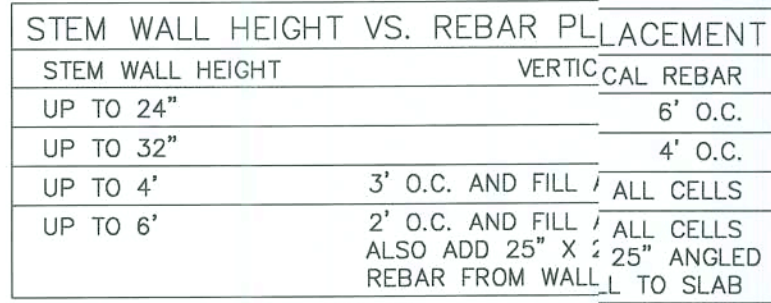


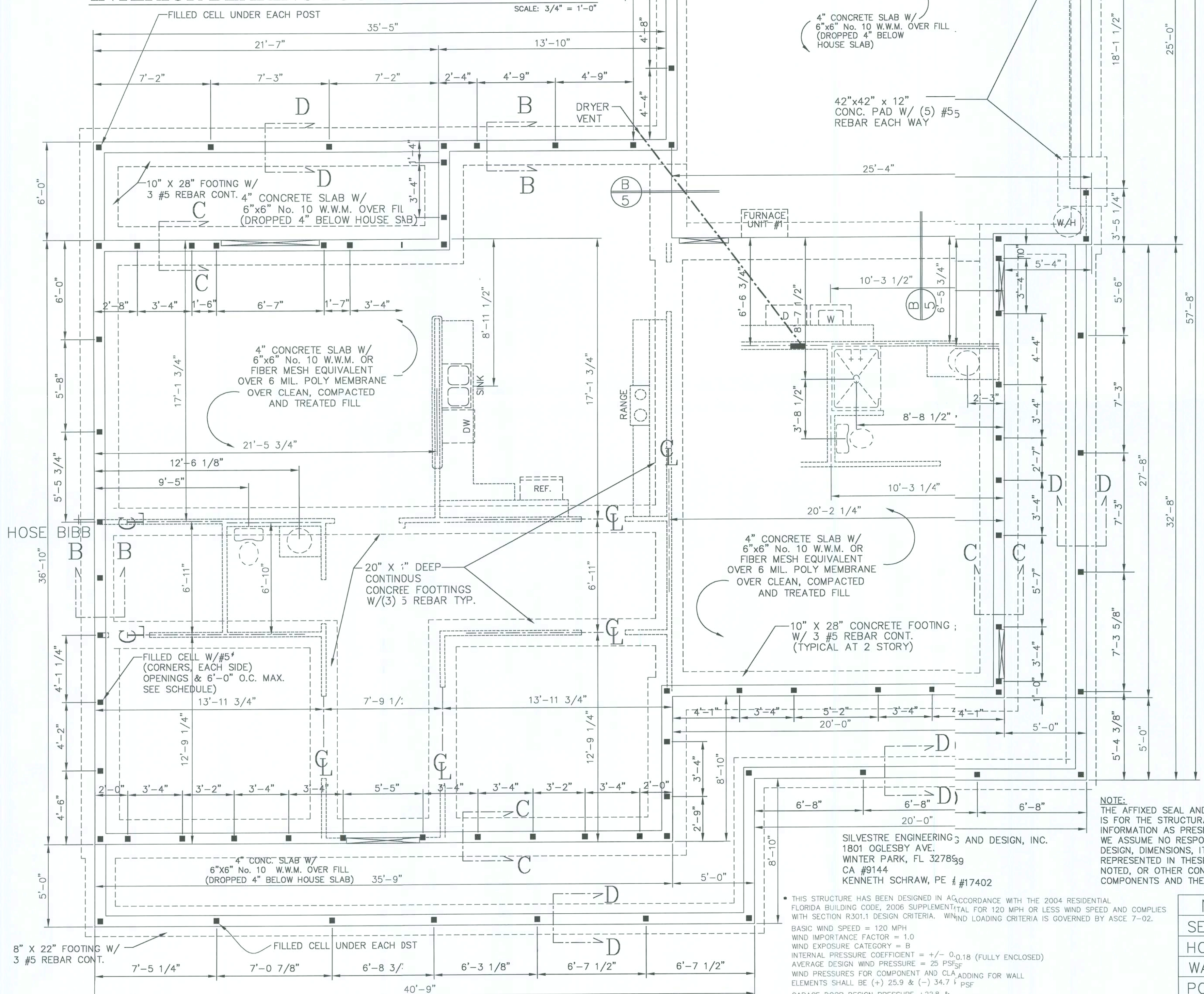
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CONTROL JOINT DETAIL



INTERIOR BEARING FOOTING

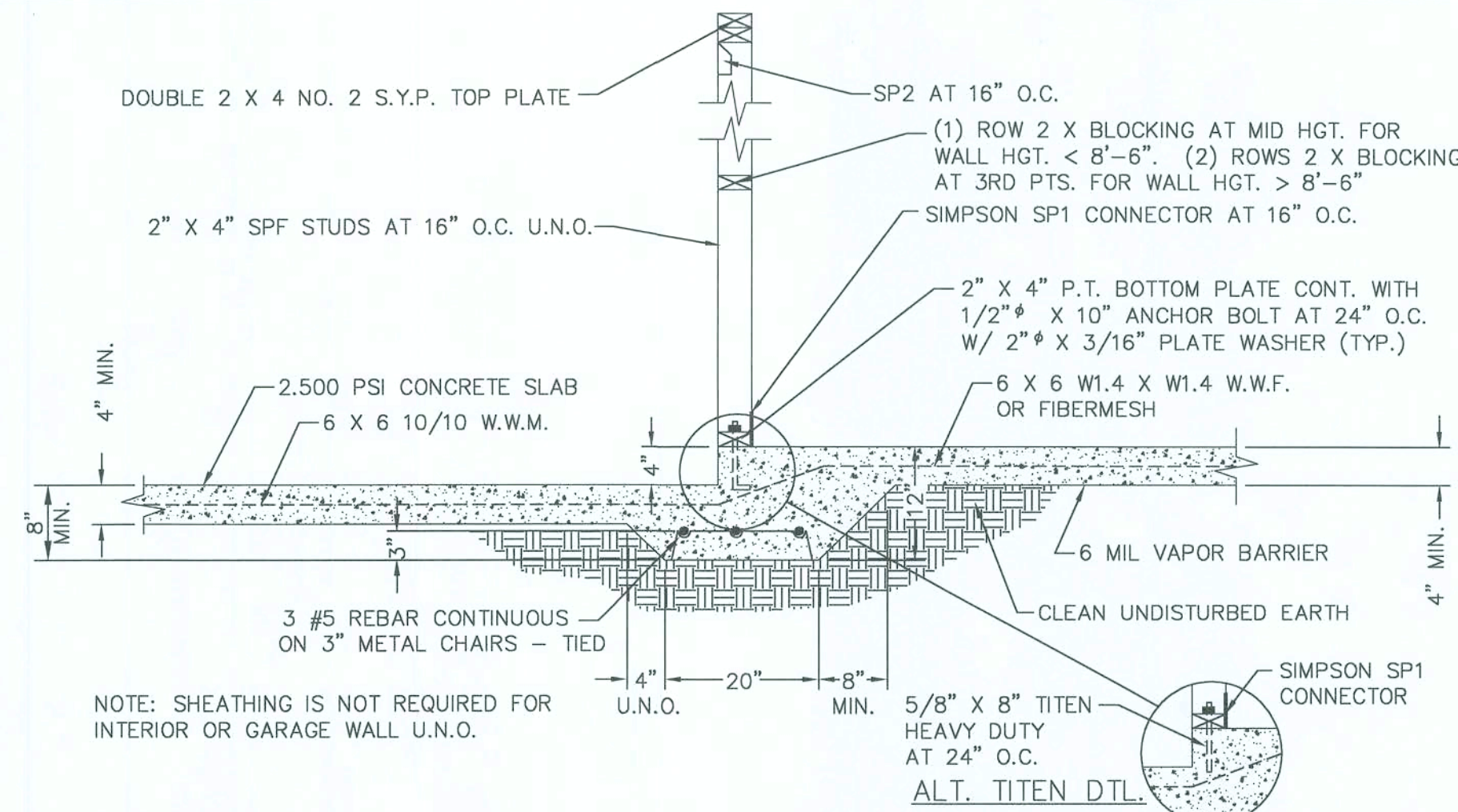


STANDARD CL. YD. FILL _____

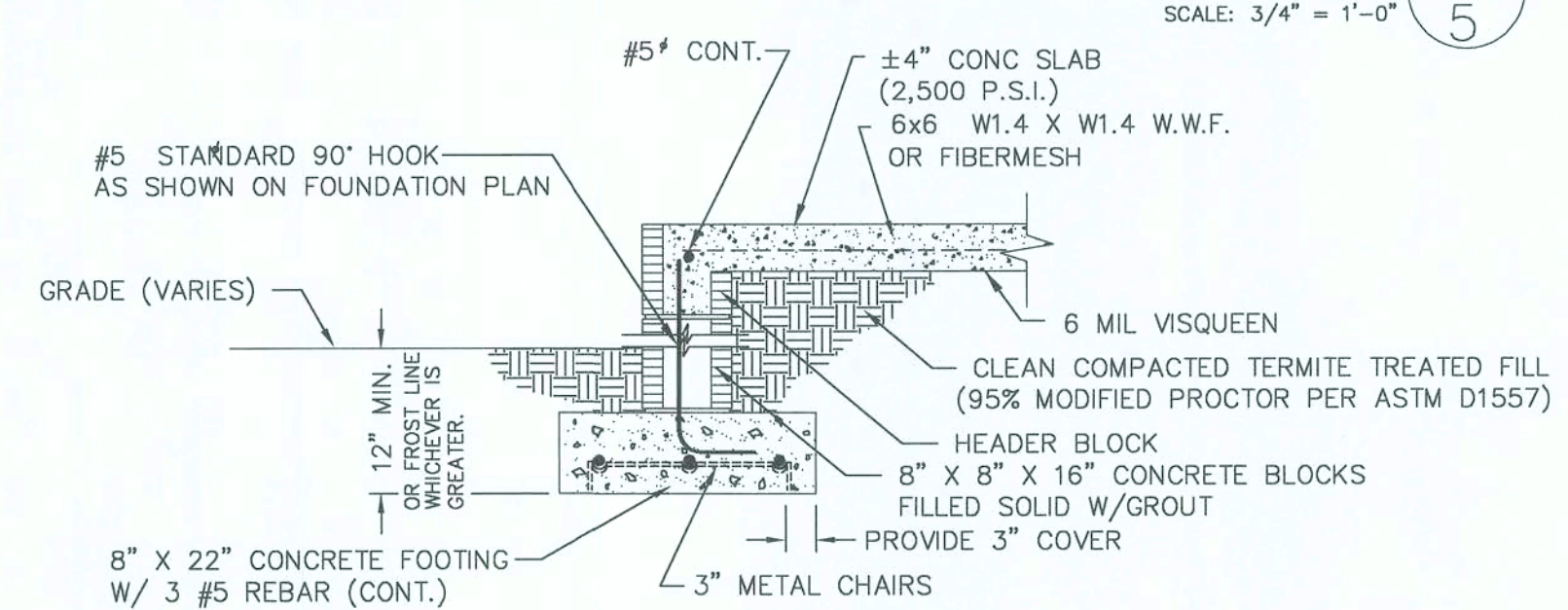
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WINTER PARK, FL 32789g
CA #9144
KENNETH SCHRAW, PE #17402

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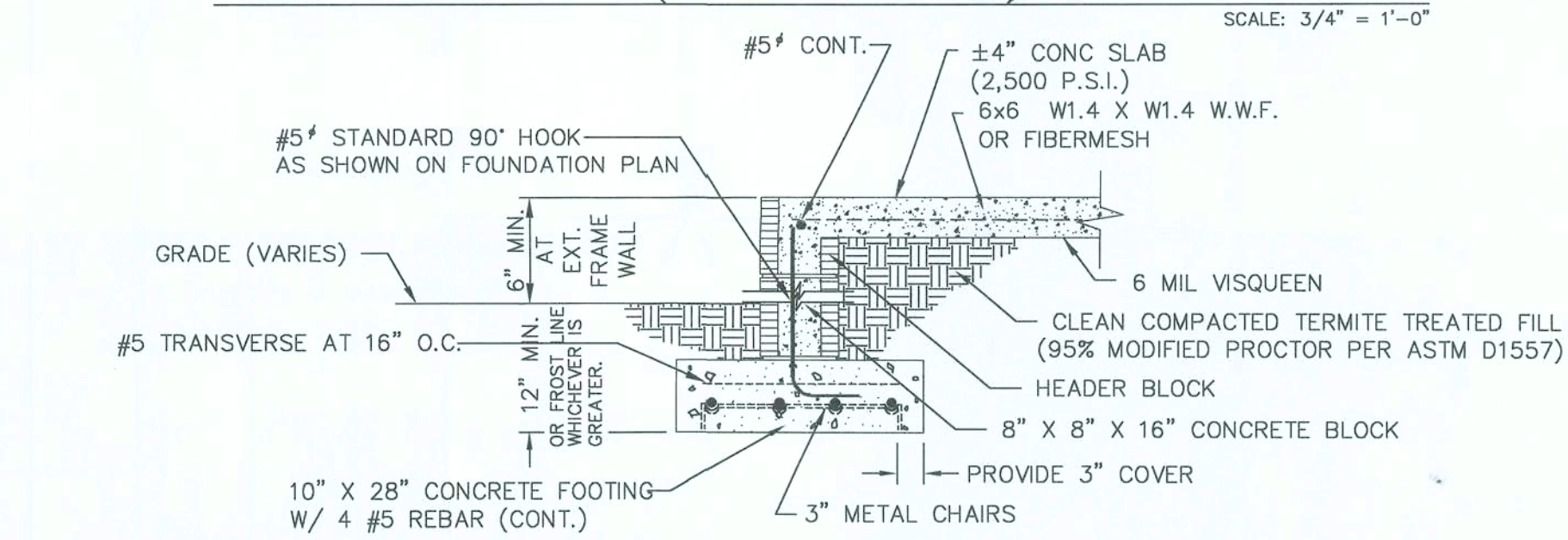
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WIND IMPORTANCE FACTOR = 1.0
WIND EXPOSURE CATEGORY = B
INTERNAL PRESSURE COEFFICIENT = +/- 0.18 (FULLY ENCLOSED)
AVERAGE DESIGN WIND PRESSURE = 25 PSFg
WIND PRESSURES FOR COMPONENT AND CLADDING FOR WALL ELEMENTS SHALL BE (+) 25.9 & (-) 34.7 l/PSF
GARAGE DOOR DESIGN PRESSURE +22.8 & -25.8 PSF
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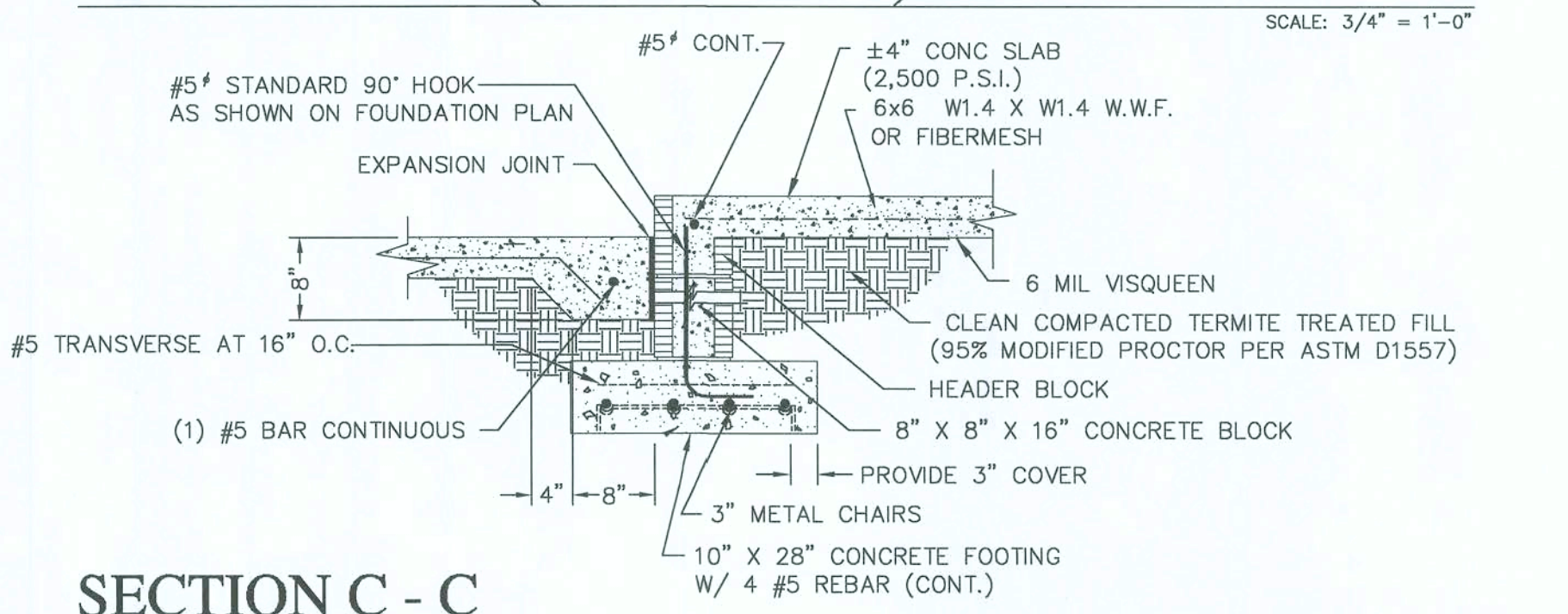
TYPICAL BEARING WALL AT STEP



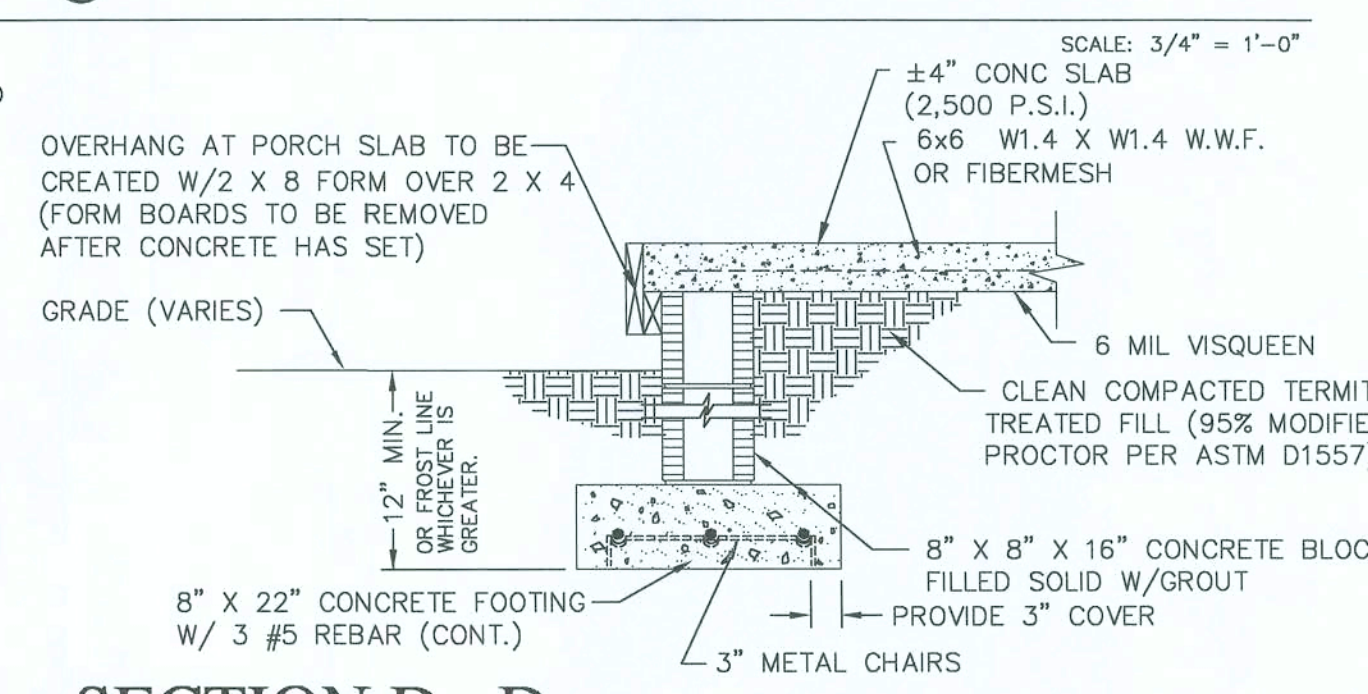
SECTION A - A (ONE STORY)



SECTION B - B (TWO STORY)



SECTION C - C



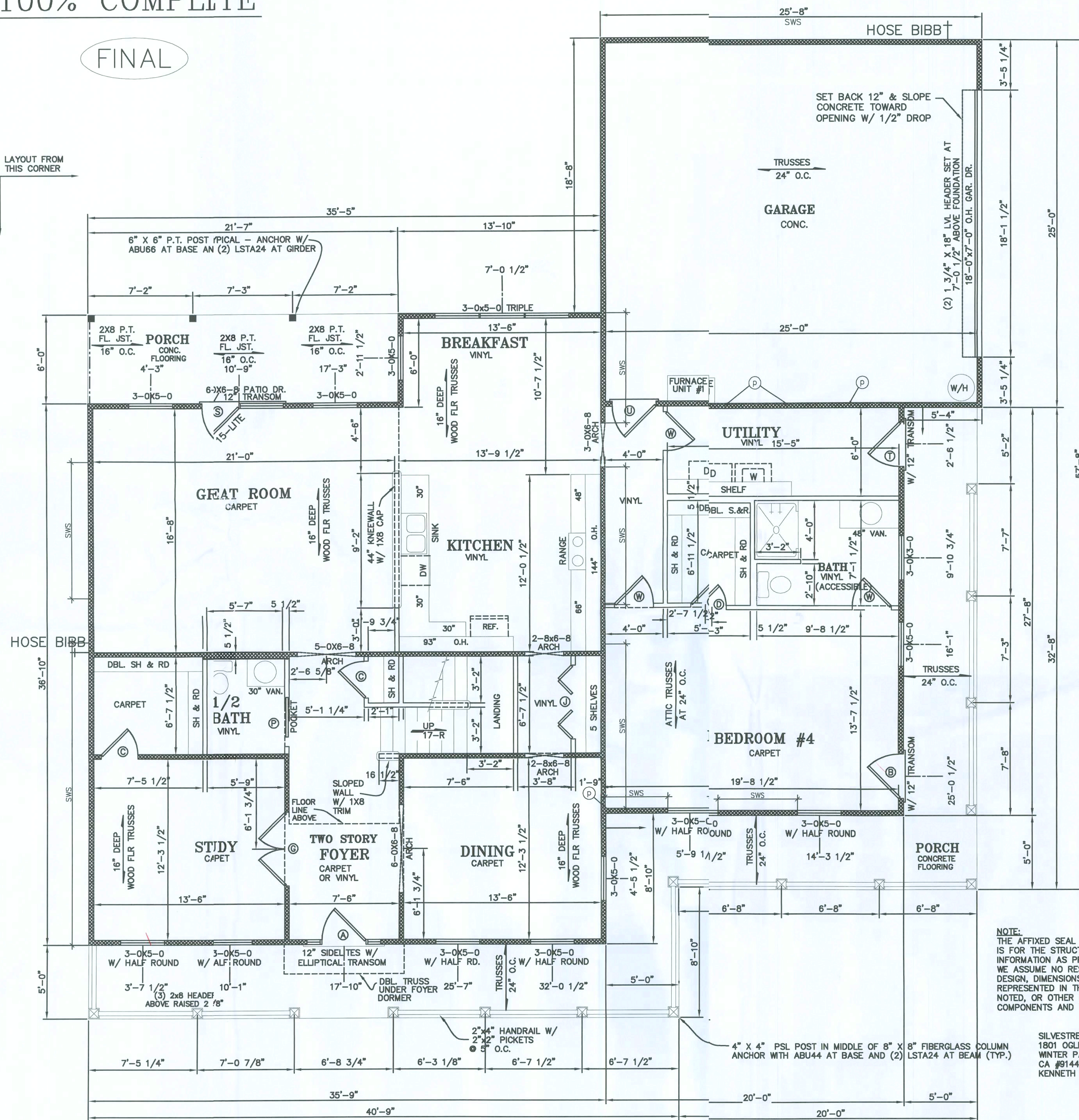
SECTION D - D

SCALE: $3/4" = 1'-0"$

PROPERTY OF AMERICA'S HOME PLACE INC.	THE HANOVER FOR MICHAEL & FADRA McINTOSH	AREAS FIRST FLOOR SECOND FLOOR AREA SECOND FLOOR HEATED TOTAL HEATED BONUS ROOM (UNFINISHED) GARAGE FRONT PORCH REAR PORCHES	DATE 2-28-08 REVISIONS - JDC 4/30/08 1952 S.F. 1344 S.F. 74 S.F. 3380 S.F. 380 S.F. 632 S.F. 488 S.F. 275 S.F.	SILVESTRE ENGINEERING & DESIGN, INC. 1801 OGLESBY AVE. - WINTER PARK - FLORIDA - 32789 PHONE - (407) 544 5859 FAX - (407) 642 4999 CA #9144	2 x 4 EXTERIOR WALLS UNLESS OTHERWISE NOTED FOUNDATION PLAN/DETAILS SCALE: 1/4" = 1'-0" DRAWN BY JDC SOLD BY KIM RICKOILT	SHEET No. 5 OF 15
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FINAL

LAYOUT FROM
THIS CORNER

NOTE: TEMPERED GLASS TO BE USED IN HAZARDOUS LOCATIONS PER FBC 2004 RESIDENTIAL CODE, SECTION R308.8.

LOWER LEVEL FLOOR PLAN

SCALE: 1/4" = 1'-0"

SYMB	DOORS	ROUGH OPENING	
		WIDTH	HEIGHT
A	3'-0" X 6'-8" SIX PANEL W/SIDE LITES & TRANSOM	BY MANUF.	BY MANUF.
B	3'-0" X 6'-8" 15 LITE EXTERIOR	3'- 2 3/4"	7'-10 1/2"
C	2'-6" X 6'-8" INTERIOR	2'- 8 1/2"	6'-10 1/2"
F	2'-4" X 6'-8" INTERIOR	2'- 6 1/2"	6'-10 1/2"
G	5'-0" X 6'-8" INTERIOR 15 LITE FRENCH	5'- 2 1/2"	6'-10 1/2"
H	3'-0" X 6'-8" INTERIOR BI-FOLD	3'- 2"	6'-10 1/2"
J	5'-0" X 6'-8" INTERIOR BI-FOLD	5'- 2"	6'-10 1/2"
P	2'-8" X 6'-8" INTERIOR POCKET	BY MANUF.	BY MANUF.
S	6'-0" X 6'-8" EXTERIOR PATIO 15 LITE	6'- 2 3/4"	6'-10 1/2"
T	3'-0" X 6'-8" EXTERIOR 9 LITE	3'- 2 3/4"	6'-10 1/2"
U	3'-0" X 6'-8" EXTERIOR SIX PANEL	3'- 2 3/4"	6'-10 1/2"
V	2'-8" X 6'-8" EXTERIOR SIX PANEL	2'-10 3/4"	6'-10 1/2"
W	2'-8" X 6'-8" INTERIOR	2'- 10 1/2"	6'-10 1/2"

NOTE: HEIGHT OF ROUGH OPENING IS MEASURED FROM FINISH FLOOR.

WINDOWS	ROUGH OPENING	
	WIDTH	HEIGHT
2040 (FRAME)	23 3/4"	47 3/4"
3050 (FRAME) - EGRESS	35 3/4"	59 3/4"
4040 (FRAME)	47 3/4"	47 3/4"
3040 (FRAME)	35 3/4"	47 3/4"
3010 (FRAME)+ROUND TOP	35 3/4"	BY MANUF.
2810 (FRAME)+ROUND TOP	31 3/4"	BY MANUF.
3050 TRIPLE (FRAME)	BY MANUF.	59 3/4"

NOTE: VERIFY ALL OPENING SIZES WITH MANUFACTURER.
NOTE: EGRESS WINDOWS HAVE A CLEAR OPENING OF
32 1/2" W. X 25 9/16" H.

NOTE: CLOSET SHELF
HGT. OFF FLOOR=
65" (SGL.)
42" & 84" (DBL.)

NOTE: 9'-1 1/8" HIGH
CEILING ON 1st FLOOR

FRAMING INSPECTION
* ALL PLUMBING, ELECTRICAL, AND MECHANICAL
ROUGH-INS MUST BE COMPLETED, INSPECTED, AND
APPROVED BEFORE REQUESTING THE FRAMING
INSPECTION (PER FBC 105.0).

