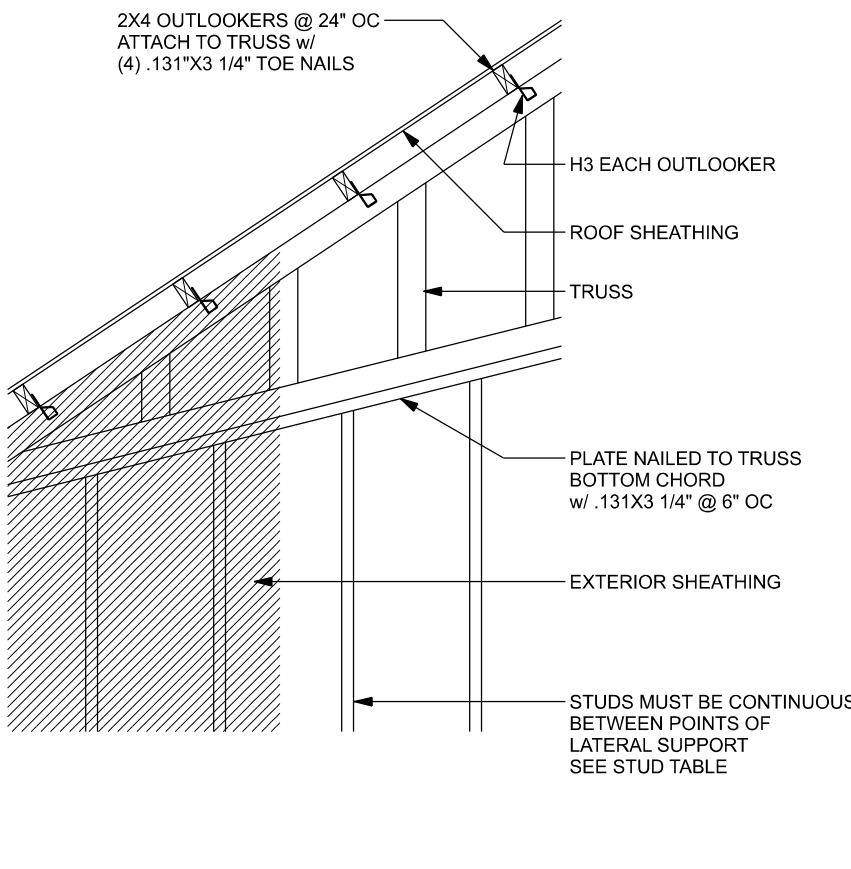
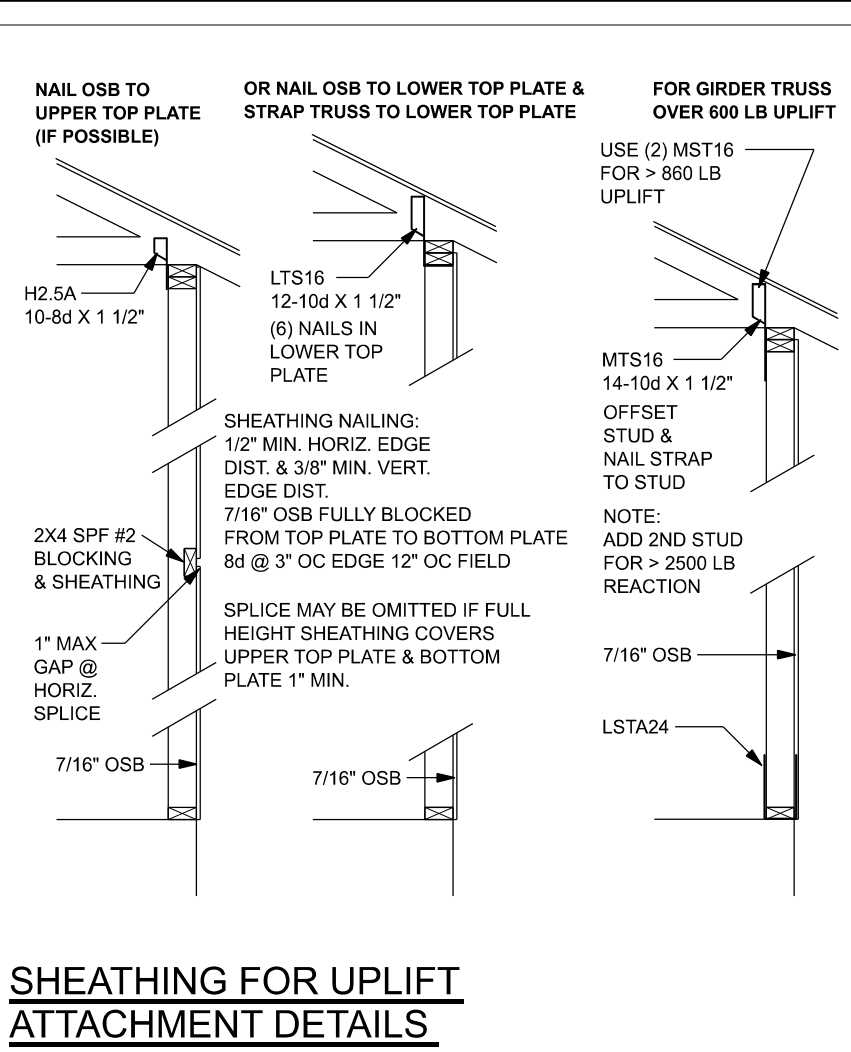
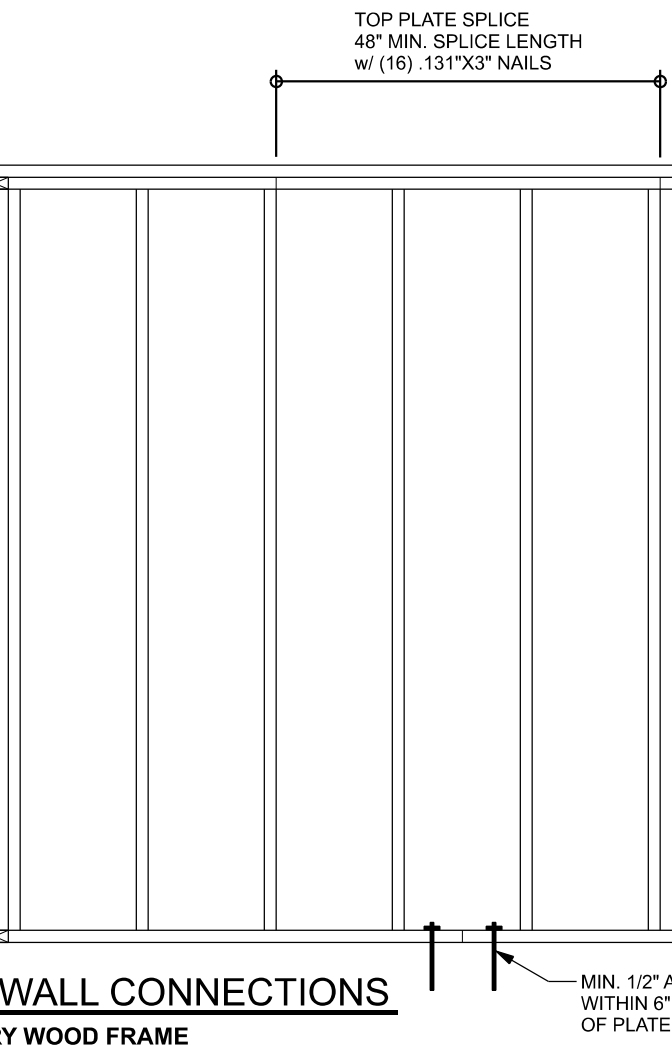
