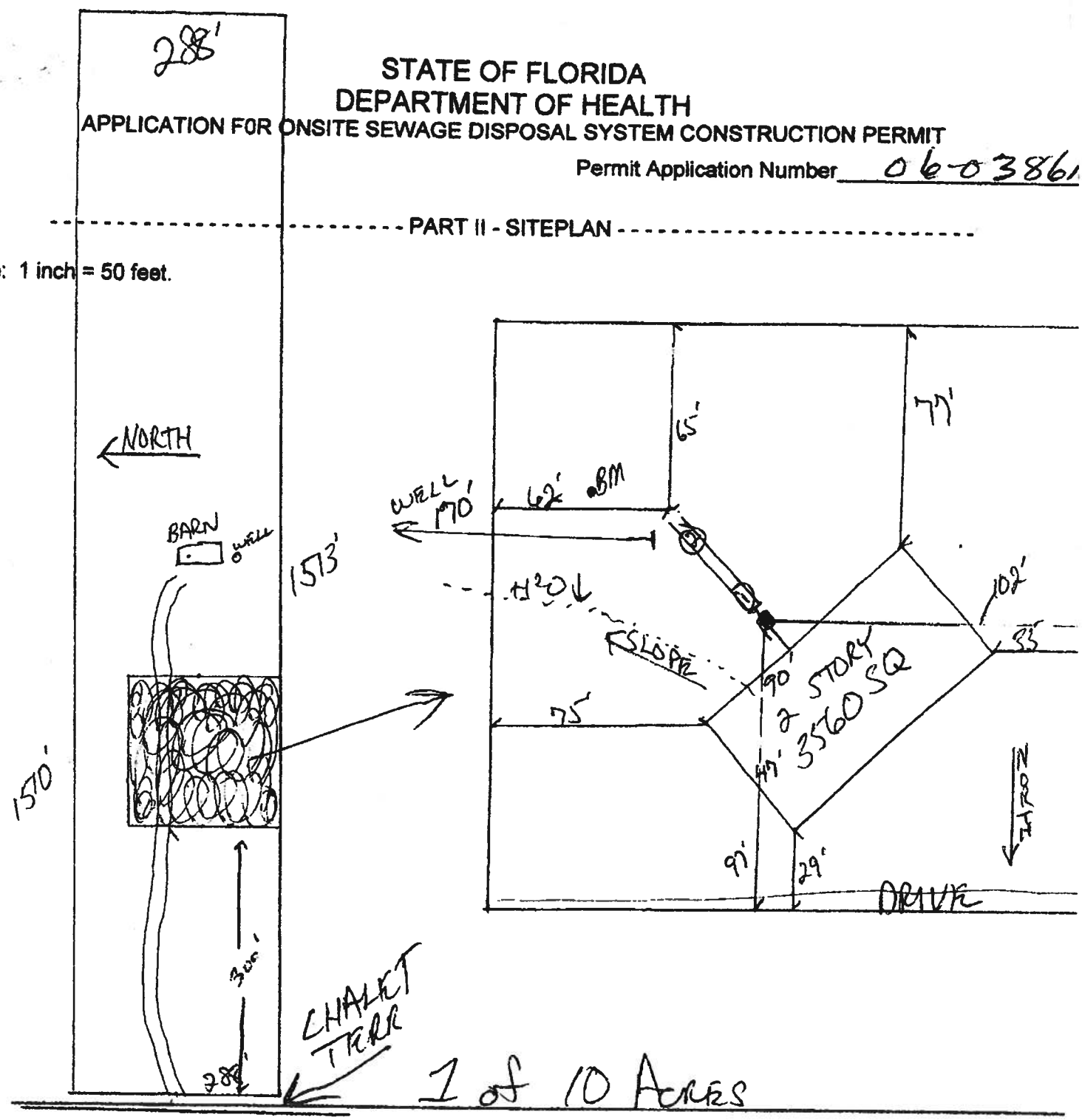
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-03861

---------- PART II - SITEPLAN ----------

Scale: 1 inch = 50 feet.



Notes: 1 of 10 Acres

Site Plan submitted by: Rocky Tyo

Plan Approved ✓ Not Approved ____

MASTER CONTRACTOR

By Marcus Zn Columbia County Health Department

Date 4/24/06

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $25,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

(X) Single Family Dwelling
( ) Two-Family Residence
( ) Farm Outbuilding
( ) Other_____
( ) New Construction
( ) Addition, Alteration, Modification or other Improvement

NEW CONSTRUCTION OR IMPROVEMENT

I Albert E. Moniz, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number_____

Albert E Moniz 8/18/06
Signature Date

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date_____ Building Official/Representative_____

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number R10025-114 **PERMIT NUMBER** 0608-70

1. **Description of property: (legal description of the property and street address or 911 address)**

COMM NW COR, RUN W 1111.14 FT TO SW R/W ABANDONED SCL RR, SE ALONG R/W TO ITS INTERS WITH W R/W BOB-CAT LANE, RUN S 3438.08 FT FOR POB, CONT S 288.21 FT, W 1513.08 FT, N 288.26 FT, E 1510.40 FT TO POB

455 S.W Chalet Terr.
Fort White, FL 32038

2. **General description of improvement:** HOME DWELLING

3. **Owner Name & Address** Albert E. Moniz, 455 S.W. Chalet Terrace, Ft. White, FL 32038 **Interest in Property** Owner

4. **Name & Address of Fee Simple Owner (if other than owner):**

5. **Contractor Name** Albert E Moniz **Phone Number**
Address 455 SW Chalet Terrace Ft White FL 32038

6. **Surety Holders Name** None
Address
Amount of Bond None

Inst:2006019952 Date:08/22/2006 Time:14:04
DC,P.DeWitt Cason,Columbia County B:1093 P:1511

7. **Lender Name** None
Address

8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
Name **Phone Number**
Address

9. **In addition to himself/herself the owner designates** of to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –
(a) 7. Phone Number of the designee

10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, *(Unless a different date is specified)***

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Albert E Moniz
Signature of Owner

Sworn to (or affirmed) and subscribed before day of August 3 **20** 06

NOTARY STAMP/SEAL

NOEL BURMEISTER
MY COMMISSION # DD536652
EXPIRES: June 22, 2010
Fl. Notary Discount Assoc. Co.
1-800-NOTARY

Noel Burmeister
Signature of Notary

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	MONIZ	Builder:	ALBERT MONIZ
Address:		Permitting Office:	24936 Columbia
City, State:	,	Permit Number:	
Owner:		Jurisdiction Number:	221000
Climate Zone:	North		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	2	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	3559 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description Area (Dble Default) 412.0 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 412.0 ft²	
8.	Floor types		
a.	Raised Wood, Adjacent	R=11.0, 1656.0ft²	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=19.0, 2152.0 ft²	
b.	Frame, Wood, Adjacent	R=13.0, 195.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1656.0 ft²	
b.	Under Attic	R=19.0, 660.0 ft²	
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Attic	Sup. R=6.0, 224.0 ft	
b.	Sup: Unc. Ret: Unc. AH: Outdoors	Sup. R=6.0, 188.0 ft	
12.	Cooling systems		
a.	Central Unit	Cap: 28.8 kBtu/hr	
		SEER: 15.00	
b.	Central Unit	Cap: 28.6 kBtu/hr	
		SEER: 13.00	
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 28.6 kBtu/hr	
		HSPF: 8.50	
b.	Electric Heat Pump	Cap: 28.8 kBtu/hr	
		HSPF: 8.50	
c.	N/A		
14.	Hot water systems		
a.	LP Gas	Cap: 50.0 gallons	
		EF: 0.66	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.12

Total as-built points: 35451

Total base points: 39256

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: SUNCOAST INSULATORS 828 NW 253rd Terrace Newberry, FL 32669 (352) 472-8895 Fax (352) 472-2835

DATE: 8/9/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	3559.0	20.04	12838.0	Double, Clear	W	2.0	6.0	220.0	38.52	0.85	7198.9	
				Double, Clear	S	2.0	6.0	8.0	35.87	0.78	222.7	
				Double, Clear	N	2.0	6.0	70.0	19.20	0.90	1209.7	
				Double, Clear	E	2.0	6.0	114.0	42.06	0.85	4066.5	
				As-Built Total:				**412.0**			**12697.8**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	195.0	0.70	136.5	Frame, Wood, Exterior			19.0	2152.0	0.90		1936.8	
Exterior	2152.0	1.70	3658.4	Frame, Wood, Adjacent			13.0	195.0	0.60		117.0	
Base Total:	**2347.0**		**3794.9**	**As-Built Total:**				**2347.0**			**2053.8**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	2.40	48.0	Exterior Insulated				44.0	4.10		180.4	
Exterior	44.0	6.10	268.4	Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**64.0**		**316.4**	**As-Built Total:**				**64.0**			**212.4**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1656.0	1.73	2864.9	Under Attic			30.0	1656.0	1.73 X 1.00		2864.9	
				Under Attic			19.0	660.0	2.34 X 1.00		1544.4	
Base Total:	**1656.0**		**2864.9**	**As-Built Total:**				**2316.0**			**4409.3**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	0.0(p)	0.0	0.0	Raised Wood, Adjacent			11.0	1656.0	0.70		1159.2	
Raised	1656.0	-3.99	-6607.4									
Base Total:			**-6607.4**	**As-Built Total:**				**1656.0**			**1159.2**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	3559.0	10.21	36337.4					3559.0	10.21		36337.4	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 49544.2			**Summer As-Built Points:**					**56869.8**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 28800 btuh ,SEER/EFF(15.0) Ducts:Unc(S),Unc(R),Att(AH),R6.0(INS)					
			56870	0.50	(1.09 x 1.147 x 1.11)	0.228	1.000	8676.6
			(sys 2: Central Unit 28600 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Out(AH),R6.0(INS)					
			56870	0.50	(1.09 x 1.147 x 1.02)	0.263	1.000	9941.9
49544.2	**0.4266**	**21135.5**	**56869.8**	**1.00**	**1.336**	**0.244**	**1.000**	**18523.5**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	3559.0	12.74	8161.5	Double, Clear	W	2.0	6.0	220.0	20.73	1.04	4755.0	
				Double, Clear	S	2.0	6.0	8.0	13.30	1.26	133.9	
				Double, Clear	N	2.0	6.0	70.0	24.58	1.00	1728.5	
				Double, Clear	E	2.0	6.0	114.0	18.79	1.06	2272.1	
				As-Built Total:				**412.0**			**8889.6**	
WALL TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM	=	Points	
Adjacent	195.0	3.60	702.0	Frame, Wood, Exterior			19.0	2152.0	2.20		4734.4	
Exterior	2152.0	3.70	7962.4	Frame, Wood, Adjacent			13.0	195.0	3.30		643.5	
Base Total:	**2347.0**		**8664.4**	**As-Built Total:**				**2347.0**			**5377.9**	
DOOR TYPES	Area X	BWPM	= Points	Type				Area X	WPM	=	Points	
Adjacent	20.0	11.50	230.0	Exterior Insulated				44.0	8.40		369.6	
Exterior	44.0	12.30	541.2	Adjacent Insulated				20.0	8.00		160.0	
Base Total:	**64.0**		**771.2**	**As-Built Total:**				**64.0**			**529.6**	
CEILING TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1656.0	2.05	3394.8	Under Attic			30.0	1656.0	2.05 X 1.00		3394.8	
				Under Attic			19.0	660.0	2.70 X 1.00		1782.0	
Base Total:	**1656.0**		**3394.8**	**As-Built Total:**				**2316.0**			**5176.8**	
FLOOR TYPES	Area X	BWPM	= Points	Type			R-Value	Area X	WPM	=	Points	
Slab	0.0(p)	0.0	0.0	Raised Wood, Adjacent			11.0	1656.0	3.60		5961.6	
Raised	1656.0	0.96	1589.8									
Base Total:			**1589.8**	**As-Built Total:**				**1656.0**			**5961.6**	
INFILTRATION	Area X	BWPM	= Points					Area X	WPM	=	Points	
	3559.0	-0.59	-2099.8					3559.0	-0.59		-2099.8	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**20481.8**	**Winter As-Built Points:**					**23835.6**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 28600 btuh ,EFF(8.5) Ducts:Unc(S),Unc(R),Att(AH),R6.0					
			23835.6	0.498	(1.069 x 1.169 x 1.10)	0.401	1.000	6467.9
			(sys 2: Electric Heat Pump 28800 btuh ,EFF(8.5) Ducts:Unc(S),Unc(R),Out(AH),R6.0					
			23835.6	0.502	(1.069 x 1.169 x 1.07)	0.401	1.000	6513.1
20481.8	**0.6274**	**12850.3**	**23835.6**	**1.00**	**1.358**	**0.401**	**1.000**	**12981.0**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
2		2635.00	5270.0	50.0	0.66	2	1.00	1973.45	1.00	3946.9	
				As-Built Total:						**3946.9**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
21136	**12850**	**5270**	**39256**	**18523**	**12981**	**3947**	**35451**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.4

The higher the score, the more efficient the home.

1.	New construction or existing	New		12. Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 28.8 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 15.00
4.	Number of Bedrooms	2		b. Central Unit	Cap: 28.6 kBtu/hr
5.	Is this a worst case?	Yes			SEER: 13.00
6.	Conditioned floor area (ft²)	3559 ft²		c. N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)				
a.	U-factor:	Description Area		13. Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 412.0 ft²		a. Electric Heat Pump	Cap: 28.6 kBtu/hr
b.	SHGC:				HSPF: 8.50
	(or Clear or Tint DEFAULT) 7b.	(Clear) 412.0 ft²		b. Electric Heat Pump	Cap: 28.8 kBtu/hr
8.	Floor types				HSPF: 8.50
a.	Raised Wood, Adjacent	R=11.0, 1656.0ft²		c. N/A	
b.	N/A				
c.	N/A			14. Hot water systems	
9.	Wall types			a. LP Gas	Cap: 50.0 gallons
a.	Frame, Wood, Exterior	R=19.0, 2152.0 ft²			EF: 0.66
b.	Frame, Wood, Adjacent	R=13.0, 195.0 ft²		b. N/A	
c.	N/A				
d.	N/A			c. Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1656.0 ft²		15. HVAC credits	
b.	Under Attic	R=19.0, 660.0 ft²		(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Attic	Sup. R=6.0, 224.0 ft		MZ-C-Multizone cooling,	
b.	Sup: Unc. Ret: Unc. AH: Outdoors	Sup. R=6.0, 188.0 ft		MZ-H-Multizone heating)	

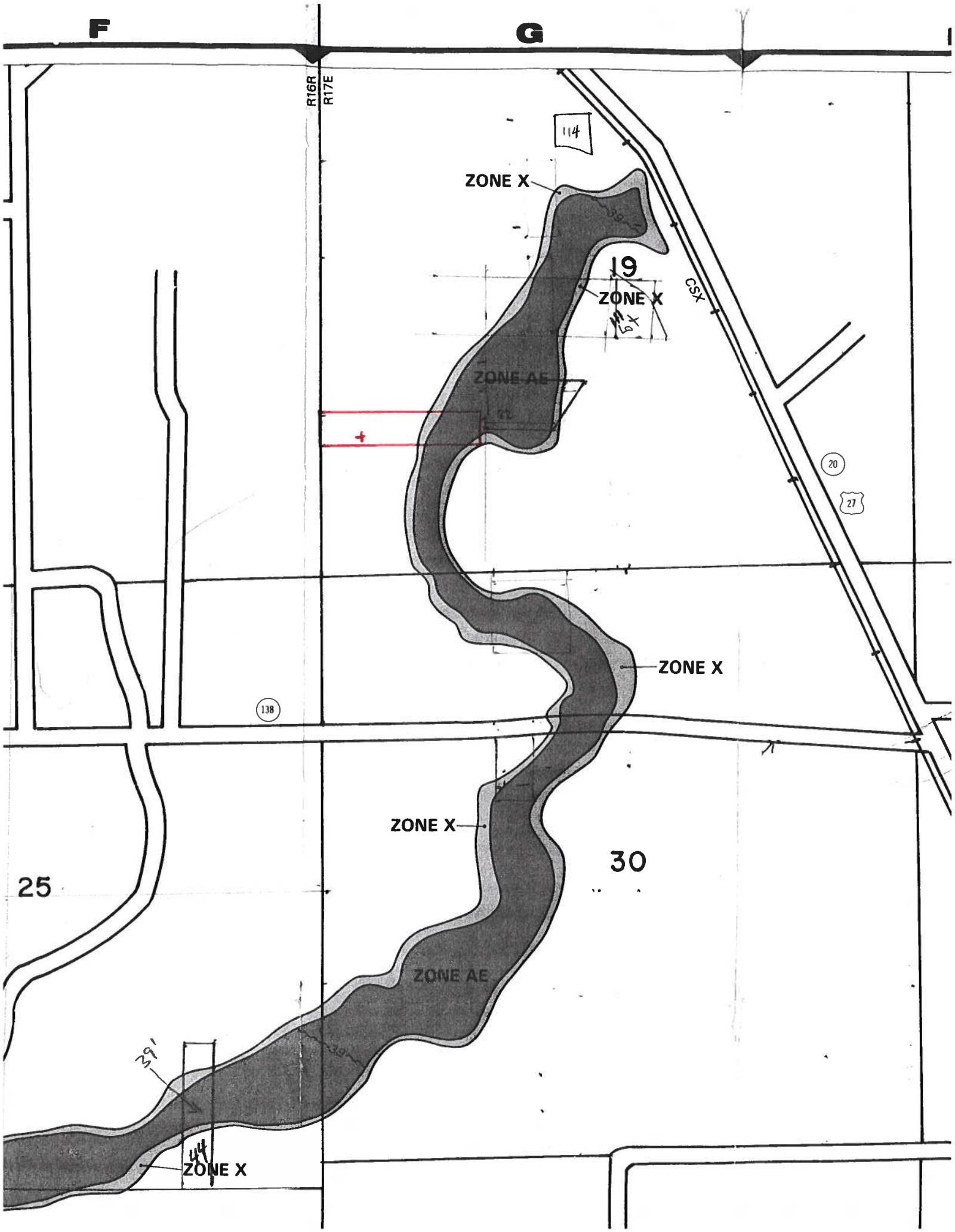
I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.


F
G
R16R
R17E
114
ZONE X
19
ZONE X
CSX
ZONE AE
20
27
138
ZONE X
ZONE X
30
25
ZONE AE
39'
44
ZONE X

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0608-70

Owner/Builder Albert E. Moniz Property ID# 19-7s-17-10025-114

On the date of August 24, 2006 application 0608-70 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0608-70 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

To help ensure compliance with the Florida Residential Code 2004 the comments below need to be addressed on the plans.

1. Please show the total height of the structure from the established grade to the highest roof ridge.
2. The garage area shall meet the following FRC requirements.

A. Sections R309.1 openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.

B. This section (R309.1) will also apply to the door from the garage to the HVAC equipment room.

C. In the garage area show compliance with the FRC-2004 sections R309.2 Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

3. Please provide a drawing of all the staircases detailing the requirements of the FRC- R311.5.3 Stair treads and risers. (See attached section FRC-R311.5.3)

4. Please verify that in each second story bedroom one window will comply with section FBC-2004 Section R310.1.1 Minimum opening area: All emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet (0.530 m2). The minimum net clear opening

height shall be 24 inches (610 mm): R310.1.3 Minimum opening width. The minimum net clear opening width shall be 20 inches (508 mm).

5. On the foundation plan show the access opening to the crawl space R408.3 Access shall be provided to all under-floor spaces. Access openings through the floor shall be a minimum of 18 inches by 24 inches (457 mm by 610 mm). Openings through a perimeter wall shall be 16 inches by 24 inches (407 mm by 610 mm). When any portion of the through wall access is below grade, an areaway of not less than 16 inches by 24 inches (407 mm by 610 mm) shall be provided. The bottom of the areaway shall be below the threshold of the access opening. Through wall access openings shall not be located under a door to the residence. See M1305.1.4 for access requirements where mechanical equipment is located under floors.

6. On the foundation plan show the requirements of sections R408.2 Openings for under-floor ventilation: The minimum net area of ventilation openings shall not be less than 1 square foot (0.0929 m2) for each 150 square feet (100 m2) of under-floor space area. One such ventilating opening shall be within 3 feet (914 mm) of each corner of the building. Ventilation openings shall be covered for their height and width with any of the following materials provided that the least dimension of the covering shall not exceed ¼ inch (6.4 mm):

1. Perforated sheet metal plates not less than 0.070 inch (1.8 mm) thick.
2. Expanded sheet metal plates not less than 0.047 inch (1.2 mm) thick.

3. Cast iron grills or grating.

4. Extruded load-bearing brick vents.

5. Hardware cloth of 0.035 inch (0.89 mm) wire or heavier.

6. Corrosion-resistant wire mesh, with the least dimension being 1/8 inch (3.2 mm).

7. On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

8. Show on the electrical plans as required by the National Electrical Code article 210.12 that all branch circuits supplying outlets installed within a bedroom shall be protected by Arc-Fault interrupter devices.

9. Please show compliance the FRC -2004 sections R313.1 Smoke alarms.

Smoke alarms shall be installed in the following locations:

1. In each sleeping room.

2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.

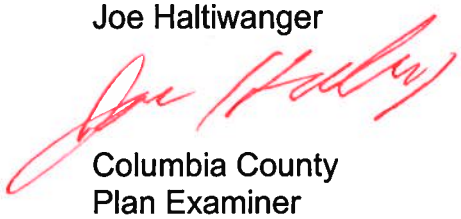
3. On each additional story of the dwelling, including basements but not including crawl spaces and uninhabitable attics. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a

smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.

When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit. The alarm shall be clearly audible in all bedrooms over background noise levels with all intervening doors closed.

All smoke alarms shall be listed and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

Joe Haltiwanger



Columbia County
Plan Examiner

Sept. 1, 2007

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit #000024936, located at 455 S.W. Chalet Terr., Ft. White, Fl.

Thank you for your consideration to this request.

Sincerely Yours,

Albert E. Moniz

Albert E. Moniz

Dec. 1, 2007

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit #000024936, located at 455 S.W. Chalet Terr., Ft. White, Fl.

Thank you for your consideration to this request.

Sincerely Yours,

Albert E. Moniz

Albert E. Moniz

March 1, 2008

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Albert E. Moniz

Albert E. Moniz

May 29, 2008

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Albert E Moniz

Albert E. Moniz

Sept. 1, 2008

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Albert E Moniz

Albert E. Moniz

Dec. 1st, 2008

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Albert E. Moniz

Albert E. Moniz

February 27, 2009

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Albert E. Moniz

Albert E. Moniz

May 28, 2009

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Albert E. Moniz

Albert E. Moniz

Aug. 28, 2009

Columbia County Bldg. Dept.
135 N.E. Hernando Ave., Su.B-21
Lake City, Fla. 32055

To whom it may concern:

Due to construction delays, I am requesting an extension of the existing building permit, #000024936, located at 455 S.W. Chalet Terr., Ft. White, FL.

Thank you for your consideration to this request.

Sincerely yours,

Albert E. Moniz

Albert E. Moniz

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 19-7S-17-10025-114 **Building permit No.** 000024936

Use Classification SFD/UTILITY **Fire:** 134.42

Permit Holder ALBERT E. MONIZ, OWNER BLDR. **Waste:** 184.25

Owner of Building ALBERT E. MONIZ **Total:** 318.67

Location: 455 SW CHALET TERR., FT. WHITE, FL

Date: 11/16/2009

Wayne A. Rust

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Pex Pipe Pg. #2

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.4 Water service pipe.

P2904.4 Water service pipe.
Water service pipe shall conform to NSF 61 and shall conform to one of the standards listed in Table P2904.4.1. Water service pipe or tubing, installed underground and outside of the structure, shall have a minimum working pressure rating of 160 psi at 73°F (1100 kPa at 23°C).

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.4 Water service pipe. / P2904.4.1 Water service installation.

P2904.4.1 Water service installation.
Trenching, pipe installation and backfilling shall be in accordance with Section P2604. Water-service pipe is permitted to be located in the same trench with a building sewer provided such sewer is constructed of materials listed for underground use within a building in Section P3002.1. If the building sewer is not constructed of materials listed in Section P3002.1, the water-service pipe shall be separated from the building sewer by a minimum of 5 feet (1524 mm), measured horizontally, of undisturbed or compacted earth or placed on a solid ledge at least 12 inches (305 mm) above and to one side of the highest point in the sewer line.

Exception: The required separation distance shall not apply where a water service pipe crosses a sewer pipe, provided the water service pipe is sleeved to at least 5 feet (1524 mm), horizontally from the sewer pipe centerline, on both sides of such crossing with pipe materials listed in Tables P2904.4.1, P3002.1 or P3002.2.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.4 Water service pipe. / P2904.4.1 Water service installation. / TABLE P2904.4.1 WATER SERVICE PIPE

TABLE P2904.4.1
WATER SERVICE PIPE

MATERIAL	STANDARD
Acrylonitrile butadiene styrene (ABS) plastic pipe	ASTM D 1527; ASTM D 2282
Asbestos-cement pipe	ASTM C 296
Brass pipe	ASTM B 43
Chlorinated polyvinyl chloride (CPVC) plastic pipe	ASTM D 2846; ASTM F 441; ASTM F 442; CSA B137.6
Copper or copper-alloy pipe	ASTM B 42; ASTM B 302
Copper or copper-alloy tubing (Type K, WK, L, WL, M or WM)	ASTM B 75; ASTM B 88; ASTM B 251; ASTM B 447

MATERIAL	STANDARD
Cross-linked polyethylene/aluminum/cross-linked polyethylene (PEX-AL-PEX) pipe	ASTM F 1281; CSA B137.10
Cross-linked polyethylene (PEX) plastic tubing	ASTM F 876; ASTM F 877; CSA B137.5
Ductile iron water pipe	AWWA C151; AWWA C115
Galvanized steel pipe	ASTM A 53
Polybutylene (PB) plastic pipe and tubing	ASTM D 2662; ASTM D 2666; ASTM D 3309; CSA B13
Polyethylene/aluminum/polyethylene (PE-AL-PE) pipe	ASTM F 1282; CSA B137.9M
Polyethylene (PE) plastic pipe	ASTM D 2239; CSA-B137.1
Polyethylene (PE) plastic tubing	ASTM D 2737; CSA B137.1
Polyvinyl chloride (PVC) plastic pipe	ASTM D 1785; ASTM D 2241; ASTM D 2672; CSA B13
Stainless steel (Type 304/304L) pipe	ASTM A312; ASTM A778
Stainless steel (Type 316/316L) pipe	ASTM A312; ASTM A778

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.5 Water-distribution pipe.

P2904.5 Water-distribution pipe.
Water-distribution piping within dwelling units shall conform to NSF 61 and shall conform to one of the standards listed in Table P2904.5. All hot-water-distribution pipe and tubing shall have a minimum pressure rating of 100 psi at 180°F (689 kPa at 82°C).

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.5 Water-distribution pipe. / TABLE P2904.5 WATER DISTRIBUTION PIPE

TABLE P2904.5
WATER DISTRIBUTION PIPE

MATERIAL	STANDARD
Brass pipe	ASTM B 43
Chlorinated polyvinyl chloride (CPVC) plastic pipe and tubing	ASTM D 2846; ASTM F 441; ASTM F 442; CSA
Copper or copper-alloy pipe	ASTM B 42; ASTM B 302
Copper or copper-alloy tubing (Type K, WK, L, WL, M or WM)	ASTM B 75; ASTM B 88; ASTM B 251; ASTM B 447
Cross-linked polyethylene (PEX) plastic tubing	ASTM F 877; CSA B137.5
Cross-linked polyethylene/aluminum/cross-linked polyethylene (PEX-AL-PEX) pipe	ASTM F 1281; CSAB137.10
Galvanized steel pipe	ASTM A 53
Polybutylene (PB) plastic pipe and tubing	ASTM D 3309; CSA B137.8
Polyethylene/aluminum/polyethylene (PE-AL-PE) composite pipe	ASTM F 1282
Stainless steel (Type 304/304L) pipe	ASTM A312; ASTM A778

MATERIAL	STANDARD
Stainless steel (Type 316/316L) pipe	ASTM A312; ASTM A778

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.5 Water-distribution pipe. / P2904.5.1 Under concrete slabs.

P2904.5.1 Under concrete slabs.
Inaccessible water distribution piping under slabs shall be copper water tube minimum Type M, brass, ductile iron pressure pipe, cross-linked polyethylene/aluminum/cross-linked polyethylene (PEX-AL-PEX) pressure pipe, chlorinated polyvinyl chloride (CPVC) or polybutylene (PB) or cross-linked polyethylene (PEX) plastic pipe or tubing—all to be installed with approved fittings or bends. The minimum pressure rating for plastic pipe or tubing installed under slabs shall be 100 psi at 180°F (689 kPa at 82°C).

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.6 Fittings.

P2904.6 Fittings.
Pipe fittings shall be approved for installation with the piping material installed, and shall conform to the respective pipe standards listed in Table P2904.6. Pipe fittings utilized in the water supply system shall also conform to NSF 61.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.6 Fittings. / TABLE P2904.6 PIPE FITTINGS

TABLE P2904.6
PIPE FITTINGS

Acrylonitrile butadiene styrene (ABS) plastic	ASTM D 2468
Brass	ASTM F1974
Cast-iron	ASME B16.4; ASME B16.12
Chlorinated polyvinyl chloride (CPVC) plastic	ASTM F 437; ASTM F 438; ASTM F 439
Copper or copper alloy	ASME B16.15; ASME B16.18; ASME B16.22; ASME B16.23; ASME B16.26; ASME B16.29
Fittings for cross-linked polyethylene (PEX) plastic tubing	ASTM F 1807; ASTM F 1960; ASTM F 2080
Gray iron and ductile iron	AWWA C110; AWWA C153
Malleable iron	ASME B16.3
Polyethylene (PE) plastic	ASTM D 2609
Polyvinyl chloride (PVC) plastic	ASTM D 2464; ASTM D 2466; ASTM D 2467; CSA B137.2

Stainless steel (Type 304/304L) pipe	ASTM A312; ASTM A778
Stainless steel (Type 316/316L) pipe	ASTM A312; ASTM A778
Steel	ASME B16.9; ASME B16.11; ASME B16.28

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints.

P2904.9 Plastic pipe joints.
Joints in plastic piping shall be made with approved fittings by solvent cementing, heat fusion, corrosion-resistant metal clamps with insert fittings or compression connections. Flared joints for polyethylene pipe are permitted in accordance with Section P2904.3.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing.

P2904.9.1 Solvent cementing.
Solvent-cemented joints shall comply with Sections P2904.9.1.1 through P2904.9.1.3.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.1 ABS plastic pipe.

P2904.9.1.1 ABS plastic pipe.
Solvent cement for ABS plastic pipe conforming to ASTM D 2235 shall be applied to all joint surfaces.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.2 CPVC plastic pipe.

P2904.9.1.2 CPVC plastic pipe.
Solvent cement for CPVC plastic pipe conforming to ASTM F 493 shall be applied to all joint surfaces. The joint shall be made while the cement is wet, and in accordance with ASTM D 2846 or ASTM F 493. Solvent-cement joints shall be permitted above or below ground.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS /

P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.3 PVC plastic pipe.

P2904.9.1.3 PVC plastic pipe.
A primer complying with ASTM F 656 shall be applied to all PVC solvent cemented joints. Solvent cement for PVC plastic pipe conforming to ASTM D 2564 shall be applied to all joint surfaces.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.4 Cross-linked polyethylene plastic (PEX).

P2904.9.1.4 Cross-linked polyethylene plastic (PEX).
Joints between cross-linked polyethylene plastic tubing or fittings shall comply with Section P2904.9.1.4.1 or Section P2904.9.1.4.2.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.4 Cross-linked polyethylene plastic (PEX). / P2904.9.1.4.1 Flared joints.

P2904.9.1.4.1 Flared joints.
Flared pipe ends shall be made by a tool designed for that operation.

Residential 2004 / Part VII — Plumbing / CHAPTER 29 WATER SUPPLY AND DISTRIBUTION / SECTION P2904 MATERIALS, JOINTS AND CONNECTIONS / P2904.9 Plastic pipe joints. / P2904.9.1 Solvent cementing. / P2904.9.1.4 Cross-linked polyethylene plastic (PEX). / P2904.9.1.4.2 Mechanical joints.

P2904.9.1.4.2 Mechanical joints.
Mechanical joints shall be installed in accordance with the manufacturer's instructions. Fittings for cross-linked polyethylene (PEX) plastic tubing as described in ASTM F 1807, ASTM F 1960 and ASTM F 2080 shall be installed in accordance with the manufacturer's instructions.

ARROW EXTERMINATORS
TERMITE AND PEST CONTROL

#24936

5602 N.W. 13th STREET
GAINESVILLE, FLORIDA 32653-2198

P.O. BOX 5875
GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642
FAX (352) 373-9037

CERTIFICATE OF PROTECTIVE TREATMENT

Builder: Albert Moniz

Date: 11-20-06 Time: _____ AM _____ PM

Site Location: 455 SW Child St

Area Treated: Porches

Product Used: BifenIT Chemical Used: Bifenthrin

% Concentration: .06% # Gallons Used: 220

Applicator: Jerry

ARROW EXTERMINATORS
TERMITE AND PEST CONTROL

#24936

5602 N.W. 13th STREET
GAINESVILLE, FLORIDA 32653-2198

P.O. BOX 5875
GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642
FAX (352) 373-9037

CERTIFICATE OF PROTECTIVE TREATMENT

Builder: Albert Mann

Date: 10-2-06 Time: _____ AM _____ PM

Site Location: 455 SW Chalet St

Area Treated: Footing

Product Used: Biten IT Chemical Used: Bifenthrin

% Concentration: .06% # Gallons Used: 200

Applicator: Jerry

ARROW EXTERMINATORS
TERMITE AND PEST CONTROL

#24936

5602 N.W. 13th STREET
GAINESVILLE, FLORIDA 32653-2198

P.O. BOX 5875
GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642
FAX (352) 373-9037

CERTIFICATE OF PROTECTIVE TREATMENT

Builder: Albert Moniz

Date: 11-2-06 Time: _____ AM _____ PM

Site Location: 455 SW Chalet

Area Treated: Garage, Block Cells, Crawl Space

Product Used: Bifen IT Chemical Used: Bifenthrin

Concentration: .06% # Gallons Used: 350

Jerry

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber	Plywood + OSB	5/8" Plywood on Roof - OSB for Walls	
4. Railing		PVC	
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms		Icynene + spider	
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1. Porch Col.	Fiberglass		FL 3451.1
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Albert E Money

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # **(FOR STAFF USE ONLY)**

Albert & Patricia Moniz
HVAC Load Calculations

for

Albert & Patricia Moniz
455 Chalet Terr
Ft White Fl 32038

Elite Software

RHVAC RESIDENTIAL HVAC LOADS

Prepared By:
Chuck Fischer
North Central Florida Air Conditioning
P.O Box 700
High Springs Fl 32655-0700
386-454-4767
Thursday, May 25, 2006

Project Report

General Project Information

Project Filename:	C:\Documents and Settings\Heat\My Documents\Projects\AutoLoad MJ8.rhv
Project Title:	Albert & Patricia Moniz
Designed By:	Chuck Fischer
Project Date:	May 26,2006
Client Name:	Albert & Patricia Moniz
Client Address:	455 Chalet Terr
Client City:	Ft White Fl 32038
Client Phone:	386-454-0855
Client Comment:	
Company Name:	North Central Florida Air Conditioning
Company Representative:	Chuck Fischer
Company Address:	P.O Box 700
Company City:	High Springs Fl 32655-0700
Company Phone:	386-454-4767
Company Fax:	386-454-4854
Company Comment:	Bedroom 2,3 & 4 R/A are 10x10x8 Master bedroom R/A is 12x12x9 Main R/A is 20x20x18

Design Data

Reference City:		Gainesville, Florida
Daily Temperature Range:		Medium
Latitude:	29	Degrees
Elevation:	152	ft.
Altitude Factor:	0.995	
Elevation Sensible Adj. Factor:	1.000	
Elevation Total Adj. Factor:	1.000	
Elevation Heating Adj. Factor:	1.000	
Elevation Heating Adj. Factor:	1.000	

	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum	Indoor Dry Bulb	Grains Difference
Winter:	31	0	0	68	0
Summer:	93	77	50	75	50

Check Figures

Total Building Supply CFM:	1,814	CFM Per Square ft.:	1.194
Square ft. of Room Area:	3,033	Square ft. Per Ton:	698
Volume (ft³) of Cond. Space:	27,585	Air Turnover Rate (per hour):	7.9

Building Loads

Total Heating Required With Outside Air:	43,205 Btuh	43.205	MBH
Total Sensible Gain:	39,683 Btuh	84	%
Total Latent Gain:	7,374 Btuh	16	%
Total Cooling Required With Outside Air:	47,057 Btuh	3.92	Tons (Based On Sensible + Latent)
		4.35	Tons (Based On 75% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

Miscellaneous Report

System 1 Main Floor Input Data	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum	Indoor Dry Bulb	Grains Difference
Winter:	31	0	50	68	30.84
Summer:	93	77	50	75	50.06

System 2 Bonas Room Input Data	Outdoor Dry Bulb	Outdoor Wet Bulb	Indoor Rel.Hum	Indoor Dry Bulb	Grains Difference
Winter:	31	0	30	68	10.25
Summer:	93	77	50	75	50.06

Duct Sizing Inputs

	Main Trunk		Runouts	
Calculate:	Yes		Yes	
Use Schedule:	No		No	
Roughness Factor:	0.00300		0.01000	
Pressure Drop:	0.1000	in.wg./100 ft.	0.1000	in.wg./100 ft.
Minimum Velocity:	650	ft./min	450	ft./min
Maximum Velocity:	900	ft./min	750	ft./min
Minimum Height:	0	in.	0	in.
Maximum Height:	0	in.	0	in.

Outside Air Data

	Winter		Summer	
Infiltration:	0.900	AC/hr	0.400	AC/hr
Volume of Conditioned Space:	X 27585	Cu.ft.	X 27585	Cu.ft.
	24,827	Cu.ft./hr	11,034	Cu.ft./hr
	X 0.0167		X 0.0167	
Total Building Infiltration:	414	CFM	184	CFM
Total Building Ventilation:	0	CFM	0	CFM

---System 1---

Infiltration & Ventilation Sensible Gain Multiplier:	19.69	= (1.10 X 0.995 X 18.00 Summer Temp. Difference)
Infiltration & Ventilation Latent Gain Multiplier:	33.85	= (0.68 X 0.995 X 50.06 Grains Difference)
Infiltration & Ventilation Sensible Loss Multiplier:	40.48	= (1.10 X 0.995 X 37.00 Winter Temp. Difference)

---System 2---

Infiltration & Ventilation Sensible Gain Multiplier:	19.69	= (1.10 X 0.995 X 18.00 Summer Temp. Difference)
Infiltration & Ventilation Latent Gain Multiplier:	33.85	= (0.68 X 0.995 X 50.06 Grains Difference)
Infiltration & Ventilation Sensible Loss Multiplier:	40.48	= (1.10 X 0.995 X 37.00 Winter Temp. Difference)

Load Preview Report

Scope	Area	Sens Gain	Lat Gain	Net Gain	Sens Loss	Win CFM	Sum CFM	Sys CFM	Duct Size
Building: 3.92 Net Tons, 4.35 Recommended Tons, 698 ft²/Ton, 43.21 MBH Heating									
Building	3,033	39,683	7,374	47,057	43,205	564	1,814	1,814	
System 1: 2.13 Net Tons, 2.38 Recommended Tons, 696 ft²/Ton, 24.34 MBH Heating									
System 1	1,656	21,985	3,594	25,579	24,341	318	1,005	1,005	14x14
Zone 1	1,656	21,985	3,594	25,579	24,341	318	1,005	1,005	
1-Bath 1	126	1,620	380	2,000	2,368	31	74	74	1-5
2-Dining Room	190	2,180	425	2,605	2,701	35	100	100	1-6
3-Kitchen	204	3,716	521	4,237	1,966	26	170	170	1-8
4-Nook	168	2,399	224	2,623	2,551	33	110	110	1-6
5-Gathering Room	272	3,840	730	4,570	5,594	73	176	176	1-8
6-Parior	213	3,235	732	3,967	4,763	62	148	148	1-7
7-Foyer	129	1,704	170	1,874	1,489	19	78	78	1-5
8-Pantry	49	101	0	101	112	1	5	5	1-2
9-Hall	112	983	90	1,073	816	11	45	45	1-4
10-Back Stairs	76	1,056	224	1,280	1,300	17	48	48	1-4
11-Front Stairs	70	636	0	636	160	2	29	29	1-3
12-Closet	47	512	98	610	521	7	23	23	1-3
System 2: 1.79 Net Tons, 1.97 Recommended Tons, 700 ft²/Ton, 18.86 MBH Heating									
System 2	1,377	17,698	3,780	21,478	18,864	246	809	809	13x13
Zone 1	1,377	17,698	3,780	21,478	18,864	246	809	809	
13-Master Bedroom	333	4,493	1,243	5,736	5,555	73	205	205	2-6
14-Master W.I.C	80	1,057	334	1,391	1,656	22	48	48	1-4
15-Master Bath	120	1,224	210	1,434	1,362	18	56	56	1-4
16-Bath 3	72	893	148	1,041	898	12	41	41	1-4
17-Bedroom 2	178	2,672	538	3,210	2,266	30	122	122	1-7
18-Bedroom 3	175	3,127	712	3,839	3,264	43	143	143	1-7
19-Hall	160	1,885	315	2,200	2,040	27	86	86	1-5
20-Laundry	35	563	0	563	79	1	26	26	1-3
21-Closet	48	191	76	267	444	6	9	9	1-2
22-Open To Below	176	1,590	204	1,794	1,300	17	73	73	1-5

Total Building Summary Loads

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	135	2,746	0	2,558	2,558
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	103.5	2,104	0	1,540	1,540
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 50% coverage	12.5	255	0	168	168
10C-m: Glazing-French door, double pane low-e glass (e = 0.40), metal frame no break, ground reflectance = 0.32	35.4	1,870	0	1,228	1,228
2B-v-o: Glazing-Double pane low-e (e = 0.60), fixed sash, vinyl frame, ground reflectance = 0.32	31.3	625	0	905	905
2A-b-o: Glazing-Double pane low-e (e = 0.60), operable window, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	10.9	339	0	294	294
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, external shade screen coefficient of 0.45 and 100% coverage	11.1	226	0	268	268
11P: Door-Polyurethane Core	53	570	0	447	447
12E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	2152.8	4,379	0	1,857	1,857
12E-4bw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, brick finish, wood studs	195.1	397	0	146	146
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	3033.3	3,590	0	4,171	4,171
19C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	3033.3	2,151	0	1,047	1,047
Subtotals for structure:		19,252	0	14,629	14,629
People:	5		1,150	1,500	2,650
Equipment:			0	1,200	1,200
Lighting:	3555			12,123	12,123
Ductwork:		7,203	0	6,612	6,612
Infiltration: Winter CFM: 414, Summer CFM: 184		16,750	6,224	3,619	9,843
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
Total Building Load Totals:		43,205	7,374	39,683	47,057

Check Figures

Total Building Supply CFM:	1,814	CFM Per Square ft.:	1.194
Square ft. of Room Area:	3,033	Square ft. Per Ton:	698
Volume (ft³) of Cond. Space:	27,585	Air Turnover Rate (per hour):	7.9

Total Building Summary Loads (cont'd)

Building Loads

Total Heating Required With Outside Air:	43,205	Btuh	43.205	MBH
Total Sensible Gain:	39,683	Btuh	84	%
Total Latent Gain:	7,374	Btuh	16	%
Total Cooling Required With Outside Air:	47,057	Btuh	3.92	Tons (Based On Sensible + Latent)
			4.35	Tons (Based On 75% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 1 Main Floor Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	21.8	444	0	391	391
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	103.5	2,104	0	1,540	1,540
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 50% coverage	12.5	255	0	168	168
10C-m: Glazing-French door, double pane low-e glass (e = 0.40), metal frame no break, ground reflectance = 0.32	35.4	1,870	0	1,228	1,228
2B-v-o: Glazing-Double pane low-e (e = 0.60), fixed sash, vinyl frame, ground reflectance = 0.32	31.3	625	0	905	905
11P: Door-Polyurethane Core	53	570	0	447	447
12E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	1095.2	2,228	0	944	944
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1656.2	1,961	0	2,278	2,278
19C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	1656.2	1,175	0	572	572
Subtotals for structure:		11,232	0	8,473	8,473
People:	1		230	300	530
Equipment:			0	1,200	1,200
Lighting:	1875			6,394	6,394
Ductwork:		4,058	0	3,663	3,663
Infiltration: Winter CFM: 224, Summer CFM: 99		9,051	3,364	1,955	5,319
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
System 1 Main Floor Load Totals:		24,341	3,594	21,985	25,579

Check Figures

Supply CFM:	1,005	CFM Per Square ft.:	0.607
Square ft. of Room Area:	1,656	Square ft. Per Ton:	696
Volume (ft³) of Cond. Space:	14,906	Air Turnover Rate (per hour):	4.0

System Loads

Total Heating Required With Outside Air:	24,341 Btuh	24.341 MBH	
Total Sensible Gain:	21,985 Btuh	86 %	
Total Latent Gain:	3,594 Btuh	14 %	
Total Cooling Required With Outside Air:	25,579 Btuh	2.13	Tons (Based On Sensible + Latent)
		2.38	Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 2 Bonas Room Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	113.1	2,302	0	2,167	2,167
2A-b-o: Glazing-Double pane low-e (e = 0.60), operable window, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	10.9	339	0	294	294
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, external shade screen coefficient of 0.45 and 100% coverage	11.1	226	0	268	268
12E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	1057.7	2,151	0	913	913
12E-4bw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, brick finish, wood studs	195.1	397	0	146	146
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1377.1	1,629	0	1,893	1,893
19C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	1377.1	976	0	475	475
Subtotals for structure:		8,020	0	6,156	6,156
People:	4		920	1,200	2,120
Equipment:			0	0	0
Lighting:	1680			5,729	5,729
Ductwork:		3,145	0	2,949	2,949
Infiltration: Winter CFM: 190, Summer CFM: 85		7,699	2,860	1,664	4,524
Ventilation: Winter CFM: 0, Summer CFM: 0		0	0	0	0
System 2 Bonas Room Load Totals:		18,864	3,780	17,698	21,478

Check Figures

Supply CFM:	809	CFM Per Square ft.:	0.587
Square ft. of Room Area:	1,377	Square ft. Per Ton:	700
Volume (ft³) of Cond. Space:	12,679	Air Turnover Rate (per hour):	3.8

System Loads

Total Heating Required With Outside Air:	18,864 Btuh	18.864 MBH	
Total Sensible Gain:	17,698 Btuh	82 %	
Total Latent Gain:	3,780 Btuh	18 %	
Total Cooling Required With Outside Air:	21,478 Btuh	1.79 Tons (Based On Sensible + Latent)	
		1.97 Tons (Based On 75% Sensible Capacity)	

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 1, Zone 1 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	21.8	444	0	391	391
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	103.5	2,104	0	1,540	1,540
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.32, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 50% coverage	12.5	255	0	168	168
10C-m: Glazing-French door, double pane low-e glass (e = 0.40), metal frame no break, ground reflectance = 0.32	35.4	1,870	0	1,228	1,228
2B-v-o: Glazing-Double pane low-e (e = 0.60), fixed sash, vinyl frame, ground reflectance = 0.32	31.3	625	0	905	905
11P: Door-Polyurethane Core	53	570	0	447	447
12E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	1095.2	2,228	0	944	944
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1656.2	1,961	0	2,278	2,278
19C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	1656.2	1,175	0	572	572
Subtotals for structure:		11,232	0	8,473	8,473
People:	1		230	300	530
Equipment:			0	1,200	1,200
Lighting:	1875			6,394	6,394
Ductwork:		4,058	0	3,663	3,663
Infiltration: Winter CFM: 224, Summer CFM: 99		9,051	3,364	1,955	5,319
System 1, Zone 1 Load Totals:		24,341	3,594	21,985	25,579

Check Figures

Supply CFM:	1,005	CFM Per Square ft.:	0.607
Square ft. of Room Area:	1,656	Square ft. Per Ton:	696
Volume (ft³) of Cond. Space:	14,906	Air Turnover Rate (per hour):	4.0

Zone Loads

Total Heating Required:	24,341 Btuh	24.341 MBH	
Total Sensible Gain:	21,985 Btuh	86 %	
Total Latent Gain:	3,594 Btuh	14 %	
Total Cooling Required:	25,579 Btuh	2.13	Tons (Based On Sensible + Latent)
		2.38	Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 2, Zone 1 Summary Loads (Average Method)

Component Description	Area Quan	Sen Loss	Lat Gain	Sen Gain	Total Gain
2A-v-o: Glazing-Double pane low-e (e = 0.60),-operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	113.1	2,302	0	2,167	2,167
2A-b-o: Glazing-Double pane low-e (e = 0.60), operable window, metal frame with break, ground reflectance = 0.23, outdoor insect screen with 100% coverage, light color blinds at 45° with 25% coverage, external shade screen coefficient of 0.45 and 100% coverage	10.9	339	0	294	294
2A-v-o: Glazing-Double pane low-e (e = 0.60), operable window, vinyl frame, ground reflectance = 0.23, outdoor insect screen with 100% coverage, external shade screen coefficient of 0.45 and 100% coverage	11.1	226	0	268	268
12E-4sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, siding finish, wood studs	1057.7	2,151	0	913	913
12E-4bw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, R-4 board insulation, brick finish, wood studs	195.1	397	0	146	146
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1377.1	1,629	0	1,893	1,893
19C-11sp: Floor-Over enclosed unconditioned crawl space, R-11 insulation on exposed walls, sealed crawl space, passive, R-11 blanket	1377.1	976	0	475	475
Subtotals for structure:		8,020	0	6,156	6,156
People:	4		920	1,200	2,120
Equipment:			0	0	0
Lighting:	1680			5,729	5,729
Ductwork:		3,145	0	2,949	2,949
Infiltration: Winter CFM: 190, Summer CFM: 85		7,699	2,860	1,664	4,524
System 2, Zone 1 Load Totals:		18,864	3,780	17,698	21,478

Check Figures

Supply CFM:	809	CFM Per Square ft.:	0.587
Square ft. of Room Area:	1,377	Square ft. Per Ton:	700
Volume (ft³) of Cond. Space:	12,679	Air Turnover Rate (per hour):	3.8

Zone Loads

Total Heating Required:	18,864 Btuh	18.864	MBH
Total Sensible Gain:	17,698 Btuh	82	%
Total Latent Gain:	3,780 Btuh	18	%
Total Cooling Required:	21,478 Btuh	1.79	Tons (Based On Sensible + Latent)
		1.97	Tons (Based On 75% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.
All computed results are estimates as building use and weather may vary.
Be sure to select a unit that meets both sensible and latent loads.

System 1 Room Load Summary

No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Air Sys CFM
—Zone 1—										
1	Bath 1	126	2,368	31	1-5	543	1,620	380	74	74
2	Dining Room	190	2,701	35	1-6	508	2,180	425	100	100
3	Kitchen	204	1,966	26	1-8	487	3,716	521	170	170
4	Nook	168	2,551	33	1-6	559	2,399	224	110	110
5	Gathering Room	272	5,594	73	1-8	503	3,840	730	176	176
6	Parior	213	4,763	62	1-7	553	3,235	732	148	148
7	Foyer	129	1,489	19	1-5	571	1,704	170	78	78
8	Pantry	49	112	1	1-2	212	101	0	5	5
9	Hall	112	816	11	1-4	515	983	90	45	45
10	Back Stairs	76	1,300	17	1-4	553	1,056	224	48	48
11	Front Stairs	70	160	2	1-3	592	636	0	29	29
12	Closet	47	521	7	1-3	476	512	98	23	23
	System 1 total	1,656	24,341	318			21,985	3,594	1,005	1,005

System 1 Main Trunk Size: 14x14 in.
Velocity: 819 ft./min
Loss per 100 ft.: 0.089 in.wg

Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Total Btuh
Net Required:	2.13	86% / 14%	21,985	3,594	25,579
Recommended:	2.38	77% / 23%	21,985	6,567	28,552
Actual:	2.40	78% / 22%	22,350	6,450	28,800

Equipment Data

	Heating System	Cooling System
Type:	Air Cooled Condensor	Air Cooled Condensor
Model:	RHF30C2*-CAPF042B2+TXV-MBE1600**-1	RHF30C2*CHPF042B2+TXV-MBE1600**-1
Brand:	Amana	Amana
Efficiency:	8.5 HFSP	15 SEER
Sound:		
Capacity:	28.600	28,800
Sensible Capacity:	n/a	22,350 Btuh
Latent Capacity:	n/a	6,450 Btuh

System 2 Room Load Summary

No	Room Name	Area SF	Htg Sens Btuh	Htg Nom CFM	Run Duct Size	Run Duct Vel	Clg Sens Btuh	Clg Lat Btuh	Clg Nom CFM	Air Sys CFM
—Zone 1—										
13	Master Bedroom	333	5,555	73	2-6	523	4,493	1,243	205	205
14	Master W.I.C	80	1,656	22	1-4	554	1,057	334	48	48
15	Master Bath	120	1,362	18	1-4	641	1,224	210	56	56
16	Bath 3	72	898	12	1-4	468	893	148	41	41
17	Bedroom 2	178	2,266	30	1-7	457	2,672	538	122	122
18	Bedroom 3	175	3,264	43	1-7	535	3,127	712	143	143
19	Hall	160	2,040	27	1-5	632	1,885	315	86	86
20	Laundry	35	79	1	1-3	524	563	0	26	26
21	Closet	48	444	6	1-2	400	191	76	9	9
22	Open To Below	176	1,300	17	1-5	533	1,590	204	73	73
	System 2 total	1,377	18,864	246			17,698	3,780	809	809

System 2 Main Trunk Size: 13x13 in.
Velocity: 757 ft./min
Loss per 100 ft.: 0.083 in.wg

Cooling System Summary

	Cooling Tons	Sensible/Latent Split	Sensible Btuh	Latent Btuh	Total Btuh
Net Required:	1.79	82% / 18%	17,698	3,780	21,478
Recommended:	1.97	75% / 25%	17,698	5,899	23,597
Actual:	2.38	78% / 22%	22,350	6,250	28,600

Equipment Data

	Heating System	Cooling System
Type:	Air Cooled Condensor	Air Cooled Condensor
Model:	RHF30C2*-CHPF042B2+TXV-MBE1600**-1	RHF30C2*-CHPF042B2+TXV-MBE1600**-1
Brand:	Amana	Amana
Efficiency:	8.5 HFPF	13 Seer
Sound:		
Capacity:	28.800	28.600
Sensible Capacity:	n/a	22,350 Btuh
Latent Capacity:	n/a	6,250 Btuh

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☐ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☐ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☐ ☐ b) Ceiling fans

☐ ☐ c) Smoke detectors

☐ ☐ d) Service panel and sub-panel size and location(s)

☐ ☐ e) Meter location with type of service entrance (overhead or underground)

☐ ☐ f) Appliances and HVAC equipment

☐ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

☐ ☐ h) Exhaust fans in bathroom

HVAC information

☐ ☐ a) Energy Calculations (dimensions shall match plans)

☐ ☐ b) Manual J sizing equipment or equivalent computation

☐ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☐ **Private Potable Water**

☐ ☐ d) Location, size and height above roof of chimneys.
☐ ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☐ ☐ b) Shear walls identified.
☐ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
☐ ☐ d) Show safety glazing of glass, where required by code.
☐ ☐ e) Identify egress windows in bedrooms, and size.
☐ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
☑ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☑ ☐ b) All posts and/or column footing including size and reinforcing
☑ ☐ c) Any special support required by soil analysis such as piling
☑ ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☑ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☑ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

PRODUCT APPROVAL SPECIFICATION SHEET

Location: 455 S.W. CHALET TERR. **Project Name:** MONIZ RES.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
✓ 1. Swinging	MASONITE	SIDE-HINGED FIBERGLASS DOORS	FL 4668.1
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
✓ 4. Double Hung	MI WINDOWS & DOORS	CELLular PVC TRIPLE Mulled	FL 4175.1, 4173.1, & FL 686.1
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding		HARDY PLANK	
2. Soffits		HARDY PLANK	
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
✓ 10. Other		BLUE BOARD (INSIDE)	
D. ROOFING PRODUCTS	EIKO	LAMINATED ASPHALT/FIBERGLASS	FL 1476.1
✓ 1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

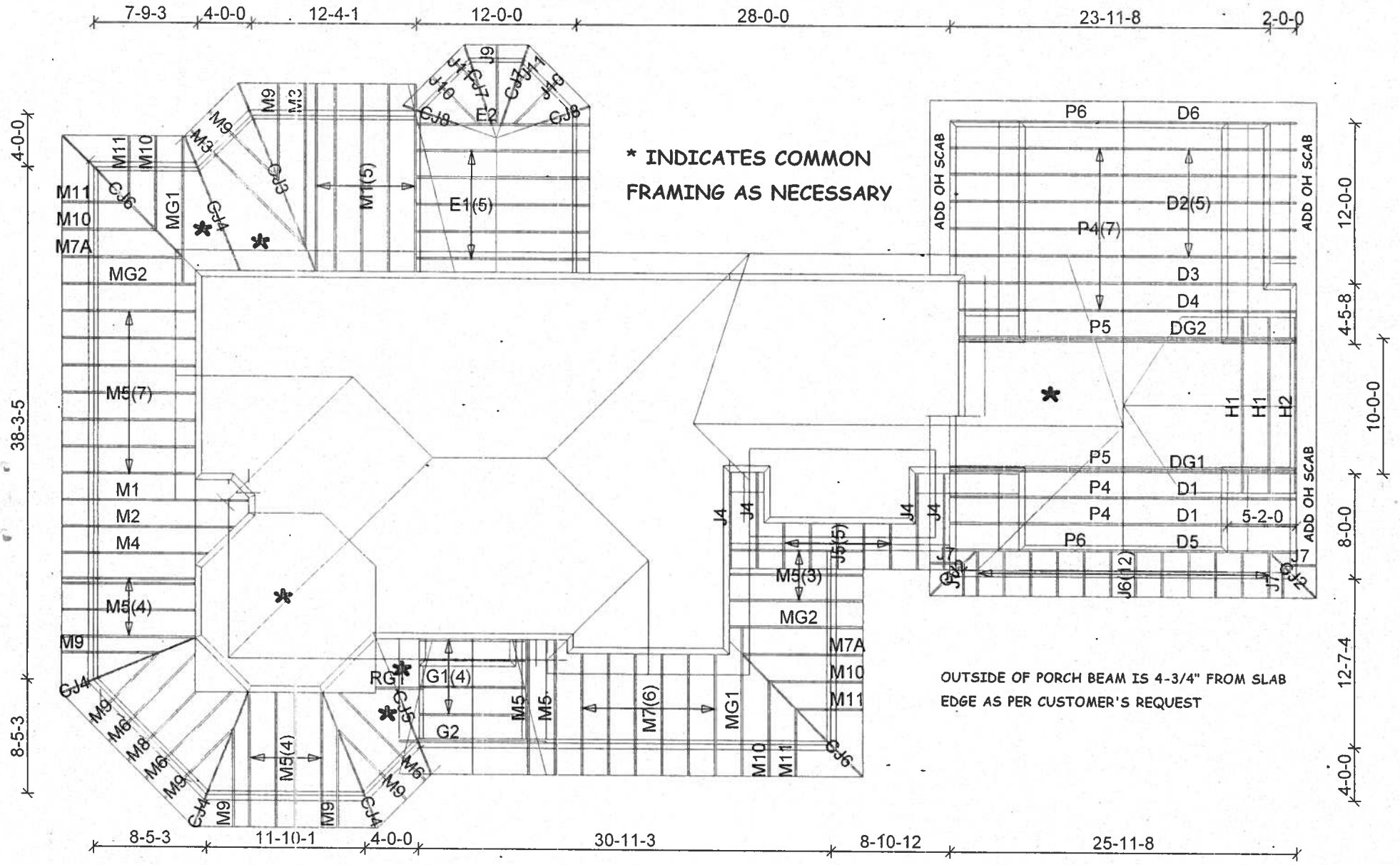
WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
☑	☐	**Elevations including:** a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation



Mayo Truss Co. Inc. 845 East US 27 MAYO, FL 32066 (386)294-3988 (877)-558-6262	ALBERT MONIZ 110 MPH ASCE WIND LOAD	Roof Loading TC Live: 20.00 psf TC Dead: 10.00 psf BC Live: 0.00 psf BC Dead: 10.00 psf TC Stress Inc: 25.00 BC Stress Inc: 25.00 Spacing: 2- 0- 0 o.c.	Account: INDIVIDUAL Job: ALBERTMONIZ Designer: A. HIGHSMITH Checker: M.MURRAY Date: 08-15-06

Permit Number: _____ Lot Number: _____

Miscellaneous: _____ Address: _____

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: ALBERTMONIZ - ALBERT MONIZ

Standard Loading:

T.C. Live	20 psf
T.C. Dead	10 psf
B.C. Live	0 psf
B.C. Dead	10 psf
Total	40 psf

ROBBINS ENGINEERING, INC.