INDICATES INTERIOR / EXTERIOR
FRAME BEARING WALL

Ⓟ - DENOTES APPROX. POST LOCATION - REF. TRUSS
ENCL. FOR EXACT SIZE AND LOCATION

SWS - SHEARWALL SEGMENT - REF. STRUCTURAL DETAILS

NOTE: SITE DROP	
SEWER	—
HOSE BIBB	+
WATER LINE	---
POWER METER	Ⓜ
GAS LINE	I
UNDERGROUND POWER	—
OVERHEAD POWER	—

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BASIC WIND SPEED = 120 MPH
WIND IMPORTANCE FACTOR = 1.0
WIND EXPOSURE CATEGORY = B
INTERNAL PRESSURE COEFFICIENT = +/- 0.18 (FULLY ENCLOSED)
AVERAGE DESIGN WIND PRESSURE = 25 PSF
WIND PRESSURES FOR COMPONENT AND CLADDING FOR WALL
ELEMENTS SHALL BE (+) 25.8 & (-) 34.7 PSF
GARAGE DOOR DESIGN PRESSURE +22.6 & -25.8 PSF

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2 x 4 EXTERIOR WALLS
UNLESS OTHERWISE NOTEDLOWE'S F.V.F.F.L.
FLOOR PLAN 1
SCALE: 1/4" = 1'-0"DRAWN BY JDC
SOLD BY KIM RICKOLT
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AREAS

THE HANOVER

PROPERTY OF

AMERICA'S HOME PLACE INC. FOR MICHAEL & FADRA

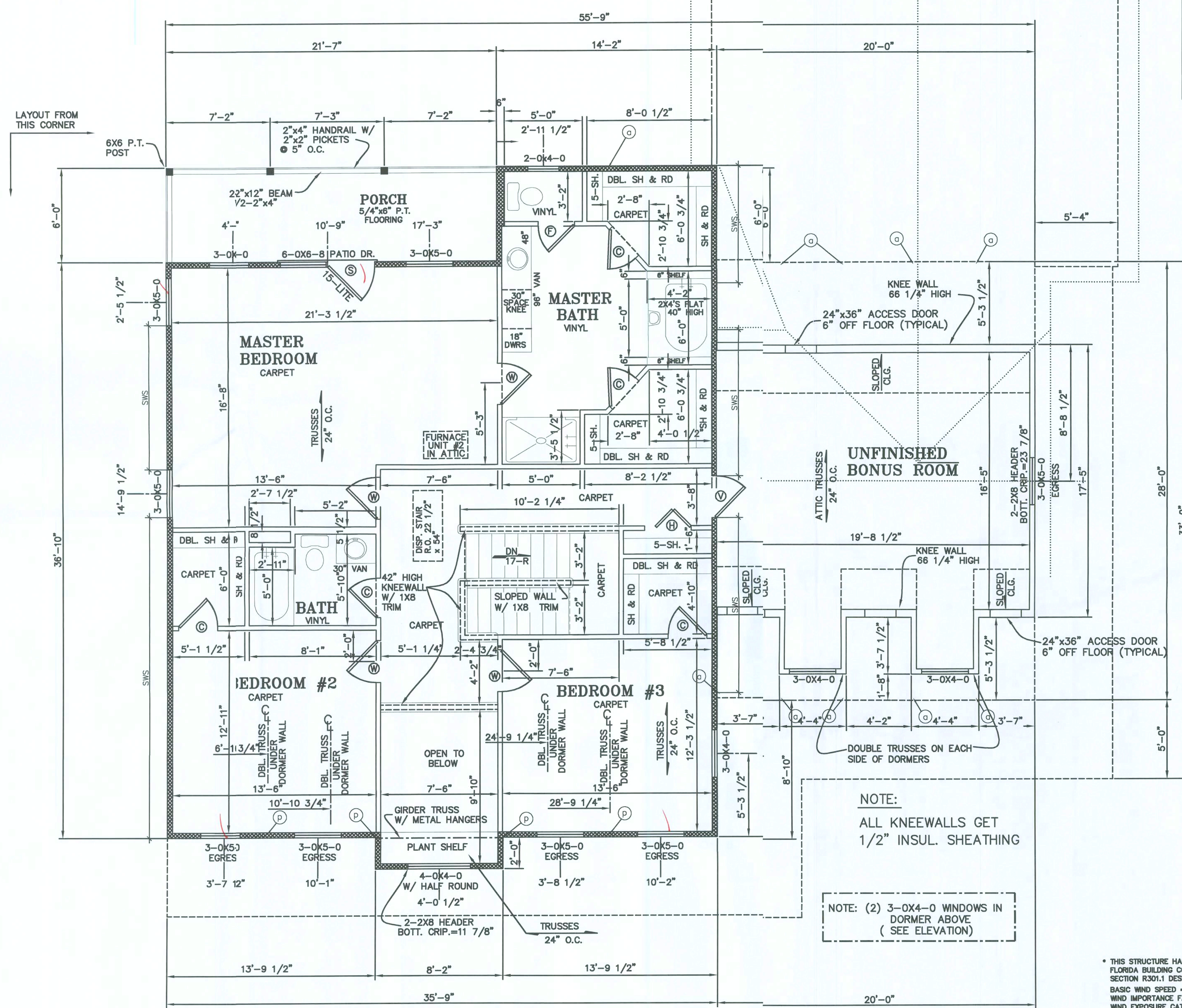
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UPPER LEVEL FLOOR PLAN

SCALE: 1/4" = 1'-0"

SYMB	DOORS	ROUGH OPENING	
		WIDTH	HEIGHT
A	3'-0" X 6'-8" SIX PANEL W/SIDE LITES & TRANSOM	BY MANUF.	BY MANUF.
B	3'-0" X 6'-8" 15 LITE EXTERIOR	3'- 2 3/4"	7'-10 1/2"
C	2'-6" X 6'-8" INTERIOR	2'- 8 1/2"	6'-10 1/2"
F	2'-4" X 6'-8" INTERIOR	2'- 6 1/2"	6'-10 1/2"
G	5'-0" X 6'-8" INTERIOR 15 LITE FRENCH	5'- 2 1/2"	6'-10 1/2"
H	3'-0" X 6'-8" INTERIOR BI-FOLD	3'- 2"	6'-10 1/2"
J	5'-0" X 6'-8" INTERIOR BI-FOLD	5'- 2"	6'-10 1/2"
P	2'-8" X 6'-8" INTERIOR POCKET	BY MANUF.	BY MANUF.
S	6'-0" X 6'-8" EXTERIOR PATIO 15 LITE	6'- 2 3/4"	6'-10 1/2"
T	3'-0" X 6'-8" EXTERIOR 9 LITE	3'- 2 3/4"	6'-10 1/2"
U	3'-0" X 6'-8" EXTERIOR SIX PANEL	3'- 2 3/4"	6'-10 1/2"
V	2'-8" X 6'-8" EXTERIOR SIX PANEL	2'-10 3/4"	6'-10 1/2"
W	2'-8" X 6'-8" INTERIOR	2'- 10 1/2"	6'-10 1/2"

NOTE: HEIGHT OF ROUGH OPENING IS MEASURED FROM FINISH FLOOR.

WINDOWS	ROUGH OPENING	
	WIDTH	HEIGHT
2040 (FRAME)	23 3/4"	47 3/4"
3050 (FRAME) - EGRESS	35 3/4"	59 3/4"
4040 (FRAME)	47 3/4"	47 3/4"
3040 (FRAME)	35 3/4"	47 3/4"
3010 (FRAME)+ROUND TOP	35 3/4"	BY MANUF.
2810 (FRAME)+ROUND TOP	31 3/4"	BY MANUF.
3050 TRIPLE (FRAME)	BY MANUF.	59 3/4"

NOTE: VERIFY ALL OPENING SIZES WITH MANUFACTURER.
NOTE: EGRESS WINDOWS HAVE A CLEAR OPENING OF 32 1/2" W. X 25 9/16" H.

NOTE: 8'-1 1/8" HIGH CEILING

NOTE: CLOSET SHELF HGT. OFF FLOOR= 65" (SGL.) 42" & 84" (DBL.)

FRAMING INSPECTION
ALL PLUMBING, ELECTRICAL AND MECHANICAL ROUGH-INS MUST BE COMPLETED, INSPECTED, AND APPROVED BEFORE REQUESTING THE FRAMING INSPECTION (PER FBC 105.6).
INDICATES INTERIOR / EXTERIOR FRAME BEARING WALL
(P) - DENOTES APPROX. POST LOCATION - REF. TRUSS ENG. FOR EXACT SIZE AND LOCATION
SWS - SHEARWALL SEGMENT - REF. STRUCTURAL DETAILS

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NOTE: SITE DROP	
SEWER	—
HOSE BIBB	+
WATER LINE	---
POWER METER	⏏
GAS LINE	—
UNDERGROUND POWER	—
OVERHEAD POWER	—



2 x 4 EXTERIOR WALLS
UNLESS OTHERWISE NOTED

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CA #9144

DATE 2-28-08
REVISIONS
1992 S.F.
174 S.F.
3380 S.F.
3380 S.F.
488 S.F.
275 S.F.

AREAS
FIRST FLOOR
SECOND FLOOR
BONUS ROOM (UNFINISHED)
REAR PORCH

THE HANOVER
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FOR MICHAEL & FADRA MCINTOSH
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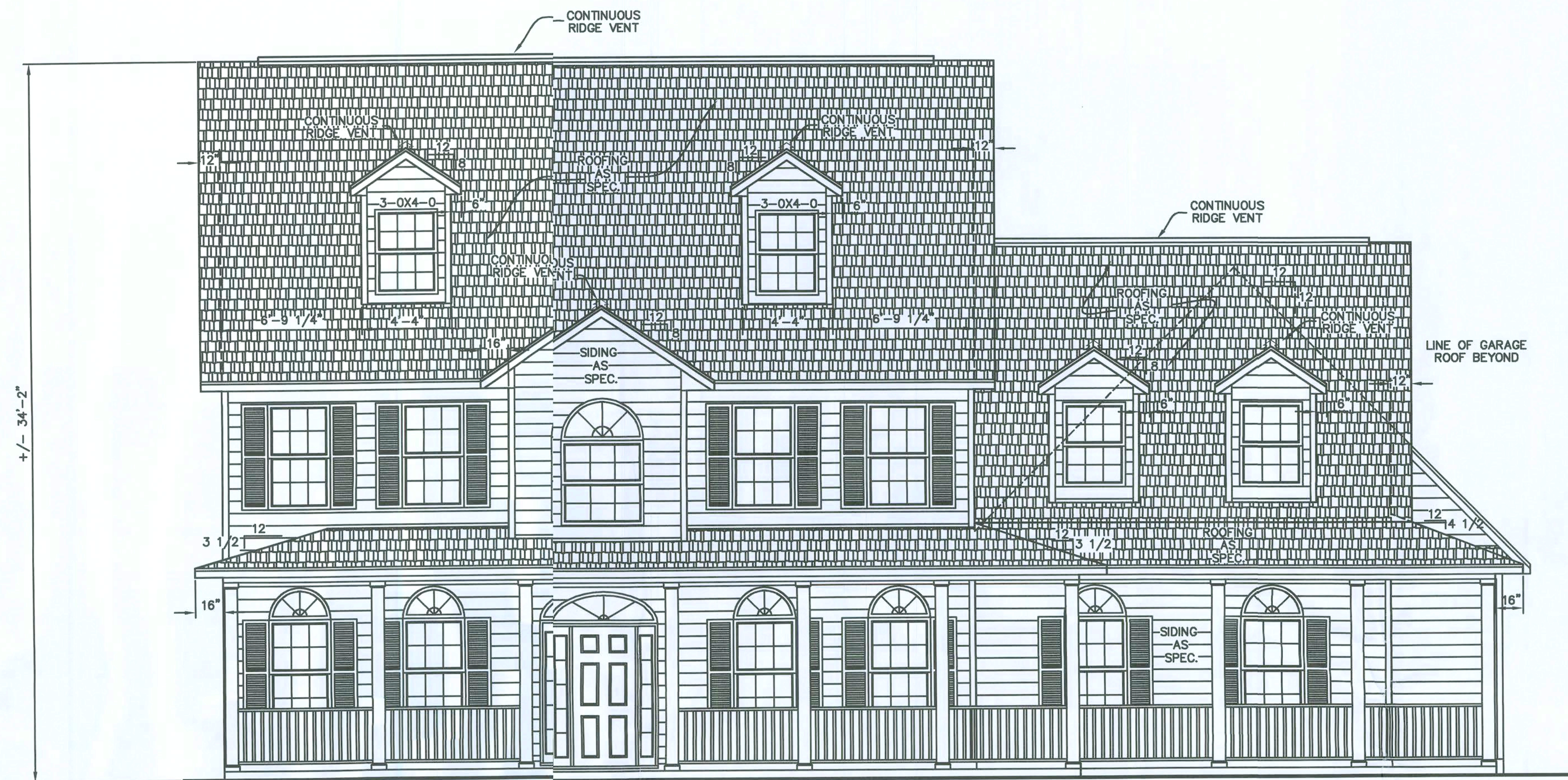
UPPER LEVEL FLOOR PLAN
FLOOR PLAN 2
SCALE: 1/4" = 1'-0"
DRAWN BY JDC
SOLD BY KIM RICKOLT

15 of

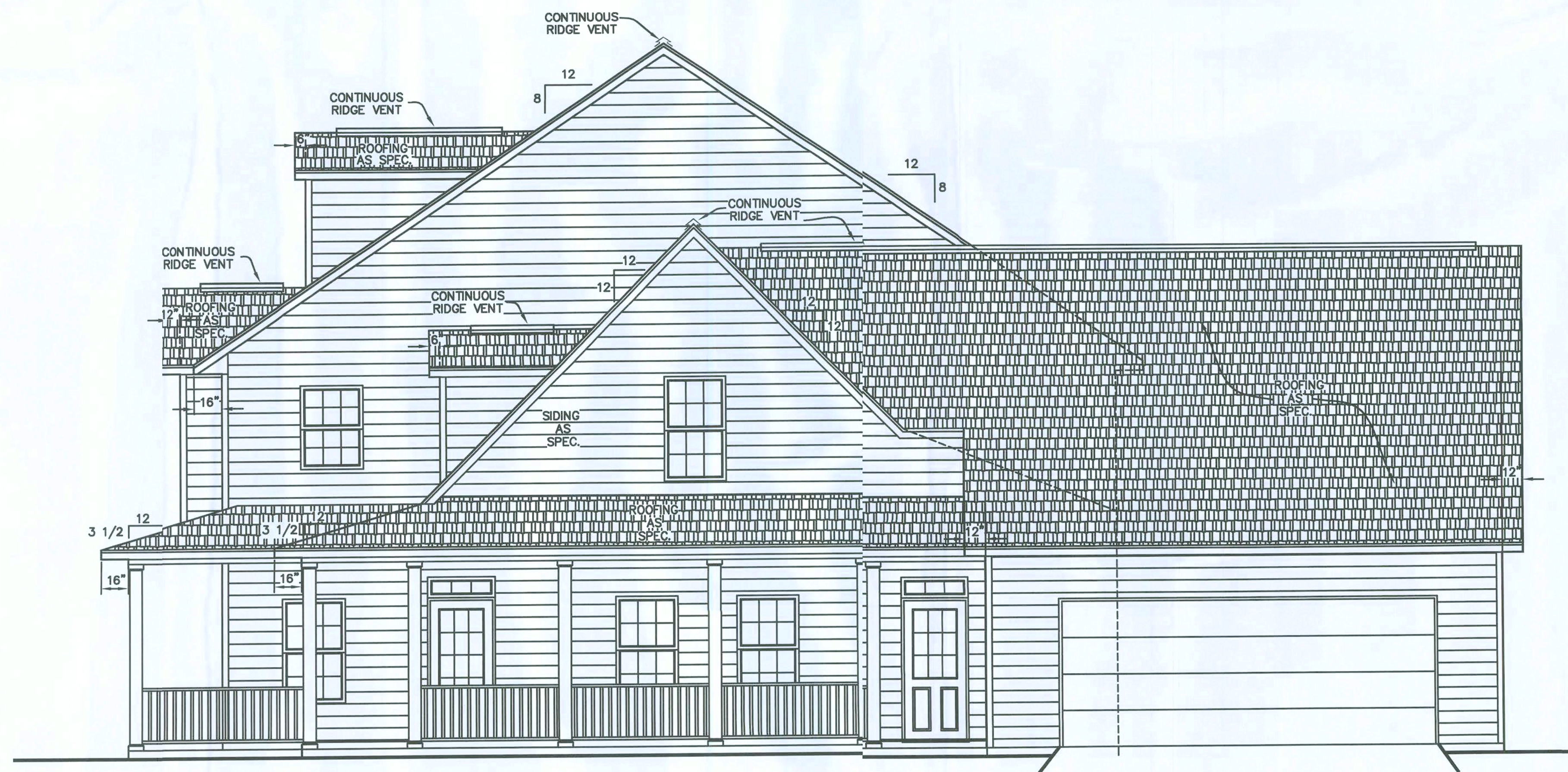
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FRONT ELEVATION



RIGHT SIDE ELEVATION

ELEVATIONS

SCALE: 1/4" = 1'-0"

8/12 ROOF TRUSS
O.W.H.= 6"
12/12 ROOF TRUSS
O.W.H.= 20 5/8"
PORCH BEAM RAISED = 2 5/8"

NOTE:
GRADE LINE SHOWN ON PLANS IN
NO WAY REFLECTS FINAL GRADE

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2 x 4 EXTERIOR WALLS
UNLESS OTHERWISE NOTED

ELEVATIONS 3
of 15

SCALE: 1/4" = 1'-0"
DRAWN BY JDC
SOLD BY KIM RICKOLT

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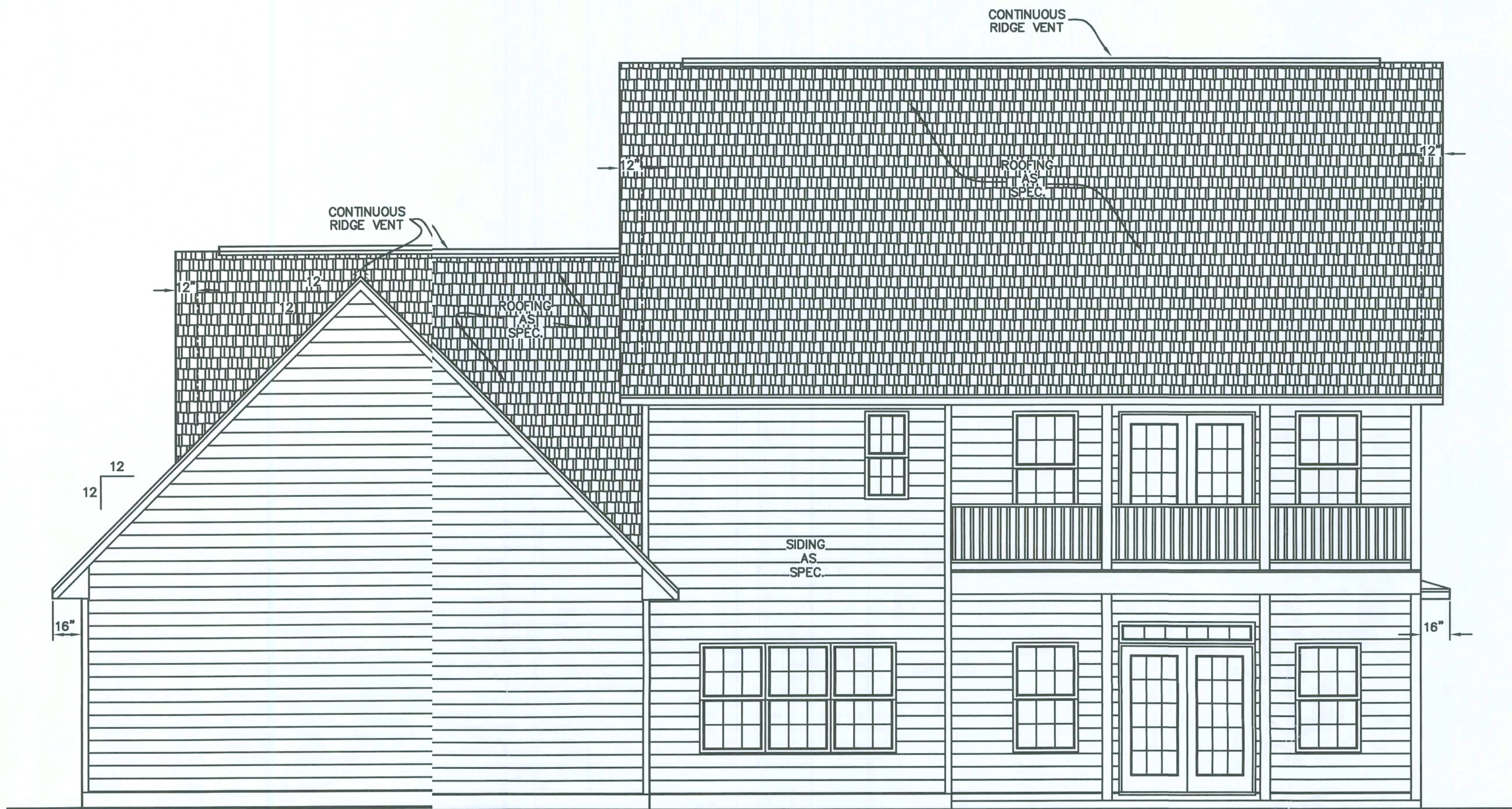
DATE	2-28-08
REVISIONS	
1982 S.F.	1374 S.F.
FIRST FLOOR	SECOND FLOOR
1374 S.F.	3380 S.F.
SECOND FLOOR AREA	BONUS ROOM (UNFINISHED)
3380 S.F.	633 S.F.
BONUS ROOM (UNFINISHED)	FRONT PORCH
633 S.F.	488 S.F.
FRONT PORCH	REAR PORCHES
488 S.F.	276 S.F.

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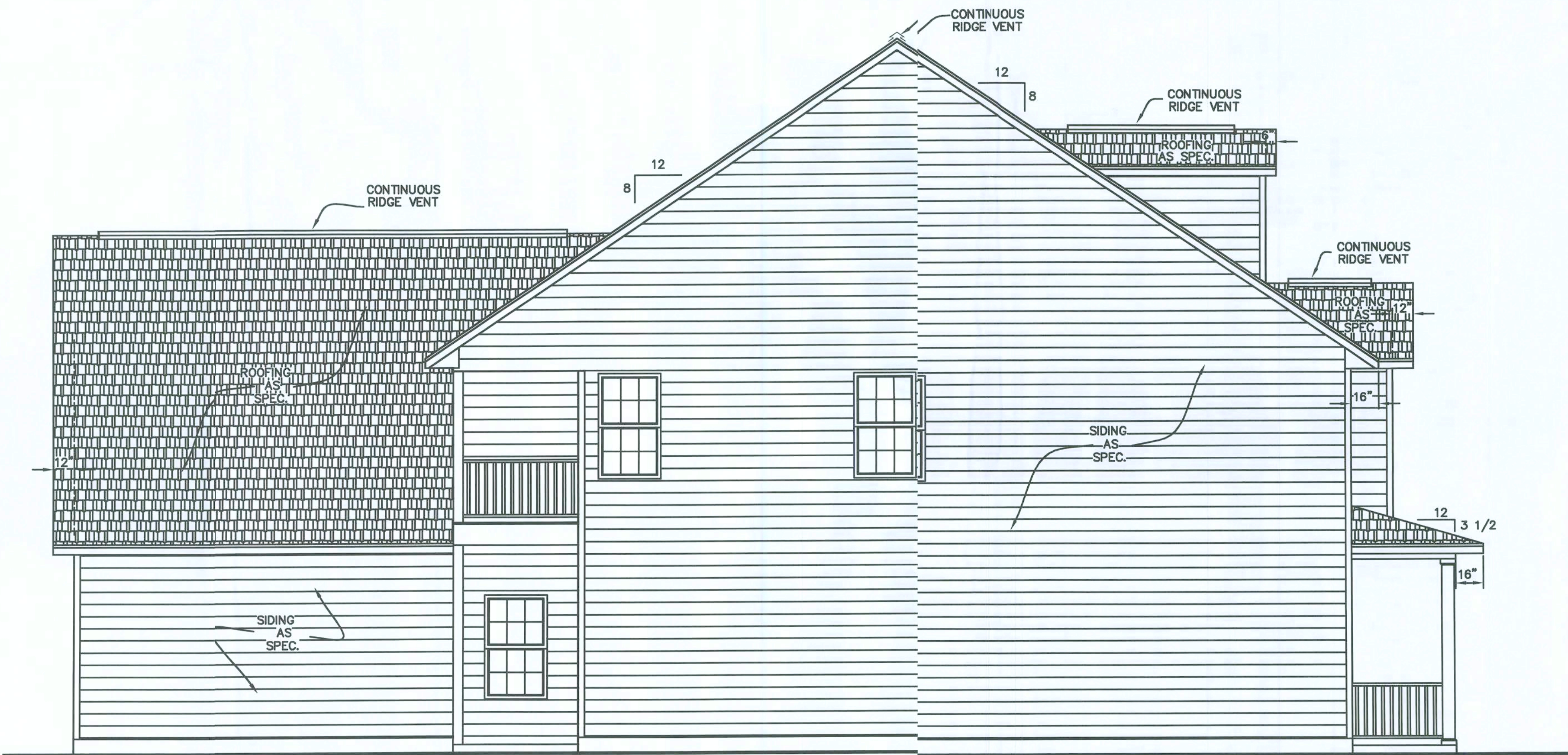
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REAR ELEVATION



LEFT SIDE ELEVATION

ELEVATIONS

SCALE: 1/4" = 1'-0"

8/12 ROOF TRUSS
O.W.H.= 6"
12/12 ROOF TRUSS
O.W.H.= 20 5/8"
PORCH BEAM RAISED = 2 5/8"

NOTE:
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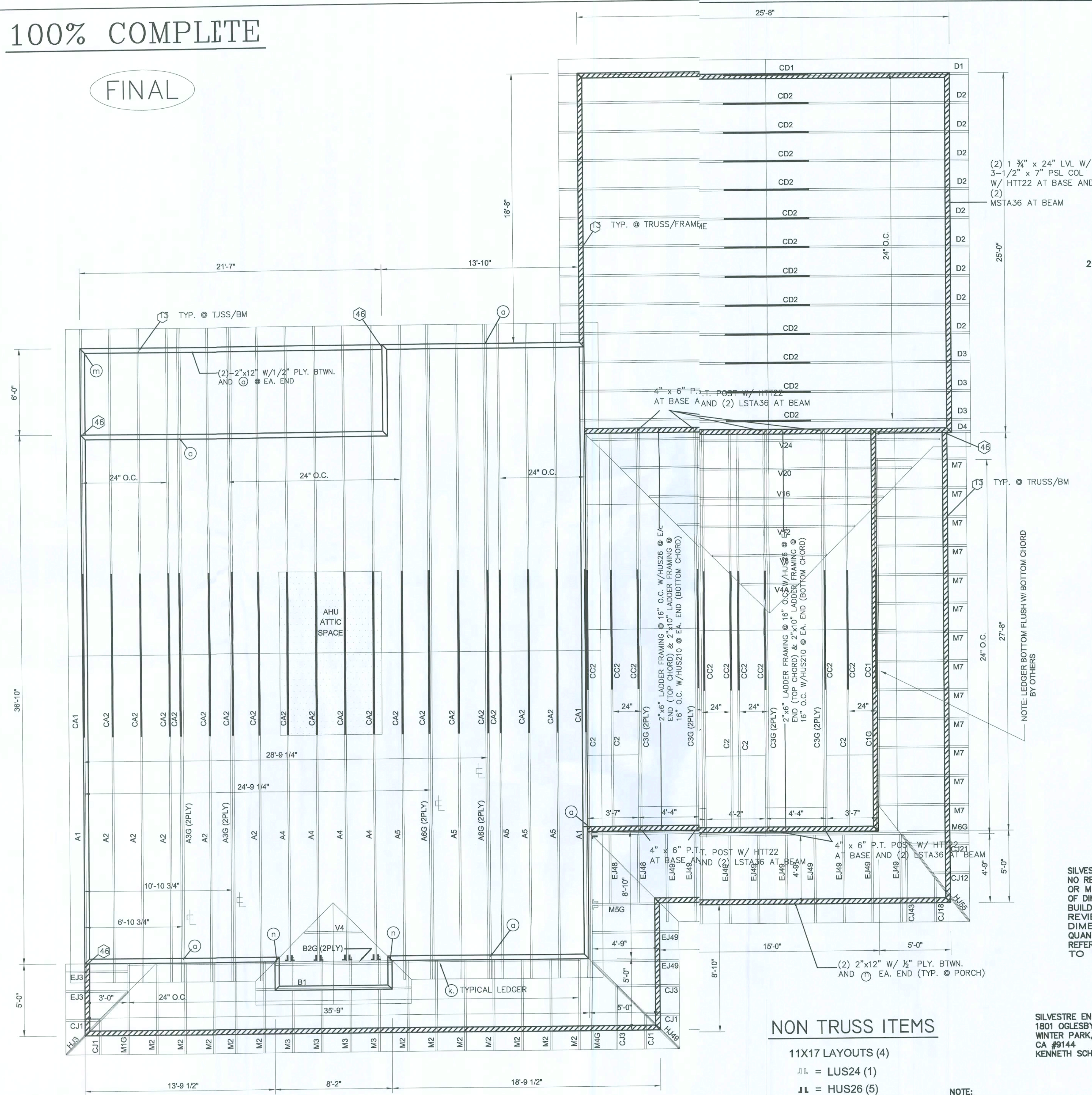
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PROPERTY OF AMERICA'S HOME PLACE INC.	THE HANOVER FOR MICHAEL & FADRA McINTOSH	DATE 2-28-08 REVISIONS	1992 S.F. 1344 S.F. 3380 S.F. 3380 S.F. 633 S.F. 486 S.F. 278 S.F.	AREAS FIRST FLOOR SECOND FLOOR TOTAL HEATED TOTAL ROOM GARAGE FRONT PORCH REAR PORCHES	1993 ALL RIGHTS RESERVED -	PROPERTY OF AMERICA'S HOME PLACE INC.	2 x 4 EXTERIOR WALLS UNLESS OTHERWISE NOTED	SHEET No. ELEVATIONS 4	SCALE: 1/4" = 1'-0"	15 of
		SILVESTRE ENGINEERING & DESIGN, INC. 1801 OGLESBY AVE. - WINTER PARK - FLORIDA - 32789 PHONE - (407) 644 5859 FAX - (407) 647 5758 CA #9144		© THESE DRAWINGS ARE DESIGNED AND THE JOINT PROPERTY OF SILVESTRE ENGINEERING & DESIGN, INC. AND NO PART OF THIS PROJECT OR INFORMATION OR INFORMATION HEREIN IS TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, WITHOUT PERMISSION IN WRITING FROM SILVESTRE ENGINEERING & DESIGN, INC.		DRAWN BY SOLD BY		JDC KIM RICKOLT		

