

D.D.S. STUDIOS
P.O. Box 213
Lake City, FL 32056
(386) 754-0181

THE MARCUS
Lot 22, Huntington
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OUR PLANS ARE DRAFTED FOR AVERAGE SITE CONDITIONS AND COMPLIANCE WITH APPLICABLE CODES IN LAKE CITY, FL. AT THE TIME THEY ARE DRAFTED. DUE TO VARYING STATE, LOCAL, AND NATIONAL CODES, RULES AND REGULATIONS, DDS STUDIOS CANNOT WARRANT COMPLIANCE WITH ALL APPLICABLE CODES. THE LOCAL AND NATIONAL CODES IN YOUR AREA OR CITY TO BE BUILT MAY VARY FROM THE CODES SHOWN. THE RESPONSIBILITY OF THE ARCHITECT AND/OR ENGINEER TO VERIFY THAT THE STRIKE IS BUILT IN STRICT ACCORDANCE WITH THE LATEST CODES, CITY, COUNTY, STATE AND FEDERAL IF YOUR CITY OR STATE REQUIRES AN ENGINEER'S STAMP, YOU WILL NEED TO HAVE THIS DONE LOCALLY BY A QUALIFIED ARCHITECT OR ENGINEER.

EXTERIOR ELEVATIONS
TYPICAL WALL SECTION

SHEET NUMBER
1 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.



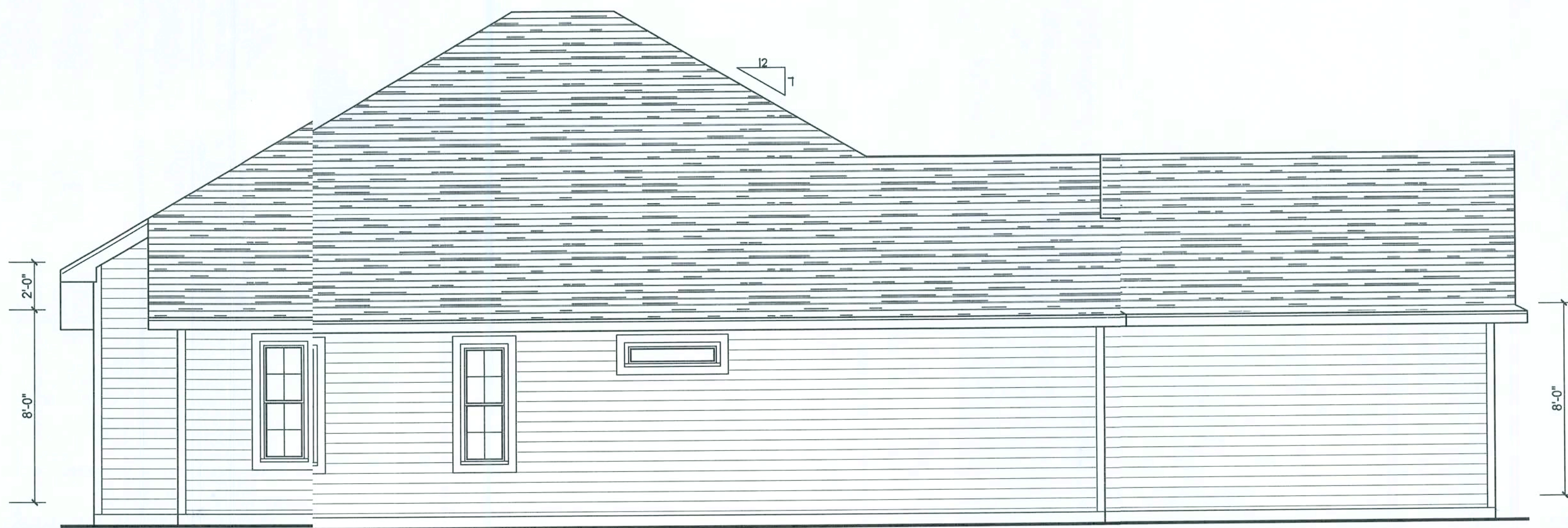
REAR ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

- SEE PLANS FOR WALL HEIGHTS
- GAF-TIMBERLINE SHINGLES W/ 4-NAILS IN EACH SHINGLE STRIP ON 30-LB FELT PAPER OVER 7/16" ORIENTED STRAND BOARD ROOF SHEATHING W/ 1/31 8d COMMON @ 4'8" O.C.
 - FLASHING: 26 ga. GALVANIZED STEEL
 - PRE-ENGINEERED WOOD ROOF TRUSSES AT 24" O.C. (SELECT TRUSS CONNECTORS PER WINDLOAD ANALYSIS)
 - BLOWN-IN INSULATION EQUAL TO R-30
 - (2) 2X4 SYP DOUBLE TOP PLATE
NOTE: SEAL ALL PENETRATIONS IN TOP PLATE AND FIRE STOP BLOCKING WITH CODE APPROVED SEALANT
 - 2X6 SYP #2 FASCIA
 - ALUMINUM DRIP EDGE MOLDING, AND VENTED SOFFIT
 - INTERIOR FINISH - 1/2" GYP&UM WALLBOARD
 - 2X4 #2 SPF PRECUT STUDS AT 16" O.C. WITH FULL-THICK FIBERGLASS INSULATION EQUAL TO R-11
 - EXTERIOR FINISH TO BE HARDI-PLANK LAP SIDING
 - 7/16" O.S.B. WALL SHEATHING (BLOCK ALL EDGES) W/ 1/31 8d COMMON @ 3'8" O.C.
 - FLOORING AND INTERIOR TRIM PER SPECIFICATIONS
 - 4" CONCRETE FLOOR SLAB REINFORCED WITH 6X6-1.4/1.4 WELDED WIRE MESH EMBEDDED 2" IN SLAB OR FIBER MESH ON 6 MIL POLY VAPOR BARRIER (6" LAPS SEALED WITH POLY TAPE) OVER COMPACTED FILL TREATED WITH TERMITICIDE
 - 2 x 4 P.T. PINE SOLE PLATE ANCHORED WITH WITH ANCHOR BOLTS AS PER WINDLOAD ANALYSIS
 - 1-#5, CONTINUOUS, IN CONCRETE BOND BEAM AT SLAB EDGE INTERSECTION WITH STEM WALL
 - APPROXIMATE FINISH GRADE