P.O. Box 280055
Tampa, FL 33682-0055
Phone: (813) 972-1135

Engineering Index Sheet

Index Page 1 of 1

ANSI/ASCE 7-02
Wind Speed - 110 MPH
Mean Roof Ht. - 15 FT
Exposure Catergory - B
Occupancy Factor - 1.00
MWFRS
Enclosed

Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06080605	08/07/2006		46

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibilty of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specificstion Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Sofware - Online Plus)

Notes: Refer to individual truss design drawings for special loading conditions.

	Date	Mark
1	08/07/06	CJ2
5	08/07/06	CJ6
9	08/07/06	D2
13	08/07/06	D6
17	08/07/06	E2
21	08/07/06	H2
25	08/07/06	J7
29	08/07/06	M1
33	08/07/06	M5
37	08/07/06	M8
41	08/07/06	MG1
45	08/07/06	P6

	Date	Mark
2	08/07/06	CJ3
6	08/07/06	CJ7
10	08/07/06	D3
14	08/07/06	DG1
18	08/07/06	G1
22	08/07/06	J4
26	08/07/06	J9
30	08/07/06	M2
34	08/07/06	M6
38	08/07/06	M9
42	08/07/06	MG2
46	08/07/06	RG1

	Date	Mark
3	08/07/06	CJ4
7	08/07/06	CJ8
11	08/07/06	D4
15	08/07/06	DG2
19	08/07/06	G2
23	08/07/06	J5
27	08/07/06	J10
31	08/07/06	M3
35	08/07/06	M7
39	08/07/06	M10
43	08/07/06	P4

	Date	Mark
4	08/07/06	CJ5
8	08/07/06	D1
12	08/07/06	D5
16	08/07/06	E1
20	08/07/06	H1
24	08/07/06	J6
28	08/07/06	J11
32	08/07/06	M4
36	08/07/06	M7A
40	08/07/06	M11
44	08/07/06	P5

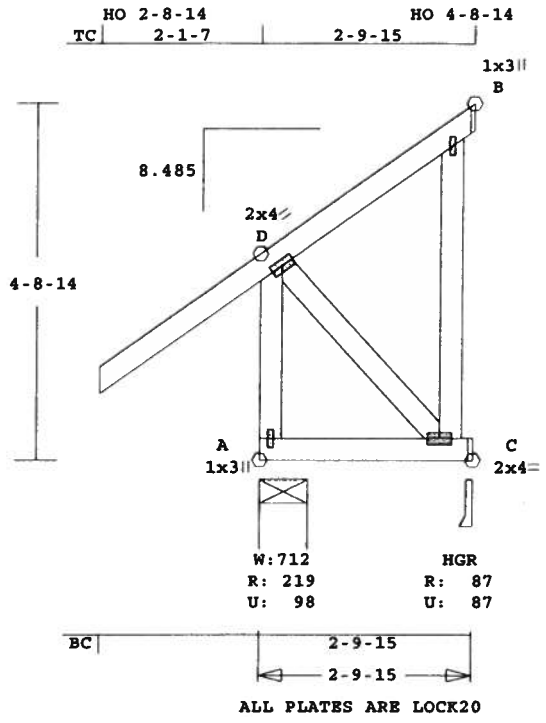
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

VUONG PHAN
LICENSE
No. 62111
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	CJ2	2	MONO.DD	20'9"	8.485	2- 1- 7	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0.389" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 34.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.01	2x 4	SP-#2
BC	0.01	2x 4	SP-#2
WB	0.03	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	2- 9-15
BC Cont.	0- 0- 0	2- 9-15

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	2.8'
BC V 0	20	0.0'	2.8'
TC V -40	-20	0.0'	
-26	-13		2.8'
BC V 0	-20	0.0'	
0	-13		2.8'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	219	98	7-12	1- 8
		Hz =	-106	
C	87	87	3- 8	1- 8

Hz = 111

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.01	47 T	0.00 0.01
--------Bottom Chords---------			
A -C	0.01	87 T	
------------Webs------------			
A -D	0.01	81 C	WindLd
D -C	0.01	82 T	
C -B	0.03	27 T	WindLd

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.01

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
D	LOCK	2.0x 4.0 Ctr	Ctr	0.75
B	LOCK	1.0x 3.0 Ctr	Ctr	0.75
A	LOCK	1.0x 3.0 Ctr	Ctr	0.75
C	LOCK	2.0x 4.0 Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

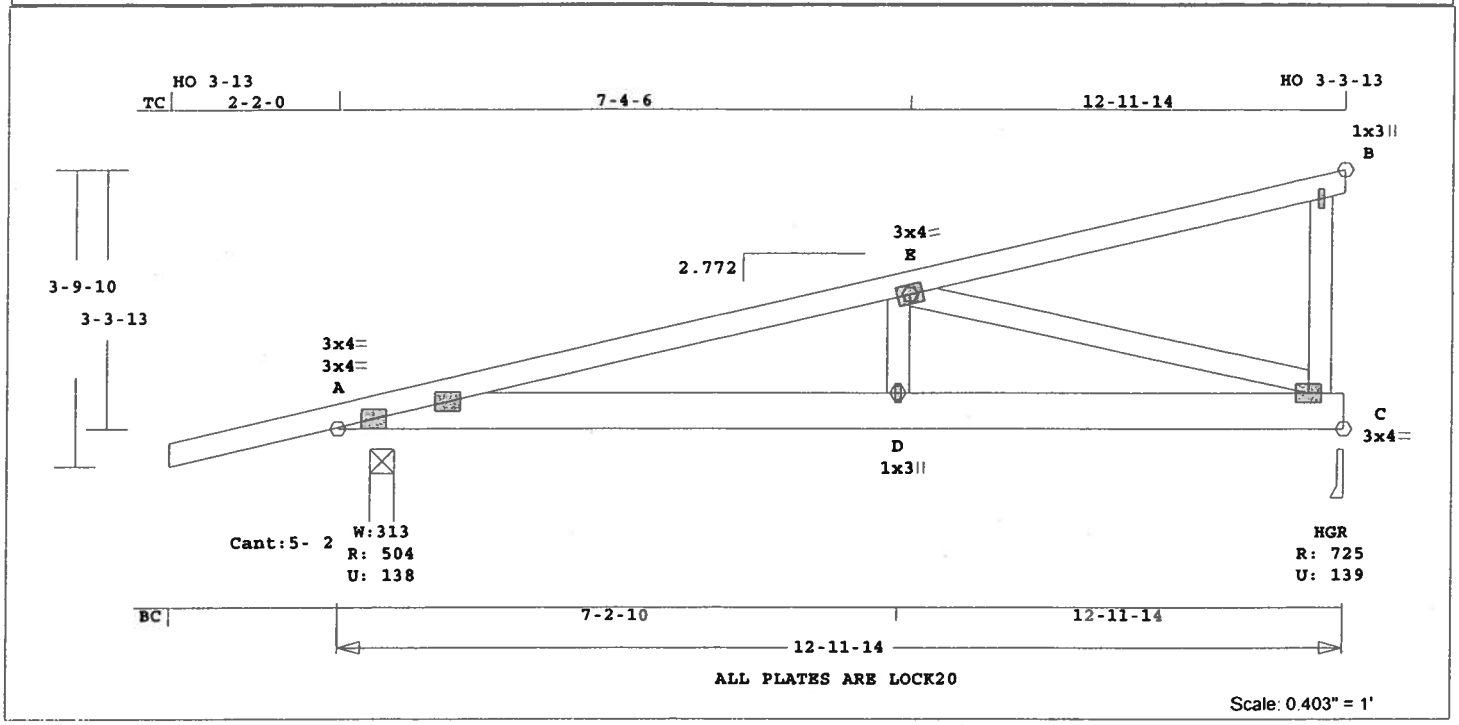
Girder King Jack
Loading TC and BC
Setback 2- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 81 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	CJ3	1	MONO	12'11¼"	2.772	2- 2- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 83.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.53	2x 4	SP-#2
BC	0.26	2x 6	SP-#2
WB	0.47	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	12-11-14
BC Cont.		0- 0- 0	12-11-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	13.0'
BC V	0	20	0.0'	13.0'
TC V	-40	-20	0.0'	
	44	22		13.0'
BC V	0	-20	0.0'	
	0	22		13.0'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	505	139	3-13	1- 8
			Hz =	56
C	725	139	3- 8	1- 8
			Hz =	108

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.49	1178 C	0.01	0.48
E -B	0.53	43 C	0.00	0.53
--------Bottom Chords---------				
A -D	0.24	1163 T	0.15	0.09
D -C	0.26	1163 T	0.15	0.11
-------------Webs-------------				
D -E	0.04	301 T		
E -C	0.47	1212 C		
C -B	0.03	263 C	WindLd	

TL Defl -0.06" in A -D L/999
LL Defl -0.03" in A -D L/999
Shear // Grain in E -B 0.35

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.46
A	LOCK	3.0x 4.0	Ctr	Ctr	0.12
E	LOCK	3.0x 4.0	Ctr	Ctr	0.65
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	3.0x 4.0	Ctr	Ctr	0.77

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

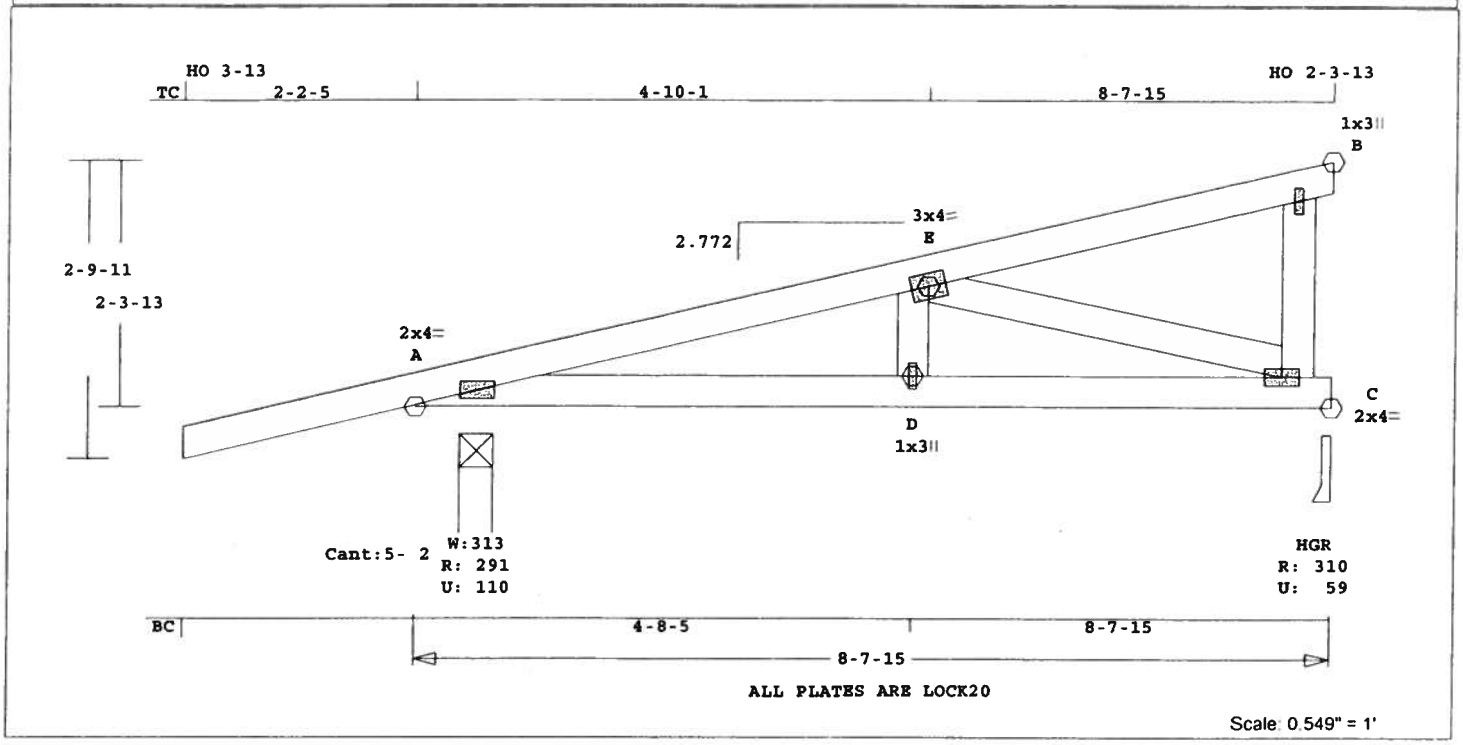
Girder King Jack
Loading TC and BC
Setback 12- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1212 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	CJ4	4	MONO	80715	2.772	2- 2- 5	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 4	SP-#2
BC	0.14	2x 4	SP-#2
WB	0.08	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	8- 7-15
BC Cont.		0- 0- 0	8- 7-15

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	8.7'
BC V	0	20	0.0'	8.7'
TC V	-40	-20	0.0'	
	14	7		8.7'
BC V	0	-20	0.0'	
	0	7		8.7'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	292	110	3-13	1- 8
			Hz =	-22
C	311	60	3- 8	1- 8
			Hz =	55

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -E	0.10	446 C	0.00	0.10
E -B	0.16	23 C	0.00	0.16
Bottom Chords				
A -D	0.10	439 T	0.05	0.05
D -C	0.14	439 T	0.08	0.06
Webs				
D -E	0.01	119 T		
E -C	0.08	459 C		
C -B	0.01	118 C	WindLd	

TL Defl -0.02" in D -C L/999
LL Defl -0.01" in D -C L/999
Shear // Grain in E -B 0.14

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.91
E	LOCK	3.0x 4.0	Ctr	Ctr	0.40
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.79

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder King Jack

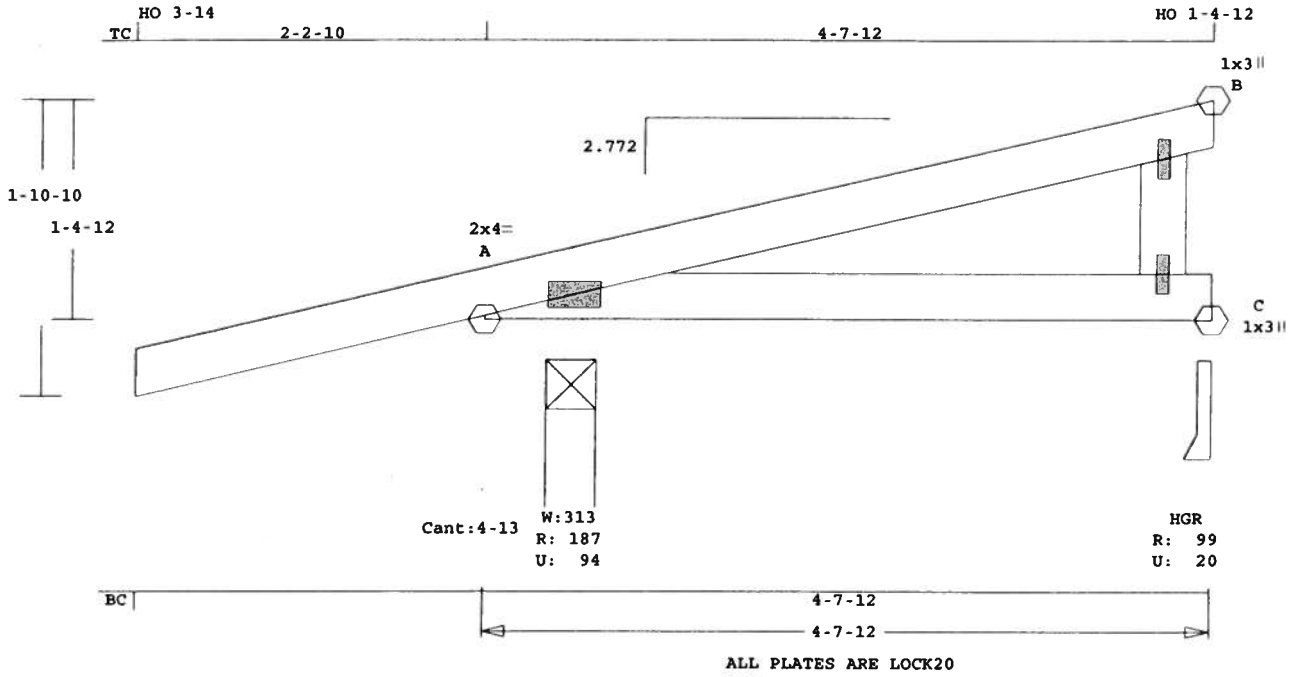
Loading TC and BC
Setback 8- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 459 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	CJ5	1	JCA2	40712	2.772	2- 2-10	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.7 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 7-12
BC Cont.	0- 0- 0	4- 7-12

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	4.6'
BC V	0	20	0.0'	4.6'
TC V	-40	-20	0.0'	
	-9	-5		4.6'
BC V	0	-20	0.0'	
	0	-5		4.6'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	187	95	3-13	1- 8
			Hz =	-9
C	100	20	3- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.07	10 C	0.00 0.07
--------Bottom Chords---------			
A -C	0.05	13 T	0.00 0.05
------------Webs------------			
C -B	0.00	59 C	WindLd

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.06

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.81
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 4- 0- 0

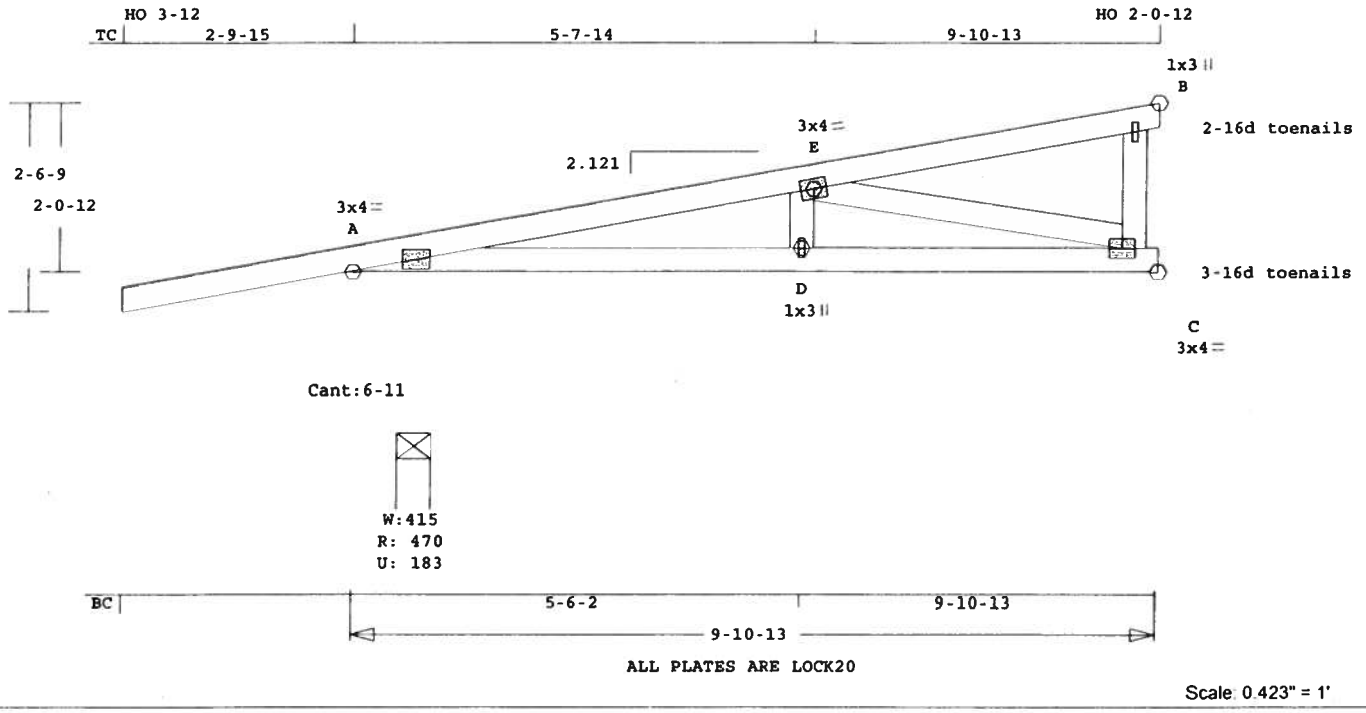
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 59 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	CJ6	2	MONO	91013	2.121	2- 9-15	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 54.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.31	2x 4	SP-#2
WB	0.23	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9-10-13
BC Cont.	0- 0- 0	9-10-13

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.9'
BC V	0	20	0.0'	9.9'
TC V	-40	-20	0.0'	
	45	22		9.9'
BC V	0	-20	0.0'	
	0	22		9.9'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	471	184	4-15	1- 8
			Hz =	44
C	339	28	1- 8	1- 8
B	232	111	1- 8	1- 8
			Hz =	67

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.19	1019 C	0.00	0.19
E -B	0.33	41 T	0.00	0.33
--------Bottom Chords---------				
A -D	0.23	1009 T	0.18	0.05
D -C	0.31	1009 T	0.18	0.13
-------------Webs-------------				
D -E	0.03	216 T		
E -C	0.23	1039 C		
C -B	0.00	0 T	WindLd	

TL Defl -0.06" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in E -B 0.26

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.74
E	LOCK	3.0x 4.0	Ctr	Ctr	0.55
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	3.0x 4.0	Ctr	Ctr	0.62

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

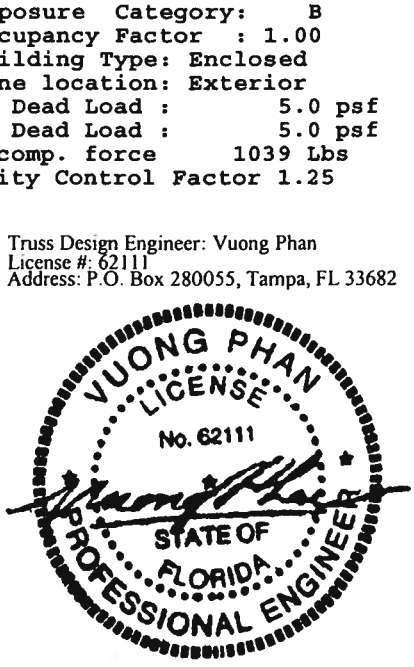
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:

Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1039 Lbs
Quality Control Factor 1.25

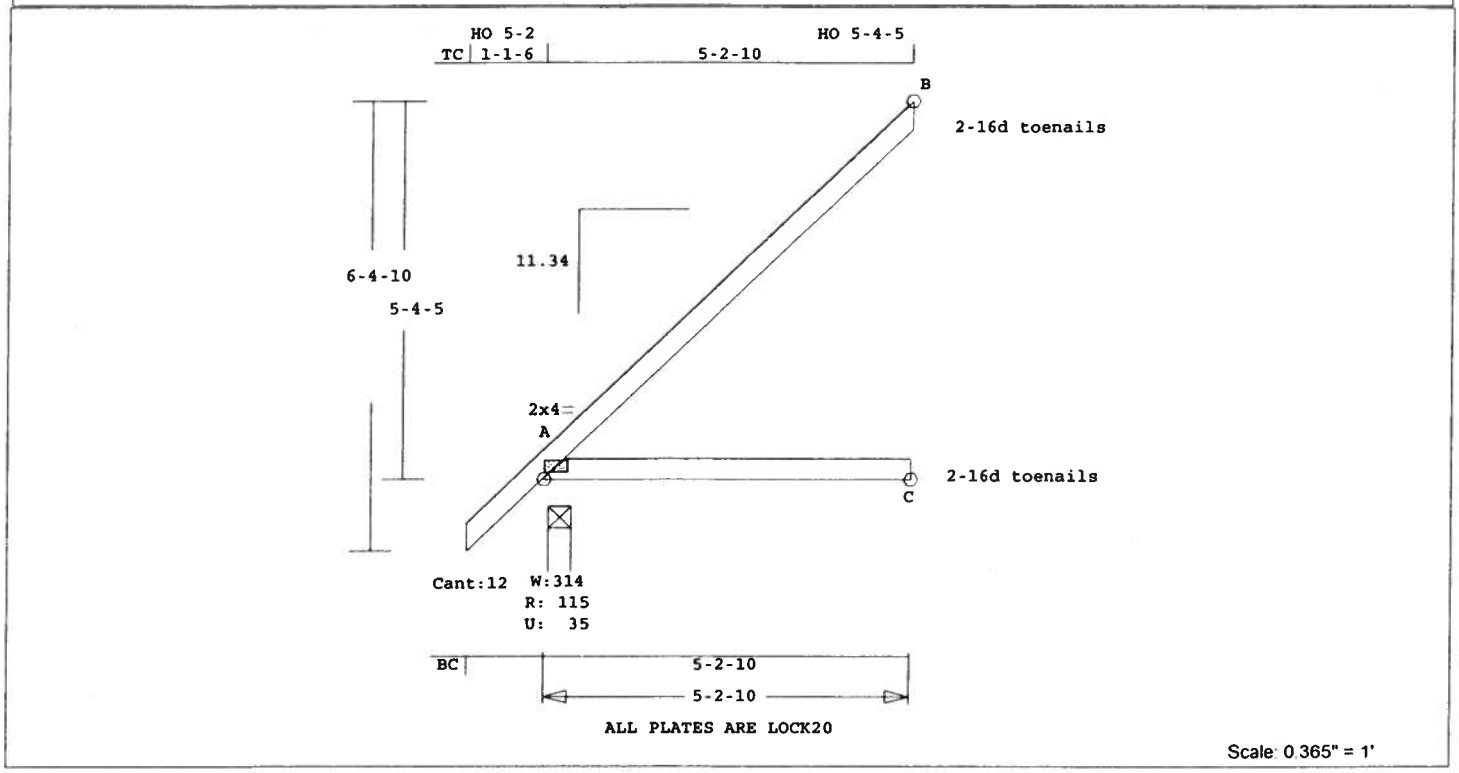
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	CJ7	2	JCA2	50210	11.34	1- 1- 6	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 26.8 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 4	SP-#2
BC	0.06	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 2-10
BC Cont.	0- 0- 0	5- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	5.2'
BC V	0	20	0.0'	5.2'
TC V	-40	-20	0.0'	
	-14	-7		5.2'
BC V	0	-20	0.0'	
	0	-7		5.2'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	116	35	3-14	1- 8
			Hz = 47	
C	42	0	1- 8	1- 8
B	59	46	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.10	37 C	0.00 0.10
--------Bottom Chords---------			
A -C	0.06	0 T	0.00 0.06

TL Defl -0.02" in A -C L/999
LL Defl -0.01" in A -C L/999
Shear // Grain in A -B 0.05

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	2.0x 4.0	0.3	0.3	0.59

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

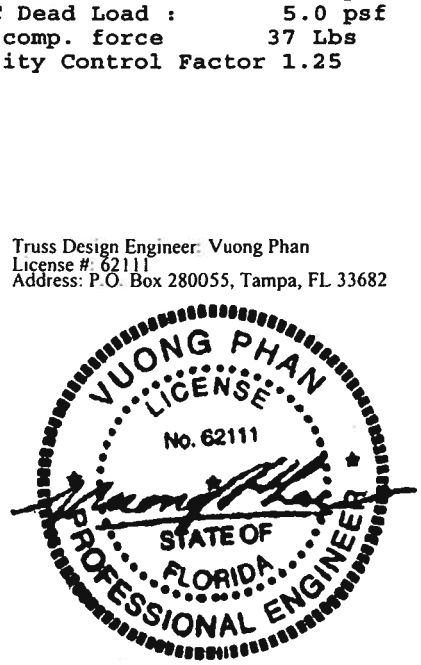
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder King Jack
Loading TC and BC

Hz = 32
Setback 4-11- 4
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 37 Lbs
Quality Control Factor 1.25

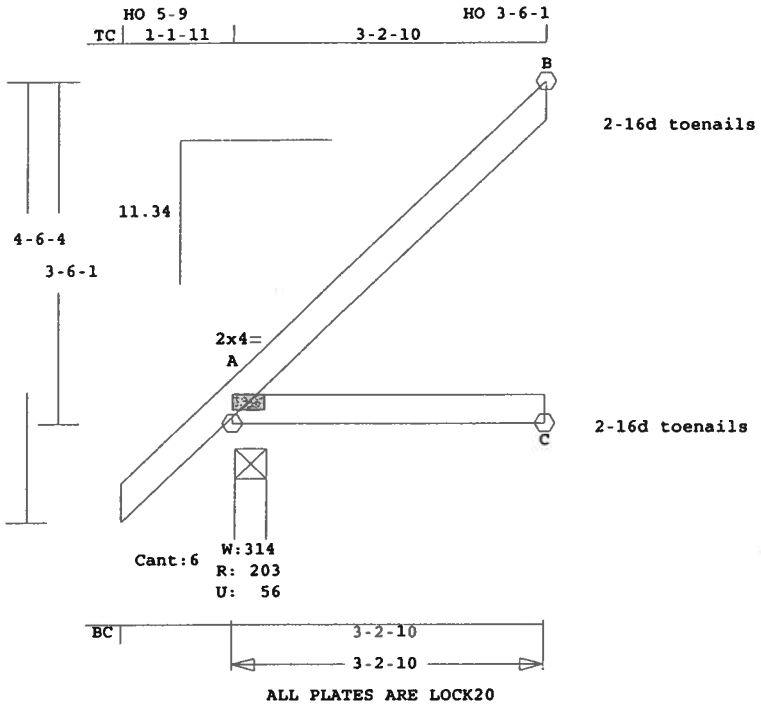
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	CJ8	2	JCA2	' 30210	11.34	1- 1-11	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20

Scale: 0.504" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 17.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 4	SP-#2
BC	0.07	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 2-10
BC Cont.	0- 0- 0	3- 2-10

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	203	57	3-14	1- 8
			Hz =	91
C	59	0	1- 8	1- 8
B	82	64	1- 8	1- 8
			Hz =	62

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.10	54 C	0.00 0.10
--------Bottom Chords---------			
A -C	0.07	0 T	0.00 0.07

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.5	0.5	0.61

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

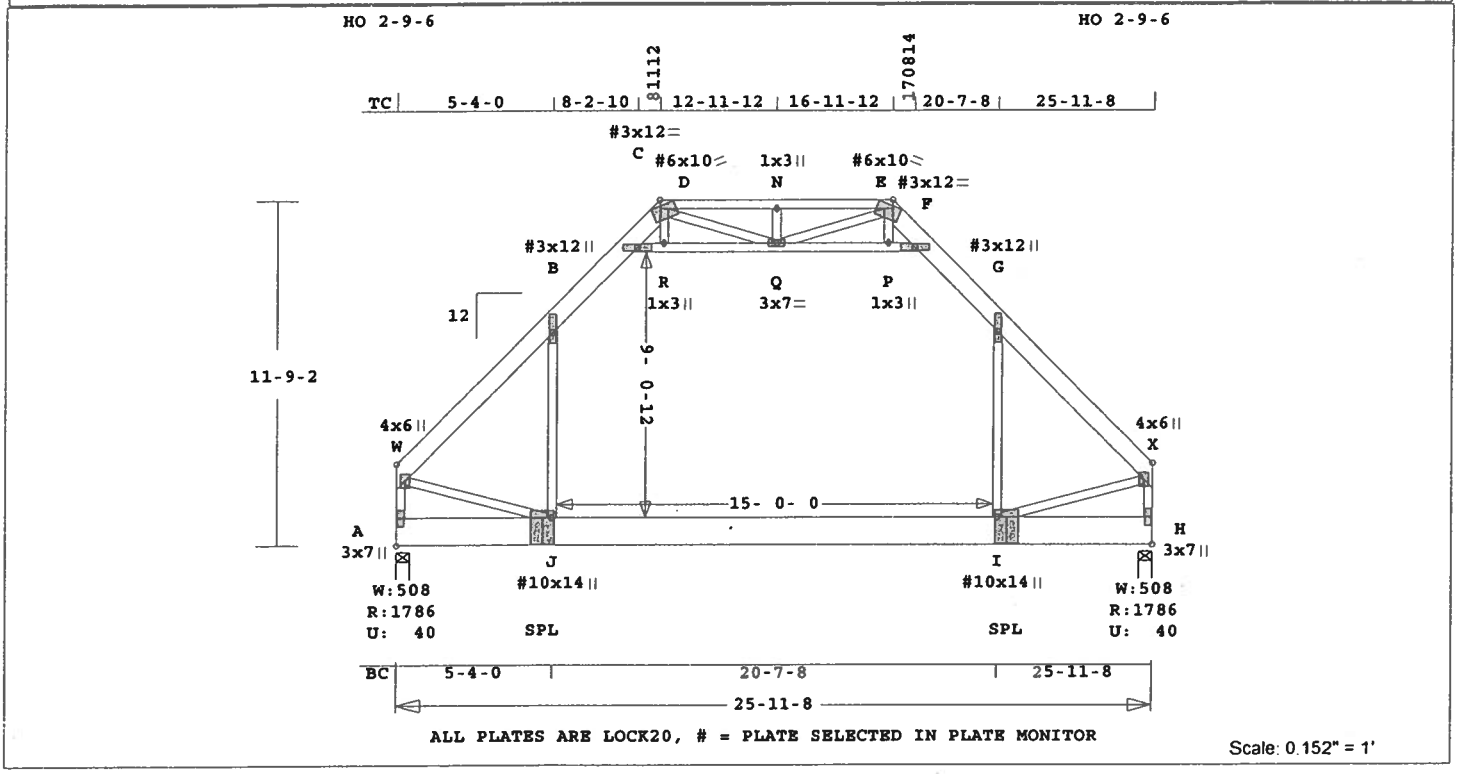
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 54 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D1	2	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.152" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 350.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	Size	Lumber
TC	0.62	2x 8	SP-#2
EX D -E		2x 4	SP-#2
BC	0.80	2x12	SP-#2
WB	0.29	2x 4	SP-#2
ACT	0.16	2x 4	SP-#2
AWT	0.13	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8-11-12
TC 2- 0- 0	8-11-12	16-11-12
TC Cont.	16-11-12	25-11- 8
BC Cont.	0- 0- 0	25-11- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	40	20	0.0'	26.0'
BC V	0	20	0.0'	26.0'
TC V	0	10	5.5'	8.1'
TC V	0	10	17.9'	20.5'
BC V	80	10	5.5'	20.5'
MA V	0	10	8.2'	17.7'
MA V	0	10	0.5'	7.3'
MA V	0	10	0.5'	7.3'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1787	40	5- 8	2- 2
			Hz =	-282
H	1787	40	5- 8	2- 2
			Hz =	283

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
W -B	0.58	1785	C	0.00	0.58
B -C	0.62	1149	C	0.00	0.62
C -D	0.62	340	T	0.00	0.62
D -N	0.26	318	T	0.00	0.26
N -E	0.26	318	T	0.00	0.26
E -F	0.62	340	T	0.00	0.62
F -G	0.62	1149	C	0.00	0.62
G -X	0.58	1785	C	0.00	0.58
Bottom Chords					
A -J	0.56	267	T	0.00	0.56
J -I	0.80	1178	T	0.11	0.69
I -H	0.56	267	T	0.00	0.56
Webs					
A -W	0.23	1972	C	WindLd	
W -J	0.29	1265	T		
J -B	0.18	971	T		
I -G	0.18	971	T		
I -X	0.29	1265	T		
H -X	0.23	1972	C	WindLd	
Attic Chords (Top)					
C -R	0.16	1675	C	0.16	0.00
R -Q	0.08	1677	C	0.02	0.06
Q -P	0.08	1677	C	0.02	0.06
P -F	0.16	1675	C	0.16	0.00
Attic Webs (Top)					
R -D	0.01	52	T		
D -Q	0.13	576	T		
Q -N	0.03	299	C		
Q -E	0.13	576	T		
P -E	0.01	52	T		

TL Defl -0.44" in J -I L/685
LL Defl -0.32" in J -I L/926
Shear // Grain in B -C 0.45

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
W LOCK	4.0x 6.0	Ctr	Ctr	0.77
B# LOCK	3.0x12.0	Ctr	1.6	0.26
C# LOCK	3.0x12.0	-0.1	Ctr	0.37
D# LOCK	6.0x10.0	0.2	-1.3	0.57
N LOCK	1.0x 3.0	Ctr	Ctr	0.81
E# LOCK	6.0x10.0	-0.3	-1.3	0.57
F# LOCK	3.0x12.0	0.1	Ctr	0.37
G# LOCK	3.0x12.0	Ctr	1.6	0.26
X LOCK	4.0x 6.0	Ctr	Ctr	0.77
A LOCK	3.0x 7.0	Ctr	Ctr	0.77
J# LOCK	10.0x14.0	Ctr	1.8	0.42
I# LOCK	10.0x14.0	Ctr	1.8	0.42
H LOCK	3.0x 7.0	Ctr	Ctr	0.77
R LOCK	1.0x 3.0	Ctr	Ctr	0.81
Q LOCK	3.0x 7.0	Ctr	Ctr	0.77
P LOCK	1.0x 3.0	Ctr	Ctr	0.81

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

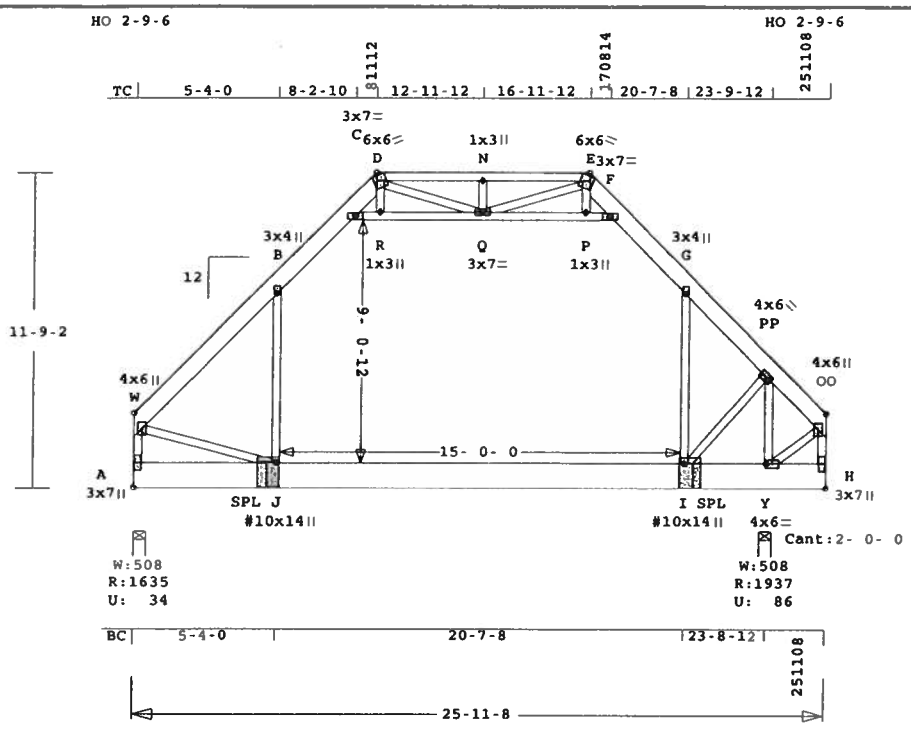
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D1	2	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ

Load Factors = 1.00 and 0.00
Max comp. force 1972 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D2	5	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.139" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 359.4 LBS

Online Plus -- Version 19.0.034
RUN DATE: 07-AUG-06

	CSI	Size	Lumber
TC	0.52	2x 8	SP-#2
EX D -E		2x 4	SP-#2
BC	1.00	2x12	SP-#2
WB	0.43	2x 4	SP-#2
ACT	0.12	2x 4	SP-#2
AWT	0.13	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	25-11- 8
BC Cont.	0- 0- 0	25-11- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	40	20	0.0'	26.0'
BC V	0	20	0.0'	26.0'
TC V	0	10	5.5'	8.1'
TC V	0	10	17.9'	20.5'
BC V	80	10	5.5'	20.5'
MA V	0	10	8.2'	17.7'
MA V	0	10	0.5'	7.3'
MA V	0	10	0.5'	7.3'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	1636	35	5- 8	1-15
		Hz =	-282	
Y	1938	87	5- 8	2- 2
		Hz =	283	

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
W -B	0.52	1471 C	0.00 0.52
B -C	0.51	985 C	0.00 0.51
C -D	0.37	178 T	0.00 0.37
D -N	0.20	295 C	0.00 0.20
N -E	0.20	295 C	0.00 0.20
E -F	0.52	221 T	0.00 0.52
F -G	0.52	1011 C	0.00 0.52
G -PP	0.34	1504 C	0.00 0.34
PP-OO	0.08	141 C	0.00 0.08
Bottom Chords			
A -J	0.27	267 T	0.00 0.27
J -I	1.00	965 T	0.09 0.91
I -Y	0.92	269 T	0.01 0.91
Y -H	0.19	25 C	0.00 0.19
Webs			
A -W	0.19	1657 C	WindLd
W -J	0.23	1036 T	
J -B	0.13	787 T	
I -G	0.15	876 T	
I -PP	0.31	1358 T	
Y -PP	0.43	2324 C	
Y -OO	0.04	174 T	
H -OO	0.03	253 C	WindLd
Attic Chords (Top)			
C -R	0.11	1249 C	0.11 0.00
R -Q	0.07	1252 C	0.01 0.06
Q -P	0.07	1342 C	0.01 0.06
P -F	0.12	1339 C	0.12 0.00
Attic Webs (Top)			
R -D	0.01	53 T	
D -Q	0.10	462 T	
Q -N	0.03	268 C	
Q -E	0.13	583 T	
P -E	0.01	54 T	

TL Defl -0.37" in J -I L/744
LL Defl -0.28" in J -I L/990
LL Cant 0.01" in Y -H L/999
Shear // Grain in J -I 0.49

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
W	LOCK	4.0x 6.0	Ctr	Ctr	0.65
B	LOCK	3.0x 4.0	Ctr	1.6	0.67
C	LOCK	3.0x 7.0	-0.1	Ctr	0.28
D	LOCK	6.0x 6.0	1.6-3.9		0.55
N	LOCK	1.0x 3.0	Ctr	Ctr	0.81
E	LOCK	6.0x 6.0	-1.6-3.9		0.55
F	LOCK	3.0x 7.0	0.1	Ctr	0.30
G	LOCK	3.0x 4.0	Ctr	1.6	0.75
PP	LOCK	4.0x 6.0	0.4-0.4		0.87
OO	LOCK	4.0x 6.0	Ctr	Ctr	0.58
A	LOCK	3.0x 7.0	Ctr	Ctr	0.77
J#	LOCK	10.0x14.0	0.7	1.4	0.34
I#	LOCK	10.0x14.0	-1.2	1.5	0.33
Y	LOCK	4.0x 6.0	Ctr	Ctr	0.57
H	LOCK	3.0x 7.0	Ctr	Ctr	0.77
R	LOCK	1.0x 3.0	Ctr	Ctr	0.81
Q	LOCK	3.0x 7.0	Ctr	Ctr	0.77
P	LOCK	1.0x 3.0	Ctr	Ctr	0.81

\# = Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

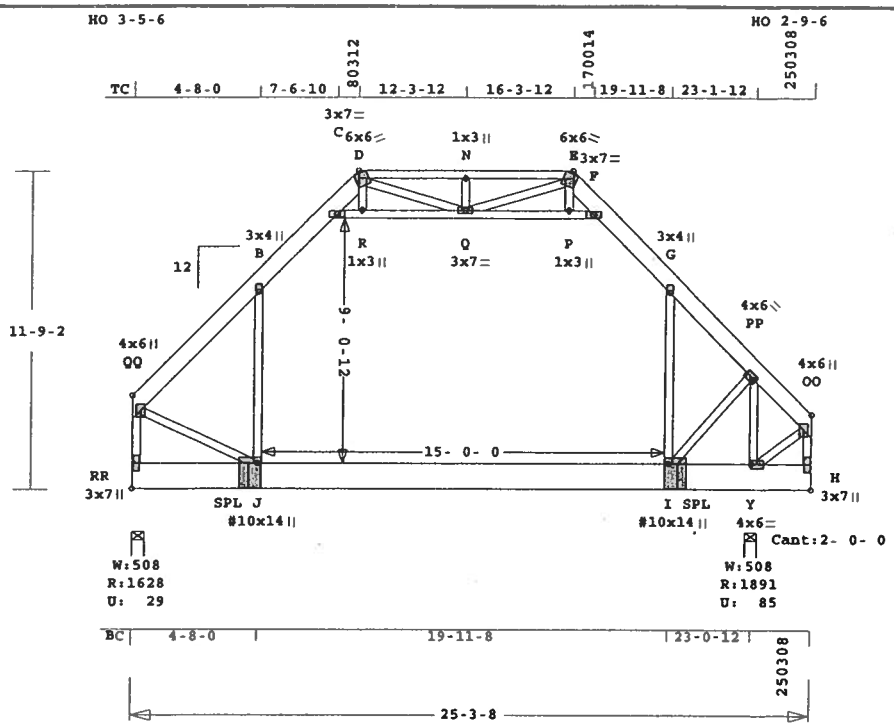
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D2	5	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ

Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 2324 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D3	1	ATPB	250308	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 352.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 07-AUG-06

	CSI	-Size-	----Lumber----
TC	0.45	2x 8	SP-#2
EX D -E		2x 4	SP-#2
BC	0.89	2x12	SP-#2
WB	0.41	2x 4	SP-#2
ACT	0.11	2x 4	SP-#2
AWT	0.12	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	25- 3- 8
BC Cont.		0- 0- 0	25- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	40	20	0.0'	25.3'
BC V	0	20	0.0'	25.3'
TC V	0	10	4.8'	7.4'
TC V	0	10	17.2'	19.8'
BC V	80	10	4.8'	19.8'
MA V	0	10	7.6'	17.1'
MA V	0	10	0.5'	7.3'
MA V	0	10	0.5'	7.3'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
RR	1629	29	5- 8	1-15
			Hz =	-288
Y	1892	85	5- 8	2- 1
			Hz =	280

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
QQ-B	0.45	1392	C	0.00	0.45
B -C	0.45	949	C	0.00	0.45
C -D	0.34	211	C	0.00	0.34
D -N	0.19	334	C	0.00	0.19
N -E	0.19	334	C	0.00	0.19
E -F	0.45	191	C	0.00	0.45
F -G	0.45	968	C	0.00	0.45
G -PP	0.34	1419	C	0.00	0.34
PP-OO	0.08	132	C	0.00	0.08
--------Bottom Chords---------					
RR-J	0.37	268	T	0.00	0.37
J -I	0.89	908	T	0.08	0.81
I -Y	0.82	266	T	0.01	0.81
Y -H	0.18	25	C	0.00	0.18
------------Webs------------					
RR-QQ	0.22	1747	C	WindLd	
QQ-J	0.24	1058	T		
J -B	0.12	753	T		
I -G	0.13	824	T		
I -PP	0.29	1278	T		
Y -PP	0.41	2202	C		
Y -OO	0.03	165	T		
H -OO	0.02	240	C	WindLd	
------Attic Chords (Top)------					
C -R	0.10	1157	C	0.10	0.00
R -Q	0.07	1159	C	0.01	0.06
Q -P	0.07	1225	C	0.01	0.06
P -F	0.11	1223	C	0.11	0.00
-------Attic Webs (Top)-------					
R -D	0.01	53	T		
D -Q	0.10	463	T		
Q -N	0.03	259	C		
Q -E	0.12	552	T		
P -E	0.01	54	T		

TL Defl -0.34" in J -I L/783
LL Defl -0.26" in J -I L/999
LL Cant 0.01" in Y -H L/999
Shear // Grain in J -I 0.47

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
QQ	LOCK	4.0x 6.0	Ctr	Ctr	0.68
B	LOCK	3.0x 4.0	Ctr	1.6	0.62
C	LOCK	3.0x 7.0-0.1	Ctr		0.26
D	LOCK	6.0x 6.0	1.6-3.9		0.54
N	LOCK	1.0x 3.0	Ctr	Ctr	0.81
E	LOCK	6.0x 6.0-1.6-3.9			0.54
F	LOCK	3.0x 7.0	0.1	Ctr	0.27
G	LOCK	3.0x 4.0	Ctr	1.6	0.68
PP	LOCK	4.0x 6.0	0.4-0.4		0.82
OO	LOCK	4.0x 6.0	Ctr	Ctr	0.57
RR	LOCK	3.0x 7.0	Ctr	Ctr	0.76
J	LOCK	10.0x14.0	0.7	1.4	0.34
I	LOCK	10.0x14.0-0.8	1.4		0.33
Y	LOCK	4.0x 6.0	Ctr	Ctr	0.54
H	LOCK	3.0x 7.0	Ctr	Ctr	0.76
R	LOCK	1.0x 3.0	Ctr	Ctr	0.81
Q	LOCK	3.0x 7.0	Ctr	Ctr	0.76
P	LOCK	1.0x 3.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

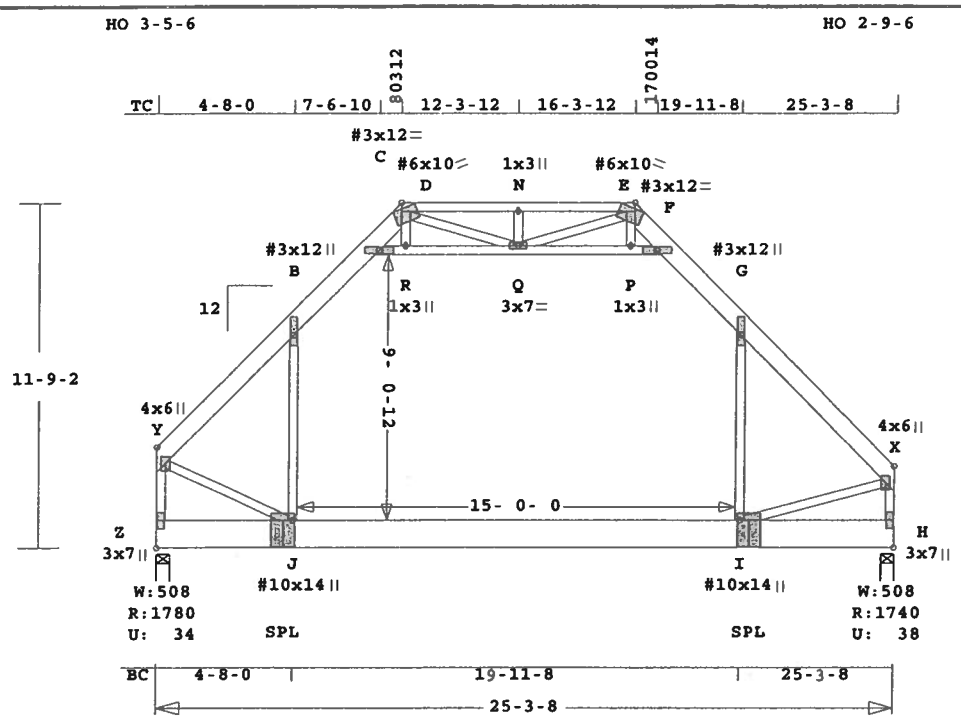
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D3	1	ATPB	25'03'08	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ

Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 2202 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D4	1	ATPB	250308	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.152" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 343.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	Size	Lumber
TC	0.58	2x 8	SP-#2
EX D -E		2x 4	SP-#2
BC	0.77	2x12	SP-#2
WB	0.29	2x 4	SP-#2
ACT	0.15	2x 4	SP-#2
AWT	0.13	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 3-12
TC 2- 0- 0	8- 3-12	16- 3-12
TC Cont.	16- 3-12	25- 3- 8
BC Cont.	0- 0- 0	25- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	40	20	0.0'	25.3'
BC V	0	20	0.0'	25.3'
TC V	0	10	4.8'	7.4'
TC V	0	10	17.2'	19.8'
BC V	80	10	4.8'	19.8'
MA V	0	10	7.6'	17.1'
MA V	0	10	0.5'	7.3'
MA V	0	10	0.5'	7.3'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Z	1780	35	5- 8	2- 2
			Hz =	-288
H	1740	39	5- 8	2- 1
			Hz =	281

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
Y -B	0.51	1693 C	0.00 0.51
B -C	0.58	1105 C	0.00 0.58
C -D	0.58	300 T	0.00 0.58
D -N	0.24	268 T	0.00 0.24
N -E	0.24	268 T	0.00 0.24
E -F	0.54	288 T	0.00 0.54
F -G	0.55	1099 C	0.00 0.55
G -X	0.56	1687 C	0.00 0.56
Bottom Chords			
Z -J	0.67	268 T	0.00 0.67
J -I	0.77	1112 T	0.10 0.67
I -H	0.47	264 T	0.00 0.47
Webs			
Z -Y	0.27	2091 C	WindLd
Y -J	0.29	1294 T	
J -B	0.17	930 T	
I -G	0.16	913 T	
I -X	0.27	1194 T	
H -X	0.22	1874 C	WindLd
Attic Chords (Top)			
C -R	0.15	1565 C	0.15 0.00
R -Q	0.08	1567 C	0.02 0.06
Q -P	0.07	1544 C	0.02 0.05
P -F	0.14	1542 C	0.14 0.00
Attic Webs (Top)			
R -D	0.01	53 T	
D -Q	0.13	575 T	
Q -N	0.03	288 C	
Q -E	0.12	543 T	
P -E	0.01	52 T	

TL Defl -0.41" in J -I L/712
LL Defl -0.31" in J -I L/954
Shear // Grain in J -I 0.45

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
Y	LOCK	4.0x 6.0	Ctr	Ctr	0.82
B#	LOCK	3.0x12.0	Ctr	1.6	0.25
C#	LOCK	3.0x12.0	-0.1	Ctr	0.35
D#	LOCK	6.0x10.0	0.2	-1.4	0.57
N	LOCK	1.0x 3.0	Ctr	Ctr	0.81
E#	LOCK	6.0x10.0	-0.3	-1.3	0.56
F#	LOCK	3.0x12.0	0.1	Ctr	0.34
G#	LOCK	3.0x12.0	Ctr	1.6	0.25
X	LOCK	4.0x 6.0	Ctr	Ctr	0.73
Z	LOCK	3.0x 7.0	Ctr	Ctr	0.76
J#	LOCK	10.0x14.0	Ctr	1.8	0.42
I#	LOCK	10.0x14.0	Ctr	1.8	0.42
H	LOCK	3.0x 7.0	Ctr	Ctr	0.76
R	LOCK	1.0x 3.0	Ctr	Ctr	0.81
Q	LOCK	3.0x 7.0	Ctr	Ctr	0.76
P	LOCK	1.0x 3.0	Ctr	Ctr	0.81

