		· •	y Building P	ei iiiit	PERMIT
APPLICANT LI	This Pe NDA RODER	rmit Expires One Y	Year From the Date PHONE		000026185
<u></u>	87 SW KEMP COU	RT	LAKE CTY	386.752.2281	EI 22024
	DAM'S FRAMING & CON		PHONE	386.623.2383	FL 32024
ADDRESS 24			LAKE CITY	300.023.2383	FL 32025
CONTRACTOR	ADAM PAPKA		PHONE	386.623.2383	<u> 32023</u>
LOCATION OF PI	ROPERTY 90-W T	O PINEMOUNT ROAD	TL TO BIRLEY AVENU		
		DOWN ON THE R.			
TYPE DEVELOPM	MENT SFD/UTILITY	E	STIMATED COST OF CO	ONSTRUCTION	71050.00
HEATED FLOOR	AREA 1421.00	TOTAL AR	REA 2022.00	HEIGHT 19.	.00 STORIES 1
FOUNDATION	CONC WA	LLS FRAMED	ROOF PITCH 6'12	FLO	OOR CONC
LAND USE & ZON	NING RSF-2		MA>	K. HEIGHT 35	
Minimum Set Back	Requirments: STREE	T-FRONT 25.00	0 REAR	15.00	SIDE 10.00
NO. EX.D.U.	FLOOD ZONE	<u>x</u>	DEVELOPMENT PER	MIT NO.	
PARCEL ID 08-	4S-16-02814-004	SUBDIVISIO	ON GERALD RIGGLE	E DEVELOPMENT	
LOT 2 BL	OCK PHASE	 UNIT		AL ACRES 0.55	
00000144		_		7) V/ /I	
000001442 Culvert Permit No.	Culvert Waiver	CBC1253409		she lall	9
18"X32'MITERED	06-0558-N	Contractor's License Nur BLK	200000000000000000000000000000000000000	Applicant/Owner/Co	ontractor
Driveway Connection		T 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	_	TH	N
	AT REQUIRES FINISH FL		ng checked by App	roved for Issuance	New Resident
· ·	IRMATION LETTER REQ				
				Check # or Cash	1417
50.05	FOR BI	III DING 8 ZONIA			1 1417
Temporary Power	TONBO	Foundation	NG DEPARTMENT		(footer/Slab)
-	date/app. by		date/app. by	Monolithic	date/app. by
Under slab rough-in p	olumbing	Slab		Sheathing/Nai	date/app. by
				Oncaumiz/ivai	ling
Framina	date/ap	p. by	date/app. by	Sheathing/Ivan	lingdate/app. by
Framingda	·		date/app. by		
	date/ap	Rough-in plumbing abo	pove slab and below wood	floor	
da Electrical rough-in	·		pove slab and below wood		date/app. by
da	ite/app. by	Rough-in plumbing about Heat & Air Duct C.O. Final	pove slab and below wood slab and below w	floor	date/app. by date/app. by
da Electrical rough-in Permanent power	date/app. by	Rough-in plumbing about Heat & Air Duct C.O. Final	pove slab and below wood	eri. beam (Lintel) Culvert	date/app. by
da Electrical rough-in Permanent power	date/app. by	Rough-in plumbing above the Air Duct C.O. Final date/app.	date/app. by ate/app. by	floor eri. beam (Lintel) Culvert Pool	date/app. by date/app. by date/app. by date/app. by
Determinent power M/H tie downs, blocking Reconnection	date/app. by	Rough-in plumbing above the Air Duct C.O. Final date/app. Pump pole	date/app. by ate/app. by Utility Pole	floor eri. beam (Lintel) Culvert Pool	date/app. by date/app. by
Electrical rough-in Permanent power M/H tie downs, blocking	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by	Rough-in plumbing above the Ro	date/app. by ate/app. by Utility Pole app. by	eri. beam (Lintel) Culvert Pool date/app. by Re-roof	date/app. by date/app. by date/app. by date/app. by
Electrical rough-in Permanent power M/H tie downs, blockin Reconnection M/H Pole date/app.	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by Trav	Rough-in plumbing above the Ro	date/app. by ate/app. by Utility Pole app. by te/app. by	eri. beam (Lintel) Culvert Pool date/app. by Re-roof	date/app. by date/app. by date/app. by date/app. by
Electrical rough-in Permanent power M/H tie downs, blockin Reconnection M/H Pole date/app.	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by Trav by EEE \$ 360.00	Rough-in plumbing above the second se	date/app. by ate/app. by Utility Pole app. by te/app. by	eri. beam (Lintel) Culvert Pool date/app. by Re-roof	date/app. by date/app. by date/app. by date/app. by date/app. by date/app. by
Description day and the second of the second	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by Trav by EEE \$ 360.00	Rough-in plumbing above the state of the sta	date/app. by Lility Pole app. by te/app. by \$ 10.11 FIRE FEE \$ 0.00	floor eri. beam (Lintel) Culvert Pool date/app. by Re-roof da SURCHARGE FEE WASTE FE	date/app. by date/app. by date/app. by date/app. by date/app. by te/app. by
Electrical rough-in Permanent power M/H tie downs, blocking Reconnection M/H Pole date/app. BUILDING PERMIT F MISC. FEES \$	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by Trav by EE\$ 360.00 ZONING O	Rough-in plumbing above the state of the sta	date/app. by ate/app. by Utility Pole app. by te/app. by 10.11	reri. beam (Lintel) Culvert Pool date/app. by Re-roof da SURCHARGE FEE WASTE FE	date/app. by date/app. by date/app. by date/app. by date/app. by te/app. by
Electrical rough-in Permanent power M/H tie downs, blocking Reconnection M/H Pole date/app. BUILDING PERMIT F MISC. FEES \$	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by Trav by EE \$ 360.00 ZONING O	Rough-in plumbing above the state of the sta	date/app. by ate/app. by Utility Pole app. by te/app. by \$ 10.11 FIRE FEE \$ 0.00 CULVERT FEE \$ 25	reri. beam (Lintel) Culvert Pool date/app. by Re-roof da SURCHARGE FEE WASTE FE	date/app. by date/app. by date/app. by date/app. by date/app. by te/app. by te/app. by FEE 480.22
Description date description d	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by Trav by ZEE \$ 360.00 NT FEE \$ FILOCO NT O THE REQUIREMENTS CONTROLLED TO THE REQUIREMENTS CONTRO	Rough-in plumbing above the state of the sta	date/app. by ate/app. by Utility Pole app. by te/app. by \$ 10.11 FIRE FEE \$ 0.00 CULVERT FEE \$ 25 CLERKS OFFICE	reri. beam (Lintel) Culvert Pool date/app. by Re-roof da SURCHARGE FEE WASTE FE	date/app. by date/app. by date/app. by date/app. by date/app. by te/app. by FEE 480.22
Electrical rough-in Permanent power M/H tie downs, blockin Reconnection M/H Pole date/app. BUILDING PERMIT F MISC. FEES \$	date/app. by date/app. by date/app. by ng, electricity and plumbing date/app. by Trav by EE \$ 360.00 ZONING O	Rough-in plumbing above the seconds of this permit, there is considered as water management.	date/app. by ate/app. by Utility Pole app. by te/app. by \$ 10.11 FIRE FEE \$ 0.00 CULVERT FEE \$ 25 CLERKS OFFICE MAY BE ADDITIONAL REST NTY. AND THERE MAY BE NT DISTRICTS, STATE AGE	rictions applicate Additional Perminders, or Federal	date/app. by date/app. by date/app. by date/app. by date/app. by te/app. by E\$ 10.11 E\$ \$ FEE 480.22 BLE TO THIS IITS REQUIRED AGENCIES.

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Lot 2 Gerald Riggle

	ing remit Application
For OI lee Use Only Application # 0708-50 Date	Received 8/28/07By G Permit # 1442/2019
App cation Approved by - Zoning Official BLK Da	to 27.08.07 Plans Examiner OKTH Date 8-23-0
Fig d.Zone Development Permit _V/A Zon	ling RSF / Land Use Plan Man Culeanon Rec L. Oc
Con mante Plat Requires Finish Floor out 1/3	5 St Elevilan Consider Letter Don and
1 had / /mad = -	State Road Info a Parent Parcel # c Developm nt Perm
	2 7C7 77 27
Name Authorized Person Signing Permit Linds or Me	anie Coder Phone 752-2281
Address 387 SW Kempet Cake City FC	3.2024
Owners Name Adams Framing + Construc	tion Phone 623-2383
911 Address 2441 SW Birley Fre Lake CH	4 FL 32025
Contractors Name Aclaim Papka	Phone 623-2383
Address POB 1921 Lake City FL 30	1056
Fee Simple Owner Name & Address NA	
Bonding Co. Name & Address	
Architect/Engineer Name & Address Will Myers/	Mark Disosway
Mortgage Lenders Name & Address	,
Circle the correct power company - FL Power & Light - Clay I	Since of Suprempose Walley Floor - Pro tracelyo En any
00 15 1	
Subdivision Name Gerald (2996 Suppliers)	stimated Cost of Construction
Driving Directions 90 W, Turn Lon Pinemou	
Lots gre I mile down on R	
Type of Construction SFD Nu	omber of Edsting Dwellings on Properl
	rt Permit or Culvert Walver or Have in Existing vive
Actual Distance of Structure from Property Lines - Front 548	
	edted Floor Area 142 P Roof P ch 6-12
	101AL 2022
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuance of a permit and	tallations as Indicated. I certify that no work or
all laws regulating construction in this jurisdiction.	ring an work pe benotined to meet a 3 statuald of
OWNERS AFFICAVIT: I hereby certify that all the foregoing inform	nation is accurate and all work will be done in
compliance with all applicable laws and regulating construction a	•
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTE	ND TO OBTAIN FINANCING, CONSULT WITH YOUR
LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE O	F COMMENCEMENT
Owner Builder or Agent (Including Contractor)	Contractor Signature Contractors License Number CBC 1253409
STATE OF FLORIDA Linda R. Roder Commission #DD303275	
COUNTY OF CCLUMBIA Expires: Mar 24, 2008 Bonded Thru	
Sworn to (or affirmed) and subscribed before mentic Bonding Co., Inc.	1 0 Widolla
this day of 20	(The hallow
Personally known or Produced Identification	Notary Signature
	Notary Signature

JW ADVISED LINEA 8.28.07

Columbia County Building Department Culvert Permit

Culvert Permit No. 000001442

DATE $08/2$	29/2007 PARCEL ID # 08-4	S-16-02814-004	
APPLICANT	LINDA RODER	PHONE	386.752.2281
ADDRESS _	387 SW KEMP COURT	LAKE CTY	FL 32024
OWNER A	DAM'S FRAMING & CONSTRUCTION,LLC	PHONE	386.623.2383
ADDRESS 2	441 SW BIRLEY AVENUE	LAKE CITY	FL 32025
CONTRACTO	OR ADAM PAPKA	PHONE	386.623.2383
LOCATION O	OF PROPERTY 90-W TO PINEMOUNT ROAD,T	L TO BIRLEY AVENU	JE,TL AND IT'S
1 MILE DOWN C	ON TH R.		
X	INSTALLATION REQUIREMENTS Culvert size will be 18 inches in diameter wit driving surface. Both ends will be mitered 4 thick reinforced concrete slab. INSTALLATION NOTE: Turnouts will be recally a majority of the current and existing drives by the driveway to be served will be paved Turnouts shall be concrete or paved a majority of the current or paved and concrete or paved driveway, whichever is current and existing paved or concreted.	quired as follows: veway turnouts are or formed with con- inimum of 12 feet v s greater. The width turnouts.	pe and poured with a 4 inch paved, or; crete. wide or the width of the n shall conform to the
	Culvert installation shall conform to the app	roved site plan star	ndards.
	Department of Transportation Permit install	ation approved star	ndards.
	Other		

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21

Phone: 386-758-1008 Fax: 386-758-2160

Lake City, FL 32055

Amount Paid 25.00



Prepared by: Elaine R. Davis / Megan Marable American Title Services of Lake City, Inc. 321 SW Main Boulevard, Suite 105 Lake City, Florida 32025

File Number: 07-220

Inst:200712012877 Date:6/12/2007 Time:1:11 PM
Doc Stamp-Deed:234.50
DC,P.DeWitt Cason ,Columbia County Page 1 of 1

Warranty Deed

Made this June 8, 2007 A.D.

By Aaron Simque and Mark A. Cook, Post Office Box 2695, Lake City, Florida 32056, hereinafter called the grantor,

to Adam's Framing and Construction, LLC, whose post office address is: 691 SW Sisters Welcome Road, Lake City, Florida 32025, hereinafter called the grantee:

(Whenever us d herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth, that the grantor, for and in consideration of the sum of Ten Dollars, (\$10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

LOT 2 of GEF ALD RIGGLE DEVELOPMENT, a subdivision according to the Plat thereof as recorded in Plat Book 8 Page 67, Public Records of Columbia County, Florida.

SUBJECT TO A NON EXCLUSIVE PERPETUAL EASEMENT FOR INGRESS AND EGRESS OVER AND ACROSS THE SOUTH 15 FEET.

Said property is no the homestead of the Grantor(s) under the laws and constitution of the State of Florida in that neither Grantor(s) or any members of the household of Grantor(s) reside thereon.

Parcel II > Number: 02814-001

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

MEGAN M MARABLE

MY COMMISSION 4 DD412865

-1 tombifelite the \$44 to NFRISH

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31, 2006.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Man Mark A. Cook

Witness Printed Name

Megan Marable

Mark A. Cook

Address:

State of Florida

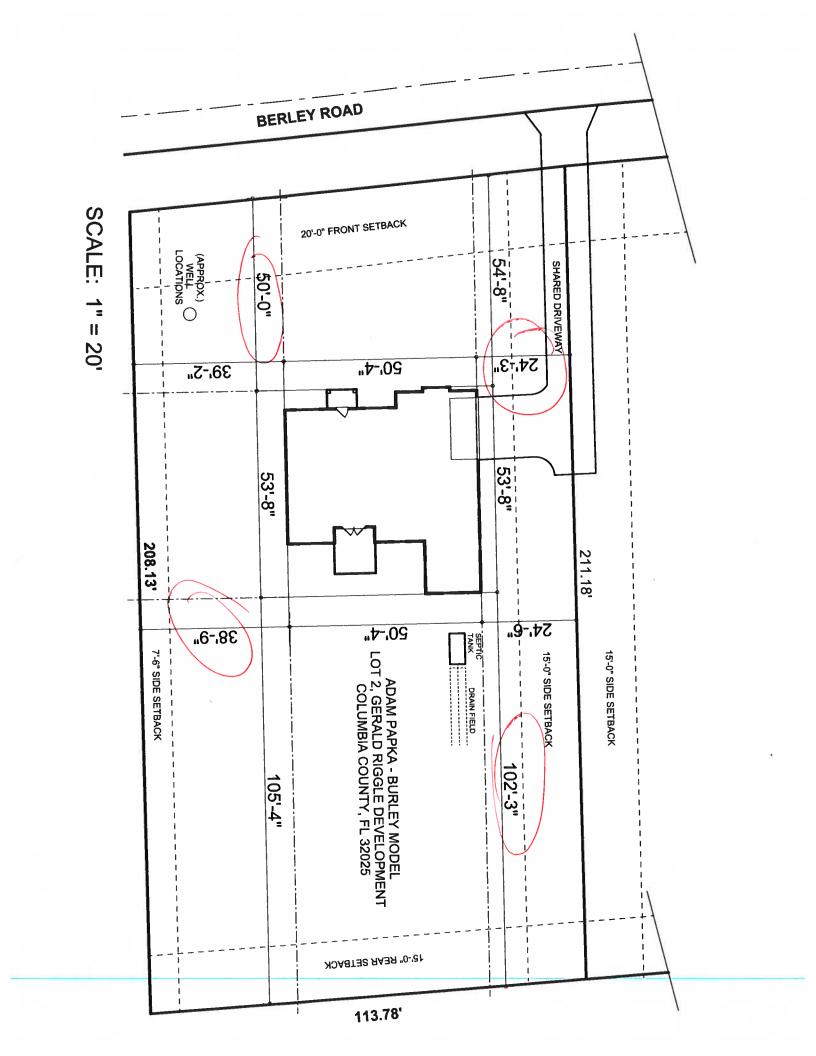
County of Coll mha

The foregoing inst ument was acknowledged before me this 8th day of June, 2007, by Aaron Simque and Mark A. Cook, who is/are personally issues as identification.

Mark A. Cook, who is/are personally issues as identification.

Notary Public Print Name:

DEED Individual Warre Closers' Choice



FILE

HALL'S FIND & WELL SERVICE, INC.

PECIALIZING IN 4" O' WELLS



DOWN PARTY HALL

PHONE (00) 788-78

FAX (et 9 788-78

LAGE ETV. 158-58

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity disphram tank on all new wells. This will insure a minimum of one (1) minute draw down of one (1) minute refill. If a smaller disphram tank is used then we will install a cycle stop walve which will produce the same results.

If you have any quastions please feel free to call our office anytime.

Thank, you,

Donald D. Hall

DDE 11A

0708-50

nst. Number: 200712018454 Book: 1128 Page: 563 Date: 8/14/2007 Time: 3:38:13 PM

Return To: Eddie Anderson

THIS INSTRUMENT PREPARED BY & RETURN TO: Columbia Bank 173 NW Hillsboro Street Lake City, FL 32055

Inet 200712018454 Cale: 8/14/2007 Time: 3:38 PM 54 S-DC, P. DeWitt Cason , Columbia County Pag. 1 of 1

NOTICE OF COMMENCEMENT

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accondance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commenc ment:

1. Description of Property:

Lot 2 of Gerald Riggle Development a subdivision according to the Plat thereof as recorded in PB 8 Page 67, Tax Parcel # 08 45-16-02814-004 of the Public Records of Columbia County, Florid. .