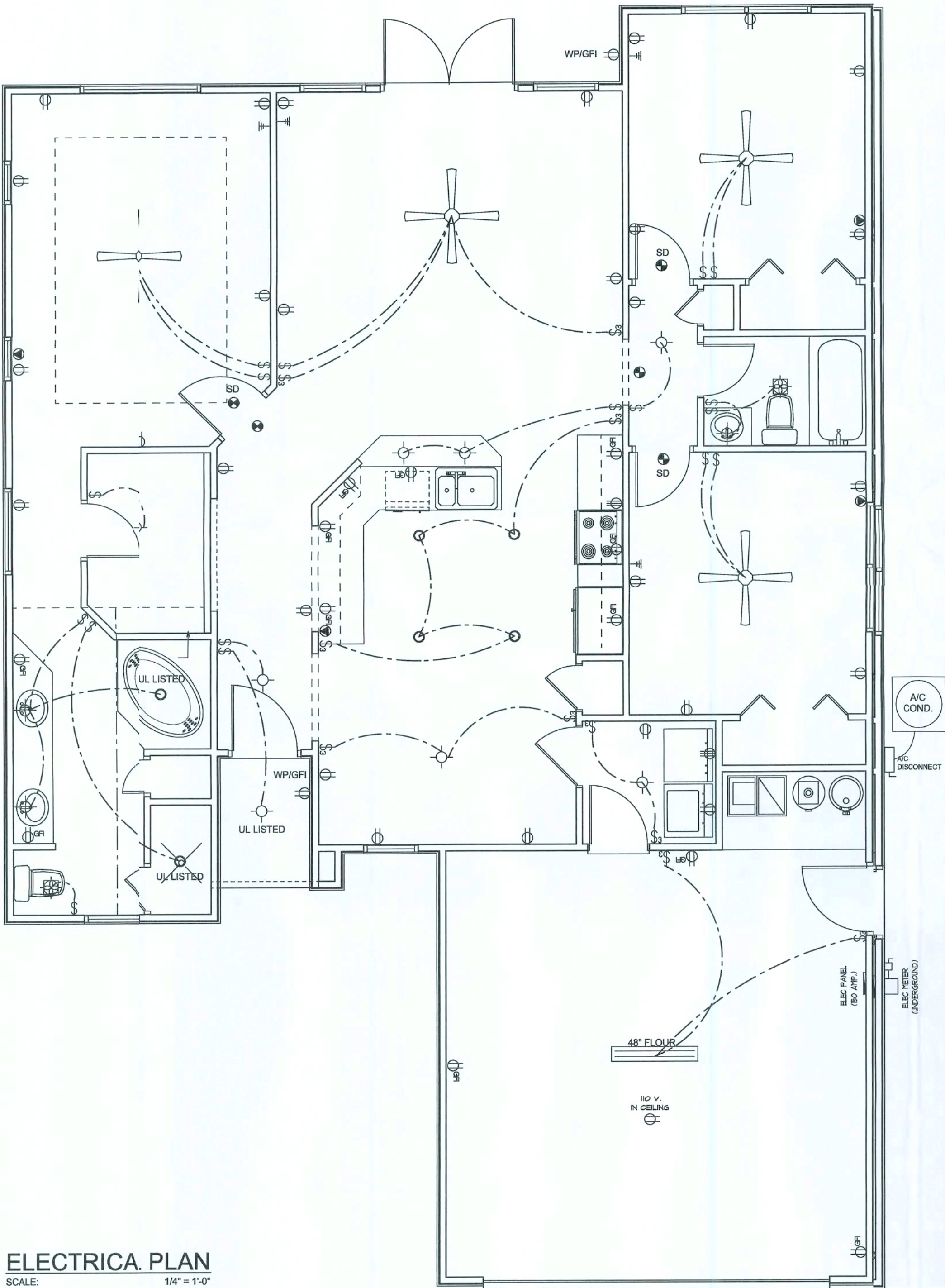
TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

ELECTRICAL LEGEND

- CEILING FAN
- EXHAUST FAN
- INCANDESCENT LIGHT FIXTURE
- EXHAUST FAN/LIGHT COMBO
- CAN LIGHT
- EYEBALL CAN LIGHT
- WALL SCONCE
- DOUBLE SPOTLIGHT
- TELEPHONE OUTLET
- TELEVISION OUTLET
- LOW WATTAGE LIGHTING
- SMOKE DETECTOR
- SWITCH
- 3-WAY SWITCH
- DIMMER SWITCH
- RECEPTACLE
- RECEPTACLE W/ GROUND FAULT INTERRUPT
- SPLIT RECEPTACLE (1 SWITCHED, 1 CONSTANT)
- 220 VOLT RECEPTACLE

ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"



ELECTRICAL PLAN NOTES

- E-1 ALL WORK SHALL COMPLY WITH THE NATIONAL ELECTRICAL CODE, LATEST EDITION, AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES.
- E-2 OPT'L SECURITY LIGHT SWITCH LOCATIONS TO BE DETERMINED DURING CONSTRUCTION
- E-3 NOTE: ALL SMOKE DETECTORS TO BE WIRED TOGETHER TO ACTIVATE ALL ALARMS IF ANY ONE UNIT IS ACTUATED
- E-4 PROVIDE WIRING AS REQUIRED FOR AIR CONDITIONING, HEATING AND WATER HEATING EQUIPMENT.

NOTE: PROVIDE OUTLETS PER CODE REQUIREMENTS

NOTE: PROVIDE SMOKE DETECTORS PER CODE REQUIREMENTS

NOTE: THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGGESTED SWITCH, RECEPTACLE, AND LIGHT FIXTURE LOCATIONS. DUE TO VARYING LOCAL AND STATE CODES, REGULATIONS, AND STATUTES, IT IS THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND STATE CODES, REGULATIONS AND STATUTES.

WIND LOAD DESIGN DATA

PURSUANT TO SECTION 904.1.1 OF THE FLORIDA BUILDING CODE 2001, THE FOLLOWING DATA RELATING TO WIND LOADS WAS USED IN PREPARATION OF THIS PLAN:

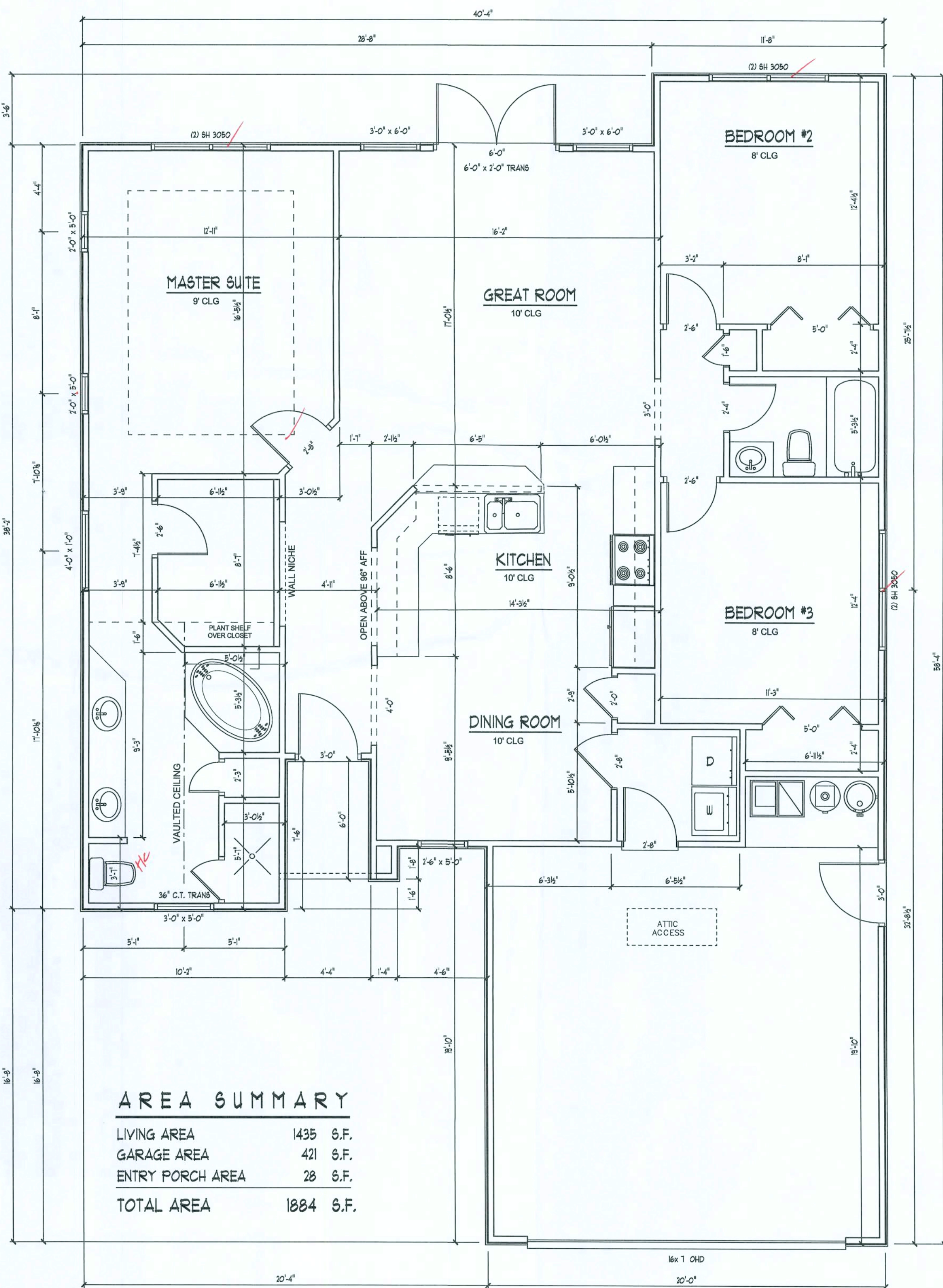
- 1) BASIC WIND SPEED = 110 MPH
- 2) WIND IMPORTANCE FACTOR = 1
BUILDING CATEGORY = II
- 3) WIND EXPOSURE = B
- 4) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 5) DESIGN WIND PRESSURE (DOORS & WINDOWS) = 21.8 / 23.1 PSF
GARAGE DOOR 8'X1 AND LARGER = 19.3 / 24.1 PSF