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

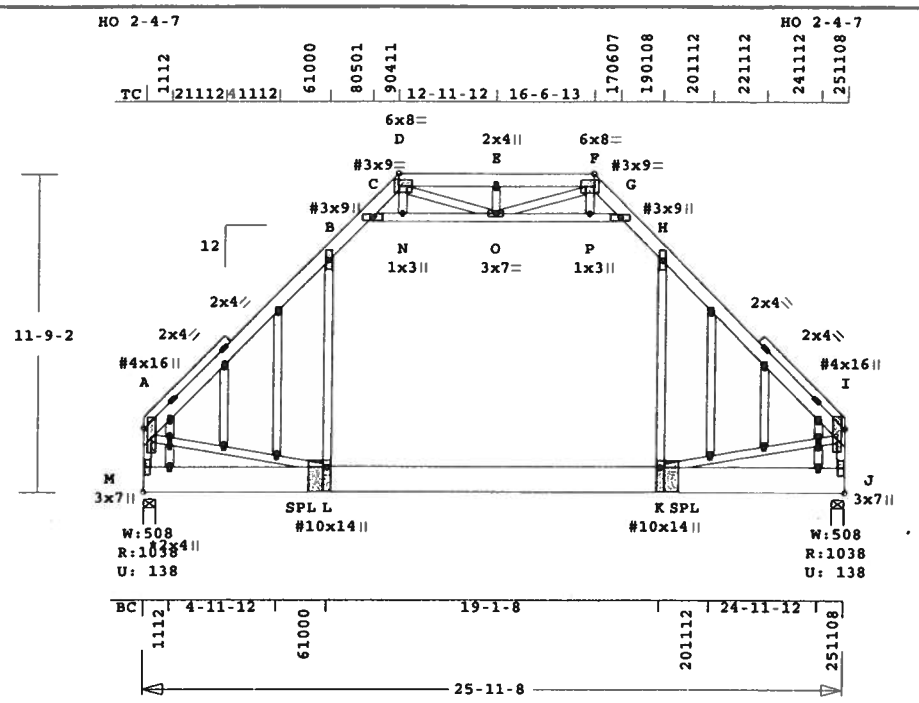
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D4	1	ATPB	250308	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ

Load Factors = 1.00 and 0.00
Max comp. force 2091 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D5	1	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See * For Typical Gable Plate Size and Placement

Scale: 0.140" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 395.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.35	2x 6	SP-#2
BC	0.15	2x12	SP-#2
WB	0.11	2x 4	SP-#2
ACT	0.05	2x 4	SP-#2
AWT	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	25-11- 8
BC Cont.	0- 0- 0	25-11- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
M	1038	138	5- 8	1- 8
			Hz =	-284
J	1038	138	5- 8	1- 8
			Hz =	285

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -B	0.28	863	C 0.00	0.28
B -C	0.35	668	C 0.00	0.35
C -D	0.35	142	C 0.00	0.35
D -E	0.04	239	C 0.00	0.04
E -F	0.04	239	C 0.00	0.04
F -G	0.35	142	C 0.00	0.35
G -H	0.35	668	C 0.00	0.35
H -I	0.28	863	C 0.00	0.28
Bottom Chords				
M -L	0.14	272	T 0.00	0.14
L -K	0.15	614	T 0.01	0.14
K -J	0.14	272	T 0.00	0.14
Webs				
M -A	0.09	974	C WindLd	
A -L	0.11	634	T	
L -B	0.06	298	T	
K -H	0.06	298	T	
K -I	0.11	634	T	
J -I	0.09	974	C WindLd	
Attic Chords (Top)				
C -N	0.05	680	C 0.05	0.00
N -O	0.04	681	C 0.04	0.00
O -P	0.04	681	C 0.04	0.00
P -G	0.05	680	C 0.05	0.00
Attic Webs (Top)				
N -D	0.00	17	T	
D -O	0.05	321	T	
O -E	0.02	212	C	
O -F	0.05	321	T	
P -F	0.00	17	T	

TL Defl -0.09" in L -K L/999
LL Defl -0.04" in L -K L/999
Shear // Grain in C -D 0.21

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A#	LOCK	4.0x16.0	Ctr	1.4	0.58
B#	LOCK	3.0x 9.0	Ctr	0.3	0.26
C#	LOCK	3.0x 9.0	Ctr	Ctr	0.26
D	LOCK	6.0x 8.0	Ctr	Ctr	0.72
E	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	6.0x 8.0	Ctr	Ctr	0.72
G#	LOCK	3.0x 9.0	Ctr	Ctr	0.26
H#	LOCK	3.0x 9.0	Ctr	0.3	0.26
I#	LOCK	4.0x16.0	0.1	2.2	0.56
M	LOCK	3.0x 7.0	Ctr	Ctr	0.77
L#	LOCK	10.0x14.0	-1.5	1.8	0.41
K#	LOCK	10.0x14.0	1.5	1.8	0.41
J	LOCK	3.0x 7.0	Ctr	Ctr	0.77
N	LOCK	1.0x 3.0	Ctr	Ctr	0.81
O	LOCK	3.0x 7.0	Ctr	Ctr	0.77
P	LOCK	1.0x 3.0	Ctr	Ctr	0.81

= Plate Monitor used
8 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

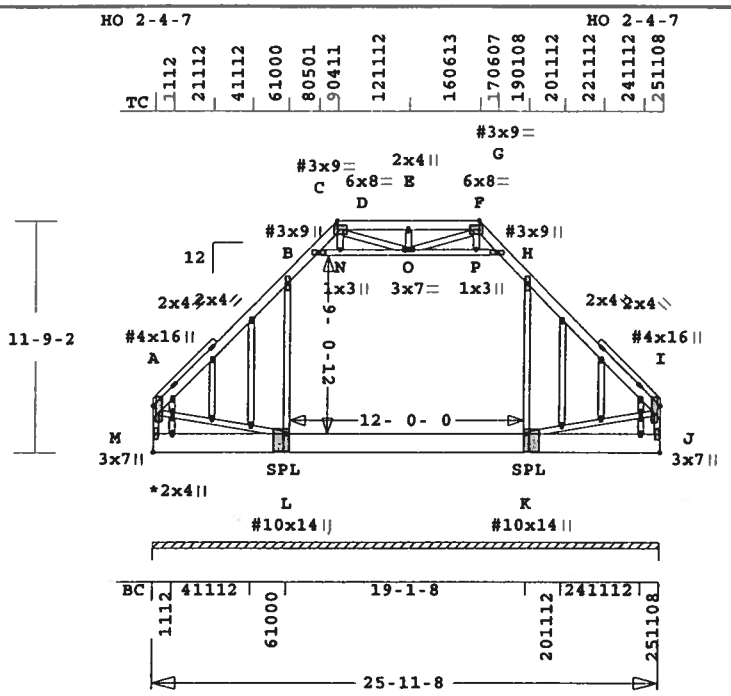
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 2- 0- 0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 974 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Scaled: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D6	1	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See * For Typical Gable Plate Size and Placement

Scale: 0.102" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 395.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 6	SP-#2
BC	0.37	2x12	SP-#2
WB	0.38	2x 4	SP-#2
ACT	0.04	2x 4	SP-#2
AWT	0.07	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	25-11- 8
BC Cont.	0- 0- 0	25-11- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.00
Plate Duration Factor			1.00

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	40	20	0.0'	26.0'
BC V	0	20	0.0'	26.0'
TC V	0	10	7.0'	8.3'
TC V	0	10	17.7'	19.0'
BC V	80	10	7.0'	19.0'
MA V	0	10	8.4'	17.5'
MA V	0	10	0.5'	8.6'
MA V	0	10	0.5'	8.6'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
Cont. Brg	0- 0- 0 to 25-11- 8			
	3274	113	Hz =	285

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.29	278 C	0.00 0.29
B -C	0.29	417 C	0.00 0.29
C -D	0.05	419 C	0.00 0.05
D -E	0.07	563 C	0.00 0.07
E -F	0.07	563 C	0.00 0.07
F -G	0.05	419 C	0.00 0.05
G -H	0.29	417 C	0.00 0.29
H -I	0.29	278 C	0.00 0.29
--------Bottom Chords---------			
M -L	0.36	0 T	0.00 0.36
L -K	0.37	0 T	0.00 0.37
K -J	0.36	0 T	0.00 0.36
------------Webs------------			
M -A	0.05	427 C	WindLd
A -L	0.05	269 T	
L -B	0.38	473 C	
K -H	0.38	473 C	
K -I	0.05	269 T	
J -I	0.05	427 C	WindLd
------Attic Chords (Top)------			
C -N	0.04	87 T	0.01 0.03
N -O	0.04	84 T	0.01 0.03
O -P	0.04	84 T	0.01 0.03
P -G	0.04	87 T	0.01 0.03
-------Attic Webs (Top)-------			
N -D	0.01	43 T	
D -O	0.07	304 T	
O -E	0.01	163 C	
O -F	0.07	304 T	
P -F	0.01	43 T	

TL Defl -0.09" in L -K L/999
LL Defl -0.07" in L -K L/999
Shear // Grain in L -K 0.33

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A# LOCK	4.0x16.0	Ctr	2.2	0.58
B# LOCK	3.0x 9.0	Ctr	0.3	0.26
C# LOCK	3.0x 9.0	Ctr	Ctr	0.26
D LOCK	6.0x 8.0	Ctr	Ctr	0.72
E LOCK	2.0x 4.0	Ctr	Ctr	0.40
F LOCK	6.0x 8.0	Ctr	Ctr	0.72
G# LOCK	3.0x 9.0	Ctr	Ctr	0.26
H# LOCK	3.0x 9.0	Ctr	0.3	0.26
I# LOCK	4.0x16.0	0.1	2.0	0.56
M LOCK	3.0x 7.0	Ctr	Ctr	0.77
L# LOCK	10.0x14.0	-1.5	1.8	0.41
K# LOCK	10.0x14.0	1.5	1.8	0.41
J LOCK	3.0x 7.0	Ctr	Ctr	0.77
N LOCK	1.0x 3.0	Ctr	Ctr	0.81
O LOCK	3.0x 7.0	Ctr	Ctr	0.77
P LOCK	1.0x 3.0	Ctr	Ctr	0.81

= Plate Monitor used
8 Gable studs to be attached with 2.0x4.0 plates each end.

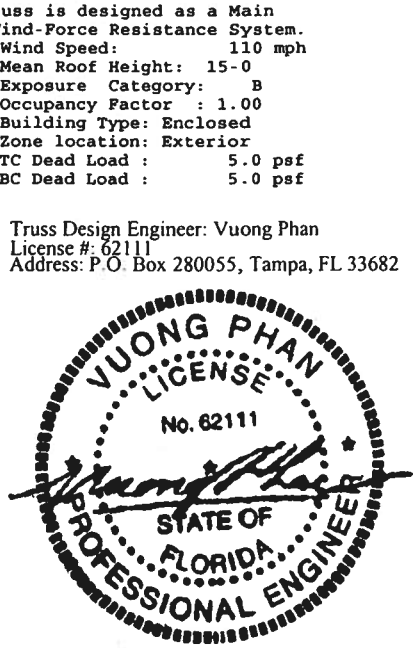
REVIEWED BY:

Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Refer to Gen Det 3 series for web bracing and plating.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

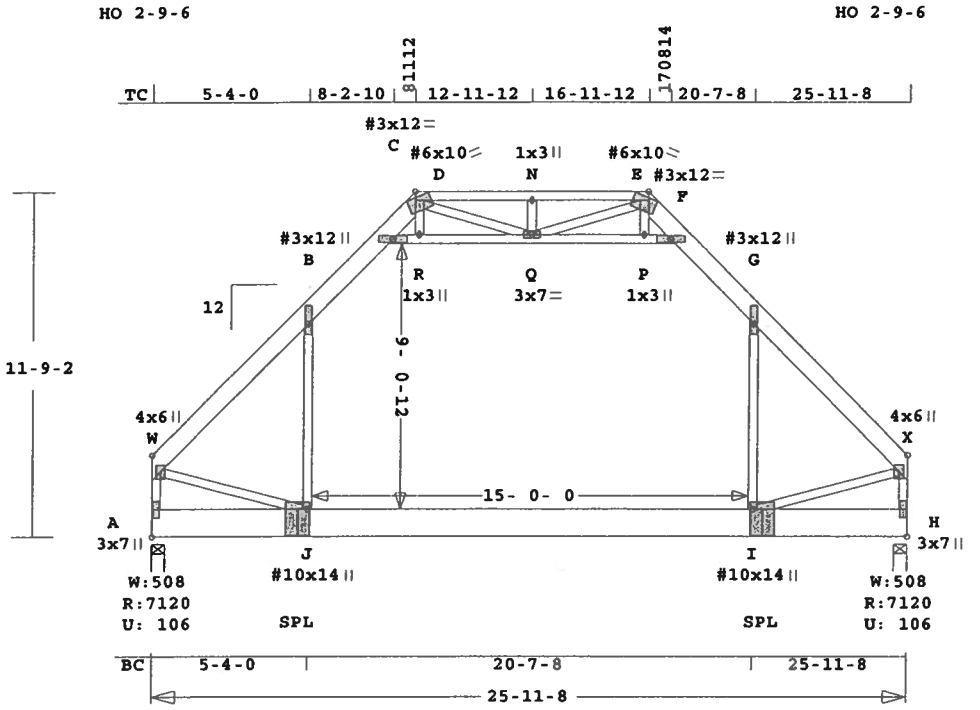
Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	D6	1	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ

Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 563 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	DG1	1*3P	ATPB	' 251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.152" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 350.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.74	2x 8	SP-#2
EX D -E		2x 4	SP-#2
BC	0.84	2x12	SP-#2
WB	0.33	2x 4	SP-#2
ACT	0.19	2x 4	SP-#2
AWT	0.13	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	2- 0- 0	0- 0- 0	8-11-12
TC	2- 0- 0	8-11-12	16-11-12
TC	2- 0- 0	16-11-12	25-11- 8
BC	2- 0- 0	0- 0- 0	25-11- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			72.0"

Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 NonStandard Load
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	120	60	0.0'	26.0'
BC V	0	60	0.0'	26.0'
TC V	0	30	5.5'	8.1'
TC V	0	30	17.9'	20.5'
BC V	240	30	5.5'	20.5'
TC V	0	120	0.0'	
	0	0		5.3'
BC V	240	30	0.0'	5.3'
BC V	240	30	20.6'	26.0'
TC V	0	0	20.6'	
	0	120		26.0'
MA V	0	30	8.2'	17.7'
MA V	0	30	0.5'	7.3'
MA V	0	30	0.5'	7.3'

Load Case # 2 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	120	60	0.0'	26.0'
BC V	0	60	0.0'	26.0'
TC V	0	30	5.5'	8.1'
TC V	0	30	17.9'	20.5'
BC V	240	30	5.5'	20.5'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	7121	106	5- 8	2-13
			Hz =	-847
H	7121	106	5- 8	2-13
			Hz =	848

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
W -B	0.73	6143 C	0.01 0.72
B -C	0.73	3783 C	0.00 0.73
C -D	0.74	1021 T	0.01 0.73
D -N	0.30	954 T	0.00 0.30
N -E	0.30	954 T	0.00 0.30
E -F	0.74	1021 T	0.01 0.73
F -G	0.73	3783 C	0.00 0.73
G -X	0.73	6143 C	0.01 0.72
Bottom Chords			
A -J	0.56	801 T	0.00 0.56
J -I	0.84	4009 T	0.13 0.71
I -H	0.56	801 T	0.00 0.56
Webs			
A -W	0.26	6910 C	WindLd
W -J	0.33	4305 T	
J -B	0.18	2914 T	
I -G	0.18	2913 T	
I -X	0.33	4305 T	
H -X	0.26	6910 C	WindLd
Attic Chords (Top)			
C -R	0.19	5649 C	0.19 0.00
R -Q	0.10	5658 C	0.03 0.07
Q -P	0.10	5658 C	0.03 0.07
P -F	0.19	5649 C	0.19 0.00
Attic Webs (Top)			
R -D	0.01	157 T	
D -Q	0.13	1730 T	
Q -N	0.03	961 C	
Q -E	0.13	1730 T	
P -E	0.01	157 T	

TL Defl -0.44" in J -I L/685
LL Defl -0.32" in J -I L/924
Shear // Grain in B -C 0.54

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
W	LOCK	4.0x 6.0	Ctr	Ctr	0.90
B#	LOCK	3.0x12.0	Ctr	1.6	0.26
C#	LOCK	3.0x12.0	-0.1	Ctr	0.42
D#	LOCK	6.0x10.0	0.2	-1.3	0.57
N	LOCK	1.0x 3.0	Ctr	Ctr	0.77
E#	LOCK	6.0x10.0	-0.2	-1.3	0.57
F#	LOCK	3.0x12.0	0.1	Ctr	0.42
G#	LOCK	3.0x12.0	Ctr	1.6	0.26
X	LOCK	4.0x 6.0	Ctr	Ctr	0.90
A	LOCK	3.0x 7.0	Ctr	Ctr	0.77
J#	LOCK	10.0x14.0	Ctr	1.8	0.42
I#	LOCK	10.0x14.0	Ctr	1.8	0.42
H	LOCK	3.0x 7.0	Ctr	Ctr	0.77
R	LOCK	1.0x 3.0	Ctr	Ctr	0.77
Q	LOCK	3.0x 7.0	Ctr	Ctr	0.77
P	LOCK	1.0x 3.0	Ctr	Ctr	0.77

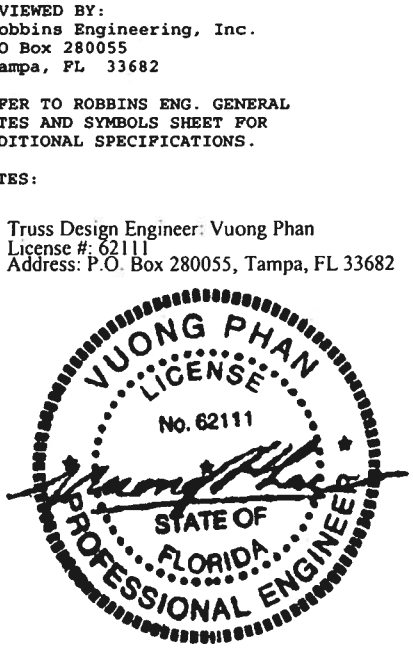
= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	DG1	1*3P	ATPB	251108	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ

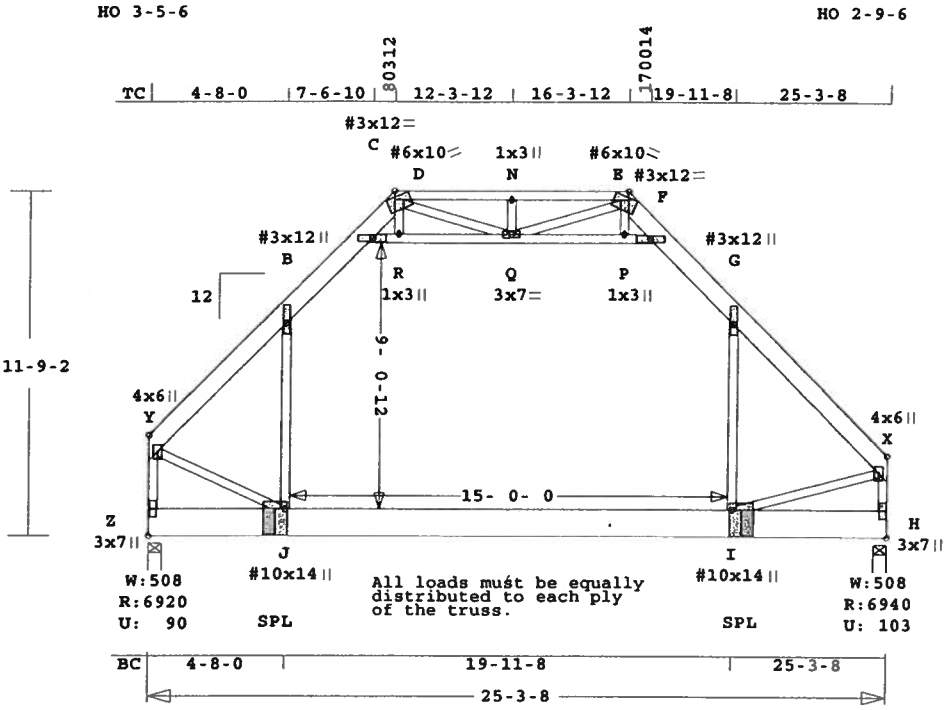
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	Rows	Nails (In)	Screws (In)	Bolts (In)
TC	1	12	20	0
BC	3	12	24	0
WB	1	8	8	

No bolts in 2x4s or smaller.
Provide connection to bearing for 848 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 6910 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	DG2	1*3P	ATPB	250308	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.152" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 343.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 3-Ply Truss *
****

	CSI	Size	Lumber
TC	0.70	2x 8	SP-#2
EX D -E		2x 4	SP-#2
BC	0.81	2x12	SP-#2
WB	0.33	2x 4	SP-#2
ACT	0.18	2x 4	SP-#2
AWT	0.13	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	2- 0- 0	0- 0- 0	8- 3-12
TC	2- 0- 0	8- 3-12	16- 3-12
TC	2- 0- 0	16- 3-12	25- 3- 8
BC	2- 0- 0	0- 0- 0	25- 3- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			72.0"

Lumber Duration Factor 1.00
Plate Duration Factor 1.00
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 1 NonStandard Load
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	120	60	0.0'	25.3'
BC V	0	60	0.0'	25.3'
TC V	0	30	4.8'	7.4'
TC V	0	30	17.2'	19.8'
BC V	240	30	4.8'	19.8'
TC V	0	120	0.0'	
	0	0		4.7'
BC V	240	30	0.0'	4.7'
TC V	0	0	20.0'	
	0	120		25.3'
BC V	240	30	20.0'	25.3'
MA V	0	30	7.6'	17.1'
MA V	0	30	0.5'	7.3'
MA V	0	30	0.5'	7.3'

Load Case # 2 Attic Loading
Lumber Duration Factor 1.00
Plate Duration Factor 1.00

plf -	Live	Dead	From	To
TC V	120	60	0.0'	25.3'
BC V	0	60	0.0'	25.3'
TC V	0	30	4.8'	7.4'
TC V	0	30	17.2'	19.8'
BC V	240	30	4.8'	19.8'
MA V	0	30	7.6'	17.1'
MA V	0	30	0.5'	7.3'
MA V	0	30	0.5'	7.3'

Plus 6 Wind Load Case(s)
Plus 2 Unbalanced Load Cases
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
Z	6921	91	5- 8	2-12
			Hz =	-867
H	6941	103	5- 8	2-12
			Hz =	842

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
Y -B	0.63	5773	C	0.01	0.62
B -C	0.67	3614	C	0.00	0.67
C -D	0.67	900	T	0.00	0.67
D -N	0.27	806	T	0.00	0.27
N -E	0.27	806	T	0.00	0.27
E -F	0.63	866	T	0.00	0.63
F -G	0.68	3586	C	0.00	0.68
G -X	0.70	5762	C	0.01	0.69
Bottom Chords					
Z -J	0.67	805	T	0.00	0.67
J -I	0.81	3751	T	0.12	0.69
I -H	0.47	794	T	0.00	0.47
Webs					
Z -Y	0.28	7234	C	WindLd	
Y -J	0.33	4368	T		
J -B	0.17	2791	T		
I -G	0.16	2739	T		
I -X	0.31	4028	T		
H -X	0.25	6528	C	WindLd	
Attic Chords (Top)					
C -R	0.18	5161	C	0.18	0.00
R -Q	0.10	5170	C	0.03	0.07
Q -P	0.08	5046	C	0.03	0.05
P -F	0.17	5037	C	0.17	0.00
Attic Webs (Top)					
R -D	0.01	159	T		
D -Q	0.13	1726	T		
Q -N	0.03	920	C		
Q -E	0.12	1631	T		
P -E	0.01	158	T		

TL Defl -0.41" in J -I L/712
LL Defl -0.31" in J -I L/953
Shear // Grain in B -C 0.49

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.

BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
Y	LOCK	4.0x 6.0	Ctr	Ctr	0.94
B#	LOCK	3.0x12.0	Ctr	1.6	0.25
C#	LOCK	3.0x12.0	-0.1	Ctr	0.38
D#	LOCK	6.0x10.0	0.2	-1.2	0.55
N	LOCK	1.0x 3.0	Ctr	Ctr	0.76
E#	LOCK	6.0x10.0	-0.2	-1.2	0.55
F#	LOCK	3.0x12.0	0.1	Ctr	0.38
G#	LOCK	3.0x12.0	Ctr	1.6	0.25
X	LOCK	4.0x 6.0	Ctr	Ctr	0.85
Z	LOCK	3.0x 7.0	Ctr	Ctr	0.76
J#	LOCK	10.0x14.0	Ctr	1.8	0.42
I#	LOCK	10.0x14.0	Ctr	1.8	0.42
H	LOCK	3.0x 7.0	Ctr	Ctr	0.76
R	LOCK	1.0x 3.0	Ctr	Ctr	0.76
Q	LOCK	3.0x 7.0	Ctr	Ctr	0.76
P	LOCK	1.0x 3.0	Ctr	Ctr	0.76

= Plate Monitor used

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	DG2	1*3P	ATPB	250308	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ

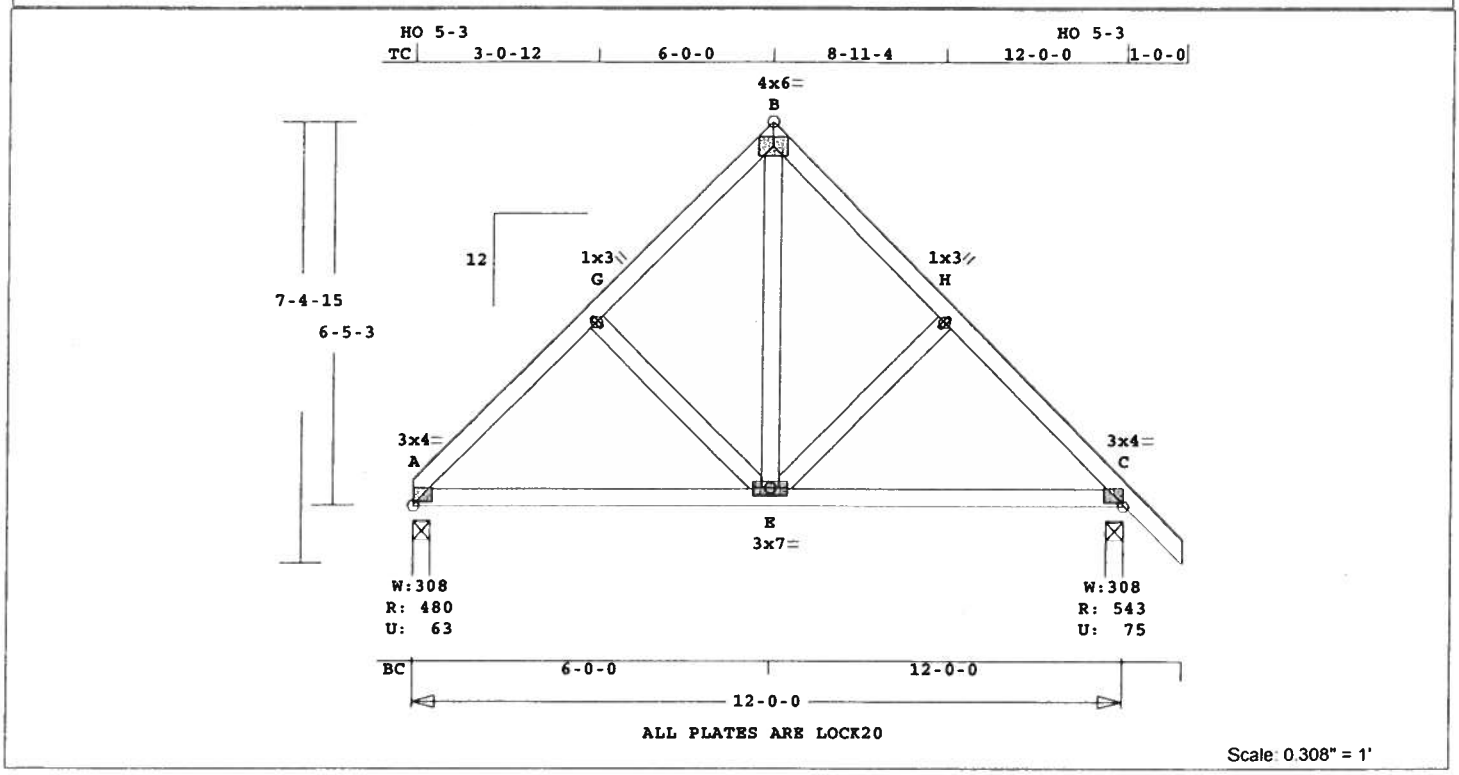
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	1	12	20	0
BC	3	12	24	0
WB	1	8	8	

No bolts in 2x4s or smaller.
Provide connection to bearing for 868 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
NOTE: USER MODIFIED PLATES
This design may have plates selected through a plate monitor.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Unbalanced Loads Checked
Load Factors = 1.00 and 0.00
Max comp. force 7234 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	E1	5	SP	120000	12	0	1- 0- 0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 84.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.23	2x 4	SP-#2
WB	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 0- 0
BC Cont.	0- 0- 0	12- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	480	64	3- 8	1- 8
			Hz =	-135
C	544	75	3- 8	1- 8
			Hz =	135

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -G	0.07	464 C	0.00 0.07
G -B	0.07	344 C	0.00 0.07
B -H	0.07	344 C	0.00 0.07
H -C	0.07	464 C	0.00 0.07
--------Bottom Chords---------			
A -E	0.23	332 T	0.03 0.20
E -C	0.23	332 T	0.03 0.20
------------Webs------------			
G -E	0.03	129 C	
E -B	0.05	328 T	
E -H	0.03	129 C	

TL Defl -0.04" in A -E L/999
LL Defl -0.02" in A -E L/999
Shear // Grain in A -E 0.13

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	0.4	0.3	0.58
G	LOCK	1.0x 3.0	Ctr	Ctr	0.75
B	LOCK	4.0x 6.0	Ctr	-0.1	0.41
H	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	3.0x 4.0	-0.4	0.3	0.58
E	LOCK	3.0x 7.0	Ctr	Ctr	0.37

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

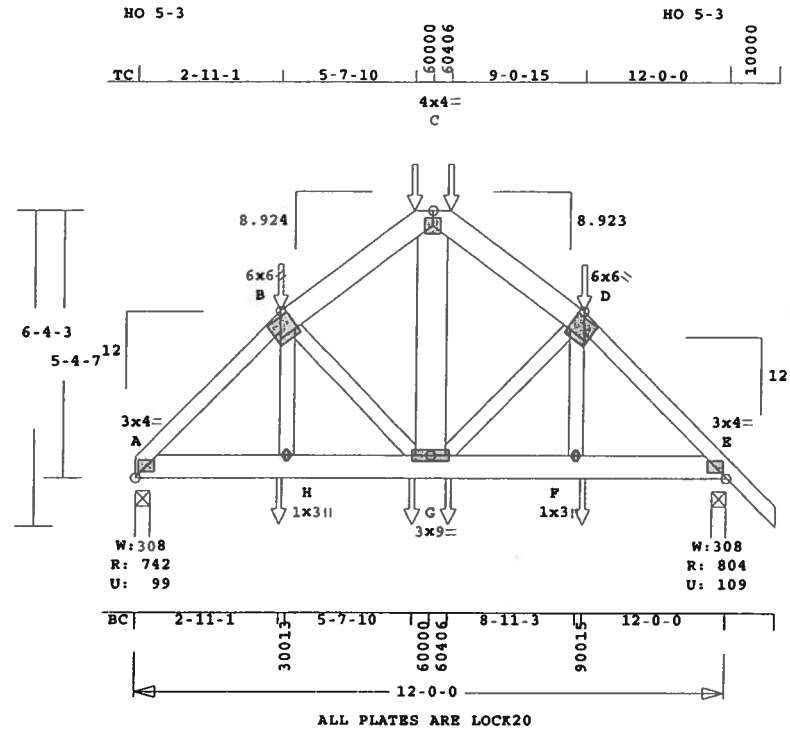
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 464 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	E2	1	SP	120000	12	0	1- 0- 0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0 257" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 116.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

CSI	-Size-	----Lumber----
TC 0.08	2x 4	SP-#2
EX B -C	2x 6	SP-#2
EX C -D	2x 6	SP-#2
BC 0.12	2x 6	SP-#2
WB 0.02	2x 4	SP-#2
EX G -C	2x 8	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	12- 0- 0
BC Cont.	0- 0- 0	12- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	12.0'
BC V	0	20	0.0'	12.0'
TC V	29	15	5.6'	6.4'
BC V	0	15	5.6'	6.4'
BC V	30	30	2.9'	CL-LB
TC V	41	41	2.9'	CL-LB
BC V	30	30	9.1'	CL-LB
TC V	41	41	9.1'	CL-LB
BC V	21	21	5.6'	CL-LB
TC V	30	30	5.6'	CL-LB
BC V	21	21	6.4'	CL-LB
TC V	30	30	6.4'	CL-LB

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React	Uplft	Size Req'd	Lbs	Lbs	In-Sx	In-Sx
A	742	99				3- 8	1- 8
						Hz =	-110
E	805	110				3- 8	1- 8
						Hz =	111

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -B	0.07	827 C	0.00		0.07
B -C	0.05	604 C	0.00		0.05
C -D	0.05	603 C	0.00		0.05
D -E	0.08	825 C	0.00		0.08
Bottom Chords					
A -H	0.11	589 T	0.07		0.04
H -G	0.12	587 T	0.07		0.05
G -F	0.12	587 T	0.07		0.05
F -E	0.11	589 T	0.07		0.04
Webs					
H -B	0.01	115 T			
B -G	0.02	127 C			
G -C	0.04	405 T			
G -D	0.02	127 C			
F -D	0.01	112 T			

TL Defl -0.02" in G -F L/999
LL Defl -0.01" in G -F L/999
Shear // Grain in H -G 0.09

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.60
B	LOCK	6.0x 6.0	0.8	0.6	0.56
C	LOCK	4.0x 4.0	Ctr	Ctr	0.65
D	LOCK	6.0x 6.0	-0.8	0.6	0.56
E	LOCK	3.0x 4.0	Ctr	Ctr	0.61
H	LOCK	1.0x 3.0	Ctr	Ctr	0.75
G	LOCK	3.0x 9.0	Ctr	Ctr	0.51
F	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

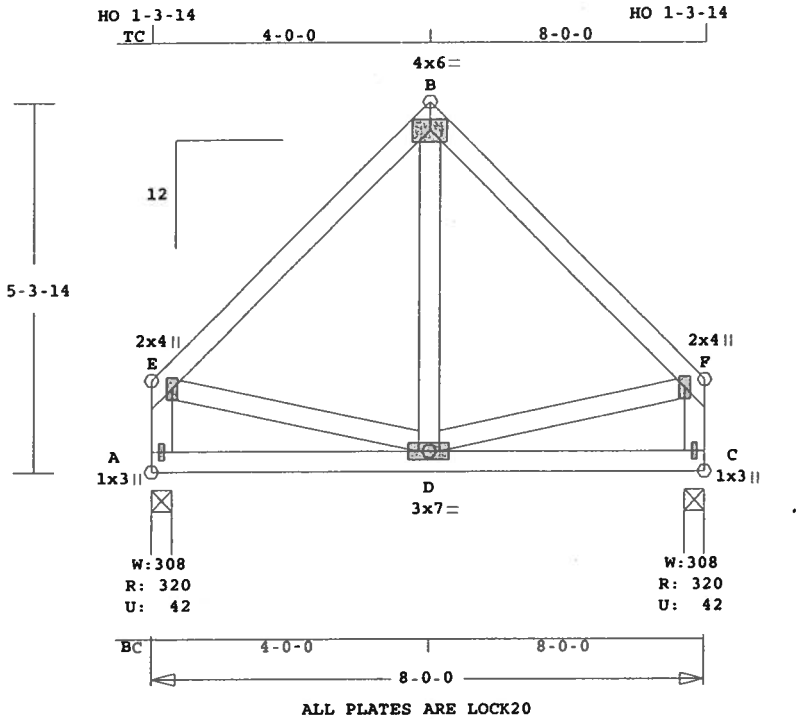
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 827 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	G1	4	TR	80000	12	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 63.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.14	2x 4	SP-#2
BC	0.10	2x 4	SP-#2
WB	0.02	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 0- 0
BC Cont.	0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	320	43	3- 8	1- 8
			Hz =	-122
C	320	43	3- 8	1- 8
			Hz =	123

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
E -B	0.14	188	C	0.00	0.14
B -F	0.14	188	C	0.00	0.14
--------Bottom Chords---------					
A -D	0.10	114	T	0.00	0.10
D -C	0.10	114	T	0.00	0.10
------------Webs------------					
A -E	0.02	287	C	WindLd	
E -D	0.02	146	T		
D -B	0.01	127	T		
D -F	0.02	146	T		
C -F	0.02	287	C	WindLd	

TL Defl -0.01" in D -C L/999
LL Defl -0.01" in D -C L/999
Shear // Grain in E -B 0.11

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
E	LOCK	2.0x 4.0	Ctr	Ctr	0.76
B	LOCK	4.0x 6.0	Ctr	-0.1	0.36
F	LOCK	2.0x 4.0	Ctr	Ctr	0.76
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	3.0x 7.0	Ctr	Ctr	0.52
C	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

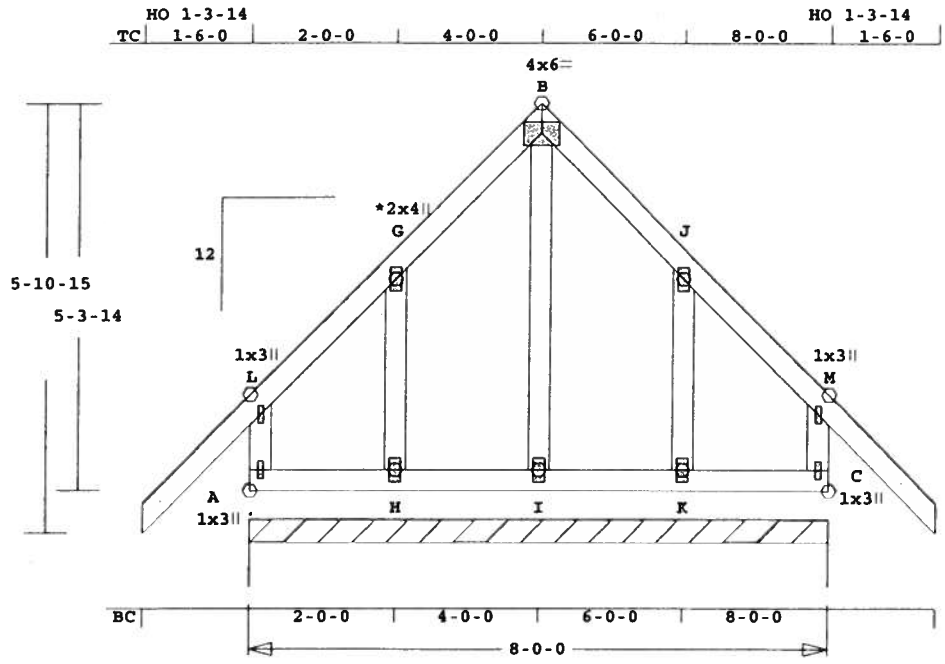
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 287 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	G2	1	TR	80000	12	1- 6- 0	1- 6- 0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20

See Joint G For Typical Gable Plate Size and Placement

Scale: 0.376" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 68.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.03	2x 4	SP-#2
BC	0.04	2x 4	SP-#2
WB	0.05	2x 4	SP-#2
GW	0.04	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8- 0- 0
BC	Cont.	0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 8- 0- 0			
	832	119	Hz =	120

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
L -G	0.03	52	T	0.00	0.03
G -B	0.03	125	T	0.00	0.03
B -J	0.03	125	T	0.00	0.03
J -M	0.03	52	T	0.00	0.03
--------Bottom Chords---------					
A -H	0.04	0	T	0.00	0.04
H -I	0.02	0	T	0.00	0.02
I -K	0.02	0	T	0.00	0.02
K -C	0.04	0	T	0.00	0.04
------------Webs------------					
A -L	0.05	75	C	0.00	0.05
C -M	0.05	75	C	0.00	0.05
----------Gable Webs----------					
H -G	0.03	124	C	0.01	0.02
I -B	0.04	134	C		
K -J	0.03	124	C	0.01	0.02

TL Defl 0.00" in K -C L/999
LL Defl 0.00" in K -C L/999
Shear // Grain in L -G 0.05

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

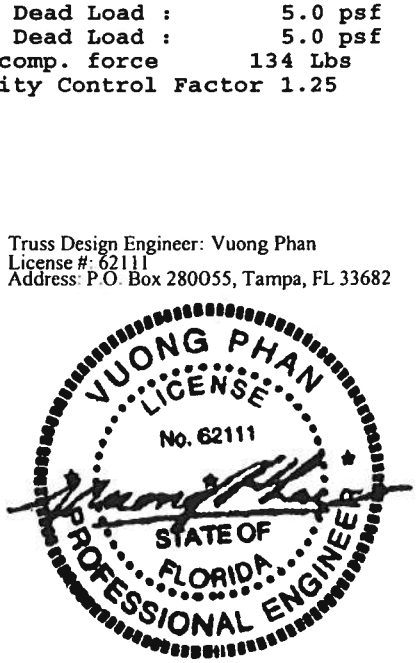
Jt	Type	Plt Size	X	Y	JSI
L	LOCK	1.0x 3.0	Ctr	Ctr	0.75
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	4.0x 6.0	Ctr	-0.1	0.36
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 134 Lbs
Quality Control Factor 1.25

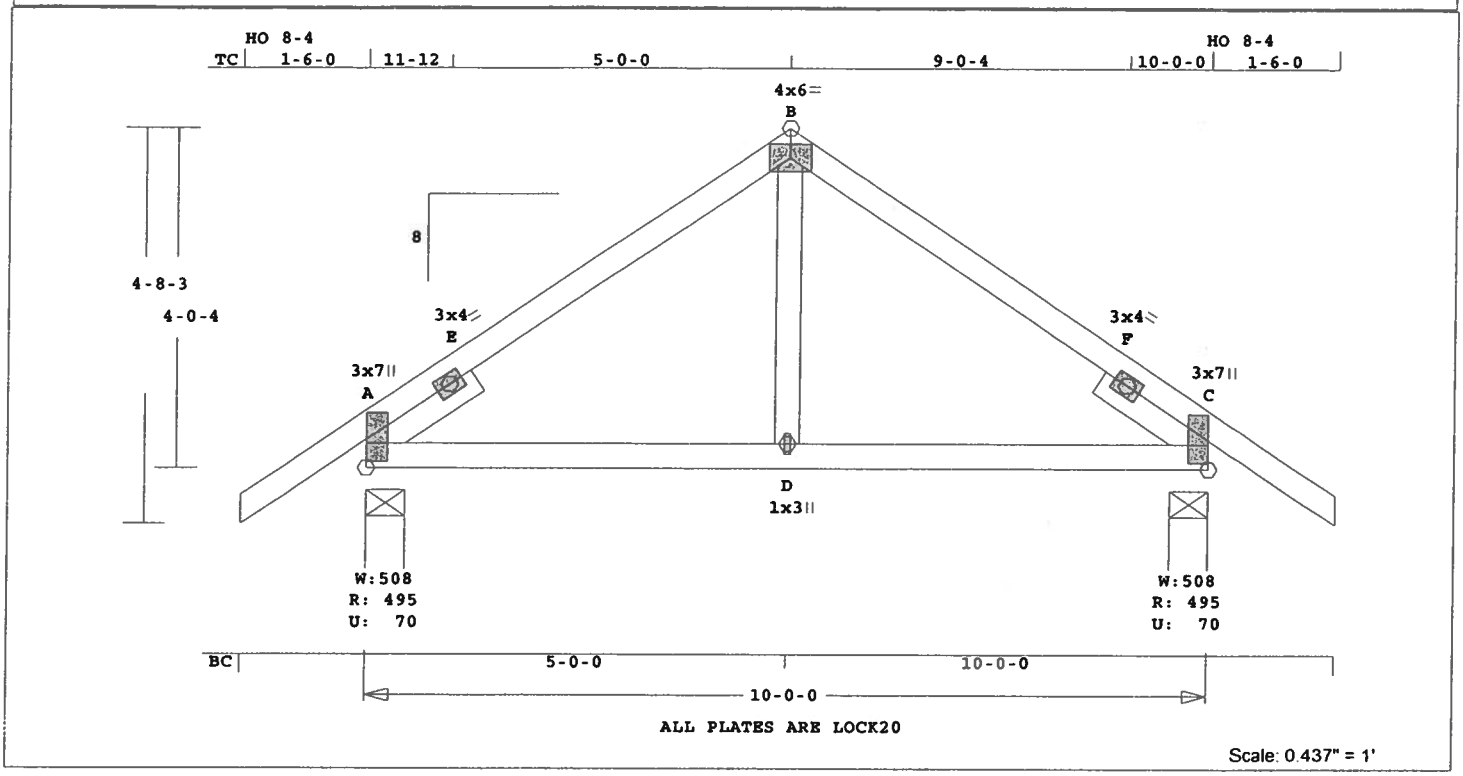
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	H1	2	TR	100000	8	1- 6- 0	1- 6- 0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.12	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.03	2x 4	SP-#2
SL	0.02	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	10- 0- 0
BC Cont.	0- 0- 0	10- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	496	70	5- 8	1- 8
			Hz =	-66
C	496	70	5- 8	1- 8
			Hz =	67

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -E	0.05	217	C	0.00	0.05
E -B	0.12	345	C	0.00	0.12
B -F	0.12	345	C	0.00	0.12
F -C	0.05	217	C	0.00	0.05
Bottom Chords					
A -D	0.16	291	T	0.03	0.13
D -C	0.16	291	T	0.03	0.13
Webs					
D -B	0.03	201	T		
Sliders					
A -E	0.02	273	C		
F -C	0.02	273	C		

TL Defl -0.02" in A -D L/999
LL Defl -0.01" in A -D L/999
Shear // Grain in E -B 0.13

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

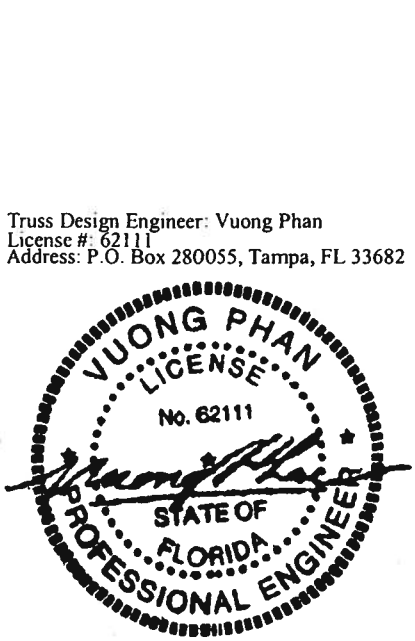
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 7.0	1.5	0.6	0.55
E	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	4.0x 6.0	Ctr	Ctr	0.40
F	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 7.0-1.5	0.6		0.55
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 345 Lbs
Quality Control Factor 1.25

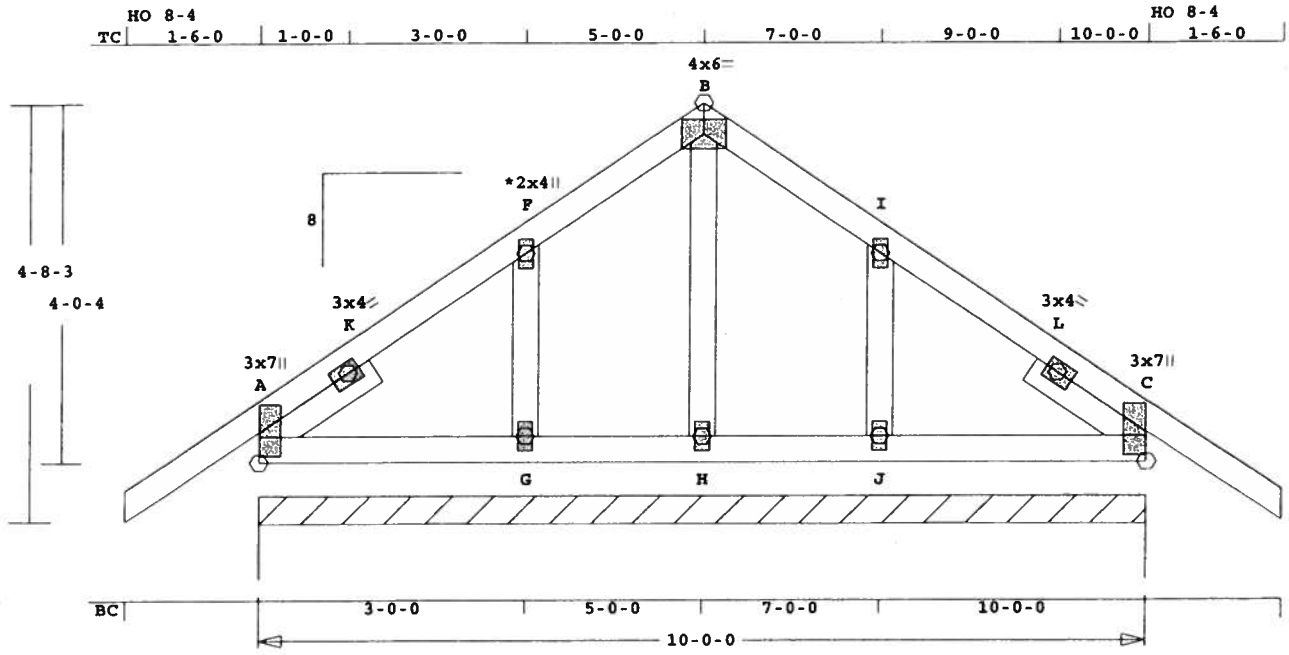
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	H2	1	TR	100000	8	1- 6- 0	1- 6- 0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20

See Joint F For Typical Gable Plate Size and Placement

Scale: 0.462" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 69.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	Size	Lumber
TC	0.05	2x 4	SP-#2
BC	0.03	2x 4	SP-#2
GW	0.01	2x 4	SP-#2
SL	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 0- 0
BC	Cont.	0- 0- 0	10- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 992	0- 0- 0 to 140 Hz =	10- 0- 0	67

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -K	0.01	84 C	0.00	0.01
K -F	0.05	118 C	0.00	0.05
F -B	0.05	133 C	0.00	0.05
B -I	0.05	133 C	0.00	0.05
I -L	0.05	118 C	0.00	0.05
L -C	0.01	84 C	0.00	0.01
Bottom Chords				
A -G	0.03	5 T	0.00	0.03
G -H	0.03	0 T	0.00	0.03
H -J	0.03	0 T	0.00	0.03
J -C	0.03	5 T	0.00	0.03
Gable Webs				
G -F	0.01	145 C		
H -B	0.01	84 C		
J -I	0.01	145 C		
Sliders				
A -K	0.00	86 C		
L -C	0.00	86 C		

TL Defl 0.00" in A -G L/999
LL Defl 0.00" in A -G L/999
Shear // Grain in K -F 0.08

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

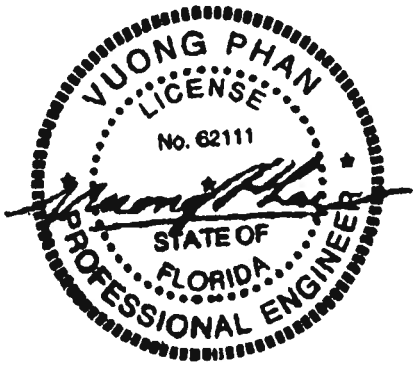
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 7.0	1.5	0.6	0.55
K	LOCK	3.0x 4.0	Ctr	Ctr	0.50
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	4.0x 6.0	Ctr	Ctr	0.40
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 7.0-1.5	0.6		0.55
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 145 Lbs
Quality Control Factor 1.25

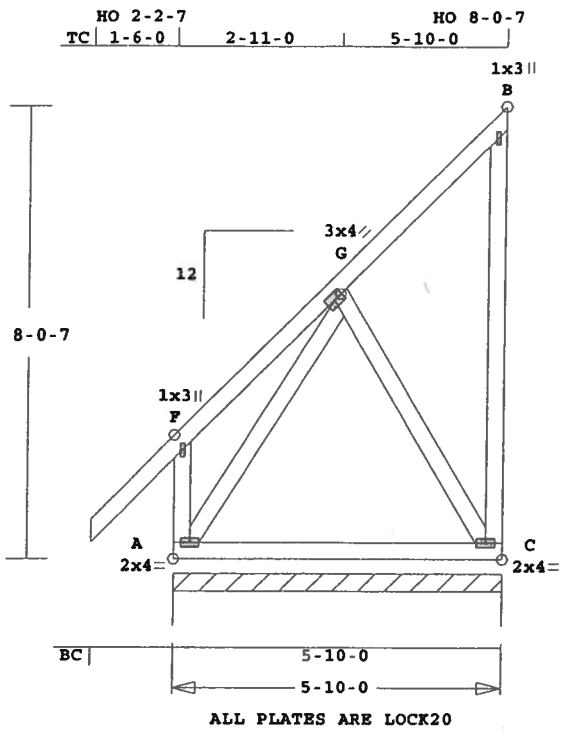
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	J4	4	MONO.DD	51000	12	1- 6- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0.292" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 67.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 4	SP-#2
BC	0.24	2x 4	SP-#2
WB	0.11	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	5-10- 0
BC	Cont.	0- 0- 0	5-10- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 5-10- 0			
	563	79	Hz = 266	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
F -G	0.10	81 T	0.01	0.09
G -B	0.09	113 C	0.00	0.09
--------Bottom Chords---------				
A -C	0.24	0 T	0.00	0.24

----------Webs----------

A -F	0.01	80 T	WindLd	
A -G	0.08	236 C		
G -C	0.04	149 T		
C -B	0.11	91 T	WindLd	

TL Defl	-0.10"	in A -C	L/712	
LL Defl	-0.05"	in A -C	L/999	
Shear // Grain		in A -C	0.12	

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
F	LOCK	1.0x 3.0	Ctr	Ctr	0.75
G	LOCK	3.0x 4.0	Ctr	Ctr	0.55
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	2.0x 4.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.75

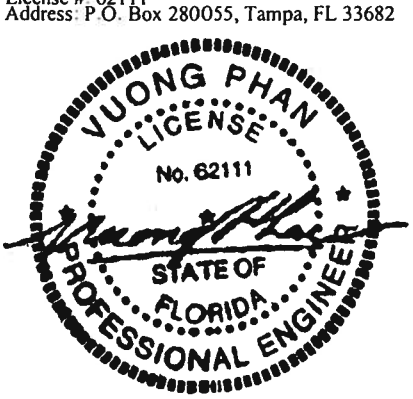
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 236 Lbs
Quality Control Factor 1.25

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

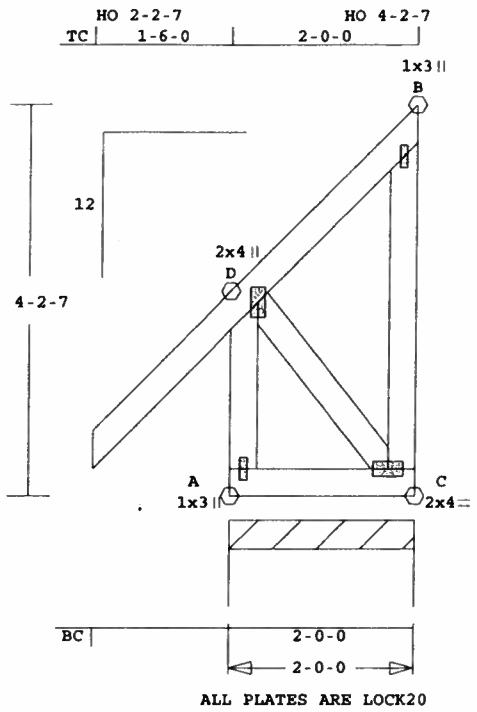
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	J5	5	MONO.DD	20000	12	1- 6- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20

Scale: 0.482" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 28.0 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.03	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
WB	0.02	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	2- 0- 0
BC Cont.	0- 0- 0	2- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 2- 0- 0			
	256	38 Hz = 129		

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.03	64 C	0.00 0.03
--------Bottom Chords---------			
A -C	0.02	0 T	0.00 0.02
------------Webs-------------			
A -D	0.01	131 C	WindLd
D -C	0.01	127 T	
C -B	0.02	60 C	WindLd

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.04

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	2.0x 4.0	Ctr	Ctr	0.75
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

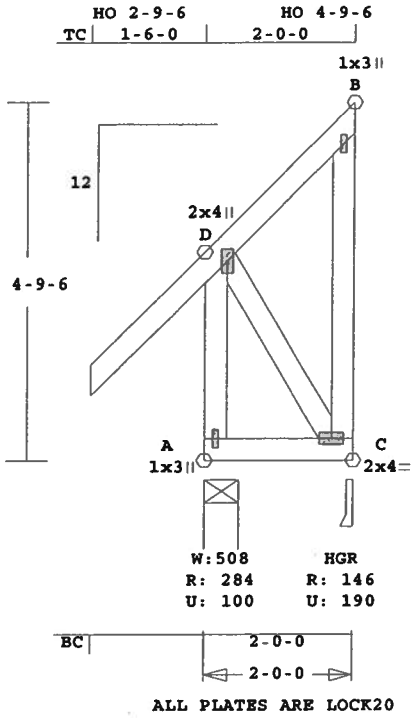
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 131 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	J6	12	MONO.DD	20000	12	1- 6- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0.387" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 31.1 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.03	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
WB	0.03	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	2- 0- 0
BC Cont.	0- 0- 0	2- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	285	101	5- 8	1- 8
			Hz =	-115
C	147	190	3- 8	1- 8
			Hz =	148