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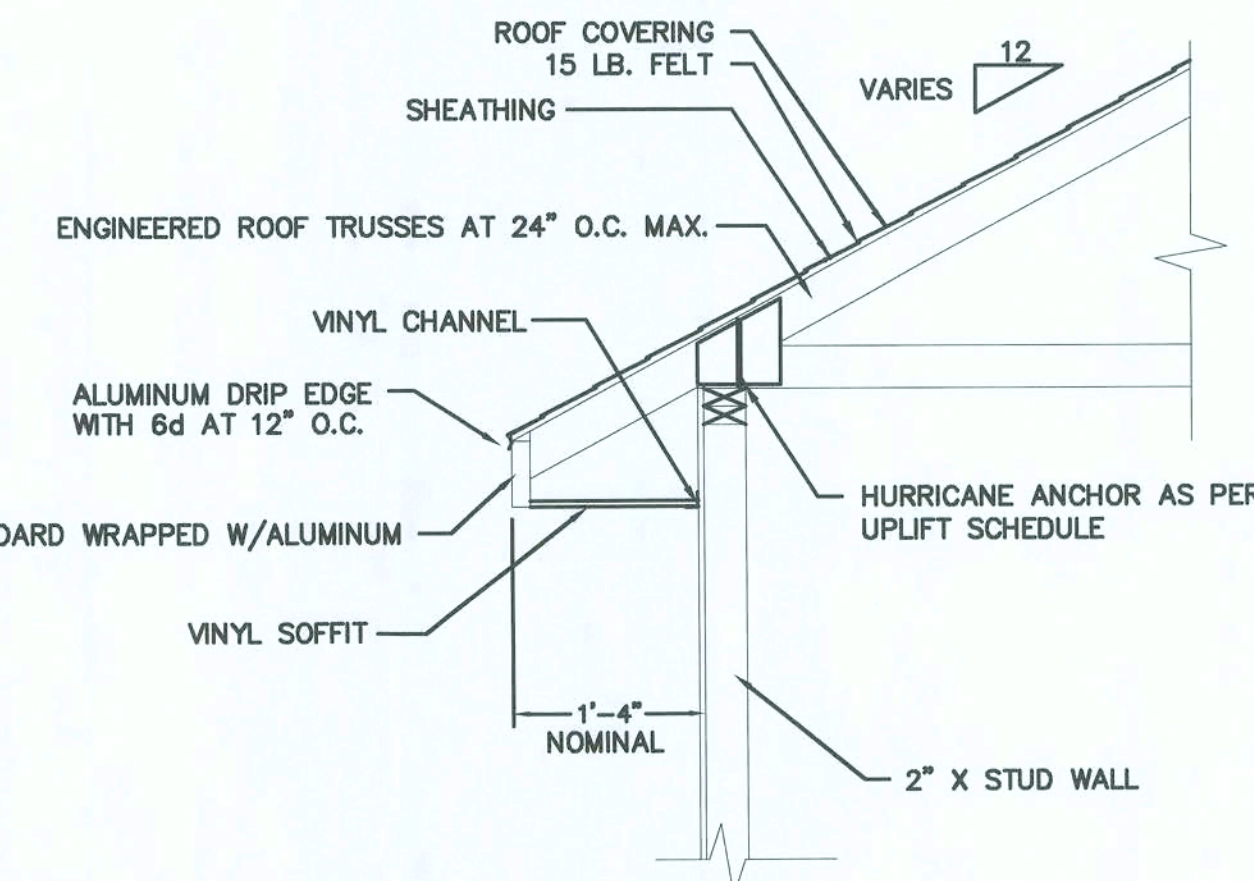


NON TRUSS ITEMS

- 11X17 LAYOUTS (4)
 11 = LUS24 (1)
 11 = HUS26 (5)

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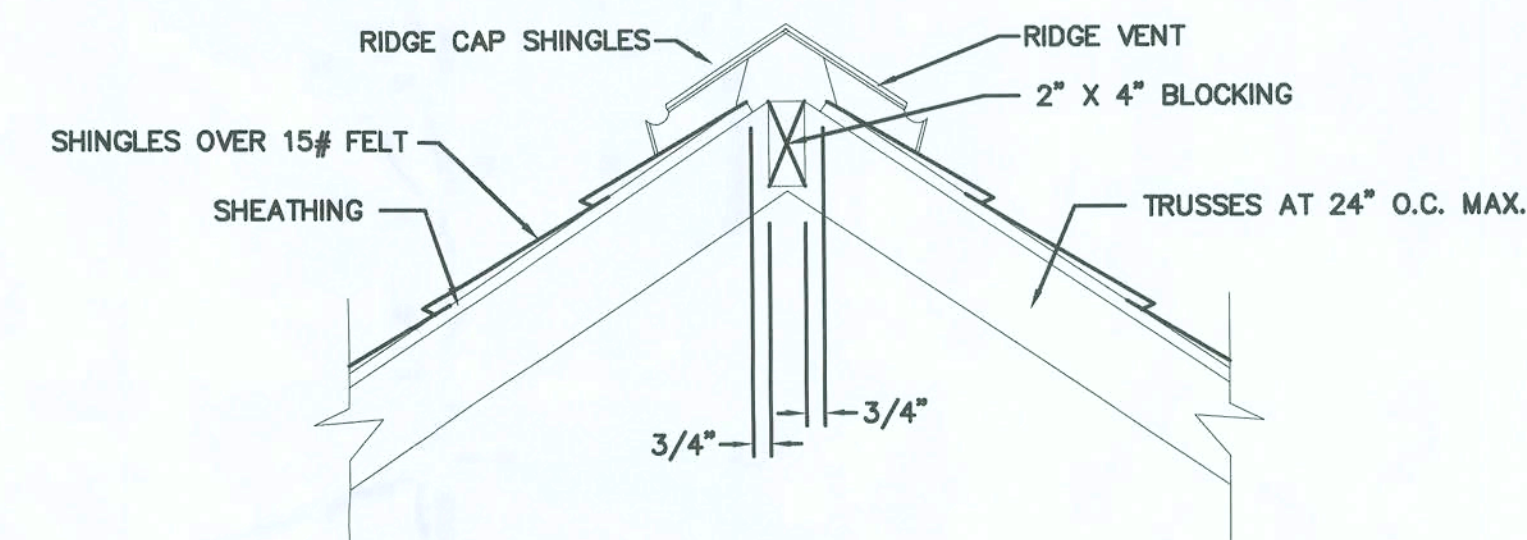
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 CA #9144
 KENNETH SCHRAW, PE #17402



BOXED EAVES DETAIL

* MITERED CUTS TO BE MADE AT ALL JOINTS OF FASCIA.
 * REFER TO DETAIL B/12 FOR STRUCTURAL INFORMATION.

SCALE: 3/4" = 1'-0"



RIDGE VENT DETAIL

NOTE: 3/4" CUT OUT ON BOTH SIDES OF RIDGE.
 CUT OUTS SHOULD STOP 12" FROM END OF RIDGE VENT ON EACH END.

SCALE: 1-1/2" = 1'-0"

NOTES:

- 1.) REF. ATTACHED CONNECTOR SCHEDULE.
- 2.) UNLESS OTHERWISE NOTED - ALL INTERIOR BEARING HEADERS TO BE (2) 2" X 12" WITH 1/2" PLYWOOD BETWEEN AND @ EACH END.
- 3.) ALL STRUCTURAL FRAMING TO BE MINIMUM NO. 2 SOUTHERN YELLOW PINE.
- 4.) TRUSS/TRUSS CONNECTOR PER TRUSS MANUFACTURER.
- 5.) IF ANY STRUCTURAL CONDITION SHOULD OCCUR IN THE FIELD THAT IS NOT NOTED ON THESE DRAWINGS, CONTRACTOR MUST NOTIFY ENGINEER OF RECORD IN WRITING IMMEDIATELY AND PRIOR TO ANY FURTHER CONSTRUCTION COMMENCING IN AFFECTED AREA(S).

TRUSS FRAMING NOTES

1. ALL CONNECTIONS PER SOMPSON CATALOG (LATEST EDITION). CONTACT ENGINEER FOR ALTERNATE CONNECTIONS.
2. PRE-ENGINEERED WOOD TRUSSES SPACED AT 24" O/C TYPICAL, UNLESS OTHERWISE NOTED.
3. SEE FLOOR PLAN AND ELEVATIONS FOR ADDITIONAL ROOF INFORMATION.

TRUSS ENGINEERING AND LAYOUTS SHOWN HERE WERE PRODUCED BY:
 UNIVERSAL FOREST PRODUCTS
 ON: 03 / 21 / 2008

* THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2004 RESIDENTIAL FLORIDA BUILDING CODE, 2008 SUPPLEMENTAL FOR 120 MPH OR LESS WIND SPEED AND COMPLIES WITH SECTION R301.1 DESIGN CRITERIA. WIND LOADING CRITERIA IS GOVERNED BY ASCE 7-02.

BASIC WIND SPEED = 120 MPH
 WIND IMPORTANCE FACTOR = 1.0
 WIND EXPOSURE CATEGORY = B
 INTERNAL PRESSURE COEFFICIENT = +/- 0.18 (FULLY ENCLOSED)
 AVERAGE DESIGN WIND PRESSURE = 25 PSF
 WIND PRESSURES FOR COMPONENT AND CLADDING FOR WALL ELEMENTS SHALL BE (+) 25.9 & (-) 34.7 PSF
 GARAGE DOOR DESIGN PRESSURE +22.8 & -25.8 PSF

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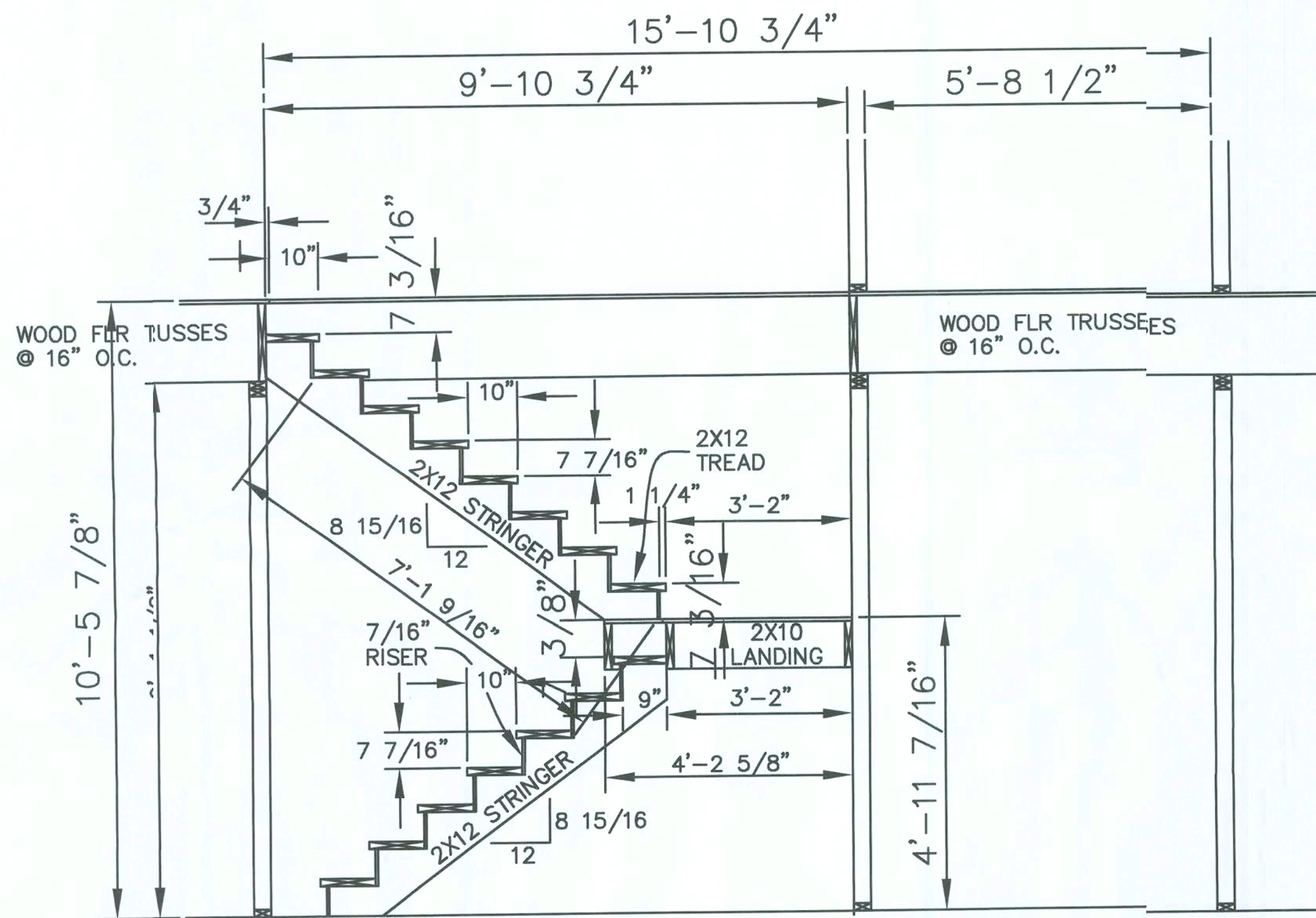
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2 x 4 EXTERIOR WALLS UNLESS OTHERWISE NOTED		ROOF TRUSS PLAN/DETAILS SHEET NO. 7	SCALE: 1/4" = 1'-0" DRAWN BY JDC SOLD BY KIM RICKOLT
SILVESTRE ENGINEERING & DESIGN, INC. 1801 OGLESBY AVE. - WINTER PARK, FL 32789 PHONE - (407) 644-5659 FAX - (407) 647-5758 CA #9144		DATE 2-28-08 REVISIONS 1802 S.F. 1344 S.F. 74 S.F. 3300 S.F. 633 S.F. 486 S.F. 275 S.F.	
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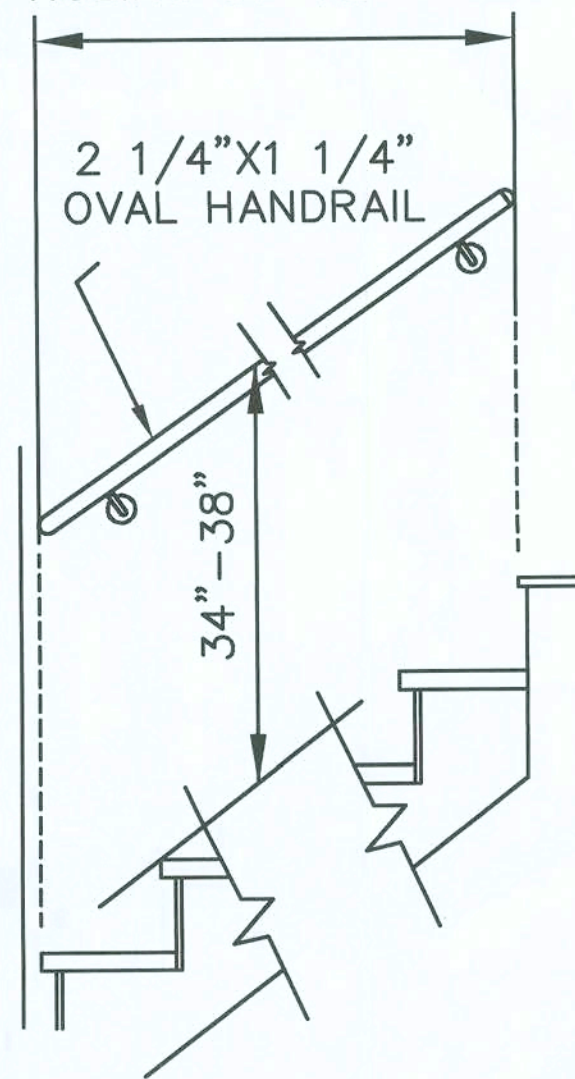
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STAIR FRAMING DETAIL

SCALE: 1/2" = 1'-0"

(1) HANDRAIL TO SPAN FROM
NOSING OF BOTTOM RISER
TO NOSING OF TOP RISER



STAIR HANDRAIL LOCATION DETAIL

SCALE: 1/2" = 1'-0"

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THE HANOVER
for MICHAEL & FADRA
McINTOSH

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AREAS	DATE 2-28-08	REVISIONS
FIRST FLOOR	1892 S.F.	
SECOND FLOOR	1274 S.F.	
SECOND FLOOR HEATED	74 S.F.	
BONUS ROOM (UNFINISHED)	3360 S.F.	
GARAGE	633 S.F.	
FRONT PORCH	486 S.F.	
REAR PORCHES	275 S.F.	

2 x 4 EXTERIOR WALLS
UNLESS OTHERWISE NOTED

STAIR SECTION 8

SCALE: 3/4" = 1'-0"

DRAWN BY JDC

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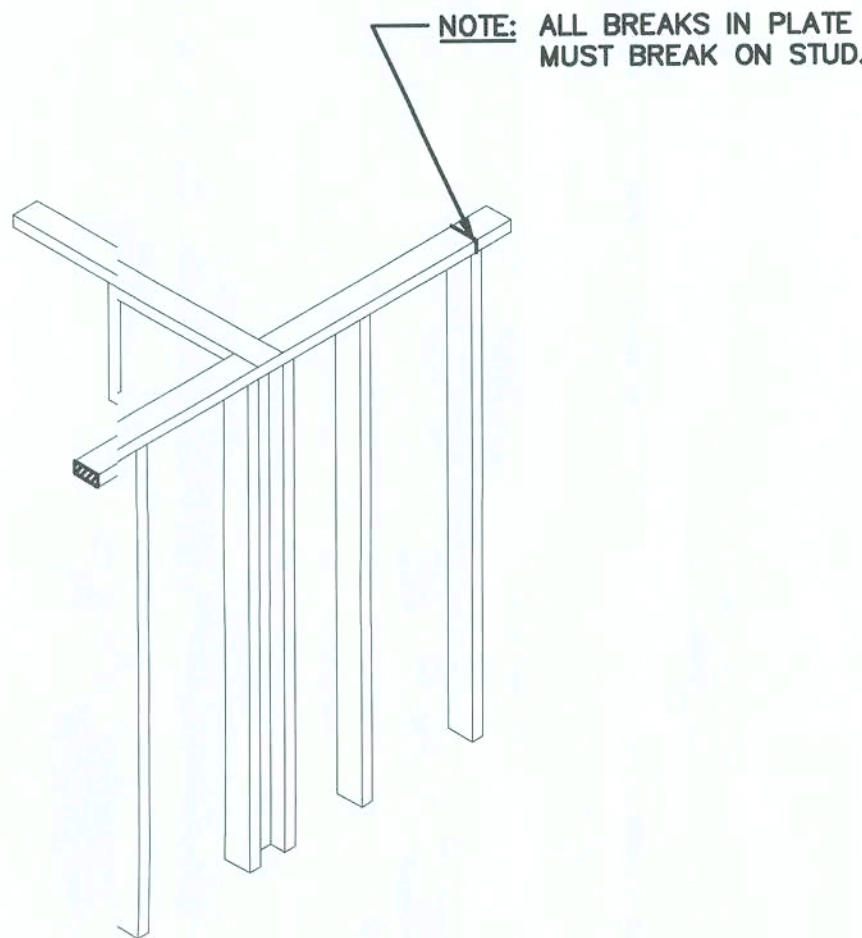
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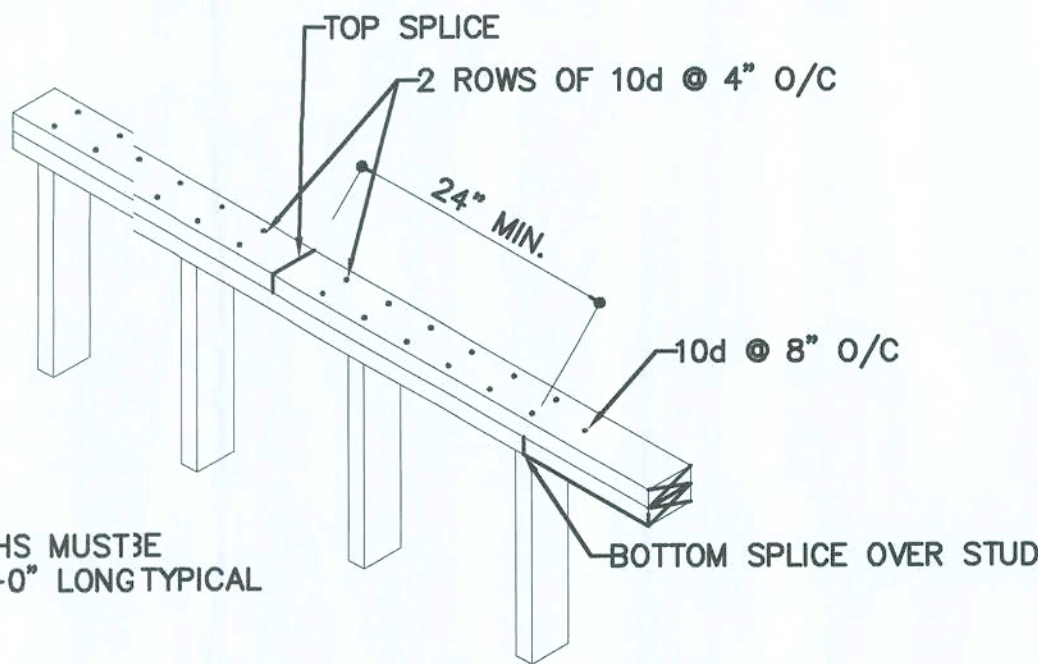
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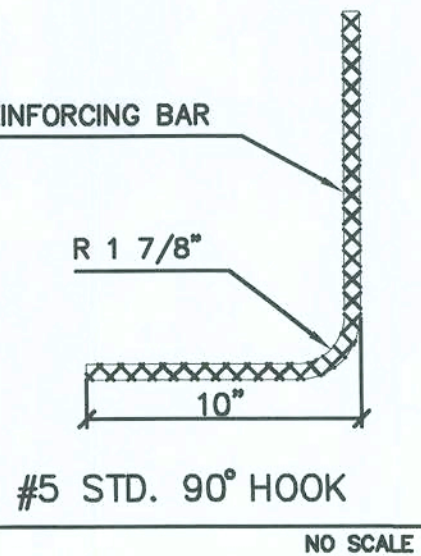
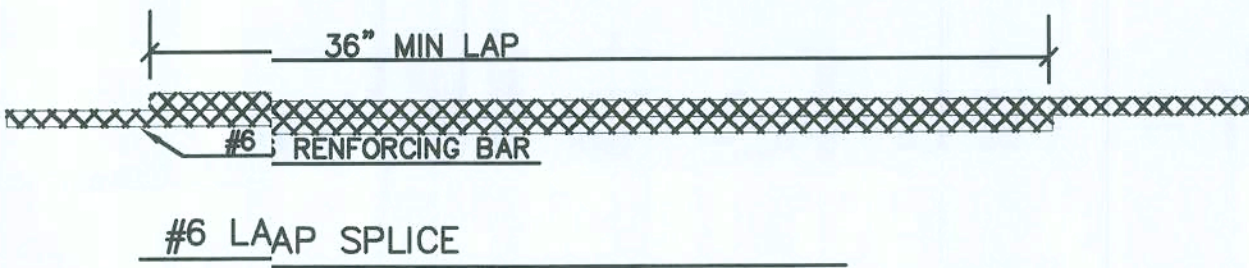
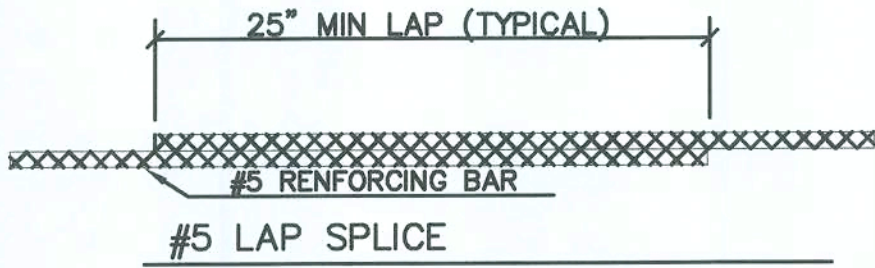
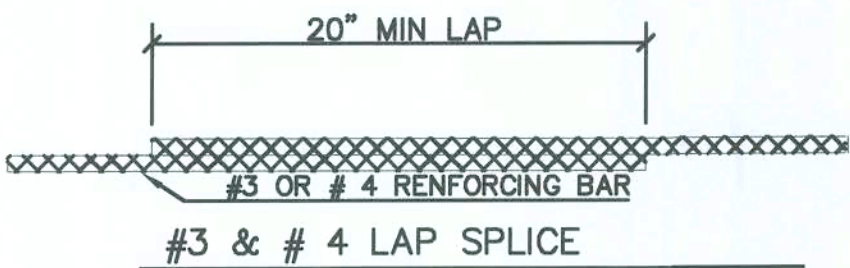


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NO SCALE



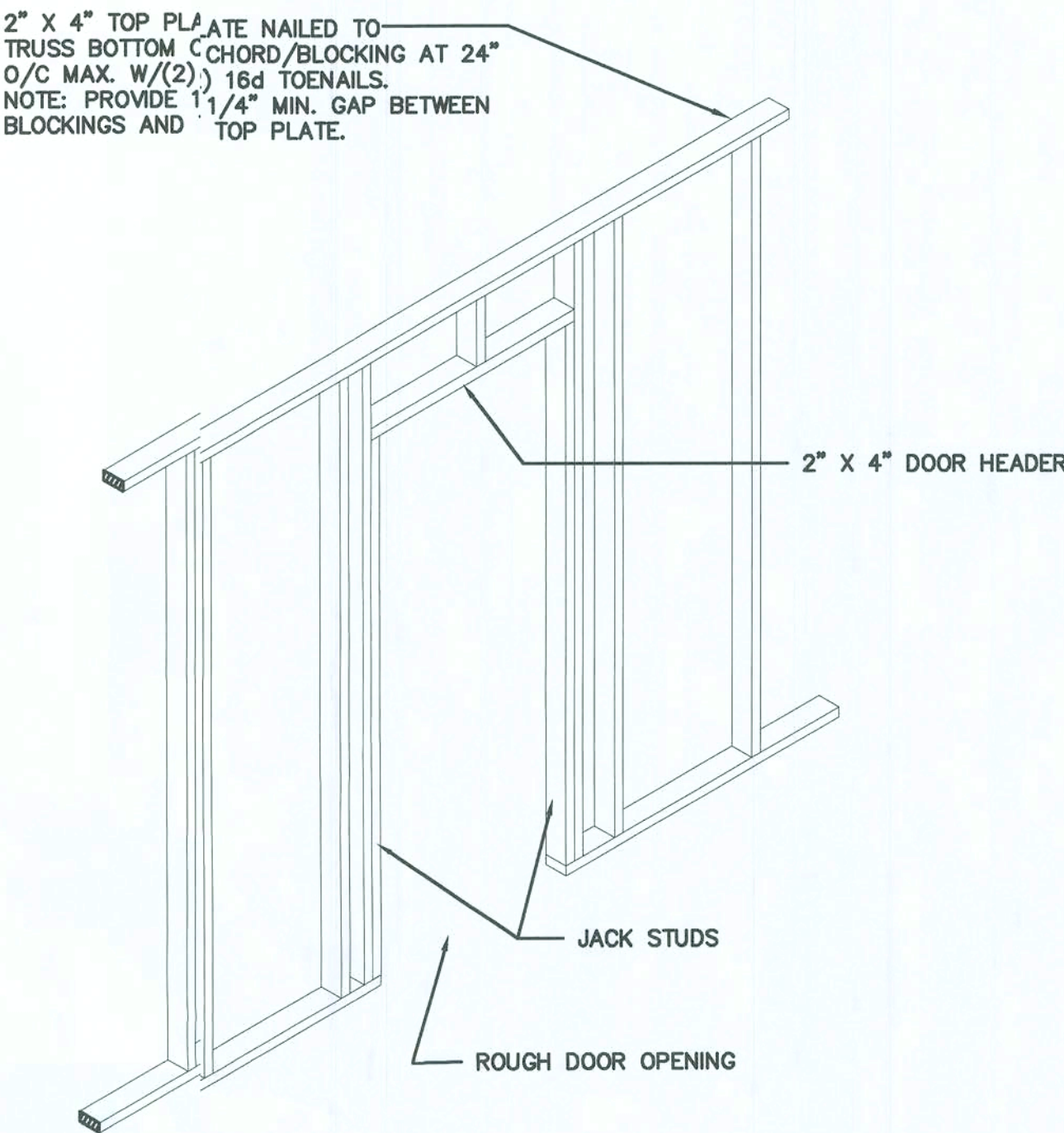
TOP PLATE SPLICE DETAIL

SCALE: 3/4"=1'-0"



LAP SPLICE DETAILS

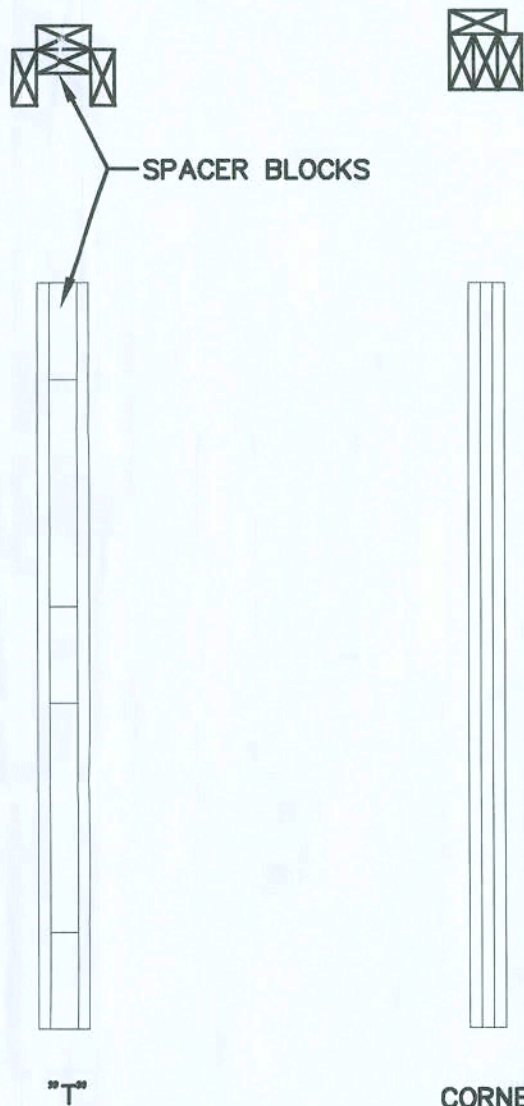
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TYPICAL DOOR HEADER DETAIL

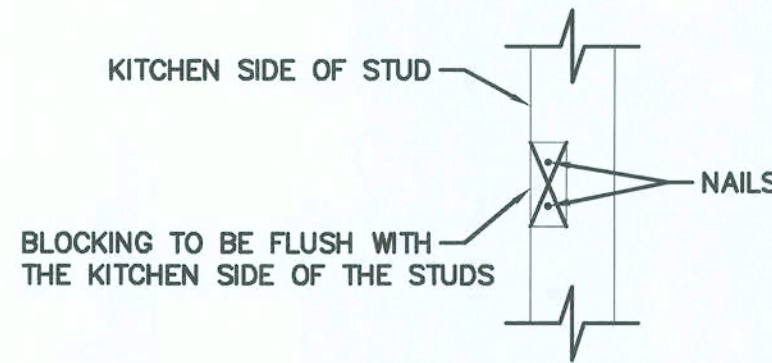
AT NON BEARING WALL.

