

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FLOOR TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT, CONNECTIONS, TIE-BACKS, BRACING, JOISTS, WALLS, AND TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S DESIGN AND TO OBTAIN THE NECESSARY PERMITS AND APPROVALS. CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR THE INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS DESIGN. ALL TRUSS DESIGN TO BE IN ACCORDANCE WITH THE MIN. UPLIFT CONNECTION 15LB/EACH END, 2X8 RATTENERS 70 LB/EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1600 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVE OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, FC = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" $\times$ 9" $\times$ 14" $\times$ W14 + 6# BS&S; WELDED WIRE REINFORCED: FIBER CONCRETE SLAB 2 INCHES THICK. SLABAGE AMOUNT FROM 0.15 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLY TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOISTS: WHEN SPECIFIED, SAWN CONTROL JOISTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ADO 302. JOISTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND THE LENGTH / WIDTH RATIO OF JOIST AREAS SHALL NOT EXCEED 1.5.

RECOMMENDED LOCATION OF CONTROL JOISTS IS SUBJECT TO OWNER AND CONTRACTOR'S DISCRETION. JOISTS SHOULD BE LOCATED TO MINIMIZE CRACKS (CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A615, GRADE 40, DEFORMED BARS. FY + 40SI; LAP SPACES 40" DB 25# FOR #5 BARS; UNO. ALL REINFORCEMENT SHALL BE DETAIL PLACED AND LAPPED IN ACCORDANCE WITH 01555. U.S.C.A.

ROOF SHEATHING: ALL ROOF ROOF HORIZONTAL DIAPHRAGMS, SHEATHING, AND INSULATION SHALL BE APPLIED PERMANENTLY TO FRAMING, OVER A MINIMUM OF 3 FRAMING

STRUCTURAL CONNECTIONS, MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTIONS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT TYPE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY PROVIDED PRODUCT. TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO AVOID OVER SATURATION.

ANCHOR BOLTS - 3/8" ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR IN GROUTED CMU.

OWNER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SIFTED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCK REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IT MUST VERIFY THE PLANS OBTAIN A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER FOR CLARIFICATION.

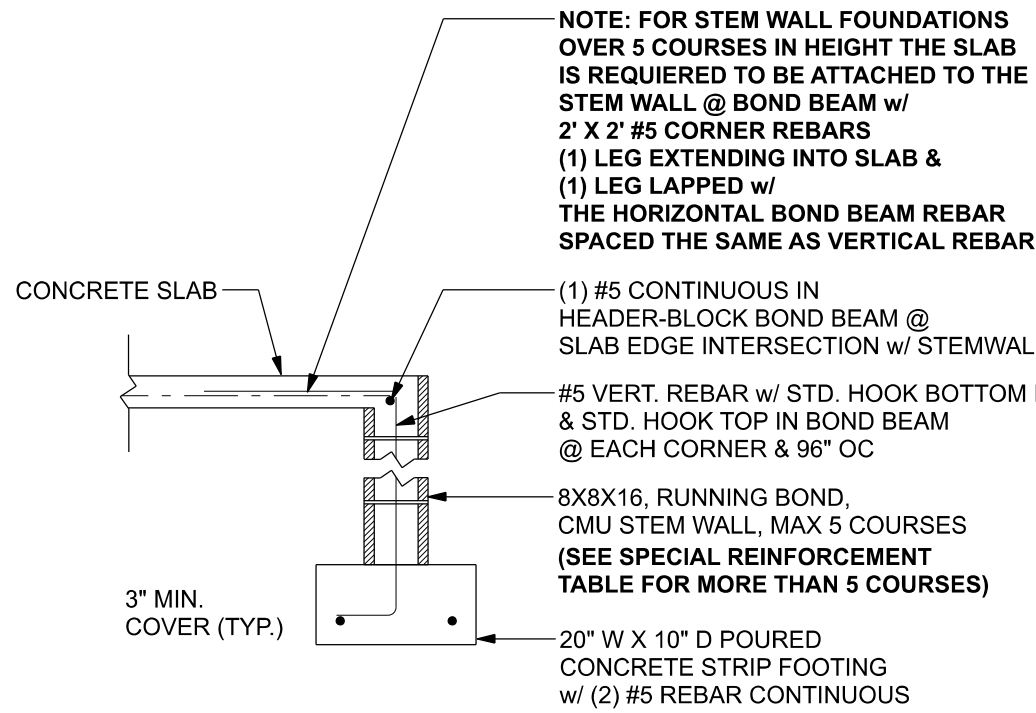
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, RAUUS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