2. General Description of Improvements:

Construction of a single family dwelling.

3. Owner Information:

Adam's Framing and construction, LLC P O Box 1921

Lake City, Fl 32056 Phone: 386-752-4202

Owner's Interest in Property:

Fee Simple

4. Contractor:

Adam's Framing and construction, LLC 429 SW Meadow Terrace

Lake City, Fl 32024 Phone: 386-752-4202

5, Lender:

Columbia Bank 173 NW Hillsborg Street Lake City, FL 32055

6. Additional persons within the State of Florida designated by Owner upon whom notices or other documents hay be served as provided by Suction 713.13(1)(a)7., Florida Statutes:

7. Expiration date of Notice of Commencement is one (1) year from the date of recording.

Adam's Framing and Construction, LLC

Adam Papka, Manager

STATE OF FLORIDA COUNTY OF Columbia

> The foregoing instrument was acknowledged before me this 8th day of June, 2007 by Adam Papka, Manager of Adam's Framing and Construction, LLC

NNCE KANN CONCERTS
OF COMMISSION PRESSING STATE OF Florida at Large
EXPRES ANTI, 2010 Personally Known:

duced Identification:

Type:

(NOC)

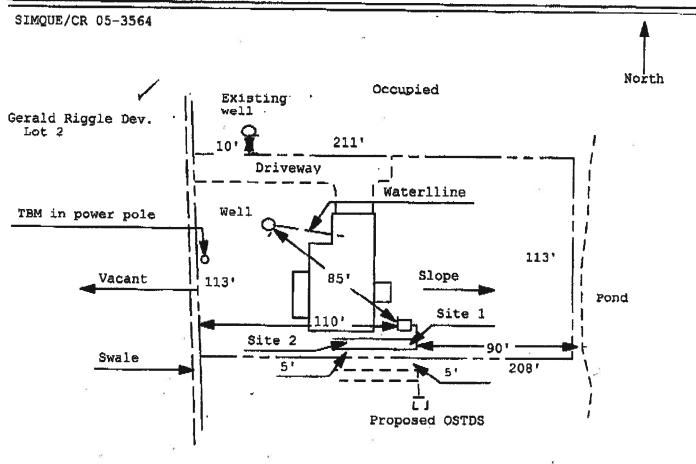
My Commission Expires:

LUT L Lot 2 Gerald Regile

0703-50

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 06-05581

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



	$-\Omega$	1 inch = 50 feet
Site Plan Submitted By Plan Approved Not A	pproved Date	Date 6/8/06
By Salli Sha	ldz. ESII	СРНО
Notes:		
in the second se	Ü	Olumbia CHD

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: Adam Papka Burley Road Lake City, FL 32 Owner: Climate Zone: Adam Papka Burley Road Lake City, FL 32 Burley Road Specification North		Builder: Permitting Office: Permit Number: Jurisdiction Number:	Adam's Framing & Con
New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 32.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	_
5. Is this a worst case?	No	<u></u>	
6. Conditioned floor area (ft²)	1421 ft²	c. N/A	P
7. Glass type 1 and area: (Label reqd. by 13-1	04.4.5 if not default)		<u> </u>
	escription Area	13. Heating systems	
(or Single or Double DEFAULT) 7a.(Dt b. SHGC:	ole Default) 249.0 ft ²	a. Electric Heat Pump	Cap: 32.0 kBtu/hr HSPF: 7.70
(or Clear or Tint DEFAULT) 7b.	(Clear) 249.0 ft ²	b. N/A	2 <u></u>
8. Floor types			2 <u></u>
a. Slab-On-Grade Edge Insulation	R=5.0, 178.0(p) ft	c. N/A	_
b. N/A		_	_
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 80.0 gallons
a. Frame, Wood, Exterior	R=13.0, 1090.0 ft ²	- 1	EF: 0.90
b. Frame, Wood, Adjacent	$R=13.0, 223.0 \text{ ft}^2$	b. N/A	_
c. N/A		_	-
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar	
10. Ceiling types		DHP-Dedicated heat pump)	
a. Under Attic	R=30.0, 1500.0 ft ²	15. HVAC credits	PT,
b. N/A		(CF-Ceiling fan, CV-Cross ventilation	on,
c. N/A		HF-Whole house fan,	
11. Ducts(Leak Free)	0 0 0 0 0 0	PT-Programmable Thermostat,	
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 40.0 ft	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
Glass/Floor Area: 0.18 I hereby certify that the plans and specific this calculation are in compliance with the	Total base	Review of the plans and specifications covered by this	OF THE STATE
PREPARED BY: 8-20-	>	calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for	A TURNOR THE TOTAL PROPERTY OF THE TOTAL PRO
I hereby certify that this building, as designified with the Florida Energy Code.	ned, is in complian	compliance with Section 553.908 Florida Statutes.	

BUILDING OFFICIAL:

DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Burley Road, Lake City, FL, 32024- PERMIT #:

BASE		AS-BU	IILT	"	
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area	•	verhang nt Len Hgt	Area X SPM	X SOF	= Points
.18 1421.0 18.59 4755.0	1.Double, Clear 2.Double, Clear 3.Double, Clear 4.Double, Clear 5.Double, Clear 6.Double, Clear 7.Double, Clear 8.Double, Clear 8.Double, Clear	W 1.5 9. W 1.5 9. N 1.5 9. N 1.5 9. N 1.5 9. E 1.5 9. E 1.5 9. S 1.5 9.	0 90.0 38. 0 40.0 38. 0 36.0 19. 0 9.0 19. 0 4.0 19. 0 36.0 42. 0 18.0 42.	52 0.97 52 0.97 20 0.98 20 0.98 20 0.98 06 0.97 06 0.97	3364.0 1495.0 674.0 168.0 74.0 1468.0 734.0 541.0
WALL TYPES Area X BSPM = Points	Туре	R-Valu		SPM =	Points
Adjacent 223.0 0.70 156.1 Exterior 1090.0 1.70 1853.0	Frame, Wood, Exterior Frame, Wood, Adjacent	13.0 13.0		1.50 0.60	1635.0 133.8
Base Total: 1313.0 2009.1	As-Built Total:		1313.0		1768.8
DOOR TYPES Area X BSPM = Points	Туре		Area X	SPM =	Points
Adjacent 20.0 2.40 48.0 Exterior 20.0 6.10 122.0	1.Exterior Insulated 2.Adjacent Insulated			4.10 1.60	82.0 32.0
Base Total: 40.0 170.0	As-Built Total:		40.0		114.0
CEILING TYPES Area X BSPM = Points	Туре	R-Value	Area X SPM	X SCM =	Points
Under Attic 1421.0 1.73 2458.3	1. Under Attic	30.0	1500.0 1.73 X	1.00	2595.0
Base Total: 1421.0 2458.3	As-Built Total:		1500.0	<u></u>	2595.0
FLOOR TYPES Area X BSPM = Points	Туре	R-Valu	e Area X	SPM =	Points
Slab 178.0(p) -37.0 -6586.0 Raised 0.0 0.00 0.00	Slab-On-Grade Edge Insulation	5.0	178.0(p -3	6.20	-6443.6
Base Total: -6586.0	As-Built Total:		178.0		-6443.6
INFILTRATION Area X BSPM = Points			Area X	SPM =	Points
1421.0 10.21 14508.4			1421.0	10.21	14508.4

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Burley Road, Lake City, FL, 32024- PERMIT #:

	BASE		AS-BUILT								
Summer Ba	se Points: 1	7314.8	Summer As-Built Points: 21060.								
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)								
17314.8	0.3250	5627.3	(sys 1: Central Unit 32000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS) 21061 1.00 (1.09 x 1.000 x 1.00) 0.260 0.950 5670.1 21060.6 1.00 1.090 0.260 0.950 5670.1								

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Burley Road, Lake City, FL, 32024-

PERMIT #:

	BASE					AS-	BUI	LT				
GLASS TYPES .18 X Conditio Floor Ar		WPM =	Points	Type/SC		rhang Len	Hgt	Area X	WI	PM X	WOI	= Points
.18 1421	.0	20.17	5159.0	1.Double, Clear 2.Double, Clear 3.Double, Clear	W W	1.5 1.5 1.5	9.0 9.0 9.0	90.0 40.0 36.0	2	20.73 20.73 24.58	1.01 1.01 1.00	1880.0 835.0 885.0
				4.Double, Clear 5.Double, Clear 6.Double, Clear	N N E	1.5 1.5 1.5	9.0 9.0 9.0	9.0 4.0 36.0	2	24.58 24.58 8.79	1.00 1.00 1.02	221.0 98.0 687.0
				7.Double, Clear 8.Double, Clear	E S	1.5 1.5	9.0 9.0	18.0 16.0	1	8.79 3.30	1.02 1.02	343.0 217.0
				As-Built Total:				249.0				5166.0
WALL TYPES	Area X	BWPM	= Points	Туре		R-	√alue	Area	Х	WPM	1 =	Points
Adjacent Exterior	223.0 1090.0	3.60 3.70	802.8 4033.0	Frame, Wood, Exterior Frame, Wood, Adjacent			13.0 13.0	1090.0 223.0		3.40 3.30		3706.0 735.9
Base Total:	1313.0		4835.8	As-Built Total:				1313.0				4441.9
DOOR TYPES	Area X	BWPM	= Points	Туре	·	•		Area	Х	WPN	1 =	Points
Adjacent Exterior	20.0 20.0	11.50 12.30	230.0 246.0	Exterior Insulated Adjacent Insulated				20.0 20.0		8.40 8.00		168.0 160.0
Base Total:	40.0		476.0	As-Built Total:				40.0				328.0
CEILING TYPES	S Area X	BWPM	= Points	Туре	R-	Value	Ar	ea X W	PM	x wc	:M =	Points
Under Attic	1421.0	2.05	2913.1	1. Under Attic		;	30.0	1500.0	2.05	X 1.00		3075.0
Base Total:	1421.0		2913.1	As-Built Total:				1500.0				3075.0
FLOOR TYPES	Area X	BWPM	= Points	Туре		R-\	/alue	Area	X	WPN	l =	Points
Slab Raised	178.0(p) 0.0	8.9 0.00	1584.2 0.0	1. Slab-On-Grade Edge Insu	lation		5.0	178.0(p		7.60		1352.8
Base Total:			1584.2	As-Built Total:				178.0				1352.8
INFILTRATION	Area X	BWPM	= Points					Area	Χ	WPM	1 =	Points
	1421.0	-0.59	-838.4					1421.0	0	-0.59		-838.4

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Burley Road, Lake City, FL, 32024- PERMIT #:

	BASE		AS-BUILT								
Winter Base	Points:	14129.7	Winter As-Built Points: 13525.3								
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)								
14129.7	0.5540	7827.8	(sys 1: Electric Heat Pump 32000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 13525.3 1.000 (1.069 x 1.000 x 1.00) 0.443 0.950 6082.9 13525.3 1.00 1.069 0.443 0.950 6082.9								

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Burley Road, Lake City, FL, 32024- PERMIT #:

BASE					AS-BUILT								
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	x	Tank X Ratio	Multiplier	X Credit Multipli		Total
3		2635.00		7905.0	80.0	0.90	3		1.00	2693.56	1.00		8080.7
					As-Built To	otal:							8080.7

	CODE COMPLIANCE STATUS												
	BASE						AS-BUILT						
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
5627 7828 7905 21360 5670 6083 8081										19834			

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Burley Road, Lake City, FL, 32024-

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall;	
		foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility	
		penetrations; between wall panels & top/bottom plates; between walls and floor.	
		EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends	
		from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.	
		EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed	
		to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases,	
		soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate;	
		attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is	
		installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a	
		sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from	
		conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA,	
		have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK	
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.		
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.		
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.		
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.		
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.		
		Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.		

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.6

The higher the score, the more efficient the home.

Burley Road Spec, Burley Road, Lake City, FL, 32024-

1.	Now construction or opicting	Mann	12	C1'		
2.	New construction or existing Single family or multi-family	New Single family		Cooling systems Central Unit	Cap: 32.0 kBtu/hr	
3.	Number of units, if multi-family	Single failing	a.	Central Ollit	SEER: 13.00	
4.	Number of Bedrooms	3	_ h	N/A	SEEK. 15.00	_
5.	Is this a worst case?	No		N/A		_
6.	Conditioned floor area (ft²)	1421 ft ²		N/A		-
7.	Glass type 1 and area: (Label reqd. b			IVA		3. 3
	U-factor:	Description Area	13	Heating systems		
•••	(or Single or Double DEFAULT)	7a (Dbla Dafault) 240 0 ft2		Electric Heat Pump	Cap: 32.0 kBtu/hr	
b.	SHGC:	rus (Doie Delauit) 249.0 it		Diodate fleat i dinp	HSPF: 7.70	3. 7 - 3 3
٠.	(or Clear or Tint DEFAULT)	7b. (Clear) 249.0 ft ²	h	N/A	11011.7.70	3==0
8.	Floor types	(Cicar) 247.0 it				
a.	Slab-On-Grade Edge Insulation	R=5.0, 178.0(p) ft	c.	N/A		
	N/A	тото, тото (р) то				
c.	N/A		14.	Hot water systems		
9.	Wall types			Electric Resistance	Cap: 80.0 gallons	
a.	Frame, Wood, Exterior	R=13.0, 1090.0 ft ²			EF: 0.90	
b.	Frame, Wood, Adjacent	R=13.0, 223.0 ft ²	— b.	N/A		
c.	N/A	·				
d.	N/A		c.	Conservation credits		
e.	N/A			(HR-Heat recovery, Solar		
10.	Ceiling types			DHP-Dedicated heat pump)		
a.	Under Attic	R=30.0, 1500.0 ft ²	15.	HVAC credits	PT,	
b.	N/A		_	(CF-Ceiling fan, CV-Cross ventilation	•	
c.	N/A		_	HF-Whole house fan,		
	Ducts(Leak Free)			PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 40.0 ft	_	MZ-C-Multizone cooling,		
b.	N/A		_	MZ-H-Multizone heating)		
Con in th base	rtify that this home has complie istruction through the above end is home before final inspection ed on installed Code compliant ider Signature:	ergy saving features which a. Otherwise, a new EPL I features.	h will be ins Display Car	stalled (or exceeded)	OF THE STATE	RIORID,
_ ~		-				
Add	lress of New Home:		City/FL Zi	ip:	GOD WE TRUST	A
*NC	OTE: The home's estimated ener	rgy performance score is	only availa	ble through the FLA/RES comput	ter program.	
				86 for a US EPA/DOE EnergySta		
				s if you obtain a Florida Energy (
				Gauge web site at www.fsec.ucf.ed		

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

Construction, contact the Department of Community Affairs at 850/487-1824.

information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building

Energy Code Compliance

Duct System Performance Report

Project Name:

Adam Papka

Burley Road

Address: City, State: Owner:

Lake City, FL 32024-Burley Road Spec

Climate Zone:

North

Builder:

Adam's Framing & Const.

Permitting Office: Permit Number:

Jurisdiction Number:

Total Duct System Leakage Test Results

CFM25 Total Duct Leakage Test Values						
Line	System	Duct Leakage Total	Duct Leakage to Outdoors			
1	System1	cfm25(tot)	cfm25(out)			
2	System2	cfm25(tot)	cfm25(out)			
3	System3	cfm25(tot)	cfm25(out)			
4	System4	cfm25(tot)	cfm25(out)			
5	Total House Duct System Leakage	Sum lines 1-4 Divide by (Total Conditioned Floor Area) =(Q _n ,tot) Receive credit if Q _n ,tot≤ 0.03	Sum lines 1-4 Divide by (Total Conditioned Floor Area) =(Q _n ,out) Receive credit if Q _n ,out≤ 0.03 AND Q _n ,tot≤ 0.09			

I hereby certify that the above duct testing performance results demonstrate compliance with the Florida Energy Code requirements in accordance with Section 610.1.A.1, Florida Building Code, Building Volume, Chapter 13 for leak free duct system credit.

Signature: _____
Printed Name:

Florida Rater Certification #:

DATE:

Florida Building Code requires that testing to confirm leak free duct systems be performed by a Class 1 Florida Energy Gauge Certified Energy Rater. Certified Florida Class 1 raters can be found at: http://energygauge.com/search.htp



BUILDING OFFICIAL: _

DATE:



COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 08-4S-16-02814-004

Building permit No. 000026185

57.78

Fire:

Use Classification SFD/UTILITY

Permit Holder ADAM PAPKA

Waste: 150.75

208.53

Building Inspector

Location: 2441 SW BIRLEY AVE, LAKE CITY, FL

Owner of Building ADAM'S FRAMING & CONSTRUCTION, LLC Total:

Date: 01/29/2008

POST IN A CONSPICUOUS PLACE Business Places Only Project Information for: L244941

Builder: ADAM'S FRAMING

Lot: Subdivision: 441 SW BURLEY AVE.

7/31/2007 Start Number: SEI Ref: L244941

146

Date:

County or City: **COLUMBIA COUNTY**

Truss Page Count: 37

Truss Design Load Information (UNO) Design Program: MiTek

Gravity Wind **Building Code:** FBC2004

Roof (psf): 42 Wind Standard: **ASCE 7-02** 55 Wind Speed (mph): Floor (psf): 110

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

PAPKA, ADAM R. CBC1253409

P.O. BOX 1921 Address:

> LAKE CITY, FL 32056 Designer:

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987

Structural Engineering and Inspections, Inc. EB 9196 Company:

Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

- 2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI

 3. The seal date shown on the individual truss component drawings and account of the Structure is the responsibility of the Building Designer of the Structure is the struc

4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component squitting. Designer shall verify the suitablity and use of Carrying Member hanger capacity.