DESIGN LOADS

ROOF LIVE LOAD: 20psf
FLOOR LIVE LOAD: 40psf
FLOOR LIVE LOAD: BEDROOMS 30 psf

SHEARWALLS NAILED AT 4" oc. ON EDGES AND 8" oc. IN INTERMEDIATE SUPPORTS WITH 8d COMMON NAILS.

NOTE: ALL LOAD BEARING HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYPMS (L.O.C.)

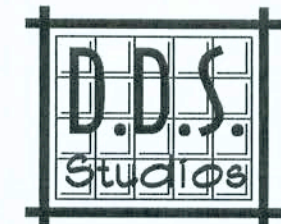


AREA SUMMARY

LIVING AREA	1435	S.F.
GARAGE AREA	421	S.F.
ENTRY PORCH AREA	28	S.F.
TOTAL AREA	1884	S.F.

FRONT ELEVATION

SCALE: 1/4" = 1'



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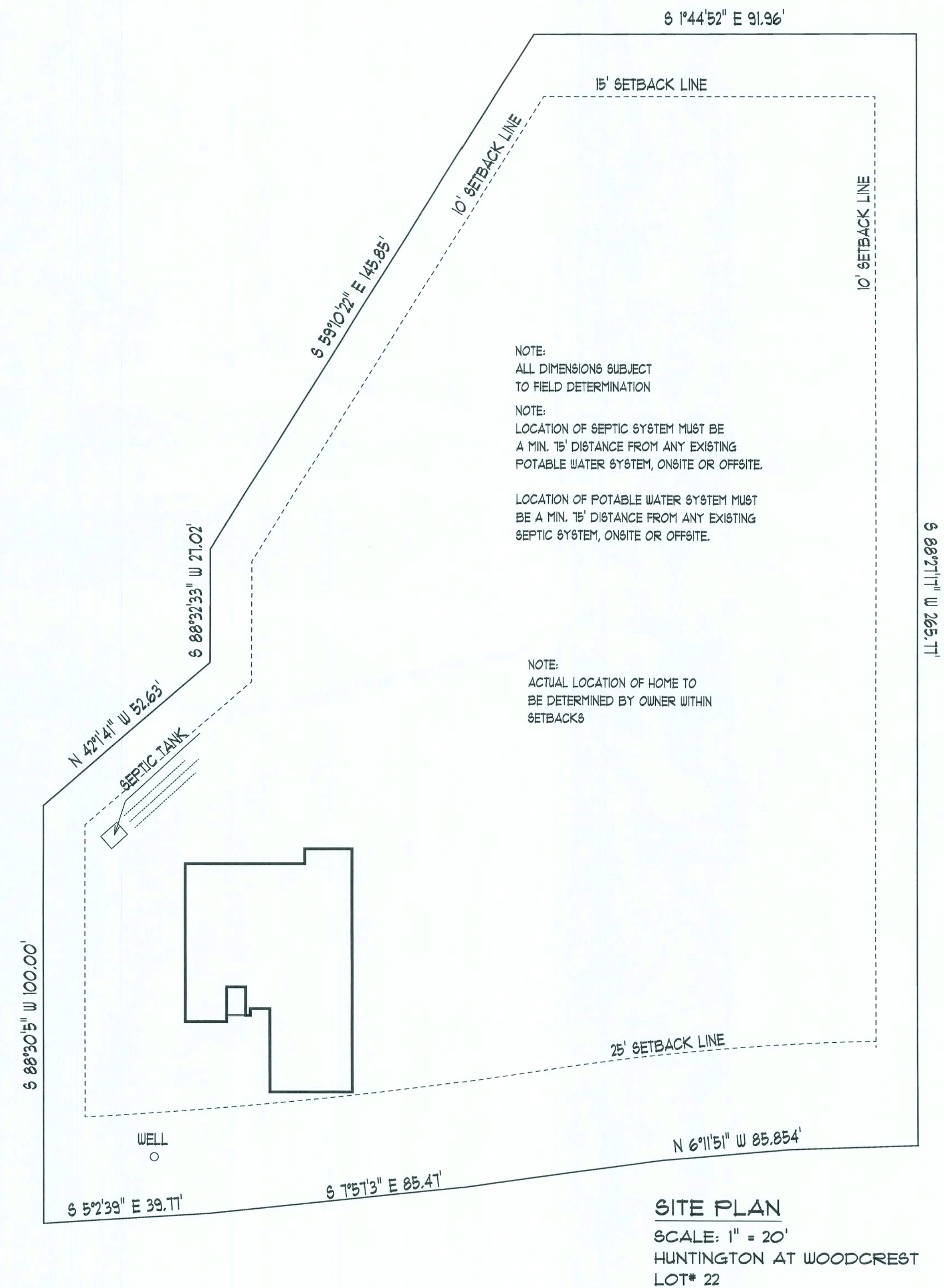
FOUNDATION PLAN
FLOOR PLAN

SHEET NUMBER
2 of 3

All work shall comply with the standard building codes and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.



