

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d		0.0 7.0	
< 3335	< 3335	HPAHD22	16-16d			
< 2200	< 2200	ABU44	12-16d		1/2" AP	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
	2000	7.6500	12-10u		1/2" AB	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODI						
(ENCLOSED SIMPLE DIAPHRAGM BUILDING MEAN ROOF HEIGHT NOT EXCEEDING LEAS ON UPPER HALF OF HILL OR ESCARPMENT SLOPE AND UNOBSTRUCTED UPWIND FOR	ST HORIZOI 60FT IN EX	NTAL [(P. B. 3	DIMEI BOFT	NSION IN FX	OR 60	FT; NOT
BUILDING IS NOT IN THE HIGH VELOCITY HI	JRRICANE :	ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEB	RIS REGIO	N				
1.) BASIC WIND SPEED = 110 MPH						
2.) WIND EXPOSURE = B						
3.) WIND IMPORTANCE FACTOR = 1.0						
4.) BUILDING CATEGORY = II						
5.) ROOF ANGLE = 10-45 DEGREES						
6.) MEAN ROOF HEIGHT = <30 FT						
7.) INTERNAL PRESSURE COEFFICIENT =	N/A (ENCLO	DSED I	BUILE	DING)		
8.) COMPONENTS AND CLADDING DESIGN				*		2(2)1
2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		19.9 19.9 21.8 21.8 Wind	-42.1 -42.1 -23.6 -29.1	18.1 18.5	-18.1 -29.1 -29.1 -20.4 -22.6	
5 2	(Zone	e 5, 10 f	t2)			
4 /3/ 4 5	8x7 Ga			19.5	-21.3	
55 22 22	lox/ G	arage D	JOOF	18.5	-20.4	
DESIGN LOADS						
	201401					
FLOOR 40 PSF (ALL OTHER DWELLING RO	JUMS)					
FLOOR 40 PSF (ALL OTHER DWELLING RO 30 PSF (SLEEPING ROOMS)	JOMS)					

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

40 PSF (DECKS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

60 PSF (EXTERIOR BALCONIES

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

/INDLOAD ENGINER: Mark Disosway, PE No.53915, POB368, Lake City, FL 32056, 386-754-549 Stated dimensions upercede scaled limensions. Refer al questions to Mark Disosway, P.E for resolution. Do not proceed without clarification.

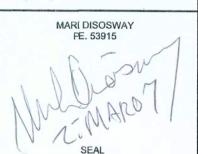
REVISIONS

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amined this plan, and that the applicable ortions of the plan relating to wind engine comply with sectionR301.2.1, florida building code residential 204, to the best of my

IMITATION: This esign is valid for one building, at specifie location.



RICHARD KEEN

Spec House 960 CR 100A

APDRESS: 96CCR 100A ColumbiaCounty, Florida

Mark Dsosway P.E. P.O Box 868 Lake City Florida 32056 Phone: (336) 754 - 5419 Fax: (383) 269 - 4871

PRINTED DATE: March27, 2007 DRAWN BY: STRUCTURAL B' Ben Sparks Ben Sparks

FINALS DATE:

27 / Mar / 07

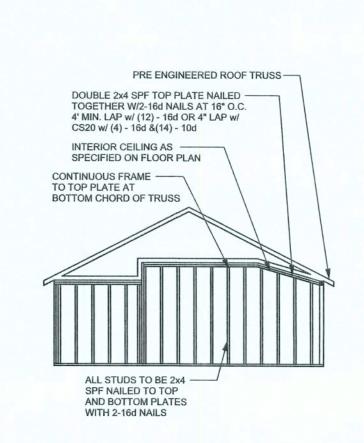
JOB NUMBER: 7)3262

DRAWNG NUMBER

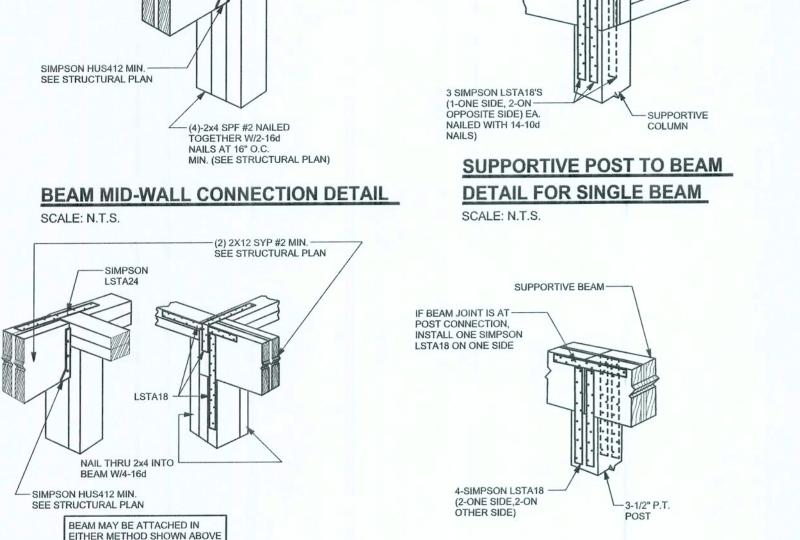
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GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL**



EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x6 @ 16" OC TO 18'-10' STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

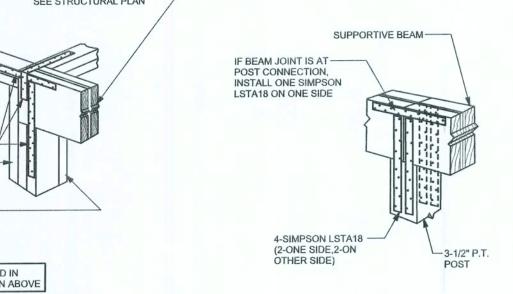
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

(1) 2x4 @ 12" OC

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

BEAM CORNER CONNECTION. DETAIL

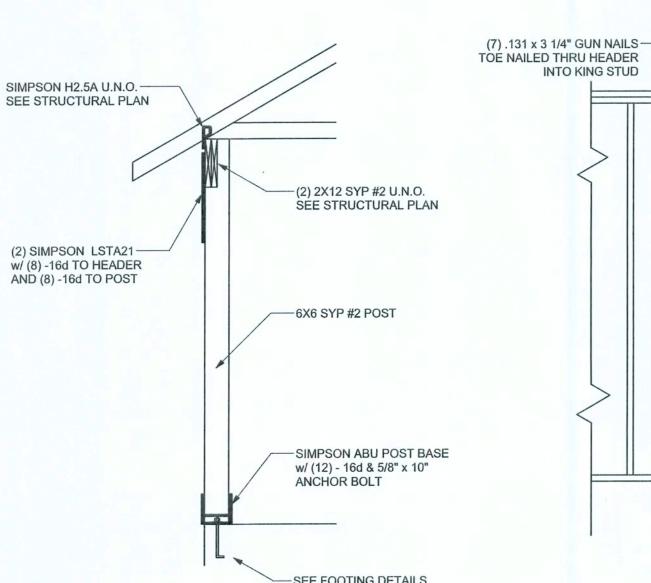
(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN TO 13'-0" STUD HEIGHT



SUPPORTIVE-



2X4 LADDER BEAM



-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL SCALE: 1/2" = 1'-0"

IF TRUSS TO WALL STRAPS ARE NAILED

TO THE HEADER THE SPH4/6 @ 48" O.C.

INTO KING STUD

ARE NOT REQUIRED

TYPICAL 1 STODRY HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

FOR LESS THAN 1500 Ib UPLIFT USE

FOR LESS THAN 3750 Ib UPLIFT USE

-NAIL SHEATHING TO HEADER AND TOP PLATE WITH 8d AT 3" O.C. FOR UPLIFT

-SP4/6 @ 48" O.C. (U.N.O.) /---(7) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU HEADER

INTO KING STUD

2 X 2 X 1/8" WASHER

3 X 3 X 1/8" WASHER

CRIPPLES IF REQUIRED

(5)5) .131 x 3 1/4" GUN NAILS

INTO JACK STUD U.N.O.

-T.TOE NAILED THRU SILL-

TY YPICAL STRAPPING (U.N.O.)

(1) 2X6.6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4:4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4:4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120:0 MPH, 10'-0" WALL HEIGHT U.N.O.)

(SISEE STRUCTURAL PLAN)

- HURRICANEJE CLIP H-2.5 OR EQUAL

TOP CHORRD OF GABLE END TRUSS

- CONT. 2X4 SCAB FROM TOP TO

BOTTOM CHORD @ X-BRACING

(PROVIDE A ADDITIONAL 2X4'S @

VERTICAL IF IF HIGHER THAN 48",

- TOE NAIL TITRUSS TO DOUBLE PLATE w/ 1616d COM @8" OC.

- BOTTOM CHORD OF GABLE

- SIMPSON L'LSTA 24 @ 48" OC.

TO FORM AAN "L" SHAPE.)

- 2X4 BARGE E RAFTER CONT.

48" OC.