	·	and doe or carrying mon			and the same of th	T-0-0	64604
				-			VAL EN
#	Truss ID	Dwg.#	Seal Date	#	Truss ID	Dwg.#	Seal Date
1	CJ1	0731071003	7/31/2007				
2	CJ3	0731071004	7/31/2007				
3	CJ5	0731071005	7/31/2007				
4	EJ2	0731071006	7/31/2007				
5	EJ7	0731071007	7/31/2007				
6	EJ7A	0731071008	7/31/2007				
7	EJ7B	0731071009	7/31/2007				
8	HJ3	0731071010	7/31/2007				1
9	HJ9	0731071011	7/31/2007				
10	T01	0731071012	7/31/2007				
11	T02	0731071013	7/31/2007				1.7
12	T03	0731071014	7/31/2007				
13	T04	0731071015	7/31/2007				
14	T05	0731071016	7/31/2007				
15	T06	0731071017	7/31/2007				
16	T07	0731071018	7/31/2007				1
17	T08	0731071019	7/31/2007				1
18	T10	0731071020	7/31/2007				1
19	T11	0731071021	7/31/2007				
20	T12	0731071022	7/31/2007		 		
21	T13	0731071023	7/31/2007				
22	T14	0731071024	7/31/2007	-			1
23	T15	0731071025	7/31/2007				
24	T16	0731071026	7/31/2007		1		-
25	T17	0731071027	7/31/2007				· · · · · · · · · · · · · · · · · · ·
26	T18	0731071028	7/31/2007				ě.
27	T18G	0731071029	7/31/2007				
28	T19	0731071030	7/31/2007		1		
29	T20	0731071031	7/31/2007		† †		
30	T21	0731071032	7/31/2007				
31	T22	0731071033	7/31/2007				
32	T23	0731071034	7/31/2007		† · · · · · · · · · · · · · · · · · · ·		1
33	T24	0731071035	7/31/2007				
34	T25	0731071036	7/31/2007				1
35	T25G	0731071037	7/31/2007		 		
36	T26	0731071038	7/31/2007		† · · · · · · · · · · · · · · · · · · ·	•	
37	T27	0731071039	7/31/2007		†		
	1		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				
	1				 	<u> </u>	+

eterer Engineering and Engeletions, Inc. 10103 N. Plonda Ave. Se D Ediz, PE 33,149 Thomas Miliet, PE 306//, EB 9196 LATERAL TOE-NAIL DETAIL STITOENAIL Page 1 of 1 MiTek Industries, Chesieriela, MO NOTES: 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN. 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD. 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIE FOR MEMBERS OF DIFFERENT SPECIES. TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) SQUARE CUT SYP DIAM 83.3 .131 SIDE VIEW (2x4, 2x6) 89.0 .135 (2x3) 3 NAILS 2 NAILS 118.3 .162 ູ້ດ NEAR SIDE 1 (7) FAR SIDE LONG 80.5 128 NEAR SIDE 83.3 .131 102.1 .148 100 LONG 70.5 120 80.5 .128 83.3 3.0" .131 30.00° 102.1 .148 VALUES SHOWN ARE CAPACITY PER TOE-NAIL APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED. L/3 45 DEGREE ANGLE This detail may only be applied to Pre-BEVEL CUT engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc. SIDE VIEW (2x3, 2x4) 2 NAILS NEAR SIDE

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer.



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Licensee Information

Name:

PAPKA, ADAM RUSSELL (Primary Name)

ADAM S FRAMING AND CONSTRUCTION LLC (DBA

Name)

Main Address: PO BOX 1921

LAKE CITY Florida 32056

COLUMBIA

License Mailing:

County:

LicenseLocation:

erm Glossary

Online Help

License Information

License Type:

Certified Building Contractor

Rank:

Cert Building

License Number:

CBC1253409

Status:

Current, Active

Licensure Date:

09/26/2005

Expires:

08/31/2008

Special Qualifications

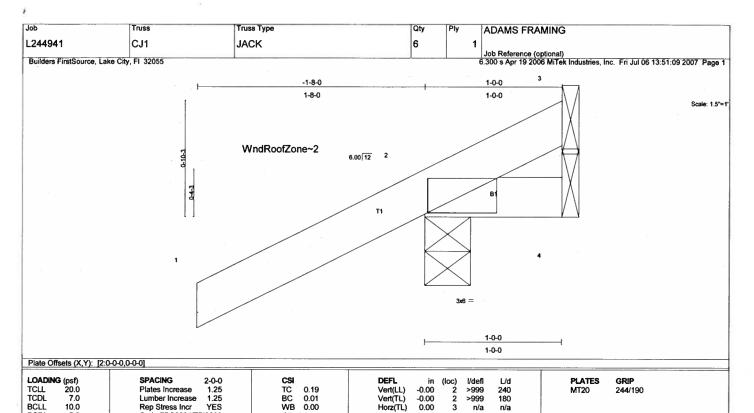
Qualification Effective

Qualified Business

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BCDL LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD

n/a

n/a

Structural wood sheathing directly applied or 1-0-0 oc purlins, Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 6 lb

BOT CHORD

REACTIONS (lb/size) 2=214/0-4-0, 4=14/Mechanical, 3=-56/Mechanical Max Horz 2=76(load case 5)

Max Uplift2=-210(load case 5), 3=-56(load case 1)
Max Grav 2=214(load case 1), 4=14(load case 1), 3=81(load case 5)

Code FBC2004/TPI2002

YES

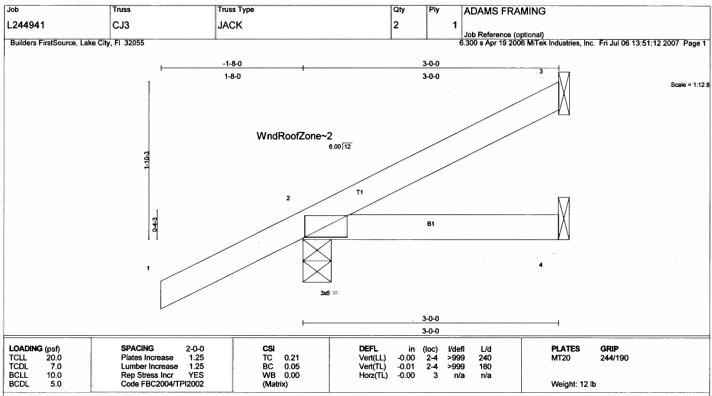
FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/39, 2-3=-53/47 BOT CHORD 2-4=0/0

JOINT STRESS INDEX 2 = 0.11

NOTES 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 2 and 56 lb uplift at joint 3.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (Ib/size) 3=42/Mechanical, 2=248/0-4-0, 4=42/Mechanical Max Horz 2=121(load case 5) Max Uplift3=-33(load case 6), 2=-169(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/39, 2-3=-51/13 BOT CHORD 2-4=0/0

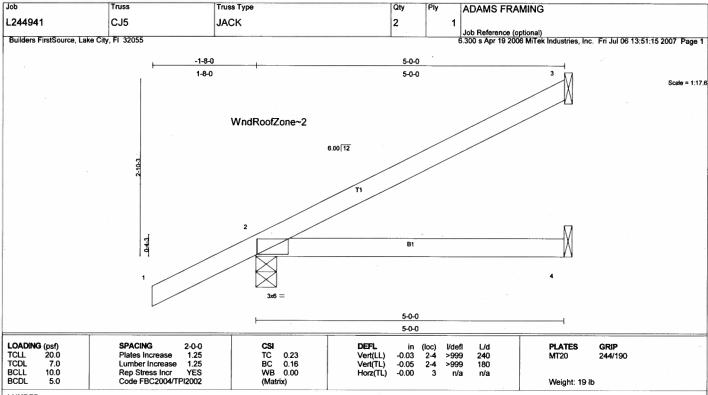
JOINT STRESS INDEX

2 = 0.11

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 169 lb uplift at joint 2.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD**

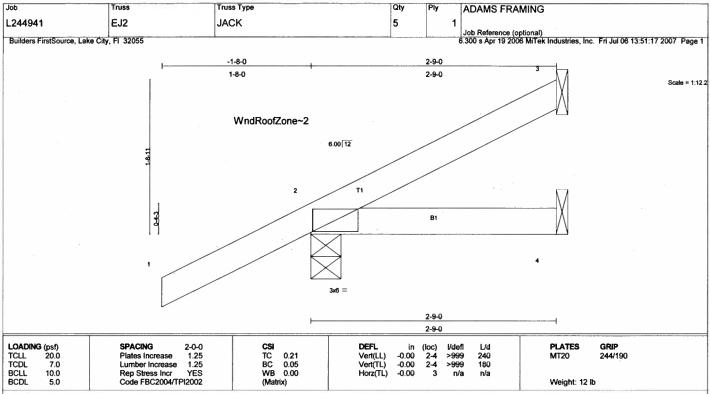
REACTIONS (lb/size) 3=110/Mechanical, 2=318/0-4-0, 4=72/Mechanical Max Horz 2=167(load case 5) Max Uplift3=-96(load case 5), 2=-172(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/39, 2-3=-93/39 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 3 and 172 lb uplift at joint 2.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-9-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=32/Mechanical, 2=241/0-4-0, 4=38/Mechanical Max Horz 2=115(load case 5)
Max Uplift3=-27(load case 6), 2=-171(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/39, 2-3=-50/9 BOT CHORD 2-4=0/0

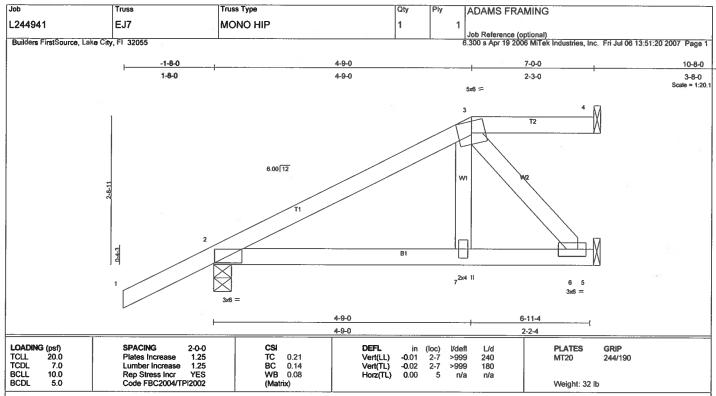
JOINT STRESS INDEX

2 = 0.11

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3 and 171 lb uplift at joint 2.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=24/Mechanical, 2=397/0-4-0, 5=247/Mechanical Max Horz 2=163(load case 5)
Max Uplift4=-37(load case 3), 2=-202(load case 5), 5=-84(load case 5)
Max Grav 4=45(load case 10), 2=397(load case 1), 5=247(load case 1)

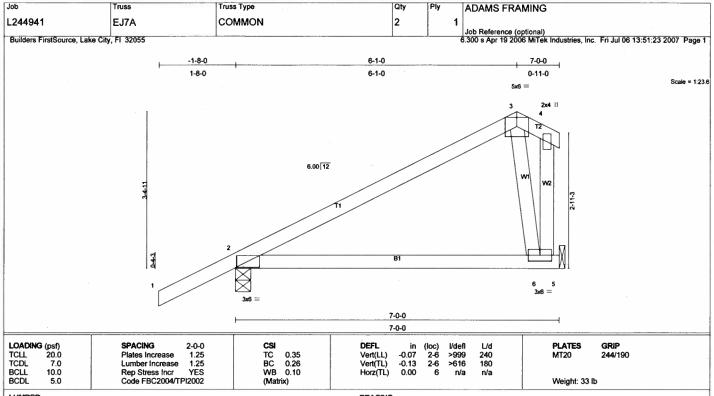
FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/39, 2-3=-306/41, 3-4=-0/0 BOT CHORD 2-7=-77/219, 6-7=-78/210, 5-6=0/0

WEBS 3-7=0/158, 3-6=-334/124

JOINT STRESS INDEX

2 = 0.21, 3 = 0.16, 6 = 0.16 and 7 = 0.11

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 4, 202 lb uplift at joint 2 and 84 lb uplift at joint



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=382/0-4-0, 6=276/Mechanical Max Horz 2=183(load case 5)

Max Uplift2=-193(load case 5), 6=-107(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/39, 2-3=-157/17, 3-4=-69/119 BOT CHORD 2-6=-57/84, 5-6=0/0 WEBS 3-6=-438/301, 4-6=-174/271

JOINT STRESS INDEX

2 = 0.51, 3 = 0.65, 4 = 0.19 and 6 = 0.18

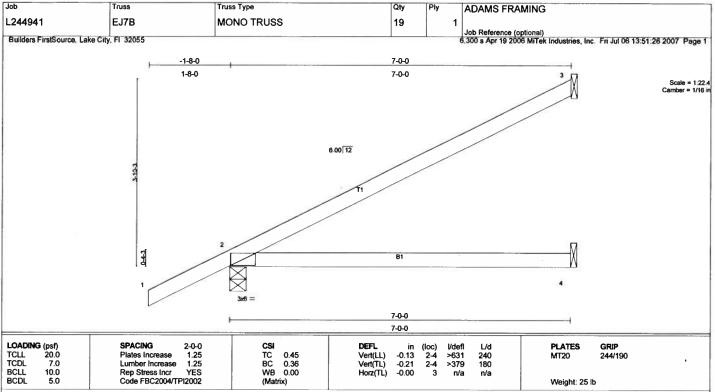
NOTES

- 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2 and 107 lb uplift at joint 6.



LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=164/Mechanical, 2=397/0-4-0, 4=107/Mechanical Max Horz 2=213(load case 5)
Max Uplift3=-137(load case 5), 2=-185(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/39, 2-3=-93/59 BOT CHORD 2-4=0/0

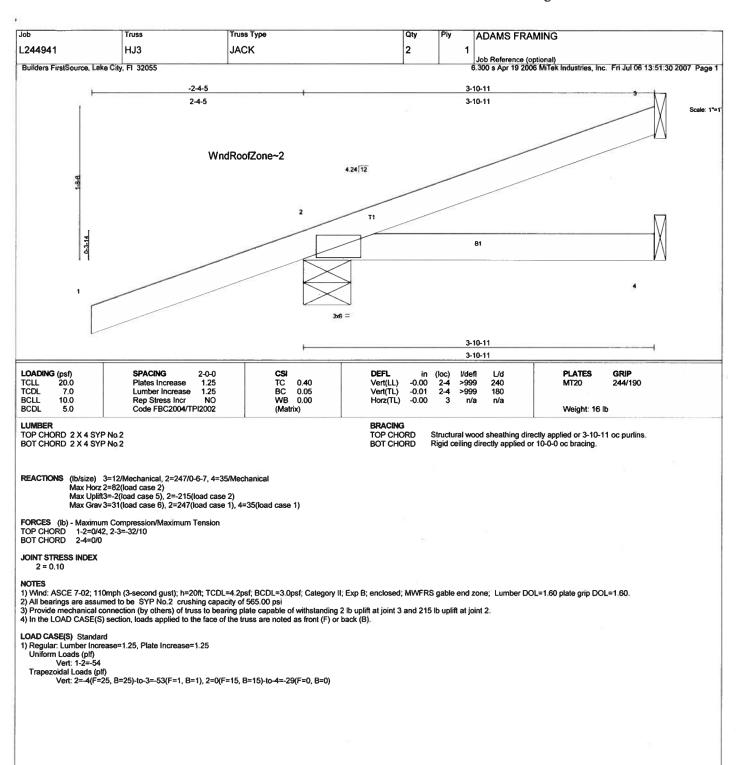
JOINT STRESS INDEX

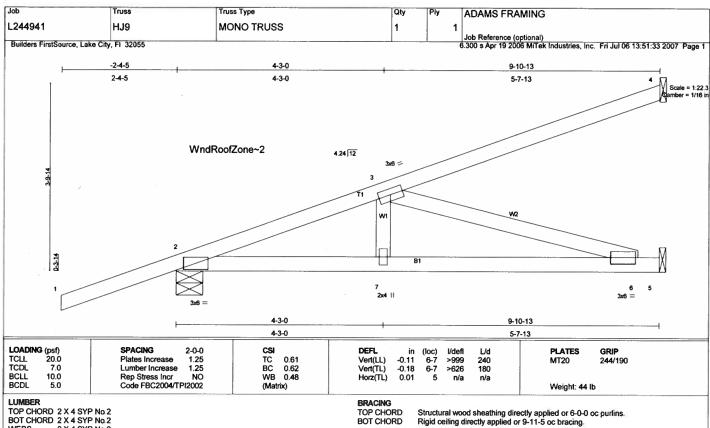
2 = 0.62

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 3 and 185 lb uplift at joint 2.





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 4=269/Mechanical, 2=504/0-6-7, 5=381/Mechanical Max Horz 2=259(load case 2) Max Uplift4=-230(load case 2), 2=-249(load case 2), 5=-71(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/42, 2-3=-913/159, 3-4=-104/65 BOT CHORD WEBS 2-7=-348/847, 6-7=-348/847, 5-6=0/0 3-7=0/200, 3-6=-883/363

JOINT STRESS INDEX

2 = 0.73, 3 = 0.23, 6 = 0.25 and 7 = 0.15

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.