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.03	71 C	0.00 0.03
--------Bottom Chords---------			
A -C	0.02	96 T	0.00 0.02
------------Webs------------			
A -D	0.01	178 C	WindLd
D -C	0.02	167 T	
C -B	0.03	61 T	WindLd

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.04

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	2.0x 4.0	Ctr	Ctr	0.75
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

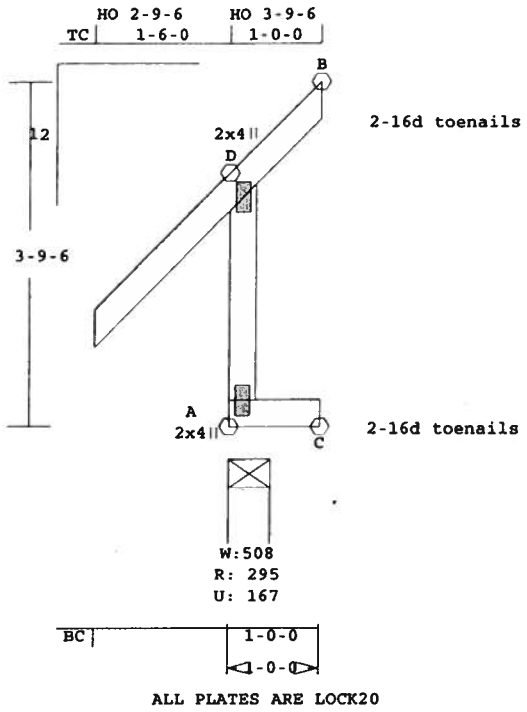
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 178 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	J7	4	MONO.DD	10000	12	1- 6- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0.471" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 13.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.08	2x 4	SP-#2
BC	0.12	2x 4	SP-#2
WB	0.13	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 0
BC Cont.	0- 0- 0	1- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	295	167	5- 8	1- 8
			Hz =	86
C	43	102	1- 8	1- 8
B	99	109	1- 8	1- 8
			Hz =	64

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.08	111 T	0.00 0.08
--------Bottom Chords---------			
A -C	0.12	0 T	0.00 0.12
----------Webs----------			
A -D	0.13	109 T	0.00 0.13

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -C 0.11

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

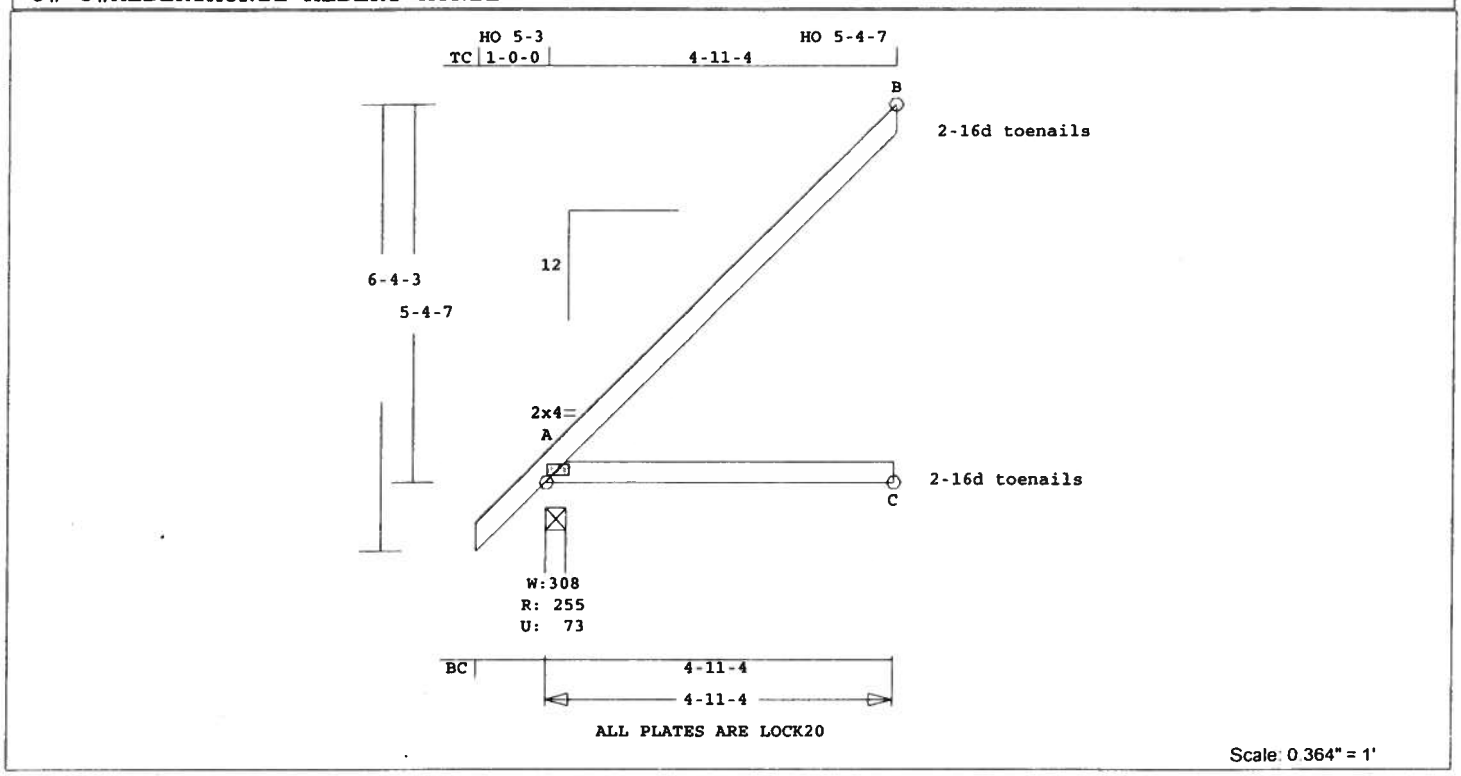
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 87 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	J9	1	JCA2	41104	12	1- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 25.7 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.18	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4-11- 4
BC Cont.	0- 0- 0	4-11- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	256	73	3- 8	1- 8
			Hz = 149	
C	92	0	1- 8	1- 8
B	126	104	1- 8	1- 8
			Hz = 101	

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.24	90 C	0.00 0.24
--------Bottom Chords---------			
A -C	0.18	0 T	0.00 0.18

TL Defl -0.05" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.14

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.4	0.3	0.58

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 90 Lbs
Quality Control Factor 1.25

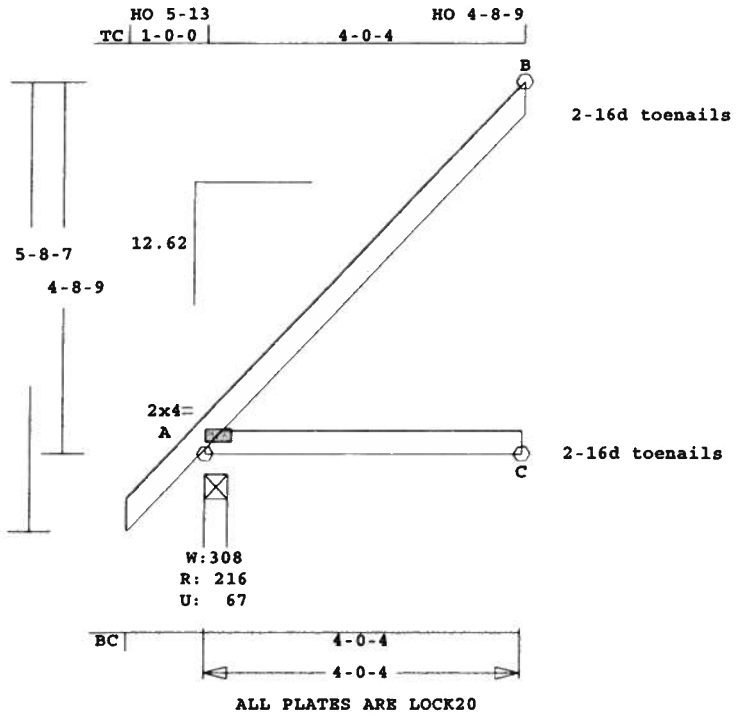
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

VUONG PHAN LICENSE No. 62111 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	J10	2	JCA2	40'0"4	12.62	1- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0.409" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 4	SP-#2
BC	0.12	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	4- 0- 4
BC Cont.	0- 0- 0	4- 0- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	216	67	3- 8	1- 8
			Hz =	128
C	75	2	1- 8	1- 8
B	103	91	1- 8	1- 8
			Hz =	87

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.16	79	C 0.00 0.16
--------Bottom Chords---------			
A -C	0.12	0	T 0.00 0.12

TL Defl -0.02" in A -C L/999
LL Defl -0.01" in A -C L/999
Shear // Grain in A -B 0.11

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.7	0.7	0.69

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

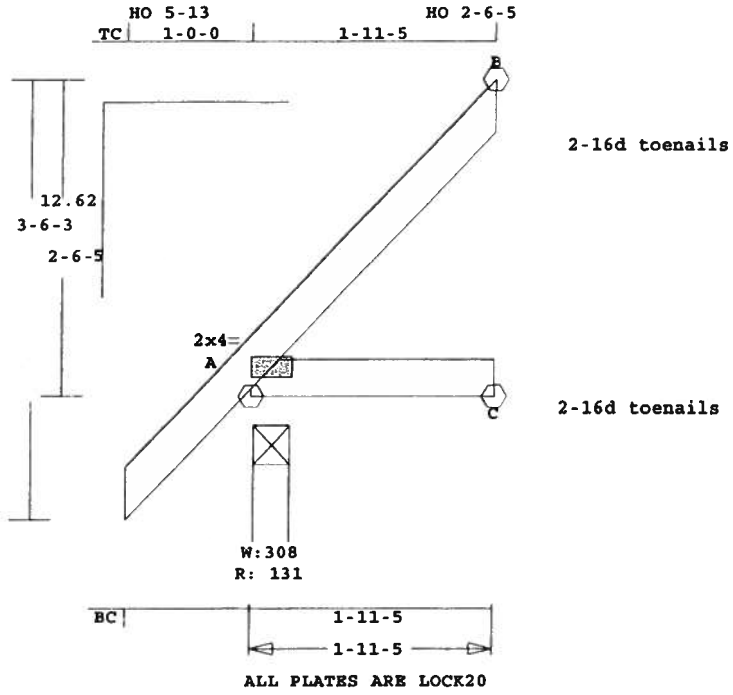
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 79 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	J11	2	JCA2	11105	12.62	1- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0.648" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 12.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.03	2x 4	SP-#2
BC	0.02	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1-11- 5
BC Cont.	0- 0- 0	1-11- 5

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	131	0	3- 8	1- 8
			Hz =	63
B	55	50	1- 8	1- 8
C	35	2	1- 8	1- 8
			Hz =	43

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.03	43 C	0.00 0.03
--------Bottom Chords---------			
A -C	0.02	42 T	0.00 0.02

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.04

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.7	0.7	0.69

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

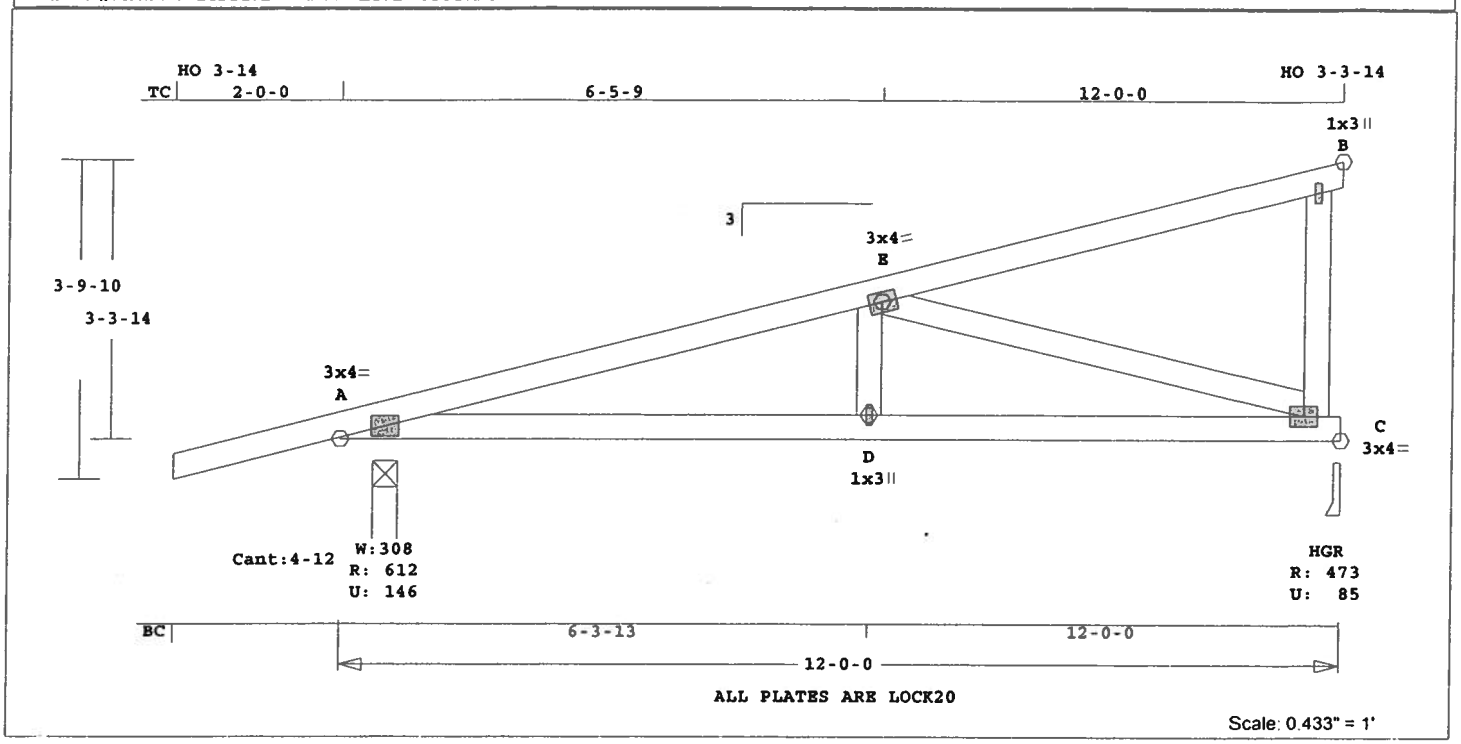
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 43 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M1	6	MONO	120000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 67.7 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.33	2x 4	SP-#2
BC	0.28	2x 4	SP-#2
WB	0.38	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 0- 0
BC	Cont.	0- 0- 0	12- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	613	146	3- 8	1- 8
			Hz =	45
C	473	85	3- 8	1- 8
			Hz =	105

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.33	965 C	0.00	0.33
E -B	0.33	34 C	0.00	0.33
--------Bottom Chords---------				
A -D	0.28	943 T	0.09	0.19
D -C	0.28	943 T	0.09	0.19
------------Webs------------				
D -E	0.03	255 T		
E -C	0.38	984 C		
C -B	0.01	136 C	WindLd	

TL Defl -0.07" in A -D L/999
LL Defl -0.03" in A -D L/999
Shear // Grain in E -B 0.24

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.73
E	LOCK	3.0x 4.0	Ctr	Ctr	0.53
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	3.0x 4.0	Ctr	Ctr	0.63

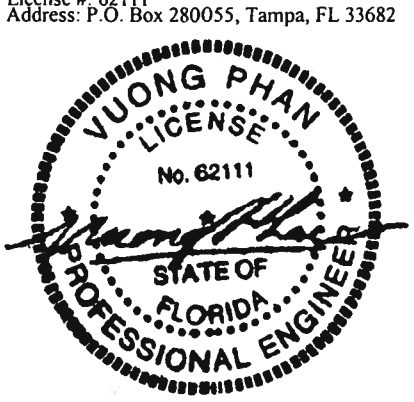
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 984 Lbs
Quality Control Factor 1.25

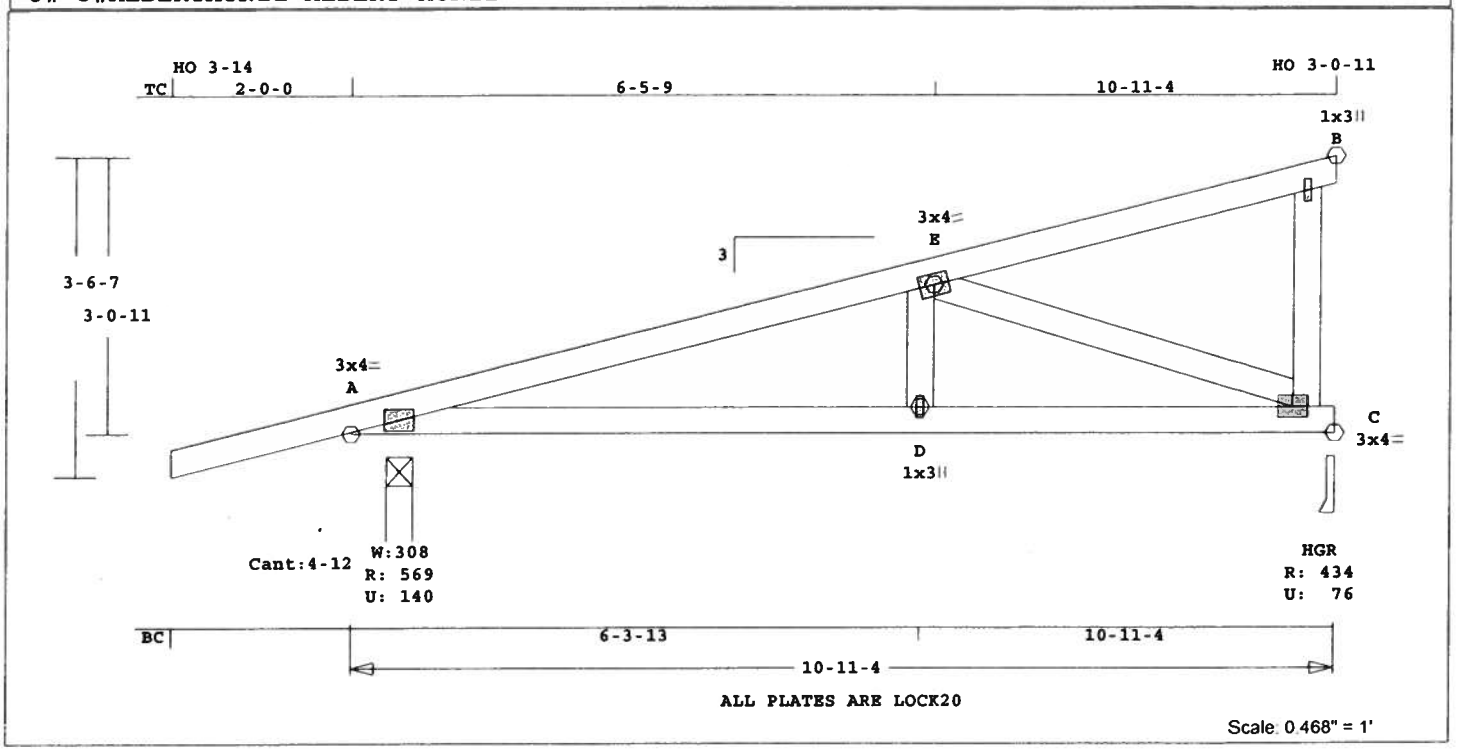
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M2	1	MONO	101104	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 61.1 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.28	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.21	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10-11- 4
BC	Cont.	0- 0- 0	10-11- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	569	140	3- 8	1- 8
			Hz =	41
C	434	77	3- 8	1- 8
			Hz =	95

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.28	784 C	0.00	0.28
E -B	0.28	29 C	0.00	0.28
--------Bottom Chords---------				
A -D	0.26	766 T	0.12	0.14
D -C	0.24	766 T	0.08	0.16
----------Webs----------				
D -E	0.03	232 T		
E -C	0.21	817 C		
C -B	0.01	102 C	WindLd	

TL Defl -0.07" in A -D L/999
LL Defl -0.03" in A -D L/999
Shear // Grain in A -E 0.23

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	LOCK	3.0x 4.0 Ctr	Ctr	0.71
E	LOCK	3.0x 4.0 Ctr	Ctr	0.44
B	LOCK	1.0x 3.0 Ctr	Ctr	0.75
D	LOCK	1.0x 3.0 Ctr	Ctr	0.75
C	LOCK	3.0x 4.0 Ctr	Ctr	0.57

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

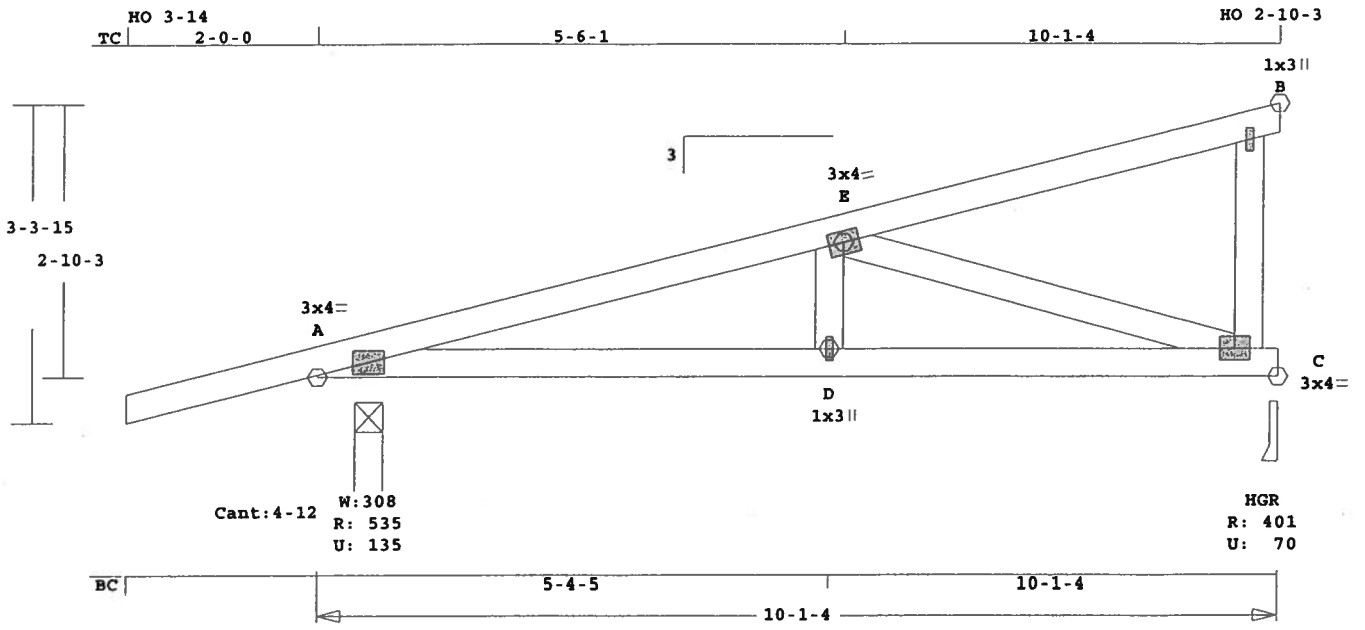
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 817 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M3	2	MONO	100104	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 57.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.20	2x 4	SP-#2
WB	0.21	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	10- 1- 4
BC	Cont.	0- 0- 0	10- 1- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	536	135	3- 8	1- 8
			Hz =	38
C	401	71	3- 8	1- 8
			Hz =	87

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.21	769 C	0.00	0.21
E -B	0.21	28 C	0.00	0.21
--------Bottom Chords---------				
A -D	0.20	751 T	0.12	0.08
D -C	0.19	751 T	0.07	0.12
-------------Webs-------------				
D -E	0.03	207 T		
E -C	0.21	786 C		
C -B	0.01	115 C	WindLd	

TL Defl -0.04" in A -D L/999
LL Defl -0.02" in A -D L/999
Shear // Grain in E -B 0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.69
E	LOCK	3.0x 4.0	Ctr	Ctr	0.42
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	3.0x 4.0	Ctr	Ctr	0.54

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

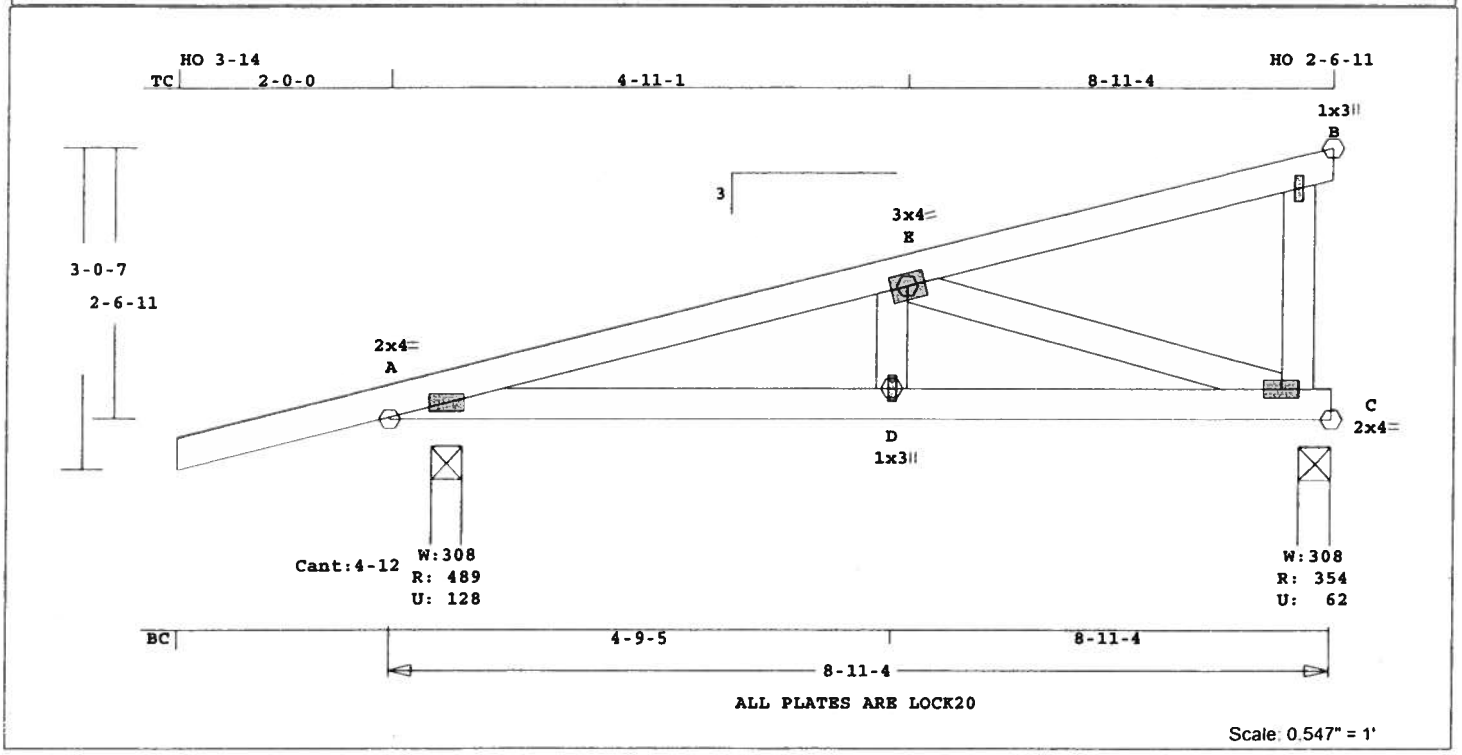
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 786 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M4	1	MONO	81104	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 50.7 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.15	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.13	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8-11- 4
BC	Cont.	0- 0- 0	8-11- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	489	128	3- 8	1- 8
			Hz =	34
C	354	63	3- 8	1- 8
			Hz =	77

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -E	0.15	650 C	0.00	0.15
E -B	0.15	24 C	0.00	0.15
--------Bottom Chords---------				
A -D	0.17	635 T	0.10	0.07
D -C	0.15	635 T	0.10	0.05
----------Webs----------				
D -E	0.02	178 T		
E -C	0.13	667 C		
C -B	0.01	102 C	WindLd	

TL Defl -0.03" in A -D L/999
LL Defl -0.01" in A -D L/999
Shear // Grain in E -B 0.17

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.89
E	LOCK	3.0x 4.0	Ctr	Ctr	0.40
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.78

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004

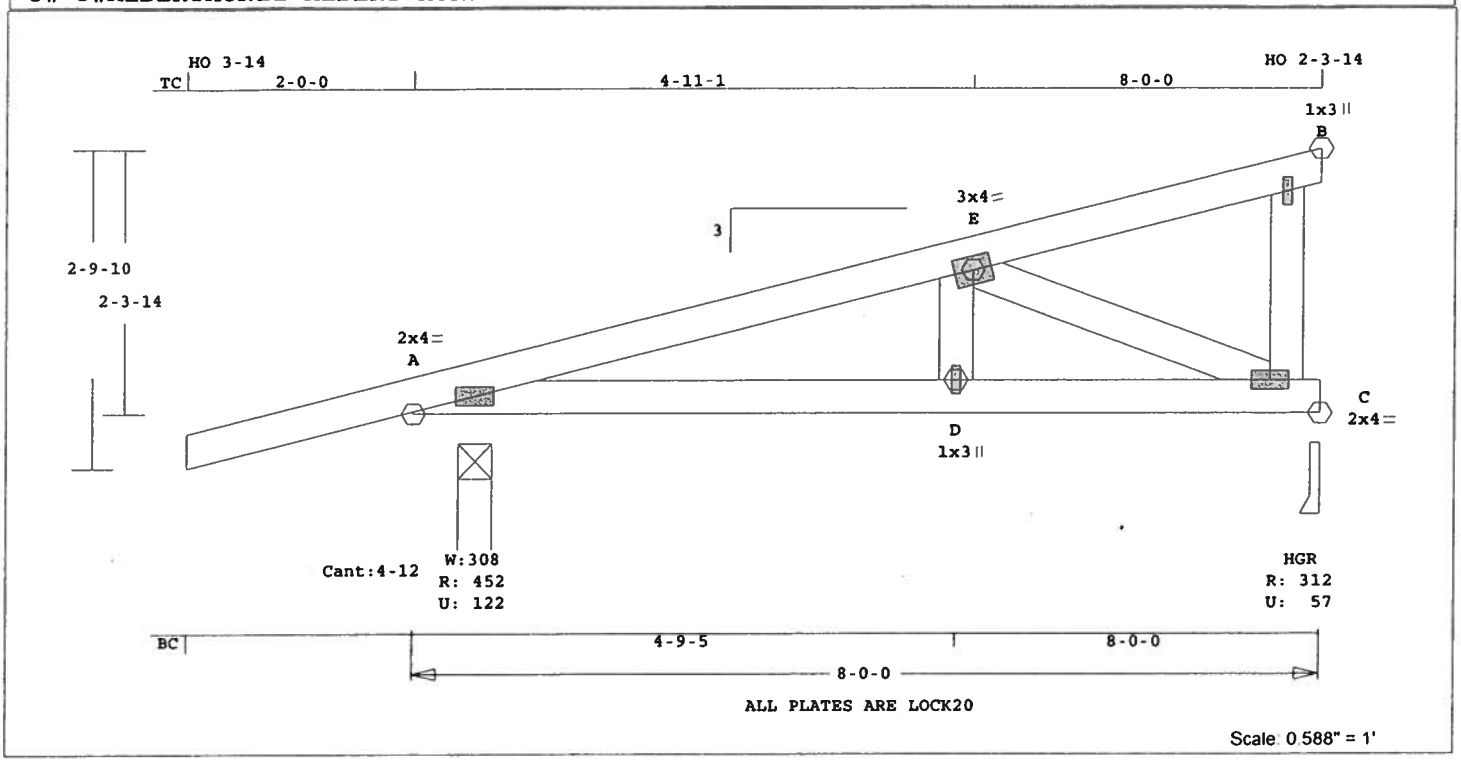
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 667 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ	M5	20	MONO	80000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 45.0 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.16	2x 4	SP-#2
WB	0.07	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8- 0- 0
BC	Cont.	0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	453	123	3- 8	1- 8
			Hz =	31
C	313	58	3- 8	1- 8
			Hz =	68

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -E	0.13	508 C	0.00 0.13
E -B	0.13	20 C	0.00 0.13
Bottom Chords			
A -D	0.16	497 T	0.08 0.08
D -C	0.12	497 T	0.05 0.07
Webs			
D -E	0.02	161 T	
E -C	0.07	538 C	
C -B	0.00	72 C	WindLd

TL Defl -0.02" in A -D L/999
LL Defl -0.01" in A -D L/999
Shear // Grain in A -E 0.16

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.87
E	LOCK	3.0x 4.0	Ctr	Ctr	0.39
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.76

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.

Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 538 Lbs
Quality Control Factor 1.25

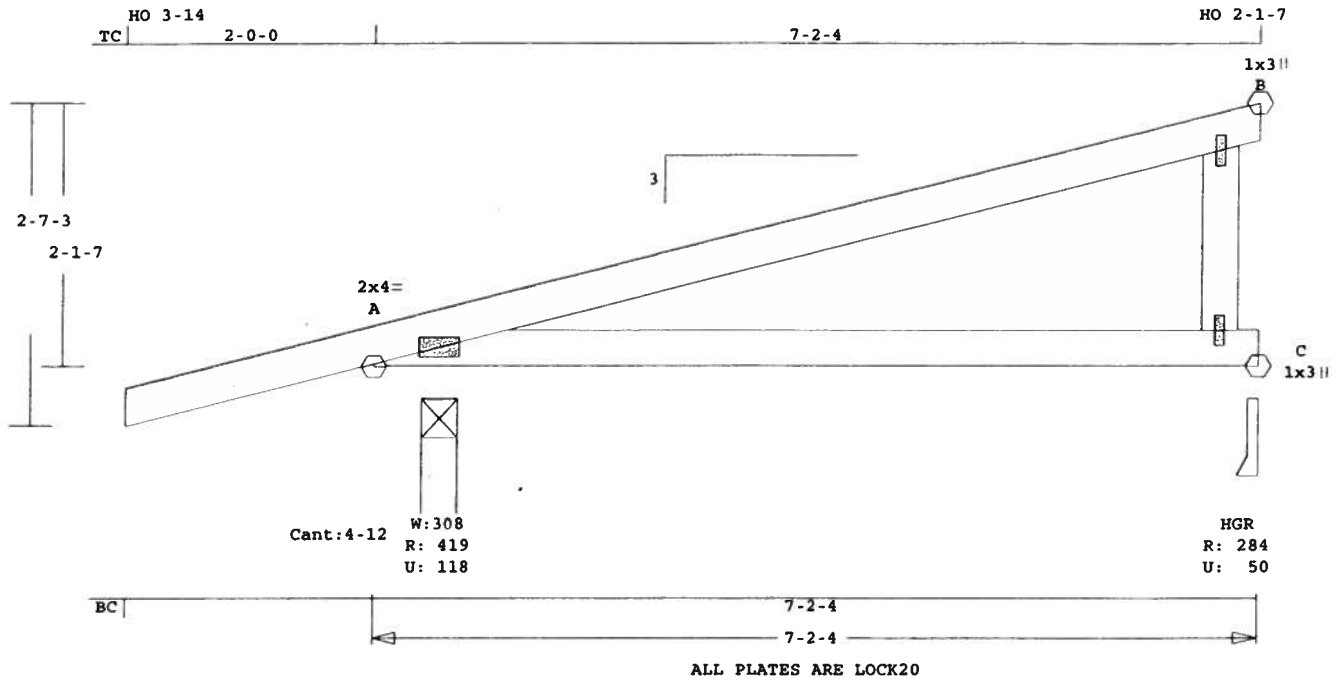
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

VUONG PHAN LICENSE No. 62111 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M6	3	JCA2.DD	70204	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 34.4 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.45	2x 4	SP-#2
BC	0.33	2x 4	SP-#2
WB	0.01	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 2- 4
BC Cont.	0- 0- 0	7- 2- 4

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	420	118	3- 8	1- 8
			Hz =	28
C	284	51	3- 8	1- 8
			Hz =	61

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.45	26 C	0.00 0.45
--------Bottom Chords---------			
A -C	0.33	47 T	0.00 0.33
------------Webs------------			
C -B	0.01	182 C	WindLd

TL Defl -0.16" in A -C L/479
LL Defl -0.06" in A -C L/999
Shear // Grain in A -B 0.22

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	LOCK	2.0x 4.0 Ctr	Ctr	0.85
B	LOCK	1.0x 3.0 Ctr	Ctr	0.75
C	LOCK	1.0x 3.0 Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

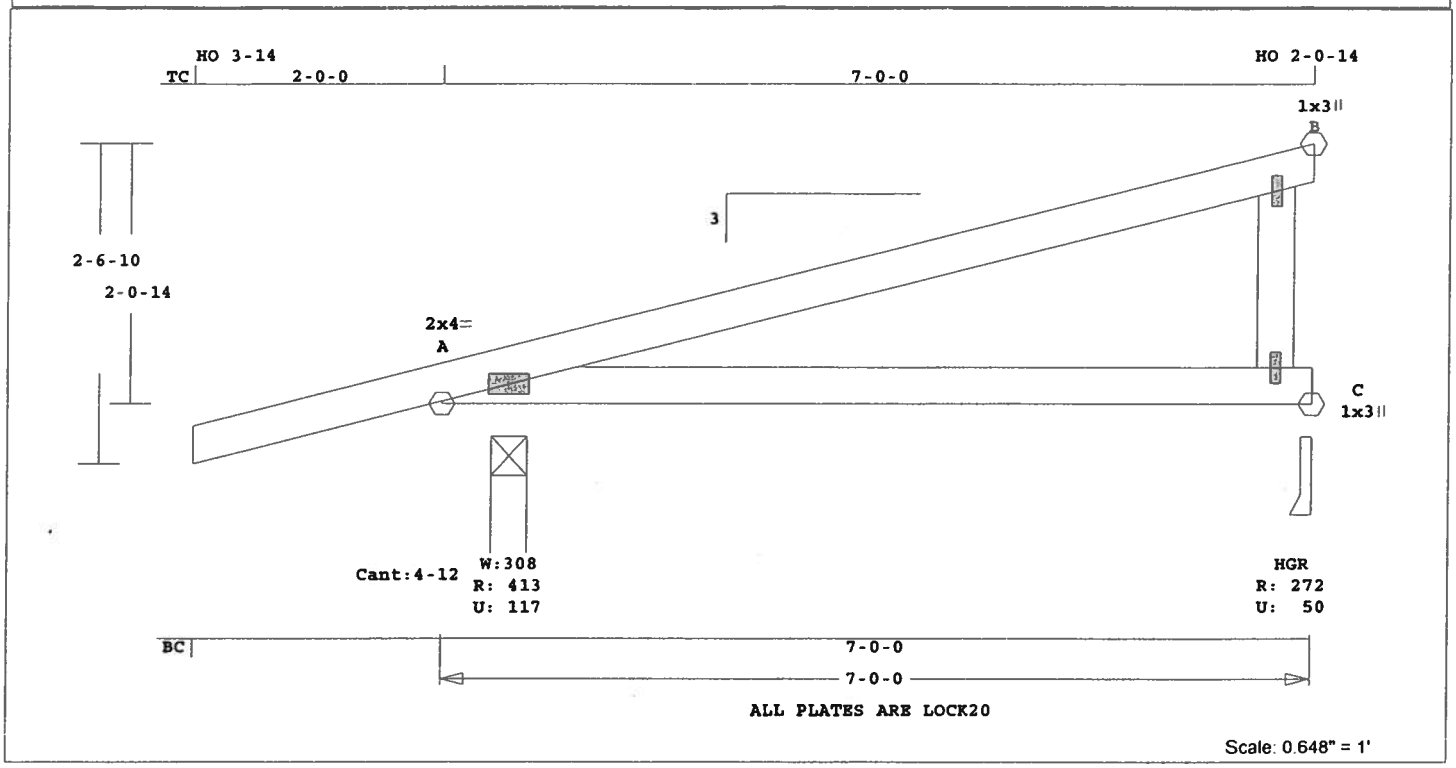
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 182 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M7	6	JCA2	70000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 33.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
WB	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 0- 0
BC	Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	413	117	3- 8	1- 8
			Hz =	27
C	273	51	3- 8	1- 8
			Hz =	59

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.43	25 C	0.00 0.43
--------Bottom Chords---------			
A -C	0.32	46 T	0.00 0.32
------------Webs------------			
C -B	0.01	178 C	WindLd

TL Defl -0.14" in A -C L/515
LL Defl -0.06" in A -C L/999
Shear // Grain in A -B 0.21

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.85
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

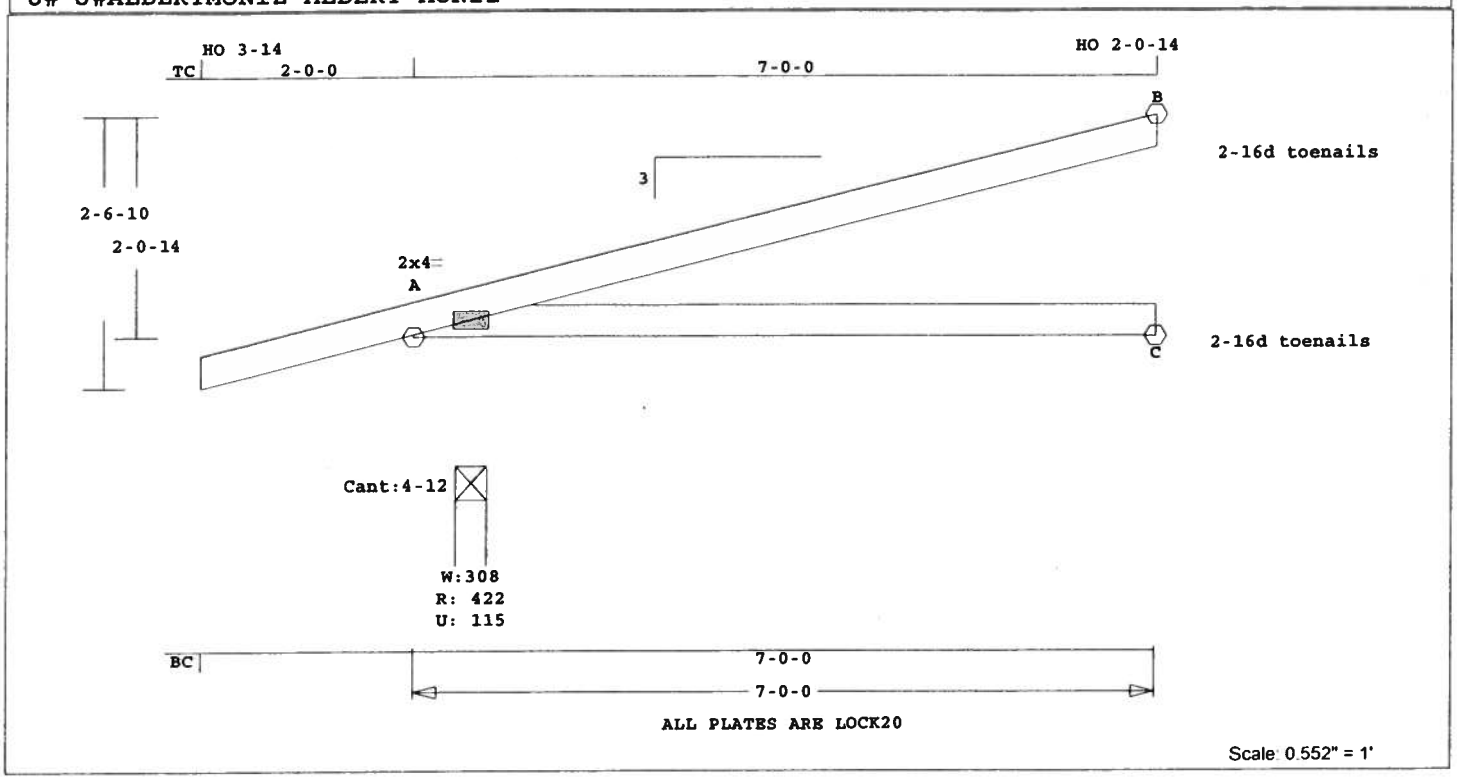
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 178 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License # 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M7A	2	JCA2	70000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 30.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.46	2x 4	SP-#2
BC	0.34	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0
BC Cont.	0- 0- 0	7- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	422	115	3- 8	1- 8
			Hz =	56
C	121	0	1- 8	1- 8
B	176	64	1- 8	1- 8
			Hz =	36

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.46	22 C	0.00 0.46
--------Bottom Chords---------			
A -C	0.34	0 T	0.00 0.34

TL Defl -0.16" in A -C L/459
LL Defl -0.07" in A -C L/999
Shear // Grain in A -B 0.22

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size X	Y	JSI
A LOCK	2.0x 4.0 Ctr	Ctr	0.85

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 22 Lbs
Quality Control Factor 1.25

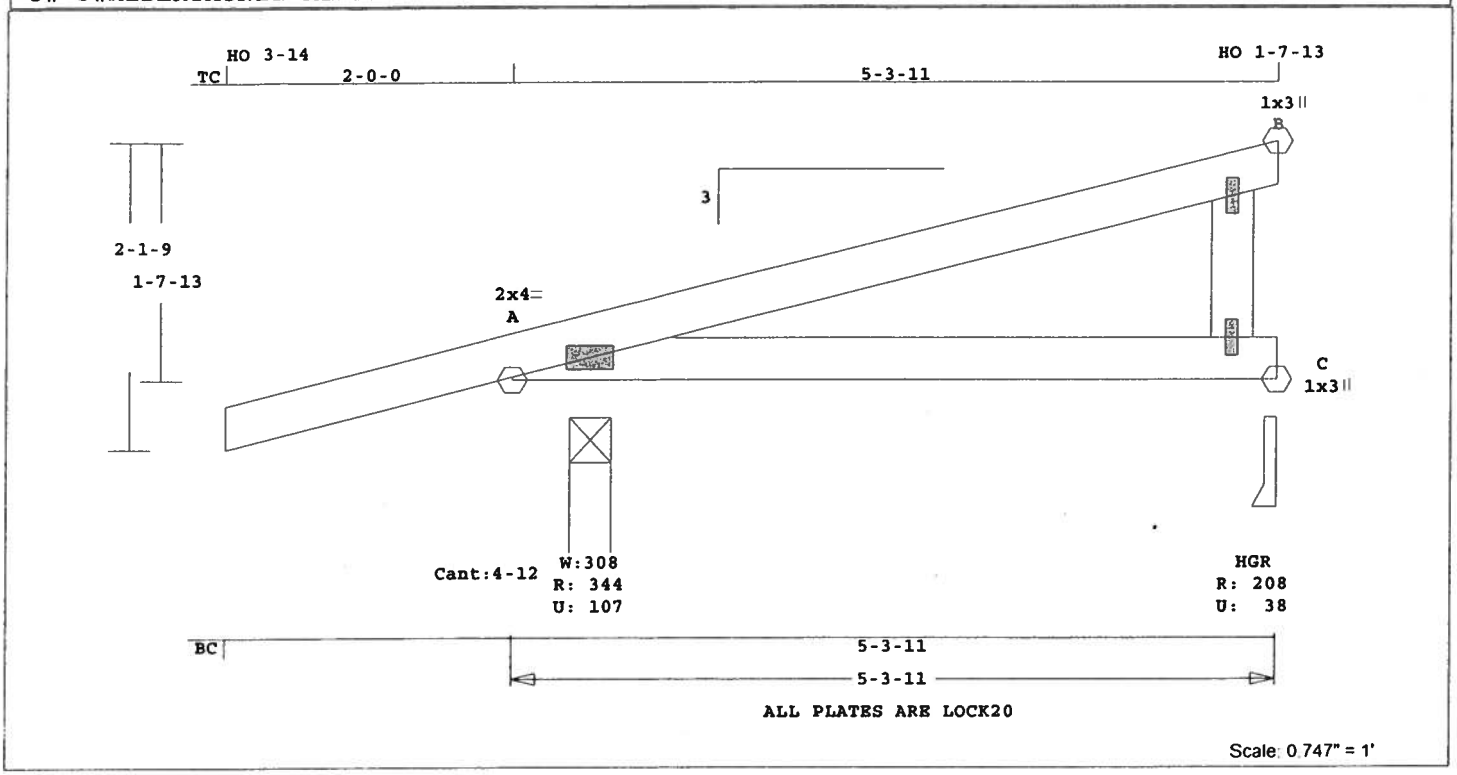
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

VUONG PHAN LICENSE No. 62111 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M8	1	JCA2	50311	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 26.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 4	SP-#2
BC	0.17	2x 4	SP-#2
WB	0.01	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 3-11
BC Cont.	0- 0- 0	5- 3-11

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	345	107	3- 8	1- 8
			Hz =	21
C	209	38	3- 8	1- 8
			Hz =	43

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.22	18 C	0.00 0.22
--------Bottom Chords---------			
A -C	0.17	33 T	0.00 0.17
----------Webs----------			
C -B	0.01	130 C	WindLd

TL Defl -0.04" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.80
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.