- FASCIA

- SHINGLE STRIP

- DROP 3 1/2'2"

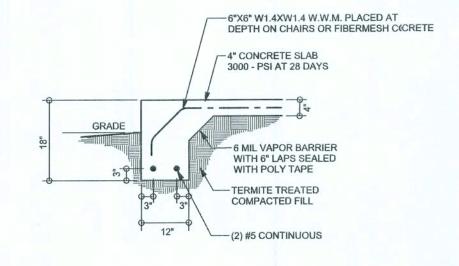
END TRUSSS

- 2 - 2X4 TOP_{P PLATE}

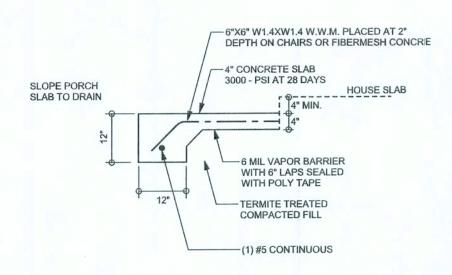
- 2X4 STUDS S @16" OC.

REVISIONS

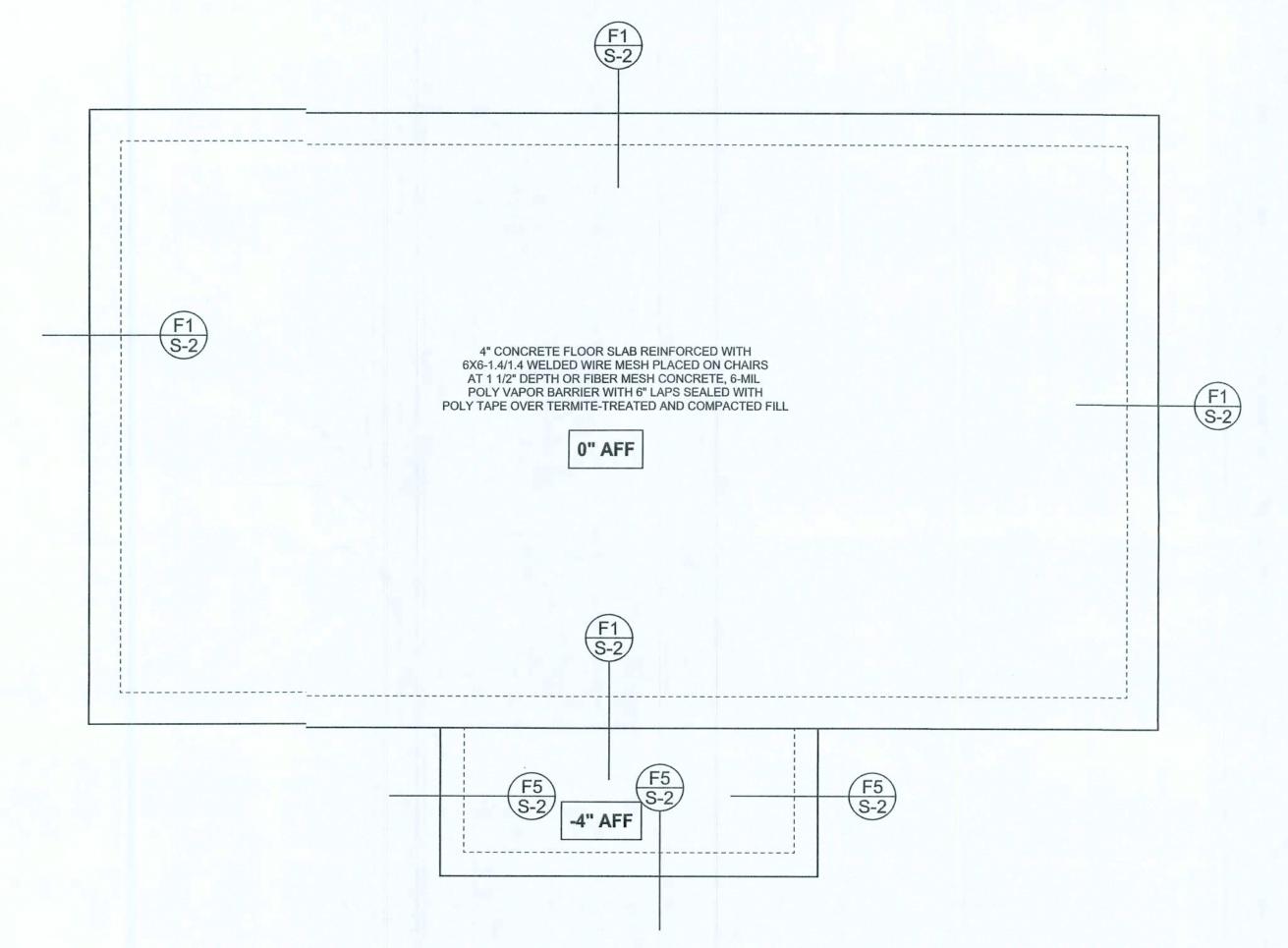
SOFIPIAN







F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"