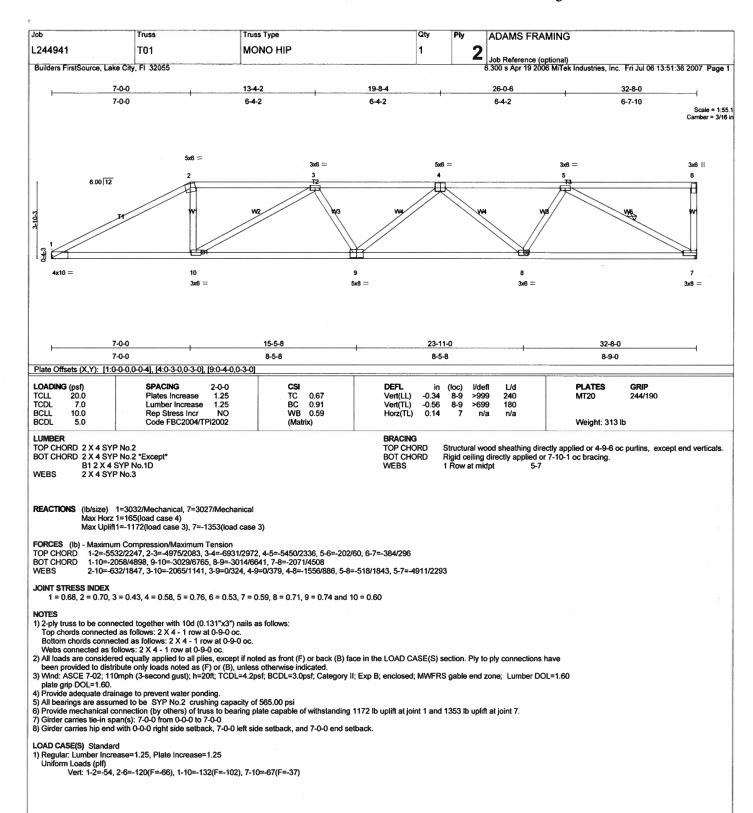
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

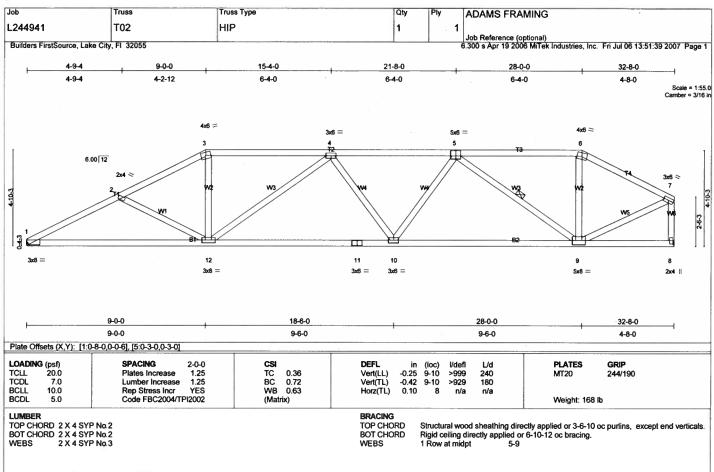
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 249 lb uplift at joint 2 and 71 lb uplift at joint 5.

4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (ptf) Vert: 1-2=-54

Trapezoidal Loads (pf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)





REACTIONS (lb/size) 1=1363/Mechanical, 8=1363/Mechanical Max Horz 1=137(load case 5)

Max Uplift1=-388(load case 5), 8=-397(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2555/761, 2-3=-2301/707, 3-4=-2036/667, 4-5=-2410/808, 5-6=-1261/432, 6-7=-1445/447, 7-8=-1323/402

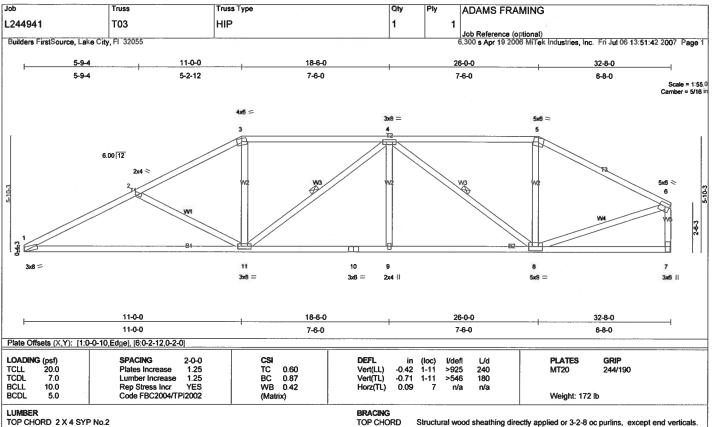
BOT CHORD WEBS

1-12=-734/2247, 11-12=-840/2443, 10-11=-840/2443, 9-10=-753/2179, 8-9=-32/34 2-12=-263/238, 3-12=-142/714, 4-12=-599/311, 4-10=-70/112, 5-10=-41/417, 5-9=-1173/474, 6-9=-51/366, 7-9=-414/1353

1 = 0.78, 2 = 0.34, 3 = 0.69, 4 = 0.41, 5 = 0.49, 6 = 0.64, 7 = 0.81, 8 = 0.48, 9 = 0.62, 10 = 0.41, 11 = 0.82 and 12 = 0.57

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

- 3) Provide adequate drainage to prevent water ponding.
 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 1 and 397 lb uplift at joint 8.



TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 3-2-8 oc purlins, except end verticals. Rigid ceiling directly applied or 7-1-5 oc bracing.

1 Row at midpt

4-11, 4-8

REACTIONS (lb/size) 1=1363/Mechanical, 7=1363/Mechanical Max Horz 1=151(load case 5)

Max Uplift1=-405(load case 5), 7=-366(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

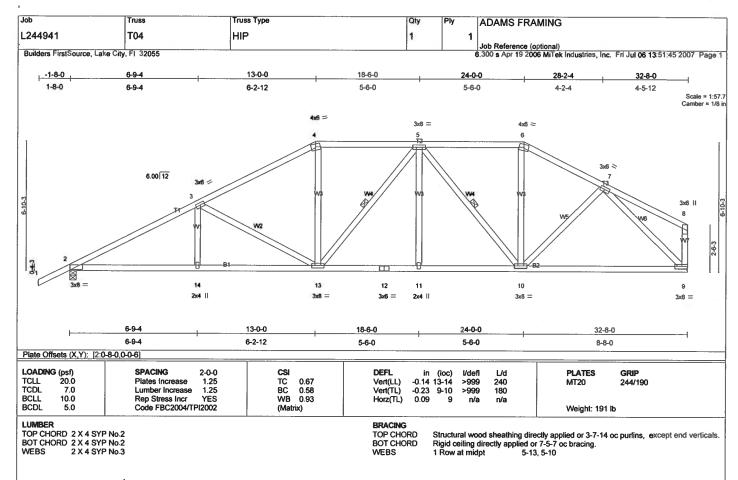
MAXIMUM COMPRESSIONMAXIMUM FEBS 12-14-14-1479, 5-6-1588/478, 6-7-1267/402
1-11-757/2179, 10-11-623/2018, 9-10-623/2018, 8-9-623/2018, 7-8-66/112
2-11-365/299, 3-11-86/590, 4-11-319/195, 4-9-0/191, 4-8-887/330, 5-8-29/359, 6-8-386/1311

JOINT STRESS INDEX

1 = 0.92, 2 = 0.34, 3 = 0.84, 4 = 0.57, 5 = 0.67, 6 = 0.80, 7 = 0.37, 8 = 0.60, 9 = 0.34, 10 = 0.69 and 11 = 0.57

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

- 3) Provide adequate drainage to prevent water ponding.
 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 1 and 366 lb uplift at joint 7.



REACTIONS (lb/size) 2=1461/0-4-0, 9=1356/Mechanical Max Horz 2=216(load case 5)

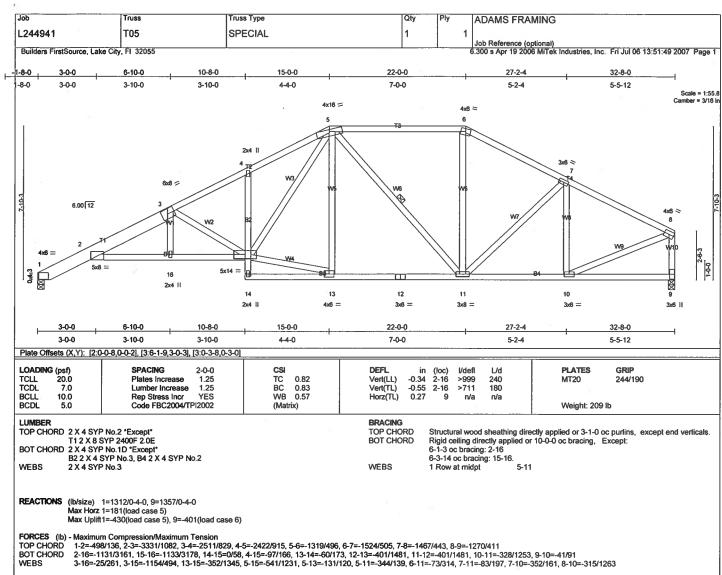
Max Uplift2=-525(load case 5), 9=-383(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD
TOP CH

2 = 0.79, 3 = 0.41, 4 = 0.70, 5 = 0.57, 6 = 0.56, 7 = 0.42, 8 = 0.34, 9 = 0.59, 10 = 0.57, 11 = 0.34, 12 = 0.63, 13 = 0.57 and 14 = 0.34

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

- 3) Provide adequate drainage to prevent water ponding.
 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 525 lb uplift at joint 2 and 383 lb uplift at joint 9.



JOINT STRESS INDEX

2 = 0.77, 3 = 0.51, 3 = 0.00, 4 = 0.56, 5 = 0.57, 6 = 0.79, 7 = 0.41, 8 = 0.73, 9 = 0.31, 10 = 0.72, 11 = 0.57, 12 = 0.64, 13 = 0.61, 14 = 0.77, 15 = 0.65 and 16 = 0.34

- NOTES

 1) Unbalanced roof live loads have been considered for this design.

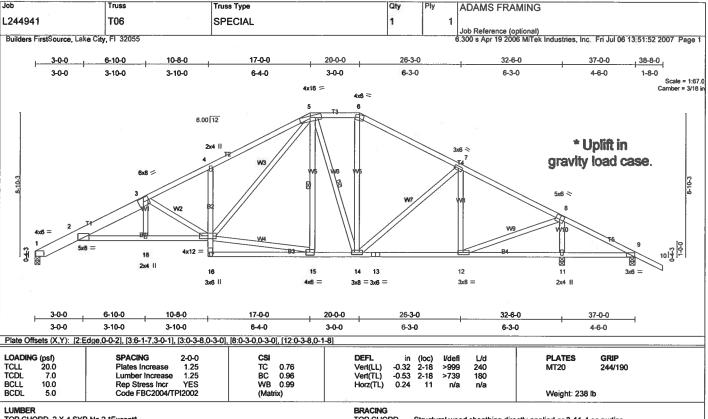
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 3) Provide adequate drainage to prevent water ponding.

 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 1 and 401 lb uplift at joint 9.



TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 8 SYP 2400F 2.0E

BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 3-11-4 oc purtins. Rigid ceiling directly applied or 2-2-0 oc bracing. 1 Row at midpt 5-15, 5-14

REACTIONS (lb/size) 11=2289/0-4-0, 1=1220/0-4-0, 9=-379/0-4-0 Max Horz 1=-169(load case 6)

Max Uplift11=-714(load case 5), 1=-412(load case 5), 9=-482(load case 9) Max Grav 11=2289(load case 1), 1=1220(load case 1), 9=102(load case 5)

FORCES (ib) - Maximum Compression/Maximum Tension

TOP CHORD

BOT CHORD WEBS

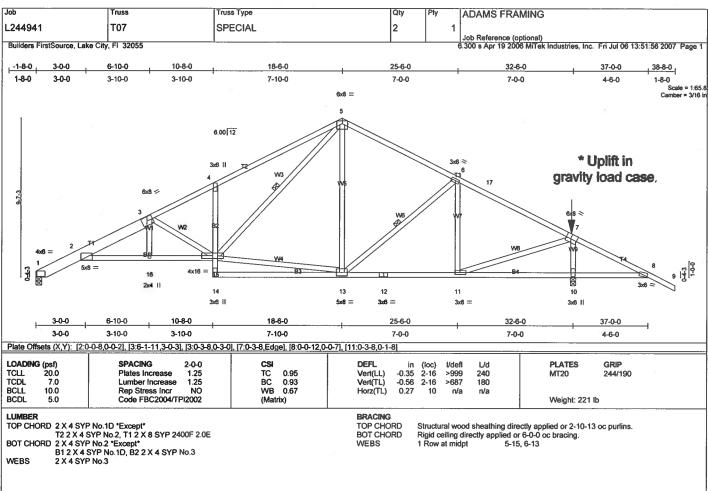
1-2=-462/292, 2-3=-3034/983, 3-4=-2267/762, 4-5=-2267/910, 5-6=-1032/476, 6-7=-1229/467, 7-8=-1050/400, 8-9=-467/1391, 9-10=0/39
2-18=-952/2880, 17-18=-953/2896, 16-17=0/105, 4-17=-238/258, 15-16=-19/225, 14-15=-203/1123, 13-14=-121/874, 12-13=-121/874, 11-12=-1099/432, 9-11=-1172/456
3-18=-18/246, 3-17=-1065/438, 15-17=-187/909, 5-17=-591/1320, 5-15=-20/59, 5-14=-414/172, 6-14=-112/324, 7-14=-76/318, 7-12=-555/255, 8-12=-581/2108, 8-11=-2106/683

JOINT STRESS INDEX

2 = 0.71, 3 = 0.49, 3 = 0.00, 4 = 0.45, 5 = 0.50, 6 = 0.54, 7 = 0.41, 8 = 0.79, 9 = 0.51, 11 = 0.77, 12 = 0.84, 13 = 0.32, 14 = 0.67, 15 = 0.39, 16 = 0.47, 17 = 0.95 and 18 = 0.34

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

a) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 714 lb uplift at joint 11, 412 lb uplift at joint 1 and 482 lb uplift at



REACTIONS (lb/size) 10=2544/0-4-0, 1=1320/0-4-0

Max Horz 1=-180(load case 6)

Max Uplift10=-1263(load case 6), 1=-473(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD

1-2=-500/33, 2-3=-3338/1168, 3-4=-2531/930, 4-5=-2563/1118, 5-6=-1452/618, 6-17=-1417/575, 7-17=-1844/736, 7-8=-517/472, 8-9=0/39 2-16=-1136/3162, 15-16=-1139/3180, 14-15=0/118, 4-15=-316/313, 13-14=-20/283, 12-13=-408/1410, 11-12=-408/1410, 10-11=-249/479, 8-10=-324/523 3-16=-36/280, 3-15=-1116/462, 13-15=-257/938, 5-15=-707/1467, 5-13=-111/368, 6-13=-361/253, 6-11=-347/278, 7-11=-714/1736, 7-10=-2372/1297

JOINT STRESS INDEX

2 = 0.78, 3 = 0.51, 3 = 0.00, 4 = 0.28, 5 = 0.64, 6 = 0.41, 7 = 0.79, 8 = 0.91, 10 = 0.42, 11 = 0.68, 12 = 0.55, 13 = 0.42, 14 = 0.59, 15 = 0.99 and 16 = 0.34

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for

- reactions specified.

 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

 4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1263 lb uplift at joint 10 and 473 lb uplift at joint 1.

 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 110 lb down and 85 lb up at 32-4-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

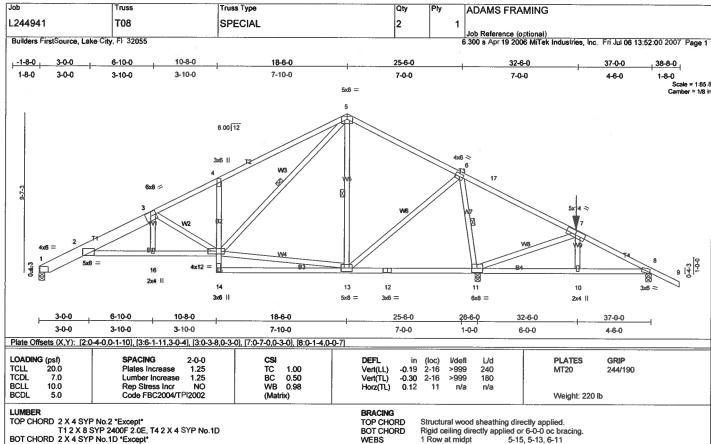
LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-2=-65, 2-5=-54, 5-17=-54, 7-9=-54, 2-15=-30, 8-14=-30 Concentrated Loads (lb)

Vert: 7=-110(B) Trapezoidal Loads (plf)

Vert: 17=-141(B=-87)-to-7=-195(B=-141)



BOT CHORD 2 X 4 SYP No.1D *Except* B2 2 X 4 SYP No.3, B3 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 11=2991/0-4-0, 1=765/0-4-0, 8=108/0-4-0 Max Horz 1=-180(load case 5) Max Upiff 11=-1314(load case 6), 1=-248(load case 6), 8=-392(load case 5) Max Grav 11=2991(load case 1), 1=765(load case 1), 8=354(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-287/252, 2-3=-1654/485, 3-4=-1047/328, 4-5=-1111/530, 5-6=-229/172, 6-17=-671/1690, 7-17=-621/1511, 7-8=-358/507, 8-9=0/39
2-16=-488/1564, 15-16=-489/1578, 14-15=0/112, 4-15=-359/334, 13-14=0/155, 12-13=-1012/561, 11-12=-1012/561, 10-11=-411/315, 8-10=-415/319
3-16=-8/211, 3-15=-785/327, 13-15=-85/239, 5-15=-589/1175, 5-13=-699/362, 6-13=-505/1468, 7-11=-1295/1048, 7-10=-183/118, 6-11=-2320/974 WEBS

JOINT STRESS INDEX

2 = 0.62, 3 = 0.49, 3 = 0.00, 4 = 0.30, 5 = 0.75, 6 = 0.66, 7 = 0.96, 8 = 0.89, 10 = 0.34, 11 = 0.32, 12 = 0.22, 13 = 0.69, 14 = 0.46, 15 = 0.83 and 16 = 0.34

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1314 lb uplift at joint 11, 248 lb uplift at joint 1 and 392 lb uplift at
- joint 8.