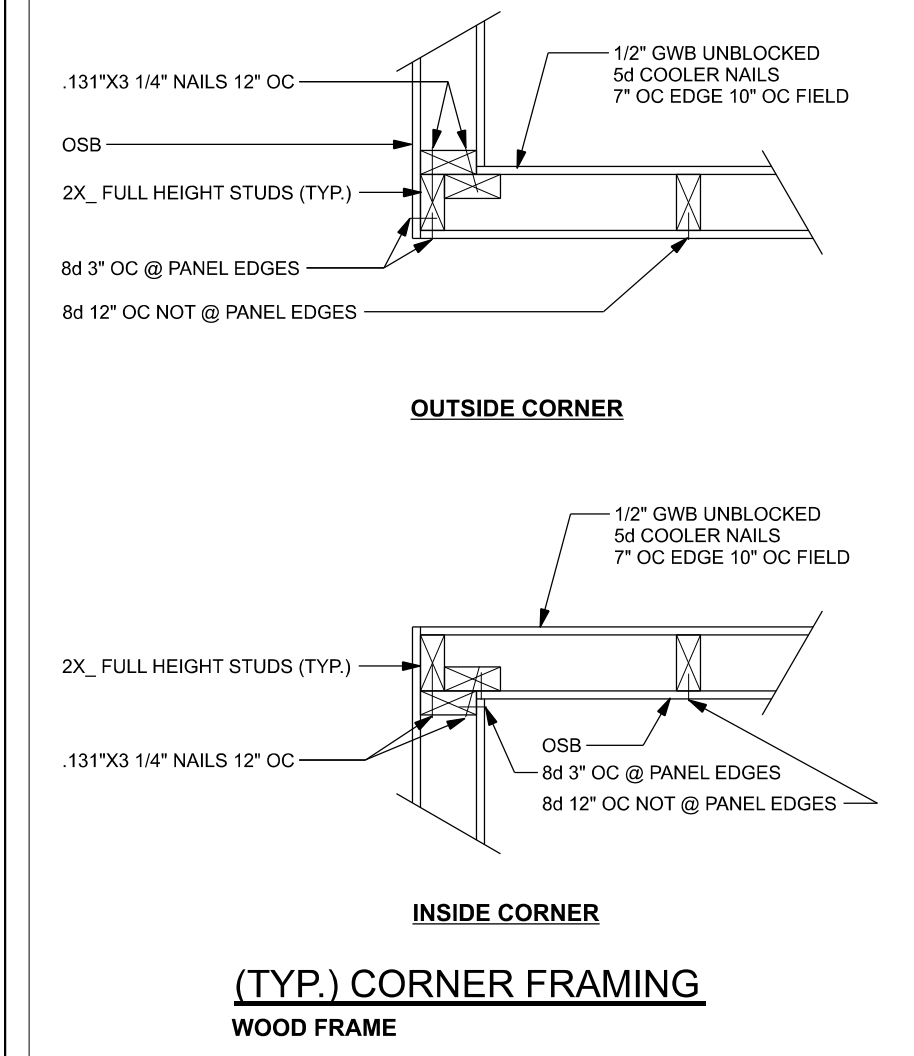
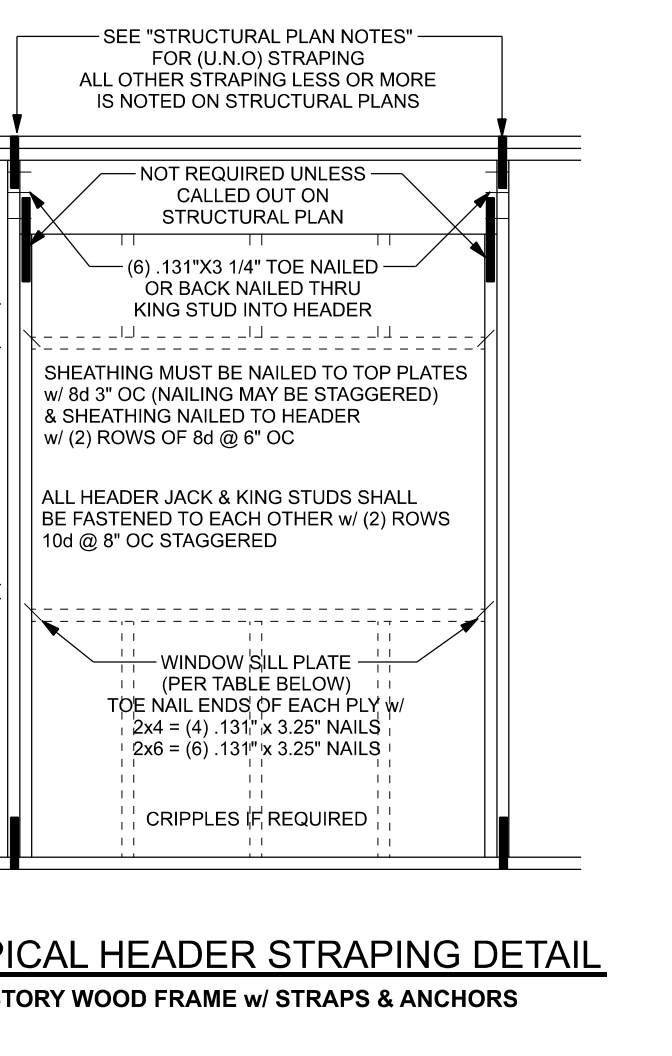
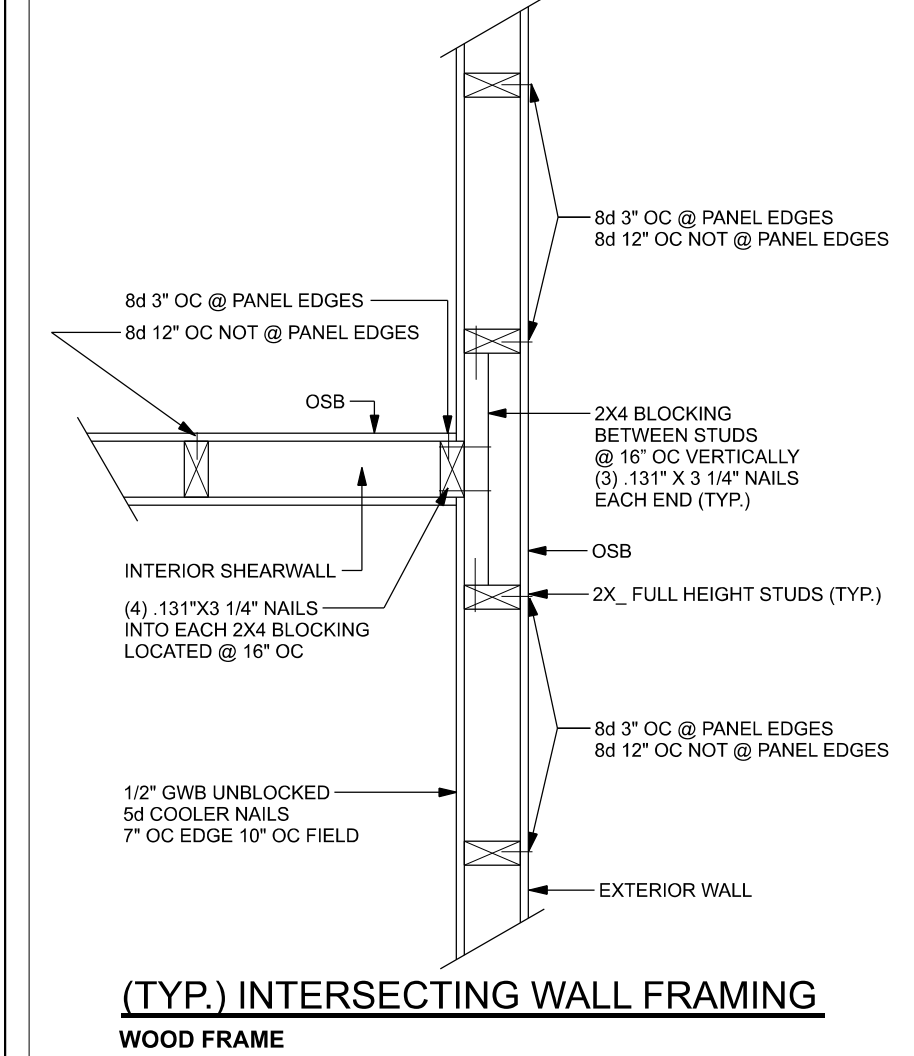