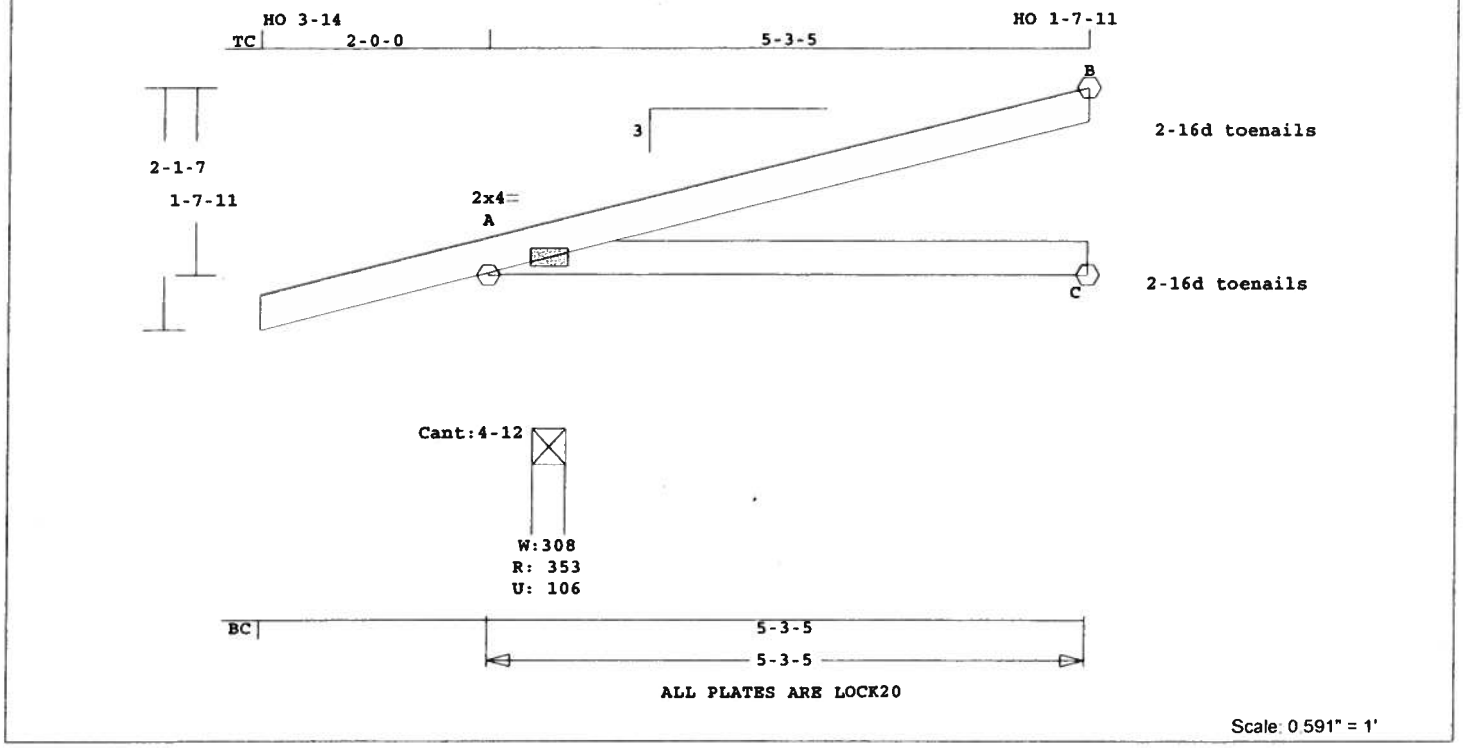
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 130 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Scaled: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M9	8	JCA2	50305	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 24.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.19	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 3- 5
BC Cont.	0- 0- 0	5- 3- 5

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	354	106	3- 8	1- 8
			Hz =	42
C	89	0	1- 8	1- 8
B	128	46	1- 8	1- 8
			Hz =	27

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.24	17 T	0.00 0.24
--------Bottom Chords---------			
A -C	0.19	0 T	0.00 0.19

TL Defl -0.05" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.16

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 16 Lbs
Quality Control Factor 1.25

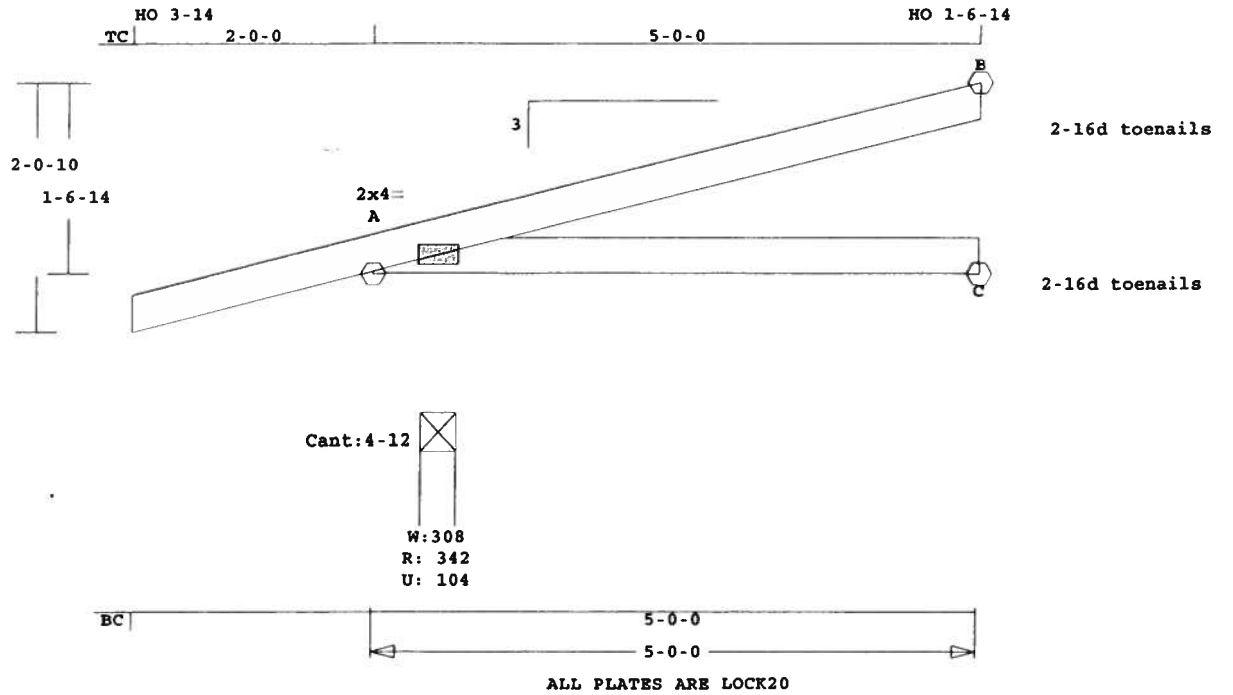
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

VUONG PHAN LICENSE No. 62111 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M10	4	JCA2	50000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.21	2x 4	SP-#2
BC	0.16	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	5- 0- 0
BC	Cont.	0- 0- 0	5- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	343	105	3- 8	1- 8
			Hz =	40
C	83	0	1- 8	1- 8
B	121	44	1- 8	1- 8
			Hz =	25

Membr	CSI	P Lbs	Axl-CSI-Bnd
Top Chords			
A -B	0.21	16 T	0.00 0.21
Bottom Chords			
A -C	0.16	0 T	0.00 0.16

TL Defl -0.04" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in A -B 0.15

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	LOCK	2.0x 4.0 Ctr	Ctr	0.80

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

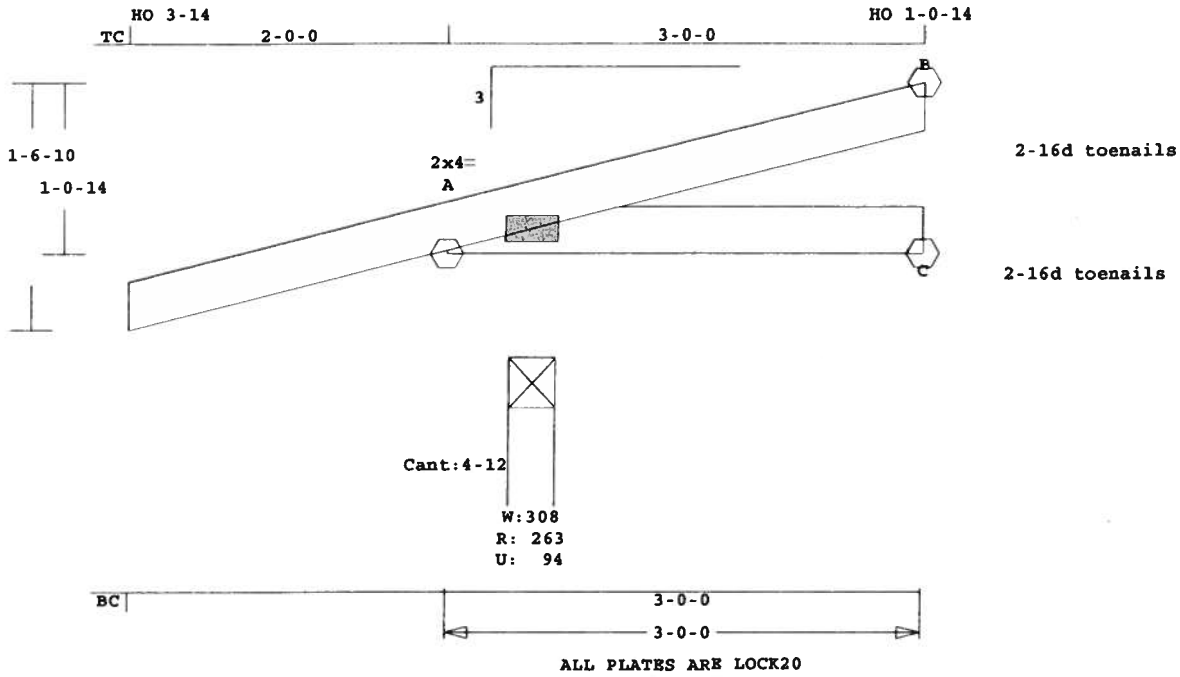
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 15 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	M11	4	JCA2	30000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Scale: 0.825" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.06	2x 4	SP-#2
BC	0.05	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	264	94	3- 8	1- 8
			Hz =	23
C	45	0	1- 8	1- 8
B	65	23	1- 8	1- 8
			Hz =	15

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.06	10 T	0.00 0.06
--------Bottom Chords---------			
A -C	0.05	0 T	0.00 0.05

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in A -B 0.08

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size X	Y	JSI
A	LOCK	2.0x 4.0 Ctr	Ctr	0.79

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

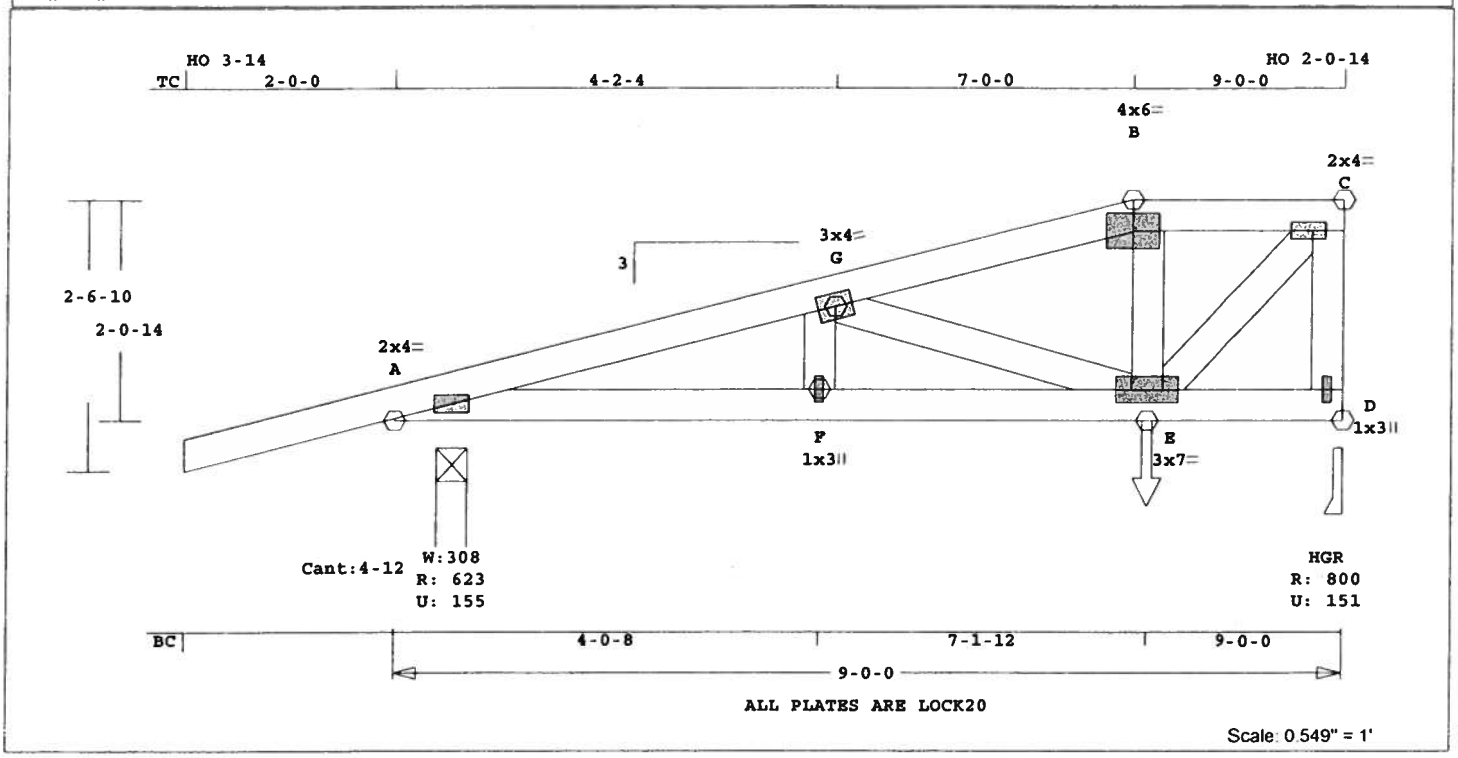
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 7 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	MG1	2	HHIP	90000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 54.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.19	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 0
BC Cont.	0- 0- 0	9- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To
TC V 40	20	0.0'	9.0'
BC V 0	20	0.0'	9.0'
TC V 50	25	7.0'	8.0'
TC V -40	-20	8.0'	9.0'
BC V 0	25	7.1'	8.0'
BC V 0	-20	8.0'	9.0'
BC V 280	280	7.1'	CL-LB

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	624	155	3- 8	1- 8
			Hz =	28
D	801	151	3- 8	1- 8
			Hz =	62

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -G	0.10	1143 C	0.01	0.09
G -B	0.09	767 C	0.00	0.09
B -C	0.06	744 C	0.06	0.00
Bottom Chords				
A -F	0.26	1107 T	0.20	0.06
F -E	0.26	1107 T	0.20	0.06
E -D	0.03	49 T	0.00	0.03
Webs				
F -G	0.01	114 T		
G -E	0.05	382 C		
E -B	0.00	61 T		
E -C	0.19	1076 T		
D -C	0.07	793 C	WindLd	

TL Defl -0.04" in F -E L/999
LL Defl -0.02" in F -E L/999
Shear // Grain in A -G 0.12

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

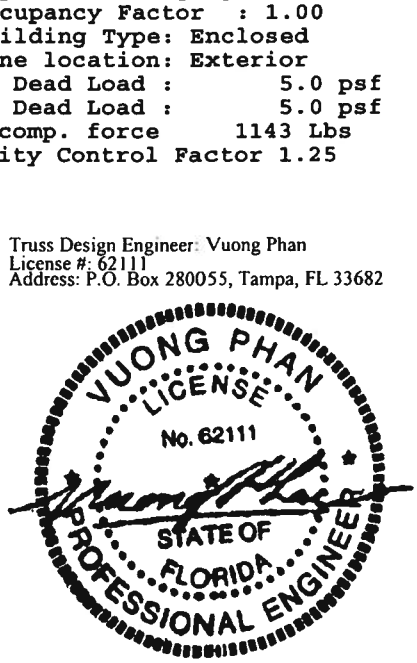
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.90
G	LOCK	3.0x 4.0	Ctr	Ctr	0.40
B	LOCK	4.0x 6.0	Ctr	Ctr	0.65
C	LOCK	2.0x 4.0-0.5	Ctr	Ctr	0.82
F	LOCK	1.0x 3.0	Ctr	Ctr	0.75
E	LOCK	3.0x 7.0	Ctr	Ctr	0.78
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 7- 0- 0
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1143 Lbs
Quality Control Factor 1.25

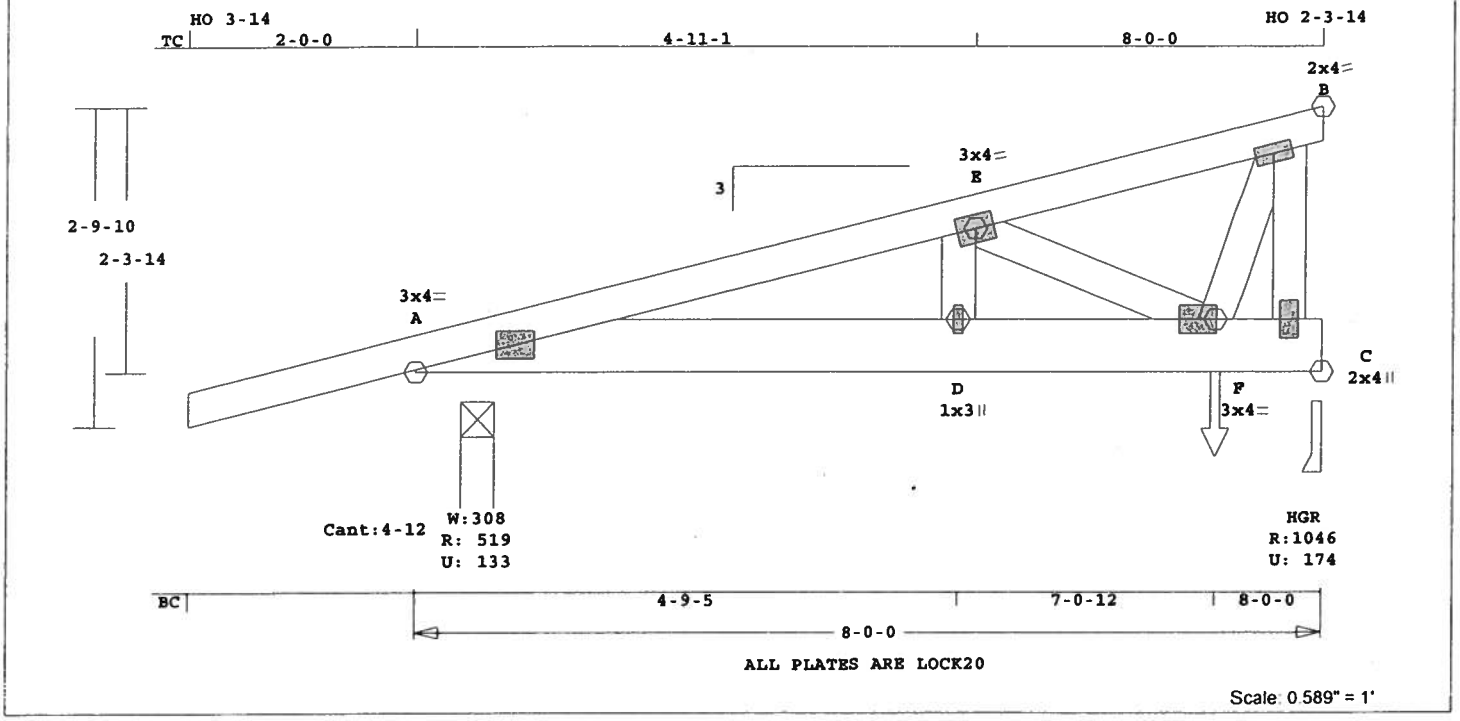
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Scaled: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	MG2	2	MONO	8'0000	3	2- 0- 0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 53.7 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.10	2x 4	SP-#2
BC	0.19	2x 6	SP-#2
WB	0.17	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 0- 0
BC Cont.	0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	8.0'
BC V	0	20	0.0'	8.0'
BC V	400	400	7.1'	CL-LB

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	519	134	3- 8	1- 8
			Hz =	31
C	1046	174	3- 8	1- 8
			Hz =	66

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -E	0.10	765 C	0.00 0.10
E -B	0.10	297 C	0.00 0.10
--------Bottom Chords---------			
A -D	0.19	746 T	0.09 0.10
D -F	0.17	746 T	0.09 0.08
F -C	0.08	51 T	0.00 0.08
------------Webs------------			
D -E	0.02	176 T	
E -F	0.05	514 C	
F -B	0.17	920 T	
C -B	0.08	883 C	WindLd

TL Defl -0.02" in A -D L/999
LL Defl -0.01" in A -D L/999
Shear // Grain in A -E 0.13

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.91
E	LOCK	3.0x 4.0	Ctr	Ctr	0.39
B	LOCK	2.0x 4.0	Ctr	Ctr	0.98
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
F	LOCK	3.0x 4.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.42

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

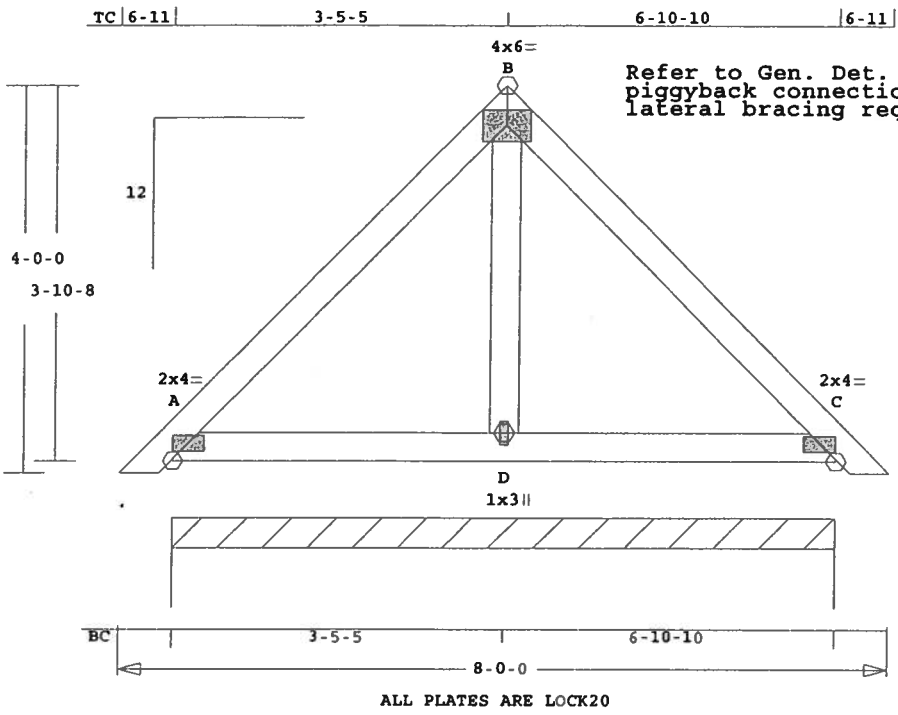
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 883 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	P4	9	TR	80000	12	6-11	6-11	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Refer to Gen. Det. 12 series for piggyback connection and top chord lateral bracing requirements.

Scale: 0.501" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.4 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	8- 0- 0
BC	Cont.	0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 6-10-10			
	622	86	Hz =	76

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -B	0.07	151 C	0.00	0.07
B -C	0.07	151 C	0.00	0.07
Bottom Chords				
A -D	0.07	0 T	0.00	0.07
D -C	0.07	0 T	0.00	0.07
Webs				
D -B	0.00	59 C		

TL Defl 0.00" in D -C L/999
LL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.08

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.4	0.3	0.62
B	LOCK	4.0x 6.0	Ctr	-0.1	0.35
C	LOCK	2.0x 4.0	-0.4	0.3	0.62
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

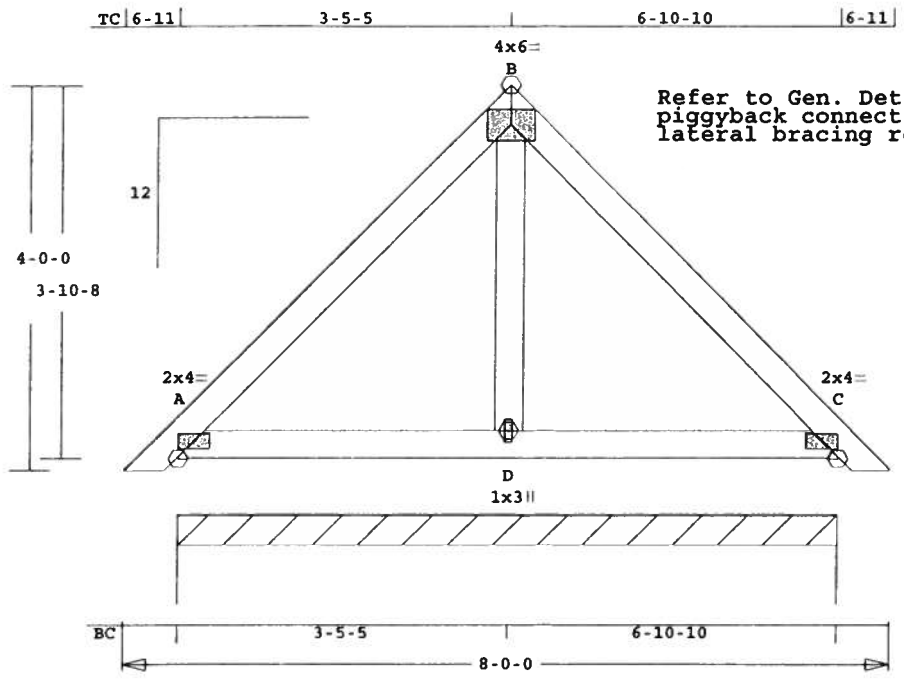
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 151 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Scaled: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	P5	2*3P	TR	80000	12	6-11	6-11	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Refer to Gen. Det. 12 series for piggyback connection and top chord lateral bracing requirements.

ALL PLATES ARE LOCK20

Scale: 0.501" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 40.4 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 3-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	2- 0- 0	0- 0- 0	8- 0- 0
BC	2- 0- 0	0- 0- 0	8- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			72.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0	to	6-10-10	
	1867	258	Hz =	229

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.07	455 C	0.00	0.07
B -C	0.07	455 C	0.00	0.07
--------Bottom Chords---------				
A -D	0.07	1 T	0.00	0.07
D -C	0.07	1 T	0.00	0.07
------------Webs------------				
D -B	0.00	178 C		

TL Defl 0.00" in D -C L/999
LL Defl 0.00" in D -C L/999
Shear // Grain in A -B 0.08

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.4	0.3	0.62
B	LOCK	4.0x 6.0	Ctr	-0.1	0.35
C	LOCK	2.0x 4.0	-0.4	0.3	0.62
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

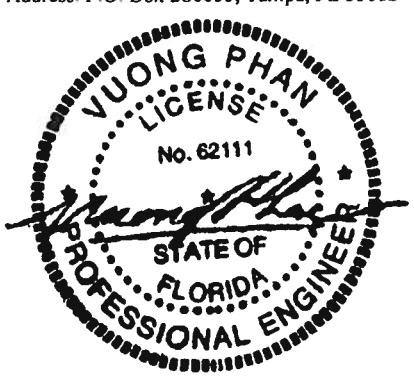
REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
3 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS4.5 screws -OR- 16d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
		----Spacing (In)----		
TC	1	12	24	0
BC	1	12	24	0
WB	1	8	8	

No bolts in 2x4s or smaller.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 455 Lbs
Quality Control Factor 1.25

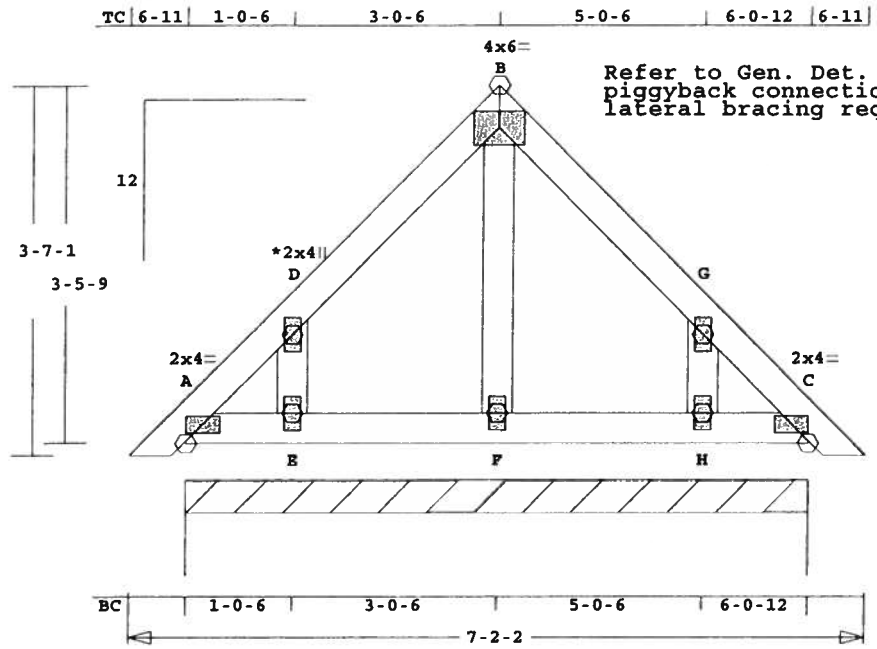
Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	P6	2	TR	70202	12	6-11	6-11	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



Refer to Gen. Det. 12 series for piggyback connection and top chord lateral bracing requirements.

ALL PLATES ARE LOCK20

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.533" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 39.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.02	2x 4	SP-#2
BC	0.02	2x 4	SP-#2
GW	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 2- 2
BC	Cont.	0- 0- 0	7- 2- 2

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to 6- 0-12			
	556	77	Hz =	67

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -D	0.02	60 C	0.00		0.02
D -B	0.02	43 C	0.00		0.02
B -G	0.02	43 C	0.00		0.02
G -C	0.02	60 C	0.00		0.02
--------Bottom Chords---------					
A -E	0.01	1 T	0.00		0.01
E -F	0.02	0 T	0.00		0.02
F -H	0.02	0 T	0.00		0.02
H -C	0.01	1 T	0.00		0.01

----------Gable Webs----------			
E -D	0.01	99 C	
F -B	0.00	61 C	
H -G	0.01	99 C	

TL Defl 0.00" in F -H L/999
LL Defl 0.00" in F -H L/999
Shear // Grain in D -B 0.05

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

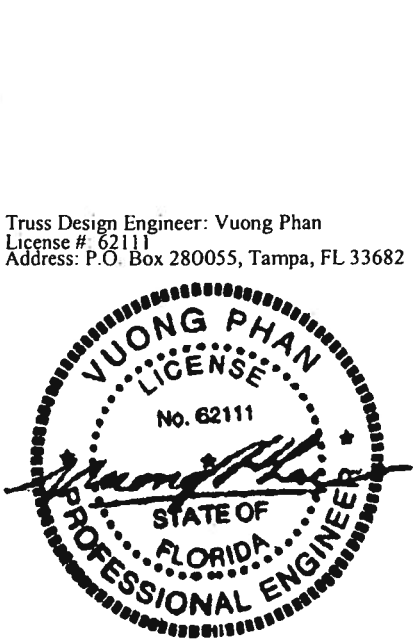
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.4	0.3	0.60
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	4.0x 6.0	Ctr	-0.1	0.34
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
C	LOCK	2.0x 4.0	-0.4	0.3	0.60
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 99 Lbs
Quality Control Factor 1.25

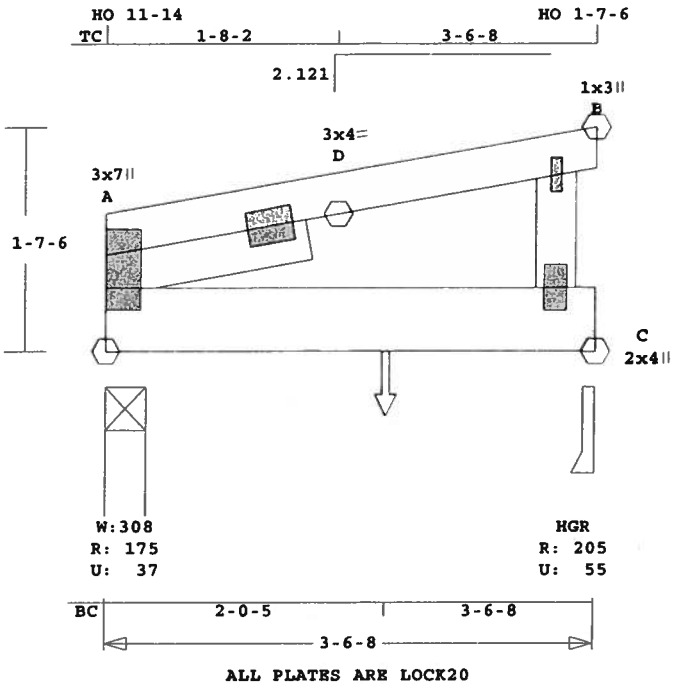
Truss Design Engineer: Vuong Phan
License # 62111
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ	RG1	1	SP	30608	2.121	0	0	T06080605

U# J#ALBERTMONIZ ALBERT MONIZ



ALL PLATES ARE LOCK20

Scale: 0.716" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 21.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.09	2x 4	SP-#2
BC	0.11	2x 6	SP-#2
WB	0.00	2x 4	SP-#2
SL	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 6- 8
BC Cont.	0- 0- 0	3- 6- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	3.5'
BC V	0	20	0.0'	3.5'
BC V	50	50	2.0'	CL-LB

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	175	38	3- 8	1- 8
			Hz =	-13
C	206	56	3- 8	1- 8
			Hz =	30

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -D	0.06	43 C	0.00	0.06
D -B	0.09	12 C	0.00	0.09
Bottom Chords				
A -C	0.11	20 T	0.00	0.11
Webs				
C -B	0.00	85 C	WindLd	
Sliders				
A -D	0.00	66 C		

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.09

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 7.0	1.5	0.1	0.93
D	LOCK	3.0x 4.0	Ctr	Ctr	0.50
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	2.0x 4.0	Ctr	Ctr	0.38

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

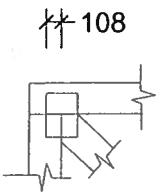
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 85 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Vuong Phan
License #: 62111
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

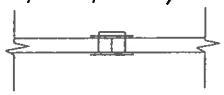
ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

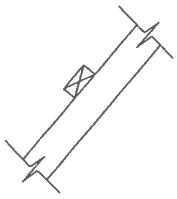
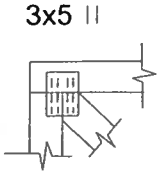


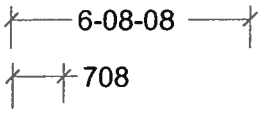
PLATE SIZE AND ORIENTATION

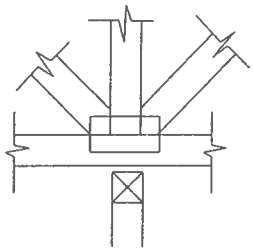


The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted.
The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA)," National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

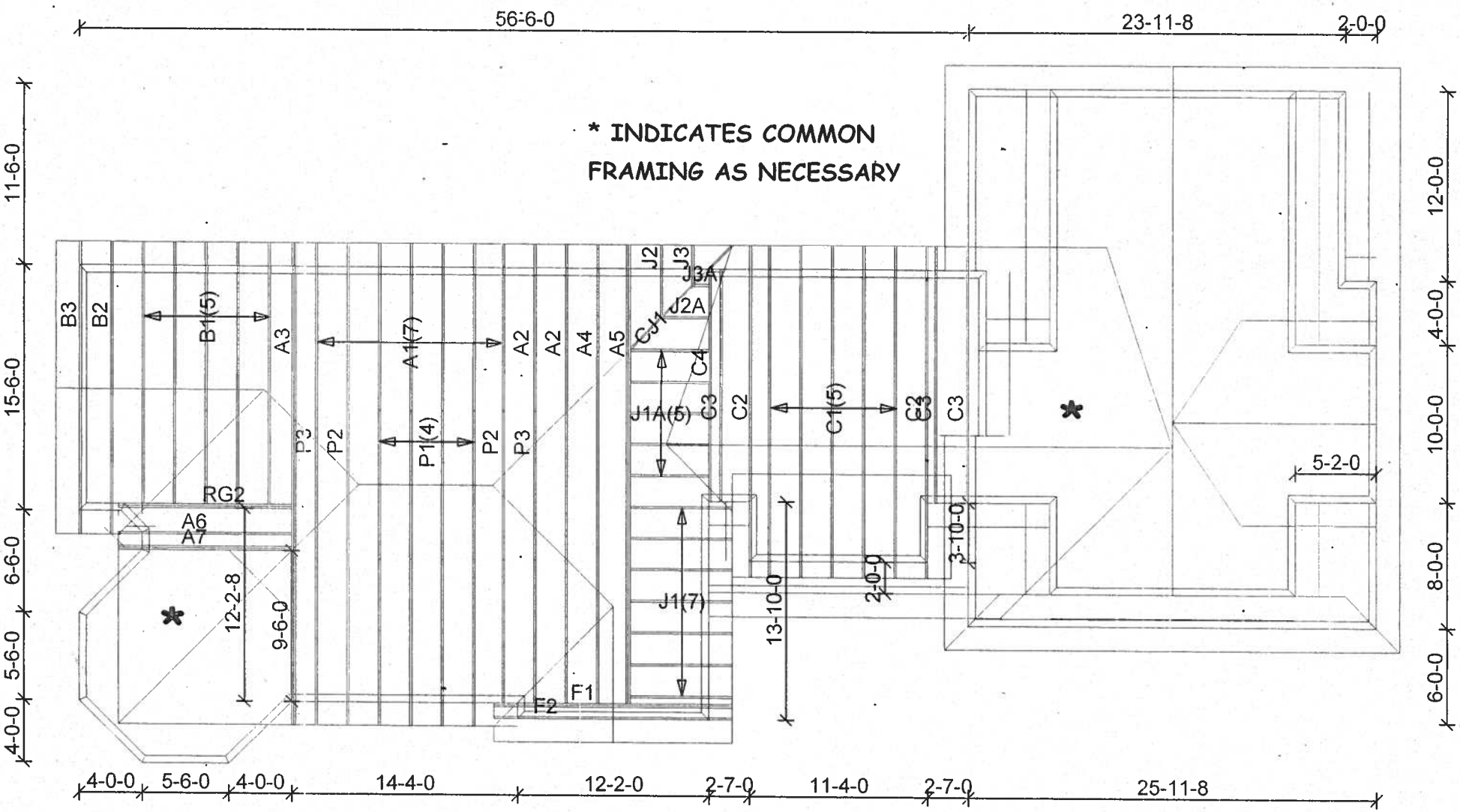
Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.

Robbins Engineering, Inc.

6904 Parke East Blvd.
Tampa, Fl 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbinseng.com



Mayo Truss Co. Inc. 845 East US 27 MAYO, FL 32066 (386)294-3988 (877)-558-6262	ALBERT MONIZ 2nd STORY 110 MPH ASCE WIND LOAD	Roof Loading TC Live: 20.00 psf TC Dead: 10.00 psf BC Live: 0.00 psf BC Dead: 10.00 psf TC Stress Inc: 25.00 BC Stress Inc: 25.00 Spacing: 2- 0- 0 o.c.	Account: INDIVIDUAL Job: ALBERTMONIZ2 Designer: M.MURRAY Checker: M.MURRAY Date: 08-15-06

Permit Number: _____ Lot Number: _____

Miscellaneous: _____ Address: _____

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator: Mayo Truss Company, Inc

Job Reference: ALBERTMONIZ2 - ALBERT MONEZ - 2nd STORY

Standard Loading:

T.C. Live	20 psf
T.C Dead	10 psf
B.C Live	0 psf
B.C. Dead	10 psf
Total	40 psf

ROBBINS ENGINEERING, INC.

P.O. Box 280055
Tampa, FL 33682-0055
Phone: (813) 972-1135

Engineering Index Sheet

Index Page 1 of 1

ANSI/ASCE 7-02
Wind Speed - 110 MPH
Mean Roof Ht. - 15 FT
Exposure Catergory - B
Occupancy Factor - 1.00
MWFRS
Enclosed

Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06080606	08/07/2006		27

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibilty of the Building Designer, as defined in ANSI/TPI 1-2002, Section 2.2. Permanent files of the original Truss Specificstion Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Sofware - Online Plus)

Notes: Refer to individual truss design drawings for special loading conditions.

	Date	Mark		Date	Mark		Date	Mark		Date	Mark
1	08/07/06	A1	2	08/07/06	A2	3	08/07/06	A3	4	08/07/06	A4
5	08/07/06	A5	6	08/07/06	A6	7	08/07/06	A7	8	08/07/06	B1
9	08/07/06	B2	10	08/07/06	B3	11	08/07/06	C1	12	08/07/06	C2
13	08/07/06	C3	14	08/07/06	C4	15	08/07/06	CJ1	16	08/07/06	F1
17	08/07/06	F2	18	08/07/06	J1	19	08/07/06	J1A	20	08/07/06	J2
21	08/07/06	J2A	22	08/07/06	J3	23	08/07/06	J3A	24	08/07/06	P1
25	08/07/06	P2	26	08/07/06	P3	27	08/07/06	RG2			

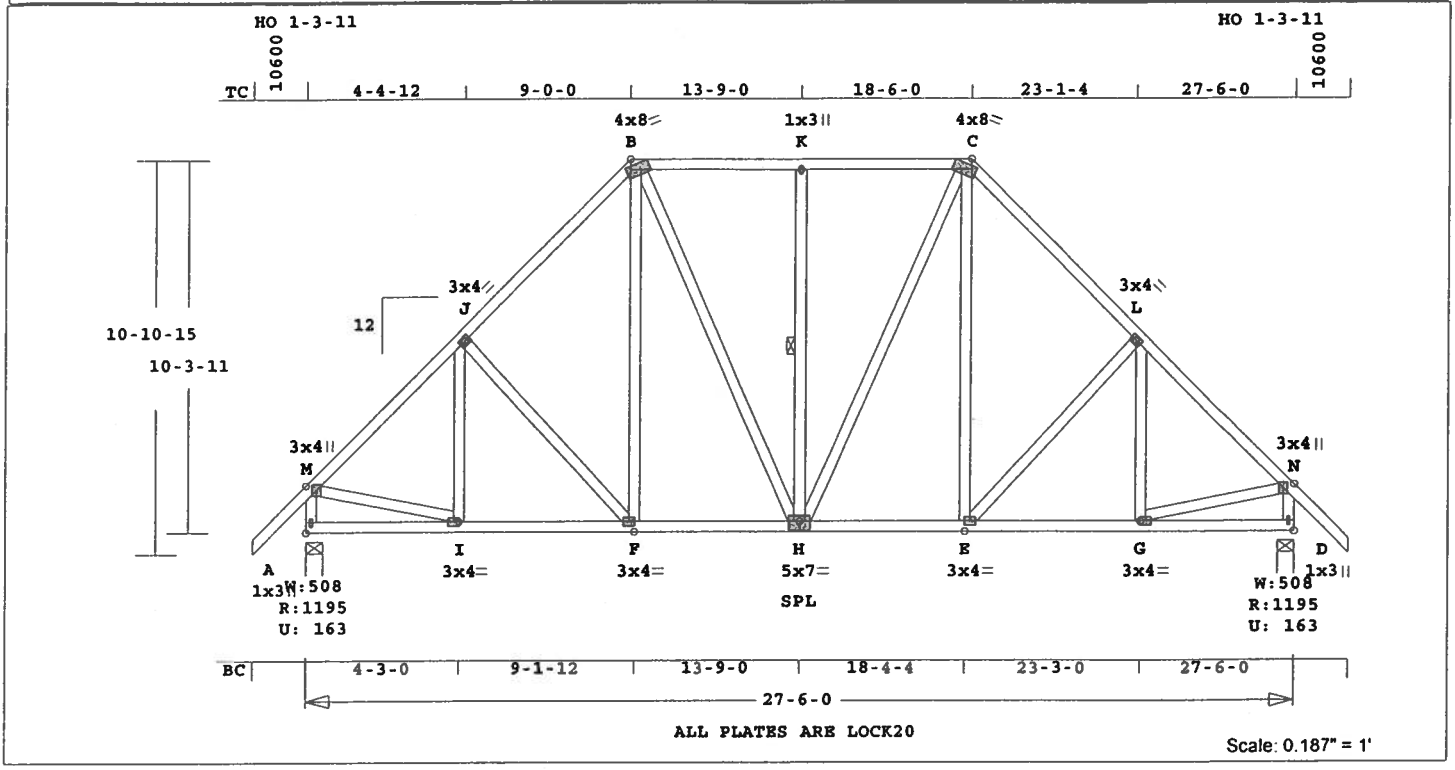
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

PHILIP J. O'REGAN
LICENSE
No. 58126
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A1	7	HIPP	270600	12	1- 6- 0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 285.1 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	Size	Lumber
TC	0.22	2x 4	SP-#2
BC	0.19	2x 4	SP-#2
WB	0.18	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 0- 0
TC	2- 0- 0	9- 0- 0	18- 6- 0
TC	Cont.	18- 6- 0	27- 6- 0
BC	Cont.	0- 0- 0	27- 6- 0

WB 1 rows CLB on H -K
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1196	163	5- 8	1- 8
			Hz =	-258
D	1196	163	5- 8	1- 8
			Hz =	259

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
M -J	0.20	1050	C	0.00	0.20
J -B	0.20	944	C	0.00	0.20
B -K	0.22	754	C	0.00	0.22
K -C	0.22	754	C	0.00	0.22
C -L	0.20	944	C	0.00	0.20
L -N	0.20	1050	C	0.00	0.20
Bottom Chords					
A -I	0.12	250	T	0.00	0.12
I -F	0.19	756	T	0.07	0.12
F -H	0.17	662	T	0.11	0.06
H -E	0.17	662	T	0.11	0.06
E -G	0.19	756	T	0.07	0.12
G -D	0.12	250	T	0.00	0.12
Webs					
A -M	0.10	1064	C	WindLd	
M -I	0.14	783	T		
I -J	0.03	105	C		
J -F	0.09	149	C		
F -B	0.07	259	T		
B -H	0.18	221	T		
H -K	0.10	311	C		1 Br
H -C	0.18	221	T		
E -C	0.07	259	T		
E -L	0.09	149	C		
G -L	0.03	105	C		
G -N	0.14	783	T		
D -N	0.10	1064	C	WindLd	

TL Defl -0.05" in F -H L/999
LL Defl -0.02" in I -F L/999
Shear // Grain in B -K 0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
M	LOCK	3.0x 4.0	Ctr	Ctr	0.79
J	LOCK	3.0x 4.0	Ctr	Ctr	0.84
B	LOCK	4.0x 8.0	2.5	-3.4	0.72
K	LOCK	1.0x 3.0	Ctr	Ctr	0.81
C	LOCK	4.0x 8.0	-2.5	-3.4	0.72
L	LOCK	3.0x 4.0	Ctr	Ctr	0.84
N	LOCK	3.0x 4.0	Ctr	Ctr	0.79
A	LOCK	1.0x 3.0	Ctr	Ctr	0.81
I	LOCK	3.0x 4.0	Ctr	Ctr	0.81
F	LOCK	3.0x 4.0	Ctr	Ctr	0.59
H	LOCK	5.0x 7.0	Ctr	-0.5	0.66
E	LOCK	3.0x 4.0	Ctr	Ctr	0.59
G	LOCK	3.0x 4.0	Ctr	Ctr	0.81
D	LOCK	1.0x 3.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.

PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

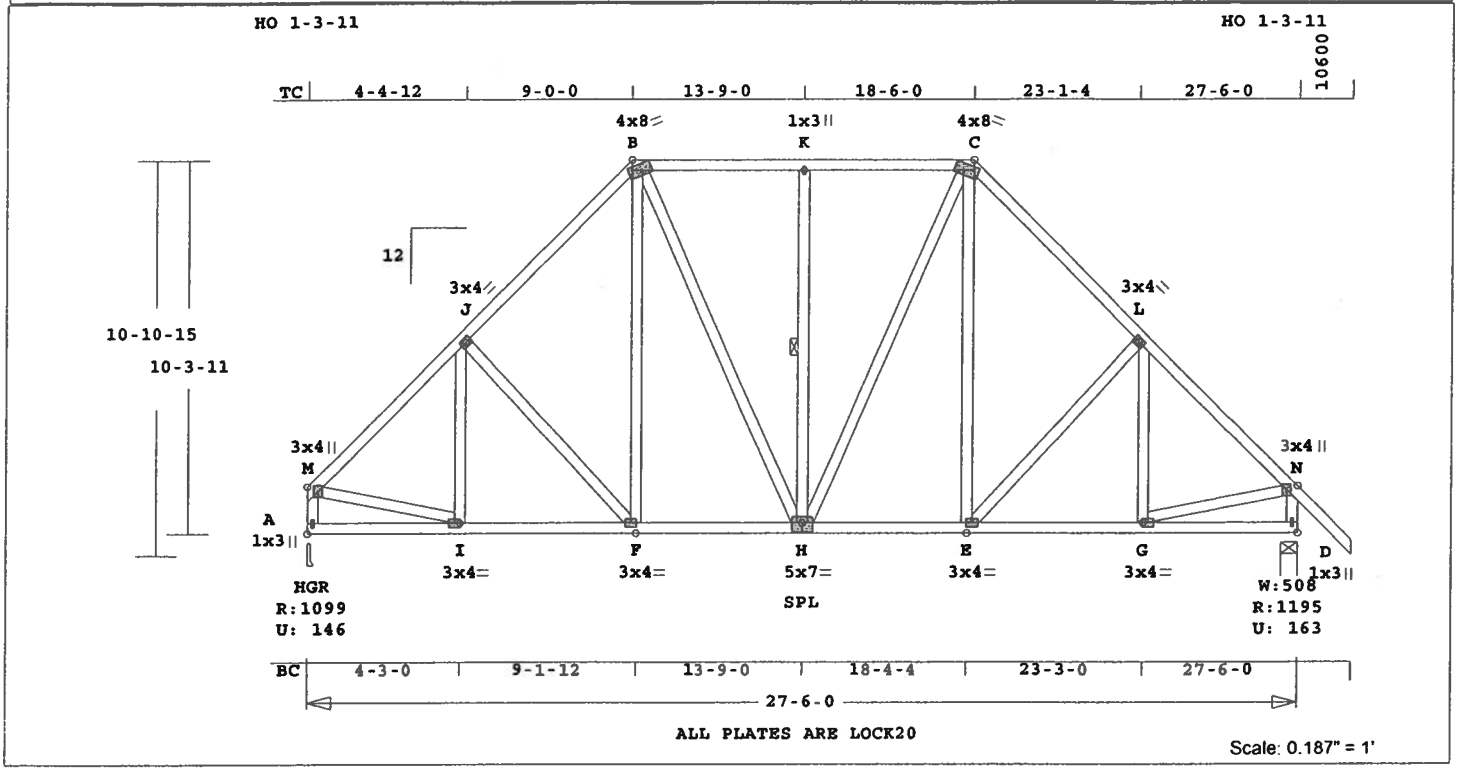
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1064 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A2	2	HIPP	270600	12	0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 281.1 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.22	2x 4	SP-#2
BC	0.19	2x 4	SP-#2
WB	0.18	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 0- 0
TC 2- 0- 0	9- 0- 0	18- 6- 0
TC Cont.	18- 6- 0	27- 6- 0
BC Cont.	0- 0- 0	27- 6- 0

WB 1 rows CLB on H -K
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1100	147	3- 8	1- 8
			Hz =	-258
D	1196	163	5- 8	1- 8
			Hz =	259

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
M -J	0.20	1050 C	0.00	0.20
J -B	0.20	944 C	0.00	0.20
B -K	0.22	754 C	0.00	0.22
K -C	0.22	754 C	0.00	0.22
C -L	0.20	944 C	0.00	0.20
L -N	0.20	1050 C	0.00	0.20
--------Bottom Chords---------				
A -I	0.12	250 T	0.00	0.12
I -F	0.19	756 T	0.07	0.12
F -H	0.17	662 T	0.11	0.06
H -E	0.17	662 T	0.11	0.06
E -G	0.19	756 T	0.07	0.12
G -D	0.12	250 T	0.00	0.12
----------Webs----------				
A -M	0.10	1064 C	WindLd	
M -I	0.14	783 T		
I -J	0.03	105 C		
J -F	0.09	149 C		
F -B	0.07	259 T		
B -H	0.18	221 T		
H -K	0.10	311 C		1 Br
H -C	0.18	221 T		
E -C	0.07	259 T		
E -L	0.09	149 C		
G -L	0.03	105 C		
G -N	0.14	783 T		
D -N	0.10	1064 C	WindLd	

TL Defl -0.05" in F -H L/999
LL Defl -0.02" in I -F L/999
Shear // Grain in B -K 0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
M	LOCK	3.0x 4.0	Ctr	Ctr	0.79
J	LOCK	3.0x 4.0	Ctr	Ctr	0.84
B	LOCK	4.0x 8.0	2.5	-3.4	0.72
K	LOCK	1.0x 3.0	Ctr	Ctr	0.81
C	LOCK	4.0x 8.0	-2.5	-3.4	0.72
L	LOCK	3.0x 4.0	Ctr	Ctr	0.84
N	LOCK	3.0x 4.0	Ctr	Ctr	0.79
A	LOCK	1.0x 3.0	Ctr	Ctr	0.81
I	LOCK	3.0x 4.0	Ctr	Ctr	0.81
F	LOCK	3.0x 4.0	Ctr	Ctr	0.59
H	LOCK	5.0x 7.0	Ctr	-0.5	0.66
E	LOCK	3.0x 4.0	Ctr	Ctr	0.59
G	LOCK	3.0x 4.0	Ctr	Ctr	0.81
D	LOCK	1.0x 3.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.

PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1064 Lbs
Quality Control Factor 1.25

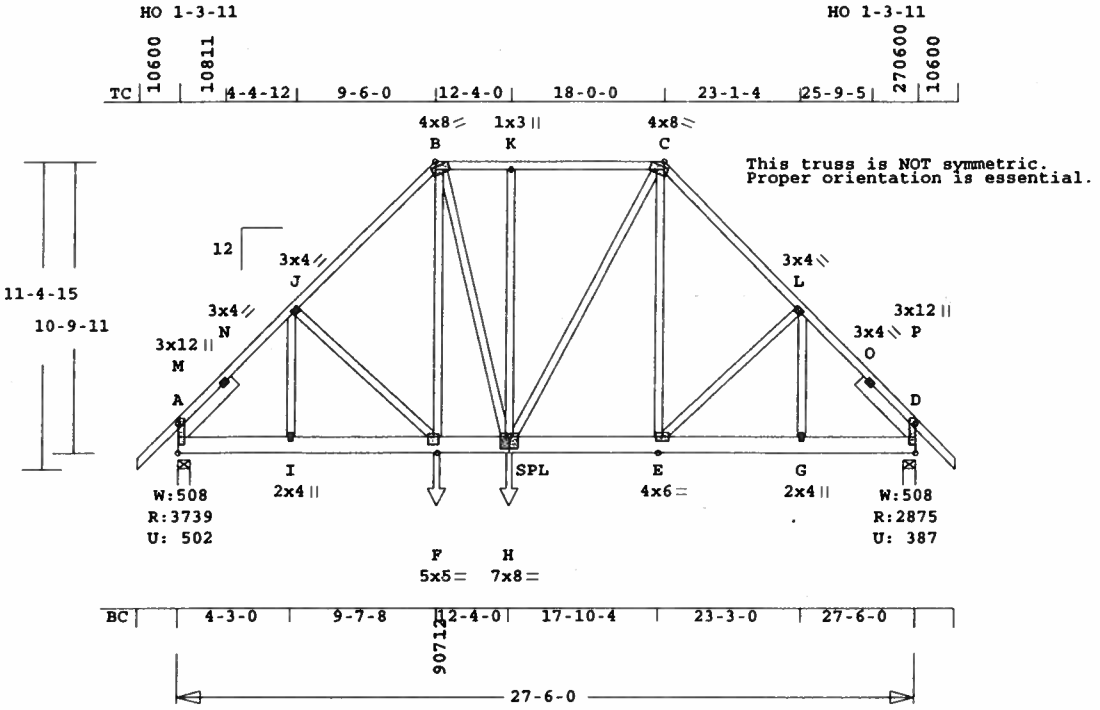
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

PHILIP J. O'REGAN LICENSE No. 58126 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ2	A3	1*2P	HIPP	270600	12	1- 6- 0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 337.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.41	2x 4	SP-#2
BC	0.63	2x 8	SP-#2
WB	0.18	2x 4	SP-#2
SL	0.05	2x 6	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	27- 6- 0
BC Cont.	0- 0- 0	27- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf - Live	Dead	From	To	
TC V	40	20	0.0'	27.5'
BC V	0	20	0.0'	27.5'
BC V	75	75	9.6'	12.3'
BC V	973	973	9.6'	CL-LB
BC V	937	937	12.3'	CL-LB

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	3740	502	5- 8	2- 3
			Hz =	-239
D	2876	387	5- 8	1-11
			Hz =	240

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
M -N	0.39	2467 C	0.01	0.38
N -J	0.41	3951 C	0.03	0.38
J -B	0.32	4163 C	0.03	0.29
B -K	0.13	3156 C	0.01	0.12
K -C	0.16	3156 C	0.02	0.14
C -L	0.23	3089 C	0.01	0.22
L -O	0.28	2956 C	0.01	0.27
O -P	0.27	1864 C	0.00	0.27
--------Bottom Chords---------				
M -I	0.63	2698 T	0.15	0.48
I -F	0.29	2698 T	0.15	0.14
F -H	0.21	2964 T	0.16	0.05
H -E	0.15	2209 T	0.12	0.03
E -G	0.23	2029 T	0.11	0.12
G -P	0.47	2029 T	0.11	0.36
------------Webs------------				
I -J	0.03	510 C		
J -F	0.03	421 T		
F -B	0.18	2025 T		
B -H	0.07	772 T		
H -K	0.05	342 C		
H -C	0.18	2013 T		
E -C	0.01	134 T		
E -L	0.02	280 T		
G -L	0.02	406 C		
----------Sliders----------				
M -N	0.05	1627 C		
O -P	0.03	1220 C		

TL Defl -0.11" in F -H L/999
LL Defl -0.06" in F -H L/999
Shear // Grain in A -I 0.17

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
M	LOCK	3.0x12.0	1.5	0.6	0.68
N	LOCK	3.0x 4.0	Ctr	Ctr	0.73
J	LOCK	3.0x 4.0	Ctr	Ctr	0.84
B	LOCK	4.0x 8.0	2.5-3.4		0.72
K	LOCK	1.0x 3.0	Ctr	Ctr	0.79
C	LOCK	4.0x 8.0	-2.5-3.4		0.91
L	LOCK	3.0x 4.0	Ctr	Ctr	0.84
O	LOCK	3.0x 4.0	Ctr	Ctr	0.73
P	LOCK	3.0x12.0	-1.5	0.6	0.68
I	LOCK	2.0x 4.0	Ctr	Ctr	0.40
F	LOCK	5.0x 5.0	Ctr-1.1		0.68
H	LOCK	7.0x 8.0	Ctr-2.0		0.62
E	LOCK	4.0x 6.0	Ctr	Ctr	0.40
G	LOCK	2.0x 4.0	Ctr	Ctr	0.40

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	Rows	Nails	Screws	Bolts
	----Spacing (In)----			
TC	1	12	24	0
BC	2	12	24	0
WB	1	8	8	

Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

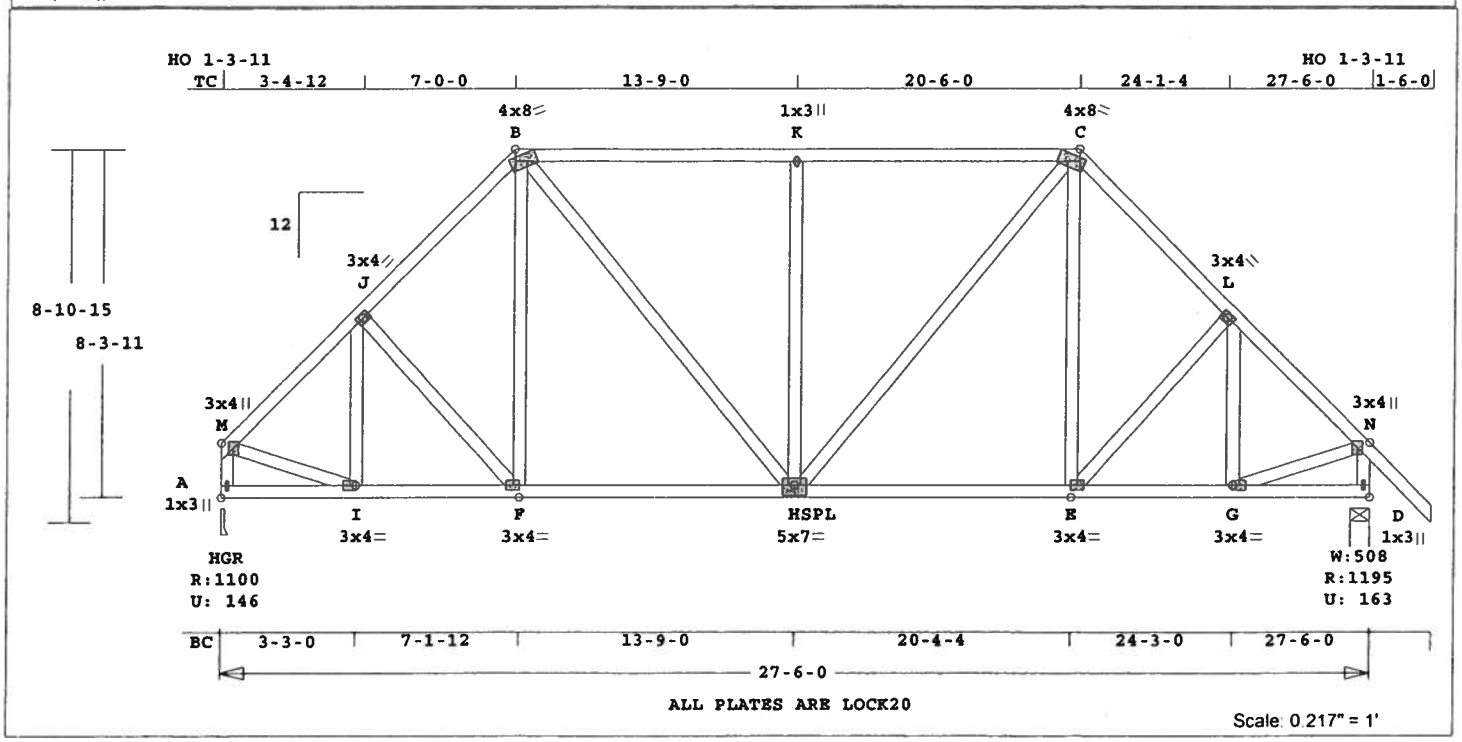
Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pk-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ2	A3	1*2P	HIPP	270600	12	1- 6- 0	1- 6- 0	T06080606
U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY								

Max comp. force 4163 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A4	1	HIPP	270600	12	0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 251.4 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.50	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
WB	0.36	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	27- 6- 0
BC Cont.	0- 0- 0	27- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	1100	147	3- 8	1- 8
			Hz =	-207
D	1196	163	5- 8	1- 8
			Hz =	208

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
M -J	0.10	1014	C	0.02	0.08
J -B	0.18	1009	C	0.00	0.18
B -K	0.50	968	C	0.00	0.50
K -C	0.50	968	C	0.00	0.50
C -L	0.18	1009	C	0.00	0.18
L -N	0.10	1014	C	0.02	0.08
Bottom Chords					
A -I	0.05	199	T	0.00	0.05
I -F	0.23	724	T	0.07	0.16
F -H	0.32	720	T	0.07	0.25
H -E	0.32	720	T	0.07	0.25
E -G	0.23	724	T	0.07	0.16
G -D	0.05	199	T	0.00	0.05
Webs					
A -M	0.10	1069	C	WindLd	
M -I	0.14	768	T		
I -J	0.04	194	C		
J -F	0.03	102	T		
F -B	0.03	215	T		
B -H	0.15	388	T		
H -K	0.36	456	C		
H -C	0.15	388	T		
E -C	0.03	215	T		
E -L	0.03	102	T		
G -L	0.04	194	C		
G -N	0.14	768	T		
D -N	0.10	1069	C	WindLd	

TL Defl -0.09" in F -H L/999
LL Defl -0.04" in F -H L/999
Shear // Grain in B -K 0.29

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

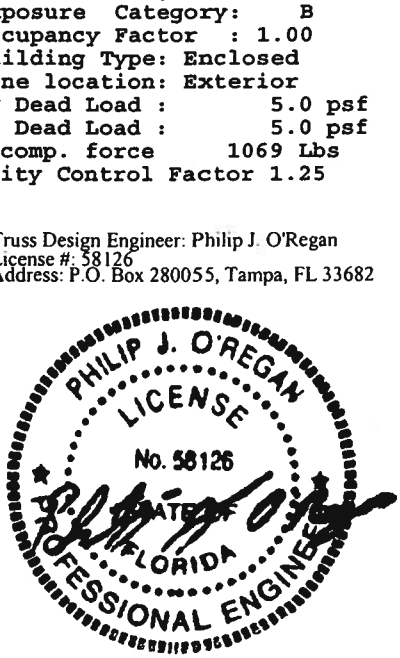
Jt	Type	Plt Size	X	Y	JSI
M	LOCK	3.0x 4.0	Ctr	Ctr	0.79
J	LOCK	3.0x 4.0	Ctr	Ctr	0.84
B	LOCK	4.0x 8.0	2.5	-3.4	0.72
K	LOCK	1.0x 3.0	Ctr	Ctr	0.81
C	LOCK	4.0x 8.0	-2.5	-3.4	0.72
L	LOCK	3.0x 4.0	Ctr	Ctr	0.84
N	LOCK	3.0x 4.0	Ctr	Ctr	0.79
A	LOCK	1.0x 3.0	Ctr	Ctr	0.81
I	LOCK	3.0x 4.0	Ctr	Ctr	0.59
F	LOCK	3.0x 4.0	Ctr	Ctr	0.59
H	LOCK	5.0x 7.0	Ctr	-0.5	0.66
E	LOCK	3.0x 4.0	Ctr	Ctr	0.59
G	LOCK	3.0x 4.0	Ctr	Ctr	0.59
D	LOCK	1.0x 3.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1069 Lbs
Quality Control Factor 1.25