 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 110 lb down and 85 lb up at 32-4-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

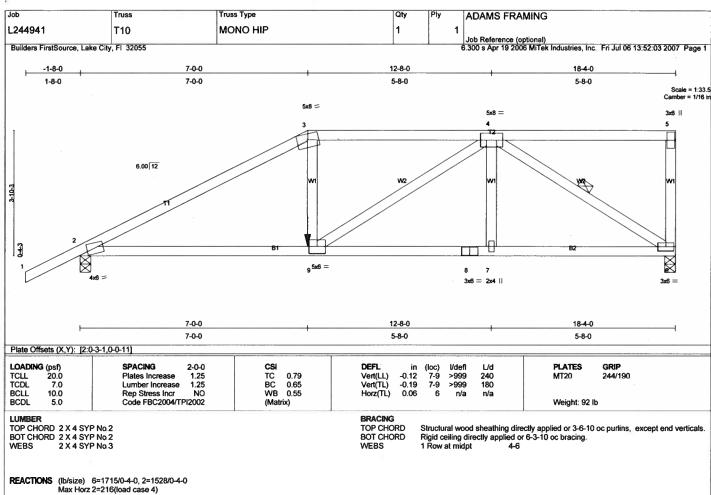
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-65, 2-5=-54, 5-17=-54, 7-9=-54, 2-15=-30, 8-14=-30

Concentrated Loads (lb) Vert: 7=-110(B)

Trapezoidal Loads (plf) Vert: 17=-141(B=-87)-to-7=-197(B=-143)



Max Uplift6=-768(load case 3), 2=-676(load case 4)

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-2651/1076, 3-4=-2316/1007, 4-5=-104/44, 5-6=-304/235
BOT CHORD 2-9=-1000/2288, 8-9=-905/2013, 7-8=-905/2013, 7-8=-905/2013

3-9=-187/651, 4-9=-214/359, 4-7=0/357, 4-6=-2271/1024

JOINT STRESS INDEX

2 = 0.87, 3 = 0.70, 4 = 0.52, 5 = 0.62, 6 = 0.85, 7 = 0.34, 8 = 0.75 and 9 = 0.23

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 2) Provide adequate drainage to prevent water ponding.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 3) All bearings are assumed to be 51° No.2. Cutshing capacity of 350.00 ps.
 4) Provide mechanical connection (by others) of fruss to bearing plate capable of withstanding 768 lb uplift at joint 6 and 676 lb uplift at joint 2.
 5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The

- design/selection of such connection device(s) is the responsibility of others.

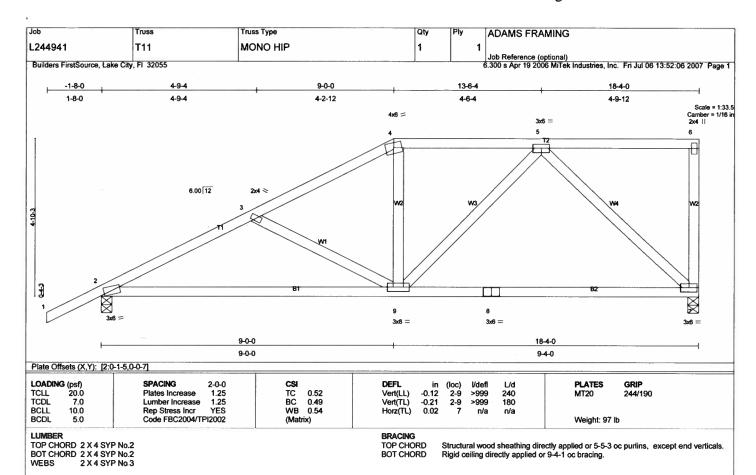
 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-117(F=-63), 2-9=-30, 6-9=-65(F=-35) Concentrated Loads (lb)

Vert: 9=-539(F)



REACTIONS (ib/size) 7=752/0-4-0, 2=861/0-4-0 Max Horz 2=261(load case 5) Max Uplift7=-262(load case 4), 2=-336(load case 5)

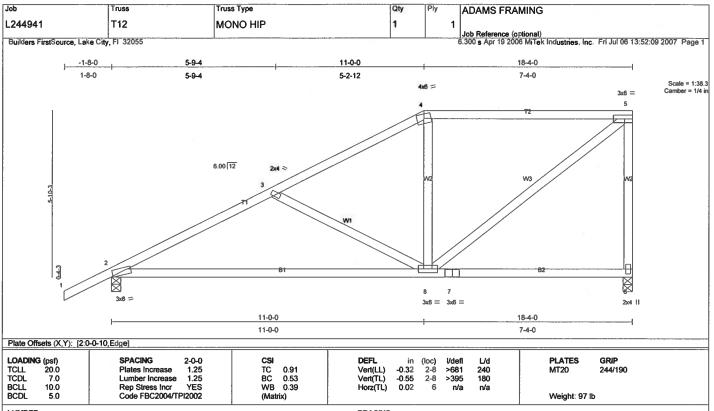
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1,2=0/39, 2-3=-1208/353, 3-4=-950/242, 4-5=-805/258, 5-6=-49/2, 6-7=-124/77
BOT CHORD 2-9=-454/1041, 8-9=-215/564, 7-8=-215/564
WEBS 3-9=-272/219, 4-9=0/168, 5-9=-103/348, 5-7=-720/298

JOINT STRESS INDEX

2 = 0.79, 3 = 0.34, 4 = 0.36, 5 = 0.37, 6 = 0.86, 7 = 0.67, 8 = 0.32 and 9 = 0.57

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 262 lb uplift at joint 7 and 336 lb uplift at joint 2.



BRACING TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins, except end verticals. Rigld ceiling directly applied or 9-0-11 oc bracing.

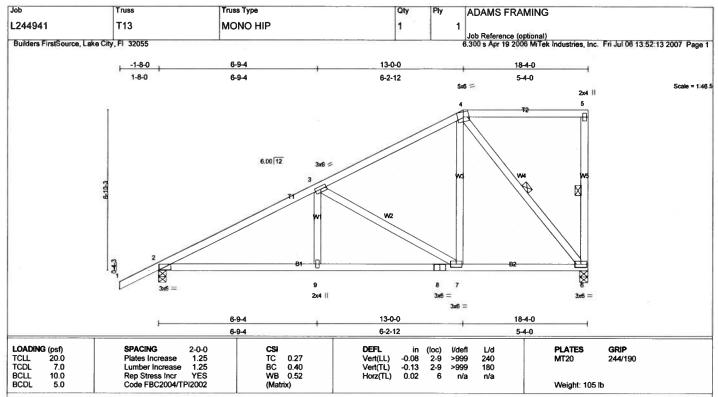
REACTIONS (Ib/size) 6=752/0-4-0, 2=861/0-4-0 Max Horz 2=307(load case 5) Max Uplift6=-250(load case 4), 2=-336(load case 5)

FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-.1124/342, 3-4=-799/202, 4-5=-658/233, 5-6=-675/260
BOT CHORD 2-8=-480/968, 7-8=-22/39, 6-7=-22/39
WEBS 3-8=-348/275, 4-8=-23/138, 5-8=-282/788

JOINT STRESS INDEX 2 = 0.88, 3 = 0.34, 4 = 0.75, 5 = 0.59, 6 = 0.71, 7 = 0.62 and 8 = 0.76

NOTES

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 250 lb uplift at joint 6 and 336 lb uplift at joint 2.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-4-12 oc purlins, except end verticals. Rigid ceiling directly applied or 9-1-5 oc bracing. 1 Row at midpt 5-6, 4-6

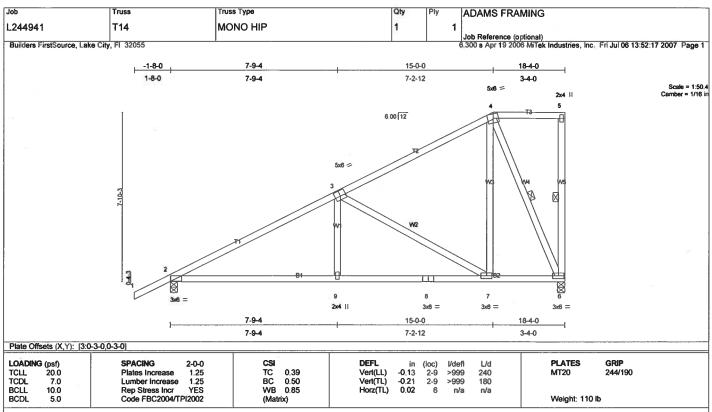
REACTIONS (lb/size) 6=752/0-4-0, 2=861/0-4-0 Max Horz 2=354(load case 5) Max Uplift6=-272(load case 5), 2=-330(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1.2=0/39, 2.3=-1204/292, 3.4=-601/156, 4.5=-17/7, 5.6=-122/93
BOT CHORD 2.9=-473/1003, 7.8=-473/1003, 6.7=-194/456
WEBS 3.9=0/217, 3.7=-615/317, 4.7=-117/470, 4.6=-708/310

JOINT STRESS INDEX

2 = 0.55, 3 = 0.41, 4 = 0.39, 5 = 0.61, 6 = 0.40, 7 = 0.35, 8 = 0.36 and 9 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 6 and 330 lb uplift at joint 2.



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-3-11 oc purlins, except end verticals. Rigid ceiling directly applied or 8-11-14 oc bracing. 1 Row at midpt 5-6, 4-6

REACTIONS (lb/size) 6=752/0-4-0, 2=861/0-4-0

Max Horz 2=400(load case 5)
Max Upliff6=-312(load case 5), 2=-318(load case 5)

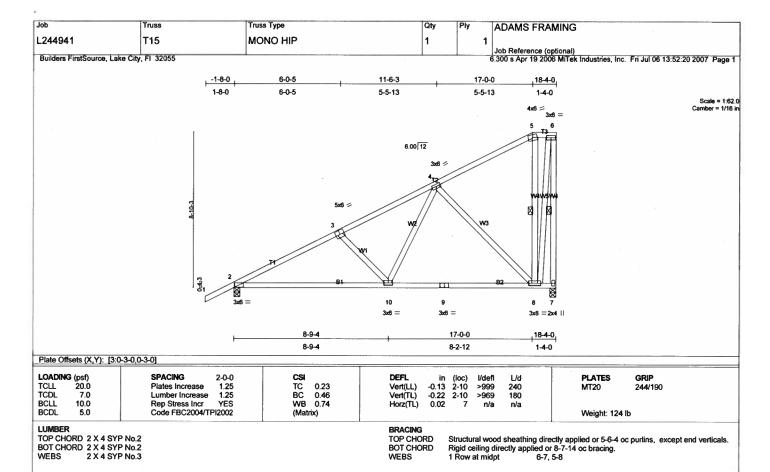
FORCES (lb) - Maximi

Maximum Compression/Maximum Tension 1-2=0/39, 2-3=-1155/249, 3-4=-431/83, 4-5=-2/2, 5-6=-38/51 2-9=-468/951, 8-9=-469/946, 7-8=-469/946, 6-7=-136/290 3-9=0/263, 3-7=-742/379, 4-7=-147/525, 4-6=-741/352 TOP CHORD BOT CHORD WEBS

JOINT STRESS INDEX

2 = 0.53, 3 = 0.75, 4 = 0.40, 5 = 0.34, 6 = 0.48, 7 = 0.35, 8 = 0.45 and 9 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 ib uplift at joint 6 and 318 lb uplift at joint 2.



REACTIONS (lb/size) 7=752/0-4-0, 2=861/0-4-0 Max Horz 2=446(load case 5) Max Uplift7=-357(load case 5), 2=-301(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2-0/39, 2-3=-1169/251, 3-4=-958/203, 4-5=-194/0, 5-6=-108/46, 6-7=-797/298 2-10=-535/996, 9-10=-301/558, 8-9=-301/558, 7-8=-4/8 3-10=-294/252, 4-10=-148/554, 4-8=-643/361, 5-8=-167/179, 6-8=-350/823

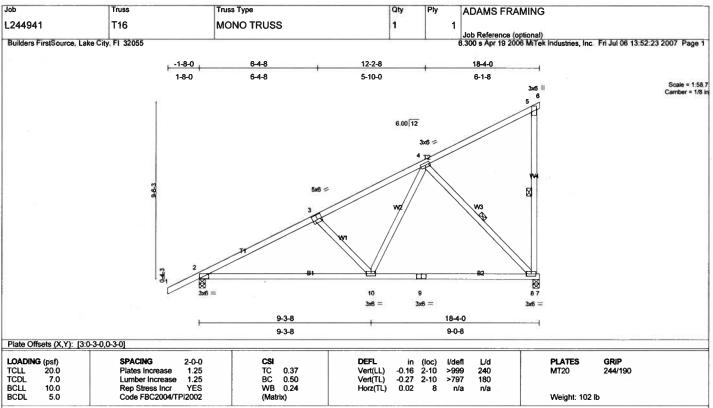
BOT CHORD

WEBS

JOINT STRESS INDEX

2 = 0.70, 3 = 0.39, 4 = 0.41, 5 = 0.42, 6 = 0.55, 7 = 0.46, 8 = 0.90, 9 = 0.35 and 10 = 0.47

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 8; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 7 and 301 lb uplift at joint 2.



BRACING

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-6-7 oc purlins, except end verticals. Rigid ceiling directly applied or 8-8-13 oc bracing. 1 Row at midpt 5-8, 4-8

REACTIONS (lb/size) 8=759/0-4-0, 2=855/0-4-0

Max Horz 2=475(load case 5) Max Uplift8=-393(load case 5), 2=-284(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1133/206, 3-4=-908/154, 4-5=-99/42, 5-6=-2/0, 5-8=-139/134
BOT CHORD 2-10=-521/963, 9-10=-273/492, 8-9=-273/492, 7-8=0/0
WEBS 3-10=-317/269, 4-10=-151/593, 4-8=-684/388

JOINT STRESS INDEX

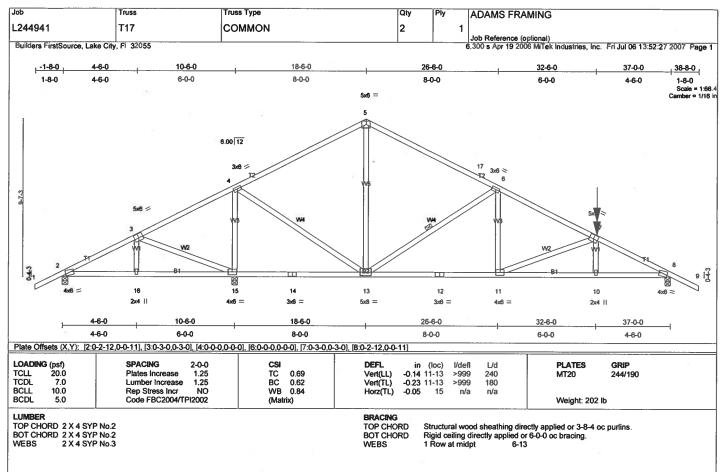
2 = 0.73, 3 = 0.43, 4 = 0.41, 5 = 0.29, 8 = 0.50, 9 = 0.26 and 10 = 0.50

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 393 lb uplift at joint 8 and 284 lb uplift at joint 2.



REACTIONS (Ib/size) 8=1463/0-4-0, 15=2082/0-4-0, 2=216/0-4-0 Max Horz 8=-161(load case 5) Max Upliff8=-685(load case 5), 15=-839(load case 6), 2=-274(load case 6) Max Grav 8=1463(load case 1), 15=2062(load case 1), 2=288(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD

1-2=0/39, 2-3=-94/307, 3-4=-271/711, 4-5=-701/321, 5-17=-608/320, 6-17=-707/293, 6-7=-1715/677, 7-8=-2608/1084, 8-9=0/39 2-16=-254/52, 15-16=-259/56, 14-15=-563/401, 13-14=-563/401, 12-13=-527/1432, 11-12=-527/1432, 10-11=-1013/2263, 8-10=-1011/2268 3-16=-202/150, 3-15=-497/438, 4-15=-1677/678, 4-13=-474/1327, 5-13=-11/191, 6-13=-1088/602, 6-11=-156/552, 7-11=-894/522, 7-10=0/134

JOINT STRESS INDEX 2 = 0.85, 3 = 0.47, 4 = 0.87, 5 = 0.67, 6 = 0.87, 7 = 0.47, 8 = 0.85, 10 = 0.34, 11 = 0.38, 12 = 0.66, 13 = 0.61, 14 = 0.66, 15 = 0.38 and 16 = 0.34

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 685 lb uplift at joint 8, 839 lb uplift at joint 15 and 274 lb uplift at 4) Provide mechanical connection (by cure's) or toos to coming paid support concentrated load(s) 280 lb down and 211 lb up at 32-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

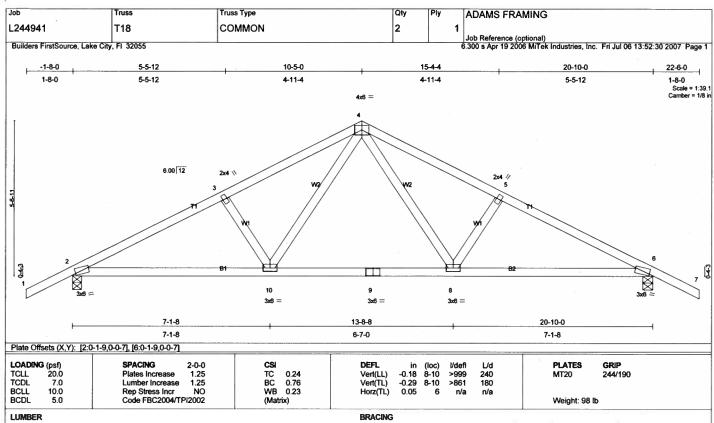
 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Uniform Loads (pif)
Vert: 1-5=-54, 5-17=-54, 7-9=-54, 2-8=-30

Concentrated Loads (lb)

Trapezoidal Loads (plf) Vert: 17=-54-to-7=-108(B=-54)



BOT CHORD

Structural wood sheathing directly applied or 4-6-4 oc purlins. Rigid ceiling directly applied or 8-10-12 oc bracing.