NO SCALE

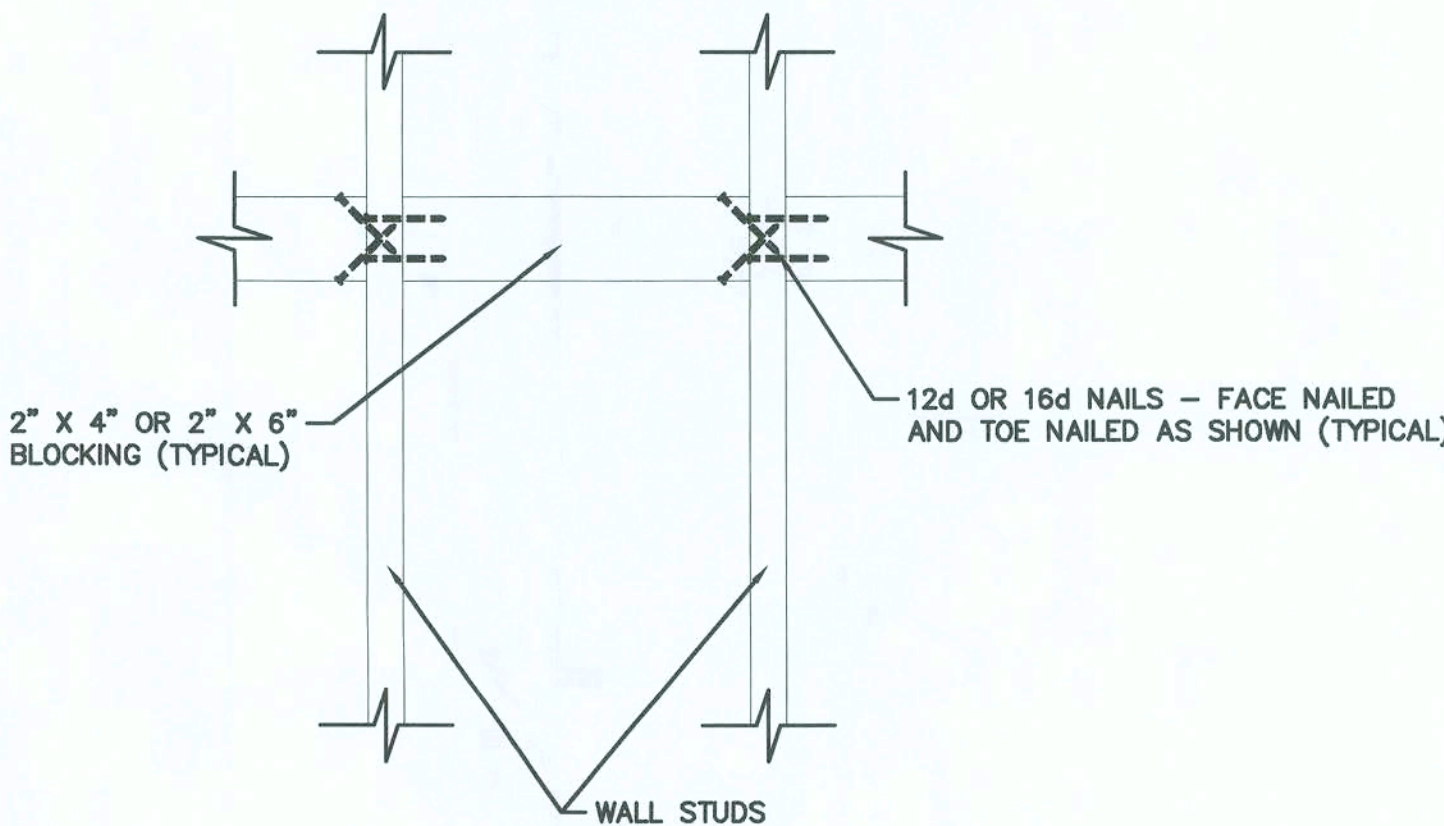


TYPICAL "T" & CORNER

NO SCALE



SIDE VIEW



FRONT VIEW

NOTE: CABINET BLOCKING WILL BE OF 2" X 4" OR 2" X 6" MATERIAL. TOP EDGE OF BLOCKING WILL BE AT 85" ABOVE THE FINISHED FLOOR. BLOCKING SHOULD BE FLUSH WITH THE KITCHEN SIDE OF THE STUDS. FASTEN WITH 12d OR 16d NAILS. INSTALL ON ALL WALLS RECEIVING UPPER CABINETS (16" OR 24" O.C. SPACING).

KITCHEN CABINET WALL BLOCKING DETAIL

SCALE: 1-1/2" = 1'-0"

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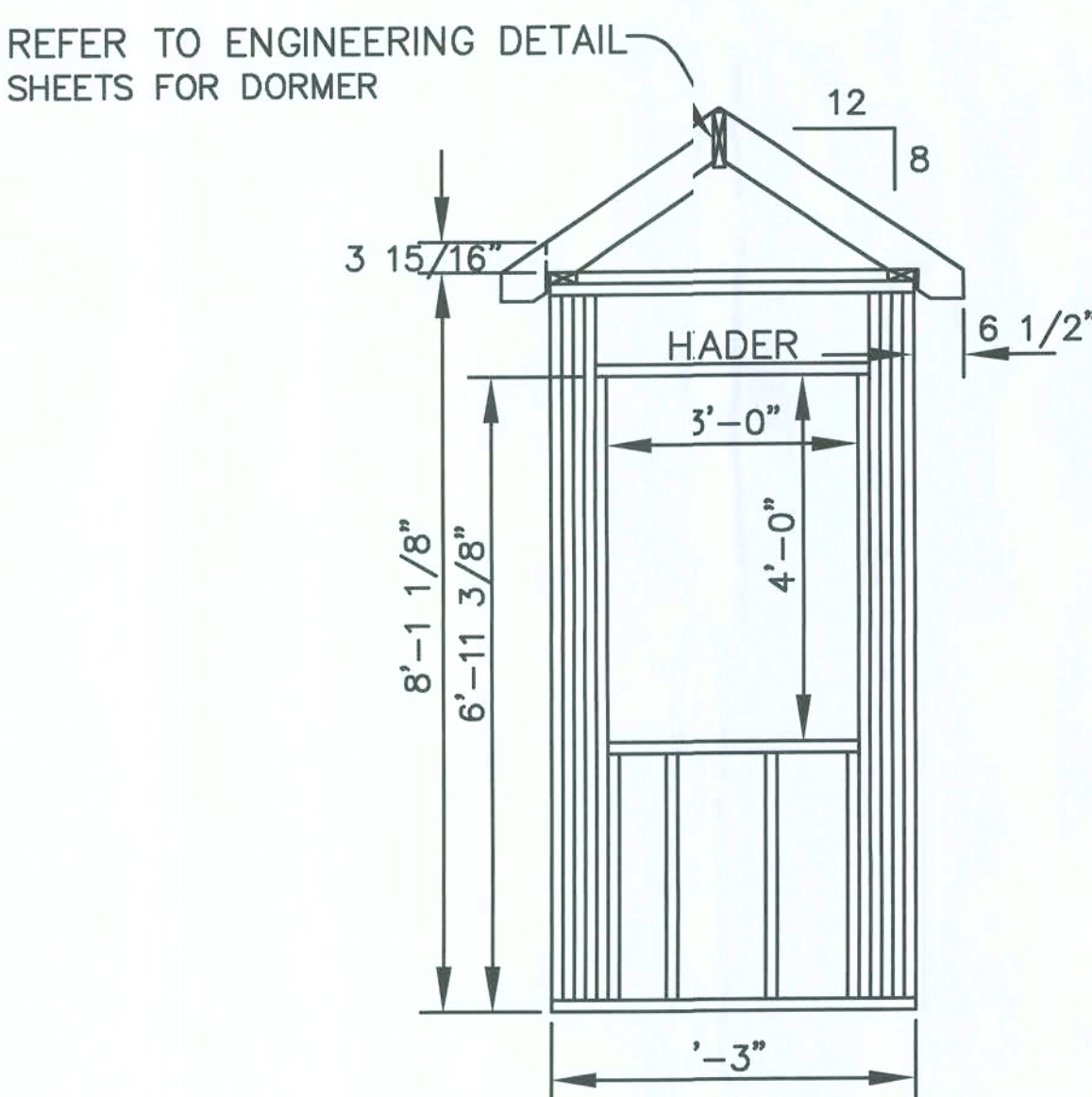
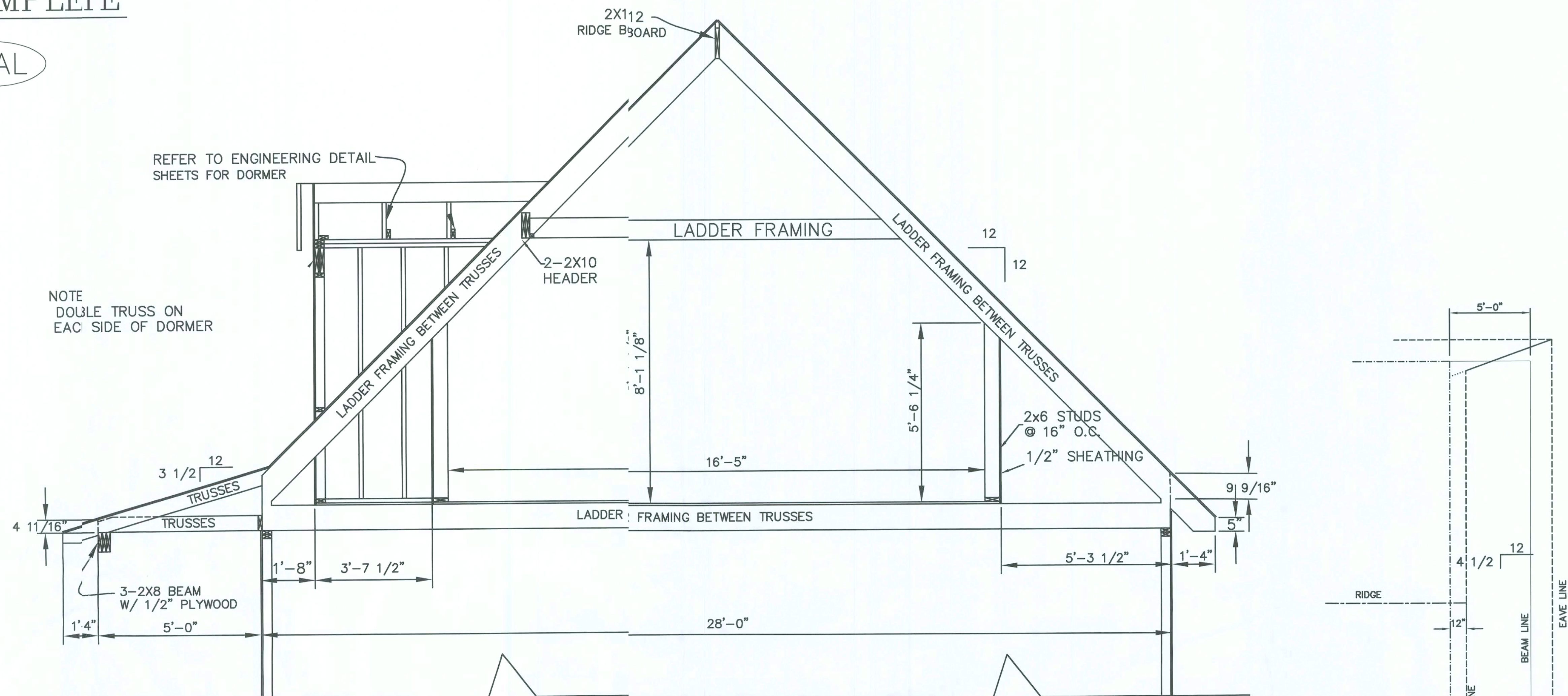
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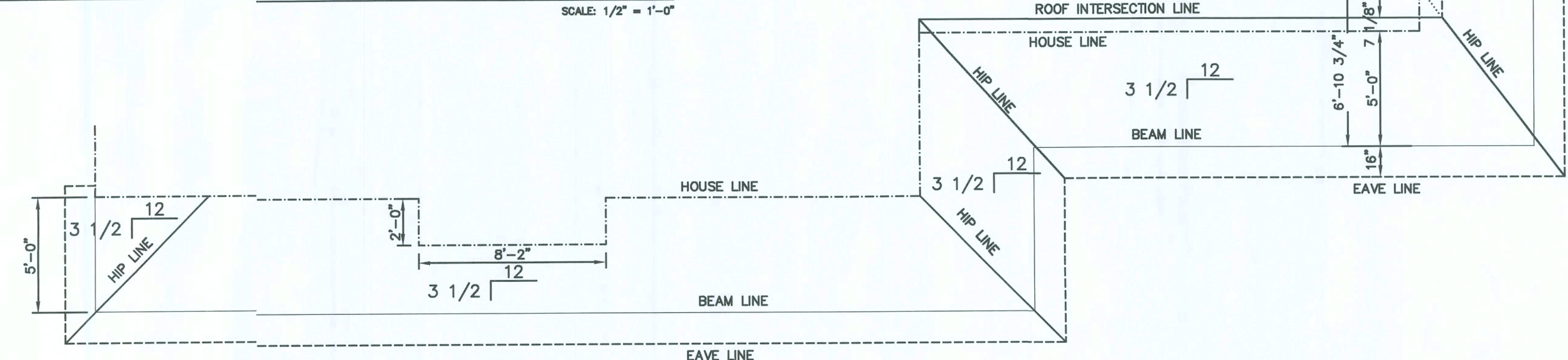


DORMER DETAIL

SCALE: 1/4" = 1'-0"

SECTION THROUGH BONUS ROOM

SCALE: 1/2" = 1'-0"



PORCH ROOF PLAN

SCALE: 1/4" = 1'-0"

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			DRAWN BY JDC		SECTION THROUGH BONUS ROOM 10	
			SOLD BY KIM RICKOLT		SHEET No. 15	

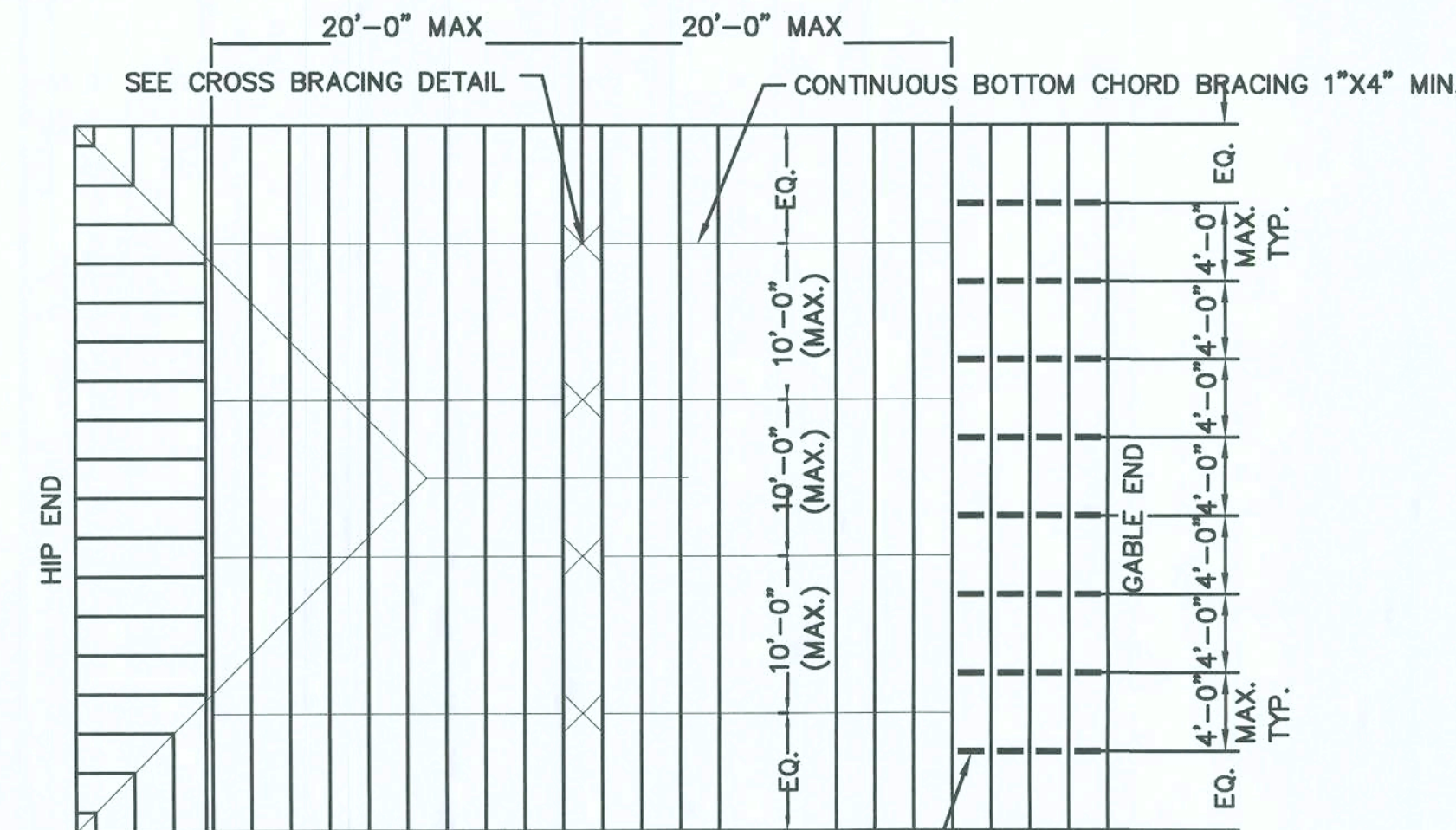
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STRUCTURAL NOTES

- 1) CODES:
- 2004 FLORIDA RESIDENTIAL BUILDING CODE, AND WIND LOAD REQUIREMENTS PER SEC. R301.1.
 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-02).
 - AMERICAN SOCIETY OF CIVIL ENGINEERS MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ASCE 7-02).
 - SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS (AISC 9TH EDITION).
 - "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" BY THE TRUSS PLATE INSTITUTE ANS/ TPI 1-1995 EDITION.
- 2) DESIGN CRITERIA:
- 2.1 DWELLING FLOORS - 40 PSF LIVE LOAD; 15 PSF DEAD LOAD
 - 2.2 BALCONIES - 60 PSF LIVE LOAD; 10 PSF DEAD LOAD
 - 2.3 WALKWAYS - 80 PSF LIVE LOAD; 10 PSF DEAD LOAD
 - 2.4 SINGLE ROOF - 20 PSF LIVE LOAD + 17 PSF DEAD LOAD (7 PSF T/C & 10 PSF B/C); DURATION FACTOR = 1.25
 - 2.5 TILE ROOF - 20 PSF LIVE LOAD + 25 PSF DEAD LOAD (15 PSF T/C & 10 PSF B/C); DURATION FACTOR = 1.25
 - 2.6 WIND - 120-MPH, 3-SECOND GUST PER ASCE 7-02 IN CATEGORY 2, ENCLOSED Bldg.
 - 2.7 OUTSIDE WIND BORNE DEBRIS REGION, EXPOSURE "B"; PORTANCE FACTOR I = 1.00; MAX. MEAN ROOF HEIGHT = 30 ft., INT. PRESSURE COEFF. = ±0.18.
 - 2.8 NET UPLIFT DEAD LOADS 10 PSF SHINGLE; 15 PSF TL.
- 3) SOIL:
- 3.1 MINIMUM ALLOWABLE SOIL PRESSURE 2000 PSF.
- 4) CONCRETE:
- 4.1 CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS: 2500 PSI (NORMAL WEIGHT).
 - 4.2 REINFORCING BARS: ASTM A615 (GRADE 40).
 - 4.3 WELDED WIRE FABRIC (W/F): ASTM A185.
 - 4.4 DETAIL REINFORCEMENT IN ACCORDANCE WITH ACI 315.
 - 4.5 CONCRETE COVERAGE OF REINFORCEMENT: FOOTINGS* BOTTOM AND SIDES.
 - 4.6 EARTH SUPPORTED SLABS: (INCLUDING EXTERIOR WALL AND DRIVE SLABS) 3 1/2" THICK MIN., REINFORCED WITH 6X6 - W1.4 X W1.4 WWF AT MID-DEPTH OF SLAB. FIBERMESH MAY BE USED IN LIEU OF WWF AT CONTRACTOR'S OPTION.
 - 4.7 CONCRETING OPERATIONS SHALL COMPLY WITH ACI STANDARDS.
 - 4.8 LAP SPLICE SHALL BE AS FOLLOWS: #5 BAR 25", #4 BAR 20", #3 BAR 15".
- 5) MASONRY:
- 5.1 DESIGN AND CONSTRUCTION SHALL CONFORM TO THE SPECIFICATION OF THE NATIONAL CONCRETE MASONRY ASSOCIATION AND ACI 530.
 - 5.2 MINIMUM MASONRY UNIT STRENGTH: 1500 PSI.
 - 5.3 MORTAR SHALL BE TYPE S.
 - 5.4 ALL BLOCK CELLS AND CAVITIES BELOW SLAB SHALL BE FILLED WITH CONCRETE WHEN STEM WALL IS GREATER THAN 24" TALL ABOVE GRADE.
- 6) WOOD:
- 6.1 WOOD - WITH THE EXCEPTION OF STUDS, STRUCTURAL FRAMING MEMBERS SHALL BE #2 SOUTHERN YELLOW PINE (SYP) WITH AN ALLOWABLE BENDING STRESS (Fb) = 1200 PSI AND A MODULUS OF ELASTICITY = 1,600,000 PSI. WALL STUDS SHALL BE CONSTRUCTION GRADE SPRUCE PINE FIR (SPF) @ 16" ON CENTER.
 - 6.2 DESIGN, FABRICATE AND ERECT WOOD TRUSSES IN ACCORDANCE WITH THE "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" BY THE TRUSS PLATE INSTITUTE, ANS/ TPI 1-1995 EDITION.
 - 6.3 ALL EXPOSED WOOD OR WOOD IN CONTACT WITH EARTH OR CONCRETE TO BE PRESURE TREATED.
 - 6.4 ROOF SHEATHING: (APA RATED EXPOSURE 1) 1/2" PLYWOOD OR 7/16" OSB MINIMUM SHINGLES OR TILE
 - 6.5 UNTREATED WOOD SHALL NOT BE IN DIRECT CONTACT WITH CONCRETE. SEAT PLATES SHALL BE PROVIDED AT BEARING LOCATIONS WITHOUT WOODEN TOP PLATE.
- 7) FLASHING:
- 7.1 ASPHALT SHINGLES:
 - 7.1.1 BASE & CAP FLASHING SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION INSTRUCTIONS. BASE FLASHING SHALL BE 26 GAGE (0.019") GALVANIZED STEEL OR MINERAL SURFACE ROLL ROOFING WEIGHING A MINIMUM OF 77 lb PER 100 Sq. Ft. CAP FLASHING SHALL BE 26 GAGE (0.019") GALVANIZED STEEL.
 - 7.1.2 VALLEY LINING SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION INSTRUCTIONS BEFORE APPLYING ASPHALT SHINGLES. VALLEY LINING OF THE FOLLOWING TYPES SHALL BE PERMITTED.
 - 7.1.2.1 FOR OPEN VALLEYS LINED WITH METAL, THE VALLEY LINING SHALL BE AT LEAST 16" WIDE 26 GAGE (0.019") GALV. STEEL. FOR OPEN VALLEYS, VALLEY LINING OF TWO LAYERS OF MINERAL SURFACE ROLL ROOFING IS PERMITTED. THE BASE LAYER SHALL BE 24" AND THE TOP LAYER SHALL BE AT LEAST 36" WIDE.
 - 7.1.2.2 FOR CLOSED VALLEYS (COVERED WITH SHINGLES) VALLEY LINING SHALL BE ONE OF THE FOLLOWING:
 - ONE PLY OF SMOOTH ROLL ROOFING AT LEAST 2" WIDE AND COMPLYING WITH ASTM D 224, OR
 - SPECIALTY UNDERLAYMENT AT LEAST 36" WIDE JO COMPLYING WITH ASTM D 1970.
 - 7.2 CONCRETE TILE:
 - 7.2.1 AT THE JUNCTURE OF THE ROOF AND VERTICAL SURFACES, FLASHING AND COUNTERFLASHING SHALL BE PROVIDED IN ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION INSTRUCTIONS, AND WHERE OF METAL, FLASHING SHALL BE 26 GAGE (0.019") GALVANIZED STEEL. THE VALLEY FLASHING SHALL EXTEND AT LEAST 11" FROM THE CENTER LINE EACH WAY AND HAVE A SPLASH DIVERTER RID NOT LESS THAN 1" HIGH AT THE FLOW LINE FORMED AS PART OF THE FLASHING. SECTIONS OF FLASHING SHALL HAVE AN END LAP OF NOT LESS THAN 4". FOR ROOF SLOPES OF 3:12 AND OVER, THE VALLEY FLASHING SHALL HAVE A LAYER OF 36" WIDE TYPE 1 UNDERLAYMENT RUNNING THE FULL LENGTH OF THE VALLEY, IN ADDITION TO OTHER REQUIRED UNDERLAYMENTS. FOR SLOPES UNDER 7:12 IN AREAS WHERE THE AVG. DAILY TEMPERATURE IN JANUARY 22°F OR LESS, OR WHERE THERE IS A POSSIBILITY OF ICE FORMING ALONG THE EAVES CAUSING A BACKUP OF WATER, THE METAL VALLEY FLASHING UNDERLAYMENT SHALL BE SOLID CEMENTED TO THE ROOFING UNDERLAYMENT.
- 8) DOORS & WINDOWS:
- 8.1 ALL EXTERIOR WINDOWS AND GLASS DOORS ARE REQUIRED TO BE TESTED IN ACCORDANCE WITH ANS/AMMA/NWMA 101/152 STANDARD AND BEAR AN AMMA OR NWMA LABEL IDENTIFYING THE MANUFACTURER, PERFORMANCE CHARACTERISTICS AND APPROVED PRODUCT TESTING ENTITY.
 - 8.2 ALL EXTERIOR WINDOWS AND DOORS SHALL BE ANCHORED PER PUBLISHED MANUFACTURER'S RECOMMENDATION TO ACHIEVE THE DESIGN PRESSURE SPECIFIED BELOW.
 - 8.3 IF BUCK THICKNESS IS LESS THAN 1.5", EXTERIOR DOORS AND WINDOWS SHALL BE ANCHORED THROUGH THE JAM, INTO THE STRUCTURAL SUBSTRATE PER THE MANUFACTURER'S SPECIFICATIONS.
 - 8.4 IF BUCK THICKNESS IS EQUAL TO 1.5", OR GREATER, THE BUCK MUST BE ATTACHED IN A MANNER (SEE MFG. SPECS.) THAT TRANSFERS THE LOAD DIRECTLY TO THE STRUCTURE. WINDOWS AND DOORS SHALL BE ANCHORED THROUGH THE JAM INTO THE WOOD BUCK.
 - 8.5 MULLIONS AND ADJACENT DOOR ASSEMBLIES SHALL BE TESTED OR ENGINEERED (BY THE MFG.) TO TRANSFER 1.5 TIMES THE DESIGN LOAD TO THE SUFFICIENT OPENING SUBSTRATE.
- 9) INSPECTIONS:
- 9.1 FOUNDATION INSPECTIONS: A FOUNDATION SURVEY SHALL BE PERFORMED AND COPY OF THE SURVEY SHALL BE ON SITE FOR THE BUILDING INSPECTOR'S USE. OR, ALL PROPERTY OWNERS SHALL BE EXPOSED AND A STRING STRETCHED FROM MARKER TO MARKER TO VERIFY REQUIRED SETBACKS.
 - 9.2 FRAMING INSPECTIONS: ALL PLUMBING, ELECTRICAL, AND MECHANICAL ROUGH-INS MUST BE COMPLETE, INSPECTED, AND APPROVED BEFORE REQUESTING THE FRAMING INSPECTION.
- 10) MICRO-LAM LUMBER:
- 10.1 MICRO-LAM STRESS GRADES SHALL PROVIDE THE FOLLOWING MINIMUM PROPERTIES:
 - E = 2,000,000 PSI
 - Fb = 2,800 PSI
 - Ft = 1,850 PSI
 - Fc = 500 PSI (PERPENDICULAR)
 - Fv = 2,700 PSI (PARALLEL)
 - Fv = 285 PSI