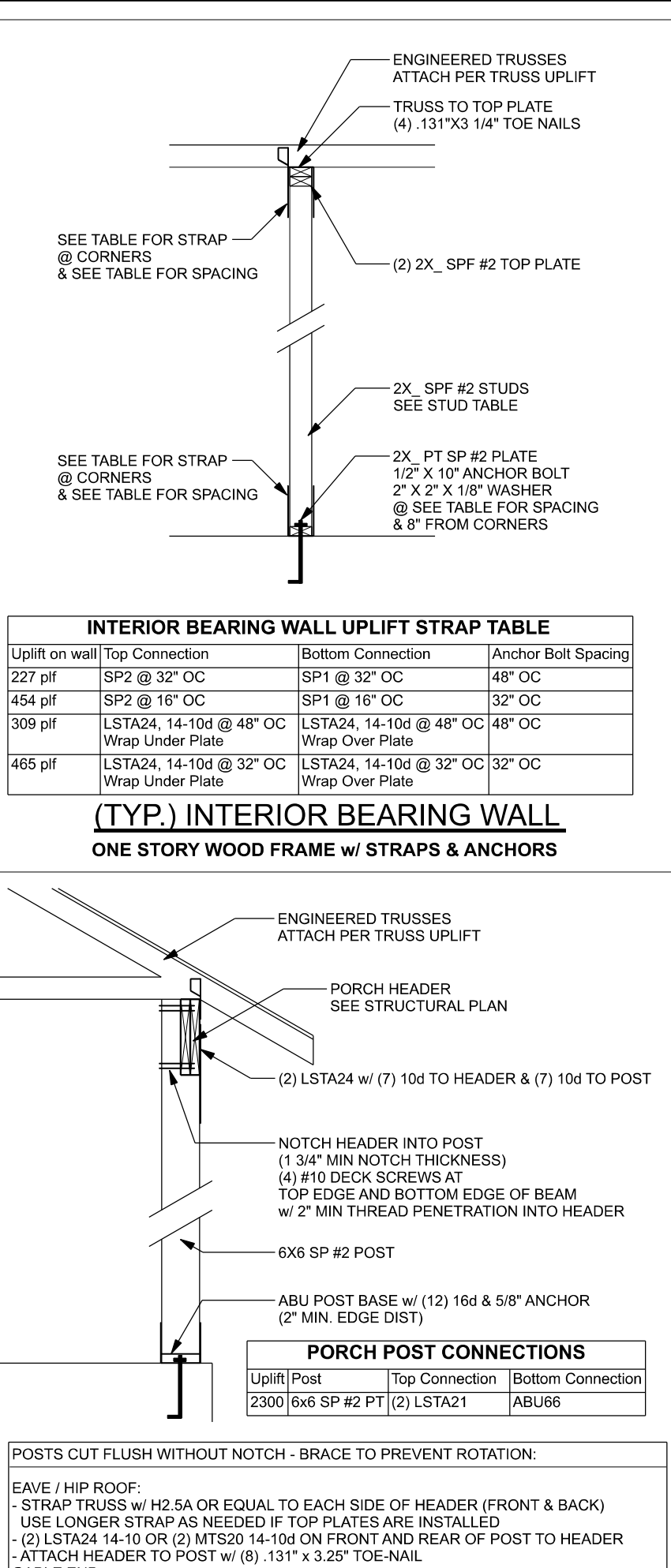
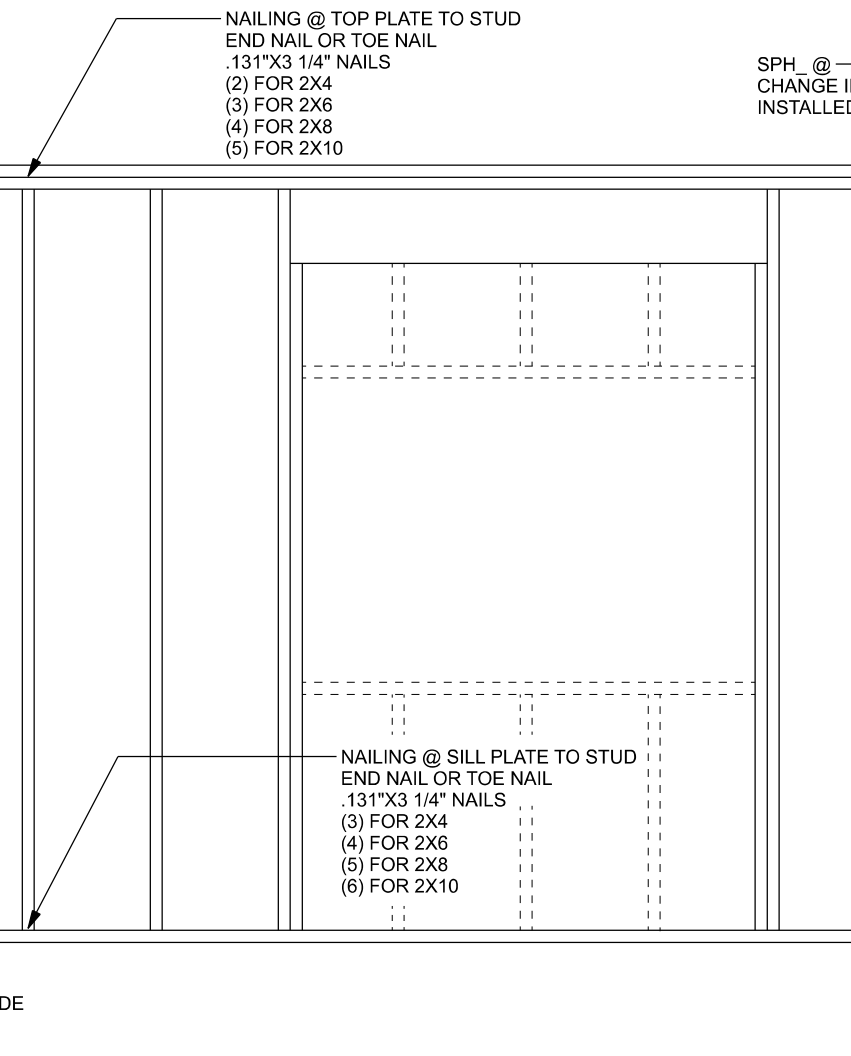
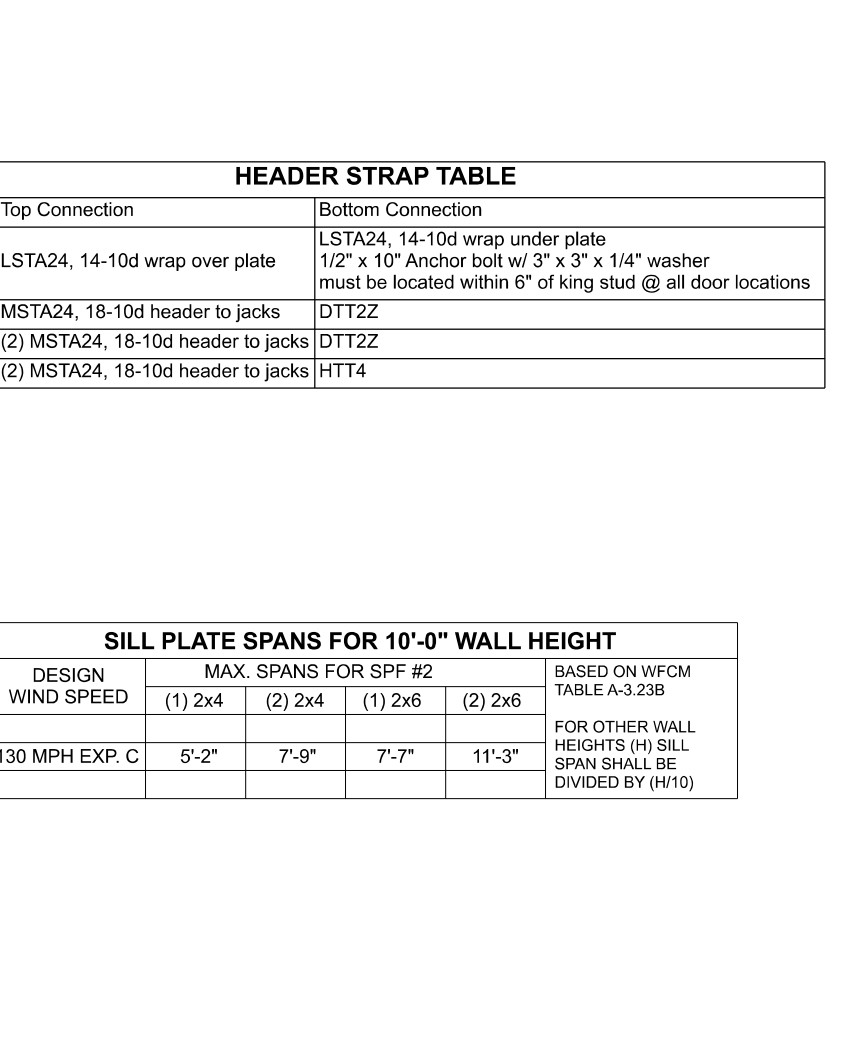