FOUNDATION PLAN SCALE: 1/4" = 1'-0" DIMENSIONS ON STRUCTURAL SHEETS ;
ARE NOT EXACT. REFER TO ARCHITECT TURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS S

WINDLOAD ENGIN:ER: Mark Disosway, PE No.53915, POB :68, Lake City, FL 32056, 386-754-541) Stated dimensions spercede scaled dimensions. Refer a questions to Mark Disosway, P.Efor resolution. Do not proceed withut clarification. COPYRIGHTS ANDPROPERTY RIGHTS:
Mark Disosway, P.E. hereby expressly reserves its common law coprights and property right in these instruments observice. This document is not to be reproduce; altered or copied in any form or manner without first the express written permission and conent of Mark Disosway. CERTIFICATION: I lereby certify that I have examined this plan, nd that the applicable portions of the plan, elating to wind engineering comply with section 301.2.1, florida building code residential 200, to the best of my LIMITATION: This design is valid for one building, at specifiedlocation. MARKDISOSWAY PE. 53915

RICHARD KEEN

Spet House 960 CR 100A

AIDRESS: 960CR 100A ColumbiaCounty, Florida

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PRINED DATE: March 27, 2007

DRAWN BY: STRUCTURAL BY: Ben Sparks Ben Sparks

FINALS DATE: 27 / Mar / 07

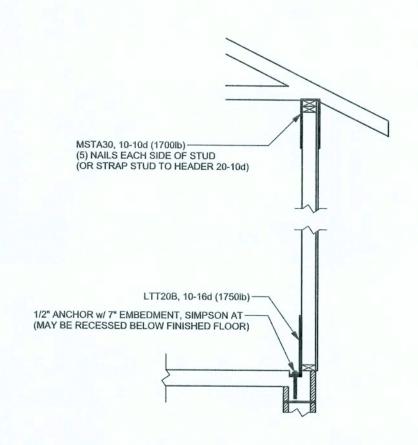
> JOB NUMBER: 703262 DRAWING NUMBER

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OF (SHEETS

REVISIONS

SOFTPILAN



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEIR: Mark Disosway, PE No.53915, POB 86, Lake City, FL 32056, 386-754-5419

Stated dimensions sujercede scaled dimensions. Refer all juestions to Mark Disosway, P.E. or resolution. Do not proceed without clarification.

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permission and consett of Mark Disosway.

CERTIFICATION: I heeby certify that I have examined this plan, ad that the applicable portions of the plan, rlating to wind engineerin comply with section R01.2.1, florida building code residential 2004 to the best of my knowledge.

LIMITATION: This deign is valid for one building, at specified lication.

MARK IISOSWAY P.E 53915

RICHARD KEEN

SpecHouse 960 CR 100A

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703262

S-3

OF 3SHEETS

PRINTED DATE: March 2, 2007 DRAWN BY: STRUCTURAL BY: Ben Sparks Ben Sparks

FINALS DATE: 27 / Mar / 07

JOB NJMBER: DRAWING NUMBER

SWS = 6.5' 1014 LBS UPLIFT -539 LBS UPLIFT -593 LBS UPLIFT -ABU (TYP.) WALL LEGEND

1014 LBS UPLIFT-

SWS # 14.0'

USE H2.5A FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM

CONNECTIONS UNLESS NOTED OTHERWISE

STRUCTURAL PLAN NOTES

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

1014 LBS UPLIFT

539 LBS UPLIFT-

593 LBS UPLIFT-

1014 LBS UPLIFT -

CJ3

CJ5

CJ5

CJ3

ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. SN-4 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

SWS = 0.0'	1ST FLOOR _R EXTERIOR WALL
SWS = 0.0'	2ND FLOOR _{R EXTERIOR}
IBW	1ST FLOOR _R INTERIOR BEARING WALLS SEE DETAILILS ON SHEET S-1
IBW	2ND FLOOF)R INTERIOR BEARING WALLS SEE DETAILILS ON SHEET S-1

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

- INDICATES LOCATION OF:

2ND FLOOR 1/2" A307 ALL THREADED ROD

TOTAL SHEAR WALL SEGMENTS

HEADER LEGEND

= 0.0' INDICATES	S SHEAR WA	LL SEGN
	REQUIRED	ACTUAL
TRANSVERSE	35.0'	46.5'
LONGITUDINAL	15.0'	61.0'

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.)

-----SPAN OF HEADER

NUMBER OF KING STUDS (FULL LENGTH) NUMBER OF JACK STUDS (UNDER HEADER)

SIZE OF HEADER MATERIAL

----NUMBER OF PLIES IN HEADER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE (JOB #L161233)