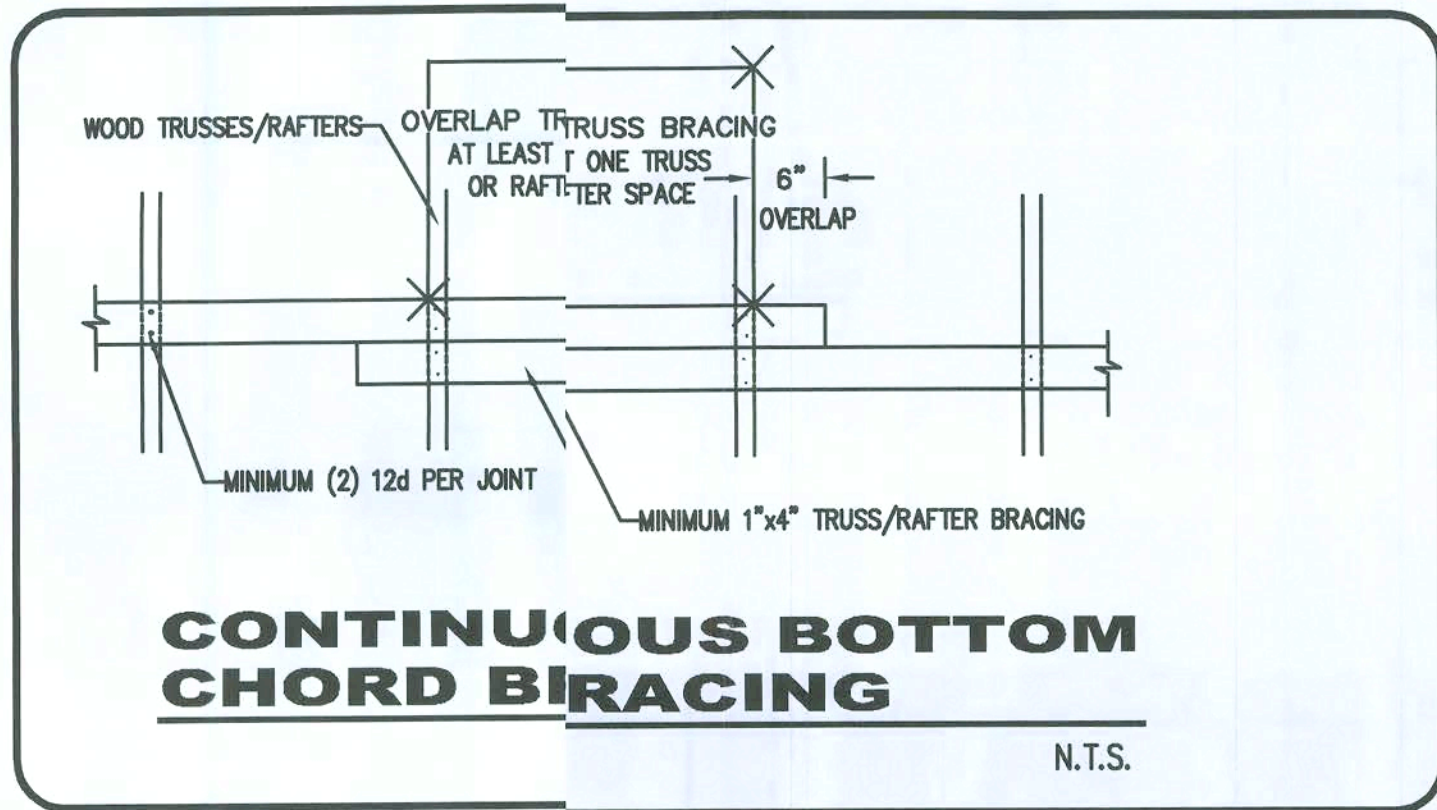
MINIMUM PERMANENT TRUSS BRACING PLAN



- NOTE:
- 1) SEE TRUSS MANUFACTURER'S TRUSS ENGINEERING DRAWINGS FOR ADDITIONAL PERMANENT BRACING THAT MAY BE REQUIRED
 - 2) "T" BRACING MUST EXTEND OVER AT LEAST 90% OF THE WEB.
- TOP & BOTTOM CHORD
GABLE END BRACING @
4'-0" O/C MAX FOR
FIRST (4) ROWS OF TRUSSES

CHEMICAL SOIL
TREATMENT FOR TERMITES

- 1- A PERMANENT SIGN WHICH IDENTIFIES THE TERMITE TREATMENT PROVIDER AND NEED FOR RE-INSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE PROVIDED. THE SIGN SHALL BE POSTED NEAR THE WATER HEATER, OR ELECTRICAL PANEL.
- 2- CONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-0" AWAY FROM THE BUILDING SIDE WALLS.
- 3- IRRIGATION / SPRINKLER SYSTEM INCLUDING ALL RISERS AND SPRAY HEADS SHALL NOT BE INSTALLED WITHIN 1'-0" OF THE BUILDING SIDE WALLS.
- 4- TO PROVIDE FOR INSPECTION OF TERMITE INFESTATION, BETWEEN WALL COVERING AND FINAL EARTH GRADE SHALL NOT BE LESS THAN 6 INCHES.
- EXCEPTION - PAINT OR DECORATIVE CEMENTITIOUS FINISH LESS THAN 5/8" THICK ADHERED DIRECTLY TO THE FOUNDATION WALL.
- 5- INITIAL TREATMENT SHALL BE DONE AFTER ALL EXCAVATION AND BACKFILL IS COMPLETE.
- 6- SOIL DISTURBED AFTER THE INITIAL TREATMENT SHALL BE RE-TREATED INCLUDING SPACED BOXED OR FORMED.
- 7- BOXED AREAS IN CONCRETE FLOORS FOR SUBSEQUENT INSTALLATION OF TRAPS, ETC., SHALL BE MADE WITH PERMANENT METAL OR PLASTIC FORMS. PERMANENT FORMS MUST BE OF A SIZE & DEPTH THAT WILL ELIMINATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMENT.
- 8- MINIMUM 6-MIL. VAPOR RETARDER MUST BE INSTALLED TO PROTECT AGAINST RAINFALL DILUTION.
- IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED.
- 9- CONCRETE OVERPOUR AND MORTAR ALONG THE FOUNDATION PERIMETER MUST BE REMOVED BEFORE EXTERIOR TREATMENT.
- 10- SOIL TREATMENT MUST BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1'-0" OF THE STRUCTURE SIDEWALLS.
- 11- AN EXTERIOR VERTICAL CHEMICAL BARRIER MUST BE INSTALLED AFTER CONSTRUCTION IS COMPLETE INCLUDING LANDSCAPING AND IRRIGATION. ANY SOIL DISTURBED AFTER THE VERTICAL BARRIER IS APPLIED SHALL BE RETREATED.
- 12- ALL BUILDINGS ARE REQUIRED TO HAVE PRE-CONSTRUCTION TREATMENT.
- 13- A CERTIFICATE OF COMPLIANCE MUST BE ISSUED TO THE BUILDING DEPARTMENT BY A LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCUPANCY WILL BE ISSUED. THE CERTIFICATE OF COMPLIANCE SHALL STATE "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. THE TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."
- 14- AFTER ALL WORK IS COMPLETED, LOOSE WOOD AND FILL MUST BE REMOVED FROM BELOW AND WITHIN 1'-0" OF THE BUILDING. THIS INCLUDES ALL GRADE STAKES, TUB TRAP BOXES, FORMS, SHORING, OR OTHER CELLULOSE CONTAINING MATERIAL.
- 15- NO WOOD, VEGETATION, STUMPS, CARDBOARD, TRASH, ETC., SHALL BE BURIED WITHIN 15'-0" OF ANY BUILDING OR PROPOSED BUILDING.



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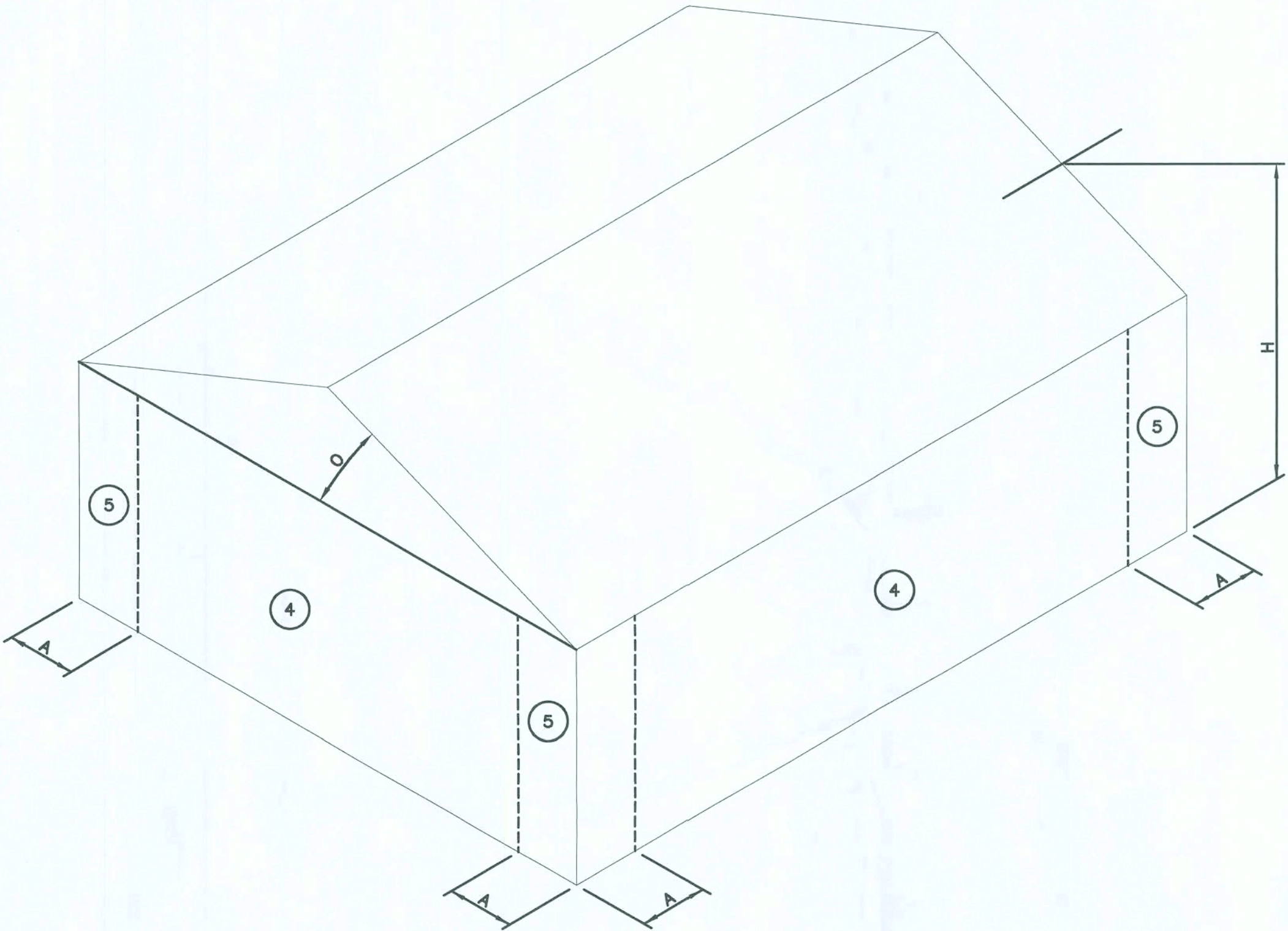
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WINTER PARK, FL 32789
CA #9144
KENNETH SCHRAW, PE #17402

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- BASIC WIND SPEED = 120 MPH
- WIND IMPORTANCE FACTOR = 1.0
- WIND EXPOSURE CATEGORY = B
- INTERNAL PRESSURE COEFFICIENT = +/- 0.18 (FULLY ENCLOSED)
- AVERAGE DESIGN WIND PRESSURE = 25 PSF
- WIND PRESSURES FOR COMPONENT AND CLADDING FOR WALL ELEMENTS SHALL BE (+) 25.9 & (-) 34.7 PSF
- GARAGE DOOR DESIGN PRESSURE +22.8 & -25.8 PSF
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PROPERTY OF AMERICA'S HOME PLACE INC.	THE HANOVER FOR MICHAEL & FADRA MCINTOSH	AREAS FIRST FLOOR SECOND FLOOR AREA SECOND FLOOR HEATED TOTAL HEATED BANKS ROOM (UNFINISHED) BANKS ROOM (FINISHED) FRONT PORCH REAR PORCHES	1982 S.F. 1344 S.F. 74 S.F. 3380 S.F. 390 S.F. 488 S.F. 275 S.F.	DATE 2-28-08 REVISIONS	SILVESTRE ENGINEERING & DESIGN, INC. 1801 OGLESBY AVE. - WINTER PARK - FLORIDA - 32789 PHONE - (407) 644-8888 FAX - (407) 647-5758 CA #9144			SHEET No. NOTES/DETAILS 11	SCALE: 1/4" = 1'-0"	DRAWN BY JDC	SOLD BY KIM RICKOLT
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COMPONENT & CLADDING DESIGN PRESSURE (119.9 MPH, 3 SEC. GUST, 2.1/12 PITCH, EXPOSURE B, HEIGHT = 0 TO 30 FEET, IMPORTANCE FACTOR = 1)							
TRIBUTARY AREA (FT ²)	ROOF				WALL		
	(GCpf = ± .18)				(GCpw = ± .18)		
	ZONE 1	ZONE 2 & 3			ZONE 4	ZONE 5	
10	14.9	-23.7	14.9	-50.1	25.9	-28.1	25.9 -34.7
20	13.6	-23.0	13.6	-45.4	24.7	-26.9	24.7 -32.4
50	11.9	-22.2	11.9	-39.3	23.2	-25.4	23.2 -29.3
100	10.5	-21.5	10.5	-34.7	22.0	-24.2	22.0 -26.9
125	10.5	-21.5	10.5	-34.7	21.7	-23.6	21.7 -26.2
NOTES: 1 FOR EFFECTIVE AREAS OR WIND SPEEDS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREA.							
2 TABLE VALUES SHALL BE ADJUSTED FOR HEIGHT AND EXPOSURE BY MULTIPLYING BY ADJUSTMENT COEFFICIENTS IN TABLE 1606.2D OF THE FLORIDA BUILDING CODE 2001.							
3 PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACES.							



A:) 10 PERCENT OF LEAST HORIZONTAL DIMENSION OR 0.4H, WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF LEAST HORIZONTAL DIMENSION OR 3 FT. (1M).

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AREAS	DATE 2-28-08	REVISIONS
FIRST FLOOR	1982 S.F.	
SECOND FLOOR AREA	1344 S.F.	
SECOND FLOOR HEATED	74 S.F.	
TOTAL HEATED	3380 S.F.	
BONUS ROOM (UNFINISHED)	320 S.F.	
FRONT PORCH	488 S.F.	
REAR PORCHES	275 S.F.	

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PHONE (107) 811 8000 FAX (107) 817 8700
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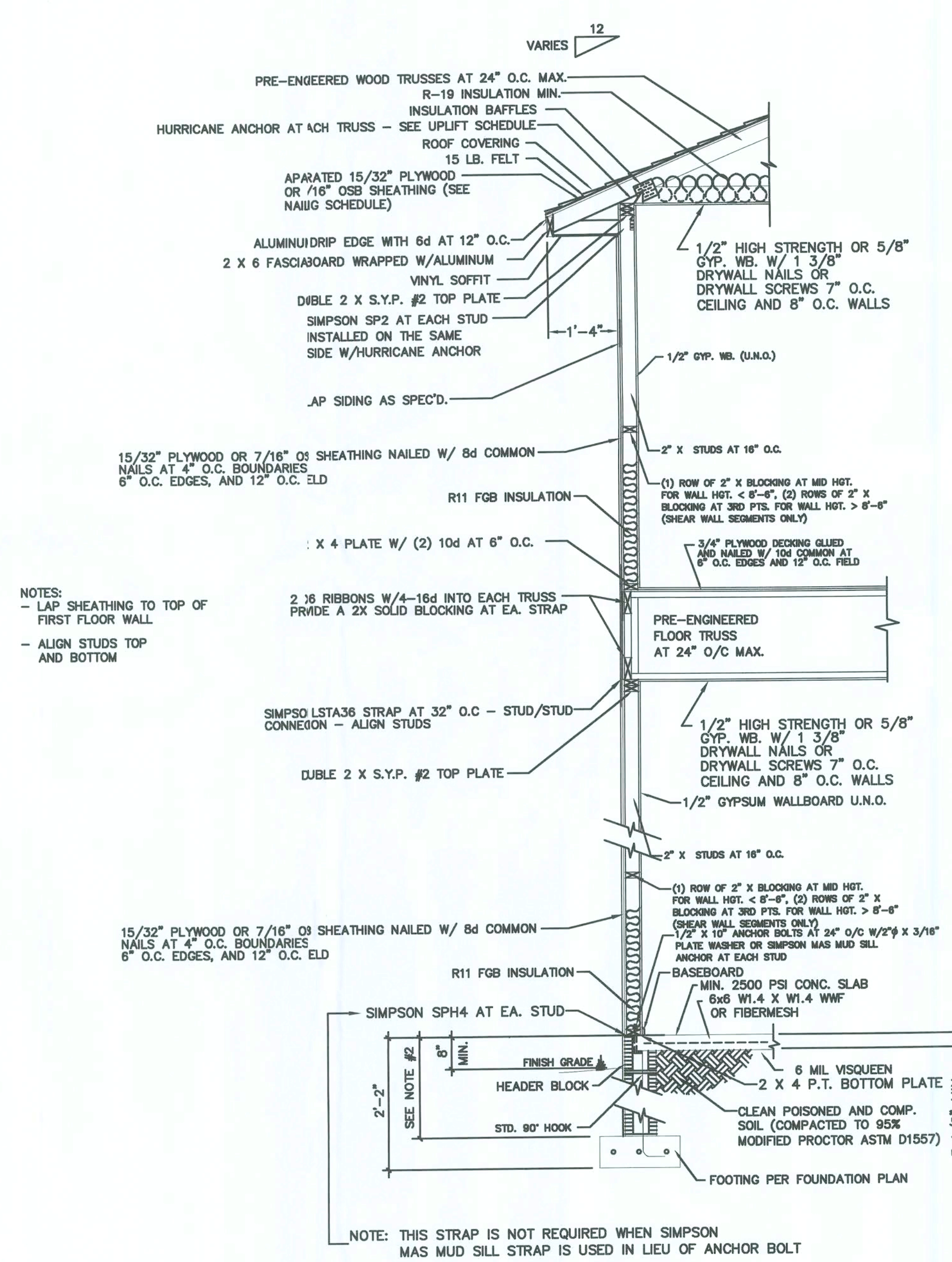
2 x 4 EXTERIOR WALLS
UNLESS OTHERWISE NOTED

DETAILS 12

SCALE: 1/4" = 1'-0"
DRAWN BY JDC
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FINAL

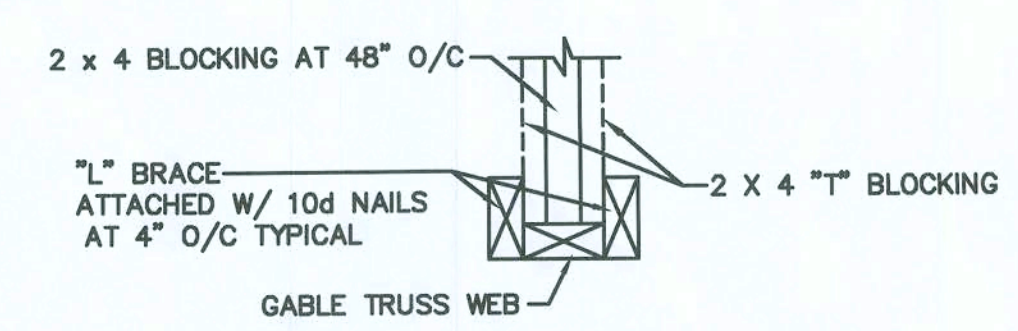


TYPICAL TWO STORY SECTION

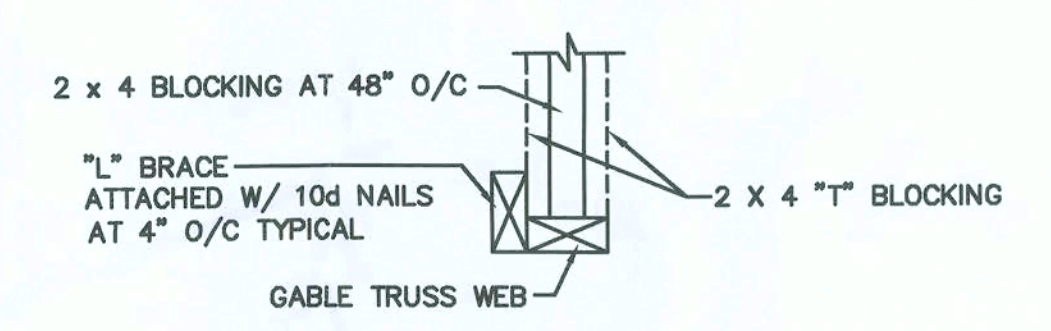
- NOTES:
1. LL ALL CELLS WHEN STEM WALL IS GREATER THAN 24" BUT LESS THAN 40".
 2. OR STEM WALLS GREATER THAN 40" HIGH CONSULT ENGINEER.

"L" BRACING REQUIREMENTS				
STUD SPACING	NO "L" BRACE	(1) 2"x4" L BRACE	(2) 2"x4" L BRACE	(3) 2"x6" L BRACE
16"	0'-0" - 4'-5"	4'-5" - 8'-0"	8'-0" - 12'-0"	12'-0" - 16'-0"
24"	0'-0" - 4'-0"	4'-0" - 7'-0"	7'-0" - 10'-0"	10'-0" - 14'-0"

NOTE:
"L" BRACE MUST EXTEND AT LEAST 90% OF WEB LENGTH



DOUBLE "L" BRACING DETAIL
SCALE: NONE 12



SINGLE "L" BRACING DETAIL
SCALE: NONE 12

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2 x 4 EXTERIOR WALLS
UNLESS OTHERWISE NOTED

DETAILS 13
SCALE: AS NOTED

DRAWN BY JDC
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PHONE - (407) 944-0038 FAX - (407) 944-5750
CA #9144

DATE: 2-28-08
REVISIONS

AREAS	1802 S.F.
FIRST FLOOR	1344 S.F.
SECOND FLOOR AREA	74 S.F.
TOTAL HEATED	3380 S.F.
Garage Area (unheated)	633 S.F.
FRONT PORCH	275 S.F.
REAR PORCH	

THE HANOVER
FOR MICHAEL & FADRA
McINTOSH

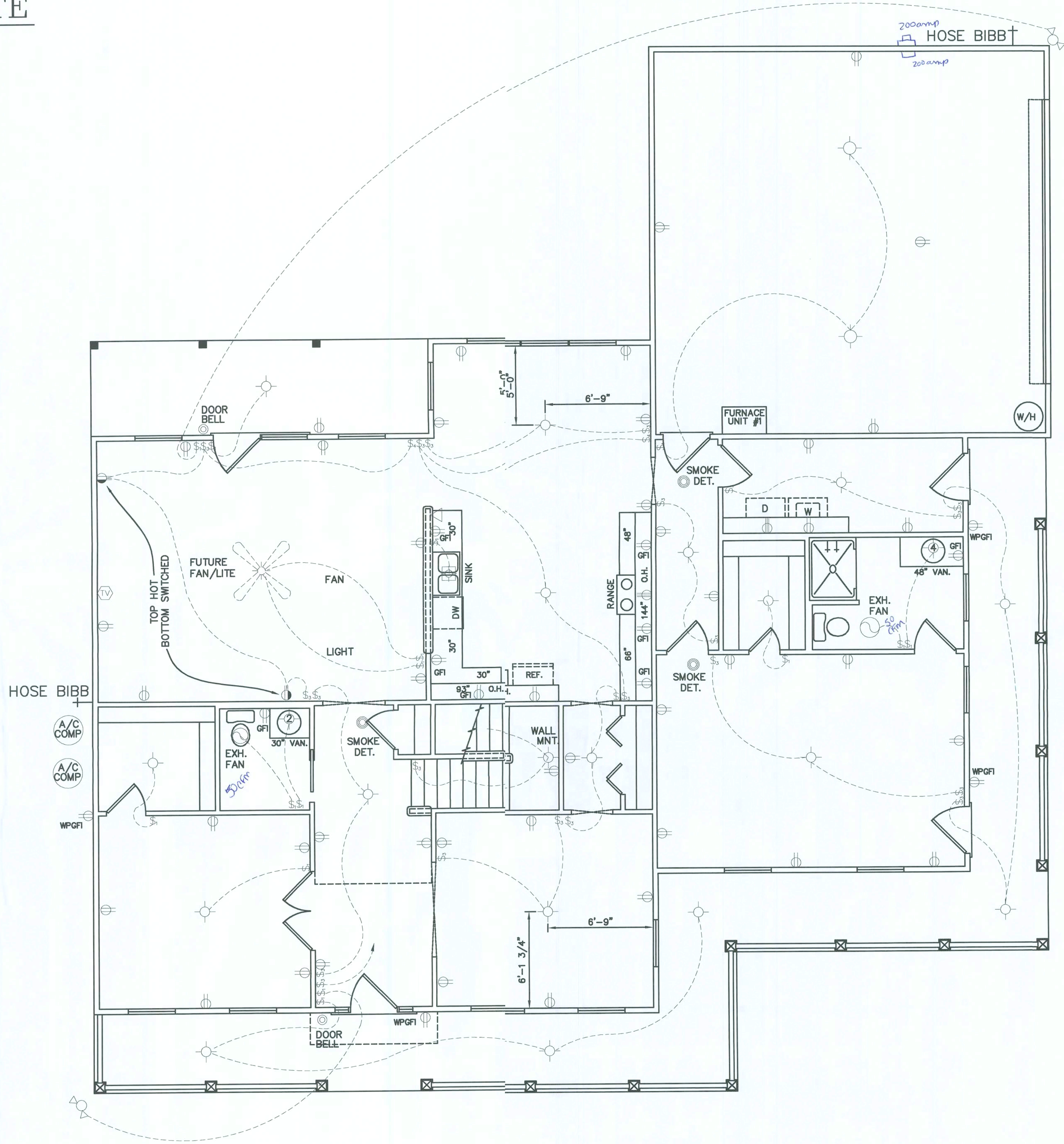
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LOWER LEVEL ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

NOTE
SERVICE DISCONNECT REQ'D

NOTES

BRACE LIVING ROOM AND ALL
BEDROOMS FOR PADDLE FAN
ALL ELECTRICAL IN BEDROOMS SHALL
BE ON AN A.F.C.I. PER NEC 2002

LEGEND

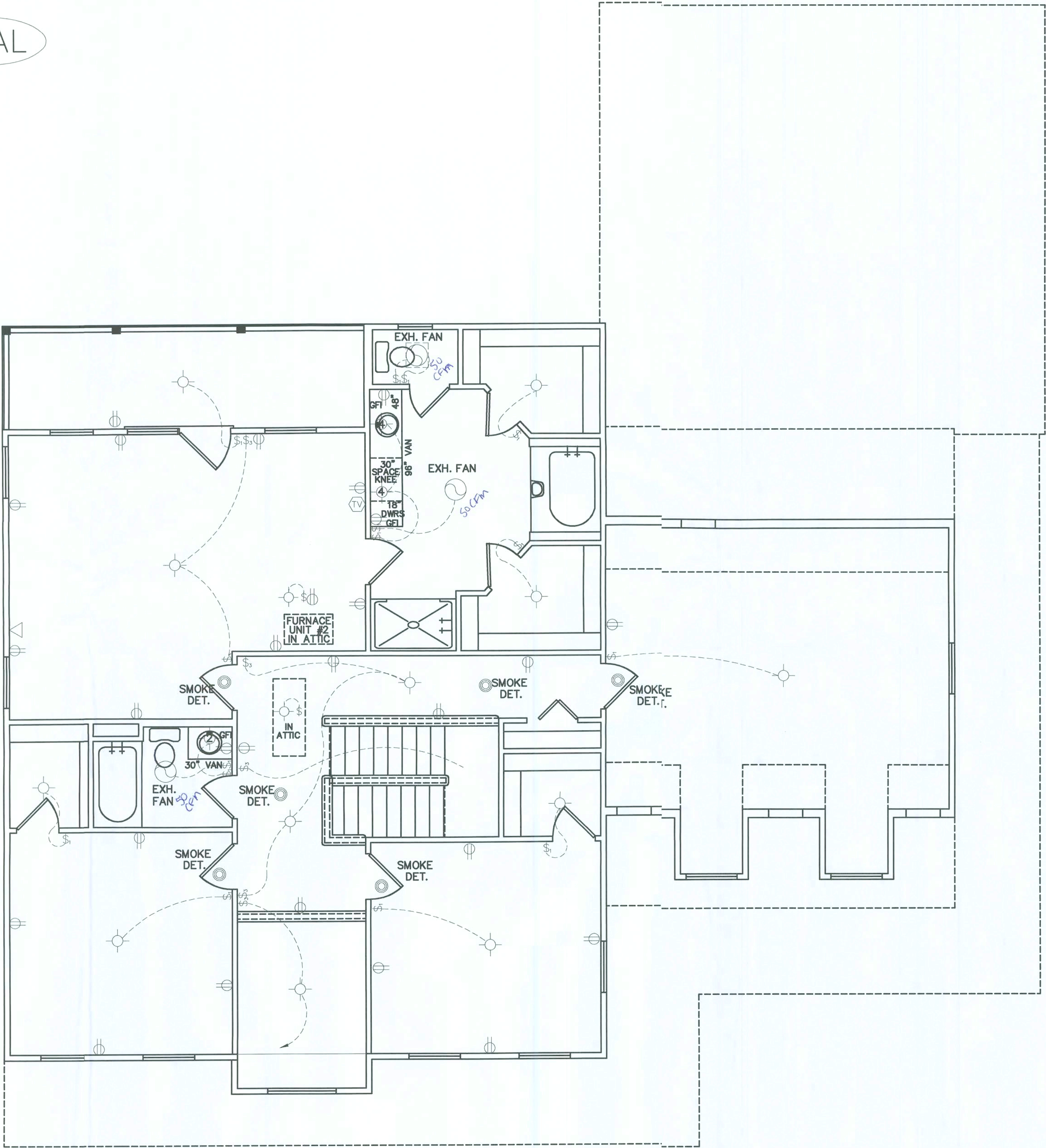
- CLG. OR WALL FIXTURE
- RECESSED CLG. FIXTURE
- FLOURESENT FIXTURE (CLG. OR WALL MNT.)
- OUTLET
- OUTLET 220V
- OUTLET-TOP HOT
- FLOOR OUTLET
- SWITCH
- SWITCH-3 WAY
- SWITCH-4 WAY
- DIMMER SWITCH
- FLOODLIGHT
- DOOR BELL
- EXH. FAN
- SMOKE DETECTOR
- TELEPHONE
- JUNCTION BOX
- THERMOSTAT
- TELEVISION

NOTE: SITE DROP	
SEWER	—
HOSE BIBB	+
WATER LINE	---
POWER METER	⏏
GAS LINE	—
UNDERGROUND POWER	—
OVERHEAD POWER	—

PROPERTY OF AMERICA'S HOME PLACE, INC.		THE HANOVER FOR MICHAEL & FADRA McINTOSH		DATE 2-28-08 REVISIONS 1992 S.F. 1344 S.F. 74 S.F. 3390 S.F. 3390 S.F. 633 S.F. 486 S.F. 275 S.F.		SILVESTRE ENGINEERING & DESIGN, INC. 1801 OGLESBY AVE. - WINTER PARK - FLORIDA - 32789 PHONE - (407) 644 5859 FAX - (407) 647 5758 CA #9144		2 x 4 EXTERIOR WALLS UNLESS OTHERWISE NOTED	
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						SOLD BY KIM RICKOLT			
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UPPER LEVEL ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

NOTES

BRACE LIVING ROOM AND ALL BEDROOMS FOR PADDLE FAN
ALL ELECTRICAL IN BEDROOMS SHALL BE ON AN A.F.C.I. PER NEC 2002

LEGEND

- CLG. OR WALL FIXTURE
- RECESSED CLG. FIXTURE
- FLOURESENT FIXTURE (CLG. OR WALL MNT.)
- OUTLET
- OUTLET 220V
- OUTLET-TOP HOT
- FLOOR OUTLET
- SWITCH
- SWITCH-3 WAY
- SWITCH-4 WAY
- DIMMER SWITCH
- FLOODLIGHT
- DOOR BELL
- EXH. FAN
- SMOKE DETECTOR
- TELEPHONE
- JUNCTION BOX
- THERMOSTAT
- TELEVISION

NOTE: SITE DROP	
SEWER	—
HOSE BIBB	+
WATER LINE	---
POWER METER	⏏
GAS LINE	I
UNDERGROUND POWER	—
OVERHEAD POWER	—

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			FIRST FLOOR	1862 SF.	SECOND FLOOR	1344 SF.	SECOND FLOOR HEATED	74 SF.	3380 SF.	BONUS ROOM (UNFINISHED)	380 SF.
			FRONT PORCH	488 SF.	REAR PORCHES	275 SF.					
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2 x 4 EXTERIOR WALLS UNLESS OTHERWISE NOTED			UPPER LEVEL WIRING PLAN			SHEET No.			15		
DRAWN BY JDC			SCALE: 1/4" = 1'-0"			DRAWN BY			15		
SOLD BY KIM RICKOLT						SOLD BY			OF		

STRUCTURAL GENERAL NOTES:

GENERAL:

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ALL DETAILS AND SECTIONS SHOWN ON THESE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUCTED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL IS SHOWN.