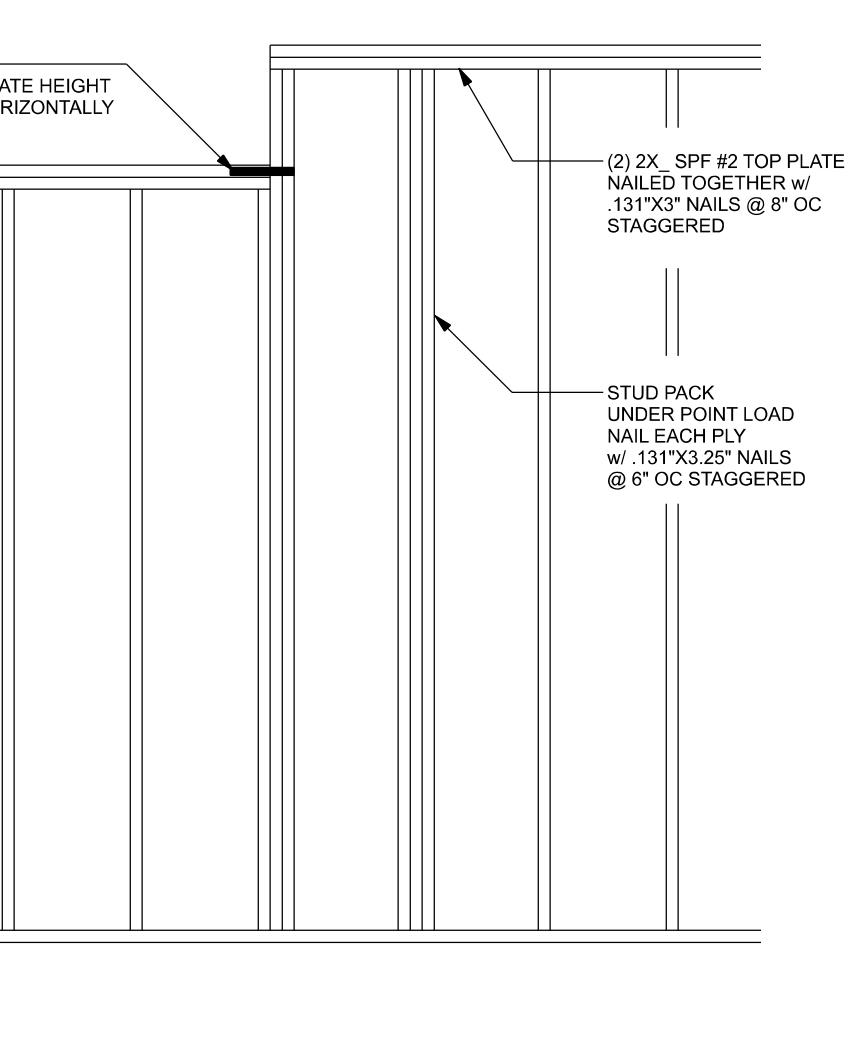
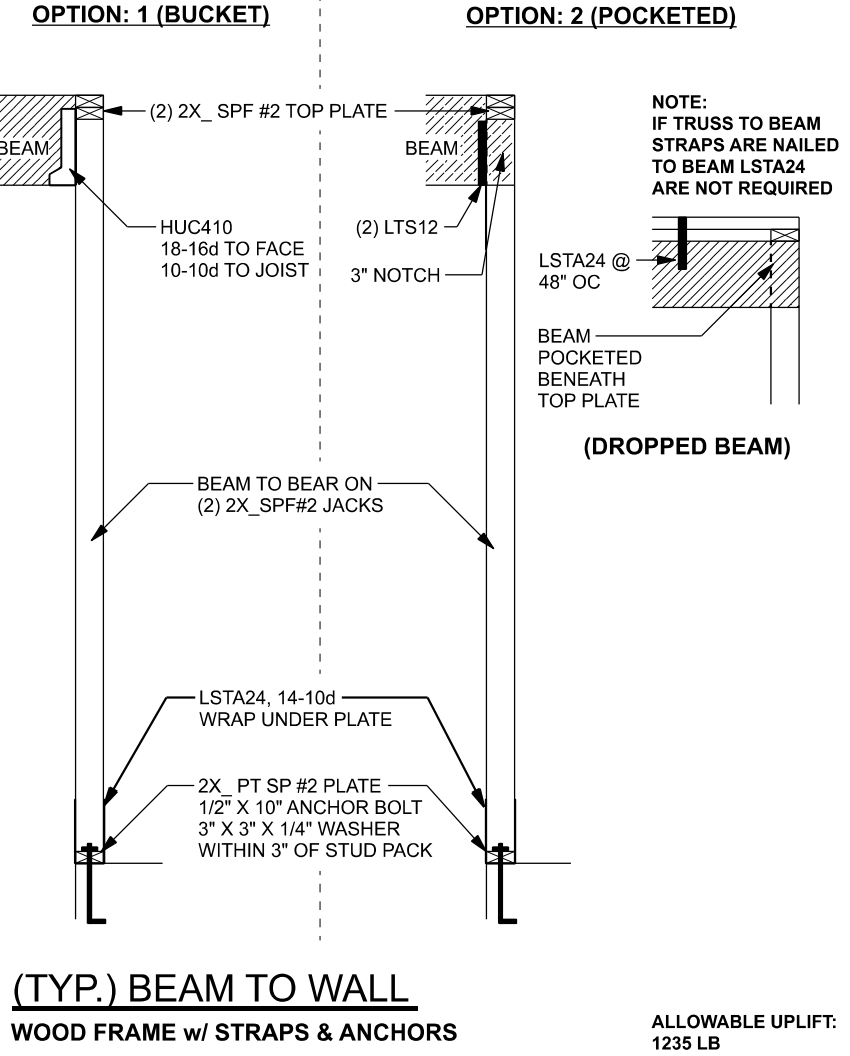
(TYP.) GABLE BRACING DETAIL  
WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING  
WOOD FRAME