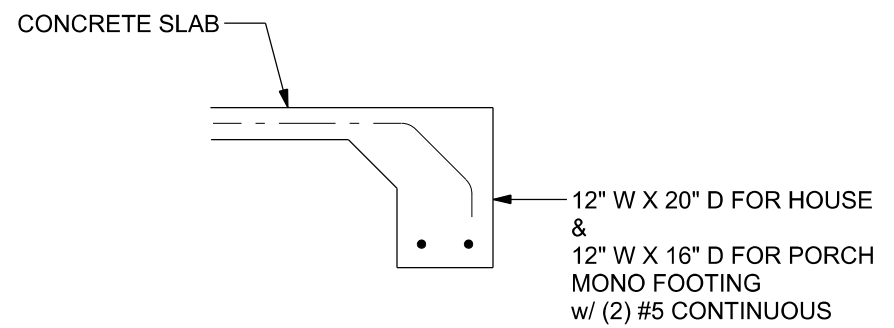
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCK. IT IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE OWNER TO OBTAIN THE DESIGN OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND THE WIND LOAD ENGINEER'S SEALING AND SIGNATURE. THE PROFESSIONAL FOR CORRECT APPLICATION OF FBCK REQUIRED.

REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

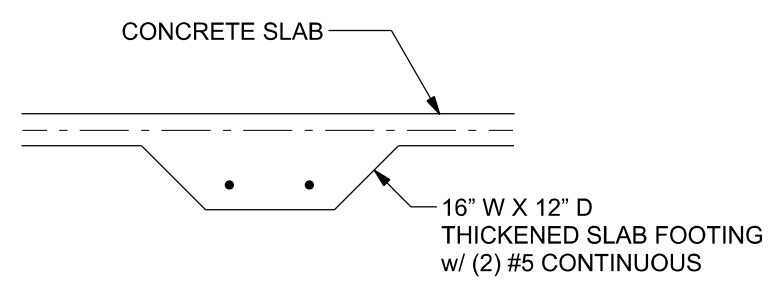
DESIGN CATEGORY & LOADS:			
BUILDING CODE		17TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)	
CODE FOR DESIGN LOADS		ASCE 7-16	
WINDLOADS			
BASIC WIND SPEED (ASCE 7-16, 3S GUST)		130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)		C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)		I	
RISK CATEGORY		II	
ENCLOSURE CLASSIFICATION		ENCLOSED	
INTERNAL PRESSURE COEFFICIENT		0.18	
ROOF ANGLE		7-45 DEGREES	
MEAN ROOF HEIGHT		30 FT	
C&D DESIGN PRESSURES		SEE TABLE	
FLOOR LOADING			
ROOMS OTHER THAN SLEEPING ROOM		40 PSF LIVE LOAD	
SLEEPING ROOMS		30 PSF LIVE LOAD	
ROOF LOADING			
FLAT OR < 4:12		20 PSF LIVE LOAD	
4:12 TO < 12:12		16 PSF LIVE LOAD	
12:12 & GREATER		12 PSF LIVE LOAD	
SOIL BEARING CAPACITY		1500 PSF	
FLOOD ZONE		THIS BUILDING IS NOT IN THE FLOOD ZONE	



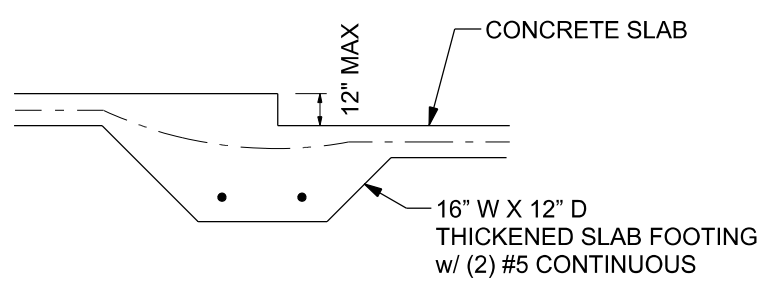
F1 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