SHOP DRAWINGS IN THE FORM OF REPRODUCIBLE SEPIAS OF ARCHITECTURAL OR STRUCTURAL DRAWINGS (CONTRACT DOCUMENTS) ARE PROHIBITED WITHOUT EXPRESS WRITTEN PERMISSION FROM ARCHITECT/ENGINEER.

MANUFACTURERS OF ANY PRE-ENGINEERED, PRE-FABRICATED SYSTEM OR COMPONENT SHALL SUBMIT SHOP DRAWINGS FOR THE ENGINEER/ARCHITECT'S APPROVAL PRIOR TO FABRICATION OR ERECTION OF SYSTEM/COMPONENT. THESE SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED ENGINEER.

THE NEW STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE 2004 EDITION OF FLORIDA BUILDING CODE. FOLLOW ALL APPLICABLE PROVISIONS FOR ALL PHASES OF CONSTRUCTION.

THE STRUCTURAL INTEGRITY OF THE COMPLETED STRUCTURE DEPENDS UPON THE INTERACTION OF VARIOUS CONNECTED COMPONENTS. PROVIDE ADEQUATE BRACING, SHORING, AND OTHER TEMPORARY SUPPORTS AS REQUIRED TO SAFELY COMPLETE THE WORK.

FOUNDATIONS:

GEOTECHNICAL ENGINEERING EVALUATION AND SUBSURFACE EXPLORATION SHALL PERFORM BY OWNER'S GEOTECHNICAL CONSULTANT.

MAXIMUM ALLOWABLE SOIL PRESSURE IS ASSUMED TO BE 2000 POUNDS PER SQUARE FOOT.

SPREAD FOOTINGS SHALL BEAR ON SOIL COMPACTED TO A DENSITY OF AT LEAST 95 % OF MODIFIED PROCTOR MAXIMUM DENSITY (A.S.T.M. D1557), FOR ALL REQUIRED FILL AND FOR AT LEAST 1'-0" BELOW FINISHED FLOOR UNLESS MORE STRINGENT REQUIREMENTS ARE RECOMMENDED BY OWNER'S GEOTECHNICAL CONSULTANT.

CONCRETE:

DESIGN OF REINFORCED CONCRETE CONFORMS WITH BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE - ACI 318, LATEST EDITION.

ALL CONCRETE FOR FOUNDATIONS, SLAB AND GRADE, CMU FILLED CELLS, CONC. COLUMN AND BEAMS SHOULD HAVE AN UNCONFINED COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.

GENERAL CONTRACTORS SHALL CHECK ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR OPENINGS, SIEVES, ANCHORS, HANGERS, SLAB DEPRESSIONS AND ANY OTHER ITEMS RELATED TO CONCRETE WORK. AND SHALL ASSUME FULL RESPONSIBILITY FOR THEIR PROPER LOCATION, BEFORE POURING CONCRETE.

AT OPENINGS IN SLABS OR WALLS, PROVIDE 2 - #6 BARS EACH 6'-0" LONGER THAN WIDTH OF OPENING ON EACH OF SIDES OF OPENINGS, UNLESS SHOWN OTHERWISE.

PITCH CONCRETE SLABS, WHERE REQUIRED TO FLOOR DRAINS SHOWN ON ARCHITECTURAL AND MECHANICAL DRAWINGS.

CONCRETE PROTECTION FOR REINFORCING BARS SHALL BE AS FOLLOWS:

FOOTINGS 3" CLEAR, BOTTOM AND SIDES, 2" CLEAR, TOP
SLABS ¾" CLEAR, 1½" CLEAR AT AREAS EXPOSED TO WEATHER
BEAMS/ COLUMNS 1½" CLEAR TO STIRRUPS/ TIES

DESIGN MIXES:

PROVIDE NORMAL WEIGHT CONCRETE WITH THE FOLLOWING PROPERTIES:

3000 PSI 28-DAY COMPRESSIVE STRENGTH, AIR CONTENT 4 TO 6 %, SLUMP 3" TO 5" SUBMIT MIX DESIGN FOR ARCHITECT'S/ENGINEER'S REVIEW PRIOR TO START OF CONSTRUCTION.

CONCRETE CURING AND TESTING:

CURING: PROTECT CONCRETE FOR 7 DAYS AGAINST MOISTURE LOSS, RAPID TEMPERATURE CHANGE, MECHANICAL INJURY AND INJURY FROM RAIN OR FLOWING WATER. MAINTAIN CONCRETE IN MOIST CONDITION AT TEMPERATURE ABOVE 50 DEGREES F. THROUGHOUT SPECIFIED CURING PERIOD. PROTECT FROM RAPID TEMPERATURE CHANGE AND RAPID DRYING FOR FIRST 24 HOURS FOLLOWING REMOVAL OF TEMPERATURE PROTECTION. START CURING ACTIVITIES AS SOON AS FREE WATER HAS DISAPPEARED FROM SURFACES OF CONCRETE AFTER PLACING AND FINISHING.

TESTING: CONCRETE TESTING FOR THIS PROJECT SHALL BE PAID FOR BY OWNER, AND SHALL CONSIST OF COMPRESSIVE TESTS MADE BY THE LABORATORY IN ACCORDANCE WITH ASTM C-31. FOLLOW ASTM C31 AND MAKE A SET OF SIX (6) STANDARD CYLINDERS FOR EACH 100 CU. YDS. OR FOR EACH DAY'S POUR EXCEEDING 5 CU. YDS. TEST PER ASTM C31 AS FOLLOWS: TWO (2) SPECIMENS TESTED AT SEVEN (7) DAYS, ONE (1) AT 4 DAYS. TWO (2) TESTED AT 28 DAYS, AND ONE (1) HELD IN RESERVE. SLUMP TEST SHALL BE MADE IN ACCORDANCE WITH ASTM C-143 FOR EACH DAY'S POUR FOR EACH LOAD, OR AS DIRECTED BY ARCHITECT/ENGINEER.

CONCRETE ANCHOR AND FASTENERS:

EMBEDMENT ANCHORS SHALL BE NELSON EMBEDDED ANCHORS WITH FLUSH ENDS (OR APPROVE EQUAL), SIZE AND SPACING AS NOTED ON THE DRAWINGS. STUDS SHALL BE END WELDED IN ACCORDANCE WITH RECOMMENDATIONS OF THE NELSON STUD WELDING COMPANY.

EXPANSION ANCHORS SHALL BE WS SERIE WEDGE ANCHORS AS MANUFACTURED BY PHILLIPS RED HEAD, OR HILTI KWIK BOLTS AS MANUFACTURED BY HILTI, INC. OR APPROVED EQUAL.

IN LIEU OF EXPANSION ANCHORS CAST-IN-PLACE INSERTS WILL BE PERMITTED AS MANUFACTURED BY HOMAN & BERNARD, OR APPROVED EQUAL.

PRECAST U-LINTELS:

ALL "U" TYPE PRECAST LINTELS INDICATED ON DRAWINGS ARE TO BE PRECAST AND/OR PRESTRESSED IN ACCORDANCE WITH SPAN AND LOAD CAPACITIES AS INDICATED IN "DESIGN MANUAL", OCT. 1975, BY CAST-CRETE CORP., TAMPA, FLORIDA (LINTEL SHALL BE BY CAST-CRETE OR APPROVE EQUAL).

ALL "U" TYPE PRECAST LINTELS SHALL HAVE 8" MINIMUM BEARING EACH END WHEN FRAMED TO MASONRY WALLS AND 1-1/2" MINIMUM BEARING EACH END WHEN FRAMED TO CONCRETE WALLS OR COLUMNS, UNLESS NOTED OR SHOWN OTHERWISE.

REINFORCING STEEL:

REINFORCING STEEL SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615-GRADE 60, EXCEPT THAT NEW BILLET STEEL CONFORMING TO ASTM A615-GRADE 40 MAY BE USED FOR COLUMN TIES AND BEAMS STIRRUPS. ALL DETAILING AND ACCESSORIES SHALL CONFORM TO TYPICAL DETAILS SHOWN IN THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES ACI 315, LATEST EDITION".

ALL CONTINUOUS VERTICAL OR HORIZONTAL BARS IN FOOTINGS, FOUNDATIONS WALLS, SLABS AND OTHER CONCRETE SHALL BE LAP-SPLICED, WHERE NECESSARY OR DESIRABLE, BY WIRING TOGETHER IN CONTACT, LENGTH OF ALL LAPS SHALL BE 48-BAR DIAMETERS OR 2'-8" MINIMUM, WHICHEVER IS GREATER (EXCEPT AS NOTED BY DRAWINGS). ALL BARS AT END OF CONTINUOUS FOOTINGS OR BEAMS SHALL BE CONT. TO FAR SIDES OF INTERSECTING ELEMENTS.

ALL SLABS ON GRADE SHALL BE 4" THICK AND REINFORCED WITH 6 X 6 - W4 X W4 W.W.F. UNLESS OTHERWISE NOTED. LAP FABRIC 8" AT EDGES AND ENDS AND PROVIDE ADDITIONAL REINFORCING WHERE SHOWN ON DRAWINGS. PLACE MESH IN CENTER OF SLAB. MOISTURE BARRIER BENEATH FLOOR SLABS SHALL BE 6 MIL POLYETHYLENE. USE FLAT SHEETS OF WELDED WIRE FABRIC. ROLLS WILL NOT BE PERMITTED.

CONCRETE MASONRY:

ALL CONCRETE MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD BEARING CONCRETE MASONRY: OF THE NATIONAL CONCRETE MASONRY ASSOCIATION".

CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-30. ALL CONCRETE BLOCKS SHALL HAVE AN ULTIMATE NET COMPRESSIVE STRENGTH OF 2000 PSI MINIMUM TO GIVE A MINIMUM COMPRESSIVE STRENGTH OF MASONRY ASSEMBLAGE (F'M) OF 1500 PSI, UNLESS NOTED OTHERWISE. USE SPLIT FACE OR REGULAR BLOCK AS SHOWN ON ARCHITECTURAL DRAWINGS.

MORTAR SHALL BE ASTM C-270, TYPE "S" FRESHLY PREPARED AND UNIFORMLY MIXED WITH MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI @ 28 DAYS.

GROUT USED IN CONCRETE BLOCKS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS.

MAXIMUM HEIGHT OF GROUT POUR SHALL BE 4'-0" UNLESS CLEAN OUT OPENINGS ARE PROVIDED AT THE BOTTOM OF CELLS TO BE FILLED. ALL GROUT SHALL BE PUDDLED WITH GROUT STICK IMMEDIATELY AFTER POURING.

FILL ALL CELLS WITH CONCRETE GROUT CONTAINING REINFORCING STEEL. CONTINUOUS INSPECTION IS REQUIRED FOR ALL MASONRY WALL CONSTRUCTION.

LIGHT GAGE METAL FRAMING:

PROVIDE FRAMING MEMBERS AND CONNECTIONS IN ACCORDANCE WITH THE AISI SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, LATEST EDITION. USE UNIMAST CORP. GYPSUM COMPANY FRAMES, BRACING AND CONNECTIONS, OR AS APPROVED EQUAL.

STEEL STUDS SHALL BE PER DESIGN CRITERIA AS NOTED HEREIN: GALV G-60 ASTM A525, MINIMUM YIELD STRESS TO BE 50,000 PSI FOR 14 & 16 GA., AND 33,000 PSI FOR 18 & 20 GA. TRACKS, BRACING AND ALL OTHER COMPONENTS TO BE 33,000 PSI.

BRACING DURING CONSTRUCTION: PROVIDE 1½" COLD ROLLED CHANNELS OR HORIZONTAL STRAP BRACING FOR WALLS IMMEDIATELY AFTER STUDS ARE ERECTED.

CONNECT STUDS TO TOP TRACK W/ (2) #12 TEK SCREWS.

STRUCTURAL STEEL:

FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH A.I.S.C. "SPECIFICATIONS FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDING", LATEST EDITION.

ALL STRUCTURAL STEEL SHALL MEET THE REQUIREMENTS OF A.S.T.M. A-36, EXCEPT STRUCTURAL STEEL TUBE SHAPES SHALL MEET THE REQUIREMENTS OF A.S.T.M. A500, GRADE B. ALL STEEL PIPE SHALL MEET THE REQUIREMENTS OF ASTM A53 E OR S, GRADE B.

ALL BOLTS SHALL MEET THE REQUIREMENTS OF A.S.T.M. A325. BOLTS SHALL BE ¾" DIAMETER EXCEPT WHERE SHOWN OTHERWISE.

ALL WELDED CONNECTIONS SHALL BE DESIGNED TO BE FULLY EQUIVALENT IN STRENGTH TO BOLTED CONNECTIONS FOR THE SAME SIZE BEAM. ALL WELDS FOR COLUMN CAP AND BASE PLATES SHALL BE CONTINUOUS 1/4" FILLET WELDS, UNLESS SHOWN OTHERWISE.

ALL WELDING SHALL BE DONE BY CERTIFIED WELDERS IN ACCORDANCE WITH AWS "CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION", LATEST EDITION. USE E70-XX ELECTRODES FOR ALL WELDING.

PROVIDE ALL STRUCTURAL STEEL LINTELS, SHELF ANGLES, AND ALL OTHER ROLLED SHAPES SHOWN ON ARCHITECTURAL DRAWINGS.

FIELD BOLT CONNECTIONS WILL BE TESTED AND INSPECTED ACCORDING TO RCSC'S "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A 325 OR A 490 BOLTS".

OPEN WEB JOISTS:

THE DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE'S "STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS", LATEST EDITION. FURNISH SIGNED & SEALED STATEMENT VERIFYING COMPLIANCE WITH THE ABOVE.

JOIST MANUFACTURER SHALL CHECK AND DESIGN JOIST DESIGNATIONS SHOWN ON THE DRAWINGS FOR LONGITUDINAL STRESSES FROM WIND UPLIFT. JOIST MANUFACTURER SHALL ALSO CHECK AND DESIGN ALL JOISTS THAT SUPPORT NON-UNIFORM LOADS. NET WIND UPLIFT OF 20 PSF WILL BE USED FOR INTERIOR ZONE AND 28 PSF FOR CORNER AND EDGE ZONE. CORNER AND EDGE ZONE IS 6'-0" WIDE.

METAL ROOF DECK:

METAL DECK SUPPORTING ROOF CONSTRUCTION SHALL BE INTERMEDIATE RIB DECK, TYPE B, 22 GAGE AS MANUFACTURED BY WHEELING CORRUGATING CO., OR APPROVED EQUAL. WELD DECK TO SUPPORTS AS SHOWN. STRUCTURAL WOOD FRAMING.

ALL FRAMING SHALL BE DESIGNED, FABRICATED, AND INSTALLED AS PER AISC AND TPI SPECIFICATIONS.

ALL WOOD STRUCTURAL MEMBERS SHALL BE CONTROLLED STRESS GRADE LUMBER HAVING A FIBER STRESS OF AT LEAST 1300 PSI.

PROVIDE TEMPORARY TOP CHORD BRACING UNTIL ROOF DECK IS IN PLACE.

ALL BOLTS SHALL BE ¾" DIAMETER AND SHALL CONFORM TO THE REQUIREMENTS OF A.S.T.M. A307 UNLESS OTHERWISE NOTED.

SHEAR PLATES SHALL BE EITHER PRESSED STEEL TYPE (SAE 1010) OR MALLEABLE IRON TYPE, (ASTM A47-33).

ALL WOOD MEMBERS EXPOSED TO WEATHER OR BEARING ON CONCRETE OR STEEL SHALL BE PRESSURE TREATED.

LIGHT GAGE METAL STUD TRUSSES:

TRUSS MANUFACTURER SHALL CHECK AND DESIGN TRUSSES SHOWN ON DRAWINGS TO ENGINEER FOR APPROVAL. SUBMITTAL SHALL INCLUDE THE FOLLOWING INFORMATION:

- LAYOUT PLAN - DESIGNATING TRUSS TYPES.
- TRUSS DIAGRAMS - SHOWING OUTLINE OF TRUSSES.
- ENGINEERING CUT SHEETS - SHOWING GRAVITY AND UPLIFT DESIGN WITH MAGNITUDE OF BOTH SETS OF REACTIONS.
- ENGINEERING GIRDER TO GIRDER CONNECTORS, AND LIST ALL STANDARD CONNECTORS FOR TRUSS TO TRUSS, TRUSS TO MASONRY OR WOOD WALLS NOT DETAILED BY STRUCTURAL DRAWINGS. USE SIMPSON CATALOG OR SUBMIT EQUAL.

THE GENERAL CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND ENGINEERING REVIEWED BY ARCHITECT/ENGINEER TO THE BUILDING DEPARTMENT PRIOR TO TRUSS ERECTION.

ROOF TRUSSES:

FOR TYPICAL SOUTHERN YELLOW PINE 2 X 4'S	
TRUSS T.C. LIVE AND DEAD LOAD (SHINGLE & METAL) 27PSF	TILE 35 PSF
B.C. DEAD LOAD	10 PSF
10 PSF	
TOTAL	37 PSF
45 PSF	

DESIGN LOADS:

LIVE LOADS:	ROOF:	20 PSF
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ALL STRUCTURAL COMPONENTS SHALL MEET DESIGN CRITERIA SET FORTH IN FLORIDA BUILDING CODE (CURRENT VERSION) CHAPTER 16. MINIMUM NET UPLIFT REQUIREMENTS SHALL BE 25 PSF TYPICAL AND 13.0' EDGE ZONE. ALL DOORS, WINDOWS & GLAZING FRAMING SHALL BE ABLE TO WITHSTAND WITH PRESSURE FOR COMPONENTS AND CLADDING PER FBC CODE, CHAPTER 16. SEE ARCHITECTURAL DRAWINGS FOR SIZES AND LOCATIONS. SEE DESIGN PRESSURE TABLE SHEET.

WIND LOADS (VELOCITY)	120 MPH
IMPORTANCE FACTOR	1.0
EXPOSURE	B
ENCLOSURE	"ENCLOSED BUILDING"
INTERNAL PRESSURE COEFFICIENT	.18
BUILDING CATEGORY	TYPE 2
HEIGHT	30'-0"

TIMBER ROOF TRUSSES:

ON THIS ITEM SHALL APPLY ALL THE PERTAINING DRAWINGS, SPECIFICATIONS AND DETAILS PROVIDED BY UNIVERSAL TIMBER STRUCTURES FOR TIMBERS, PLATES AND CONNECTIONS.

STRUCTURAL CLASSIFICATION:

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH, AND MEETS THE REQUIREMENTS OF ALL APPLICABLE STRUCTURAL SECTIONS OF FLORIDA BUILDING CODE 2004 EDITION WHERE APPLICABLE FOR 120 MILES PER HOUR OR LESS WIND SPEED CERTIFICATION BY ENGINEER APPLIES TO STRUCTURAL DESIGN ONLY. BASIC WIND SPEED = 120 MPH WIND IMPORTANCE FACTOR = 1.0 WIND EXPOSURE CATEGORY = B INTERNAL PRESSURE COEFFICIENT = +/-0.18 FULLY ENCLOSED WIND PRESSURES FOR COMPONENTS AND CLADDING FOR WALL ELEMENTS SHALL BE SHOWN ON FLOOR PLAN AND OR ELEVATIONS

REVISIONS	BY

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2004 EDITION OF THE FLORIDA BUILDING CODE. FOLLOW ALL APPLICABLE PROVISIONS FOR ALL PHASES OF CONSTRUCTION. THIS STRUCTURE IS DESIGNED FOR A WIND SPEED OF 120 MPH OR LESS WIND SPEED AND COMPLES WITH SECTION R301.1 DESIGN CRITERIA. BASIC WIND SPEED = 120 MPH WIND EXPOSURE CATEGORY = B WIND IMPORTANCE FACTOR = 1.0 INTERNAL PRESSURE COEFFICIENT = +/-0.18 FULLY ENCLOSED WIND PRESSURES FOR COMPONENTS AND CLADDING FOR WALL ELEMENTS SHALL BE (+) 25.9 & (-) 34.7 PSF (WORST CASE)

SILVESTRE ENGINEERING & DESIGN, INC.

1801 OGLESBY AVE. - WINTER PARK, FL - 32789

PHONE: (407) 644-5559

FAX: (407) 647-5758

EMAIL: mick@silvestre-engineering.com

SED

DRAWN

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DATE

SCALE

JOB NAME

McINTOSH RESIDENCE

S-1

OF SHEET

SED

CERTIFICATE OF AUTHORIZATION # 9144

SILVESTRE ENGINEERING & DESIGN
1801 OGLESBY AVE.
WINTER PARK, FL 32789
ORLANDO: (407) 644-5559



CONNECTORS TO EXISTING GROUTED CMU SIMPSON

#	CONNECTOR	FASTENER	UPLIFT (lbs.)
30	MTS16	(7) 10d x 15 NAILS & (5) 3/16" Ø x 2 1/4" TAPCONS	1000
31	HTS20	(10) 10d x 15 NAILS & (6) 3/16" Ø x 2 1/4" TAPCONS	1150
32	MTS12	(9) 10d NAILS & (6) 3/16" Ø x 2 1/4" TAPCONS	1545
33	MTS136	(13) 10d NAILS & (9) 3/16" Ø x 2 1/4" TAPCONS	1915
34	MTS140	(27) 10d NAILS & (16) 3/16" Ø x 2 1/4" TAPCONS	2475
35	CS16-R CUT LENGTH TO FIT	(11) 10d NAILS & (7) 3/16" Ø x 2 1/4" TAPCONS	1650
36	INVERTED HANGER	(2) 10d x 15 NAILS & (8) 3/16" Ø x 2 1/4" TAPCONS	1600
37	MASONRY HANGER	(16) 3/16" Ø x 2 1/4" TAPCONS	1600
38	MASONRY HANGER	(20) 3/16" Ø x 2 1/4" TAPCONS	1810
39	MBHA MASONRY HANGER	(1) 3/4" Ø THREADED ROD & TOP AND 2 FACE (4) 1/4" DIA x 1 1/2" SCREWS TO WOOD & (4) 1/4" Ø x 2 1/4" TAPCONS	3475
40	HGA10	(24) 3/16" Ø x 2 1/4" TAPCONS	2300
41	HGA48	(24) 3/16" Ø x 2 1/4" TAPCONS	2300
42	HGA55B2/10	1 2-1/2" x 5 TITEN HD TAPCONS 2 4-1/2" x 5 TITEN HD TAPCONS	5000 5400
43	FGTR	12-TITEN 1/4" x 2-3/4"	3350* G
44	HUC26-2	6-10d	305* U
45	HUC28-2	14-TITEN 1/4" x 2-3/4"	4350* G
46	HUC210-2	6-10d	305* U
47	HUC410	18-TITEN 1/4" x 2-3/4"	5005* G
48	HUC410	10-10d	1810* U

TYPICAL CONNECTIONS AND DETAILS

- (2) STUDS / (1) x 2 OR (2) x 2 TOP & (1) x 2 BASE - TYP. # 1 STORY
- (3) STUDS / (1) x 3 OR (2) x 3 TOP & (1) x 3 BASE - TYP. # 1 STORY
- (2) STUDS / (1) x 2 OR (2) x 2 TOP & (1) x 2 OR (2) x 2 LOWER BEAM OR RIMBOARD
- (3) STUDS / (1) x 2 OR (2) x 2 TOP & (1) x 2 OR (2) x 2 LOWER BEAM OR RIMBOARD
- (4) STUDS OR 6x6 F.T. POST / (2) x 2 TOP AND (1) x 2 BASE
- (2) STUDS / (1) x 2 OR (2) x 2 TOP & (1) x 2 - 52" LONG FLOOR AND 2 STUDS BELOW 22" BASE
- (3) STUDS / (1) x 3 OR (2) x 3 TOP & (1) x 2 - 52" LONG FLOOR AND 3 STUDS BELOW 23" BASE
- (3) STUDS / (1) x 2 OR (2) x 2 TOP & (1) x 2 - 52" LONG BASE W/ (13) 10d NAILS TO STUDS AND (10) 3/16" Ø x 2 1/4" TAPCONS TO GROUTED CMU
- (3) STUDS / (1) x 3 OR (2) x 3 TOP & (1) x 3 - 52" LONG BASE W/ (13) 10d NAILS TO STUDS AND (10) 3/16" Ø x 2 1/4" TAPCONS TO GROUTED CMU
- 2nd FLOOR STUDS TO FLOOR SYSTEM WITH LST36 # 32" O.C. OR CS16-R 52" LONG # 32" O.C. AND AT CORNERS, ENDS AND TWO AT EACH SIDE OF OPENINGS
- 2x10 LEDGER BOARD W/ (3) 1/4" Ø x 4" WOOD SCREWS AT 16" O.C. AT FRAME OR 1/4"x4 1/2" TAPCONS AT 5" O.C. TO CMU
- (2) 2x12 LEDGER BOARD DOUBLE STAGGERED ROW OF 3/4" Ø ANCHOR BOLTS AT 16" O.C. TO GROUTED CMU OR CONCRETE
- 5 1/4" x 5 1/4" PARALLAM POST W/ (2) x 2 POST/BEAM/ 23" BASE (TYP.)
- 6"x6" P.T. POST W/ (2) x 2 POST/BEAM/ (2) x 2 BASE (TYP.)
- 4"x4" P.T. POST W/ (2) x 2 POST/BEAM/ (2) x 2 BASE (TYP.)
- 4"x4" P.T. POST W/ (2) x 3 POST/BEAM/ (2) x 2 BASE (TYP.)

2 STORY TYPICAL CONNECTIONS AND DETAILS

- (2) STUDS / (1) x 2 OR (2) x 2 TOP & (1) x 2 POST TO POST AND (2) x 2 BASE
- (3) STUDS / (1) x 2 OR (2) x 2 TOP & (1) x 2 POST TO POST AND (2) x 2 BASE
- 5 1/4" x 5 1/4" PARALLAM POST W/ (2) x 2 POST/BEAM/ (2) x 2 POST/POST/ (2) x 2 BASE (TYP.)
- 6" x 6" P.T. POST W/ (2) x 2 POST/BEAM/ (2) x 2 POST/POST/ (2) x 2 BASE (TYP.)
- 4"x4" P.T. POST W/ (2) x 2 POST/BEAM/ (2) x 2 POST TO POST, AND (2) x 2 BASE (TYP.)
- 4"x4" P.T. POST W/ (2) x 2 POST/BEAM/ (2) x 2 POST TO POST, AND (2) x 2 BASE (TYP.)