(TYP.) PORCH POST  
ONE STORY WOOD



CONNECTOR TABLE				
Uplift SP Uplift SPFF Truss Connector	To Plate	To Truss/Rafter		
615 485 SDWC15600	4-8dX1 1/2"	4-8dX1 1/2"		
415 290 H3	4-8dX1 1/2"	4-8dX1 1/2"		
575 485 H2.5A	5-8dX1 1/2"	5-8dX1 1/2"		
1340 1015 H10A	9-10d1 1/2"	9-10d1 1/2"		
720 620 LTS12-20	6-10d1 1/2"	6-10d1 1/2"		
1000 860 MTS12-30	7-10d1 1/2"	7-10d1 1/2"		
1450 1245 HTS20-30	12-10d1 1/2"	12-10d1 1/2"		
Uplift SP Uplift SPFF Strap Ties	To One Member	To Other Member		
1235 1235 LSTA31	8-10d	8-10d		
1640 1455 MSTA24	9-10d	9-10d		
1030 1030 CS20	7-10d	7-10d		
Uplift SP Uplift SPFF Stud Plate Ties	To Stud	To Plate		
585 535 SP1	6-10d	4-10d		
1065 865 SP2	6-10d	6-10d		
771 771 LSTA24	10-10d	wrap under or over plate		
1235 1235 LSTA24	14-10d	wrap under or over plate		
Uplift SP Uplift SPFF Holdowns @ Stenwall	To Stud / Post	Anchor		
1825 1800 DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD		
4235 3640 HTT4	18-16dX2 1/2"	1/2"x12" Titen HD		
Uplift SP Uplift SPFF Holdowns @ Mono	To Stud / Post	Anchor		
1825 1800 DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD		
4235 3640 HTT4	18-16dX2 1/2"	1/2"x12" Titen HD		
Uplift SP Uplift SPFF Post Bases @ Stenwall	To Post	Anchor		
2200 ABU44	12-16d	5/8"x12" Drill & Epoxy		
2300 ABU66	12-16d	5/8"x12" Drill & Epoxy		
Uplift SP Uplift SPFF Post Bases @ Mono	To Post	Anchor		
2200 ABU44	12-16d	5/8"x7" Drill & Epoxy		
2300 ABU66	12-16d	5/8"x7" Drill & Epoxy		

FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)			
(1) 2x4 @ 16" OC		TO 10'-1" STUD HEIGHT	
(1) 2x4 @ 12" OC		TO 11'-2" STUD HEIGHT	
(1) 2x6 @ 16" OC		TO 15'-7" STUD HEIGHT	
(1) 2x6 @ 12" OC		TO 17'-3" STUD HEIGHT	

GRADE & SPECIES TABLE			
		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GRADE & SPECIES TABLE			
	Fb	E	
2x8 SP #2	925	1.4	
2x10 SP #2	800	1.4	
2x12 SP #2	750	1.4	
GLB 24F-V3 SP	2600	1.9	
LSL TIMBERSTRAND	1700	1.7	
LVL MICROLAM	2950	2.0	
PSL PARALAM	2900	2.0	



**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, LAYOUT, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X4 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" x V14 x V14, FB = 89KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH & WIDTH RATION OF SLAB EDGES SHALL NOT EXCEED 1:5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED) LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE INTERS JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-16, T.I.U.C.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED AS EXAMPLES NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A 307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

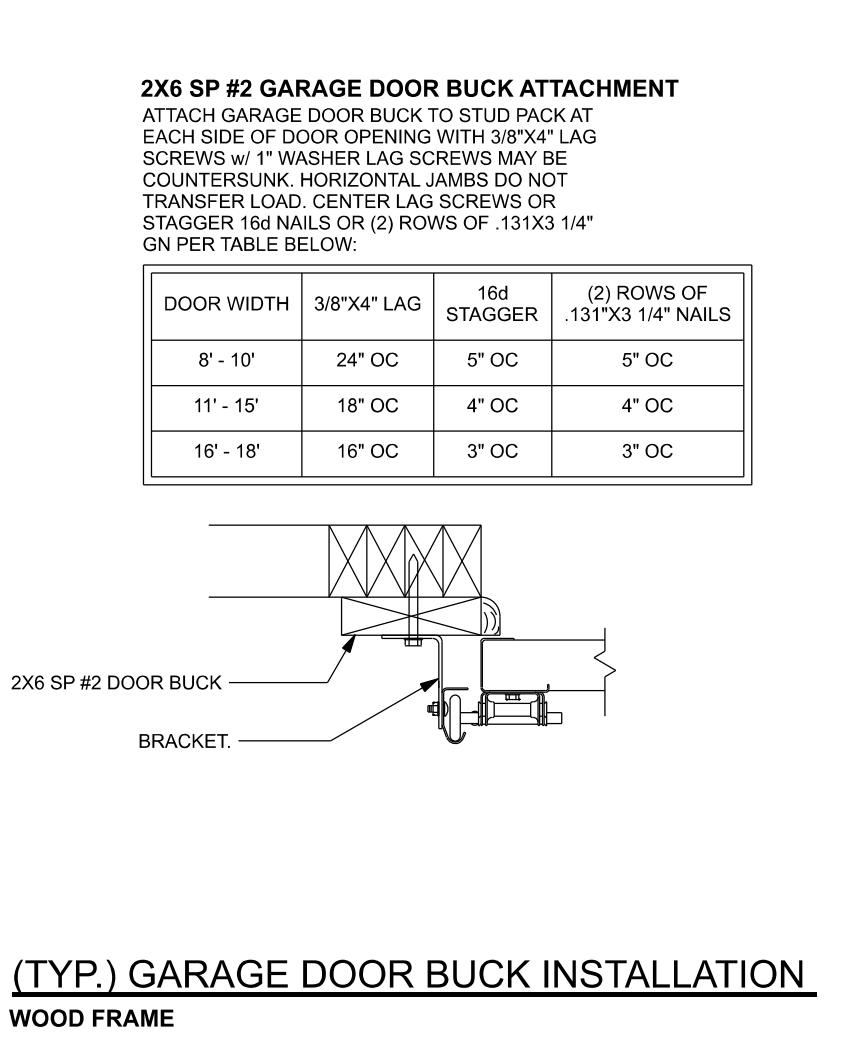
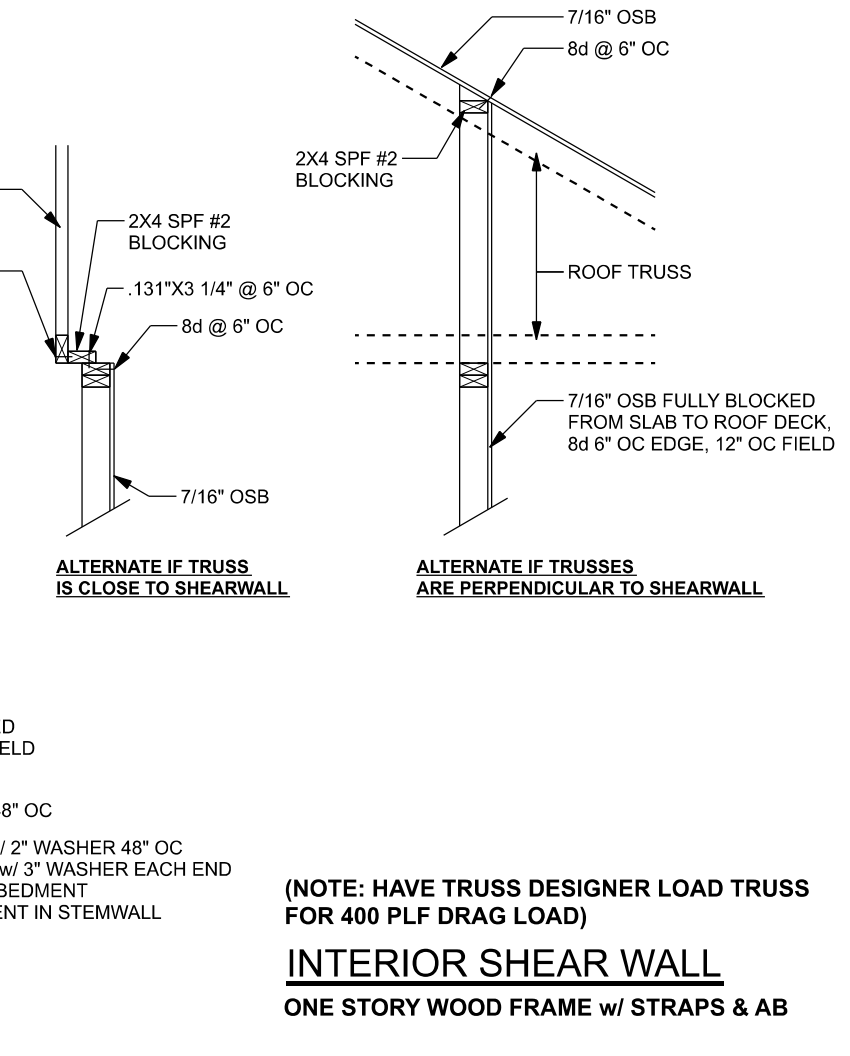
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES TO FULLY COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, LAYOUT, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED



Columbia County Building Department

Plans Reviewed for Code Compliance

State of Florida

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)			
EFFECTIVE WIND AREA (FT <sup>2</sup> )	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER	
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)	
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)	
<b>GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)</b>			
9x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)		
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)		

Chemerys Construction LLC

Spec Home - Lot 6 Fort White Station

PROJECT ADDRESS: Lot 6 Fort White Station Fort White, FL 32038

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS: Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

MARK DISOWAY P.E. 53915

THIS PDF HAS DIGITAL SIGNATURE AND ELECTRONIC SEAL. PRINTED COPIES ARE NOT CONSIDERED SIGNED OR SEALED. YOU MUST VERIFY SIGNATURE ON THIS PDF. [CLICK HERE TO VERIFY.](#)

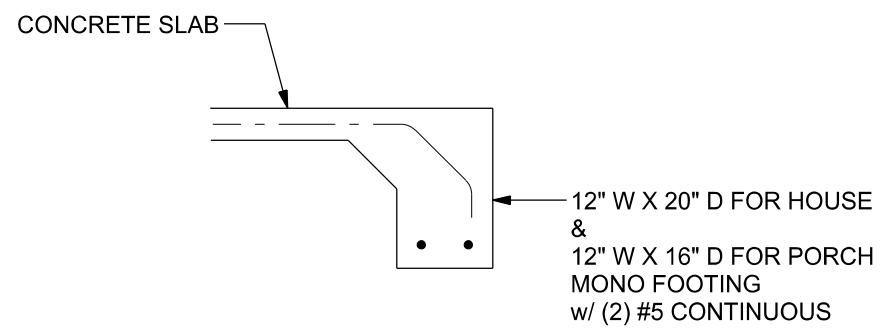
Friday, December 4, 2020

Mark Disoway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

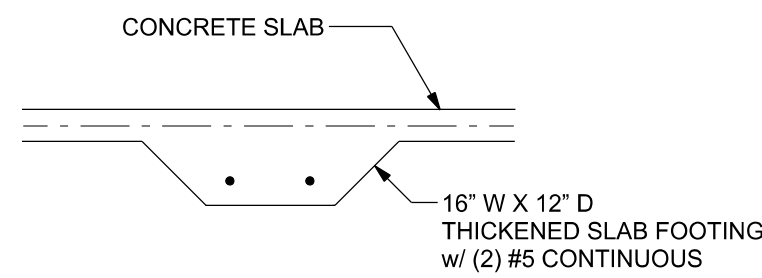
JOB NUMBER: 201305

**S-1**  
OF 3 SHEETS

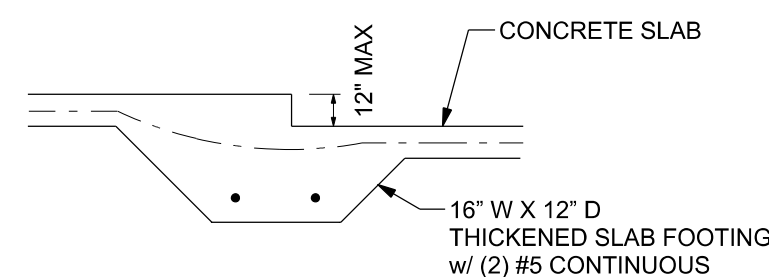




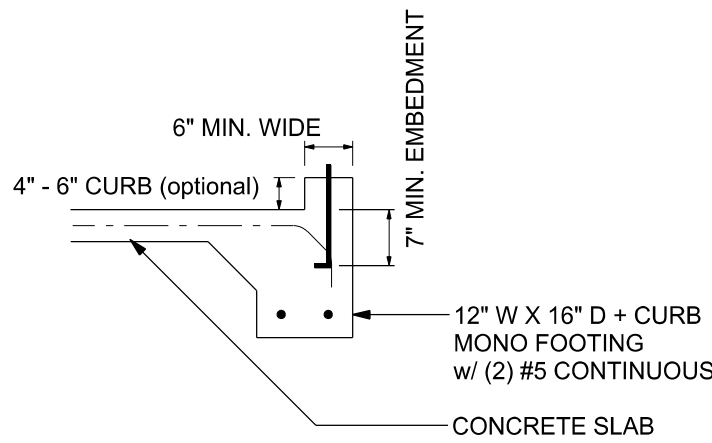
**F1 S-2**  
MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



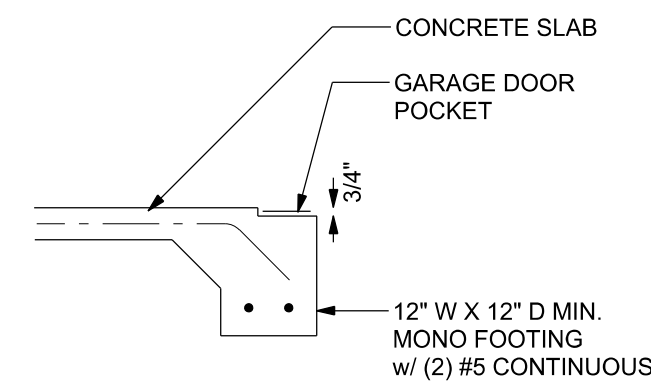
**F2 S-2**  
INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