- ROOF PLAN
SCALE: 1/8" = 1'



REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDOR TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG7		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*			TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT21	16-16d, 1 1/2"		1/2" AB
< 2775	< 3670	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3685	HTT16	18-16d		5/8" AB
< 1460	< 1460	PAH22	16-16d		
< 3335	< 3335	HPAH22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU68	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 4X18 EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W4 & W14, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER REINFORCED SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13d, 16d) PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURER'S AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE THE REQUIRED CAPACITIES.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS CAN BE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

GRADE & SPECIES TABLE

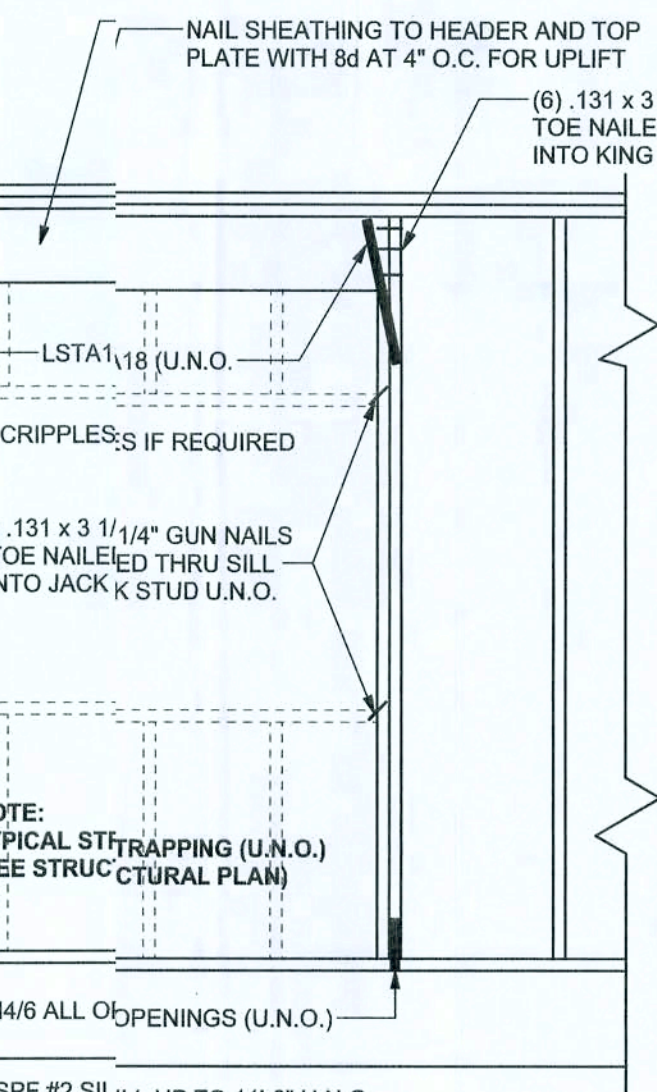
		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1800	1.9
PSL	PARALAM	2900	2.0

DOUBLE 2x4 SPF TOP PLATE NAILED TOGETHER W/ 2-16d NAILS AT 16" O.C. 4" MIN. LAP W/ (1) 12d - 16d OR 4" LAG W/ CS20 W/ (4) - 16d @ 8" (14) - 10d

INTERIOR CEILING JOISTS AS SPECIFIED ON FLOOR PLAN CONTINUOUS FRAME TO TOP PLATE AT BOTTOM CHORD OF F TRUSS

CONTINUOUS FRAME TO CEILING; DIAPHRAGM DETAIL

SCALE: N.T.S.

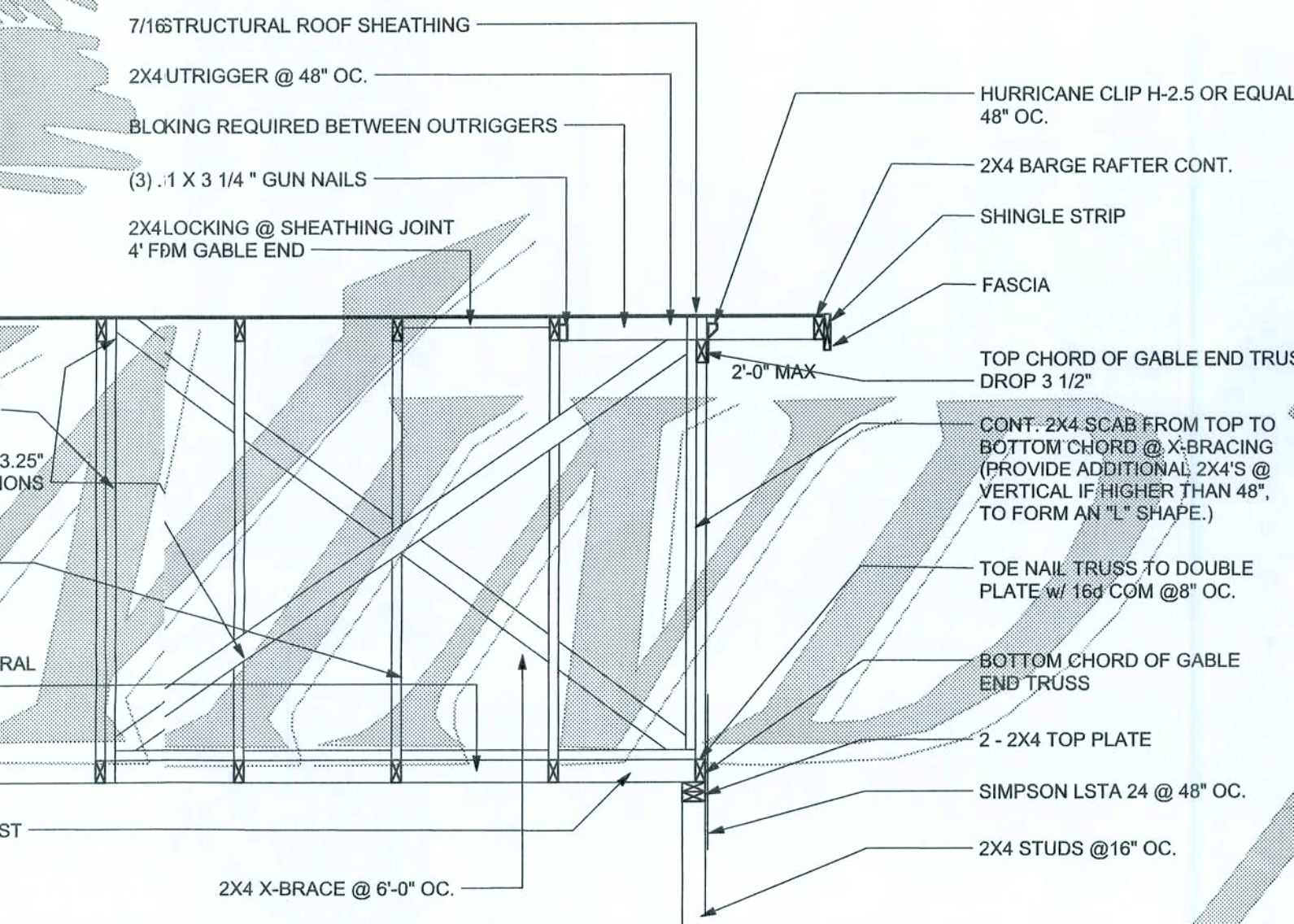


TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

INTERIOR HEAR WALL DETAIL

SCALE: 1/2" = 1'-0"



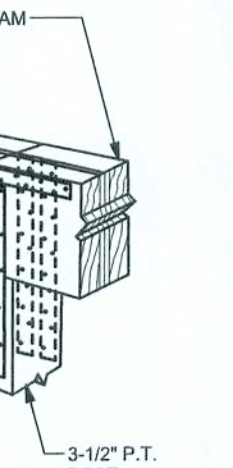
TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.