REACTIONS (lb/size) 2=1125/0-4-0, 6=1125/0-4-0

Max Horz 2=105(load case 5)

Max Uplift2=-449(load case 5), 6=-449(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-1824/586, 3-4=-1674/586, 4-5=-1674/586, 5-6=-1824/586, 6-7=0/39
BOT CHORD 2-10=-497/1561, 9-10=-256/1077, 8-9=-256/1077, 6-8=-410/1561
WEBS 3-10=-237/211, 4-10=-242/706, 4-8=-242/706, 5-8=-237/212

JOINT STRESS INDEX

2 = 0.80, 3 = 0.34, 4 = 0.57, 5 = 0.34, 6 = 0.80, 8 = 0.54, 9 = 0.79 and 10 = 0.54

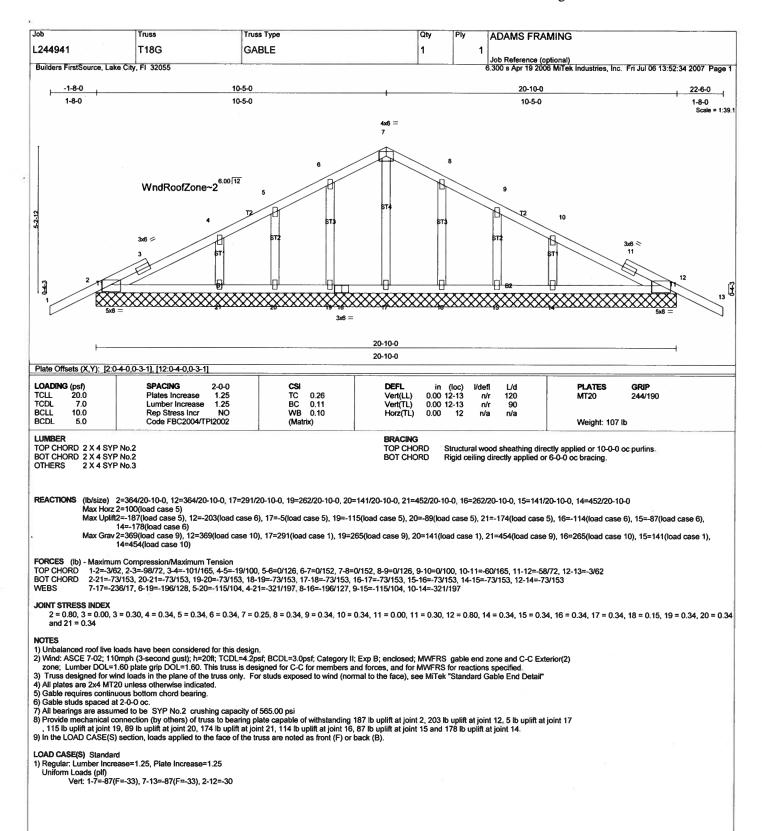
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

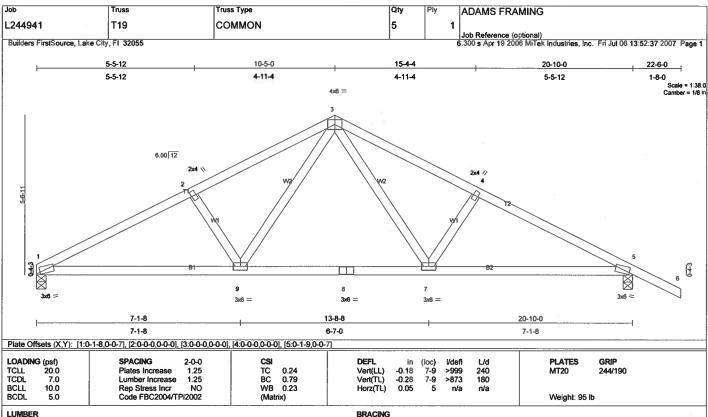
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 2 and 449 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (pif) Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30





BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-5-7 oc purlins. Rigid ceiling directly applied or 8-5-11 oc bracing.

REACTIONS (lb/size) 1=1021/0-4-0, 5=1129/0-4-0 Max Horz 1=-125(load case 6) Max Uplif11=-340(load case 5), 5=-450(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1.2e-1837/620, 2.3e-1699/618, 3-4e-1682/589, 4-5e-1832/590, 5-6e-0/39
BOT CHORD 1-9e-533/1589, 8-9e-267/1085, 7-8e-267/1085, 5-7e-420/1588
WEBS 2-9e-250/228, 3-9e-273/730, 3-7e-241/705, 4-7e-237/212

JOINT STRESS INDEX

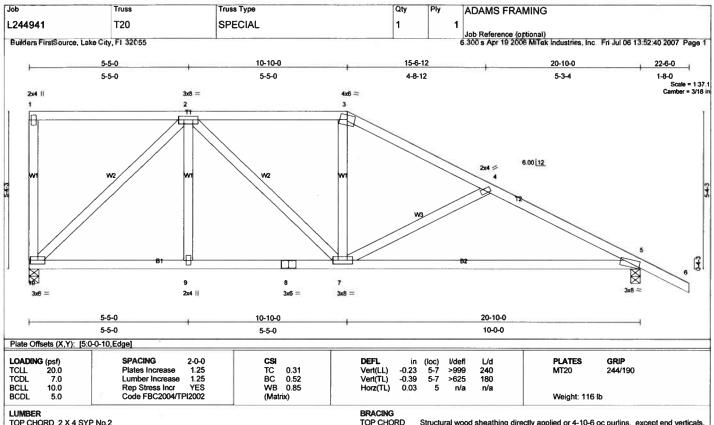
1 = 0.79, 2 = 0.35, 3 = 0.57, 4 = 0.35, 5 = 0.79, 7 = 0.56, 8 = 0.78 and 9 = 0.56

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 340 lb uplift at joint 1 and 450 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 3-6=-54, 1-9=-30, 7-9=-80(F=-50), 5-7=-30



Structural wood sheathing directly applied or 4-10-6 oc purlins, except end verticals Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=857/0-4-0, 5=965/0-4-0 Max Horz 10=-284(load case 6) Max Uplift10=-299(load case 3), 5=-366(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-10=-130/87, 1-2=-29/9, 2-3=-926/306, 3-4=-1091/290, 4-5=-1388/417, 5-6=0/39
BOT CHORD 9-10=-103/697, 8-9=-103/697, 5-7=-245/1203
WEBS 2-10=-926/325, 2-9=0/116, 2-7=-135/317, 3-7=0/195, 4-7=-318/250

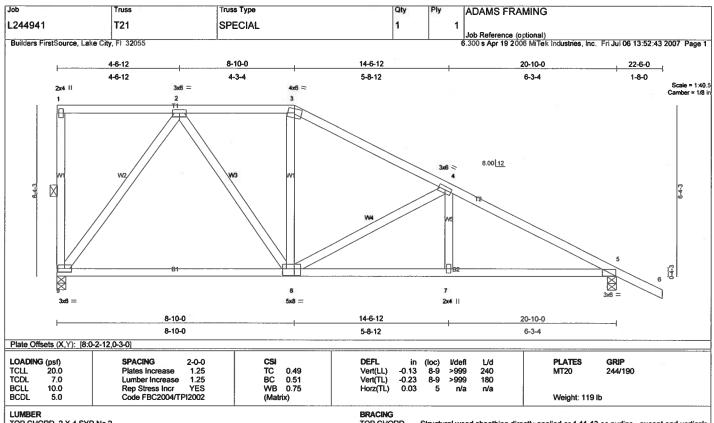
1 = 0.78, 2 = 0.57, 3 = 0.46, 4 = 0.34, 5 = 0.81, 7 = 0.57, 8 = 0.34, 9 = 0.34 and 10 = 0.38

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed, MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

BOT CHORD

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 299 lb uplift at joint 10 and 366 lb uplift at joint 5.



TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except end verticals Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS

REACTIONS (lb/size) 9=857/0-4-0, 5=965/0-4-0 Max Horz 9=-330(load case 6) Max Uplift9=-288(load case 3), 5=-367(load case 6)

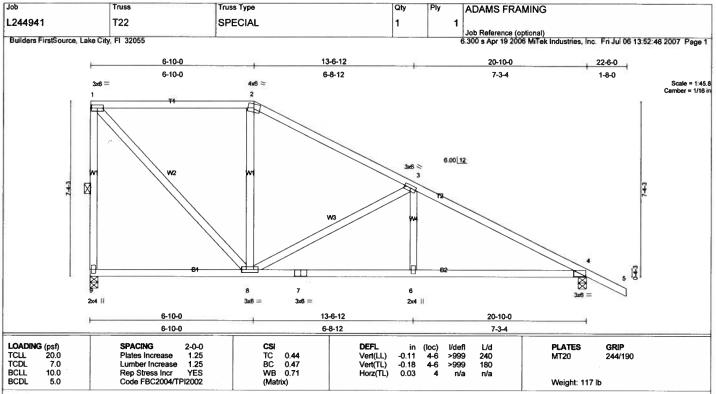
FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-9=-120/72, 1-2=-35/0, 2-3=-753/279, 3-4= BOT CHORD 8-9=-21/480, 7-8=-206/1222, 5-7=-206/1222 1-9=-120/72, 1-2=-35/0, 2-3=-753/279, 3-4=-917/254, 4-5=-1447/383, 5-6=0/39 8-9=-21/480, 7-8=-206/1222, 5-7=-206/1222

2-9=-756/298, 2-8=-181/473, 3-8=0/109, 4-8=-537/294, 4-7=0/172

1 = 0.80, 2 = 0.42, 3 = 0.45, 4 = 0.41, 5 = 0.65, 7 = 0.34, 8 = 0.89 and 9 = 0.64

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 288 lb uplift at joint 9 and 367 lb uplift at joint 5.



BRACING TOP CHORD

BOT CHORD WE8S

1 Row at midpt

Structural wood sheathing directly applied or 4-10-11 oc purlins, except end verticals Rigid ceiling directly applied or 10-0-0 oc bracing.

1-9

REACTIONS (lb/size) 9=857/0-4-0, 4=965/0-4-0 Max Horz 9=-377(load case 6) Max Uplift9=-297(load case 6), 4=-363(load case 6)

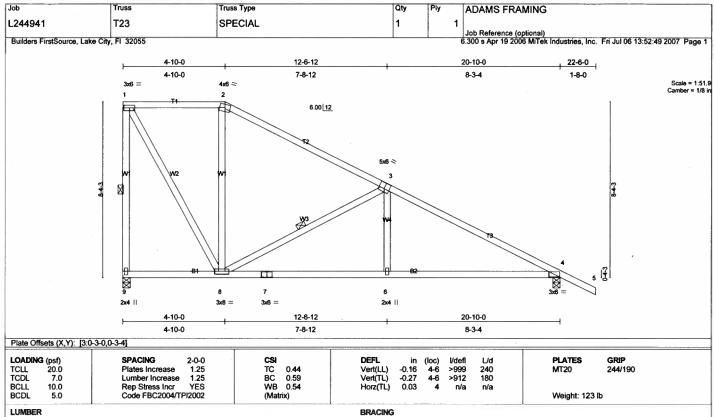
FORCES (ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-9--763/311, 1-2--582/236, 2-3--736/197, 3-4--1405/357, 4-5=0/39
BOT CHORD 8-9=0/372, 7-8--171/1177, 6-7--171/1177, 4-6--171/1177
WEBS 1-8--341/815, 2-8--82/144, 3-8--672/350, 3-6=0/238

JOINT STRESS INDEX

1 = 0.72, 2 = 0.79, 3 = 0.41, 4 = 0.63, 6 = 0.34, 7 = 0.46, 8 = 0.81 and 9 = 0.89

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 9 and 363 lb uplift at joint 4.



TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 4-8-13 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

1-9, 3-8

REACTIONS (ib/size) 9=857/0-4-0, 4=965/0-4-0 Max Horz 9=-423(load case 6) Max Uplift9=-334(load case 6), 4=-354(load case 6)

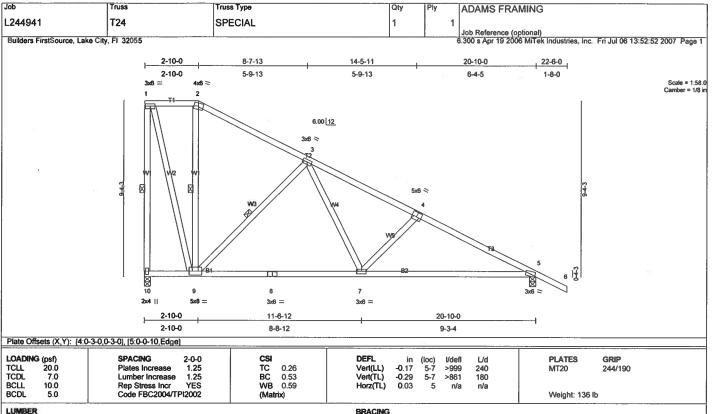
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-9=-806/340, 1-2=-410/180, 2-3=-562/128, 3-4=-1356/318, 4-5=0/39
BOT CHORD 8-9=0/423, 7-8=-125/1121, 6-7=-125/1121, 4-6=-124/1126
WEBS 1-8=-367/820, 2-8=-143/170, 3-8=-805/414, 3-6=0/285

JOINT STRESS INDEX

1 = 0.67, 2 = 0.76, 3 = 0.74, 4 = 0.62, 6 = 0.34, 7 = 0.49, 8 = 0.90 and 9 = 0.36

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 334 lb uplift at joint 9 and 354 lb uplift at joint 4.



WEBS

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-5 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 1-10, 2-9, 3-9

REACTIONS (lb/size) 10=857/0-4-0, 5=965/0-4-0

Max Horz 10=-469(load case 6)
Max Uplift10=-377(load case 6), 5=-339(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-10=-867/367, 1-2=-236/105, 2-3=-340/60, BOT CHORD 9-10=0/468, 8-9=0/710, 7-8=0/710, 5-7=-15 1-10=.867/367, 1-2=-236/105, 2-3=-340/60, 3-4=-1149/277, 4-5=-1373/329, 5-6=0/39 9-10=0/468, 8-9=0/710, 7-8=0/710, 5-7=-156/1175

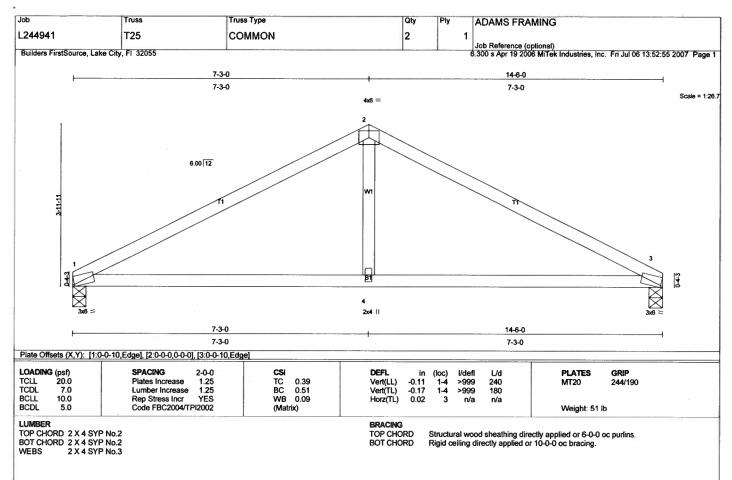
1-9=-385/866, 2-9=-143/148, 3-9=-677/382, 3-7=-157/586, 4-7=-313/269

JOINT STRESS INDEX

1 = 0.62, 2 = 0.46, 3 = 0.41, 4 = 0.45, 5 = 0.80, 7 = 0.50, 8 = 0.48, 9 = 0.51 and 10 = 0.34

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 377 lb uplift at joint 10 and 339 lb uplift at joint 5.



REACTIONS (lb/size) 1=595/0-4-0, 3=595/0-4-0 Max Horz 1=-53(load case 3)

Max Uplift1=-193(load case 5), 3=-193(load case 6)

FORCES (b) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-876/276, 2-3=-876/276
BOT CHORD 1-4=-182/714, 3-4=-182/714

2-4=0/288

JOINT STRESS INDEX

1 = 0.88, 2 = 0.85, 3 = 0.88 and 4 = 0.21

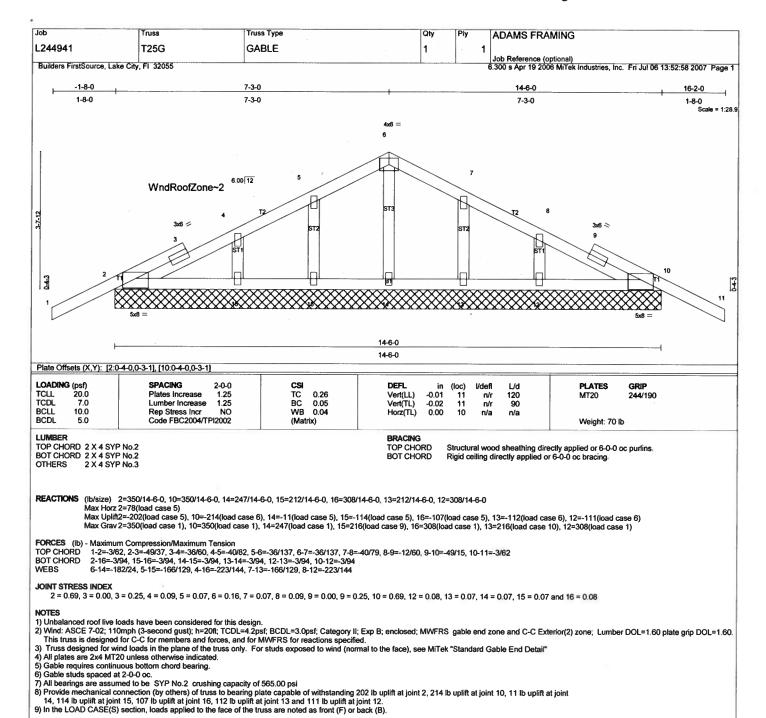
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.