**F3 S-2**  
INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"

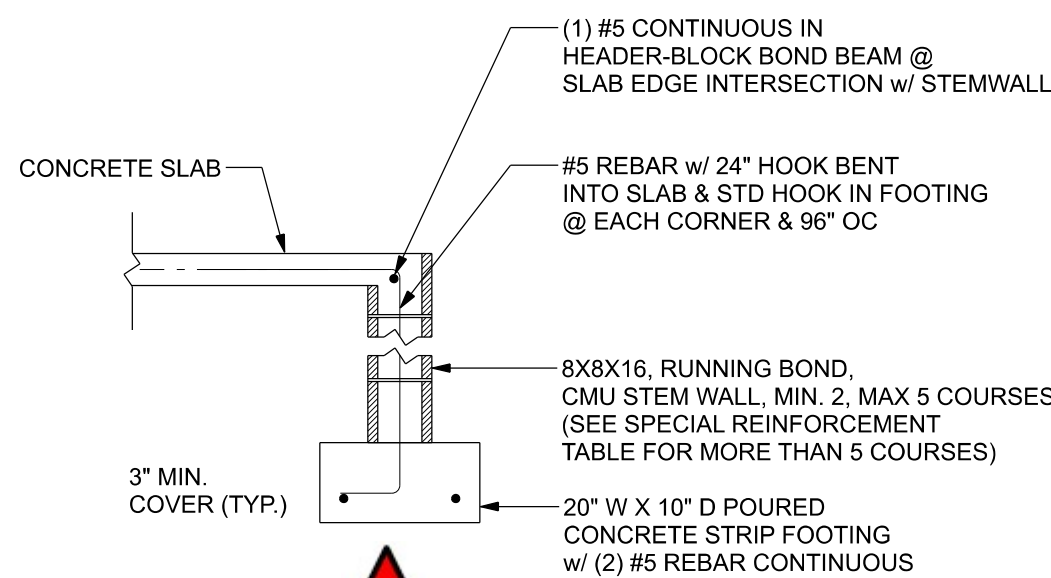


**F4 S-2**  
MONOLITHIC CURB FOOTING  
SCALE: 1/2" = 1'-0"

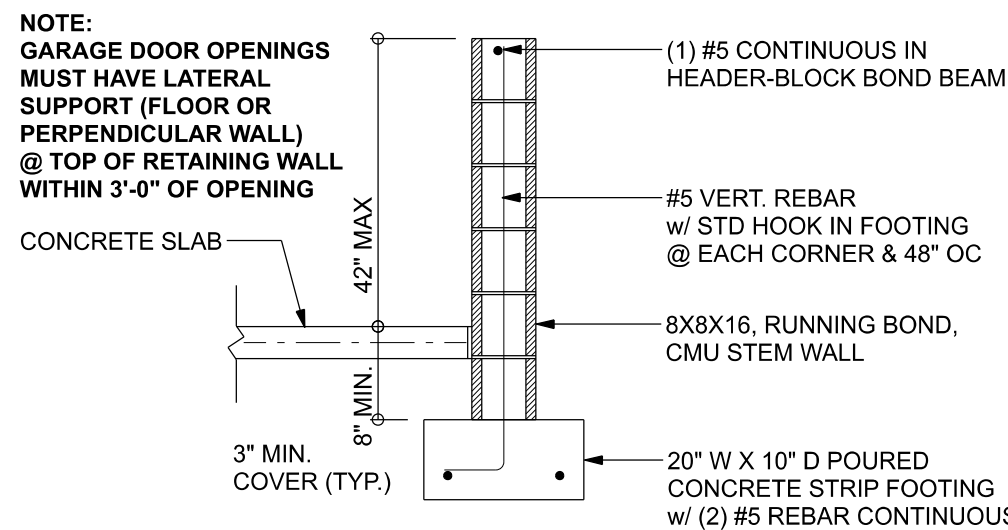


**F5 S-2**  
GARAGE DOOR POCKET FOOTING  
SCALE: 1/2" = 1'-0"

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017-RES. SECTION R403.1.4



**F1 S-2**  
OPTIONAL STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



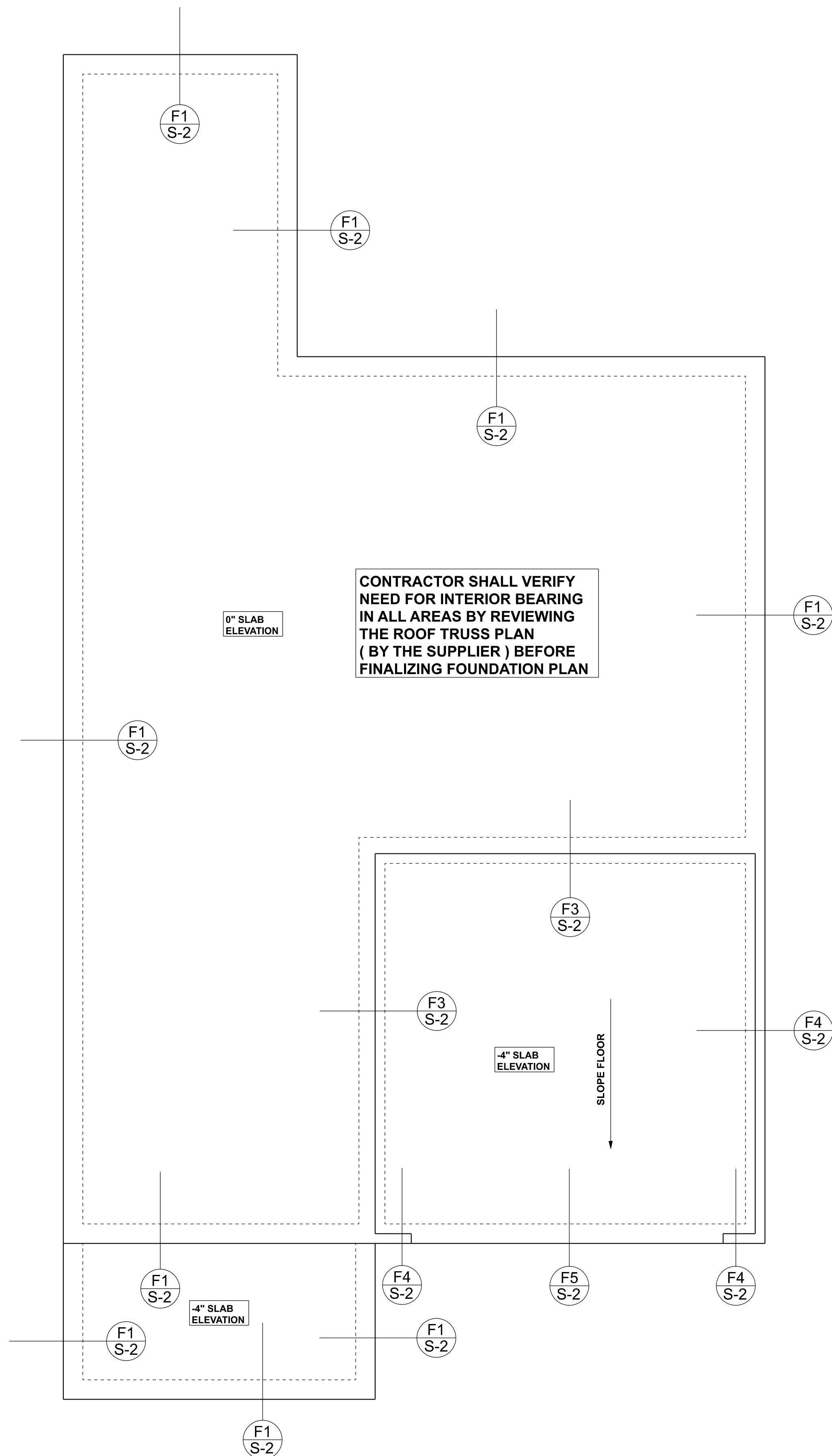
**F4 S-2**  
OPTIONAL STEM WALL CURB FOOTING  
SCALE: 1/2" = 1'-0"

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.9"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM A615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

**TALL STEM WALL TABLE:**  
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



## FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.

FN - 3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-14/14 WELDED WIRE MESH PLACED ON CHAIRS @ 1/2" DEPTH OR FIBER MESH CONCRETE, 5-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE TREATMENT METHOD CAN BE USED INSTEAD).