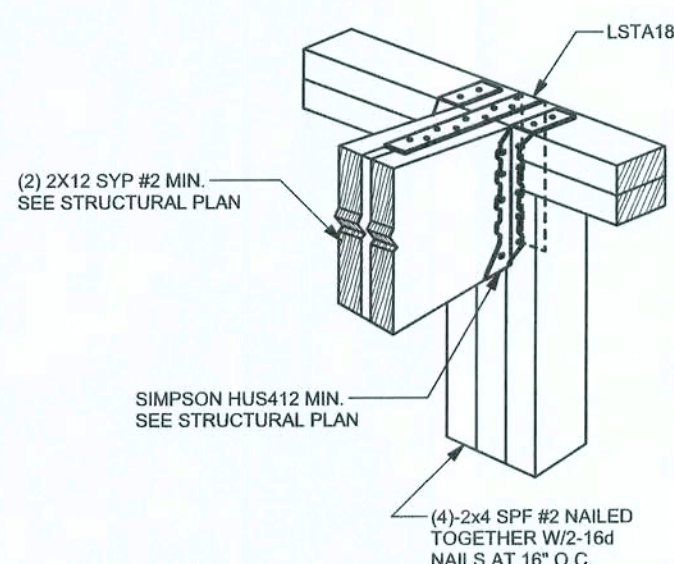
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

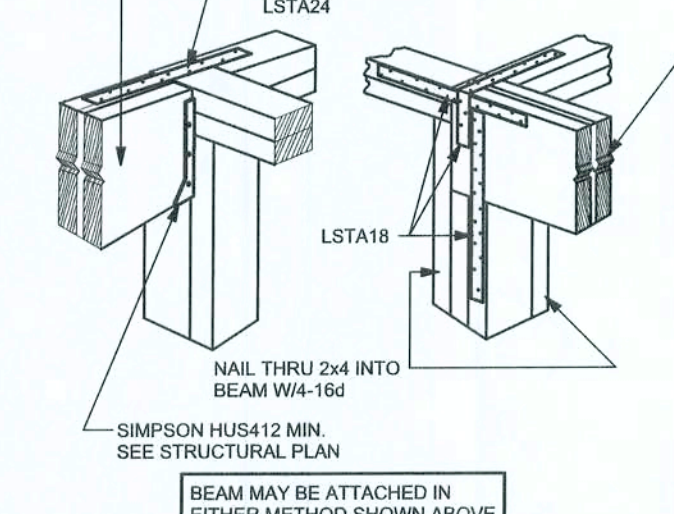
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.



BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.



WINDLOAD ENGINEER: Mark Discoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified locale.

MARK DISCOWAY
P.E. 53915
SEAL

Compass Builders

Spec House
Lot 22 Huntington S/D

ADDRESS:
Lot 22 Huntington S/D
Columbia County, Florida

Mark Discoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
May 08, 2007

DRAWN BY:
David Discoway

CHECKED BY:

FINALS DATE:
08 / May / 07

JOB NUMBER:
705064

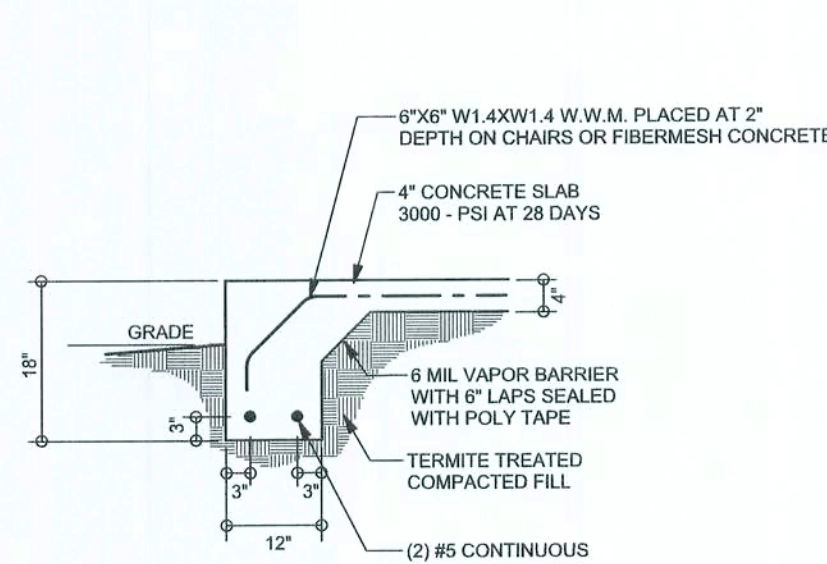
DRAWING NUMBER

S-1

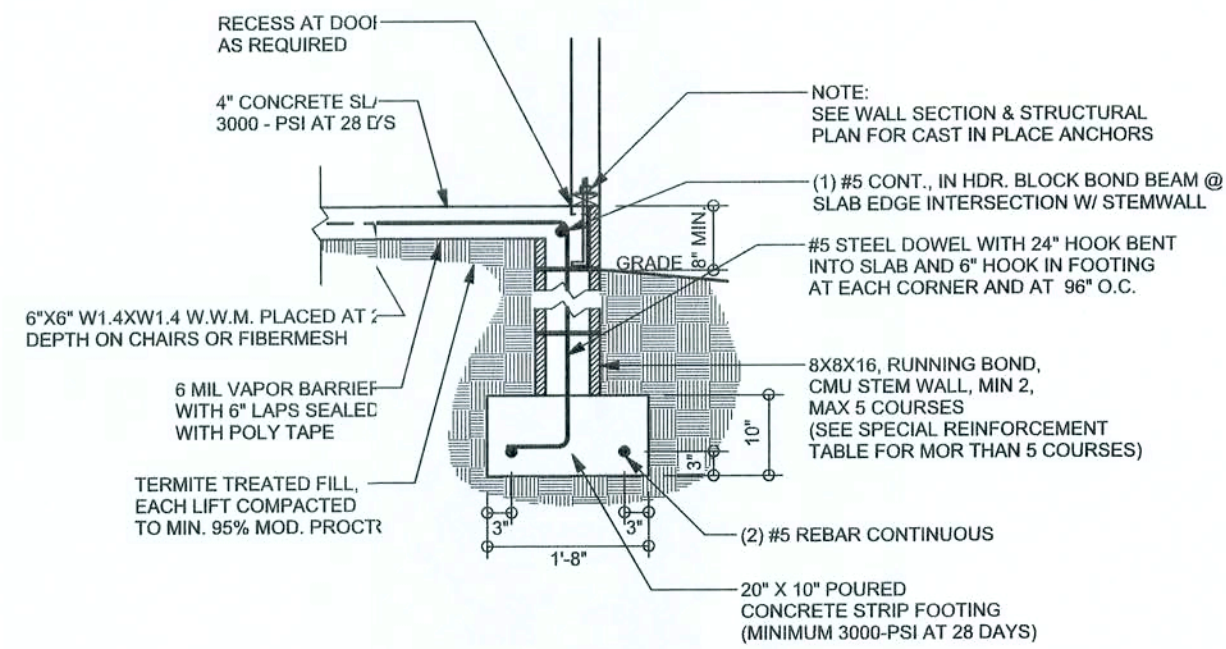
OF 3 SHEETS

REVISIONS

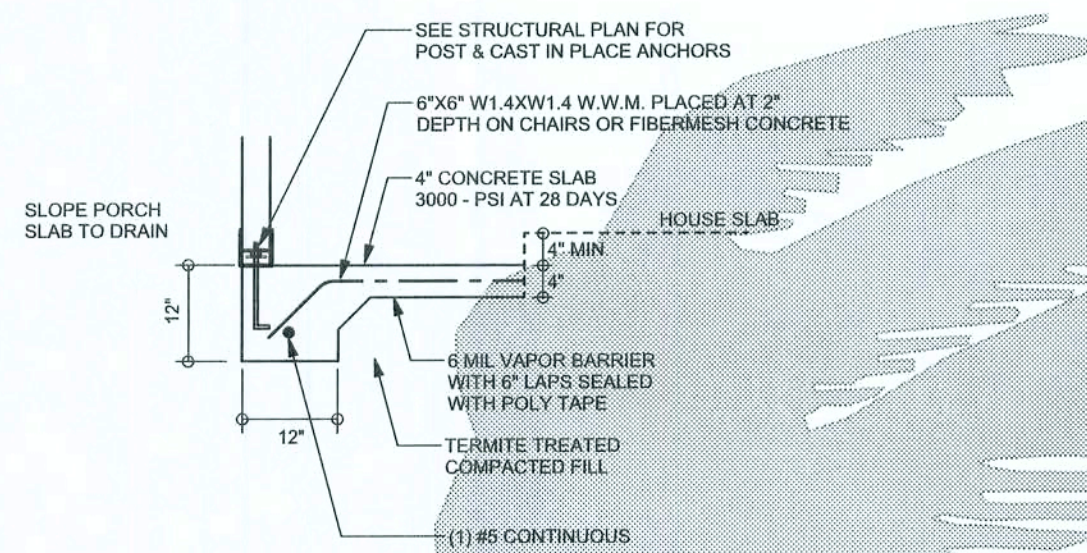
SOFTPLAN
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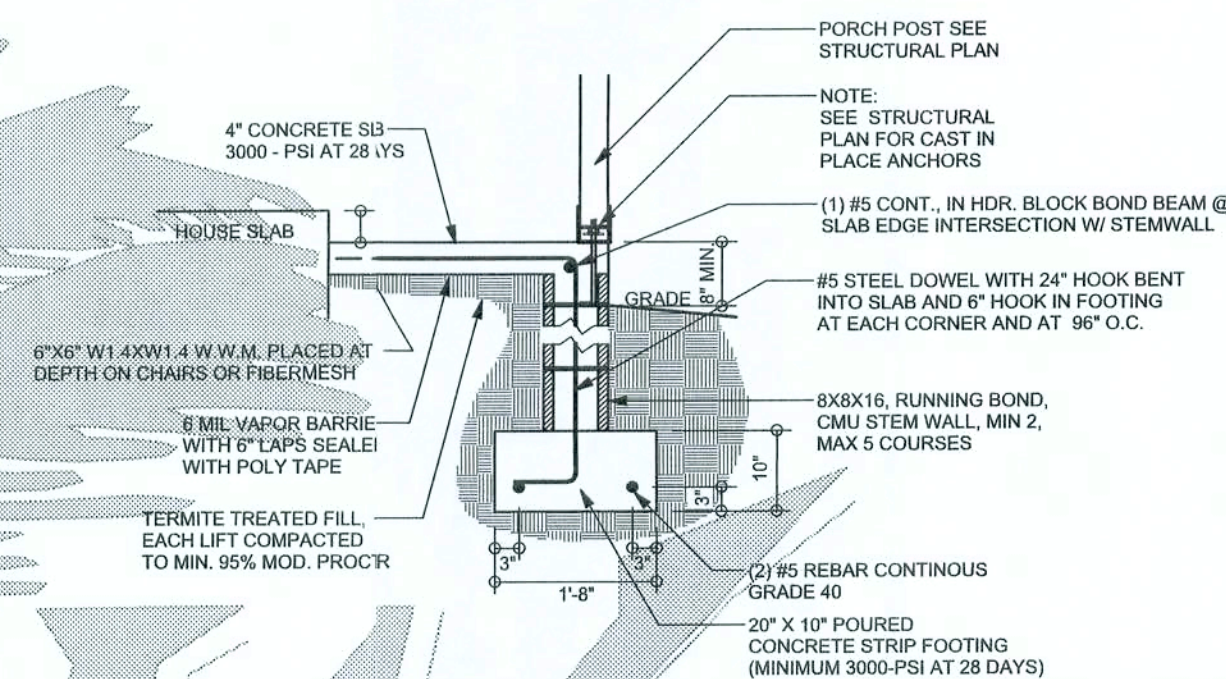
F1 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



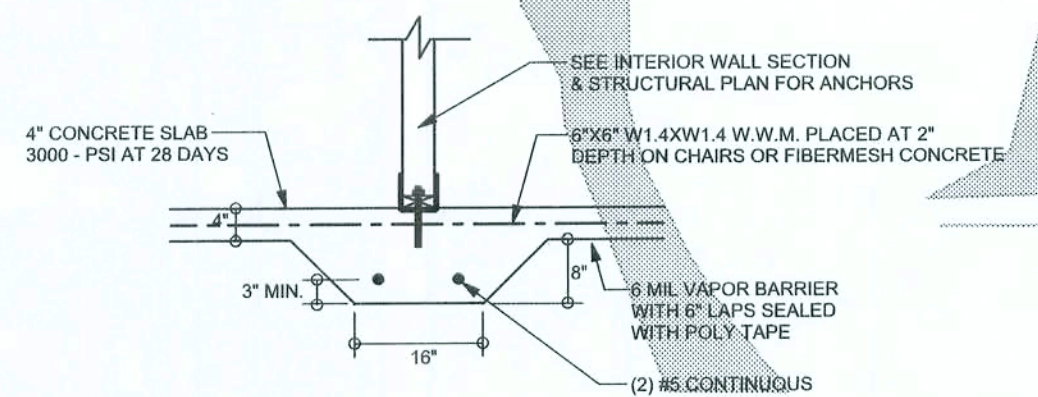
F9 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F12 STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