This truss is designed for C-C for members and foroces, and for MWFRS for reactions specified.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 1 and 193 lb uplift at joint 3.

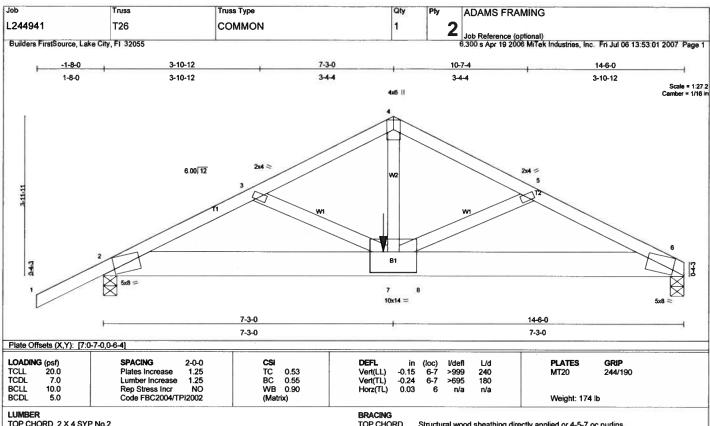


LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-6=-87(F=-33), 6-11=-87(F=-33), 2-10=-30

JULY 31, 2007,TRUSS DESIGN ENGINEER: THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987 STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196 16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP 2400F 2.0E WEBS 2 X 4 SYP No.3

Structural wood sheathing directly applied or 4-5-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=5235/0-4-0, 2=3116/0-4-0 Max Horz 2=111(load case 4) Max Uplift6=-1945(load case 5), 2=-1217(load case 4)

FORCES (b) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-6712/2479, 3-4=-6538/2427, 4-5=-6566/2424, 5-6=-6909/2578
BOT CHORD 2-7=-2215/5955, 7-8=-2288/6217, 6-8=-2288/6217

3-7=-145/159, 4-7=-2039/5597, 5-7=-437/276

JOINT STRESS INDEX

2 = 0.71, 3 = 0.12, 4 = 0.64, 5 = 0.12, 6 = 0.71 and 7 = 0.56

1-4

NOTES
1) 2-ply truss to be connected together with 10d (0.131*x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 7-4 2 X 4 - 1 row at 0-8-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60

plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1945 lb uplift at joint 6 and 1217 lb uplift at joint 2. 7) Girder carries tie-in span(s): 32-8-0 from 8-0-0 to 14-6-0

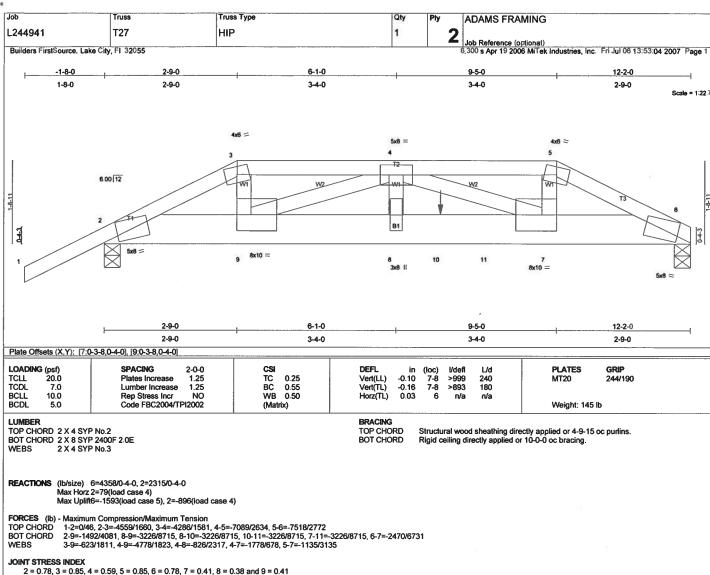
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3027 lb down and 1143 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-8=-30, 6-8=-667(F=-637) Concentrated Loads (lb)

Vert: 7=-3027(F)



- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 1 row at 0.9-0 oc.

 Bottom chords connected as follows: 2 X 8 2 rows at 0.4-0 oc.
- Webs connected as follows: 2 X 4 1 row at 0-9-0 oc.

 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 3) Unbalanced roof live loads have been considered for this design.

 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60

- 5) Provide adequate drainage to prevent water ponding.

- 6) Flowde adequate drainage to prevent water porturing.

 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1593 lb uplift at joint 6 and 896 lb uplift at joint 2.

 8) Girder carries tie-in span(s): 32-8-0 from 8-0-0 to 12-2-0

 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3032 lb down and 1145 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (pff) Vert: 1-3=-54, 3-5=-54, 5-6=-54, 2-11=-30, 6-11=-667(F=-637)

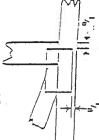
Concentrated Loads (lb) Vert: 10=-3032(F)

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate atherwise Dimensions are in Inches, Apply plates to both sides of truss and securely sear.



For 4 x 2 oilentation, tocate plates 1/8: from outside edge of truss and vertical web).

"This symbol indicates the required direction of slots in connector plates."

PLATE SIZE



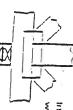
The first dimension is the width perpendicular to stots. Second dimension is the length parallel to stots

LAHERAL BRACING



Indicates facation of required continuous lateral bracing.

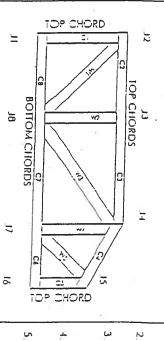
BEARING



510

which bearings (supports) occur

Numbering Syslem



JOINTS AND CHORDS ARE NUMBEREN CLOCKWISE AROUND THE TRIESS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE HUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA

ICBO

3907, 4922

96-31, 96-67

SBCCI

9667, 9432A

WISC/DILLIR 960022-W. 970036-11

561

PIER





MiTek Enginearing Reference Sheet: 1111-7473

General Safety Notes

fallure lo Follow Could Cause Properly Damage or Personal Infury

- Provide copies of this trust design to the building designer, erection supervisor, property owner and all other interested parties.
- Cul members to bear lightly against each other.
- Place plates on each face of truss of each joint and embed fully. Avoid knots and wane at joint locations.
- unless otherwise noted, locate chord splices at 1/2 panel length (1 8" from adjacent joint)

Unless offrenwise nated, moisture content of

lumber shoil not exceed 19% at time of fabrication.
Unless expressly noted. This design is not applicable for use with the retardant or

٥

- Camber is a non-shuckural consideration and is the responsibility of has labatcator. General practice is to comber for dead load deflection.
- Plate type, size and location climensions shown indicate minimum plating requirements
- I umber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- top choids must be sheathed or pullins provided at spacing shown on design.
- 11. Bollom chords require lateral bracing at 10 II. spacing, or less, II no celling is installed, unless otherwise nated.
- Anchorage and / or load transferring connections to Irusses are the responsibility of others unless shown.
- 13. Do not aveiload roof or thoor husses with stacks of construction materials.
- 14. Do not cut or alter truss member or place without pilor approval of a professional engineer.
- 15. Care should be exercised in handling, erection and installation of husses,
- © 1993 MiTek® Holdings, Inc.

Residential System Sizing Calculation

Summary Project Title:

Burley Road Spec Burley Road Lake City, FL 32024Adam Papka

Code Only **Professional Version** Climate: North

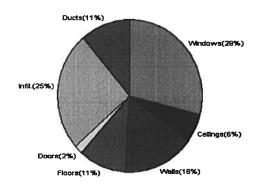
8/20/2007

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)							
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)				
Winter design temperature	33	F	Summer design temperature	92	F		
Winter setpoint	70	F	Summer setpoint	75	F		
Winter temperature difference 37 F		Summer temperature difference	17	F			
Total heating load calculation	27470	Btuh	Total cooling load calculation	41724	Btuh		
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh		
Total (Electric Heat Pump)	116.5	32000	Sensible (SHR = 0.75)	69.3	24000		
Heat Pump + Auxiliary(0.0kW)	116.5	32000	Latent	113.0	8000		
			Total (Electric Heat Pump)	76.7	32000		

WINTER CALCULATIONS

Winter Heating Load (for 1421 soft)

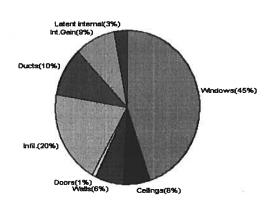
TTINIOI FIGURING LOGG (10)	1721 0410			
Load component			Load	
Window total	249	sqft	8015	Btuh
Wall total	1313	sqft	4312	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1500	sqft	1768	Btuh
Floor total	178	sqft	2911	Btuh
Infiltration	171	cfm	6907	Btuh
Duct loss			3039	Btuh
Subtotal			27470	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			27470	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1421 sqft)

Load component			Load	
Window total	249	sqft	18706	Btuh
Wall total	1313	sqft	2610	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1500	sqft	2484	Btuh
Floor total			0	Btuh
Infiltration	149	cfm	2777	Btuh
Internal gain			3780	Btuh
Duct gain			3893	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			34642	Btuh
Latent gain(ducts)			429	Btuh
Latent gain(infiltration)			5453	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occup	oants/othe	r)	1200	Btuh
Total latent gain			7082	Btuh
TOTAL HEAT GAIN			41724	Btuh





Version 8 For Florida residences only EnergyGauge® System Sizip PREPARED BY:

DATE:

EnergyGauge® FLRCPB v4.5.2

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Burley Road Spec Burley Road Lake City, FL 32024Project Title: Adam Papka Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

8/20/2007

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	90.0	32.2	2897 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	N	36.0	32.2	1159 Btuh
4	2, Clear, Metal, 0.87	N	9.0	32.2	290 Btuh
5	2, Clear, Metal, 0.87	N	4.0	32.2	129 Btuh
6	2, Clear, Metal, 0.87	Ε	36.0	32.2	1159 Btuh
7	2, Clear, Metal, 0.87	E	18.0	32.2	579 Btuh
8	2, Clear, Metal, 0.87	S	16.0	32.2	515 Btuh
	Window Total		249(sqft)		8015 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1090	3.3	3580 Btuh
2	Frame - Wood - Adj(0.09)	13.0	223	3.3	732 Btuh
	Wall Total		1313		4312 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1500	1.2	1768 Btuh
	Ceiling Total		1500	·	1768Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	178.0 ft(p)	16.4	2911 Btuh
	Floor Total		178		2911 Btuh
			Envelope Su	ıbtotal:	17524 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqf	t) CFM=	
	Natural	0.80	12789 1313	170.5	6907 Btuh
Ductload			(D	LM of 0.124)	3039 Btuh
All Zones		Sen	sible Subtotal Al	I Zones	27470 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Burley Road Spec Burley Road Lake City, FL 32024Project Title: Adam Papka Code Only Professional Version Climate: North

Cilmate: North

8/20/2007

	Subtotal Sensible Ventilation Sensible Total Btuh Loss	27470 Btuh 0 Btuh 27470 Btuh		
EQUIPMENT				

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Account to

Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details Project Title: Code C

Burley Road Spec Burley Road Lake City, FL 32024Adam Papka

Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

8/20/2007

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	90.0	32.2	2897 Btuh
2	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
3	2, Clear, Metal, 0.87	N	36.0	32.2	1159 Btuh
4	2, Clear, Metal, 0.87	N	9.0	32.2	290 Btuh
5	2, Clear, Metal, 0.87	N	4.0	32.2	129 Btuh
6	2, Clear, Metal, 0.87	Ε	36.0	32.2	1159 Btuh
7	2, Clear, Metal, 0.87	E	18.0	32.2	579 Btuh
8	2, Clear, Metal, 0.87	S	16.0	32.2	515 Btuh
	Window Total		249(sqft)		8015 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1090	3.3	3580 Btuh
2	Frame - Wood - Adj(0.09)	13.0	223	3.3	732 Btuh
	Wall Total		1313		4312 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1500	1.2	1768 Btuh
	Ceiling Total		1500		1768Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	178.0 ft(p)	16.4	2911 Btuh
	Floor Total		178		2911 Btuh
		Z	Zone Envelope Su	ıbtotal:	17524 Btuh
Infiltration	Type Natural	ACH X Vol 0.80	ume(cuft) walls(sqf 12789 1313	t) CFM= 170.5	6907 Btuh
Ductload	Pro. leak free, Supply(R6.0-	Attic), Return(R6.0-Attic) (D	LM of 0.124)	3039 Btuh
Zone #1		Sens	sible Zone Subto	otal	27470 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Burley Road Spec Burley Road Lake City, FL 32024Project Title: Adam Papka

Code Only Professional Version

Climate: North

8/20/2007

	Subtotal Sensible	27470 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	27470 Btuh
		I
EQUIPMENT		

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8
For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Burley Road Spec Burley Road Lake City, FL 32024Project Title: Adam Papka

Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

8/20/2007

Component Loads for Whole House

	Type*		Over	hang	Wind	dow Area	(sqft)	Н	ITM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross			Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	9ft.	90.0	0.0	90.0	29	80	7156	Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	9ft.	40.0	0.0	40.0	29	80	3181	Btuh
3	2, Clear, 0.87, None,N,N	N	1.5ft	9ft.	36.0	0.0	36.0	29	29	1043	Btuh
4	2, Clear, 0.87, None,N,N	N	1.5ft	9ft.	9.0	0.0	9.0	29	29	261	Btuh
5	2, Clear, 0.87, None,N,N	Й	1.5ft	9ft.	4.0	0.0	4.0	29	29	116	
6	2, Clear, 0.87, None,N,N	E	1.5ft	9ft.	36.0	0.0	36.0	29	80	2863	Btuh
7 8	2, Clear, 0.87, None,N,N 2, Clear, 0.87, None,N,N	E S	1.5ft 1.5ft	9ft. 9ft.	18.0 16.0	0.0 16.0	18.0 0.0	29 29	80 34	1431 463	Btuh Btuh
0	Excursion	3	1.510	9IL.	10.0	10.0	0.0	29	34		Btuh
	Window Total				249 (saft)				18706	
Walls	Type		R-Va	alue/U	-Value	Area(saft)		нтм	Load	Dian
1	Frame - Wood - Ext			13.0/		109			2.1		Btuh
2	Frame - Wood - Adj			13.0/		223.0			1.5		Btuh
-	Wall Total			10.07	0.03		3 (sqft)		1.5	2610	
Doors	Туре					Area (нтм	Load	Dian
1	Insulated - Exterior					20.			9.8	196	Btuh
2	Insulated - Adjacent					20.	_		9.8		Btuh
-	Door Total						0 (sqft)		0.0		Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area(нтм	Load	Dean
1	Vented Attic/DarkShingle			30.0		150			1.7		Btuh
·	Ceiling Total			00.0			0 (sqft)			2484	
Floors	Туре		R-Va	alue		Siz			НТМ	Load	
1	Slab On Grade			5.0		17	8 (ft(p))		0.0	0	Btuh
	Floor Total						0 (sqft)			0	Btuh
							- (-4.1)				
						Er	velope	Subtota	l:	24192	Btuh
nfiltration	Туре		Δ	СН	Volum	e(cuft) v	vall area	(saft)	CFM=	Load	
	SensibleNatural		•	0.70		12789	1313	(-4)	170.5	2777	Btuh
Internal			Occup			Btuh/oc		-	Appliance	Load	
gain		= '	- 00up	6		X 280		,	2400	3780	Btul
guiii				-							
						Se	ensible E	Invelope	e Load:	30749	Btuh
Duct load							(DGI	M of 0.1	27)	3893	Btul
						Sen	sible Lo	oad All	Zones	34642	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)
Project Title: Cod

Burley Road Spec Burley Road Lake City, FL 32024Adam Papka

Code Only Professional Version Climate: North

8/20/2007

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	30749	Btul
	Sensible Duct Load	3893	Btuh
	Total Sensible Zone Loads	34642	Btul
	Sensible ventilation	0	Btul
	Blower	0	Btul
Whole House	Total sensible gain	34642	Btu
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5453	Btul
	Latent ventilation gain	0	Btul
	Latent duct gain	429	Btul
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btul
	Latent other gain	0	Btul
	Latent total gain	7082	Btu
	TOTAL GAIN	41724	Btu

EQUIPMENT	

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Page 2

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details Project Title: Code C

Burley Road Spec Burley Road Lake City, FL 32024-

Adam Papka

Code Only Professional Version Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

8/20/2007

Component Loads for Zone #1: Main

	Type* Overhang		Wine	Window Area(sqft) HTM				Load			
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	9ft.	90.0	0.0	90.0	29	80	7156	Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	9ft.	40.0	0.0	40.0	29	80	3181	Btuh
= 3	2, Clear, 0.87, None,N,N	N	1.5ft	9ft.	36.0	0.0	36.0	29	29	1043	Btuh
4	2, Clear, 0.87, None,N,N	N	1.5ft	9ft.	9.0	0.0	9.0	29	29	261	Btuh
5	2, Clear, 0.87, None,N,N	N	1.5ft	9ft.	4.0	0.0	4.0	29	29	116	Btuh
6	2, Clear, 0.87, None,N,N	E	1.5ft	9ft.	36.0	0.0	36.0	29	80	2863	
7	2, Clear, 0.87, None,N,N	E	1.5ft	9ft.	18.0	0.0	18.0	29	80	1431	
8	2, Clear, 0.87, None,N,N	S	1.5ft	9ft.	16.0	16.0	0.0	29	34		Btuh
	Window Total				249 (sqft)				16514	Btuh
Walls	Туре		R-Va	ilue/U	l-Value	Area	(sqft)		НТМ	Load	
1	Frame - Wood - Ext			13.0/	0.09		0.0		2.1	2274	Btuh
2	Frame - Wood - Adj			13.0/	0.09	22	223.0		1.5	336	Btuh
	Wall Total					131	3 (sqft)			2610	Btuh
Doors	Туре					Area	(sqft)		НТМ	Load	
1	Insulated - Exterior					20	0.0		9.8	196	Btuh
2	Insulated - Adjacent					20	0.0		9.8	196	Btuh
	Door Total					4	0 (sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	lue		Area			НТМ	Load	
1	Vented Attic/DarkShingle			30.0		150	0.0		1.7	2484	Btuh
	Ceiling Total					150	0 (sqft)			2484	Btuh
Floors	Туре		R-Va	alue			ze		нтм	Load	
1	Slab On Grade			5.0		17	78 (ft(p))		0.0	0	Btuh
	Floor Total						0 (sqft)			0	Btuh
-						Z	one Enve	elope Su	ubtotal:	22000	Btuh
nfiltration	Туре		Δ	СН	Volum	e(cuft) y	wall area	(saft)	CFM=	Load	
	SensibleNatural			0.70	v Olull	12789	1313	(Sqrt)	149.2	2777	Btuh
Internal			Occup			Btuh/oc			Appliance	Load	Dian
		'	occup					,			D4. 4
gain				6		X 23	0 +		2400	3780	Btul
				(m)		S	ensible E	invelope	e Load:	28557	Btuh
Duct load	Prop. leak free, Supply(R6.0-A	attic), F	Retur	n(R6.0-	Attic)		(DGM	of 0.127)	3615	Btul
							Sensib	le Zone	Load	32172	Btuh

The following window Excursion will be assigned to the system loads.