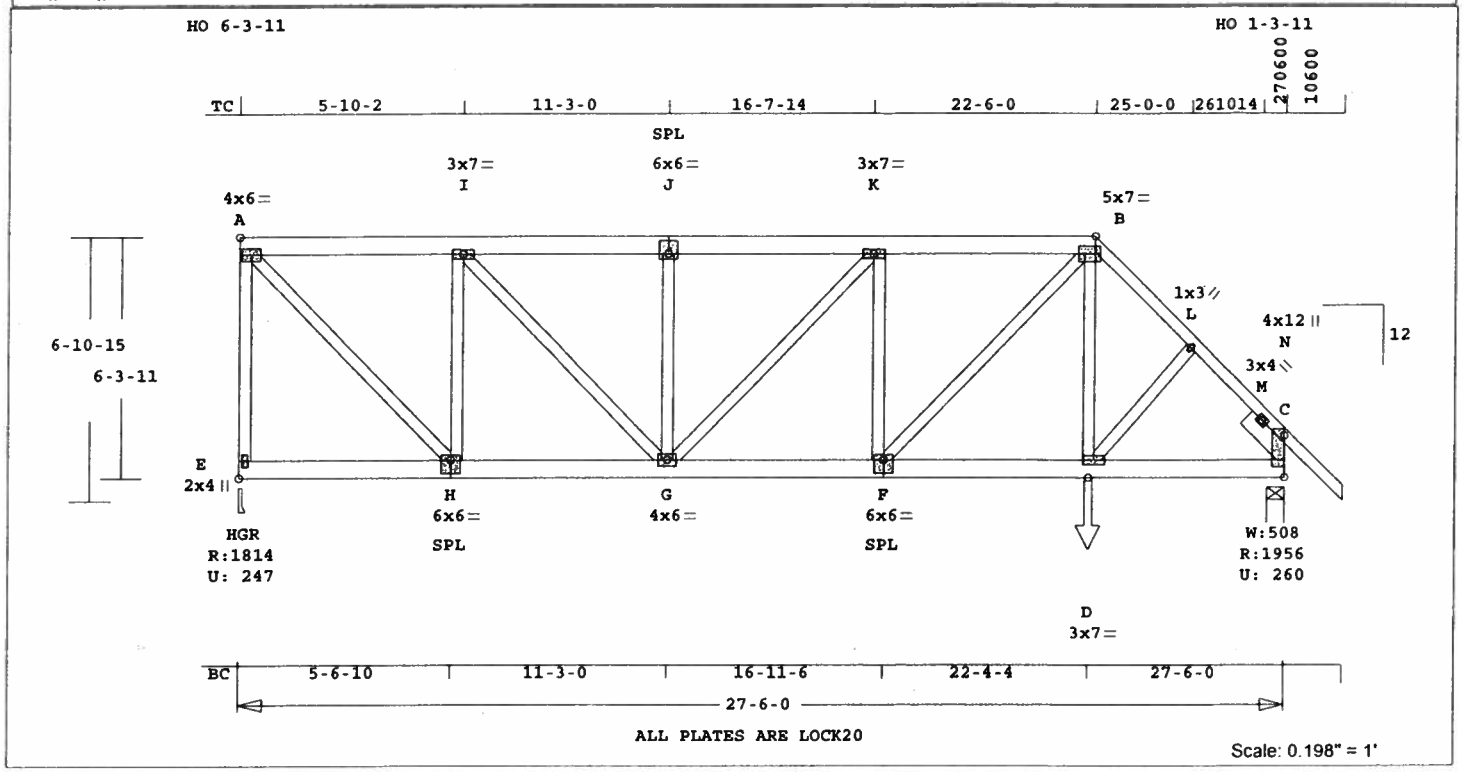
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ2	A5	1*2P	HHIP	270600	12	0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 279.0 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.15	2x 6	SP-#2
EX B -N		2x 4	SP-#2
BC	0.39	2x 6	SP-#2
WB	0.20	2x 4	SP-#2
SL	0.03	2x 6	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	27- 6- 0
BC Cont.	0- 0- 0	27- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	27.5'
BC V	0	20	0.0'	27.5'
TC V	30	15	1.0'	22.5'
TC V	-40	-20	0.0'	1.0'
BC V	0	15	1.0'	22.4'
BC V	0	-20	0.0'	1.0'
BC V	133	133	22.4'	CL-LB

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
E	1814	248	3- 8	1- 8
			Hz =	-194
C	1956	260	5- 8	1- 8
			Hz =	119

Membr	CSI	P Lbs	Ax	l-CSI-Bnd
Top Chords				
A -I	0.15	1523	C	0.00 0.15
I -J	0.15	2229	C	0.00 0.15
J -K	0.14	2229	C	0.00 0.14
K -B	0.14	2249	C	0.00 0.14
B -L	0.18	1890	C	0.00 0.18
L -M	0.38	1999	C	0.00 0.38
M -N	0.38	936	C	0.00 0.38
Bottom Chords				
E -H	0.08	146	T	0.00 0.08
H -G	0.14	1523	T	0.10 0.04
G -F	0.21	2249	T	0.15 0.06
F -D	0.18	1394	T	0.09 0.09
D -N	0.39	1264	T	0.08 0.31
Webs				
E -A	0.11	1760	C	WindLd
A -H	0.20	2213	T	
H -I	0.09	1387	C	
I -G	0.09	1025	T	
G -J	0.03	555	C	
G -K	0.00	35	C	
F -K	0.04	616	C	
F -B	0.11	1225	T	
D -B	0.01	206	T	
D -L	0.01	204	T	
Sliders				
M -N	0.03	1259	C	

TL Defl -0.07" in F -D L/999
LL Defl -0.04" in G -F L/999
Shear // Grain in M -N 0.22

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	4.0x 6.0	Ctr	Ctr	0.76
I	LOCK	3.0x 7.0	Ctr	Ctr	0.43
J	LOCK	6.0x 6.0	Ctr	1.2	0.53
K	LOCK	3.0x 7.0	Ctr	Ctr	0.43
B	LOCK	5.0x 7.0	Ctr	Ctr	0.90
L	LOCK	1.0x 3.0	Ctr	Ctr	0.79
M	LOCK	3.0x 4.0	Ctr	Ctr	0.73
N	LOCK	4.0x12.0	-2.0	1.0	0.96
E	LOCK	2.0x 4.0	Ctr	Ctr	0.65
H	LOCK	6.0x 6.0	Ctr	-1.2	0.53
G	LOCK	4.0x 6.0	Ctr	Ctr	0.52
F	LOCK	6.0x 6.0	Ctr	-1.2	0.53
D	LOCK	3.0x 7.0	Ctr	Ctr	0.44

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder Half Hip
Framing King Jacks
Jack Open Faced
Setback 5- 0- 0
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	----Spacing (In)----		
Rows	Nails	Screws	Bolts
TC 1	12	24	0
BC 2	12	24	0
WB 1	8	8	

OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

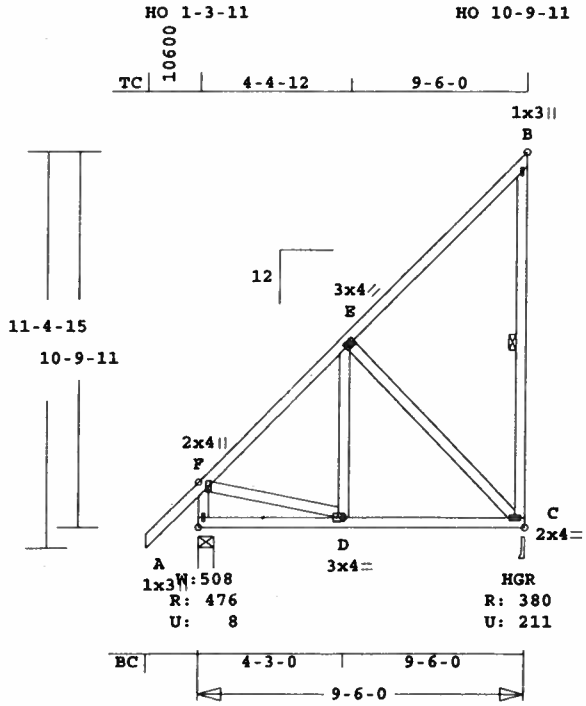
Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A5	1*2P	HHIP	270600	12	0	1- 6- 0	T06080606
U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY								

Max comp. force 2249 Lbs
Quality Control Factor 1.25

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A6	1	MONO	90600	12	1- 6- 0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



ALL PLATES ARE LOCK20

Scale: 0.180" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 98.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 4	SP-#2
BC	0.18	2x 4	SP-#2
WB	0.22	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	9- 6- 0
BC Cont.		0- 0- 0	9- 6- 0

WB 1 rows CLB on C -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	476	8	5- 8	1- 8
			Hz =	-220
C	380	212	3- 8	1- 8
			Hz =	369

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
F -E	0.29	257 C	0.00	0.29
E -B	0.29	173 C	0.00	0.29
--------Bottom Chords---------				
A -D	0.16	212 T	0.00	0.16
D -C	0.18	202 T	0.02	0.16
------------Webs------------				
A -F	0.03	348 C	WindLd	
F -D	0.03	208 T		
D -E	0.02	189 T		
E -C	0.18	287 C		
C -B	0.22	143 T	WindLd 1 Br	

TL Defl -0.04" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in E -B 0.17

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
F	LOCK	2.0x 4.0	Ctr	Ctr	0.80
E	LOCK	3.0x 4.0	Ctr	Ctr	0.56
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	3.0x 4.0	Ctr	Ctr	0.54
C	LOCK	2.0x 4.0	Ctr	Ctr	0.80

Provide connection to bearing for 368 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 348 Lbs
Quality Control Factor 1.25

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

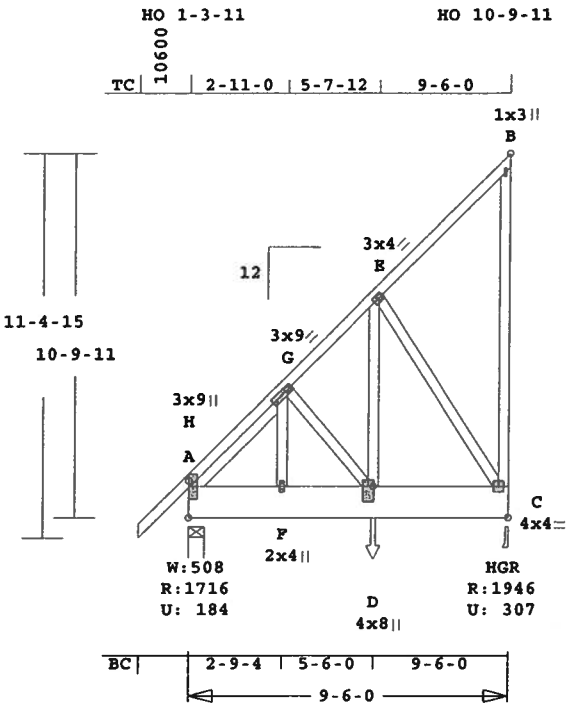
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Scaled: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	A7	1*2P	MONO	90600	12	1- 6- 0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



ALL PLATES ARE LOCK20

Scale: 0.174" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 144.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.08	2x 4	SP-#2
BC	0.14	2x12	SP-#2
WB	0.19	2x 4	SP-#2
SL	0.04	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 6- 0
BC	Cont.	0- 0- 0	9- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	9.5'
BC V	0	20	0.0'	9.5'
BC V	43	43	0.0'	9.5'
BC V	1000	1000	5.5'	CL-LB

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	1717	184	5- 8	1- 8
			Hz =	-200
C	1947	308	3- 8	1- 8
			Hz =	346

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
H -G	0.04	797 C	0.00		0.04
G -E	0.08	1268 C	0.00		0.08
E -B	0.08	155 C	0.00		0.08
--------Bottom Chords---------					
H -F	0.07	998 T	0.04		0.03
F -D	0.14	998 T	0.04		0.10
D -C	0.13	913 T	0.03		0.10
------------Webs------------					
F -G	0.02	333 T			
G -D	0.00	135 C			
D -E	0.19	2175 T			
E -C	0.15	1716 C			
C -B	0.08	126 T	WindLd		
----------Sliders----------					
H -G	0.04	778 C			

TL Defl -0.02" in F -D L/999
LL Defl -0.01" in F -D L/999
Shear // Grain in D -C 0.07

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
H	LOCK	3.0x 9.0	1.0	-0.1	0.81
G	LOCK	3.0x 9.0	Ctr	Ctr	0.53
E	LOCK	3.0x 4.0	-0.4	-0.4	0.76
B	LOCK	1.0x 3.0	Ctr	Ctr	0.75
F	LOCK	2.0x 4.0	Ctr	Ctr	0.38
D	LOCK	4.0x 8.0	Ctr	-1.6	0.56
C	LOCK	4.0x 4.0	Ctr	Ctr	0.83

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	Rows	----Spacing (In)---- Nails	Screws	Bolts
TC	1	12	24	0
BC	3	12	24	0
WB	1	8	8	

Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Provide connection to bearing for 346 Lbs Horiz Reaction
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1716 Lbs
Quality Control Factor 1.25

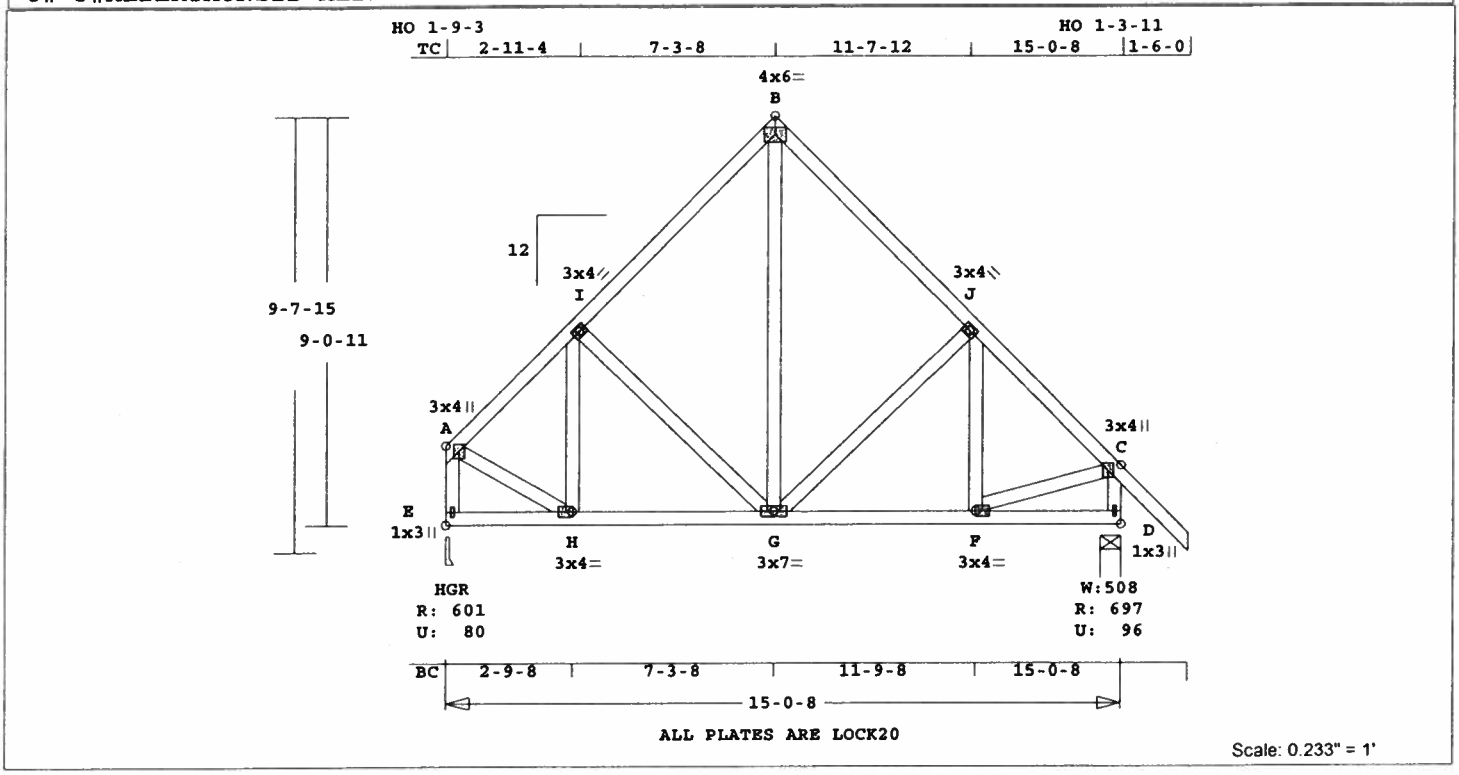
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

PHILIP J. O'REGAN LICENSE No. 58126 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ2	B1	5	TR	150008	12	0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 141.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 4	SP-#2
BC	0.14	2x 4	SP-#2
WB	0.10	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	15- 0- 8
BC Cont.		0- 0- 0	15- 0- 8

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
E	602	80	3- 8	1- 8
			Hz =	-214
D	698	97	5- 8	1- 8
			Hz =	208

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -I	0.15	443 C	0.00	0.15
I -B	0.15	375 C	0.00	0.15
B -J	0.16	376 C	0.00	0.16
J -C	0.16	503 C	0.00	0.16
Bottom Chords				
E -H	0.07	203 T	0.00	0.07
H -G	0.14	329 T	0.03	0.11
G -F	0.14	370 T	0.03	0.11
F -D	0.08	199 T	0.00	0.08
Webs				
E -A	0.05	580 C	WindLd	
A -H	0.07	382 T		
H -I	0.02	117 C		
I -G	0.04	115 T		
G -B	0.10	306 T		
G -J	0.06	147 C		
F -J	0.01	66 T		
F -C	0.07	393 T		
D -C	0.05	575 C	WindLd	

TL Defl -0.02" in H -G L/999
LL Defl -0.01" in H -G L/999
Shear // Grain in I -B 0.12

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.61
I	LOCK	3.0x 4.0	Ctr	Ctr	0.65
B	LOCK	4.0x 6.0	Ctr	-0.1	0.44
J	LOCK	3.0x 4.0	Ctr	Ctr	0.65
C	LOCK	3.0x 4.0	Ctr	Ctr	0.61
E	LOCK	1.0x 3.0	Ctr	Ctr	0.75
H	LOCK	3.0x 4.0	Ctr	Ctr	0.46
G	LOCK	3.0x 7.0	Ctr	Ctr	0.40
F	LOCK	3.0x 4.0	Ctr	Ctr	0.63
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

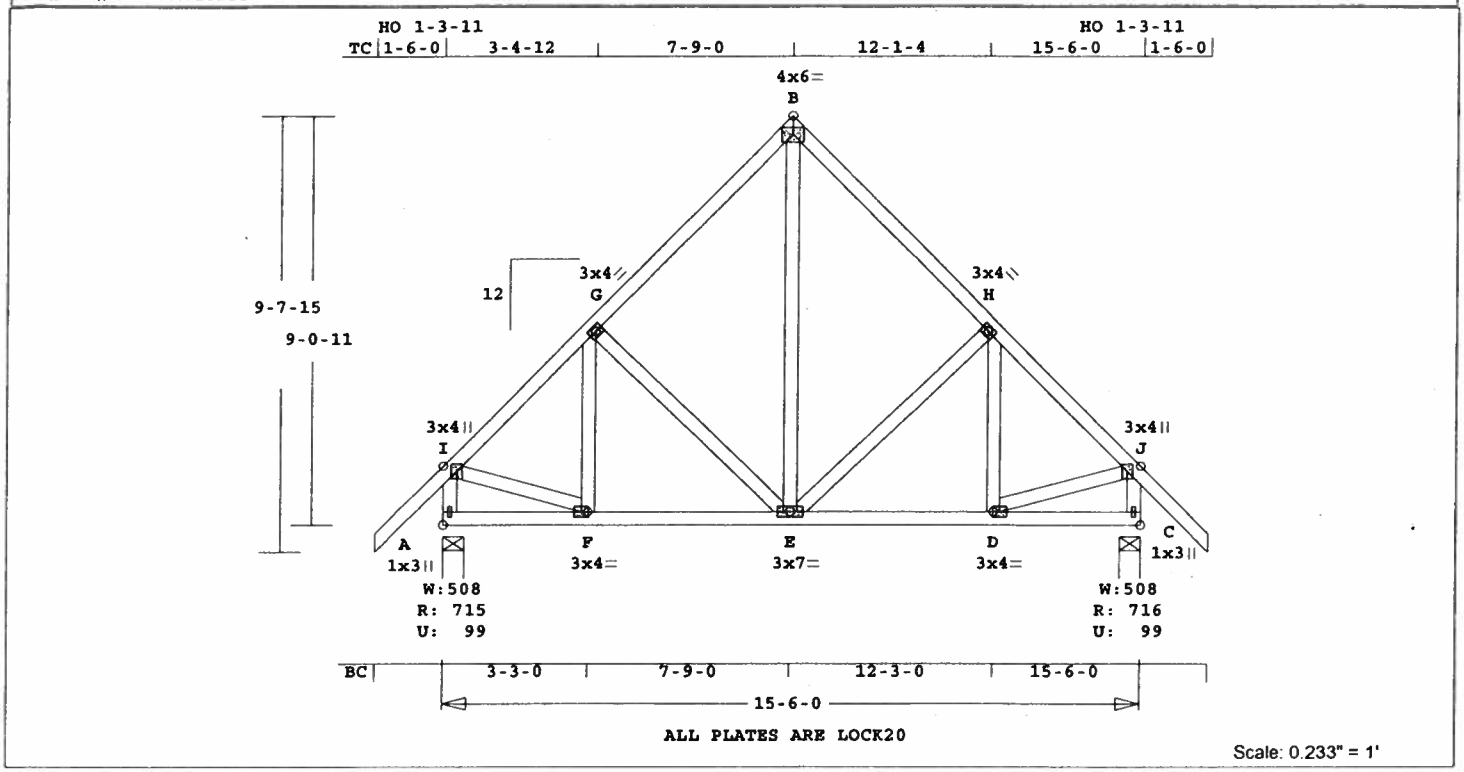
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 580 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	B2	1	TR	150600	12	1- 6- 0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 147.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 4	SP-#2
BC	0.15	2x 4	SP-#2
WB	0.11	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	15- 6- 0
BC Cont.	0- 0- 0	15- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	716	99	5- 8	1- 8
			Hz =	-209
C	716	99	5- 8	1- 8
			Hz =	210

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
I -G	0.16	522 C	0.00	0.16
G -B	0.16	398 C	0.00	0.16
B -H	0.16	398 C	0.00	0.16
H -J	0.16	522 C	0.00	0.16
--------Bottom Chords---------				
A -F	0.08	201 T	0.00	0.08
F -E	0.15	384 T	0.04	0.11
E -D	0.15	384 T	0.04	0.11
D -C	0.08	201 T	0.00	0.08
----------Webs----------				
A -I	0.05	593 C	WindLd	
I -F	0.07	407 T		
F -G	0.01	62 T		
G -E	0.06	144 C		
E -B	0.11	330 T		
E -H	0.06	144 C		
D -H	0.01	62 T		
D -J	0.07	407 T		
C -J	0.05	593 C	WindLd	

TL Defl -0.02" in F -E L/999
LL Defl -0.01" in F -E L/999
Shear // Grain in G -B 0.12

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

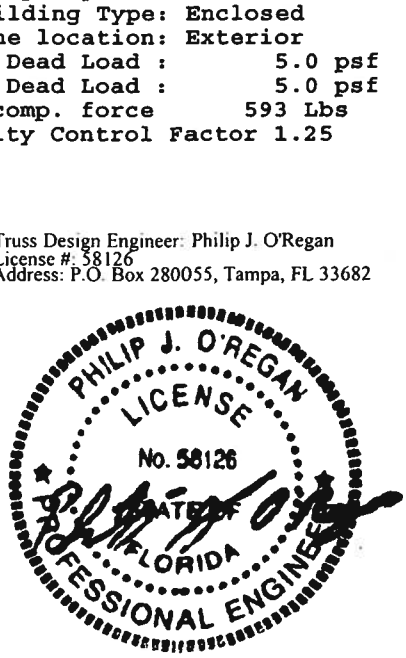
Jt	Type	Plt Size	X	Y	JSI
I	LOCK	3.0x 4.0	Ctr	Ctr	0.62
G	LOCK	3.0x 4.0	Ctr	Ctr	0.66
B	LOCK	4.0x 6.0	Ctr	-0.1	0.44
H	LOCK	3.0x 4.0	Ctr	Ctr	0.66
J	LOCK	3.0x 4.0	Ctr	Ctr	0.62
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75
F	LOCK	3.0x 4.0	Ctr	Ctr	0.64
E	LOCK	3.0x 7.0	Ctr	Ctr	0.40
D	LOCK	3.0x 4.0	Ctr	Ctr	0.64
C	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 593 Lbs
Quality Control Factor 1.25

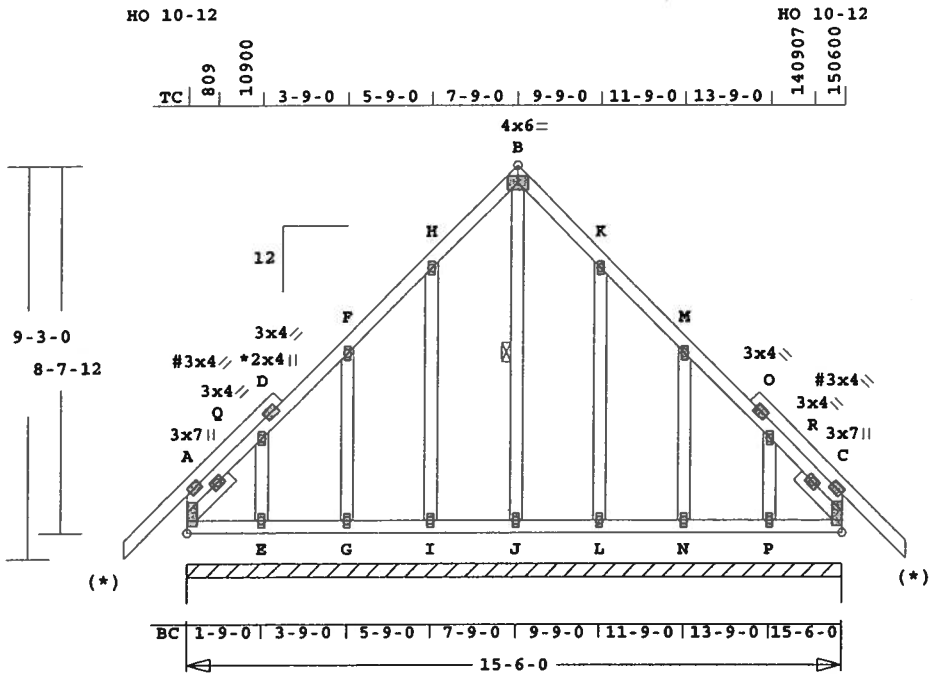
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	B3	1	TR	150600	12	0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.220" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 158.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
(*) TC	0.04	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
GW	0.06	2x 4	SP-#2
SL	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	15- 6- 0
BC Cont.		0- 0- 0	15- 6- 0

WB 1 rows CLB on J -B
Attach CLB with (2)-10d nails at each web.

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 15- 6- 0			
	1240	165	Hz =	178

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -Q	0.04	75	C	0.00	0.04
Q -D	0.04	168	C	0.00	0.04
D -F	0.03	89	C	0.00	0.03
F -H	0.03	66	T	0.00	0.03
H -B	0.03	148	T	0.00	0.03
B -K	0.03	148	T	0.00	0.03
K -M	0.03	66	T	0.00	0.03
M -O	0.03	89	C	0.00	0.03
O -R	0.04	168	C	0.00	0.04
R -C	0.04	75	C	0.00	0.04
--------Bottom Chords---------					
A -E	0.05	3	T	0.00	0.05
E -G	0.02	0	T	0.00	0.02
G -I	0.02	0	T	0.00	0.02
I -J	0.02	0	T	0.00	0.02
J -L	0.02	0	T	0.00	0.02
L -N	0.02	0	T	0.00	0.02
N -P	0.02	0	T	0.00	0.02
P -C	0.05	3	T	0.00	0.05
----------Gable Webs----------					
E -D	0.01	125	T		
G -F	0.03	123	C		
I -H	0.06	122	C		
J -B	0.03	166	C		1 Br
L -K	0.06	122	C		
N -M	0.03	123	C		
P -O	0.01	125	T		
----------Sliders----------					
A -Q	0.00	105	C		
R -C	0.00	105	C		

TL Defl 0.00" in E -G L/999
LL Defl 0.00" in E -G L/999
Shear // Grain in Q -D 0.05

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 7.0	1.5	0.7	0.96
Q	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	4.0x 6.0	Ctr	-0.1	0.44
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
M	LOCK	2.0x 4.0	Ctr	Ctr	0.00
O	LOCK	2.0x 4.0	Ctr	Ctr	0.00
R	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 7.0	-1.5	0.7	0.96
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	2.0x 4.0	Ctr	Ctr	0.00
P	LOCK	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.

PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

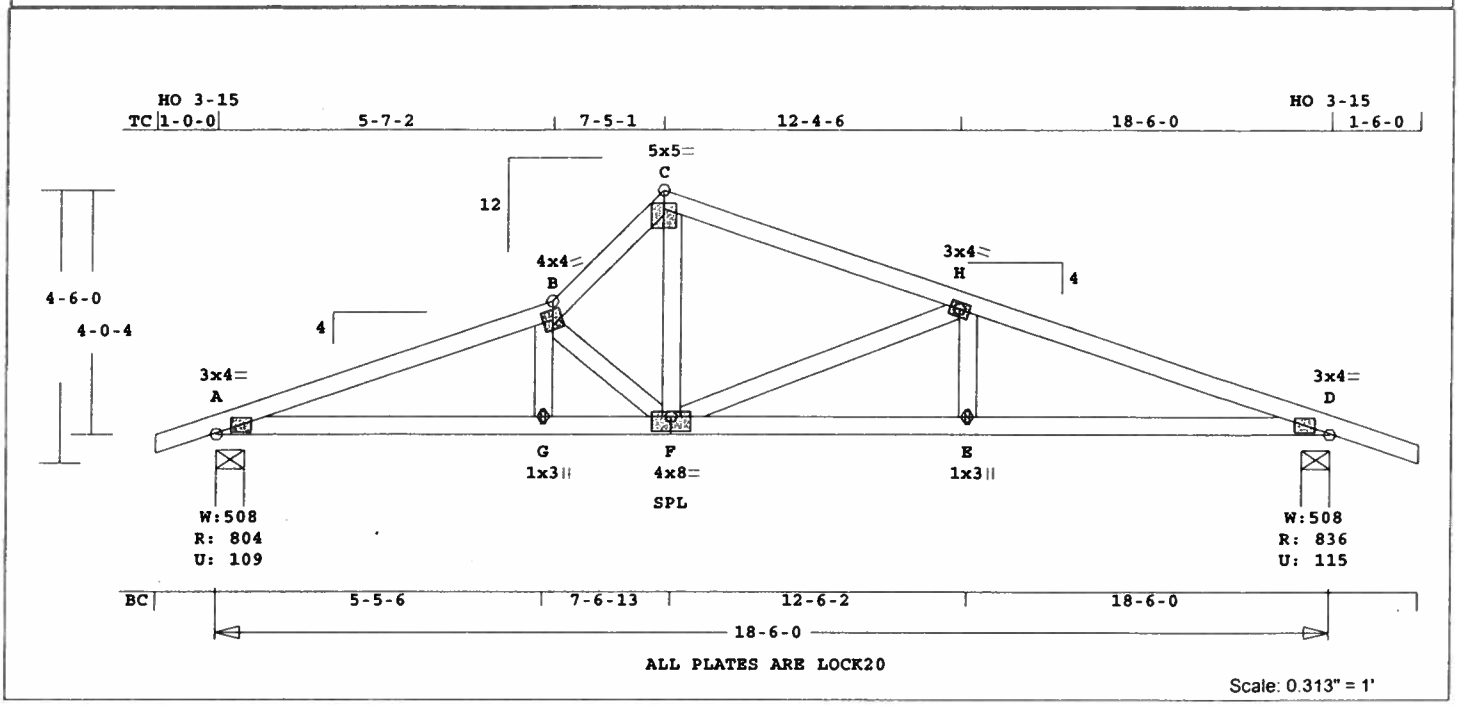
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 168 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ2	C1	5	SP	180600	4	1- 0- 0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 105.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.28	2x 4	SP-#2
BC	0.38	2x 4	SP-#2
WB	0.24	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18- 6- 0
BC Cont.	0- 0- 0	18- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	804	110	5- 8	1- 8
			Hz =	-64
D	836	115	5- 8	1- 8
			Hz =	48

Membr	CSI	P Lbs	Axl	CSI-Bnd
Top Chords				
A -B	0.20	1628	C 0.02	0.18
B -C	0.20	1166	C 0.00	0.20
C -H	0.27	943	C 0.00	0.27
H -D	0.28	1599	C 0.01	0.27
Bottom Chords				
A -G	0.38	1549	T 0.26	0.12
G -F	0.29	1546	T 0.25	0.04
F -E	0.33	1523	T 0.25	0.08
E -D	0.38	1523	T 0.25	0.13
Webs				
G -B	0.01	128	T	
B -F	0.10	849	C	
F -C	0.16	867	T	
F -H	0.24	687	C	
E -H	0.03	226	T	

TL Defl -0.13" in F -E L/999
LL Defl -0.06" in F -E L/999
Shear // Grain in H -D 0.22

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.80
B	LOCK	4.0x 4.0	Ctr	Ctr	0.83
C	LOCK	5.0x 5.0	Ctr	-1.3	0.86
H	LOCK	3.0x 4.0	Ctr	Ctr	0.52
D	LOCK	3.0x 4.0	Ctr	Ctr	0.80
G	LOCK	1.0x 3.0	Ctr	Ctr	0.81
F	LOCK	4.0x 8.0	Ctr	-1.0	0.67
E	LOCK	1.0x 3.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

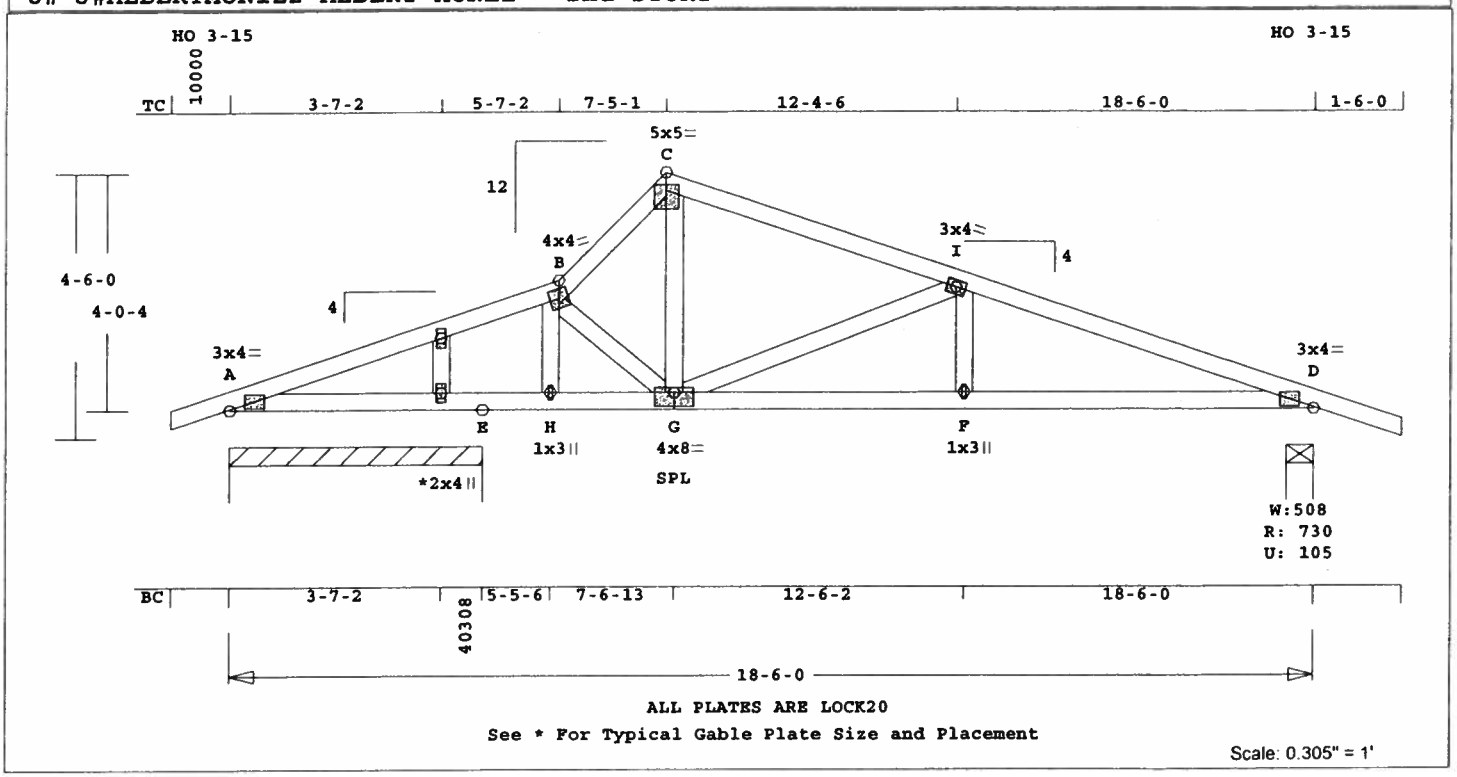
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1628 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ2	C2	2	SP	180600	4	1- 0- 0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 107.4 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.30	2x 4	SP-#2
BC	0.49	2x 4	SP-#2
WB	0.24	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	18- 6- 0
BC Cont.	0- 0- 0	18- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
Cont.	Brg 909	119	0- 0- 0 to Hz =	4- 3- 8 65
D	731	106	5- 8 Hz =	1- 8 48

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -B	0.20	726	C	0.00	0.20
B -C	0.14	782	C	0.00	0.14
C -I	0.29	622	C	0.00	0.29
I -D	0.30	1284	C	0.01	0.29
--------Bottom Chords---------					
A -E	0.38	121	T	0.00	0.38
E -H	0.49	685	T	0.11	0.38
H -G	0.42	705	T	0.11	0.31
G -F	0.29	1226	T	0.20	0.09
F -D	0.34	1226	T	0.20	0.14
------------Webs------------					
H -B	0.03	386	C		
B -G	0.01	158	C		
G -C	0.09	531	T		
G -I	0.24	696	C		
F -I	0.03	225	T		

TL Defl -0.09" in F -D L/999
LL Defl -0.04" in F -D L/999
Shear // Grain in E -H 0.43

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.80
B	LOCK	4.0x 4.0	Ctr	Ctr	0.83
C	LOCK	5.0x 5.0	Ctr	-1.3	0.86
I	LOCK	3.0x 4.0	Ctr	Ctr	0.52
D	LOCK	3.0x 4.0	Ctr	Ctr	0.80
H	LOCK	1.0x 3.0	Ctr	Ctr	0.81
G	LOCK	4.0x 8.0	Ctr	-1.0	0.67
F	LOCK	1.0x 3.0	Ctr	Ctr	0.81

1 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:

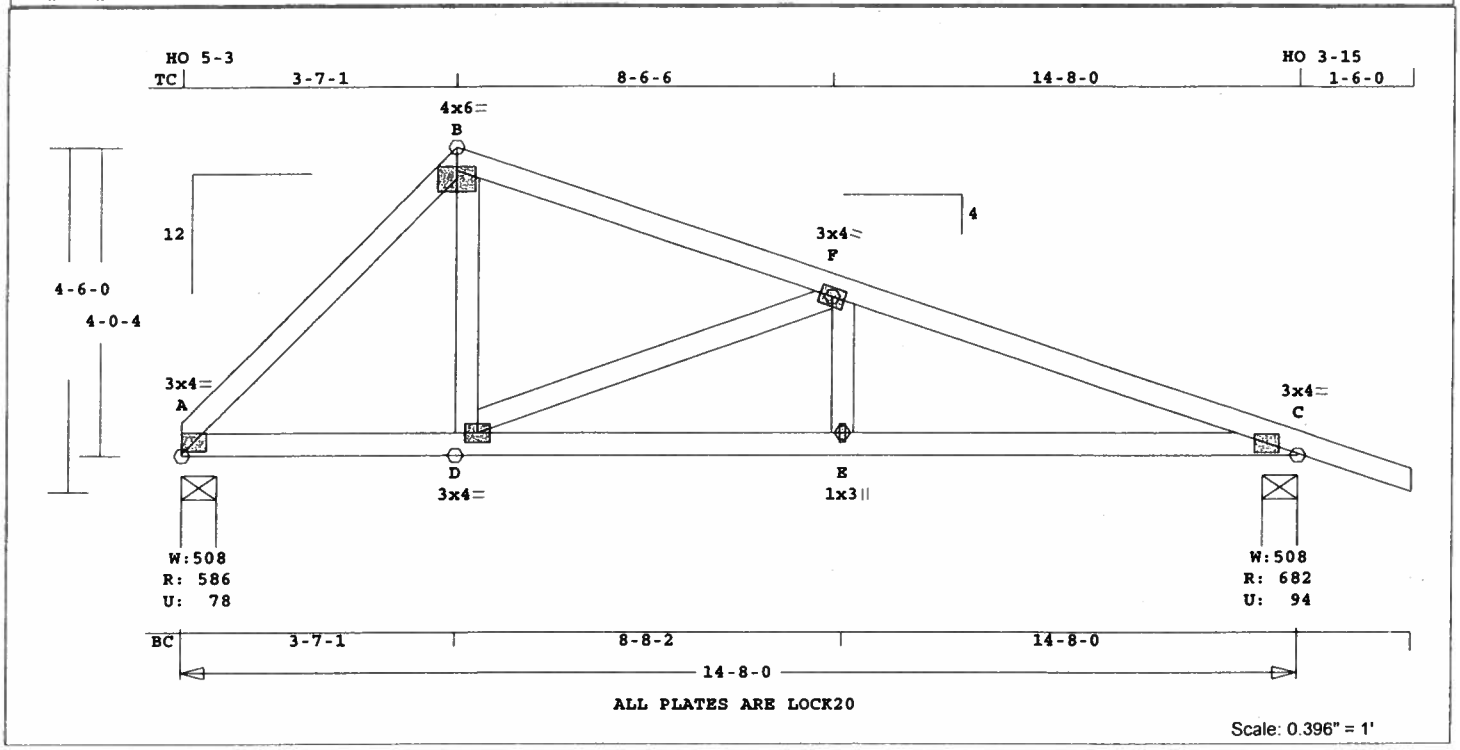
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1284 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-Hl	Left OH	Right OH	Engineering
ALBERTMONIZ2	C3	3	DUAL	140800	4	0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 82.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.29	2x 4	SP-#2
BC	0.32	2x 4	SP-#2
WB	0.24	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	14- 8- 0
BC Cont.	0- 0- 0	14- 8- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	587	78	5- 8	1- 8
			Hz =	-80
C	683	95	5- 8	1- 8
			Hz =	49

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -B	0.11	612	C	0.00	0.11
B -F	0.29	468	C	0.00	0.29
F -C	0.29	1143	C	0.00	0.29
Bottom Chords					
A -D	0.12	435	T	0.07	0.05
D -E	0.28	1092	T	0.18	0.10
E -C	0.32	1092	T	0.18	0.14
Webs					
D -B	0.06	358	T		
D -F	0.24	710	C		
E -F	0.03	231	T		

TL Defl -0.08" in E -C L/999
LL Defl -0.03" in E -C L/999
Shear // Grain in F -C 0.22

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	0.4	0.3	0.62
B	LOCK	4.0x 6.0	Ctr-1.3		0.95
F	LOCK	3.0x 4.0	Ctr	Ctr	0.48
C	LOCK	3.0x 4.0	Ctr	Ctr	0.74
D	LOCK	3.0x 4.0	Ctr	Ctr	0.64
E	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

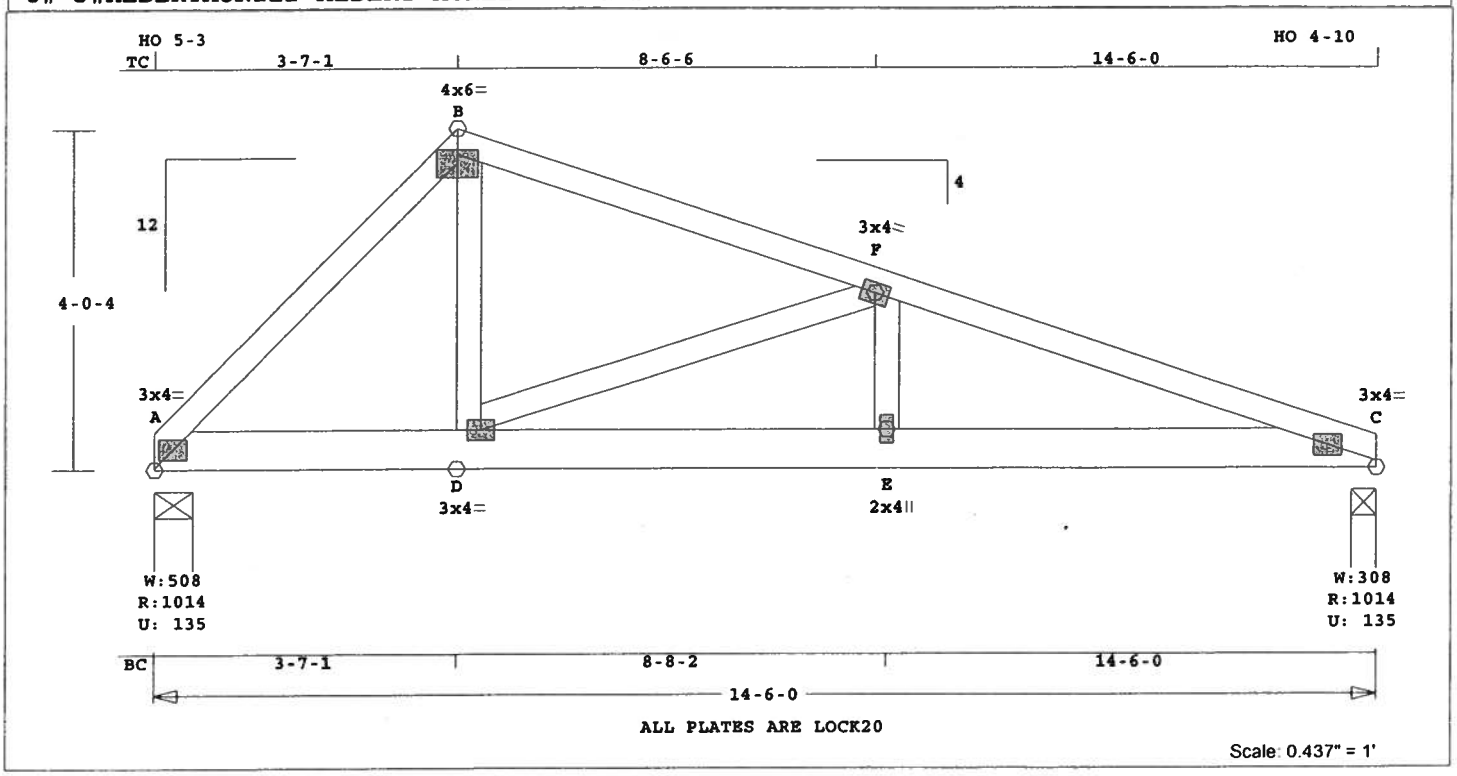
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1143 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	C4	1	DUAL	140600	4	0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 91.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.31	2x 4	SP-#2
BC	0.52	2x 6	SP-#2
WB	0.41	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	14- 6- 0
BC	Cont.	0- 0- 0	14- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	14.5'
BC V	30	50	0.0'	14.5'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Req'd In-Sx
A	1015	135	5- 8	1- 8
			Hz =	-78
C	1015	135	3- 8	1- 8
			Hz =	48

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.21	1077 C	0.00 0.21
B -F	0.29	827 C	0.00 0.29
F -C	0.31	1987 C	0.02 0.29
--------Bottom Chords---------			
A -D	0.19	777 T	0.10 0.09
D -E	0.39	1894 T	0.25 0.14
E -C	0.52	1894 T	0.25 0.27
------------Webs-------------			
D -B	0.14	771 T	
D -F	0.41	1200 C	
E -F	0.08	478 T	

TL Defl -0.09" in E -C L/999
LL Defl -0.05" in E -C L/999
Shear // Grain in B -F 0.21

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.61
B	LOCK	4.0x 6.0	Ctr	-1.3	0.94
F	LOCK	3.0x 4.0	Ctr	Ctr	0.65
C	LOCK	3.0x 4.0	Ctr	Ctr	0.84
D	LOCK	3.0x 4.0	Ctr	Ctr	0.82
E	LOCK	2.0x 4.0	Ctr	Ctr	0.51

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.