FOUNDATION DESIGN: Size footings per truss reactions and other loads. Locate footings per truss bearings. Interior shear walls require a thickened slab footing. For point loads > 5000 lb or repetitive loads > 3000 lb per truss provide pad footing 1' x 1' sqft, #5, 8"oc each way per 1500 lb of load.

Chemerys Construction LLC

Spec Home - Lot 6 Fort White Station

PROJECT ADDRESS:  
Lot 6 Fort White Station  
Fort White, FL 32038

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

**COPYRIGHTS AND PROPERTY RIGHTS:**  
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

THIS PDF HAS DIGITAL SIGNATURE AND ELECTRONIC SEAL. PRINTED COPIES ARE NOT CONSIDERED SIGNED OR SEALED. YOU MUST VERIFY SIGNATURE ON THIS PDF. [CLICK HERE TO VERIFY.](#)



Friday, December 4, 2020

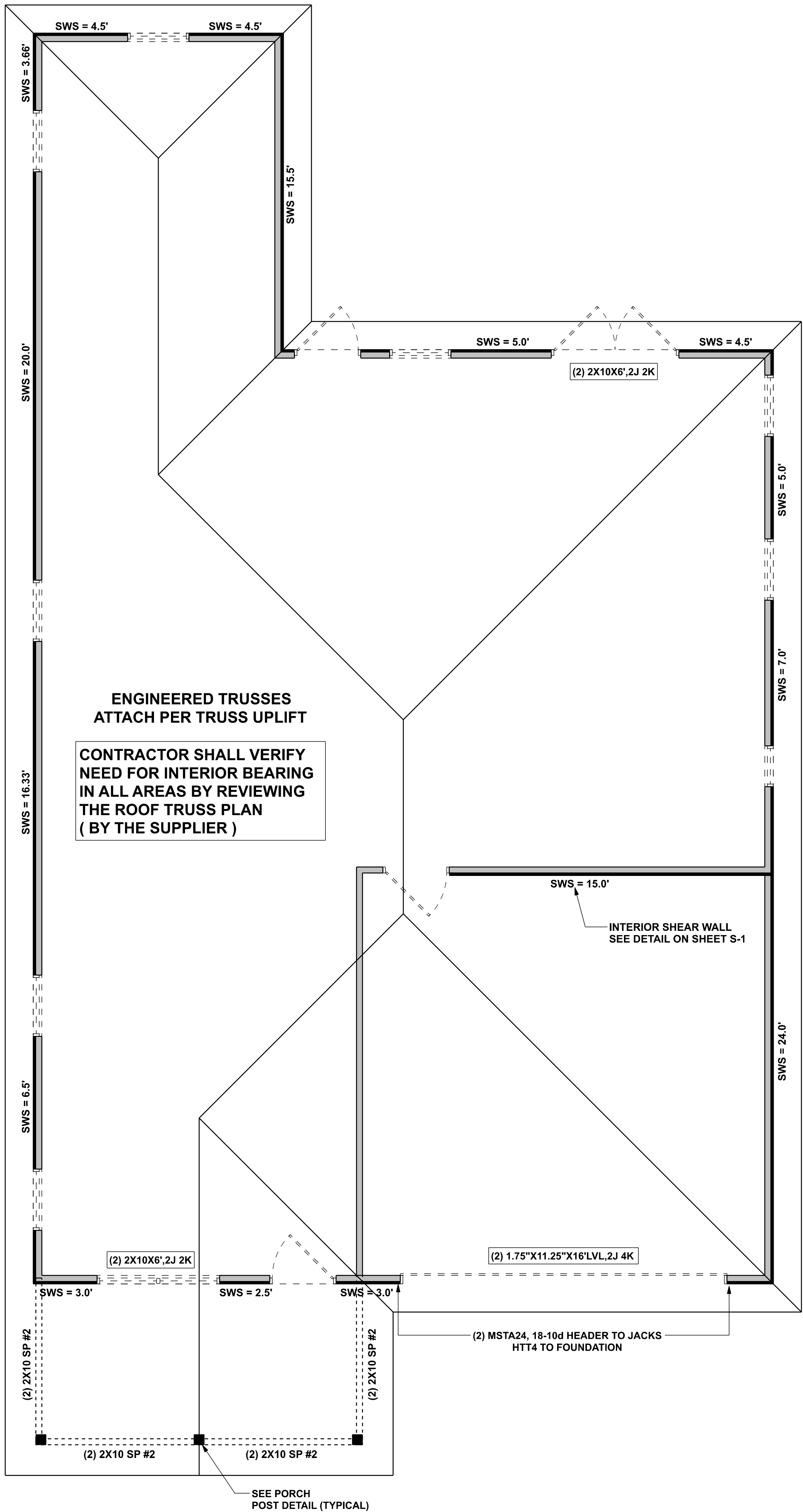
Mark Disosway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
201305

**S-2**

OF 3 SHEETS

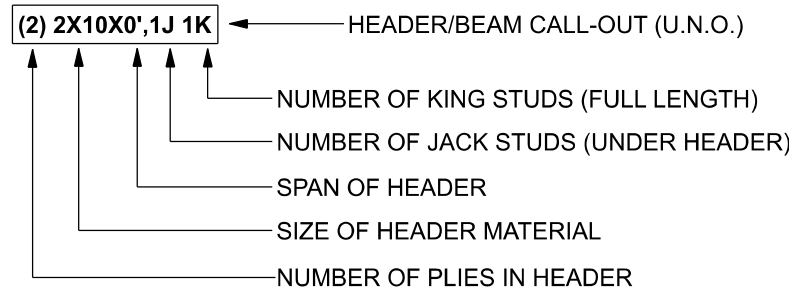




#### STRUCTURAL PLAN NOTES

- SN-1** ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2** ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3** ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4** USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5** DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6** PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

#### HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	22154 LBF	16675 LBF
REQUIRED	12629 LBF	13876 LBF

**IMPORTANT**

**JACK STUDS:** Use one jack stud girder support stud per 2500 lb load for SYP header and top plate. Calculate load at each end of the header by summing moments of truss reactions and dividing by header length. (EXAMPLE: 16' garage header with 24' truss span; 8 trusses x 1066 lb/truss / 2 = 4266 lb / 2500 lb/jack = 2 jacks)

**KING STUDS:** King studs = Header span / 4 for 10' stud; / 5 for 9' stud; / 6 for 8' stud. (EXAMPLE: 16' garage header in 10' wall; 16' / 4 = 4 king studs at each side of opening.)

**HEADER UPLIFT CONNECTIONS:** For uniform loads add up all the truss uplifts and divide by 2. For non-uniform loads sum moments of truss uplifts and divide by the header length. Select header connections from table or catalog to connect header to stud and stud to foundation. (EXAMPLE: 16' garage header with 21' truss span; 8 trusses x 360 lb/truss / 2 = 1440 lb; Top Connector, MSTA24, 18- 10d; Bottom Connector, DTT2Z.)

**CONTINUOUS LOAD PATH:** Builder is responsible to provide a continuous load path from roof to foundation. If the plan omits a connection, call the wind load engineer.

Chemneys Construction LLC

Spec Home - Lot 6 Fort White Station

PROJECT ADDRESS:  
Lot 6 Fort White Station  
Fort White, FL 32838

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

**COPYRIGHTS AND PROPERTY RIGHTS:**  
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53815

THIS PDF HAS DIGITAL SIGNATURE  
AND ELECTRONIC SEAL. PRINTED  
COPIES ARE NOT CONSIDERED  
SIGNED OR SEALED. YOU MUST  
VERIFY SIGNATURE ON THIS PDF.  
[CLICK HERE TO VERIFY.](#)



Friday, December 4, 2020

Mark Disosway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
201305

**S-3**

OF 3 SHEETS