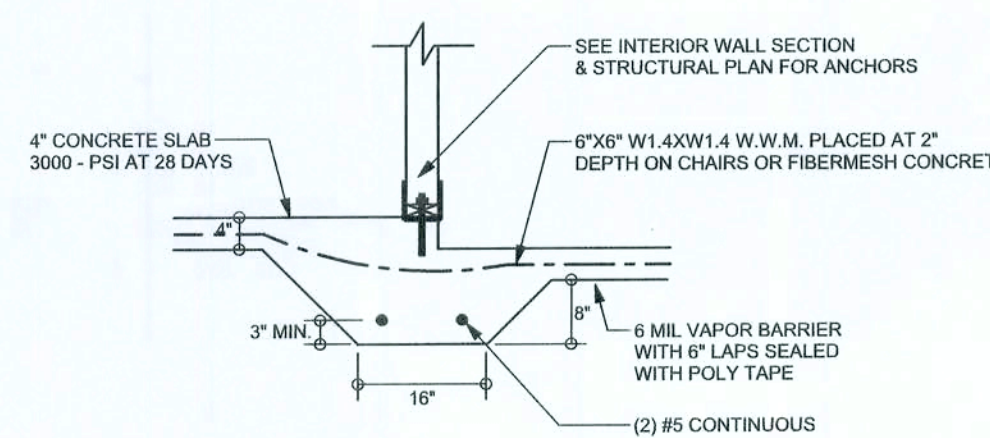


F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

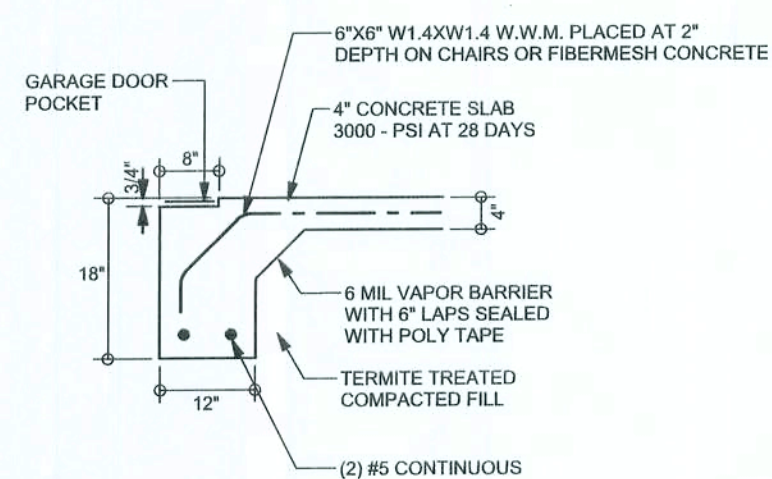
TALL STEM WALL TABLE

The table applies to tall reinforcing bars with 6" hook in the footing and bent 24" into the reinforced concrete at the top. The vertical steel is to be placed toward the tension side of the CMU wall (say from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' tall, add Diagonal ladder reinforcement at 15" O.C. vertically or a horizontal bond beam with 5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

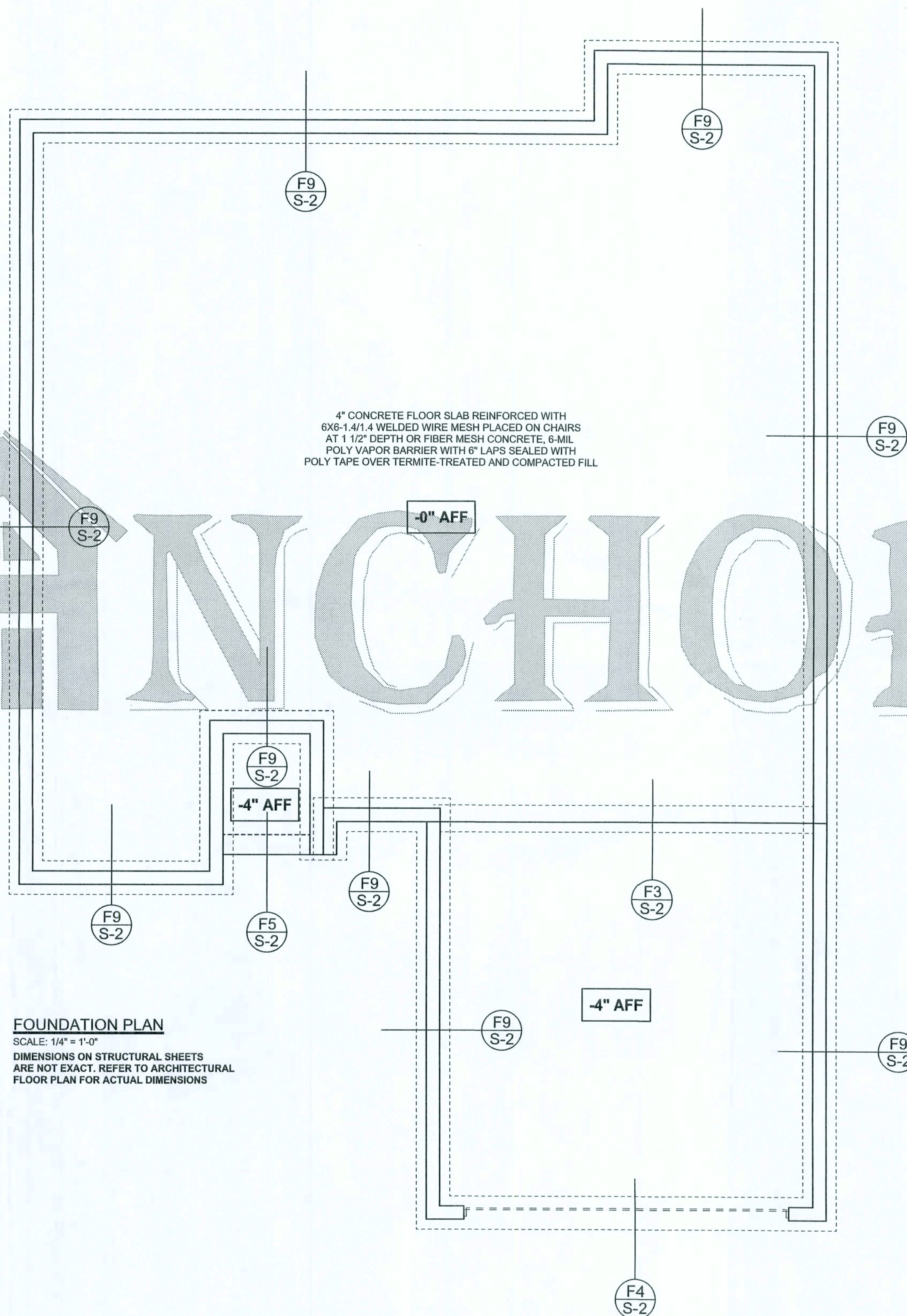
STEM WALL UNBALANCED HEIGHT (FEET)	BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53815, POB 888, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R302.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 5315

09/11/07

SEA

Compass Builders

Spec Fouse
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PRINTED DATE:
May 08, 2007

DRAWN BY: David Disoway
CHECKED BY:

FINAL DATE:
08 / May / 07

JOB NUMBER:
705084

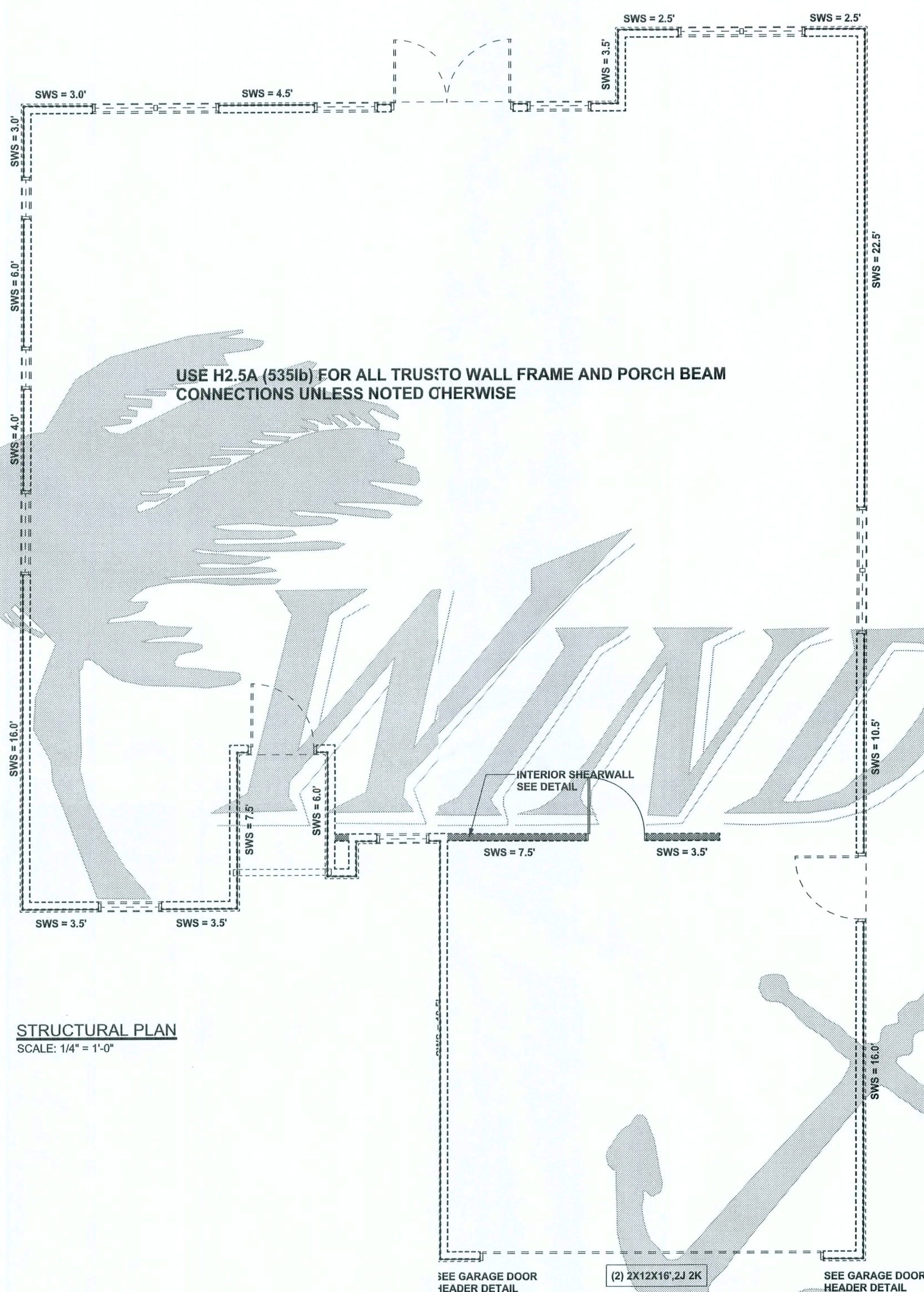
DRAWING NUMBER

S-2

OF 3 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



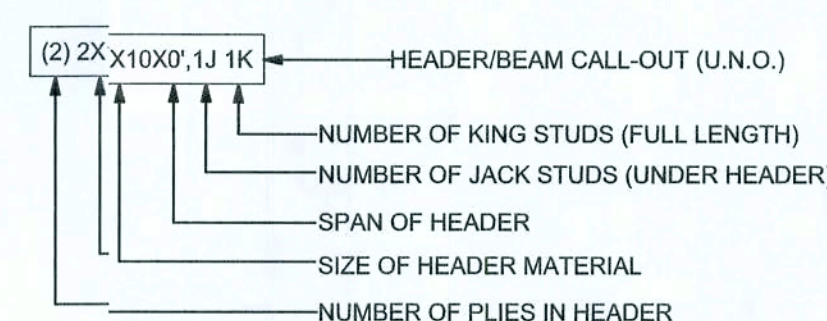
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-01, BCSI-02, & BCSI-03. BCSI-01, BCSI-02, & BCSI-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

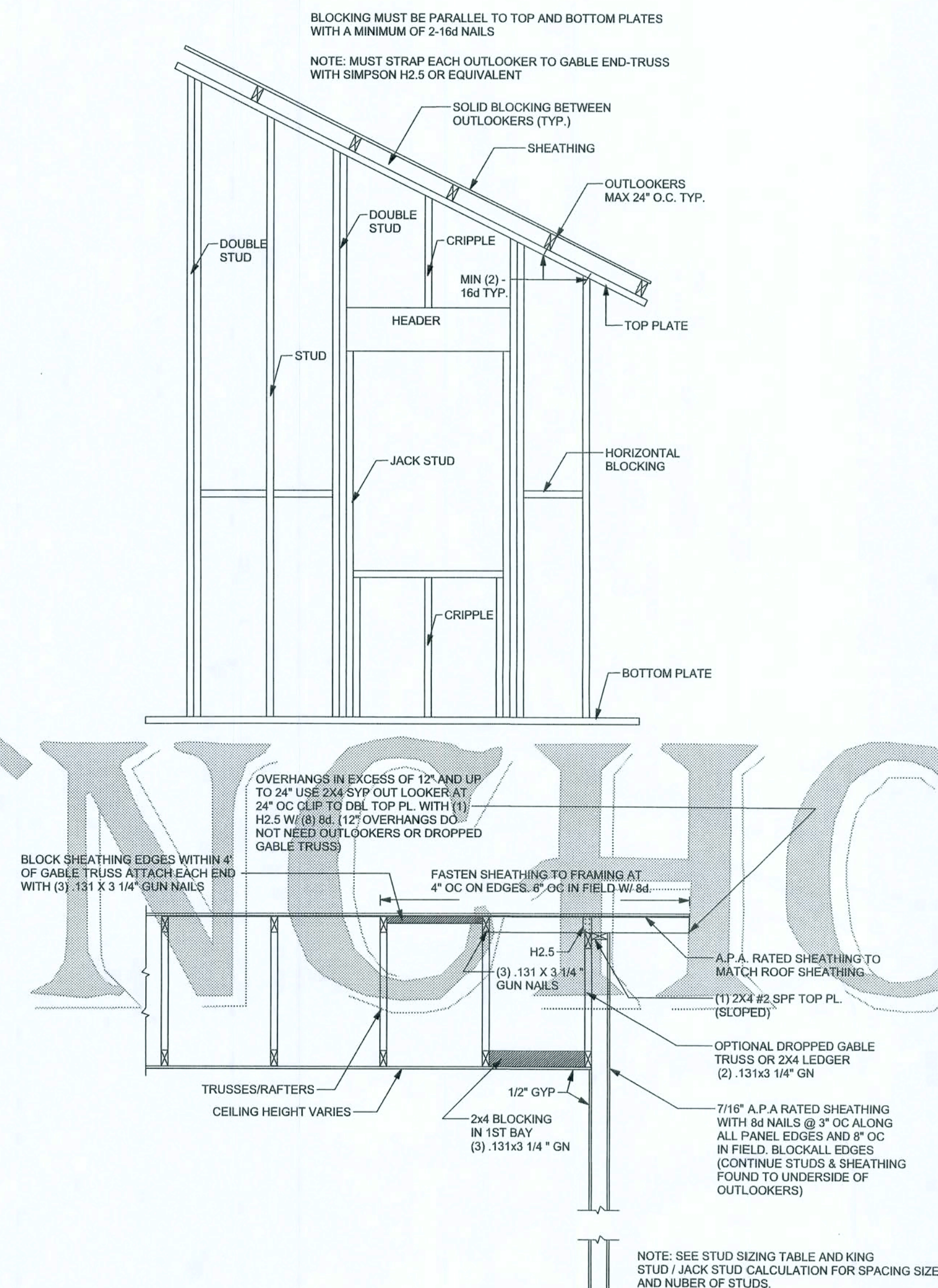
HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	30.0'	30.8'
LONGITUDINAL	28.5'	114.5'



GABLE END WALL BALLOON FRAMING DETAIL

SCALE: 1/2" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, W. B. HOWLAND TRUSS JOB #4493

WINDLOAD ENGINEER: Mark Disoway, P.E. 53915, P.O. Box 888, Lake City, FL 32066, 386-754-0419

DIMENSIONS: Stated dimensions superseded scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location

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May 08, 2007

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FINALS DATE:
08 / May / 07

JOB NUMBER:
705084

DRAWING NUMBER
S-3

OF 3 SHEETS