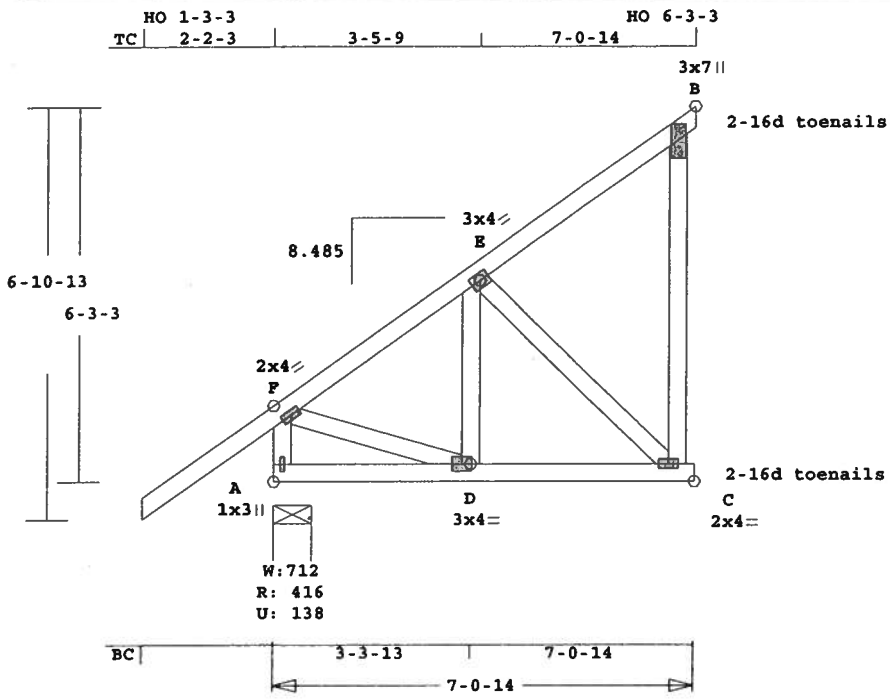
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 5- 0- 0
Design checked for 10 psf non-concurrent LL on BC.
Use properly rated hangers for loads framing into girder truss.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 1987 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	CJ1	1	MONO	70014	8.485	2- 2- 3	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Scale: 0.309" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 66.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.16	2x 4	SP-#2
BC	0.08	2x 4	SP-#2
WB	0.06	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	7- 0-14
BC Cont.	0- 0- 0	7- 0-14

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Load Case # 2 NonStandard Load
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	0	0	0.0'	7.1'
BC V	0	0	0.0'	7.1'
TC V	0	0	0.0'	
	79	40		7.1'
BC V	0	0	0.0'	
	0	40		7.1'

Plus 5 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	416	138	7-12	1- 8
			Hz =	-117
C	209	50	1- 8	1- 8
B	176	15	1- 8	1- 8
			Hz =	205

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
F -E	0.14	186 C	0.00	0.14
E -B	0.16	169 T	0.00	0.16
B -B	0.00	9 C		
Bottom Chords				
A -D	0.07	105 T	0.00	0.07
D -C	0.08	163 T	0.01	0.07
Webs				
A -F	0.02	249 C	WindLd	
F -D	0.03	172 T		
D -E	0.01	121 T		
E -C	0.06	225 C		
C -B	0.06	0 T	WindLd	

TL Defl -0.01" in D -C L/999
LL Defl 0.00" in D -C L/999
Shear // Grain in E -B 0.16

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
F	LOCK	2.0x 4.0	Ctr	Ctr	0.75
E	LOCK	3.0x 4.0	Ctr	Ctr	0.46
B	LOCK	3.0x 7.0	Ctr	-0.3	0.36
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75
D	LOCK	3.0x 4.0	Ctr	Ctr	0.38
C	LOCK	2.0x 4.0	Ctr	Ctr	0.75

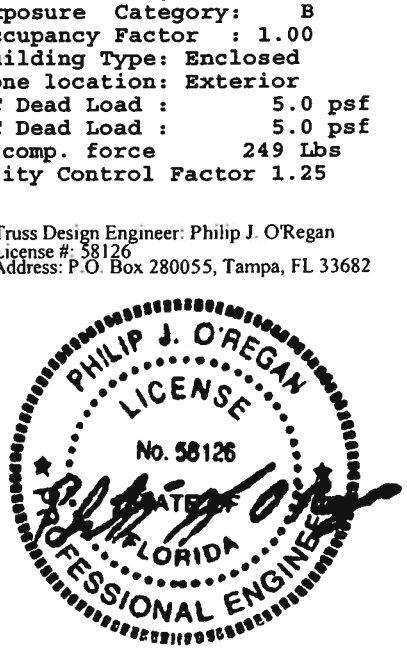
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Max gap between edge of brg and end vertical is 1/2".
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 249 Lbs
Quality Control Factor 1.25

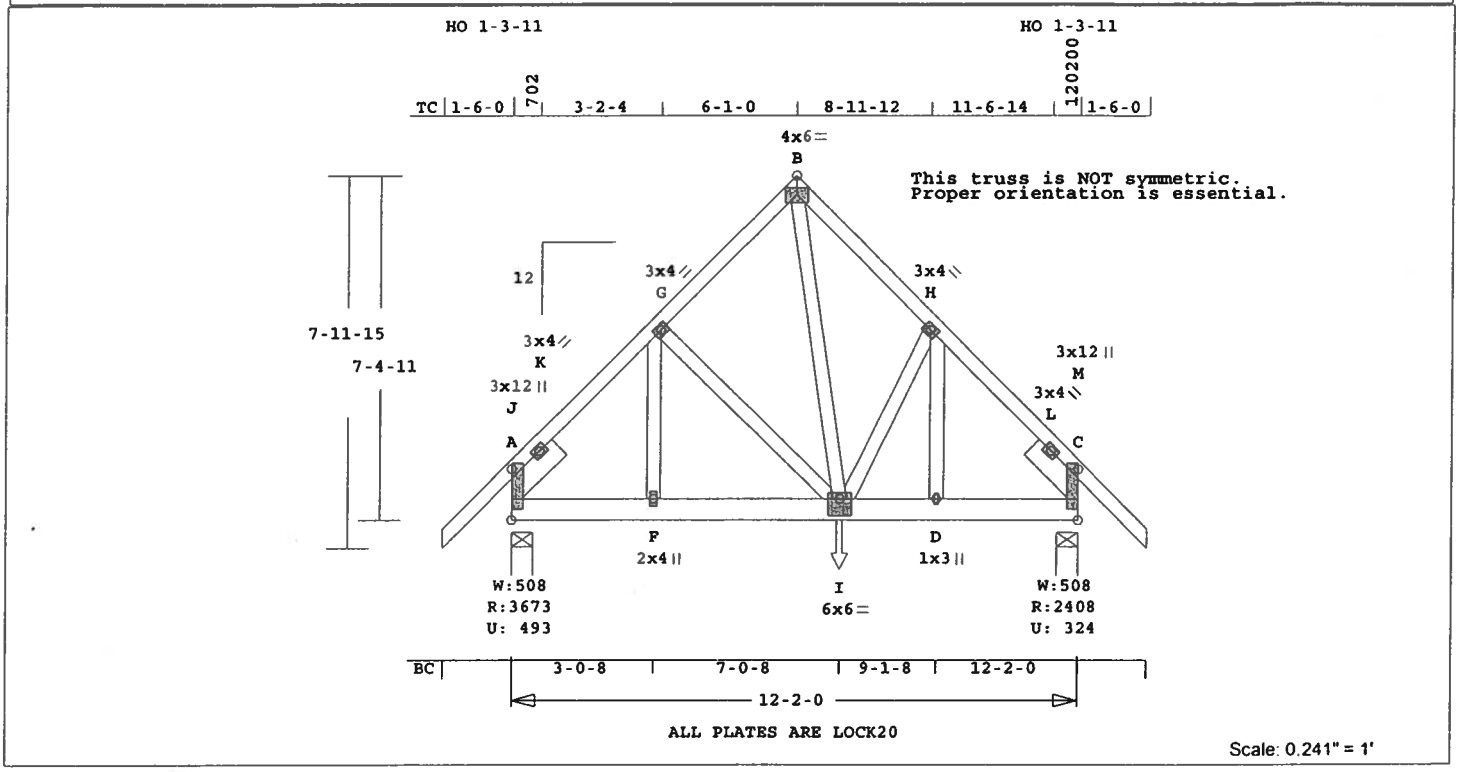
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682



Date Scaled: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	F1	1*2P	TR	120200	12	1- 6- 0	1- 6- 0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 128.3 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.35	2x 4	SP-#2
BC	0.81	2x 6	SP-#2
WB	0.27	2x 4	SP-#2
SL	0.03	2x 6	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 2- 0
BC	Cont.	0- 0- 0	12- 2- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.00 Fc=1.00 Ft=1.00
BC Fb=1.00 Fc=1.00 Ft=1.00

Load Case # 1 Standard Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	12.2'
BC V	0	20	0.0'	12.2'
BC V	255	255	0.0'	6.1'
BC V	907	907	7.0'	CL-LB

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	3674	493	5- 8	2- 3
		Hz =	-137	
C	2408	325	5- 8	1- 8
		Hz =	138	

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
J -K	0.35	2178	C	0.01	0.34
K -G	0.35	2999	C	0.01	0.34
G -B	0.15	1938	C	0.01	0.14
B -H	0.15	2528	C	0.01	0.14
H -L	0.32	2286	C	0.01	0.31
L -M	0.31	1500	C	0.00	0.31
--------Bottom Chords---------					
J -F	0.81	2027	T	0.13	0.68
F -I	0.48	2027	T	0.13	0.35
I -D	0.29	1527	T	0.10	0.19
D -M	0.59	1527	T	0.10	0.49
----------Webs----------					
F -G	0.12	1407	T		
G -I	0.05	870	C		
B -I	0.27	2936	T		
I -H	0.06	703	T		
D -H	0.04	787	C		
----------Sliders----------					
J -K	0.03	1001	C		
L -M	0.02	960	C		

TL Defl -0.08" in F -I L/999
LL Defl -0.04" in F -I L/999
Shear // Grain in A -F 0.48

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
J	LOCK	3.0x12.0	1.5	0.7	0.67
K	LOCK	3.0x 4.0	Ctr	Ctr	0.53
G	LOCK	3.0x 4.0	Ctr	Ctr	0.66
B	LOCK	4.0x 6.0	Ctr	-0.1	0.81
H	LOCK	3.0x 4.0	Ctr	Ctr	0.60
L	LOCK	3.0x 4.0	Ctr	Ctr	0.53
M	LOCK	3.0x12.0	-1.5	0.7	0.67
F	LOCK	2.0x 4.0	Ctr	Ctr	0.76
I	LOCK	6.0x 6.0	Ctr	-1.4	0.57
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	Rows	Nails	----Spacing (In)---- Screws	Bolts
TC	1	12	24	0
BC	2	12	24	0
WB	1	8	8	

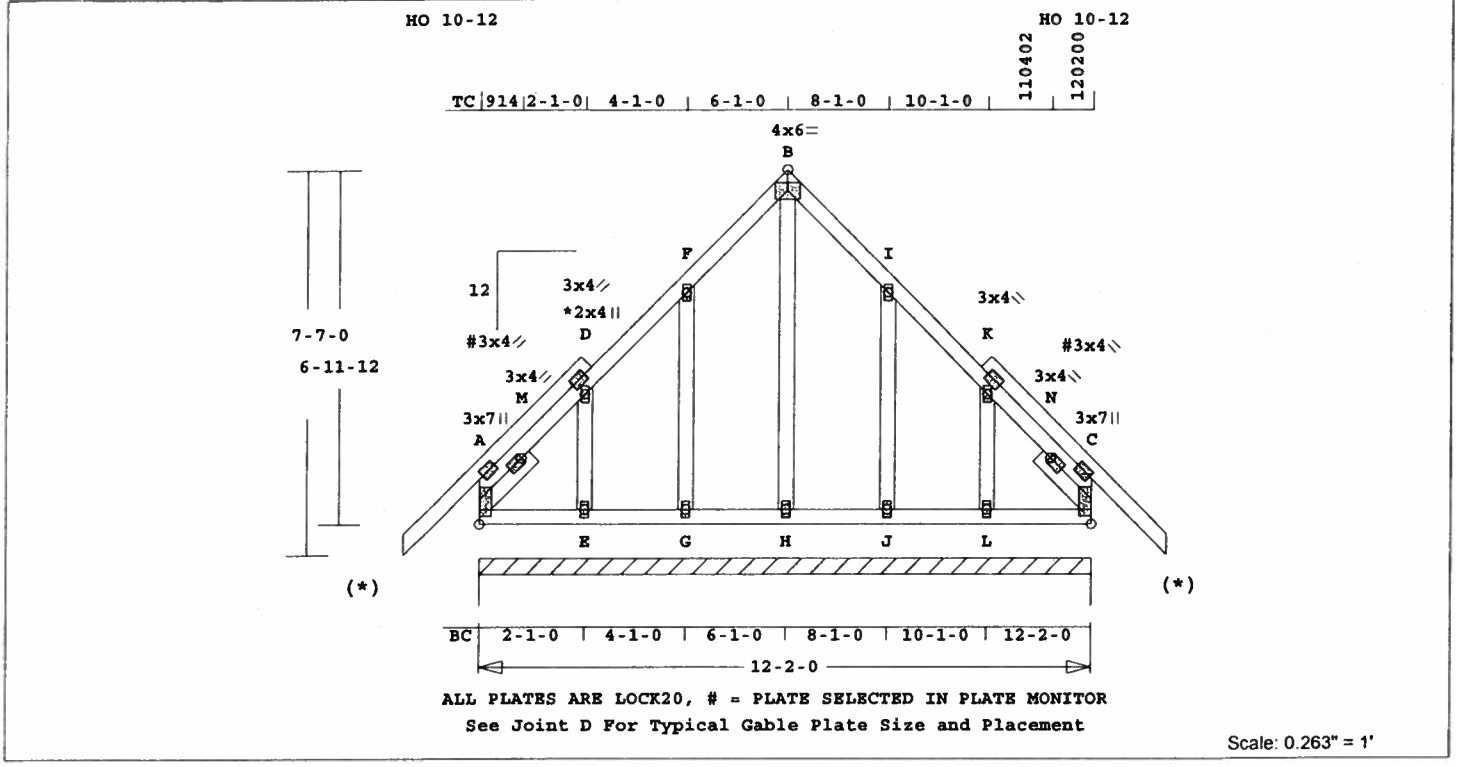
Plus clusters of nails where shown.
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 2999 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	F2	1	SP	120200	12	0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



ALL PLATES ARE LOCK20, # = PLATE SELECTED IN PLATE MONITOR
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.263" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 119.8 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	Size	Lumber
(*) TC	0.03	2x 4	SP-#2
BC	0.04	2x 4	SP-#2
GW	0.08	2x 4	SP-#2
SL	0.00	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	12- 2- 0
BC Cont.		0- 0- 0	12- 2- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to 12- 2- 0			
	973	130	Hz =	138

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -M	0.03	57	C	0.00	0.03
M -D	0.03	109	C	0.00	0.03
D -F	0.03	57	T	0.00	0.03
F -B	0.03	138	T	0.00	0.03
B -I	0.03	138	T	0.00	0.03
I -K	0.03	57	T	0.00	0.03
K -N	0.03	109	C	0.00	0.03
N -C	0.03	57	C	0.00	0.03
Bottom Chords					
A -E	0.04	2	T	0.00	0.04
E -G	0.02	0	T	0.00	0.02
G -H	0.02	0	T	0.00	0.02
H -J	0.02	0	T	0.00	0.02
J -L	0.02	0	T	0.00	0.02
L -C	0.04	2	T	0.00	0.04
Gable Webs					
E -D	0.01	121	T		
G -F	0.03	123	C		
H -B	0.08	152	C		
J -I	0.03	123	C		
L -K	0.01	121	T		
Sliders					
A -M	0.00	63	C		
N -C	0.00	63	C		

TL Defl 0.00" in A -E L/999
LL Defl 0.00" in A -E L/999
Shear // Grain in M -D 0.05

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 7.0	1.5	0.7	0.88
M	LOCK	3.0x 4.0	Ctr	Ctr	0.50
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00
F	LOCK	2.0x 4.0	Ctr	Ctr	0.00
B	LOCK	4.0x 6.0	Ctr	-0.1	0.41
I	LOCK	2.0x 4.0	Ctr	Ctr	0.00
K	LOCK	2.0x 4.0	Ctr	Ctr	0.00
N	LOCK	3.0x 4.0	Ctr	Ctr	0.50
C	LOCK	3.0x 7.0	-1.5	0.7	0.88
E	LOCK	2.0x 4.0	Ctr	Ctr	0.00
G	LOCK	2.0x 4.0	Ctr	Ctr	0.00
H	LOCK	2.0x 4.0	Ctr	Ctr	0.00
J	LOCK	2.0x 4.0	Ctr	Ctr	0.00
L	LOCK	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

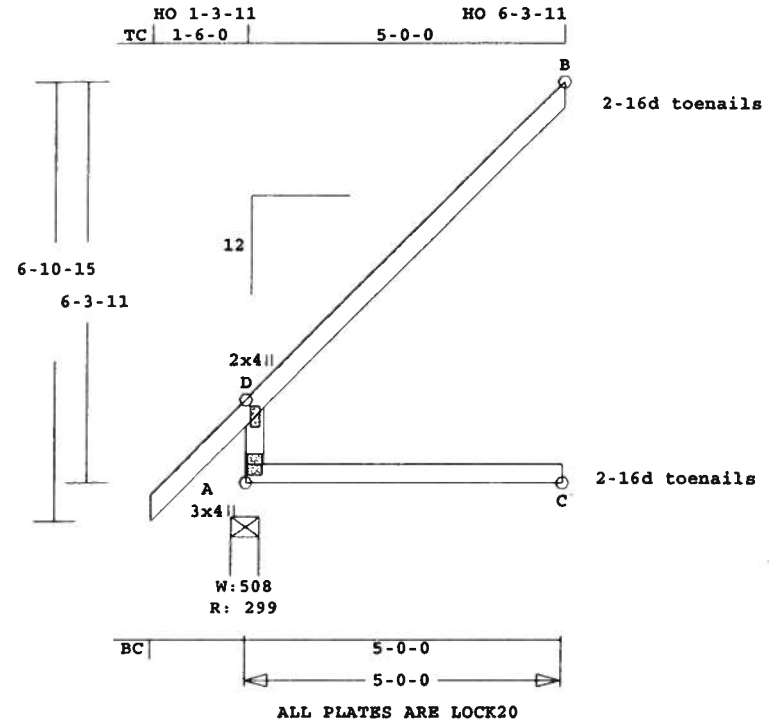
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 152 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1rH1	Left OH	Right OH	Engineering
ALBERTMONIZ2	J1	7	JCA2	50000	12	1- 6- 0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Scale: 0.329" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 29.1 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.26	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.28	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 0
BC Cont.	0- 0- 0	5- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	
A	300	0	5- 8	1- 8
			Hz =	172
C	94	13	1- 8	1- 8
			Hz =	119
B	131	123	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.26	103 C	0.00 0.26
---------Bottom Chords---------			
A -C	0.26	119 T	0.00 0.26
------------Webs------------			
A -D	0.28	164 C	0.00 0.28

TL Defl -0.05" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in D -B 0.14

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.38

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 164 Lbs
Quality Control Factor 1.25

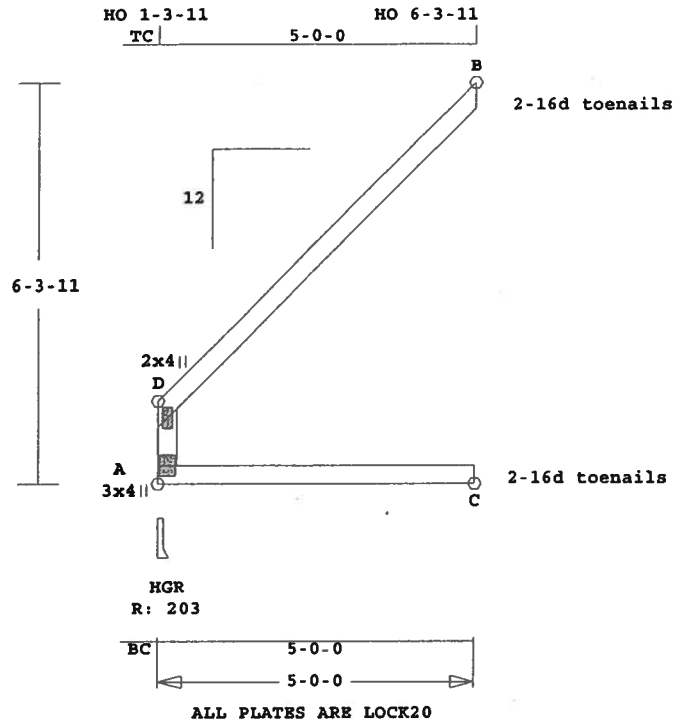
Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

PHILIP J. O'REGAN LICENSE No. 58126 STATE OF FLORIDA PROFESSIONAL ENGINEER

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	J1A	5	JCA2	50000	12	0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Scale: 0.329" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 25.0 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.26	2x 4	SP-#2
BC	0.26	2x 4	SP-#2
WB	0.28	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	5- 0- 0
BC Cont.	0- 0- 0	5- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	204	0	3- 8	1- 8
			Hz =	172
C	94	13	1- 8	1- 8
			Hz =	119
B	131	123	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.26	103 C	0.00 0.26
--------Bottom Chords---------			
A -C	0.26	119 T	0.00 0.26
-------------Webs-------------			
A -D	0.28	164 C	0.00 0.28

TL Defl -0.05" in A -C L/999
LL Defl -0.02" in A -C L/999
Shear // Grain in D -B 0.14

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	2.0x 4.0	Ctr	Ctr	0.38
A	LOCK	3.0x 4.0	Ctr	Ctr	0.38

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

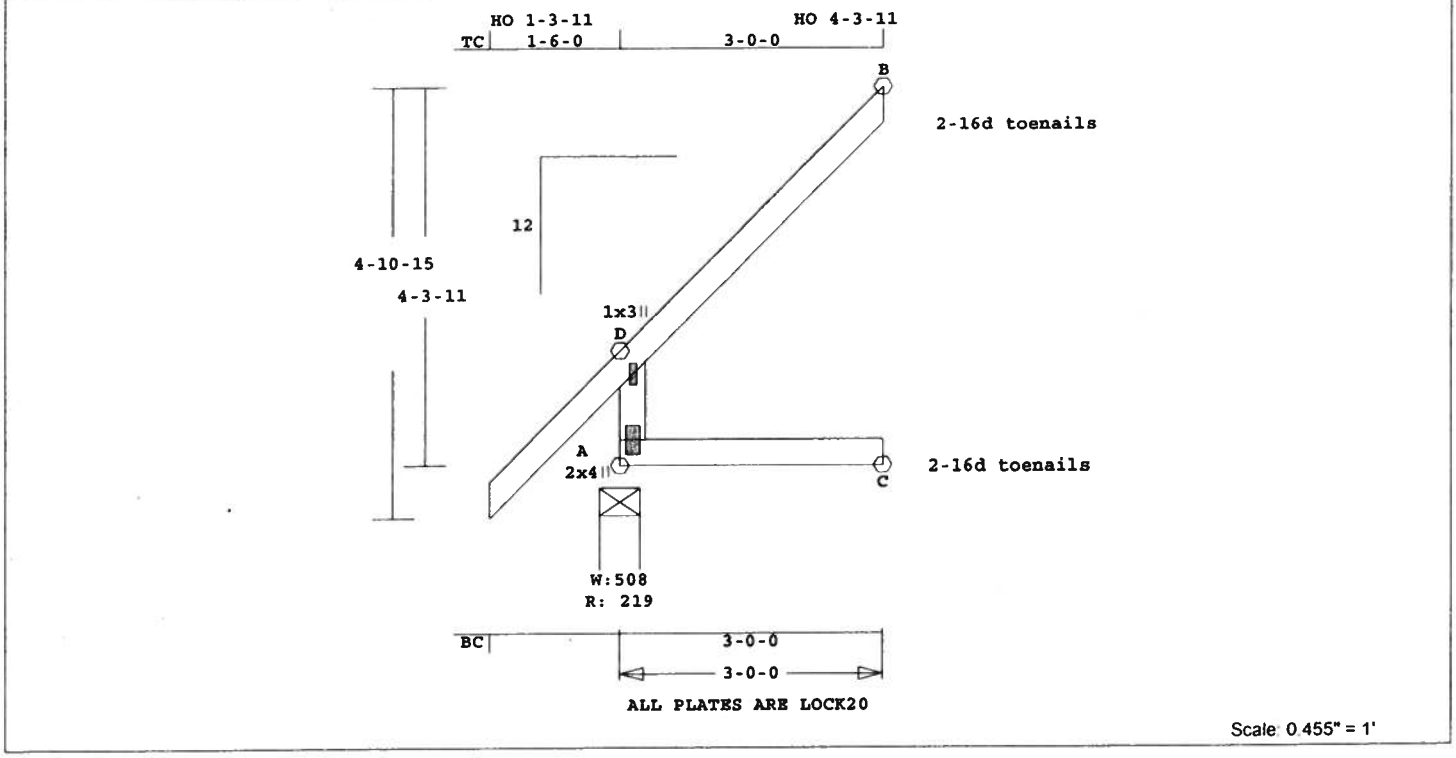
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 164 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	J2	1	JCA2	30000	12	1- 6- 0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 19.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.09	2x 4	SP-#2
BC	0.12	2x 4	SP-#2
WB	0.13	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	3- 0- 0
BC Cont.	0- 0- 0	3- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25

TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	220	0	5- 8	1- 8
			Hz =	111
C	55	16	1- 8	1- 8
			Hz =	78
B	77	81	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.09	66 C	0.00 0.09
--------Bottom Chords---------			
A -C	0.12	77 T	0.00 0.12
------------Webs------------			
A -D	0.13	98 C	0.00 0.13

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.08

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

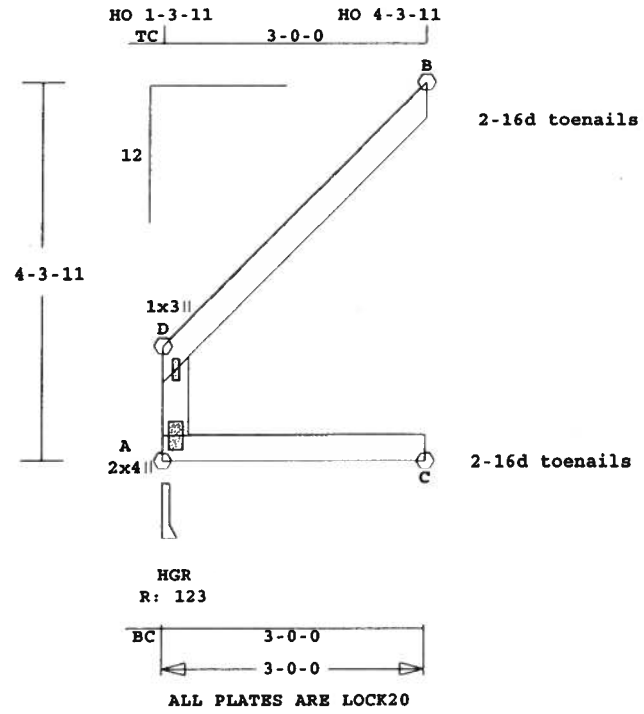
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 98 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	J2A	1	JCA2	30000	12	0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Scale: 0.455" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.9 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.09	2x 4	SP-#2
BC	0.12	2x 4	SP-#2
WB	0.13	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 0- 0
BC	Cont.	0- 0- 0	3- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size In-Sx	Req'd In-Sx
A	124	0	3- 8	1- 8
			Hz =	111
C	55	16	1- 8	1- 8
			Hz =	78
B	77	81	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
--------Top Chords--------			
D -B	0.09	66 C	0.00 0.09
--------Bottom Chords--------			
A -C	0.12	77 T	0.00 0.12
----------Webs----------			
A -D	0.13	98 C	0.00 0.13

TL Defl -0.01" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.08

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	2.0x 4.0	Ctr	Ctr	0.38

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

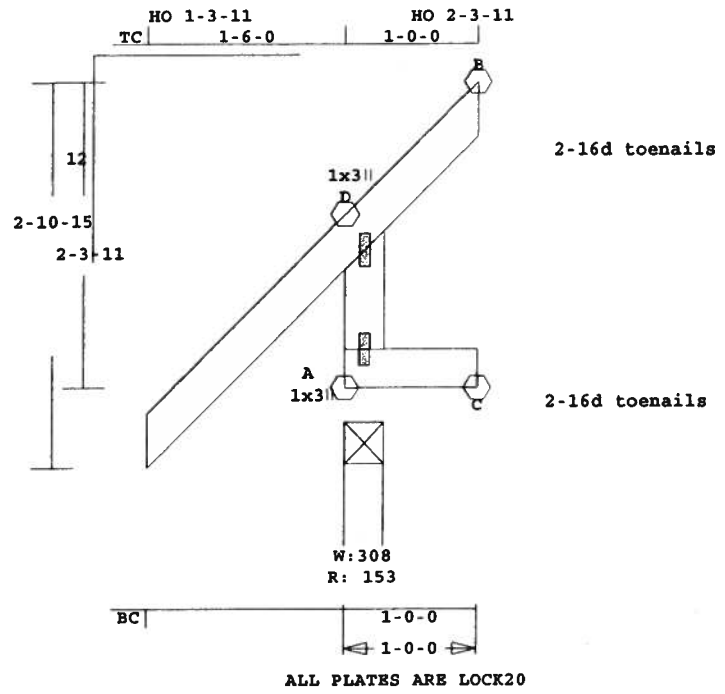
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 98 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1	H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	J3	1	JCA2	10000	12		1- 6- 0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Scale: 0.686" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 10.8 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.02	2x 4	SP-#2
BC	0.03	2x 4	SP-#2
WB	0.03	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 0
BC Cont.	0- 0- 0	1- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	154	0	3- 8	1- 8
			Hz =	50
C	15	25	1- 8	1- 8
			Hz =	36
B	22	42	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.02	32 C	0.00 0.02
--------Bottom Chords---------			
A -C	0.03	35 T	0.00 0.03
------------Webs------------			
A -D	0.03	33 C	0.00 0.03

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.03

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

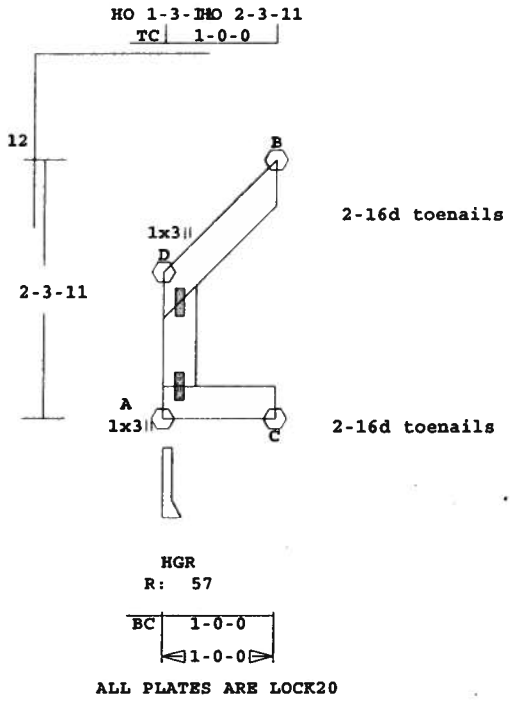
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 33 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	J3A	1	JCA2	10000	12	0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Scale: 0.580" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 6.8 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.02	2x 4	SP-#2
BC	0.03	2x 4	SP-#2
WB	0.03	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	1- 0- 0
BC Cont.	0- 0- 0	1- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	58	0	3- 8	1- 8
			Hz =	50
C	15	25	1- 8	1- 8
			Hz =	36
B	22	42	1- 8	1- 8

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
D -B	0.02	32 C	0.00 0.02
--------Bottom Chords---------			
A -C	0.03	35 T	0.00 0.03
----------Webs----------			
A -D	0.03	33 C	0.00 0.03

TL Defl 0.00" in A -C L/999
LL Defl 0.00" in A -C L/999
Shear // Grain in D -B 0.03

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75
A	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

For proper installation of toe-nails, refer to the 2001 National Design Specification (NDS) for Wood Construction

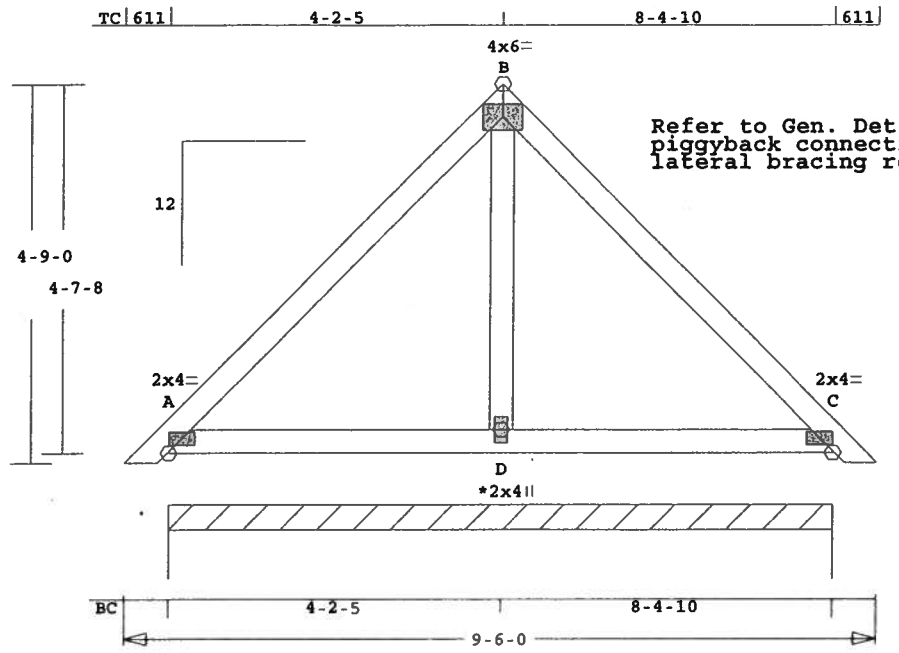
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 33 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	P1	4	TR	90600	12	6-11	6-11	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Refer to Gen. Det. 12 series for piggyback connection and top chord lateral bracing requirements.

ALL PLATES ARE LOCK20

See Joint D For Typical Gable Plate Size and Placement

Scale: 0.419" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 48.6 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.11	2x 4	SP-#2
BC	0.11	2x 4	SP-#2
GW	0.02	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	9- 6- 0
BC	Cont.	0- 0- 0	9- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg	0- 0- 0	to	8- 4-10
	742	102	Hz =	94

Membr	CSI	P Lbs	Axl-CSI-Bnd
----------Top Chords----------			
A -B	0.11	212 C	0.00 0.11
B -C	0.11	212 C	0.00 0.11
--------Bottom Chords---------			
A -D	0.11	0 T	0.00 0.11
D -C	0.11	0 T	0.00 0.11
----------Gable Webs----------			
D -B	0.02	93 C	

TL Defl -0.01" in A -D L/999
LL Defl 0.00" in A -D L/999
Shear // Grain in A -B 0.10

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.4	0.3	0.64
B	LOCK	4.0x 6.0	Ctr	-0.1	0.37
C	LOCK	2.0x 4.0	-0.4	0.3	0.64
D	LOCK	2.0x 4.0	Ctr	Ctr	0.00

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

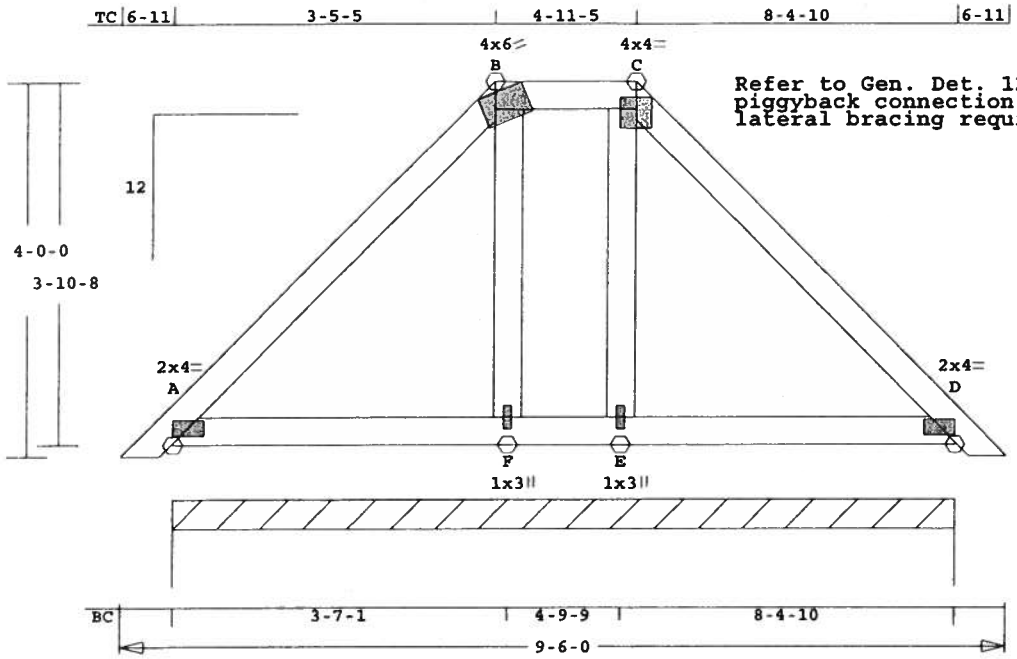
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 212 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-P1	Left OH	Right OH	Engineering
ALBERTMONIZ2	P2	2	HIPP	90600	12	6-11	6-11	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Refer to Gen. Det. 12 series for piggyback connection and top chord lateral bracing requirements.

ALL PLATES ARE LOCK20

Scale: 0.499" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 52.5 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.08	2x 4	SP-#2
BC	0.05	2x 4	SP-#2
WB	0.00	2x 4	SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	9- 6- 0
BC Cont.	0- 0- 0	9- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		8- 4-10	
	742	102	Hz =	78

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.08	228 C	0.00	0.08
B -C	0.03	156 C	0.00	0.03
C -D	0.08	228 C	0.00	0.08
--------Bottom Chords---------				
A -F	0.05	0 T	0.00	0.05
F -E	0.05	0 T	0.00	0.05
E -D	0.05	0 T	0.00	0.05
------------Webs------------				
F -B	0.00	64 T		
E -C	0.00	64 T		

TL Defl -0.01" in E -D L/999
LL Defl 0.00" in E -D L/999
Shear // Grain in A -B 0.09

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.4	0.3	0.64
B	LOCK	4.0x 6.0	1.2-3.0		0.37
C	LOCK	4.0x 4.0	Ctr-0.5		0.86
D	LOCK	2.0x 4.0-0.4	0.3		0.64
F	LOCK	1.0x 3.0	Ctr	Ctr	0.75
E	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.

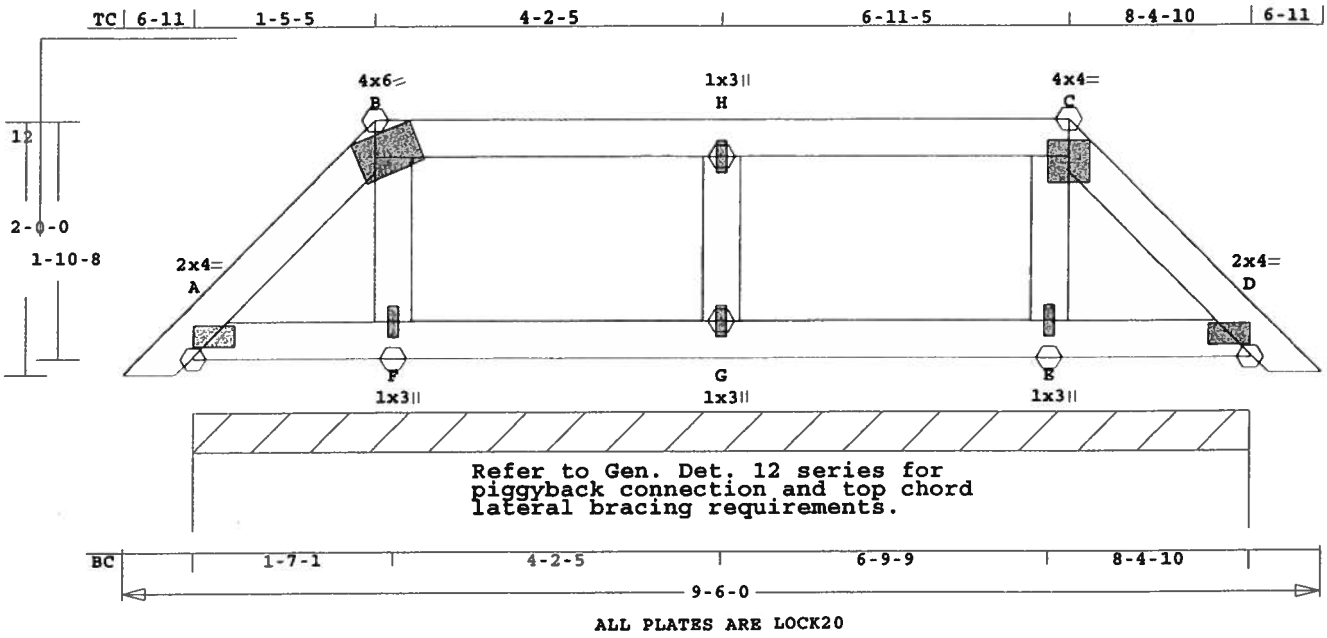
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 228 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	P3	2	HIPP	90600	12	6-11	6-11	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 44.2 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.03	2x 4	SP-#2
WB	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	9- 6- 0
BC Cont.		0- 0- 0	9- 6- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont. Brg	0- 0- 0 to		8- 4-10	
	742	102 Hz =	31	

Membr	CSI	P Lbs	Axl-CSI-Bnd	
Top Chords				
A -B	0.00	52 C		
B -H	0.07	39 T	0.00	0.07
H -C	0.07	39 T	0.00	0.07
C -D	0.00	52 C		
Bottom Chords				
A -F	0.02	0 T	0.00	0.02
F -G	0.03	0 T	0.00	0.03
G -E	0.03	0 T	0.00	0.03
E -D	0.02	0 T	0.00	0.02
Webs				
F -B	0.00	78 C		
G -H	0.01	179 C		
E -C	0.00	78 C		

TL Defl 0.00" in F -G L/999
LL Defl 0.00" in F -G L/999
Shear // Grain in B -H 0.11

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	0.4	0.3	0.64
B	LOCK	4.0x 6.0	1.2	-3.0	0.37
H	LOCK	1.0x 3.0	Ctr	Ctr	0.75
C	LOCK	4.0x 4.0	Ctr	-0.5	0.86
D	LOCK	2.0x 4.0	-0.4	0.3	0.64
F	LOCK	1.0x 3.0	Ctr	Ctr	0.75
G	LOCK	1.0x 3.0	Ctr	Ctr	0.75
E	LOCK	1.0x 3.0	Ctr	Ctr	0.75

Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 179 Lbs
Quality Control Factor 1.25

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

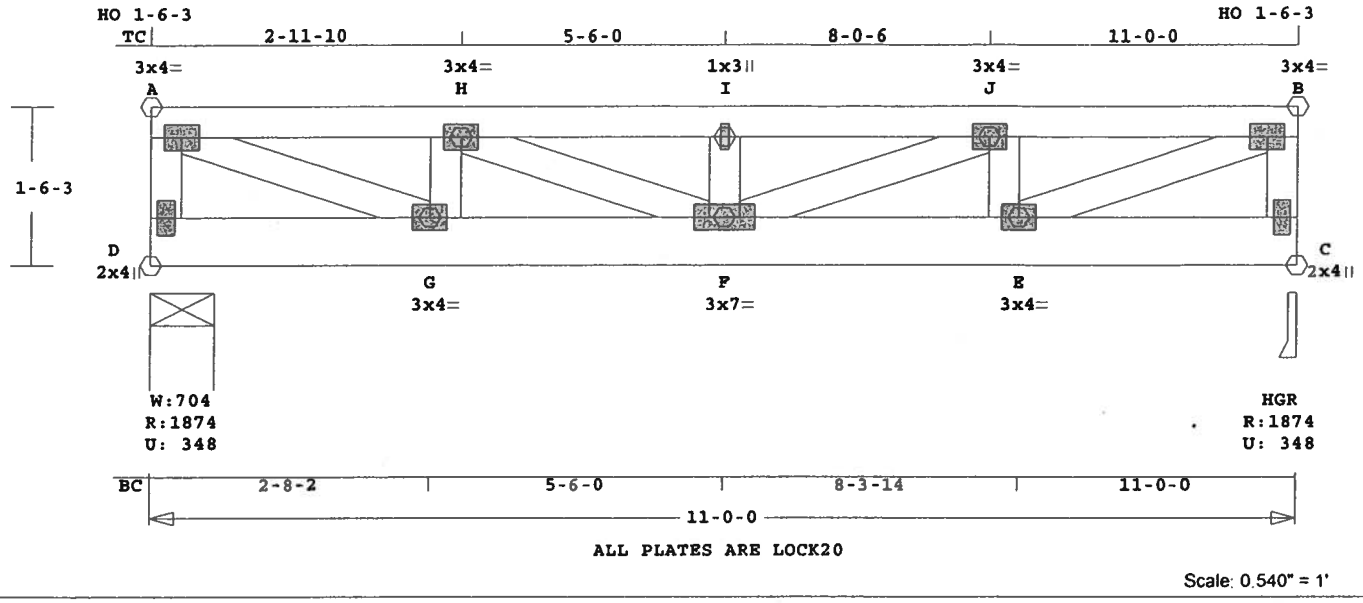
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
ALBERTMONIZ2	RG2	1*2P	FLAT	110000	10603	0	0	T06080606

U# J#ALBERTMONIZ2 ALBERT MONEZ - 2nd STORY



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 78.1 LBS

Online Plus -- Version 19.0.034
RUN DATE: 04-AUG-06
****
* 2-Ply Truss *
****

	CSI	-Size-	----Lumber----
TC	0.11	2x 4	SP-#2
BC	0.37	2x 6	SP-#2
WB	0.31	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.		0- 0- 0	11- 0- 0
BC Cont.		0- 0- 0	11- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.00	Fc=1.00	Ft=1.00	
BC Fb=1.00	Fc=1.00	Ft=1.00	

Load Case # 1 Girder Loading
Lumber Duration Factor 1.25
Plate Duration Factor 1.25

plf -	Live	Dead	From	To
TC V	40	20	0.0'	11.0'
BC V	130	150	0.0'	11.0'

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
D	1875	348	7- 4	1- 8
			Hz =	-32
C	1875	348	3- 8	1- 8
			Hz =	33

Membr	CSI	P Lbs	Ax	CSI	Bnd
----------Top Chords----------					
A -H	0.08	3179	C	0.02	0.06
H -I	0.11	4140	C	0.03	0.08
I -J	0.11	4140	C	0.03	0.08
J -B	0.08	3179	C	0.02	0.06
--------Bottom Chords---------					
D -G	0.13	24	T	0.00	0.13
G -F	0.37	3179	T	0.21	0.16
F -E	0.37	3179	T	0.21	0.16
E -C	0.13	24	T	0.00	0.13
------------Webs------------					
D -A	0.06	1448	C	WindLd	
A -G	0.31	3455	T		
G -H	0.02	574	C		
H -F	0.09	1044	T		
F -I	0.00	141	C		
F -J	0.09	1044	T		
E -J	0.02	574	C		
E -B	0.31	3455	T		
C -B	0.06	1448	C	WindLd	

TL Defl -0.08" in G -F L/999
LL Defl -0.04" in G -F L/999
Shear // Grain in G -F 0.17

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt Type	Plt Size	X	Y	JSI
A LOCK	3.0x 4.0	Ctr	Ctr	0.97
H LOCK	3.0x 4.0	Ctr	Ctr	0.41
I LOCK	1.0x 3.0	Ctr	Ctr	0.75
J LOCK	3.0x 4.0	Ctr	Ctr	0.41
B LOCK	3.0x 4.0	Ctr	Ctr	0.97
D LOCK	2.0x 4.0	Ctr	Ctr	0.45
G LOCK	3.0x 4.0	Ctr	Ctr	0.97
F LOCK	3.0x 7.0	Ctr	Ctr	0.42
E LOCK	3.0x 4.0	Ctr	Ctr	0.97
C LOCK	2.0x 4.0	Ctr	Ctr	0.45

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Girder Common
Loading BC
Span 15- 0- 8

2 COMPLETE TRUSSES REQUIRED.
Fasten together in staggered pattern. (1/2" bolts -OR- SDS3 screws -OR- 10d nails as each layer is applied.)

	Rows	----Spacing (In)---- Nails	Screws	Bolts
TC	1	12	24	0
BC	2	12	24	0
WB	1	8	8	

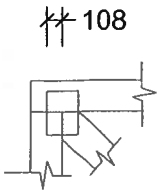
Design checked for 10 psf non-concurrent LL on BC.
Provide drainage to prevent water ponding.
Use properly rated hangers for loads framing into girder truss.
This truss must be installed as shown. It cannot be installed upside-down.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 4140 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Philip J. O'Regan
License #: 58126
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 8/7/2006

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)

(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

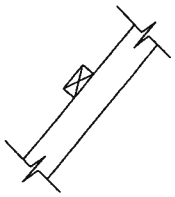
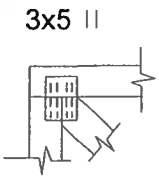


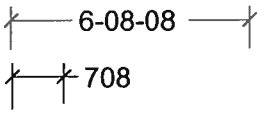
PLATE SIZE AND ORIENTATION

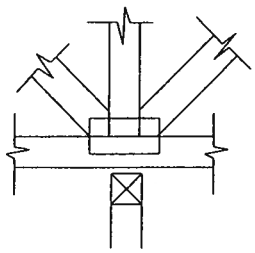


The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted.
The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA)," National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.

Robbins Engineering, Inc.

6904 Parke East Blvd.
Tampa, Fl 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbinseng.com

Monez Res. / Joist-1 FL Mayo Truss

NOTE: 2006.1 Allowable Stress Design

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	04-09-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	04-09-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSI: 0.05 VSI: 0.14

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

CODE COMPLIANCES :

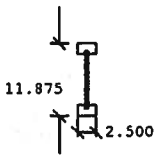
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

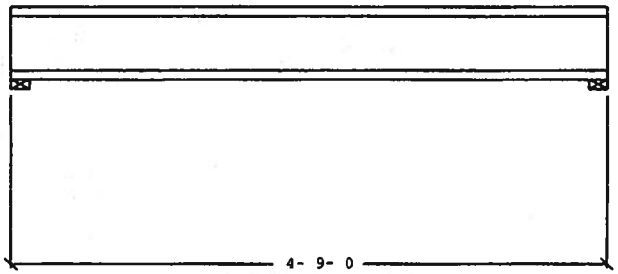
MAXIMUM BEARING NUMBER	1	2
DOWN	209	209
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.11"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.01"	0.21"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products 08/10/06 SBCCI 2006.1
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

DWG # 0608-049

SHEET # 1 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-1.SPX

Monez Res. / Joist-10 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64	PLF	00-00-00	02-08-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24	PLF	00-00-00	02-08-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSR: 0.01 VSR: 0.07

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

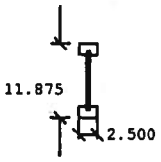
ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

SUPPORT REACTIONS (LBS):

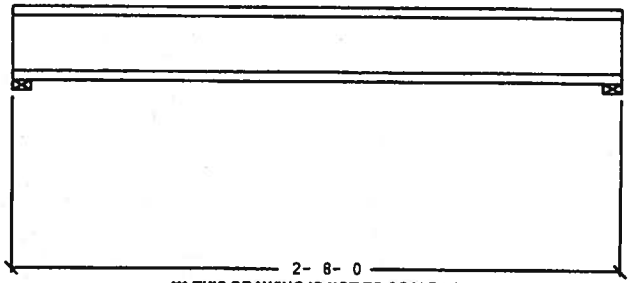
MAXIMUM B E A R I N G	NUMBER 1	2
DOWN	117	117
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

		MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE	LOAD	0.00"	0.06"
*DEAD	LOAD	0.00"	
TOTAL	LOAD	0.00"	0.11"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 2 of 42

File: L:\Engineering\Woode 2006\August\0608-049\5036-10.SPX

Monez Res. / Joist-11 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	01-11-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	01-11-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSI: 0.01 VSI: 0.05

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

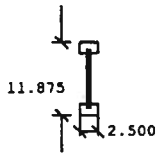
SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2
DOWN	84	84
UPLIFT	---	---

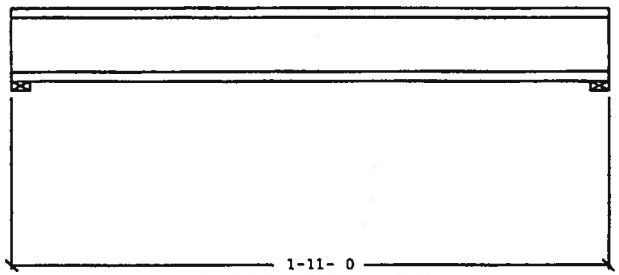
MIN BEARING SIZES (IN-SX)	
3- 8	3- 8

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.04"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.00"	0.07"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 3 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-11.SPX

Monez Res. / Joist-12 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 83" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	07-00-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	07-00-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSI: 0.13 VSI: 0.21

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

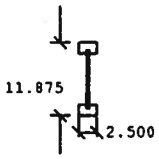
ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

SUPPORT REACTIONS (LBS):

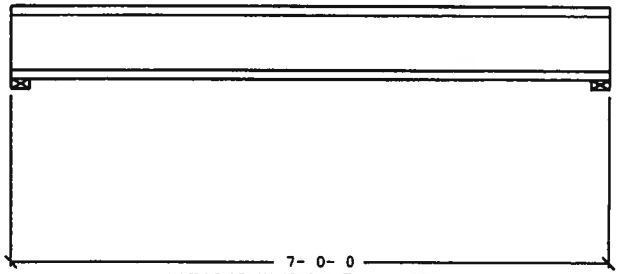
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	308	308
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.16"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.02"	0.33"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products 08/10/06 SBCCI 2006.1
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

DWG # 0608-049

SHEET # 4 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-12.SPX

Monez Res. / Joist-13 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 43" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	12-11-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	12-11-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

1 LPI 20Plus DEPTH 11.875"
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSI: 0.45 VSI: 0.41

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

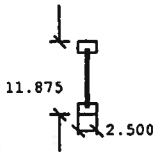
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

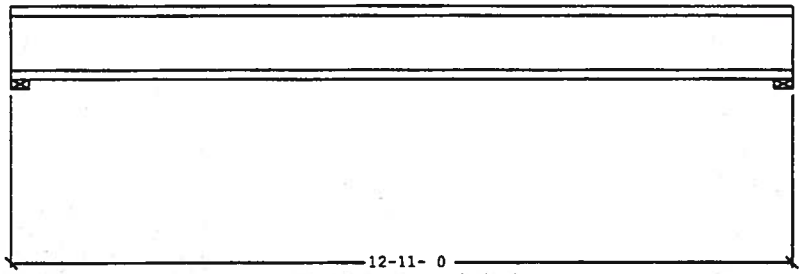
MAXIMUM BEARING NUMBER	1	2
DOWN	568	568
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.13"	0.31"
*DEAD LOAD	0.07"	
TOTAL LOAD	0.18"	0.62"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 5 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-13.SPX

Monez Res. / Joist-14 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 40" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	13-11-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	13-11-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSR 0.63 VSR 0.44

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

CODE COMPLIANCES :

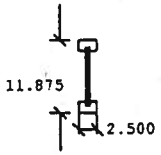
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

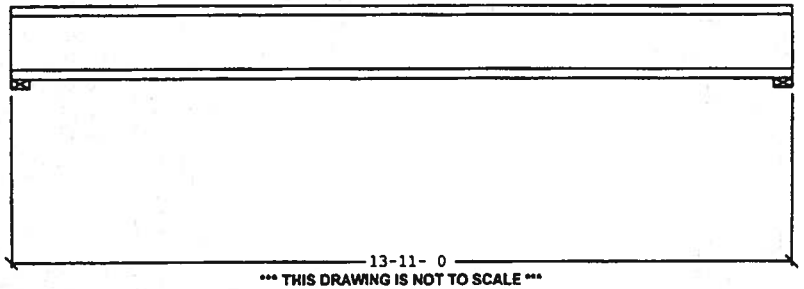
MAXIMUM BEARING NUMBER	1	2
DOWN	612	612
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

		MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE	LOAD	0.18"	0.34"
*DEAD	LOAD	0.10"	
TOTAL	LOAD	0.24"	0.67"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006 1

DWG # 0608-049

SHEET # 6 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-14.SPX

Monez Res. / Joist-15 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 69" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64	PLF	00-00-00	08-04-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24	PLF	00-00-00	08-04-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSI: 0.18 VSI: 0.26

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

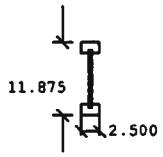
ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

SUPPORT REACTIONS (LBS):

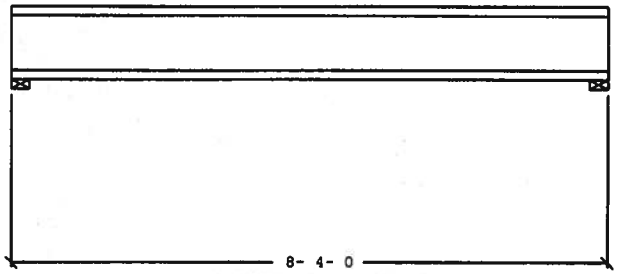
MAXIMUM BEARING NUMBER	1	2
DOWN	367	367
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.03"	0.20"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.04"	0.39"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 7 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-15.SPX

Monez Res. / Joist-16 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 48" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64	PLF	00-00-00	11-08-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24	PLF	00-00-00	11-08-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.37 VSI: 0.37

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

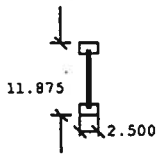
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

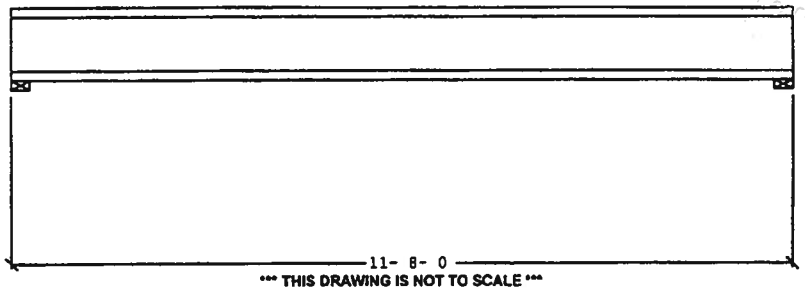
MAXIMUM BEARING NUMBER	1	2
DOWN	513	513
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.09"	0.28"
*DEAD LOAD	0.05"	
TOTAL LOAD	0.12"	0.56"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006 1

DWG # 0608-049

SHEET # 8 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-16.SPX

Monez Res. / Joist-2 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 36" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64	PLF	00-00-00	15-04-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24	PLF	00-00-00	15-04-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.60

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.65 VSI: 0.48

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

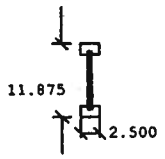
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

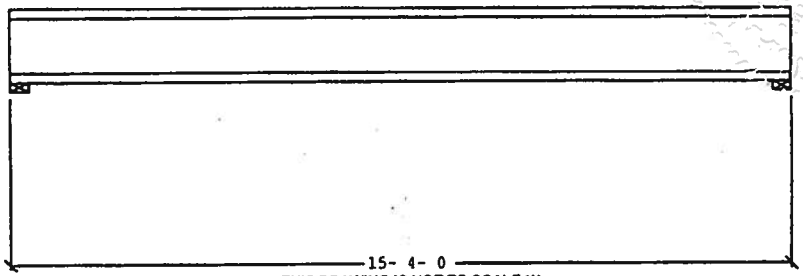
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	675	675
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.25"	0.37"
*DEAD LOAD	0.14"	
TOTAL LOAD	0.35"	0.74"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products 08/10/06 SBCCI 2006.1
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

DWG # 0608-049

SHEET # 9 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-2.SPX

Monez Res. / Joist-24 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	06-00-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	06-00-00		0.90
LINEAR	ROOF	LIVE	TOP	201 PLF	02-03-00	MINBRG=3.50"		1.00
LINEAR	ROOF	DEAD	TOP	201 PLF	02-03-00	MINBRG=3.50"		0.90
LINEAR	WALL	DEAD	TOP	80 PLF	02-03-00	MINBRG=3.50"		0.90
LINEAR	ROOF	LIVE	TOP	20 PLF	02-03-00	MINBRG=3.50"		1.25
LINEAR	ROOF	DEAD	TOP	20 PLF	02-03-00	MINBRG=3.50"		0.90

1 LPI 56A DEPTH 16.000 "
WEB: 0.4375"
FLANGE 1.50 X 3.50

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.10 VSI: 0.29

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

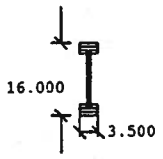
	REPORT #
NER	606
ICBO	PFC-3754
L.A. City	RR 25176
WISCONSIN	200114-W
N.Y. CITY	MEA 96-94-E
CCMC	11094-R
HUD	SEB 1091

SUPPORT REACTIONS (LBS):

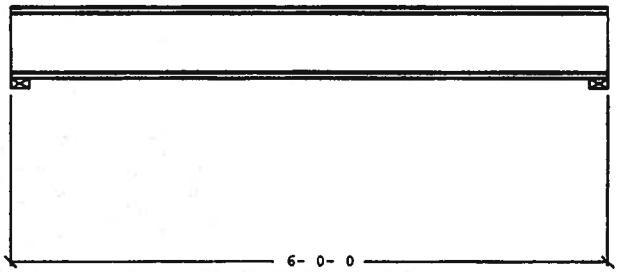
MAXIMUM BEARING NUMBER	1	2
DOWN	786	577
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.14"
*DEAD LOAD	0.02"	
TOTAL LOAD	0.02"	0.28"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.) LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products 04/10/06 SBCCI 2006.1
2706 Highway 421 North
Wilmington, NC 28401
Local 910.782.9678
National Wats 800.999.9105

DWG # 0608-049

SHEET # 10 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-24.SPX

Monez Res. / Joist-3 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 34" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	16-02-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	16-02-00		0.90

1 LPI 20Plus DEPTH 11.875 "
WEB: 0.375"
FLANGE 1.50 X 2.50

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.72 VSI: 0.51

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

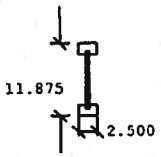
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

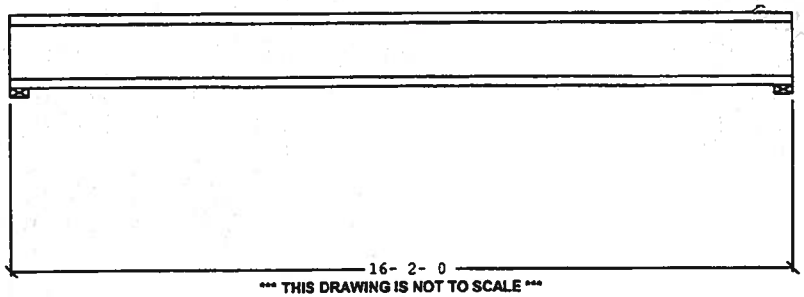
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	711	711
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.31"	0.39"
*DEAD LOAD	0.18"	
TOTAL LOAD	0.43"	0.79"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 11 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-3.SPX

Monez Res. / Joist-6 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 38" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	14-06-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	14-06-00		0.90

1 LPI 20Plus DEPTH 11.875"
WEB: 0.375"
FLANGE 1.50 X 2.50

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA :

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

MSI: 0.58
VSI: 0.46

CODE COMPLIANCES :

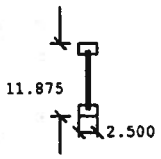
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

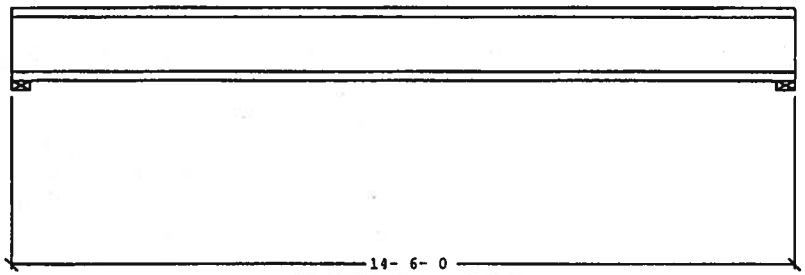
MAXIMUM B E A R I N G	NUMBER 1	2
DOWN	638	638
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.20"	0.35"
*DEAD LOAD	0.12"	
TOTAL LOAD	0.28"	0.70"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products 08/10/06 SBCCI 2006.1
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

DWG # 0608-049

SHEET # 12 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-6.SPX

Monez Res. / Joist-6 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 37" O.C. OR LESS.