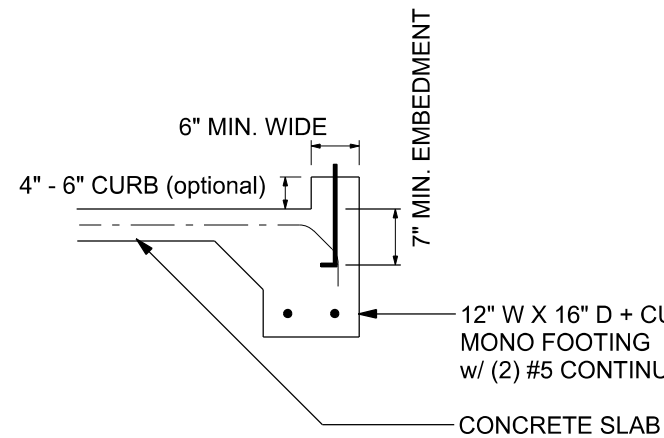
F1 S-2 OPTIONAL MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



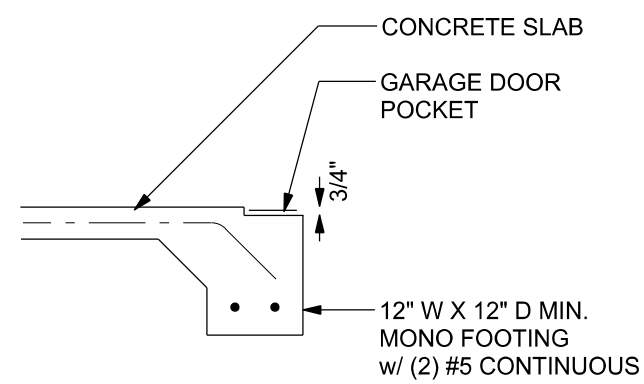
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 OPTIONAL MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"

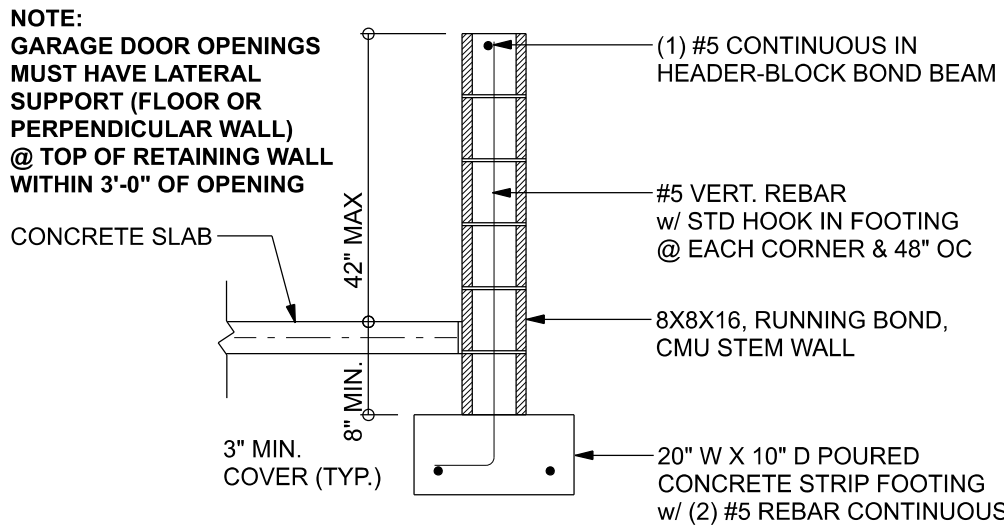


F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"