R-1 ROOF SHEATHING LAYOUT

CONNECTOR SCHEDULE SIMPSON

#	CONNECTOR	FASTENER	UPLIFT (lbs.)
1	ME118	(14) 10d x 15	1450
2	HE120	1 FLY (14) 10d x 15 2/3 FLY (12) 16d	1810
3	MTS16or MTS12	(14) 10d	840
4	H3 R H5	(8) 8d	455 uplift 125 lateral
5	HD A	7/8" Ø ANCHOR BOLT (3) 7/8" Ø BOLTS THRU	7910
6	HTS20	24 - 10d x 14 1/2"	1450
7	HHEA20	1 FLY 11 - 10d x 15 2/3 FLY 15 - 16d	2235
8	HG-2 or 3	(2) 3/4" ANCHORS	10530
9	SP1	6 - 10d	585
10	SP2	6 - 10d	890
11	SP4	6-10d x 14 1/2"	735
12	CS-R CUT LENGTH TO FIT	22 - 10d.	1705
13	LSA12	10 - 10d	805
14	LS124	18 - 10d	1235
15	LS130	22 10d	1640
16	LS136	26 - 10d	1640
17	MS136	26 - 10d	2050
18	HE116	1 ply - 11-10d x 14 1/2" 2 ply - 12-16d	1810
19	HU86	14 - 16d	1550
20	HGA48	36 - 16d	3220
21	FGTR	18-8DS-1/4" x 3" 36-8DS-1/4" x 3"	5000 5400
22	HT16	18 16d + 5/8" A Bolt	3480
23	HT22	32-16d Sinkers + 5/8" A Bolt	5250
24	ALU44	12-16d + 5/8" A Bolt	2200
25	AE56	12-16d + 5/8" A Bolt	2300
26	H2	8 - 8d x 14 1/2"	905
27	MT37	42-16d	3835
28	MT148	28 - 10d x 1 1/2"	2335

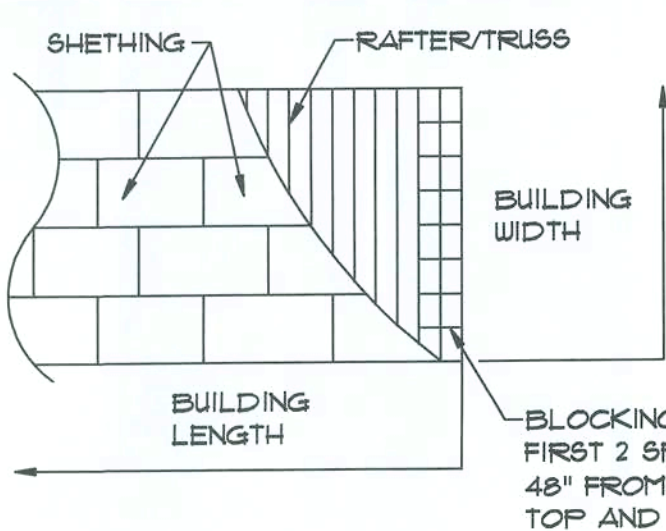
UNLESS NOTED OTHERWISE:

- WHERE CONNECTOR NOT NOTED FOR TRUSSES TO FRAME USE (3) / TRUSSES TO NEW CMU USE (1) / TRUSSES TO EXIST. CMU USE (2)
- ALL INTERIOR EAMS USE (6) x 2 ON FRAME AND (2) x 2 ON BLOCK
- ALL CONVENTIONALLY FRAMED MEMBERS USE 4 - 10d TOE NAILS AT EACH END OF MEMBERS
- USE HU826 AS 1" FLY HANGER AND HGA548 AS 2" FLY HANGER
- MINIMUM OF 1" BEDDMENT FOR ANCHOR BOLTS TO SLAB
- WHERE USED W/ WOOD FRAME WALLS, ALSO MUST USE (2) INVERTED HT122 UNDER PATES
- ALL INTERIOR RADDERS SHALL BE (2) 2 x 12 W/ 1/2" PLYWOOD BETWEEN AND (2) EACH WD
- ALL ROOF & FLOOR GIRDERS TO HAVE MULTIPLE STUD BEARING, ONE STUD PER FLY/TRAPPING AS NOTED
- ALL GIRDERS (1) CMU MUST HAVE MIN (1) FILLED CELL W/ #5 REBAR TIED TO TIE BEAM ABOVE AND FOOTING BELOW

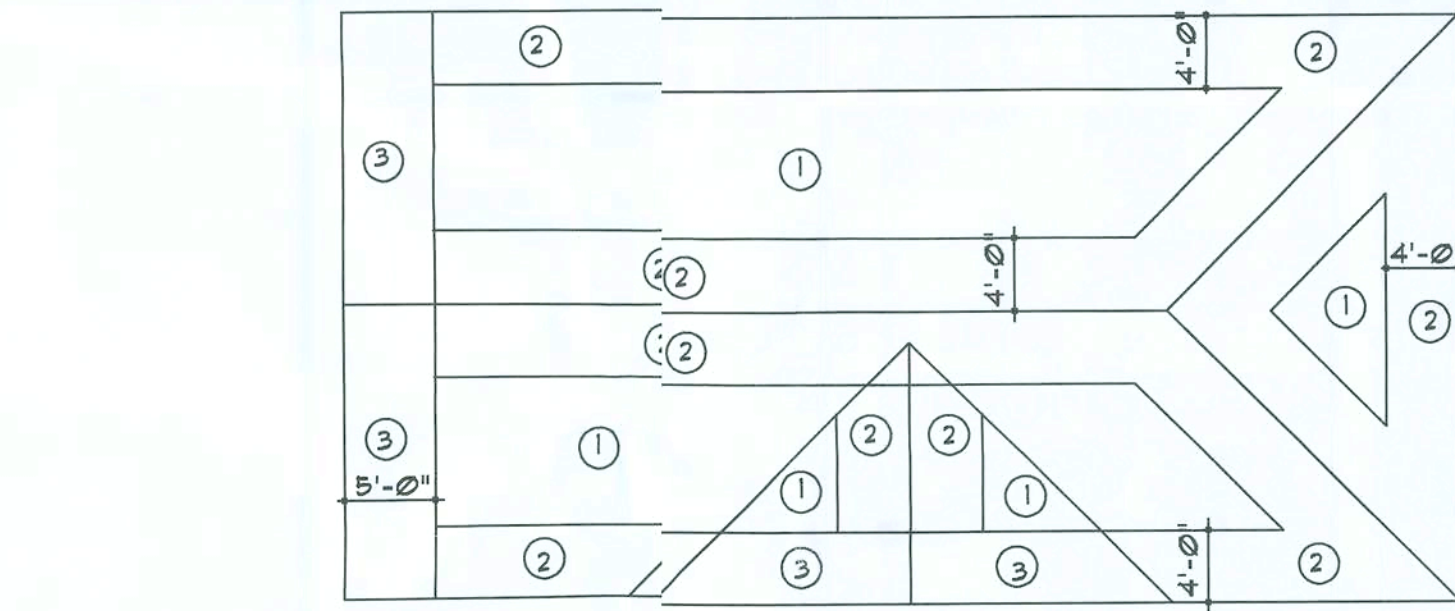
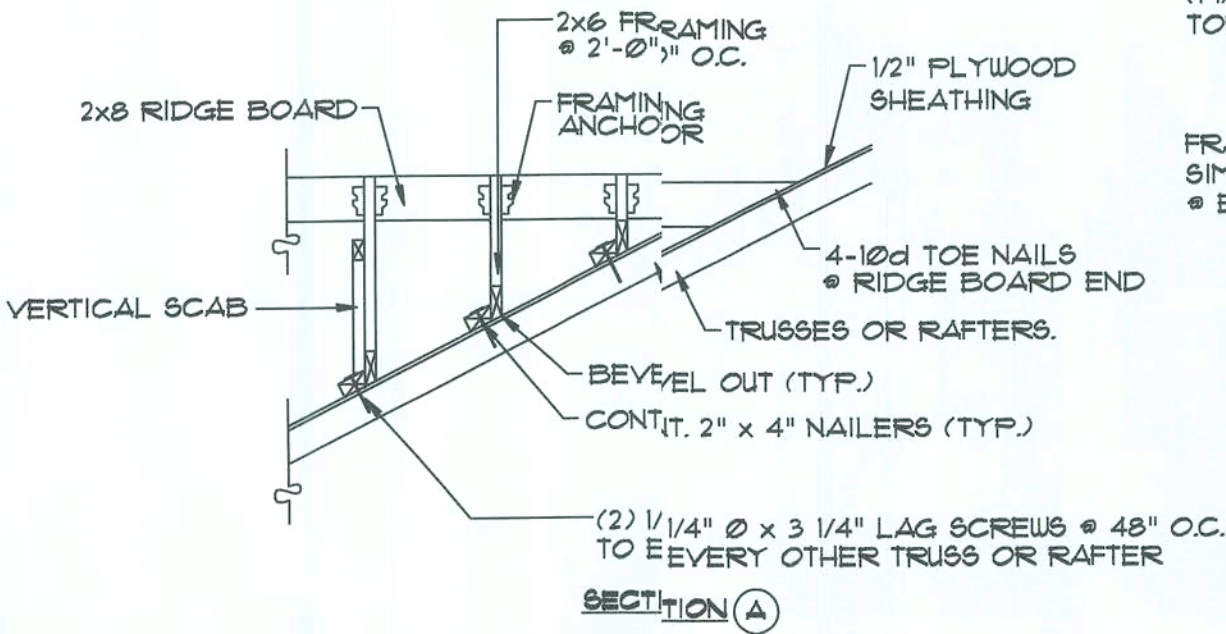
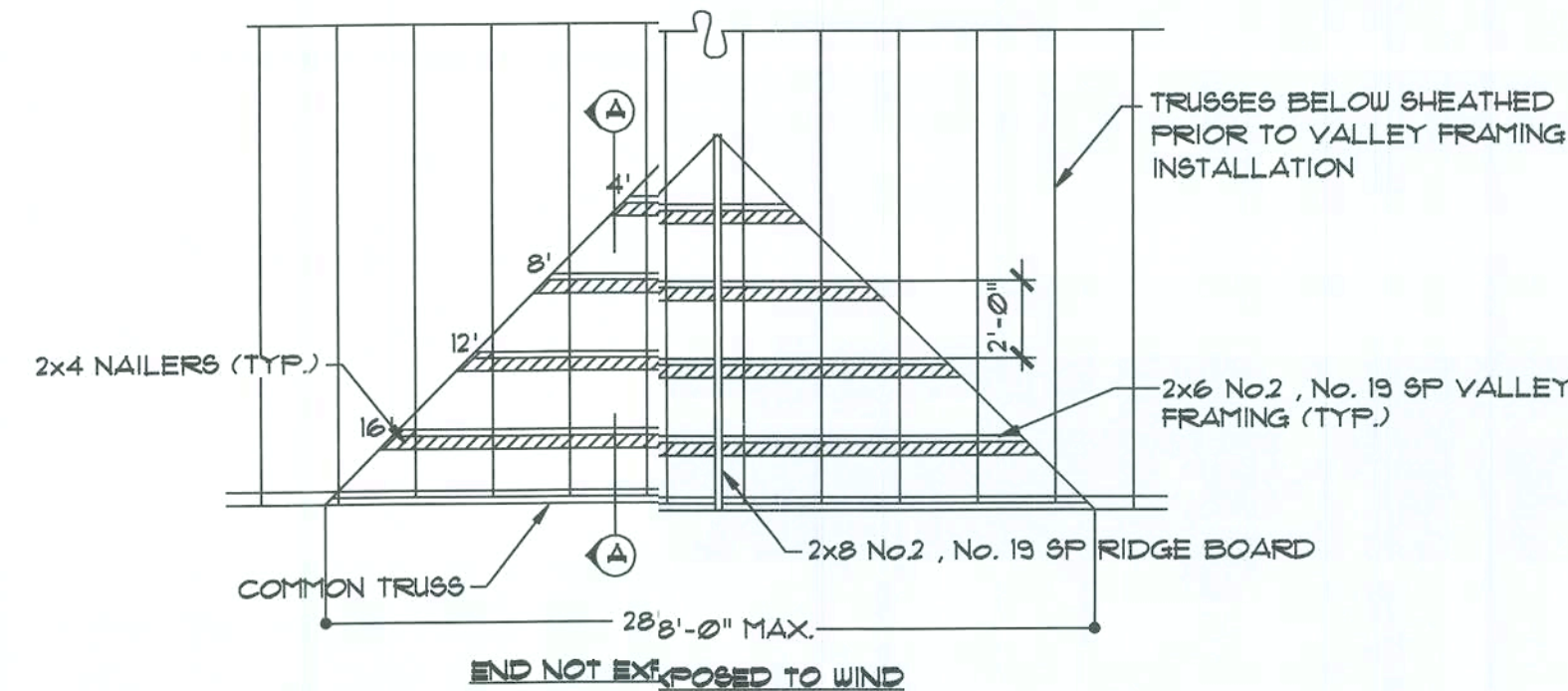
ROOF LIVE LOAD 20 P.S.F.
FLOOR LIVE LOAD 40 P.S.F.
STAIRS 100 P.S.F.
DECKS & BALCONIES 60 P.S.F.
WALLS - SHEAR ALL SEGMENTS (SEE ATTACHED DETAILS)
REF. PROJECT 3095 ENGINEERING TO BLOCK WALL

CMU WALL SYSTEM AS WELL AS ICF SEGMENTS WHICH HAVE AN UNINTERRUPTED LENGTH OF 4'-0" OR MORE SHALL BE CONSIDERED SHEAR WALL

R-3 CONNECTOR SCHEDULE



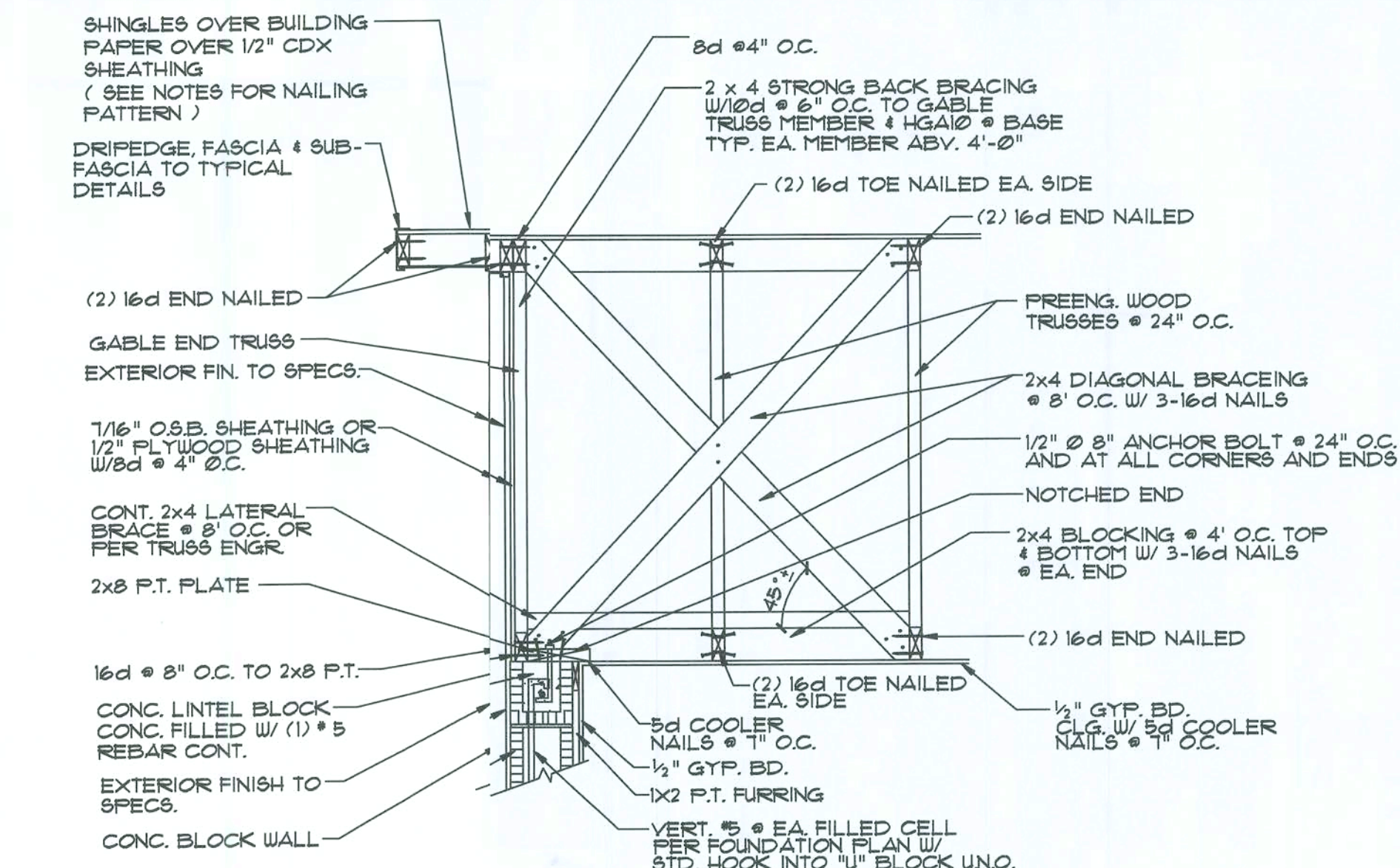
R-1 ROOF SHEATHING LAYOUT



ROOF SHEATHING	NAILS	PANEL LOCATIONS	ROOF FASTENING ZONES
1/16" STRUCTURAL SHEATHING	8d COMMON NAIL	PANEL EDGES & SUPPORTS (1) PANEL FIELD	1 6" O.C. 2 6" O.C. 3 4" O.C.

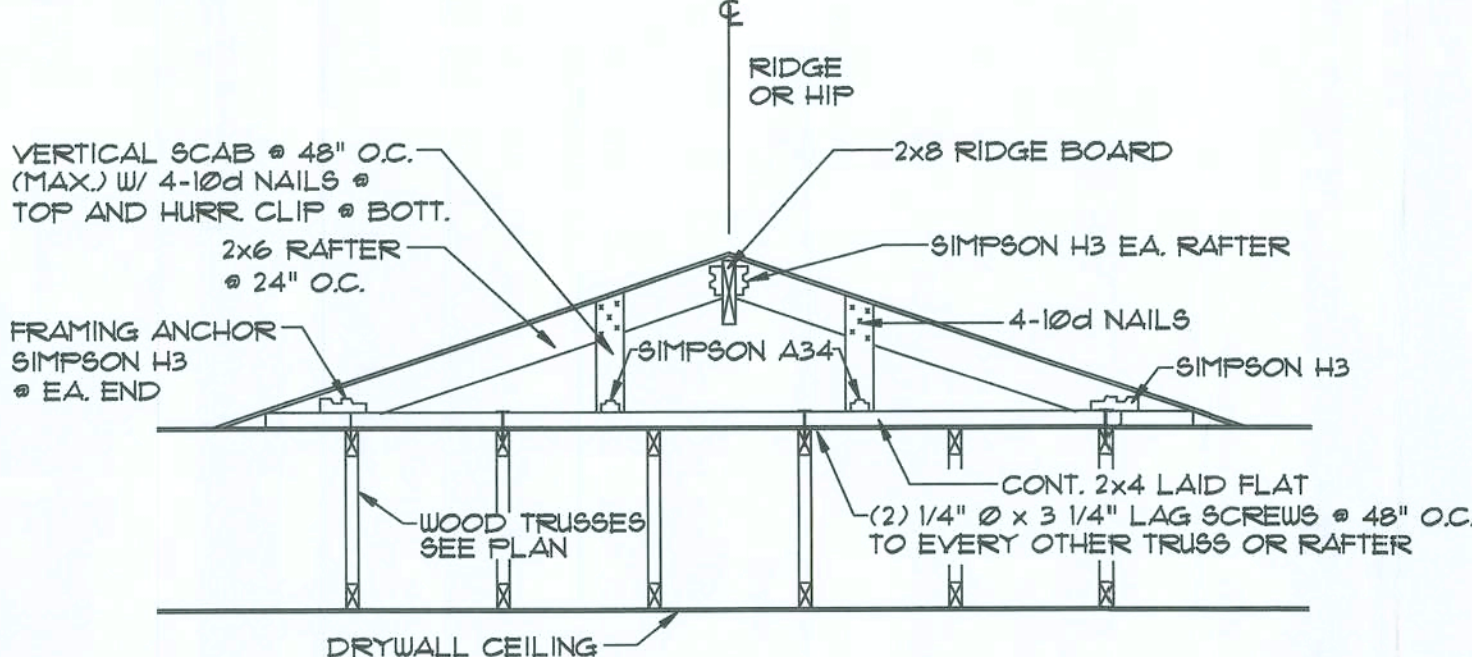
NOTE:
(1) EDGE SPACING: ALSO APPLIES OVER GABLE END WALLS OR TRUSSES
(2) PASSLOPE 2 1/4" x 2-035" DIAMETER POWER DRIVEN COATED SCREW NAILS MAY BE USED
IN LIEU OF 8d COMMON NAILS WITH REDUCED SPACING AS NOTED BELOW.
12" SPACE CHANGES TO 8", 6" TO 4", AND 4" OR 3" CHANGES TO 2 1/2".

R-2 ROOF SHEATHING FASTENING SCHEDULE

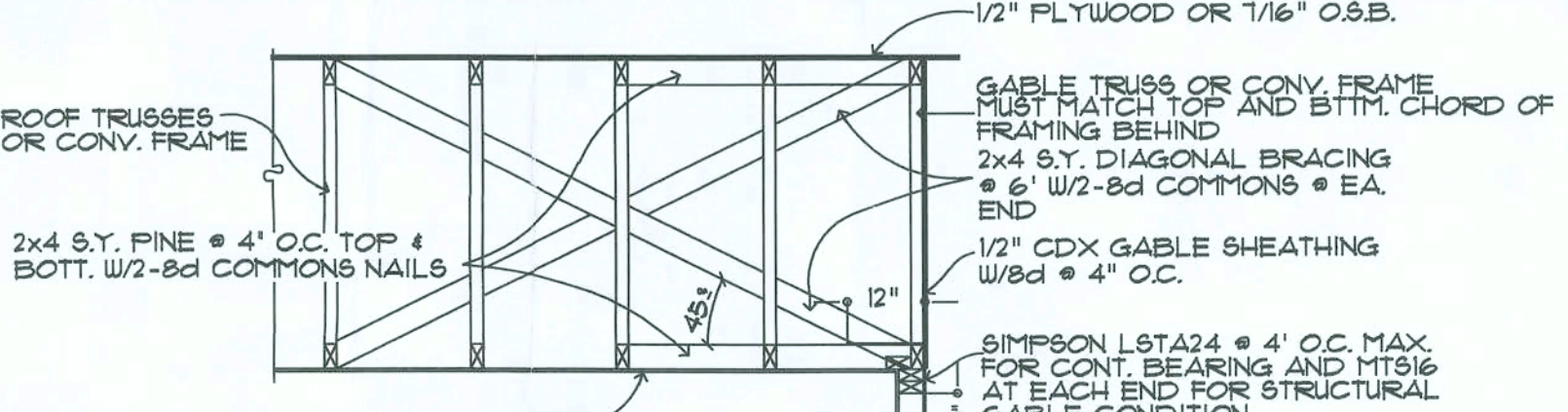


R-6 Gable End Bracing Detail

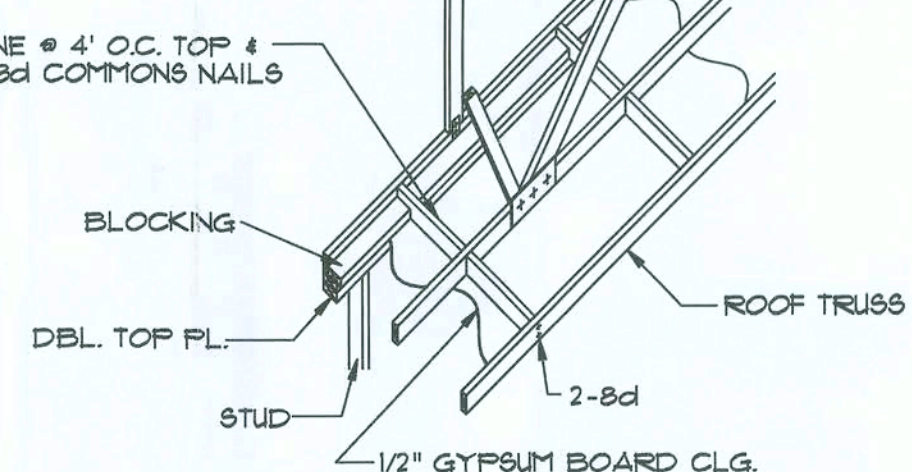
- NOTE:
- TRUSSES TO BE SHEATHED PRIOR TO VALLEY FRAMING
 - CONN. 2x4 NAILER TO ROOF W/ 2- 1/4" Ø x 3 1/4" LAG SCREWS # 48" O.C. - TO EVERY OTHER TRUSS OR RAFTER
 - CONN. 2x6 VALLEY FRAMING TO CONT. 2x4 # ONE END AND 2x8 RIDGE BOARD # OTHER END USING SIMPSON H3 FRAMING ANCHOR (OR EQUIV.)
 - ATTACH 2x4 VERT. SCAB TO 8", 12" AND 16" VALLEY FRAMING SPACED # 48" O.C. MAX. AND CONN. W/ 4 16d NAILS # TOP AND HURR. CLIP H3 # BOTTOM
 - VALLEY SECTION SHEATHED AFTER FRAMING



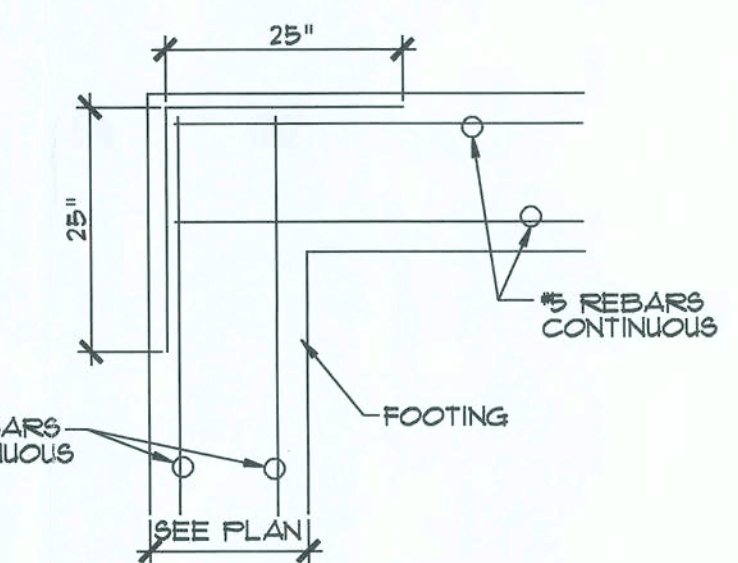
R-1 FIELD FRAMED VALLEY DETAIL



R-4 GABLE ENDWALL BRACING DETAIL FOR FLAT END OR VAULTED CEILING



R-2 CORNER FOOTING DETAIL



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CERTIFICATE OF AUTHORIZATION # 9144
SILVESTRE ENGINEERING & DESIGN
1801 OGLESBY AVE.
WINTER PARK, FL 32789
ORLANDO, (407) 644-5859

KENNETH W. SCHRAW
No. 17402
12/07/2009
FLORIDA
PROFESSIONAL ENGINEER

REVISIONS	BY

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2004 RESIDENTIAL 2006 SUPPLEMENTAL FLORIDA BUILDING CODE FOR 120 MPH OR LESS WIND SPEED AND COMPLIES WITH SECTION R501.1 DESIGN CRITERIA AND LOADS. DESIGN CRITERIA IS GOVERNED BY ASCE 7-02. UNIFORM FORCE RESISTANCE COEFFICIENT = 1.0. FULLY ENCLOSED. AVERAGE DESIGN WIND PRESSURE = 25 PSF. AVERAGE DESIGN WIND PRESSURE = 25 PSF. (Worst Case) WIND PRESSURES FOR COMPONENTS AND CLADDING FOR WALL ELEMENTS SHALL BE 17.2 PSF & 17.2 PSF (Worst Case).

SILVESTRE ENGINEERING & DESIGN, INC.
1801 OGLESBY AVE. - WINTER PARK, FL - 32789
PHONE: (407) 644-5859
FAX: (407) 644-5158
EMAIL: mick@silvestre-engineering.com

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S-2
OF SHEET



2 S.TUDS MIN.
(SEE PLAN)

HOLD DOWN PER PLAN
IN LIEU OF STRAPS

{ "x12"
EPOXY BOLT
EMBED 10" MIN.

10"

ALT. HOLD DOWN DETAIL AS REQ'D.
PER PLAN:
A { "x10" A.B. W/8" EMBEDMENT
MAY BE USED IN LIEU OF EPOXY BOLT

ADD STUDS UNDER GIRDERS
SEE PLAN FOR No. REQ'D.
UNLESS NOTED ON PLANS.
A MIN. OF 2 STUDS REQ'D.
NAIL TOGETHER AS REQ'D.
PER FASTENER SCHEDULE.

STRAP STUDS TOP & BOT.
SEE PLAN FOR TYPE

GIRDER OR BEAM
PER PLAN

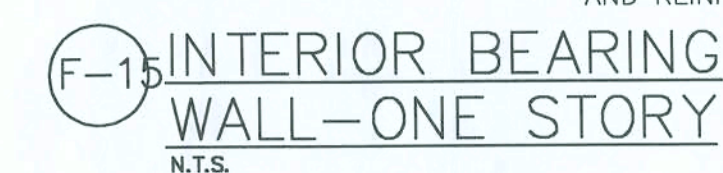
CONNECTORS
AS REQ'D.
PER PLANS

ANCHOR AS
REQ'D. PER PLAN

SIZE	#OF ROWS	2 PLY	3 PLY	4 PLY	5 PLY
2x4	1 ROW	10dx3"	20dx4"	50dx5 \ "	5/16"x7" SPIKE
2x6 2x8 2x10 2x12	2 ROWS	10dx3"	20dx4"	50dx5 \ "	5/16"x7" SPIKE

IN LIEU OF NAILMEMBERS MAY BE BOLTED TOGETHER WITH
" MACHINE BOLTS @ 16" O.C. WITH THE SAME No. OF ROWS
AS NAILS NOTED ABOVE.