NOTE FOR DOUBLE LPI

A. CONNECT DOUBLE LPI USING 2X6 + 5/8" OSB FILLERS FULL LENGTH OF JOIST. FOR JOISTS THAT ARE TOP LOADED ONLY,FILLERS NEED ONLY BE LOCATED AT EACH SUPPORT AND AT 8' ON CENTER MAX. FILLERS MUST BE AT LEAST 4' LONG. TOP LOADS MUST BE APPLIED TO BOTH I-JOISTS.
B. PROVIDE 4' LONG FILLERS AT ALL CONCENTRATED LOADS. CENTER FILLER ON LOAD
C. FASTEN FILLERS THROUGH LPI WEBS WITH 2 ROWS OF 8d(2-1/2") NAILS AT 12" OC FROM BOTH SIDES STAGGER ROWS AND CLINCH WHERE POSSIBLE.
D. MAXIMUM SIDE-APPLIED LOADS FOR STANDARD DURATION: UNIFORM LOAD = 260 PLF, CONCENTRATED LOAD = 1000 LBS. LOADS CAN BE DOUBLED FOR NAILS AT 6" OC, TRIPLED FOR NAILS AT 4" OC, ETC., AND MAY BE ADJUSTED FOR OTHER LOAD DURATIONS.
E. FLOOR SHEATHING TO BE NAILED TO FLANGES OF BOTH I-JOISTS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD LDF
UNIFORM	WALL	DEAD	TOP	80	PLF	00-00-00	14-10-00	0.90
UNIFORM	FLOOR	LIVE	TOP	40	PLF	00-00-00	14-10-00	1.00
UNIFORM	FLOOR	DEAD	TOP	15	PLF	00-00-00	14-10-00	0.90
CONCENTRATED	FLOOR	LIVE	TOP	800	LBS	12-06-00	MINBRG=3.50"	1.00
CONCENTRATED	FLOOR	DEAD	TOP	300	LBS	12-06-00	MINBRG=3.50"	0.90

2 LPI 20Plus DEPTH 11.875 "
NEB: 0.375"
FLANGE 1.50 X 2.50
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA :

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 12.00 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

MSI: 0.61
VSI: 0.71

CODE COMPLIANCES :

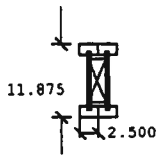
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

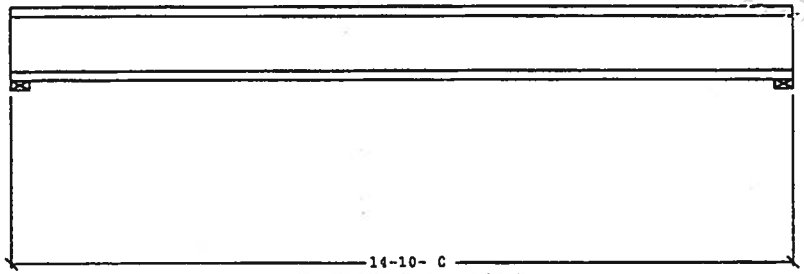
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	1174	1928
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

		MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE	LOAD	0.13"	0.36"
*DEAD	LOAD	0.28"	
TOTAL	LOAD	0.32"	0.72"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 13 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-6Dbl.SPX

Monez Res. / Joist-8 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	05-07-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	05-07-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

1 LPI 20Plus DEPTH 11.875"
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSI: 0.08 VSI: 0.17

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

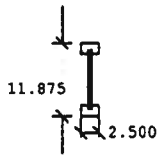
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

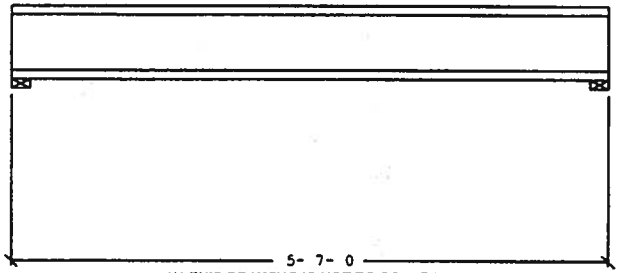
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	246	246
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

		MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE	LOAD	0.01"	0.13"
*DEAD	LOAD	0.00"	
TOTAL	LOAD	0.01"	0.26"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 14 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-8.SPX

Monez Res. / Joist-9 FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LPI FLANGES.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LPI TO SIZE.
6. THIS LPI IS TO BE USED AS A FLOOR JOIST ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT 63" O.C. OR LESS.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	64 PLF	00-00-00	10-08-00		1.00
UNIFORM	FLOOR	DEAD	TOP	24 PLF	00-00-00	10-08-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LPI JOIST AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS JOIST IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LPI JOIST SECURELY TO BEARINGS OR HANGERS.

1 LPI 20Plus DEPTH 11.875"
WEB: 0.375"
FLANGE 1.50 X 2.50

DESIGN CRITERIA : MSI: 0.31 VSI: 0.33

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

SPACING = 19.20 IN. C/C

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

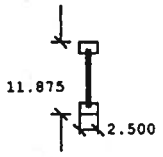
	REPORT #
ICC-ES	ESR-1305
WISCONSIN	200405-W
CCMC	12724-R
N.Y. CITY	MEA 101-00-E

SUPPORT REACTIONS (LBS):

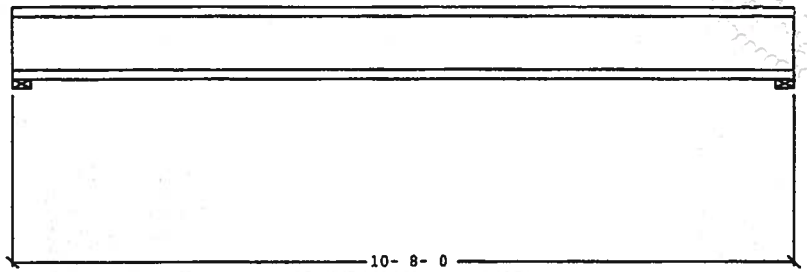
MAXIMUM BEARING NUMBER	1	2
DOWN	469	469
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.06"	0.26"
*DEAD LOAD	0.04"	
TOTAL LOAD	0.09"	0.51"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 15 of 42

File: L:\Engineering\Woode 2006\Augus\0608-049\6036-9.SPX

Monez Res. / Beam-A FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 8" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	SIDE	620 PLF	00-00-00	13-00-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	233 PLF	00-00-00	13-00-00		0.90
UNIFORM	BEAM	WEIGHT		12 PLF	00-00-00	13-00-00		0.90

2 BEAMS 1.75 X 11.875 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.24 VSI: 0.44

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

FLR LEFT SPAN CARR. : 16.00 FT
FLR RIGHT SPAN CARR. : 15.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

CODE COMPLIANCES :

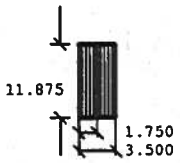
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

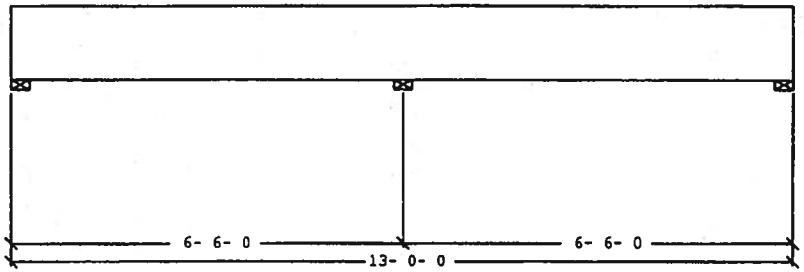
MAXIMUM B E A R I N G	N U M B E R 1	2	3
DOWN	2359	7023	2359
UPLIFT	---	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.02"	0.16"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.02"	0.32"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 16 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6038-A.SPX

Monez Res. / Beam-B FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 8" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 18d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	SIDE	600	PLF	00-00-00	16-00-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	225	PLF	00-00-00	16-00-00		0.90
UNIFORM	WALL	DEAD	TOP	160	PLF	08-00-00	16-00-00		0.90
UNIFORM	BEAM	WEIGHT		12	PLF	00-00-00	16-00-00		0.90
CONCENTRATED	FLOOR	LIVE	TOP	1604	LBS	05-02-00		MINBRG=2.50"	1.00
CONCENTRATED	FLOOR	DEAD	TOP	601	LBS	05-02-00		MINBRG=2.50"	0.90

2 BEAMS 1.75 X 11.875 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSR: 0.48 VSR: 0.74

LIVE LOAD	= 40	PSF
DEAD LOAD	= 15	PSF
TOTAL LOAD	= 55	PSF

FLR LEFT SPAN CARR. : 15.00 FT
FLR RIGHT SPAN CARR. : 15.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

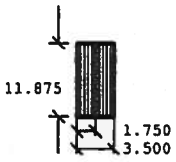
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

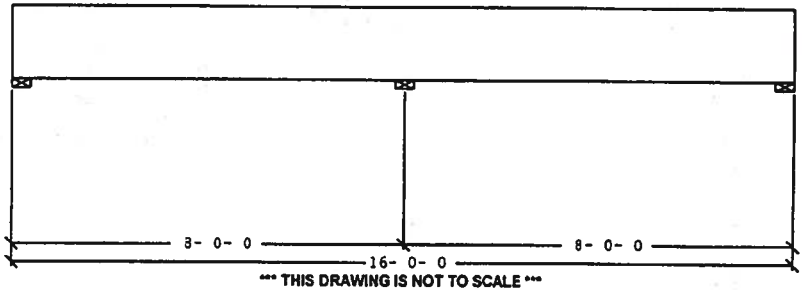
MAXIMUM BEARING NUMBER	1	2	3
DOWN	3304	11008	3314
UPLIFT	---	---	---

MIN BEARING SIZES (IN-SX)
3- 0 4- 4 3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.06"	0.20"
*DEAD LOAD	0.02"	
TOTAL LOAD	0.07"	0.39"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 17 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-B.SPX

Monez Res. / Beam-C FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 6" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	SIDE	300 PLF	00-00-00	24-00-00		1.00
UNIFORM	FLOOR	LIVE	SIDE	280 PLF	00-00-00	07-00-00		1.00
UNIFORM	WALL	DEAD	TOP	160 PLF	00-00-00	24-00-00		0.90
UNIFORM	FLOOR	LIVE	SIDE	120 PLF	07-00-00	24-00-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	113 PLF	00-00-00	24-00-00		0.90
UNIFORM	FLOOR	DEAD	SIDE	105 PLF	00-00-00	07-00-00		0.90
UNIFORM	FLOOR	DEAD	SIDE	45 PLF	07-00-00	24-00-00		0.90
UNIFORM	BEAM	WEIGHT		12 PLF	00-00-00	24-00-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

2 BEAMS 1.75 X 11.875 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.33 VSI: 0.54

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

FLR LEFT SPAN CARR. : 15.00 FT
FLR RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

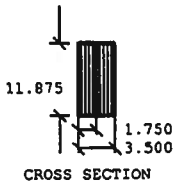
SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2	3	4
DOWN	3321	7854	6885	2594
UPLIFT	---	---	---	---

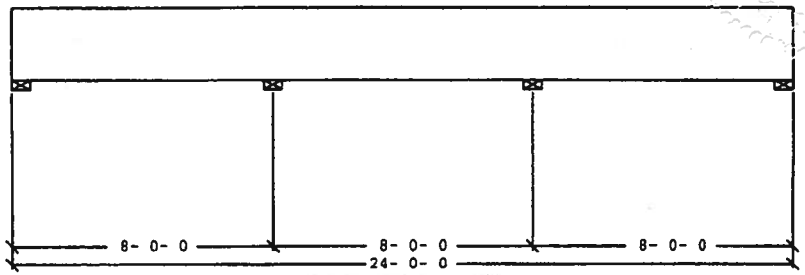
MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 8 3- 0

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.04"	0.20"
*DEAD LOAD	0.03"	
TOTAL LOAD	0.06"	0.39"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 18 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-C.SPX

Monez Res. / Beam-D FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	SIDE	320 PLF	00-00-00	13-06-00		1.00
UNIFORM	ROOF	LIVE	TOP	140 PLF	04-00-00	09-06-00		1.25
UNIFORM	ROOF	DEAD	TOP	140 PLF	04-00-00	09-06-00		0.90
UNIFORM	FLOOR	DEAD	SIDE	120 PLF	00-00-00	13-06-00		0.90
UNIFORM	WALL	DEAD	TOP	80 PLF	04-00-00	09-06-00		0.90
UNIFORM	ROOF	DEAD	TOP	80 PLF	00-00-00	13-06-00		0.90
UNIFORM	ROOF	LIVE	TOP	80 PLF	00-00-00	13-06-00		1.25
UNIFORM	BEAM	WEIGHT		16 PLF	00-00-00	13-06-00		0.90
CONCENTRATED	FLOOR	LIVE	TOP	213 LBS	00-02-00	MINBRG=2.50"		1.00
CONCENTRATED	FLOOR	DEAD	TOP	80 LBS	00-02-00	MINBRG=2.50"		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

MSI: 0.17
VSI: 0.23

DESIGN CRITERIA :

LIVE LOAD	=	40 PSF
DEAD LOAD	=	15 PSF
TOTAL LOAD	=	55 PSF

FLR LEFT SPAN CARR. : 0.00 FT
FLR RIGHT SPAN CARR. : 16.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

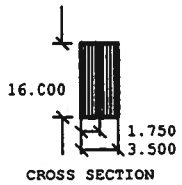
PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

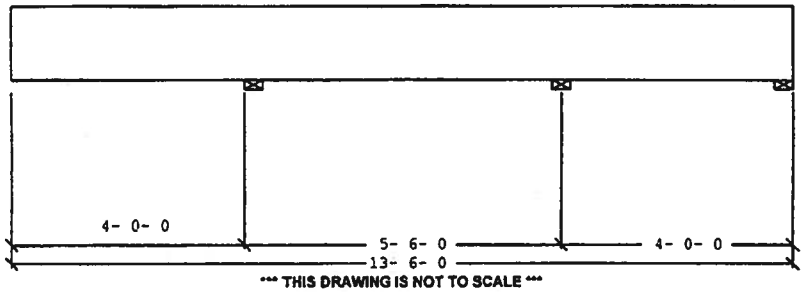
SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2	3
DOWN	6120	4693	864
UPLIFT	---	---	147

MIN BEARING SIZES (IN-SX)		
3- 8	3- 8	3- 8



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

	MAXIMUM DEFLECTIONS CALCULATED	MAXIMUM DEFLECTIONS ALLOWABLE	LEFT CANTILEVER CALCULATED	LEFT CANTILEVER ALLOWABLE
LIVE LOAD	0.04"	0.20"	0.04"	0.20"
*DEAD LOAD	0.02"		0.02"	
TOTAL LOAD	0.05"	0.40"	0.05"	0.40"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 19 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-D.SPX

Monez Res. / Beam-E (MB right hand side) FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	80 PLF	12-05-00	16-05-00		1.25
UNIFORM	ROOF	DEAD	TOP	80 PLF	12-05-00	16-05-00		0.90
UNIFORM	FLOOR	LIVE	SIDE	80 PLF	00-00-00	16-05-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	30 PLF	00-00-00	16-05-00		0.90
UNIFORM	BEAM	WEIGHT		16 PLF	00-00-00	16-05-00		0.90
CONCENTRATED	FLOOR	LIVE	TOP	413 LBS	16-04-00	MINBRG=2.50"		1.00
CONCENTRATED	FLOOR	DEAD	TOP	155 LBS	16-04-00	MINBRG=2.50"		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.12 VSI: 0.13

LIVE LOAD	=	40 PSF
DEAD LOAD	=	15 PSF
TOTAL LOAD	=	55 PSF

FLR LEFT SPAN CARR. : 2.00 FT
FLR RIGHT SPAN CARR. : 2.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

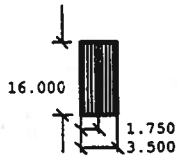
CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

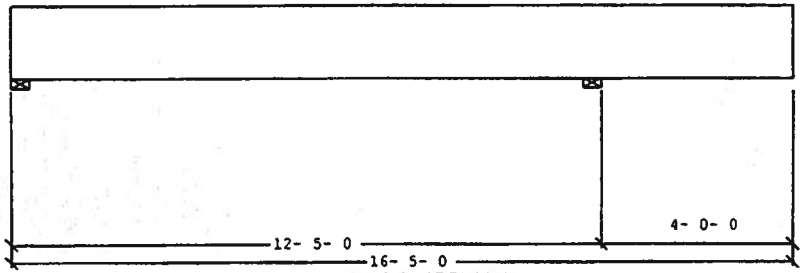
SUPPORT REACTIONS (LBS):

MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	652	2858
UPLIFT	78	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 8



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE	RIGHT CANTILEVER CALCULATED	ALLOWABLE
LIVE LOAD	0.05"	0.20"	0.05"	0.20"
*DEAD LOAD	0.02"		0.02"	
TOTAL LOAD	0.06"	0.40"	0.06"	0.40"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 20 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-E1.SPX

Monez Res. / Beam-E (at Stairwell) FL Mayo Truss

2008.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	WALL	DEAD	TOP	80	PLF	00-00-00	11-00-00		0.90
UNIFORM	FLOOR	LIVE	SIDE	40	PLF	00-00-00	16-08-00		1.00
UNIFORM	BEAM	WEIGHT		16	PLF	00-00-00	16-08-00		0.90
UNIFORM	FLOOR	DEAD	SIDE	15	PLF	00-00-00	16-08-00		0.90
CONCENTRATED	ROOF	LIVE	TOP	2344	LBS	00-09-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	DEAD	TOP	1329	LBS	00-09-00	MINBRG=2.50"		0.90
CONCENTRATED	FLOOR	LIVE	TOP	775	LBS	16-00-00	MINBRG=2.50"		1.00
CONCENTRATED	FLOOR	LIVE	TOP	509	LBS	00-02-00	MINBRG=2.50"		1.00
CONCENTRATED	FLOOR	DEAD	TOP	291	LBS	16-00-00	MINBRG=2.50"		0.90
CONCENTRATED	FLOOR	DEAD	TOP	191	LBS	00-02-00	MINBRG=2.50"		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.14 VSI: 0.43

LIVE LOAD	=	40 PSF
DEAD LOAD	=	15 PSF
TOTAL LOAD	=	55 PSF

FLR LEFT SPAN CARR. : 2.00 FT
FLR RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

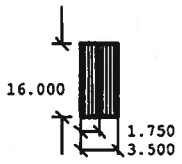
MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

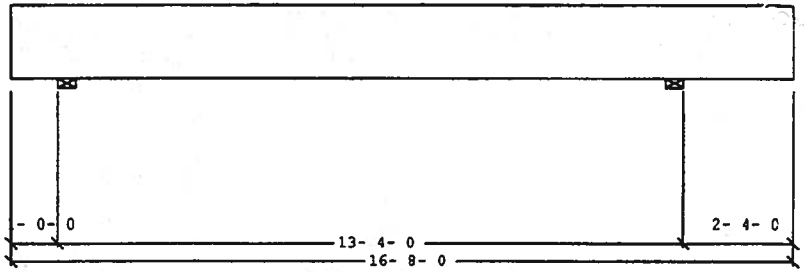
SUPPORT REACTIONS (LBS):

MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	5573	2112
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 8 3- 8



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE	LEFT CANTILEVER CALCULATED	ALLOWABLE	RIGHT CANTILEVER CALCULATED	ALLOWABLE
LIVE LOAD	-0.02"	0.33"	0.01"	0.20"	0.02"	0.12"
*DEAD LOAD	0.03"		-0.01"		-0.01"	
TOTAL LOAD	0.03"	0.65"	-0.01"	0.30"	-0.02"	0.23"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2708 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2008.1

DWG # 0608-049

SHEET # 21 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-E2.SPX

Monez Res. / Beam-F (at Stairwell) FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	320	PLF	00-00-00	07-07-00		1.25
UNIFORM	ROOF	DEAD	TOP	320	PLF	00-00-00	07-07-00		0.90
UNIFORM	WALL	DEAD	TOP	80	PLF	00-00-00	07-07-00		0.90
UNIFORM	BEAM	WEIGHT		16	PLF	00-00-00	07-07-00		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.13 VSI: 0.13

FLOOR LIVE LOAD	=	40 PSF
FLOOR DEAD LOAD	=	15 PSF
FLOOR TOTAL LOAD	=	55 PSF
ROOF LIVE LOAD	=	20 PSF
ROOF DEAD LOAD	=	20 PSF
ROOF TOTAL LOAD	=	40 PSF
FLR LEFT SPAN CARR.	:	0.00 FT
FLR RIGHT SPAN CARR.	:	0.00 FT
ROOF LEFT SPAN CARR.	:	28.00 FT
ROOF RIGHT SPAN CARR.	:	4.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

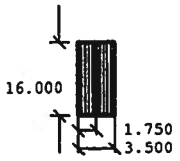
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

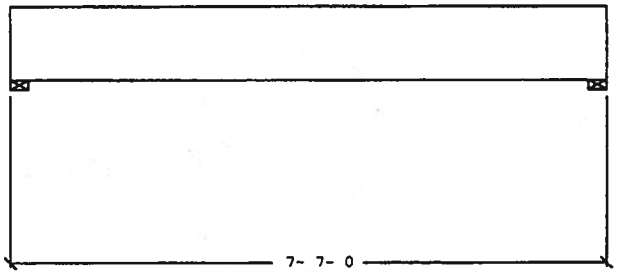
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	2791	2791
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.18"
*DEAD LOAD	0.02"	
TOTAL LOAD	0.02"	0.37"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 22 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-F1.SPX

Monez Res. / Beam-F FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	130 PLF	00-00-00	08-00-00		1.25
UNIFORM	ROOF	DEAD	TOP	130 PLF	00-00-00	08-00-00		0.90
UNIFORM	WALL	DEAD	TOP	80 PLF	00-00-00	08-00-00		0.90
UNIFORM	FLOOR	LIVE	SIDE	40 PLF	00-00-00	08-00-00		1.00
UNIFORM	BEAM	WEIGHT		16 PLF	00-00-00	08-00-00		0.90
UNIFORM	FLOOR	DEAD	SIDE	15 PLF	00-00-00	08-00-00		0.90
CONCENTRATED	ROOF	DEAD	TOP	258 LBS	08-00-00		MINBRG=2.50"	0.90
CONCENTRATED	ROOF	LIVE	TOP	258 LBS	08-00-00		MINBRG=2.50"	1.25

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.05 VSI: 0.08

FLOOR LIVE LOAD	=	40 PSF
FLOOR DEAD LOAD	=	15 PSF
FLOOR TOTAL LOAD	=	55 PSF
ROOF LIVE LOAD	=	20 PSF
ROOF DEAD LOAD	=	20 PSF
ROOF TOTAL LOAD	=	40 PSF
FLR LEFT SPAN CARR.	:	2.00 FT
FLR RIGHT SPAN CARR.	:	0.00 FT
ROOF LEFT SPAN CARR.	:	5.00 FT
ROOF RIGHT SPAN CARR.	:	8.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

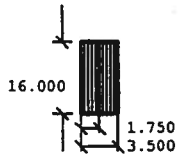
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

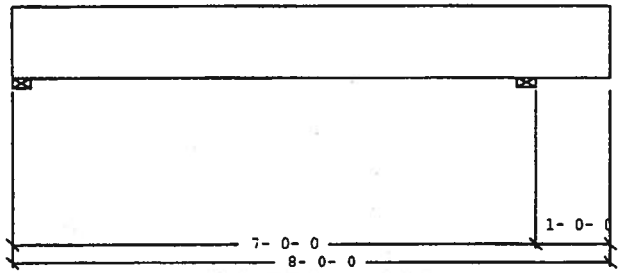
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	1385	2469
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 8

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE	RIGHT CANTILEVER CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.17"	-0.00"	0.20"
*DEAD LOAD	0.01"		-0.00"	
TOTAL LOAD	0.01"	0.34"	-0.00"	0.30"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 6d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 23 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-F2.SPX

Monez Res. / Beam-G Stair Stringer Headers FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	SIDE	120 PLF	00-00-00	03-00-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	45 PLF	00-00-00	03-00-00		0.90
UNIFORM	BEAM	WEIGHT		16 PLF	00-00-00	03-00-00		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.01 VSI: 0.02

LIVE LOAD	= 40	PSF
DEAD LOAD	= 15	PSF
TOTAL LOAD	= 55	PSF

FLR LEFT SPAN CARR. : 0.00 FT
FLR RIGHT SPAN CARR. : 6.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

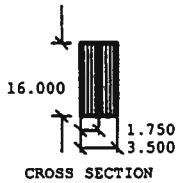
SUPPORT REACTIONS (LBS):

MAXIMUM B E A R I N G	1	2
DOWN	272	272
UPLIFT	---	---

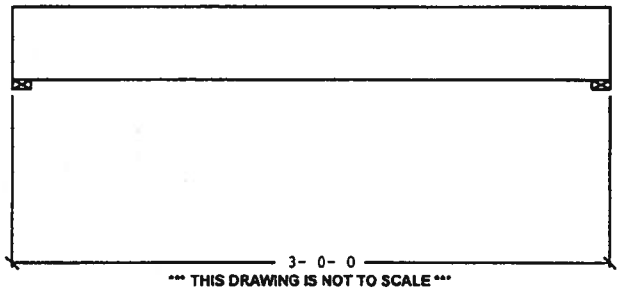
MIN BEARING SIZES (IN-SX)
3- 0 3- C

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.07"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.00"	0.14"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products 08/10/06 SBCCI 2006.1
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

DWG # 0608-049
SHEET # 24 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-G.SPX

Monez Res. / Beam-H FL Mayo Truss

NOTE: 2006.1 Allowable Stress Design

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	SIDE	320 PLF	00-00-00	05-05-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	120 PLF	00-00-00	05-05-00		0.90
UNIFORM	BEAM	WEIGHT		16 PLF	00-00-00	05-05-00		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.05 VSI: 0.11

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

FLR LEFT SPAN CARR. : 0.00 FT
FLR RIGHT SPAN CARR. : 16.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

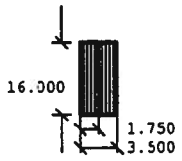
SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2
DOWN	1235	1235
UPLIFT	---	---

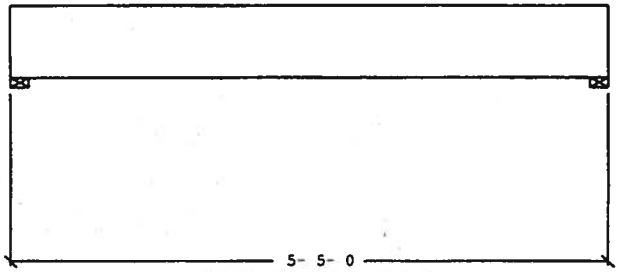
MIN BEARING SIZES (IN-SX)
3- 0 3- 0

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.13"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.00"	0.26"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 9d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products 08/10/06 SBCCI 2006.1
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

DWG # 0608-049
SHEET # 25 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-H.SPX

Monez Res. / Beam-I FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	SIDE	247 PLF	00-00-00	06-01-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	93 PLF	00-00-00	06-01-00		0.90
UNIFORM	BEAM	WEIGHT		16 PLF	00-00-00	06-01-00		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.05 VSI: 0.10

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

FLR LEFT SPAN CARR. : 0.00 FT
FLR RIGHT SPAN CARR. : 12.33 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

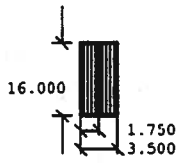
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

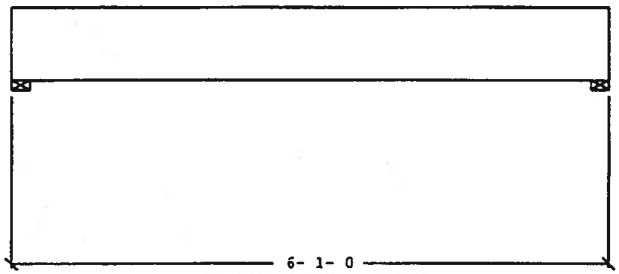
MAXIMUM BEARING NUMBER	1	2
DOWN	1080	1080
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.15"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.00"	0.29"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 26 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-I.SPX

Monez Res. / Beam-J FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. MEMBER MUST BE SIDE-LOADED TO ENSURE ADEQUATE STABILITY. PROVIDE COMPRESSION EDGE BRACING AT 24" O.C. OR LESS.

LOAD TABLE

1 BEAM 1.75 X 16.000 LP LVL2650FD-1.9E

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
TRAPEZOIDAL	FLOOR	LIVE	SIDE	0 PLF	00-00-00	04-03-00	100 PLF	1.00
TRAPEZOIDAL	FLOOR	DEAD	SIDE	0 PLF	00-00-00	04-03-00	38 PLF	0.90
UNIFORM	BEAM	WEIGHT		8 PLF	00-00-00	04-03-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.01 VSI: 0.04

LIVE LOAD	=	40 PSF
DEAD LOAD	=	15 PSF
TOTAL LOAD	=	55 PSF

FLR LEFT SPAN CARR. : 0.00 FT
FLR RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

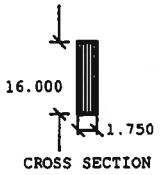
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

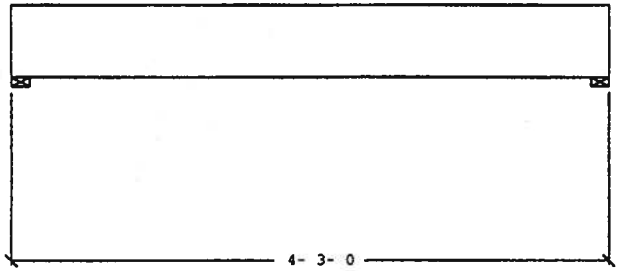
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	115	213
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.10"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.00"	0.20"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9876
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 27 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-J.SPX

Monez Res. / Beam-K FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	350	PLF	00-00-00	12-00-00		1.25
UNIFORM	ROOF	DEAD	TOP	350	PLF	00-00-00	12-00-00		0.90
UNIFORM	FLOOR	LIVE	SIDE	280	PLF	00-00-00	12-00-00		1.00
UNIFORM	FLOOR	DEAD	SIDE	105	PLF	00-00-00	12-00-00		0.90
UNIFORM	WALL	DEAD	TOP	80	PLF	00-00-00	12-00-00		0.90
UNIFORM	BEAM	WEIGHT		16	PLF	00-00-00	12-00-00		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.

ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.12 VSI: 0.26

FLOOR LIVE LOAD	=	40 PSF
FLOOR DEAD LOAD	=	15 PSF
FLOOR TOTAL LOAD	=	55 PSF
ROOF LIVE LOAD	=	20 PSF
ROOF DEAD LOAD	=	20 PSF
ROOF TOTAL LOAD	=	40 PSF
FLR LEFT SPAN CARR.	:	14.00 FT
FLR RIGHT SPAN CARR.	:	0.00 FT
ROOF LEFT SPAN CARR.	:	28.50 FT
ROOF RIGHT SPAN CARR.	:	6.50 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

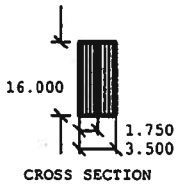
CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

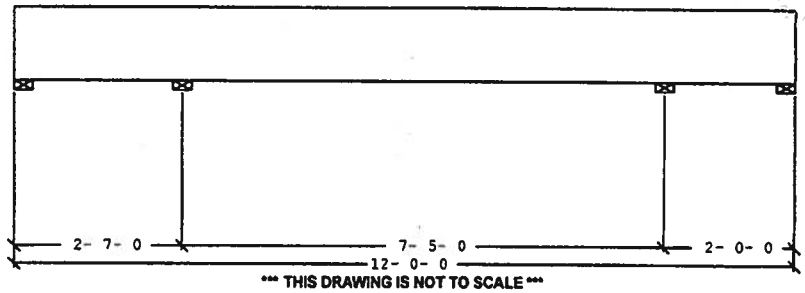
SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2	3	4
DOWN	659	7650	8023	67
UPLIFT	987	---	---	1812

MIN BEARING SIZES (IN-SX)			
3- 0	3- 8	3- 8	3- 0



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.19"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.01"	0.37"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 28 of 42

File: L:\EngineeringWoode 2006\August\0608-049\6036-K.SPX

Monez Res. / Beam-L FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	WALL	DEAD	TOP	80	PLF	00-00-00	02-10-00		0.90
UNIFORM	FLOOR	LIVE	TOP	40	PLF	00-00-00	02-10-00		1.00
UNIFORM	BEAM	WEIGHT		16	PLF	00-00-00	02-10-00		0.90
UNIFORM	FLOOR	DEAD	TOP	15	PLF	00-00-00	02-10-00		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.00 VSI: 0.00

FLOOR LIVE LOAD	=	40	PSF
FLOOR DEAD LOAD	=	15	PSF
FLOOR TOTAL LOAD	=	55	PSF
ROOF LIVE LOAD	=	20	PSF
ROOF DEAD LOAD	=	20	PSF
ROOF TOTAL LOAD	=	40	PSF

FLR LEFT SPAN CARR. : 0.00 FT
FLR RIGHT SPAN CARR. : 2.00 FT
ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

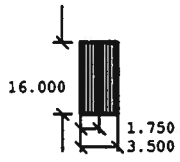
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

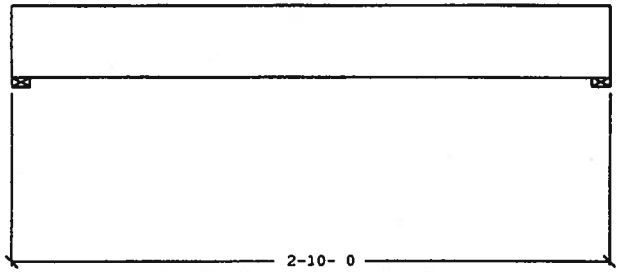
MAXIMUM BEARING NUMBER	1	2
DOWN	214	214
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.06"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.00"	0.13"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 29 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-L.SPX

Monez Res. / Beam-M FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	WALL	DEAD	TOP	80	PLF	00-00-00	05-06-00		0.90
UNIFORM	ROOF	LIVE	TOP	80	PLF	00-00-00	05-06-00		1.00
UNIFORM	ROOF	DEAD	TOP	80	PLF	00-00-00	05-06-00		0.90
UNIFORM	FLOOR	LIVE	TOP	40	PLF	00-00-00	09-03-00		1.00
UNIFORM	BEAM	WEIGHT		16	PLF	00-00-00	09-03-00		0.90
UNIFORM	FLOOR	DEAD	TOP	15	PLF	00-00-00	09-03-00		0.90
CONCENTRATED	ROOF	LIVE	TOP	215	LBS	06-08-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	DEAD	TOP	215	LBS	06-08-00	MINBRG=2.50"		0.90

2 BEAMS 1.75 X 16.000 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR LP ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.

ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.03 VSI: 0.05

FLOOR LIVE LOAD	=	40	PSF
FLOOR DEAD LOAD	=	15	PSF
FLOOR TOTAL LOAD	=	55	PSF
ROOF LIVE LOAD	=	20	PSF
ROOF DEAD LOAD	=	20	PSF
ROOF TOTAL LOAD	=	40	PSF
FLR LEFT SPAN CARR.	:	0.00	FT
FLR RIGHT SPAN CARR.	:	2.00	FT
ROOF LEFT SPAN CARR.	:	0.00	FT
ROOF RIGHT SPAN CARR.	:	0.00	FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

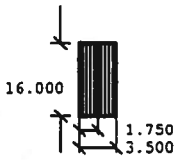
CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

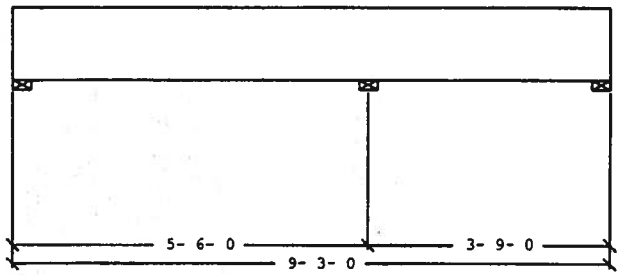
SUPPORT REACTIONS (LBS):

MAXIMUM B E A R I N G	N U M B E R 1	2	3
DOWN	713	1673	107
UPLIFT	---	---	83

MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 0



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.00"	0.13"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.00"	0.27"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

06/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 30 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-M.SPX

Monez Res. / Beam-N FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	620 PLF	00-00-00	10-08-00		1.00
UNIFORM	FLOOR	DEAD	TOP	233 PLF	00-00-00	10-08-00		0.90
UNIFORM	BEAM	WEIGHT		14 PLF	00-00-00	10-08-00		0.90

2 BEAMS 1.75 X 14.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL FLOOR BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.47 VSI: 0.37

LIVE LOAD	=	40	PSF
DEAD LOAD	=	15	PSF
TOTAL LOAD	=	55	PSF

FLR LEFT SPAN CARR. : 15.00 FT
FLR RIGHT SPAN CARR. : 16.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

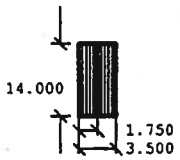
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

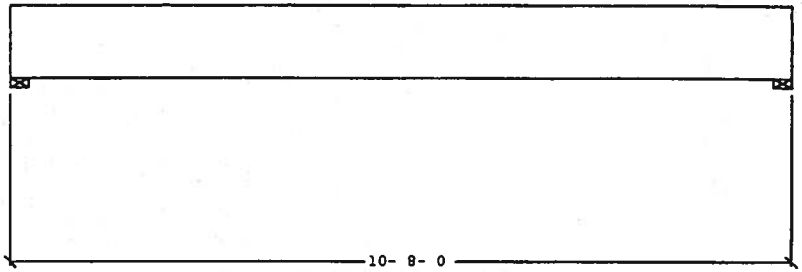
MAXIMUM BEARING NUMBER	1	2
DOWN	4621	4622
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
4- 0 4- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.10"	0.26"
*DEAD LOAD	0.06"	
TOTAL LOAD	0.15"	0.52"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth herein,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 31 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-N.SPX

Monez Res. / Beam-O FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A COMBINATION ROOF AND FLOOR BEAM ONLY.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH TWO PLIES WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC. FROM ONE FACE ONLY. STAGGER ROWS. FLIP BEAM AND ATTACH THE THIRD PLY WITH 3 ROWS OF 16d (3-1/2") NAILS AT 12" OC TO THE UN-NAILED SIDE OF THE FIRST TWO PLIES. STAGGER ROWS. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	FLOOR	LIVE	TOP	649 PLF	11-02-00	16-08-00		1.00
UNIFORM	FLOOR	LIVE	TOP	647 PLF	00-00-00	01-07-00		1.00
UNIFORM	FLOOR	DEAD	TOP	244 PLF	11-02-00	16-08-00		0.90
UNIFORM	FLOOR	DEAD	TOP	243 PLF	00-00-00	01-07-00		0.90
UNIFORM	WALL	DEAD	TOP	100 PLF	01-07-00	11-02-00		0.90
UNIFORM	BEAM	WEIGHT		27 PLF	00-00-00	16-08-00		0.90
UNIFORM	ROOF	LIVE	TOP	20 PLF	00-00-00	16-08-00		1.25
UNIFORM	ROOF	DEAD	TOP	20 PLF	00-00-00	16-08-00		0.90
CONCENTRATED	FLOOR	LIVE	TOP	5178 LBS	11-02-00	MINBRG=2.50"		1.00
CONCENTRATED	FLOOR	LIVE	TOP	5047 LBS	01-07-00	MINBRG=2.50"		1.00
CONCENTRATED	FLOOR	DEAD	TOP	1942 LBS	11-02-00	MINBRG=2.50"		0.90
CONCENTRATED	FLOOR	DEAD	TOP	1893 LBS	01-07-00	MINBRG=2.50"		0.90

3 BEAMS 1.75 X 18.000 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 3 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.69 VSI: 0.46

FLOOR LIVE LOAD	= 40	PSF
FLOOR DEAD LOAD	= 15	PSF
FLOOR TOTAL LOAD	= 55	PSF
ROOF LIVE LOAD	= 20	PSF
ROOF DEAD LOAD	= 20	PSF
ROOF TOTAL LOAD	= 40	PSF
FLR LEFT SPAN CARR.	: 0.00	FT
FLR RIGHT SPAN CARR.	: 0.00	FT
ROOF LEFT SPAN CARR.	: 0.00	FT
ROOF RIGHT SPAN CARR.	: 2.00	FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 480
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	NR 1214D

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF/FLOOR BEAM SECURELY TO BEARINGS OR HANGERS

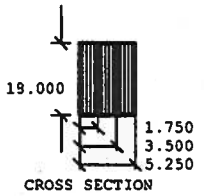
SUPPORT REACTIONS (LBS):

MAXIMUM B E A R I N G	NUMBER 1	2
DOWN	11933	10523
UPLIFT	---	---

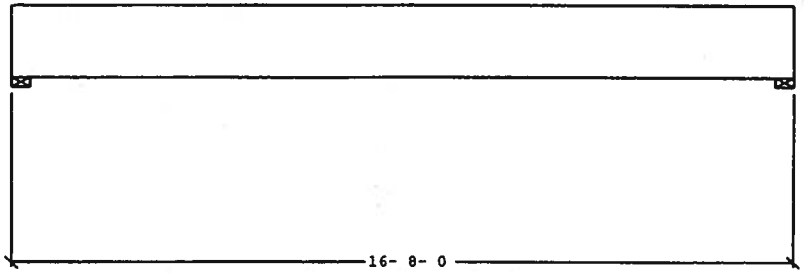
MIN BEARING SIZES (IN-SX)
3- 4 3- 0

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.25"	0.41"
*DEAD LOAD	0.20"	
TOTAL LOAD	0.38"	0.82"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 6d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 32 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-O.SPX

Monez Res. / Beam-P (at Front of House) FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	108 PLF	00-00-00	11-10-00		1.00
UNIFORM	ROOF	DEAD	TOP	108 PLF	00-00-00	11-10-00		0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	11-10-00		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.32 VSI: 0.18

LIVE LOAD	=	20	PSF
DEAD LOAD	=	20	PSF
TOTAL LOAD	=	40	PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

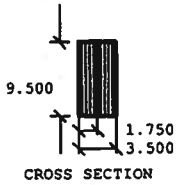
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

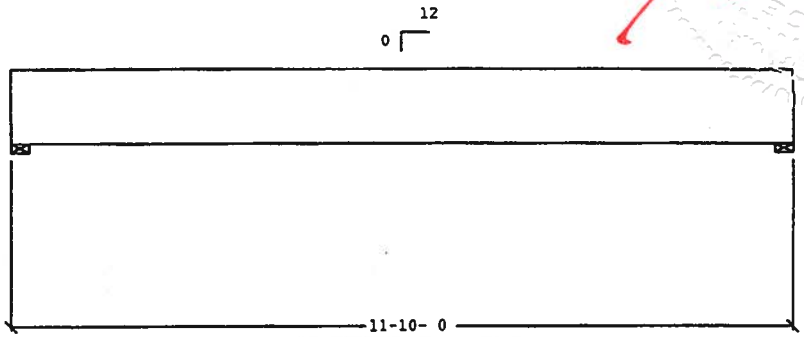
MAXIMUM BEARING NUMBER	1	2
DOWN	1328	1328
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.09"	0.39"
*DEAD LOAD	0.15"	
TOTAL LOAD	0.19"	0.58"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 33 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-P1.SPX

Monez Res. / Beam-P (at rear of house) FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	153 PLF	00-00-00	12-05-00		1.00
UNIFORM	ROOF	DEAD	TOP	153 PLF	00-00-00	12-05-00		0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	12-05-00		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSR 0.12 VSR 0.16

LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	NR 1214D

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

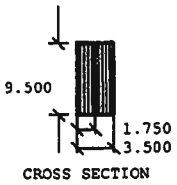
ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

SUPPORT REACTIONS (LBS):

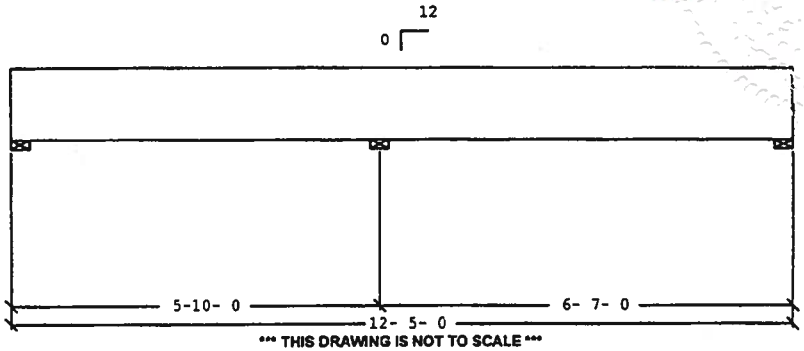
MAXIMUM BEARING NUMBER	1	2	3
DOWN	657	2456	805
UPLIFT	---	---	---

MIN BEARING SIZES (IN-SX)		
3- 0	3- 8	3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.22"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.01"	0.32"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

06/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 34 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-P2.SPX

Monez Res. / Beams-Q (at Front Entrance) FL Mayo Truss

NOTE: 2006.1 Allowable Stress Design

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	80 PLF	00-00-00	07-06-00		1.00
UNIFORM	ROOF	DEAD	TOP	80 PLF	00-00-00	07-06-00		0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	07-06-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.09 VSI: 0.08

LIVE LOAD	=	20	PSF
DEAD LOAD	=	20	PSF
TOTAL LOAD	=	40	PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

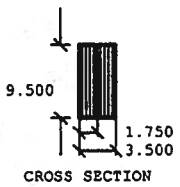
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

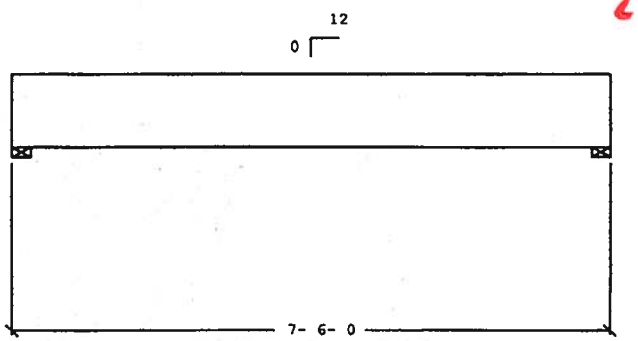
MAXIMUM B E A R I N G	N U M B E R 1	2
DOWN	636	636
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.24"
*DEAD LOAD	0.02"	
TOTAL LOAD	0.02"	0.36"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 6d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9678
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 35 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-Q1.SPX

Monez Res. / Beam-Q (at rear of house) FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
TRAPEZOIDAL	ROOF	LIVE	TOP	20 PLF	00-00-00	07-06-00	100 PLF	1.25
TRAPEZOIDAL	ROOF	DEAD	TOP	20 PLF	00-00-00	07-06-00	100 PLF	0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	07-06-00		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.06 VSI: 0.06

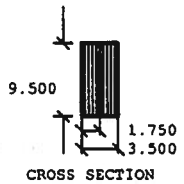
LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

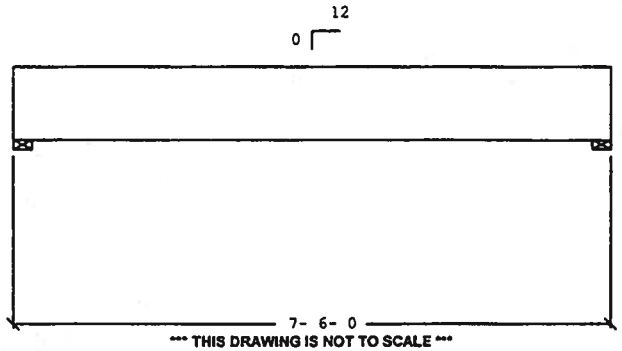
DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2
DOWN	386	586
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.24"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.02"	0.36"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 36 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-Q2.SPX

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	106 PLF	00-00-00	18-11-00		1.00
UNIFORM	ROOF	LIVE	TOP	80 PLF	00-00-00	18-11-00		1.00
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	18-11-00		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.18 VSI: 0.16

LIVE LOAD	= 20	PSF
DEAD LOAD	= 20	PSF
TOTAL LOAD	= 40	PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

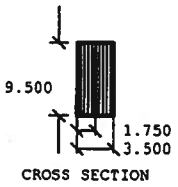
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

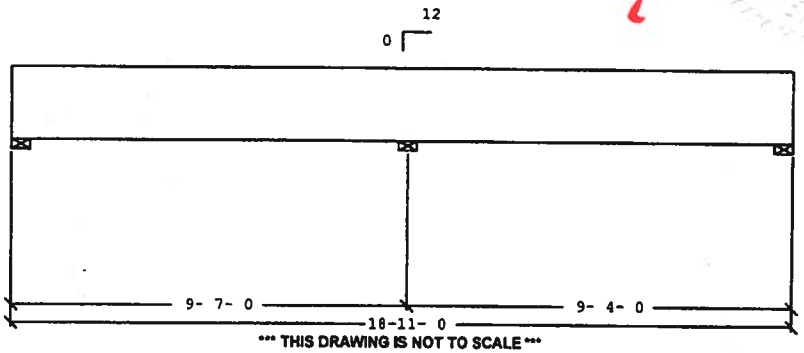
MAXIMUM BEARING NUMBER	1	2	3
DOWN	709	2312	678
UPLIFT	---	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.03"	0.32"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.03"	0.47"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 37 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-R.SPX

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	106 PLF	00-00-00	18-11-00		1.00
UNIFORM	ROOF	LIVE	TOP	80 PLF	00-00-00	18-11-00		1.00
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	18-11-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.18 VSI: 0.16

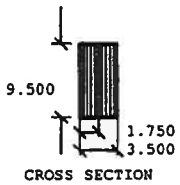
LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

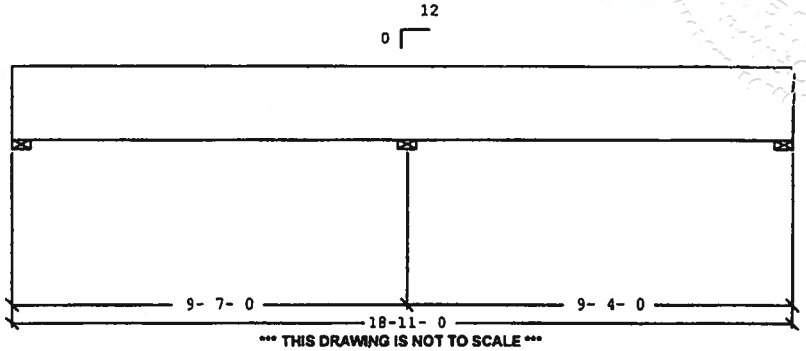
DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2	3
DOWN	709	2312	678
UPLIFT	---	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.03"	C.32"
*DEAD LOAD	0.00"	
TOTAL LOAD	0.03"	C.47"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 38 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-S.SPX

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	80 PLF	07-09-00	15-02-00		1.00
UNIFORM	ROOF	DEAD	TOP	80 PLF	07-09-00	15-02-00		0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	15-02-00		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS INDICATED BY NEGATIVE REACTIONS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT ARCHITECT, ENGINEER, OR DESIGNER.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.07 VSI: 0.09

LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

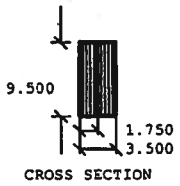
CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

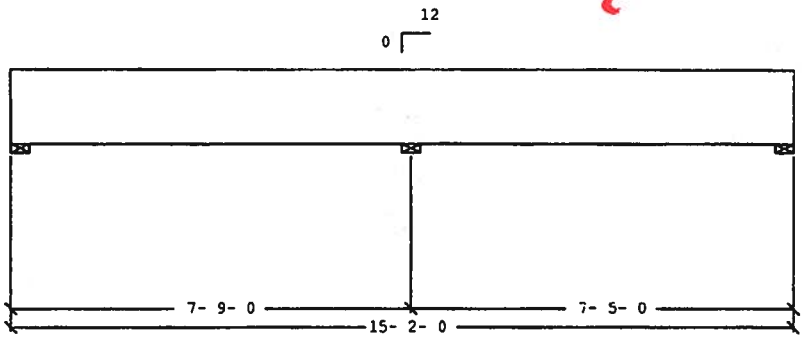
SUPPORT REACTIONS (LBS):

MAXIMUN BEARING NUMBER	1	2	3
DOWN	0	825	547
UPLIFT	41	---	---

MIN BEARING SIZES (IN-SX)		
3- 0	3- 8	3- 0



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.24"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.02"	0.36"

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 39 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-T1.SPX

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	80 PLF	00-00-00	07-09-00		1.00
TRAPEZOIDAL	ROOF	DEAD	TOP	80 PLF	07-09-00	15-02-00	0 PLF	0.90
TRAPEZOIDAL	ROOF	LIVE	TOP	80 PLF	07-09-00	15-02-00	0 PLF	1.25
UNIFORM	ROOF	DEAD	TOP	80 PLF	00-00-00	07-09-00		0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	15-05-00		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.08 VSI: 0.10

LIVE LOAD	=	20	PSF
DEAD LOAD	=	20	PSF
TOTAL LOAD	=	40	PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

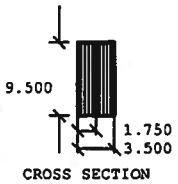
SUPPORT REACTIONS (LBS):

MAXIMUM BEARING NUMBER	1	2	3
DOWN	538	1348	93
UPLIFT	---	---	---

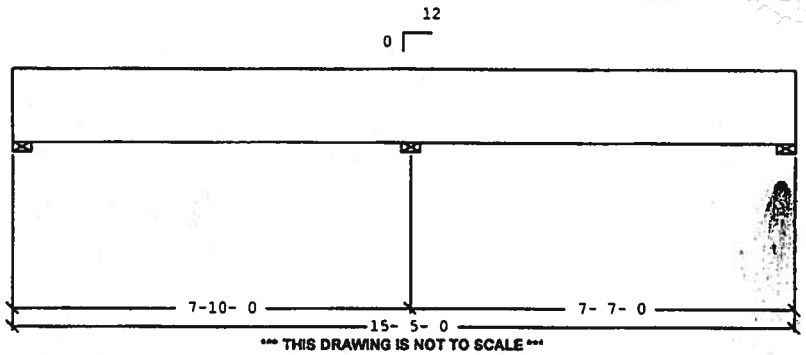
MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 0

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.26"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.02"	0.39"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

08/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 40 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-T2.SPX

Monez Res. / Beam-U FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY.
   MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	80 PLF	00-00-00	07-09-00		1.00
TRAPEZOIDAL	ROOF	DEAD	TOP	80 PLF	07-09-00	14-11-00	0 PLF	0.90
TRAPEZOIDAL	ROOF	LIVE	TOP	80 PLF	07-09-00	14-11-00	0 PLF	1.25
UNIFORM	ROOF	DEAD	TOP	80 PLF	00-00-00	07-09-00		0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	14-11-00		0.90

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.08 VSI: 0.10

LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

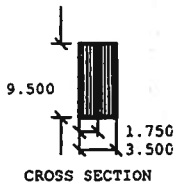
SUPPORT REACTIONS (LBS):

MAXIMUM B E A R I N G	N U M B E R 1	2	3
DOWN	548	1330	77
UPLIFT	---	---	---

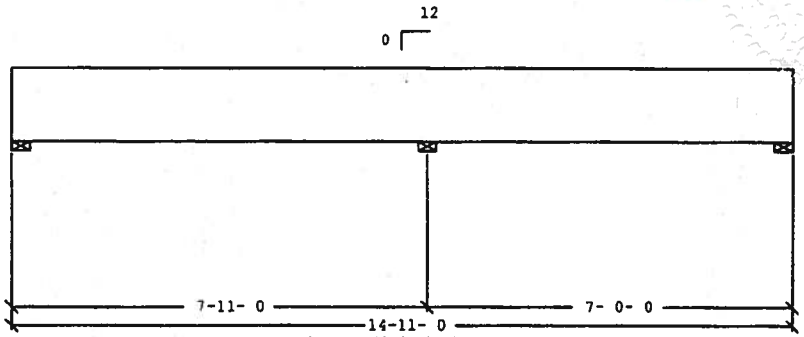
MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 0

MAXIMUM DEFLECTIONS

	CALCULATED	ALLOWABLE
LIVE LOAD	0.01"	0.26"
*DEAD LOAD	0.01"	
TOTAL LOAD	0.02"	0.39"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL and CTR, LP I-joists to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

06/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 41 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-U.SPX

Monez Res. / Beams-V

FL Mayo Truss

2006.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE COMPRESSION EDGE BRACING AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	ROOF	LIVE	TOP	110 PLF	00-00-00	06-02-00		1.00
UNIFORM	ROOF	DEAD	TOP	110 PLF	00-00-00	06-02-00		0.90
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	06-02-00		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

DESIGN CRITERIA : MSI: 0.08 VSI: 0.08

LIVE LOAD	= 20	PSF
DEAD LOAD	= 20	PSF
TOTAL LOAD	= 40	PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

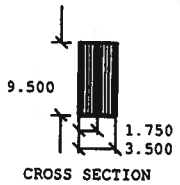
	REPORT #
ICBO	ER-5004
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
NER	622
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

SUPPORT REACTIONS (LBS):

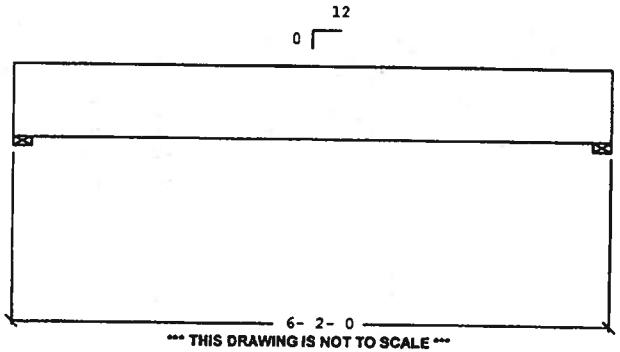
MAXIMUM BEARING NUMBER	1	2
DOWN	708	708
UPLIFT	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 0

		MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE	LOAD	0.01"	0.20"
*DEAD	LOAD	0.01"	
TOTAL	LOAD	0.01"	0.30"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL and CTR, LP I-Joists to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

06/10/06 SBCCI 2006.1

DWG # 0608-049

SHEET # 42 of 42

File: L:\Engineering\Woode 2006\August\0608-049\6036-V.SPX