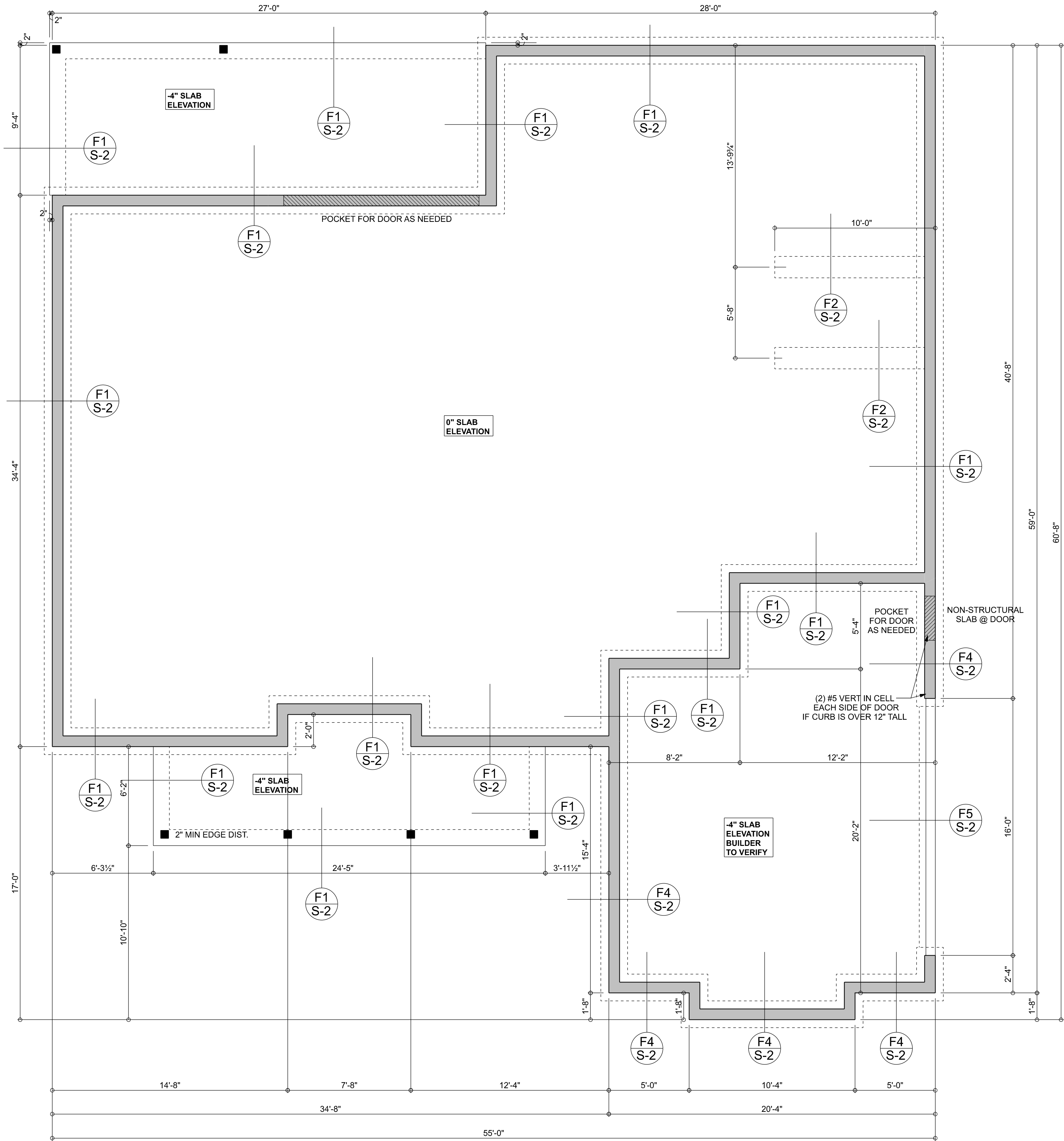
TALL STEM WALL TABLE:							
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.							
STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb2 or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wine ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb2 or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



F4 S-2 STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-1 #14 WELDED WIRE MESH PLACED ON CHAIRS @ 1'-0" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES-TREATMENT METHOD CAN BE USED INSTEAD)

Bryan Zecher Construction

Truntz Res.

PROJECT ADDRESS:
1276 SW 17th St
Ft. White, FL 32038

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915
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Wednesday, May 4, 2022

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JOB NUMBER:
220544

S-2

OF 3 SHEETS