(F-23) BUILT UP WOOD COLUMN DETAIL



*THE HEADER STUD SHALL NOT BE REQUIRED IF THE HEADER IS SUPPORTED BY A SUITABLE FRAMING ANCHOR

F-3 MINIMUM WALL AND HEADER STUD REQUIREMENTS

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2004 INTERNATIONAL BUILDING CODE (IBC) AND THE FOLLOWING DESIGN CRITERIA:
 BASIC WIND SPEED = 120 MPH OR LESS
 WIND LOADING CRITERIA IS GOVERNED BY ASCE 7-02.
 BASIC SEISMIC CATEGORY = III
 EXPOSURE CATEGORY = B
 WIND EXPOSURE FACTOR = 1.0
 AVERAGE DESIGN WIND PRESSURE = 26 PSF
 GARAGE DOOR PRESSURE = +22.8 (-25.8 WORST CASE)
 MINIMUM PRESSURES FOR COMPONENTS AND CLADDING FOR WALL ELEMENTS

SILVESTRE ENGINEERING & DESIGN, INC.

INGENIERING & CONSULTING
 1001 FSBY AVE. - WINTER PARK, FL - 32789
 PHONE: (407)-644-5859
 FAX: (407)-647-5758
 EMAIL: mick@silvestre-engineering.com

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ORLANDO: (407) 644-5859

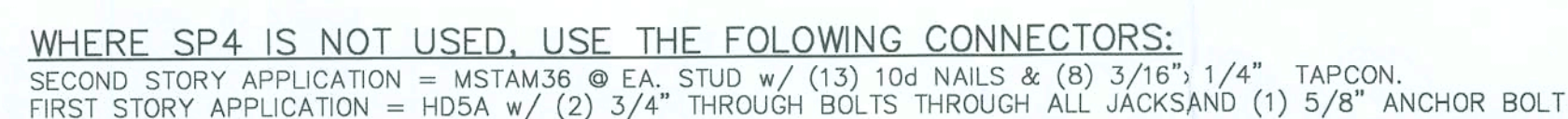
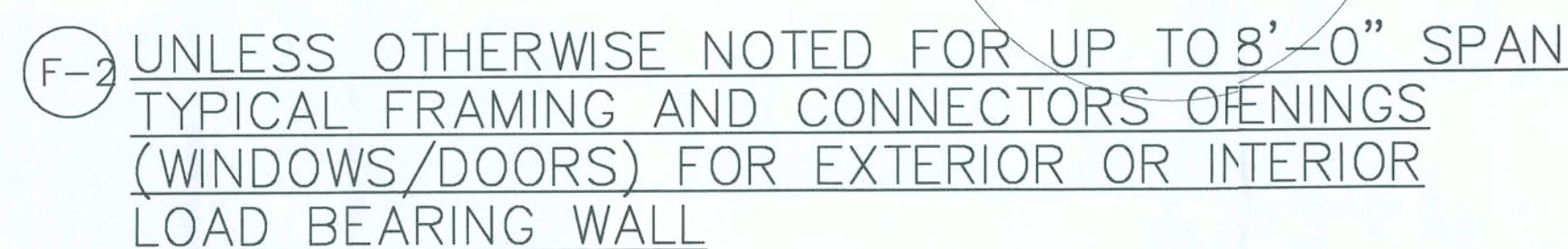
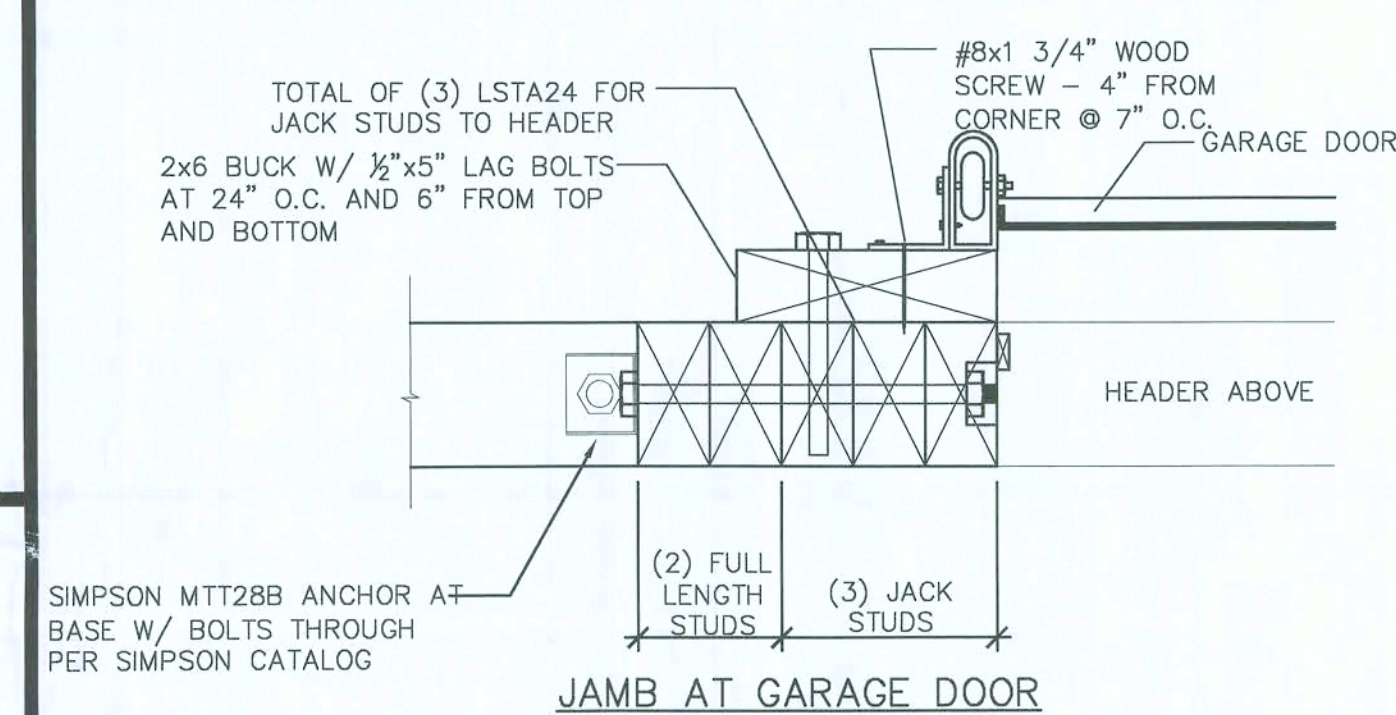


Diagram illustrating the header assembly details:

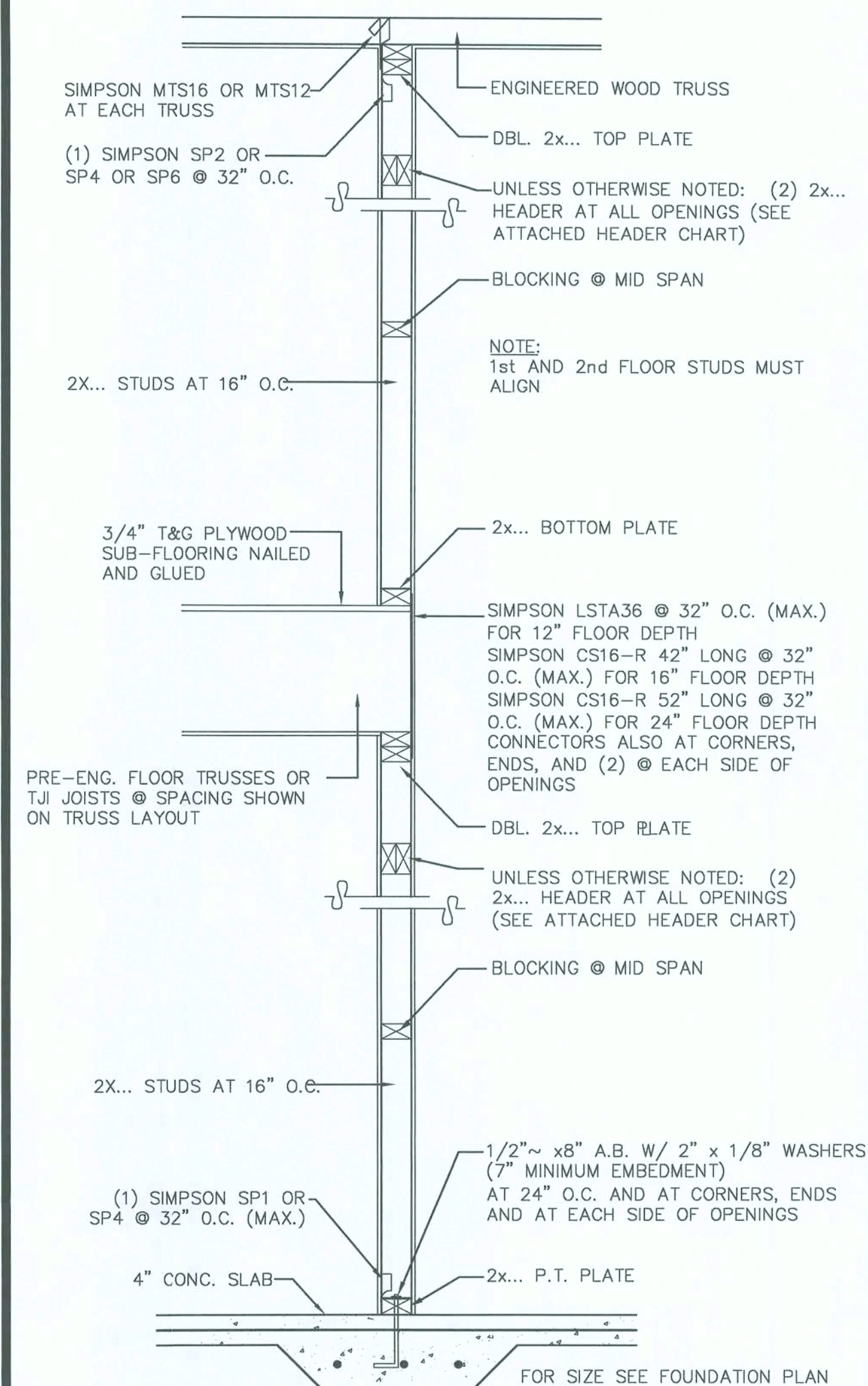
- 2x STUD CONT. TO TOP PLATE
- 16d SPIKES @ 12" TOP PLATE
- HEADER-CONT. GLUE & NAILED USING 1/2" PLYWD. FLITCH
- FOR NUMBER OF STUDS SEE ATTACHED DETAIL

HEADER SCHEDULE		
OPENING WIDTH	BEARING WALL OR SHEARWALL	NON-BEARING WALLS
0'-0" TO 3'-0"	2-2x6'S	2-2x4'S
3'-1" TO 5'-0"	2-2x10'S	2-2x6'S
5'-1" TO 7'-0"	2-2x12'S	2-2x8'S
7'-1" TO 9'-0"	2-2x12 W/ 1/2" PLYWD. FLITCH	2-2x12'S

1. USE HEADER SIZES ABOVE UNLESS OTHERWISE NOTED ON FRAMING PLAN
2. PRIMARY FRAMING (BEAMS, GIRDERS, ETC.) WERE SIZED USING 1800 'FB' EXTREME FIBER IN BENDING (SINGLE) 90 'FV' HORIZONTAL SHEAR 16E 'E' MODULES OF ELASTICITY
3. JOIST, RAFTERS, UNTELS, ETC. WERE SIZED USING: 1200 'FB' EXTREME FIBER IN BENDING (SINGLE) 90 'FV' HORIZONTAL SHEAR 1200 'FB' EXTREME FIBER IN BENDING (SINGLE) 16E 'E' MODULES OF ELASTICITY



F-26 TYPICAL WINDOW/DOOR ATTACHMENTS TO WOOD FRAME



(F-16) INTERIOR 2-STORY
BEARING WALL SECTION

REVISIONS	BY

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2004, INTERNATIONAL 2006 SUPPLEMENTAL FLORIDA BUILDING CODE FOR 120 MPH OR LESS WIND SPEED AND COMPLIES WITH SECTION R301.1. THE FOLLOWING WIND DESIGN CRITERIA IS GOVERNED BY ASCE 7-02.

BASIC WIND SPEED = 120 MPH
WIND IMPORTANCE FACTOR = 1.0
WIND EXPOSURE CATEGORY = B
INTERNAL PRESSURE COEFFICIENT = +/- 0.18 FULLY ENCLOSED
EXTERNAL PRESSURE COEFFICIENT = $-26 / Z^{0.4}$
WIND DIRECTIONAL PROBABILITY = 0.625 (Worst Case)
WIND PRESSURES FOR CLADDING AND CLADDING FOR WALL ELEMENTS SHALL BE +1/25.9 g & (-) 34.7 g PSF (Worst Case)

SILVESTRE ENGINEERING & DESIGN, INC.

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WINTER PARK, FL. 32789
ORLANDO: (407) 644-5859



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OF SHEET

OF SHEET

Simpson Strong-Tie Florida Statewide Product Approval Index 5-26-05

Product	Florida #	Product	Florida #
A21	FL474.1	CBQ46-SDS2	FL1218.4
A23	FL474.2	CBQ66-SDS2	FL1218.5
A34	FL474.3	CB544	FL474.43
A35	FL474.4	CB546	FL474.44
A35F	FL474.5	CB566	FL474.45
AB44	FL474.6	CB5Q44-SDS2	FL1218.6
AB44R	FL474.7	CB5Q46-SDS2	FL1218.7
AB46	FL474.8	CB5Q66-SDS2	FL1218.8
AB46R	FL474.9	CC106	FL1218.9
AB66	FL474.10	CC3 1/4-4	FL1218.10
AB66R	FL474.11	CC3 1/4-6	FL1218.11
ABA44	FL474.12	CC44	FL1218.12
ABA44R	FL474.13	CC46	FL1218.13
ABA46	FL474.14	CC5 1/4-4	FL1218.14
ABA46R	FL474.15	CC5 1/4-6	FL1218.15
ABA66	FL474.16	CC5 1/4-8	FL1218.16
ABA66R	FL474.17	CC64	FL1218.17
ABE44	FL474.18	CC66	FL1218.18
ABE46	FL474.19	CC68	FL1218.19
ABE66	FL474.20	CC76	FL1218.20
ABU44	FL474.21	CC77	FL1218.21
ABU46	FL474.22	CC78	FL1218.22
ABU66	FL1218.51	CC86	FL1218.23
ABU68	FL1218.52	CC88	FL1218.24
AC4	FL474.23	CC96	FL1218.25
AC6	FL474.24	CC98	FL1218.26
AC6A	FL474.25	CCQ3-4-SDS2.5	FL1218.27
AC6B	FL474.26	CCQ3-5-SDS2.5	FL1218.28
AT	FL2304.1	CCQ44-SDS2.5	FL1218.29
B Series	FL1218.2	CCQ46-SDS2.5	FL1218.30
BC4	FL474.27	CCQ48-SDS2.5	FL1218.31
BC46	FL474.28	CCQ5-4-SDS2.5	FL1218.32
BC4R	FL474.29	CCQ5-6-SDS2.5	FL1218.33
BC6	FL474.30	CCQ5-8-SDS2.5	FL1218.34
BC6R	FL474.31	CCQ64-SDS2.5	FL1218.35
CB46	FL474.32	CCQ66-SDS2.5	FL1218.36
CB52-2/4	FL503.1	CCQ68-SDS2.5	FL1218.37
CB52-3/6	FL503.2	CCQ74-SDS2.5	FL1218.38
CB1010	FL474.33	CCQ76-SDS2.5	FL1218.39
CB1012	FL474.34	CCQ77-SDS2.5	FL1218.40
CB1212	FL474.35	CCQ78-SDS2.5	FL1218.41
CB44	FL474.36	CJ13	FL1463.1
CB46	FL474.37	CJ14	FL1463.2
CB48	FL474.38	CJ15	FL1463.3
CB66	FL474.39	CJ16	FL1463.4

Simpson Strong-Tie Florida Statewide Product Approval Index 5-26-05

Product	Florida #	Product	Florida #
HU612	FL1218.164	IUS1.81/11.88	FL1463.28
HU612TF	FL1218.165	IUS1.81/14	FL1463.29
HU614	FL1218.166	IUS1.81/9.5	FL1463.30
HU614TF	FL1218.167	IUS2.37/11.88	FL1463.31
HU616	FL1218.168	IUS2.37/14	FL1463.32
HU616TF	FL1218.169	IUS2.37/16	FL1463.33
HU66	FL1218.170	IUS2.56/11.88	FL1463.34
HU66TF	FL1218.171	IUS2.56/14	FL1463.35
HU68	FL1218.172	IUS2.56/16	FL1463.36
HU68TF	FL1218.173	IUS2.56/9.5	FL1463.37
HU61.81/10	FL3750.75	IUS3.56/11.88	FL1463.38
HU6210	FL3750.76	IUS3.56/14	FL1463.39
HU6210-2	FL3750.77	IUS3.56/16	FL1463.40
HU6210-2TF	FL474.187	IUT	FL474.216
HU6212	FL474.188	J57	FL474.217
HU6212-2	FL3750.78	J813	FL474.218
HU6212-2TF	FL474.190	J8210	FL474.219
HU6214-2TF	FL474.191	J8212	FL474.220
HU626	FL3750.79	J8214	FL474.221
HU626-2	FL3750.80	J826	FL474.222
HU626-2TF	FL474.194	J828	FL474.223
HU628	FL3750.81	JP44	FL474.224
HU628-2	FL3750.82	L30	FL474.225
HU628-2TF	FL474.197	L50	FL474.226
HU6410TF	FL3750.83	L70	FL474.227
HU6412	FL474.199	L90	FL474.228
HU6412TF	FL3750.84	LB210	FL474.229
HU6412TF	FL474.201	LB212	FL474.230
HU6414TF	FL474.202	LB214	FL474.231
HU646	FL3750.85	LB216	FL474.232
HU646TF	FL474.204	LB26	FL474.233
HU648	FL3750.86	LB28	FL474.234
HU648TF	FL474.206	LBV Series	FL1218.177
HU6210-2	FL474.207	LCB44	FL474.235
HU6212-2	FL474.208	LCB46	FL474.236
HU628-2	FL474.209	LCB66	FL474.237
HU628-2	FL474.210	LEQ3	FL1218.178
HU6C410	FL474.211	LEQ5	FL1218.179
HU6C412	FL474.212	LEQ7	FL1218.180
HU6C46	FL474.213	LF7A	FL474.238
HU6C48	FL474.214	LG72	FL1423.13
HW Series	FL1218.175	LPQ4	FL474.239
HWJ Series	FL1218.176	LPQ6	FL474.240
ICFLC	FL1725.3	LS30	FL474.241
ICFLC-CW	FL1725.4	LS50	FL474.242

ENGINEERED LUMBER	
MANUFACTURER	FLORIDA NUMBER
GEORGIA-PACIFIC CORP.	FL2023-R1
GEORGIA-PACIFIC CORP.	FL2147-R1

LINTELS	
MANUFACTURER	FLORIDA NUMBER
QUALITY PRECAST	FL5092
QUALITY PRECAST	FL1774-R1
QUALITY PRECAST	FL1418-R1

Simpson Strong-Tie Florida Statewide Product Approval Index 5-26-05

Product	Florida #	Product	Florida #
C516	FL1901.4	FGTR	FL3750.1
C518	FL1901.5	FGTRE	FL3750.2
C520	FL1901.6	FGTRHL	FL3750.3
C522	FL1901.7	FGTRHR	FL3750.4
CWB106	FL474.49	FHA12	FL1901.8
CWB126	FL474.50	FHA18	FL1901.9
DBT1	FL474.51	FHA24	FL1901.10
DIA	FL402.1	FHA30	FL1901.11
DJT14	FL474.57	FHA6	FL1901.12
DPT5	FL474.58	FHA9	FL1901.13
DPT6	FL474.59	FJA	FL474.98
DPT7	FL474.60	FA	FL474.99
DRT8	FL474.61	FTA2	FL1218.45
DSP	FL1423.1	FTA5	FL1218.46
EG5	FL1218.42	FTA7	FL1218.47
EG7	FL1218.43	GA1	FL503.3
EG9	FL1218.44	GA2	FL503.4
EPB44	FL474.62	GB Series	FL1218.48
EPB44A	FL474.63	GBC	FL3751.1
EPB44B	FL474.64	GBA6-6	FL474.101
EPB46	FL474.65	GBA6-8	FL474.102
EPB66	FL474.66	GBA6-6	FL474.103
EPD44	FL474.67	GBA6-8	FL474.104
EPD44-16	FL474.68	GLB5A	FL1218.49
EPD46	FL474.69	GLB5B	FL1218.50
EPD46-16	FL474.70	GLB5C	FL1218.51
EPD48-16	FL474.71	GLB5D	FL1218.52
EPD48-16	FL474.72	GLB7A	FL1218.53
EPD64	FL474.73	GLB7B	FL1218.54
EPD64-16	FL474.74	GLB7C	FL1218.55
EPD66	FL474.75	GLB7D	FL1218.56
EPD66-16	FL474.76	GLB7E12	FL1218.57
EPD68	FL474.77	GLB7S16	FL1218.58
EPD68A	FL474.78	GLB7S18	FL1218.59
EPD68B	FL474.79	GLB7S12	FL1218.60
EPD88	FL474.80	GLB7S16	FL1218.61
ET-2	FL402.2	GLB7E20	FL1218.62
F24	FL474.81	GLS3-5	FL1218.63
F26-2	FL474.82	GLS3-7	FL1218.64
F26P	FL474.83	GLS3-9	FL1218.65
F34	FL474.84	GLS3-5	FL1218.66
F36	FL474.85	GLS5-7	FL1218.67
F44	FL474.86	GLS7-7	FL1218.68
F46	FL474.87	GLS7-9	FL1218.69
FA	FL474.88	GLS73	FL1218.70

Simpson Strong-Tie Florida Statewide Product Approval Index 5-26-05

Product	Florida #	Product	Florida #
LSUS28	FL474.247	LUS23-2	FL3750.95
LSUS410	FL474.248	LUS28	FL3750.107
LSUS425	FL474.249	LUS28-2	FL3750.97
LSUS435	FL474.250	LUS28-3	FL3750.98
LST12	FL1901.30	LUS310	FL3750.99
LSTA15	FL1901.31	LUS36	FL3750.100
LSTA18	FL1901.32	LUS38	FL3750.101
LSTA21	FL1901.33	LUS410	FL3750.102
LSTA24	FL1901.34	LUS414	FL3750.103
LSTA30	FL1901.35	LUS44	FL3750.104
LSTA36	FL1901.36	LUS46	FL3750.105
LSTA9	FL1901.37	LUS48	FL3750.106
LSTH08	FL503.22	LUSC210-2	FL474.293
LSTH08RJ	FL503.23	LUSC214-2	FL474.294
LSTH49	FL1901.38	LUSC24-2	FL474.295
LSTH73	FL1901.39	LUSC26-2	FL474.296
LSTH26	FL474.252	LUSC28-2	FL474.297
LTA1	FL1423.14	LUSC40	FL474.298
LTS0	FL474.253	LUSC414	FL474.299
LTB40	FL474.254	LUSC44	FL474.300
LTHA26	FL3751.2	LUSC46	FL474.301
LTHUL	FL474.255	LUSC48	FL474.302
LTHUR	FL474.256	MA4	FL474.303
LTHMA	FL474.257	MA6	FL474.304
LTH4	FL474.258	MA615	FL474.305
LTS12	FL474.259	MA623	FL474.306
LTS16	FL474.260	MA6	FL474.307
LTS18	FL474.261	MA6B	FL474.308
LTS20	FL474.262	MBHA3.12/11.25	FL538.16
LTT19	FL474.263	MBHA3.12/9.25	FL538.17
LTT20B	FL474.264	MBHA3.56/11.25	FL538.18
LTT31	FL474.265	MBHA3.56/11.88	FL538.19
LUT10	FL474.266	MBHA3.56/14	FL538.20
LU24	FL474.267	MBHA3.56/16	FL538.21
LU26	FL474.268	MBHA3.56/18	FL538.22
LU28	FL474.269	MBHA3.56/9.25	FL538.23
LUP210	FL474.270	MBHA3.56/11.25	FL538.24
LUP24	FL474.271	MBHA3.56/11.88	FL538.25
LUP26	FL474.272	MBHA3.56/14	FL538.26
LUP28	FL474.273	MBHA3.56/16	FL538.27
LUS210	FL3750.87	MBHA3.56/18	FL538.28
LUS210-2	FL3750.88	MBHA3.56/9.25	FL538.29
LUS210-3	FL3750.89	MEG5	FL1218.181
LUS212	FL474.277	MEG7	FL1218.182
LUS214-2	FL3750.90	META12	FL1901.40

Simpson Strong-Tie Florida Statewide Product Approval Index 5-26-05

Product	Florida #	Product	Florida #
GLTV5.5	FL474.105	HD6A	FL503.12
GT2	FL474.106	HD8A	FL503.13
GTF	FL474.107	HDC10/22-SDS2.5	FL1463.21
H1	FL474.108	HDC10/4-SDS2.5	FL1463.22
H10	FL474.109	HDC5/22-SDS2.5	FL1463.23
H10-2	FL474.110	HDC5/4-SDS2.5	FL1463.24
H10R	FL474.111	HDC8-SDS3	FL1901.14
H14	FL1423.2	HETA12	FL1901.15
H15	FL474.112	HETA16	FL1901.16
H15-2	FL474.113	HETA20	FL1901.17
H16	FL1423.3	HETA24	FL1901.18
H16-2	FL1423.4	HETA40	FL1901.19
H16-2S	FL1423.5	HETA12	FL1901.20
H16S	FL1423.6	HETA16	FL1901.21
H2	FL474.114	HETAL20	FL1218.75
H2.5	FL474.115	HF24N	FL1218.76
H2.5A	FL503.5	HF26N	FL1218.77
H3	FL474.116	HF46	FL474.130
H4	FL474.117	HF48	FL474.131
H5	FL474.118	HGA10	FL1423.8
H6	FL474.119	HGA10M1	FL1423.9
H7	FL474.120	HGB Series	FL1218.77
H8	FL1423.7	HGLBA	FL1218.78
HB Series	FL1218.74	HGLBB	FL1218.79
HCA11-5	FL1463.5	HGLBC	FL1218.80
HCA11-6	FL1463.6	HGLBD	FL1218.81
HCA11-7	FL1463.7	HGLBS	FL1218.82
HCA11-9	FL1463.8	HGLS7	FL1218.83
HCA5-5	FL1463.9	HGLS9	FL1218.84
HCA5-6	FL1463.10	HGLS15	FL1218.85
HCA5-7	FL1463.11	HGLS17	FL1218.86
HCA5-9	FL1463.12	HGLS19	FL1218.87
HCA7-5	FL1463.13	HGLT Series	FL1218.88
HCA7-6	FL1463.14	HGLTV3.5	FL474.133
HCA7-7	FL1463.15	HGLTV5.5	FL474.134
HCA7-9	FL1463.16	HGT-2	FL474.135
HCA9-5	FL1463.17	HGT-3	FL474.136
HCA9-6	FL1463.18	HGT-4	FL474.137
HCA9-7	FL1463.19	HGU10-2	FL3750.5
HCA9-8	FL1463.20	HGU1010-3	FL3750.6
HCP1.81	FL474.121	HGU10210-3	FL3750.7
HCP2	FL474.122	HGU1026-2	FL3750.8
HCP4	FL474.123	HGU26-3	FL3750.9
HD10A	FL503.6	HGU26-4	FL3750.10
HD14A	FL503.7	HGU28-2	FL3750.11

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META22	FL1901.45	MUS26	FL3750.106
META24	FL1901.46	MUS28	FL3750.107
META40	FL1901.47	NCA	FL474.330
MGT	FL1423.15	PA18	FL474.331
MIT	FL474.306	PA23	FL474.332
MPAI32	FL474.309	PA28	FL474.333
MPAI44	FL1901.32	PA35	FL474.334
MSC1.81	FL474.311	PA51	FL474.335
MSC2	FL474.312	PA68	FL474.336
MSC4	FL474.313	PAH42	FL1218.183
MST27	FL1901.48	PA118	FL474.337
MST37	FL1901.49	PA123	FL474.338
MST48	FL1901.50	PA128	FL474.339
MST60	FL1901.51	PA165	FL474.340
MST72	FL1901.52	PATM25	FL1974.341
MSTA15	FL1901.53	PB44	FL474.342
MSTA16	FL1901.54	PB44R	FL474.343
MSTA18	FL1901.55	PB46	FL474.344
MSTA21	FL1901.56	PB46R	FL474.345
MSTA24	FL1901.57	PB66	FL474.346
MSTA26	FL1901.58	PB66R	FL474.347
MSTA36	FL1901.59	PSB44A	FL474.348
MSTA9	FL1901.60	PSB46	FL474.349
MSTAM24	FL1901.61	PSB66	FL474.350
MSTAM36	FL1901.62	PC44	FL474.351
MSTC28	FL1901.63	PC44-16	FL474.352
MSTC40	FL1901.64	PC46	FL474.353
MSTC48B3	FL1901.65	PC46-16	FL474.354
MSTC4953	FL1901.66	PC48	FL474.355
MSTC66	FL1901.67	PC48-16	FL474.356
MSTC66B3	FL1901.68	PC64	FL474.357
MSTC78	FL1901.69	PC66-16	FL474.358
MSTCM40	FL1901.70	PC64-16	FL474.359
MSTC98	FL1901.71	PC66-16	FL474.360
MST126	FL1901.72	PC68	FL474.361
MST148	FL1901.73	PC84	FL474.362
MST160	FL1901.74	PC86	FL474.363
MST172	FL1901.75	PC88	FL474.364
MTS12	FL474.326	PC118	FL1463.41
MTS16	FL474.328	PC123	FL1463.42
MTS18	FL474.327	PC127	FL1463.43
MTS20	FL474.328	PC138	FL1463.44
MTS24C	FL1423.16	PF24	FL474.365
MTS28C	FL1423.17	PF24A	FL474.366
MTS30	FL474.329	PF26	FL474.367