Windows	July excursion for System 1 Excursion	on Subtotal: 2193 Btuh 2193 Btuh
Duct load		278 Btuh
	Sancible 6	Excursion Load 2470 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Burley Road Spec Burley Road Lake City, FL 32024-

Project Title: Adam Papka Code Only **Professional Version** Climate: North

8/20/2007

WHOLE HOUSE TOTALS

		2-	
	Sensible Envelope Load All Zones	30749	Btuh
	Sensible Duct Load	3893	Btuh
	Total Sensible Zone Loads	34642	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	34642	Btul
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5453	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	429	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	7082	Btul
	TOTAL GAIN	41724	Btu

CONTROL AND A PROTECTION OF THE PARTY OF THE	CONTRACTOR STATE OF S	
EQUIPMENT		15.18年6月1日中华市大陆区域的

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R)) (ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

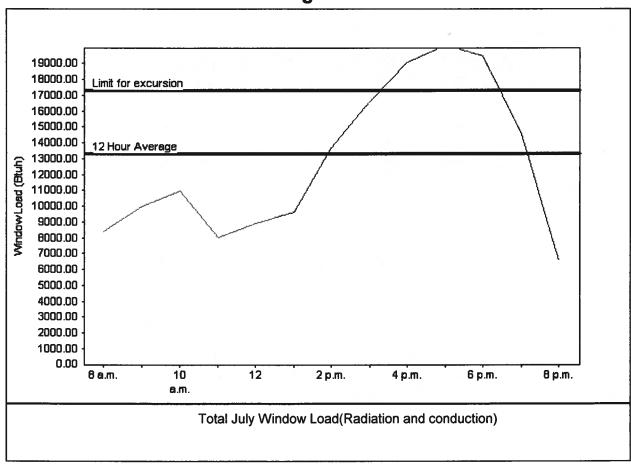
MidSummer

Burley Road Spec Burley Road Lake City, FL 32024Project Title: Adam Papka Code Only Professional Version Climate: North

8/20/2007

Weather data for: Gainesville - Defaults							
Summer design temperature	92	F	Average window load for July	13294 Btu			
Summer setpoint	75	F	Peak window load for July	20186 Btu			
Summer temperature difference	17	F	Excusion limit(130% of Ave.)	17282 Btu			
Latitude	29	North	Window excursion (July)	2905 Btuh			

WINDOW Average and Peak Loads



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY:

DATE:



Community Affairs



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Product Approval
USER: Public User

Product Approval Menu > Product or Application Search > Application List > Application Detail

Application Status Application Type Code Version

Comments

2004 FL1956-R1 Revision

Approved

Product Manufacturer Address/Phone/Email

Archived

PO Box 1404 TAMKO Building Products, Inc. fred_oconnor@tamko.com (800) 641-4691 ext 2394 Joplin, MO 64802

Authorized Signature

fred_oconnor@tamko.com Frederick O'Connor

Address/Phone/Email Technical Representative

Frederick J. O'Connor fred_oconnor@tamko.com Joplin, MO 64802 PO Box 1404 (800) 641-4691

1 of 5

Quality Assurance Representative

Address/Phone/Email

Category Subcategory

Roofing

Compliance Method

Asphalt Shingles

Certification Agency

Certification Mark or Listing

Underwriters Laboratories Inc.

Referenced Standard and Year (of Standard)

Standard ASTM D 3462

Year 2001

Equivalence of Product Standards Certified By

Product Approval Method

06/09/2005

Method 1 Option A

Date Submitted
Date Validated
Date Pending FBC Approval

Date Approved

06/20/2005

06/25/2005 06/29/2005

Summary of Products

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slopes of 2:12 or greater. Not approved for use in

Back

Next

Department of Community Affairs
Florida Bullding Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
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Product Approval Accepts: DCA Administration





5 of 5





339 Přínjálen Roe J Northolock, L. 601 52-2096 USA www.l.com aet 1 847 272 85 00

Jun 17, 2005

Tan ko Roofing Products Ms. Kerri Eden P.O Box 1404 220 W. 4th Street Jopl n. MO 64802-1404

Our Reference: R2919

This is to confirm that "Elite Glass-Seal AR", "Heritage 30 AR", "Heritage 50 AR", "Gli se-Seal AR" manufactured at Tusculoosa, AL and "Elite Glass-Seal AR", "Heritage 30 / R", "Heritage XL AR", "Heritage 50 AR" manufactured at Frederick, MD and "He itage 30 AR", "Heritage XL AR", and "Heritage 50 AR" manufactured in Dallas, TX are IL Listed aspiralt glass must shingles and have been evaluated in accordance with AN: I/UL 790, Glass A (ASTM E108), ASTM D3462, ASTM P3161 or UL 997 mor fied to 110 mph when secured with four nails.

Let ne know if you have any further questions.

Ver truly yours,

Alps sh Patel (Ext. 42522)

Eng seer Project

Fire Protection Division

Reviewed by

Randall K. Laymon (Ext. 42687)

Engineer Sr Stuff

Fire Protection Division





Application Instructions for

• HERITAGE® VINTAGE™ AR - Phillipsburg, KS

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO BUILDING PRODU :TS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.

THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IN COLD WI ATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTAN : It is not necessary to remove the plastic strip from the back of the shingles.

I. BOOF IN CK

These shing as are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per toot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special in: tructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from a improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be in: tailed at eaves and rakes.

PLYWOOD All plywood shall be exterior grade as defined by the American PI wood Association. Plywood shall be a minimum of 3/8 in. thickness at 1 applied in accordance with the recommendations of the American PI wood Association.

SHEATHIN: BOARDS: Boards shall be well-seasoned tongue-and-groove boa ds and not over 6 in. nominal width. Boards shall be a 1 in. nomin il minimum thickness. Boards shall be properly spaced and nailed.

TAMKO do a not recommend re-roofing over existing roof.

2. WENTER STREET

inadequate ventilation of attic spaces can cause accumulation of moisture in /inter months and a build up of heat in the summer. These conditions (in lead to:

- 1 Vapor Condensation
- 2 Buckling of shingles due to deck movement.
- 3 Rotting of wood members.
- 4 Premature failure of roof.

To insure a lequate ventilation and circulation of air, place louvers of sufficient at a high in the gable ends and/or install continuous ridge and soffit vents. FHA minimum property standards require one square foot of net tree ver litation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm a se of the ceiling or if at least one half of the ventilation is provided in ar the ridge. If the ventilation openings are screened, the total area s lould be doubled.

IT IS PART CULARLY IMPORTANT TO PROVIDE ADEQUATE VEN-TILATION.

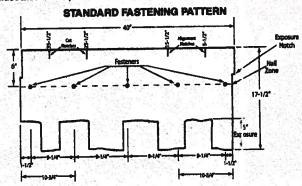
3. Photogram

WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, this will result in the termination of TAMKO's flabilities under the limited warranty. TAMKO will not be responsible for damage to shingles caused by winds in excess of the applicable miles per hour as stated in the limited warranty. See limited warranty for details.

FASTENING PATTERNS: Fasteners must be placed 6 in. from the top edge of the shingle located horizontally as follows:

1) Standard Festening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1-1/2 in. back from each end, one 10-3/4 in. back from each end and one 20 in. from one end of the shingle for a total of 5 fasteners. (See standard fastening pattern illustrated below).



2) Manaard or Steep Slope Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) Use standard nailing instructions with four additional nails placed 6 in. from the butt edge of the shingle making certain nails are covered by the next (successive) course of shingles.

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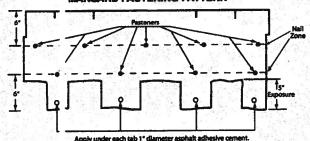


(CONTINUED from Pg. 1)

ERPAGE® VINTAGE™ AR - Phillipsburg, KS

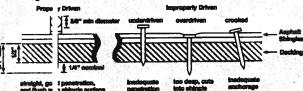
Each shingk tab must be sealed underneath with quick setting asphalt adhesive ce rent immediately upon Installation. Spots of cement must be equivaler in size to a \$.25 piece and applied to shingles with a 5 in. exposure, u: 3 9 fasteners per shingle.

MANSARD FASTENING PATTERN



the shingle : urface.

NAHLS: TAN (O recommends the use of nails as the preferred method of applicatio . Standard type roofing nails should be used. Nail shanks should be it ade of minimum 12 gauge wire, and a minimum head diameter of 3/1 in. Nalls should be long enough to penetrate 3/4 in. into the roof dec . Where the deck is less than 3/4 in. thick, the nails should be long eno gh to penetrate completely through plywood decking and extend at le. st 1/8 in. through the roof deck. Drive nail head flush with



4. United ATTEMPT

LINDERLAY WENT: An underlayment consisting of asphalt saturated felt must be applied over the entire deck before the installation of TAMKO shingles. Fe lure to add underlayment can cause premature failure of the shingles and leaks which are not covered by TAMKO's limited warranty. Apply the felt when the deck is dry. On roof decks 4 in. per foot and greater apply the felt parallel to the eaves lapping each course of the felt over the lower course at least 2 in. Where ends join, lap the felt 4 in. If left er cosed, the underlayment felt may be adversely affected by moisture an I weathering. Laying of the underlayment and the shingle application nust be done together.

Products witch are acceptable for use as underlayment are:

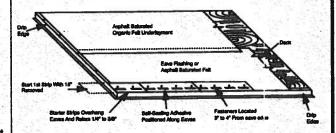
- 「AMKO No. 15 Asphalt Saturated Organic Felt
- \ non-perforated asphalt saturated organic felt
- which meets ASTM: D226, Type I or ASTM D4869; Type I
- Iny TAMKO non-perforated asphalt saturated organic felt
- FAMIKO TW Metal and Tile Underlayment, **IW Underlayment and Moisture Guard Plus® (additional** rentilation maybe required. Contact TAMKO's technical services department for more Information)

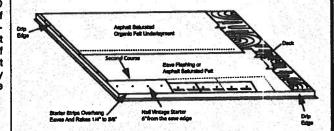
In areas where ice builds up along the eaves or a back-up of water from frozen or clogged gutters is a potential problem, TAMKO's Moisture Guard Pluse waterproofing underlayment (or any specialty eaves flashing product) may be applied to eaves, rakes, ridges, valleys, around chimneys, skylights or dormers to help prevent water damage. Contact TAMKO's Technical Services Department for more information. TAMKO does not recommend the use of any substitute products as shingle underlayment.

B. Application instructions

STARTER COURSE: Two starter course layers must be applied prior to application of Heritage Vintage AR Shingles

The first starter course may consist of TAMKO Shingle Starter, three tab self-sealing type shingles or a 9 inch wide strip of mineral surface roll roofing. If three tab self-sealing shingles are used, remove the exposed tab portion and install with the factory applied adhesive adjacent to the eaves. If using three tab self-sealing shingles or shingle starter, remove 18 in. from first shingle to offset the end joints of the Vintage Starter. Attach the first starter course with approved fasteners along a line parallel to and 3 in. to 4 in. above the eave edge. The starter course should overhang both the eave and rake edge 1/4 in. to 3/8 in. Over the first starter course, install Heritage Vintage Starter AR and begin at the left rake edge with a full size shingle and continue across the roof nailing the Heritage Vintage Starter AR along a line parallel to and 6 in. from the eave edge.





Note: Do not allow Vintage Starter AR joints to be visible between shingle tabs. Cutting of the starter may be required.

> HERFTAGE VINTAGE STARTER AR 12 1/2" x 36" 20 PIECES PER BUNDLE 60 LINEAL FT. PER BUNDLE

> > (Continued)

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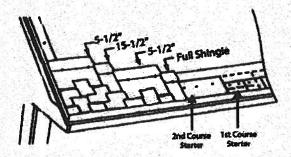
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(CONTINUED from Pg. 2)

HERTAGE WINTAGETM AR - Phillipsburg, (S

SHINGLE APPLICATION: Start the first course at the left rake edge with a full size shingle and overhang the rake edge 1/4 in. to 3/8 in.. To begin the second course, align the right side of the shingle with the 5-1/2 in. alignment notch on the first course shingle making sure to align the exposure notch. (See shingle illustration on next page) Cut the appropriate amount from the rake edge so the overhang is 1/4" to 3/6". For the third course, align the shingle with the 15-1/2 in. alignment notch at the top of the second course shingle, again being sure to align the exposure notch. Cut the appropriate amount from the rake edge. To begin the fourth course, align the shingle with the 5-1/2 in. alignment notch from the third course shingle while aligning the exposure notch. Cut the appropriate amount from the rake edge. Continue up the rake in as many rows as necessary using the same formula as outlined above. Cut pieces may be used to complete courses at the right side. As you work across the roof, install full size shingles taking care to align the exposure notches. Shingle joints should be no closer than 4 in.



& LAW SLOPE APPLICATION

On pitches 2 in. per foot to 4 in. per foot cover the deck with two layers of underlayment. Begin by applying the underlayment in a 19 in. wide strip along the saves and overhanging the drip edge by 1/4 to 3/4 in. Place a tuil 36 in. wide sheet over the 19 in. wide starter piece, completely overlapping it. All succeeding courses will be positioned to overlap the preceding course by 19 in. If winter temperatures average 25°F or less, thoroughly cement the laps of the entire underlayment to each other with plastic cement from eaves and rakes to a point of a least 24 in. inside the interior wall line of the building. As an atternative, TAMKO's Moisture (luard Plus self-adhering waterproofing underlayment may be used in lieu of the cemented felts.

7. WALLEY APPLICATION

TAMKO recommends an open valley construction with Heritage Vintage AR shingkes.

To begin, center a sheet of TAMKO Moisture Guard Plus, TW Underlayment or TW Metal & Tile Underlayment in the valley.

After the underlayment has been secured, install the recommended corrosion resistant metal (26 gauge galvanized metal or an equivalent) in the valley. Secure the valley metal to the roof deck. Overlaps should be 12" and cemented.

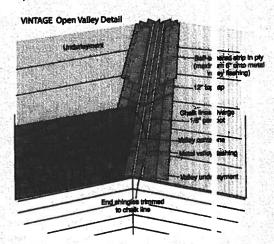
Following valley metal application; a 9" to 12" wide st p of TAMKO Moisture Guard Plus, TW Underlayment or TW fetal & Tile Underlayment should be applied along the edges of the metal valley flashing (max. 6" onto metal valley flashing) and on to; of the valley underlayment. The valley will be completed with shingle application.

SHINGLE APPLICATION INSTRUCTIONS (OPEN VA LEY)

- · Snap two chalk lines, one on each side of the valle centerline over the full length of the valley flashing. Locate the upper ends of the chalk lines 3" to either side of the valley cent rline.
- The lower end should diverge from each other by 1 8" per foot. Thus, for an 8' long valley, the chalk lines should be 7" either side of the centerline at the eaves and for a 16' val by 8".

As shingles are applied toward the valley, trim the last s lingle in each course to fit on the chalk line. Never use a shingle tr nmed to less than 12" in length to finish a course running into a valley if necessary, trim the adjacent shingle in the course to allow a longe portion to be

- · Clip 1" from the upper corner of each shingle on a 5° angle to direct water into the valley and prevent it from p netrating between the courses.
- Form a tight seal by comenting the shingle to the valley lining with a 3° width of asphalt plastic cement (conformi g to ASTM D 4586).



CAUTION:

Adhesive must be applied in smooth, thin, even layer

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.

(Continued)

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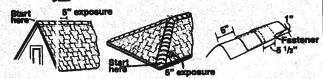
S. MIP AND RIDGE PASTERING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener on each side, 5-1/2 in. back from the exposed end and 1 in. up from the edge. TAMKO recommends the use of TAMKO Heritage Vintage Hip & Ridge shingle products.

Fasteners should be 1/4 in. longer than the ones used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLE IN COLD WEATHER.

Direction of prevailing wind



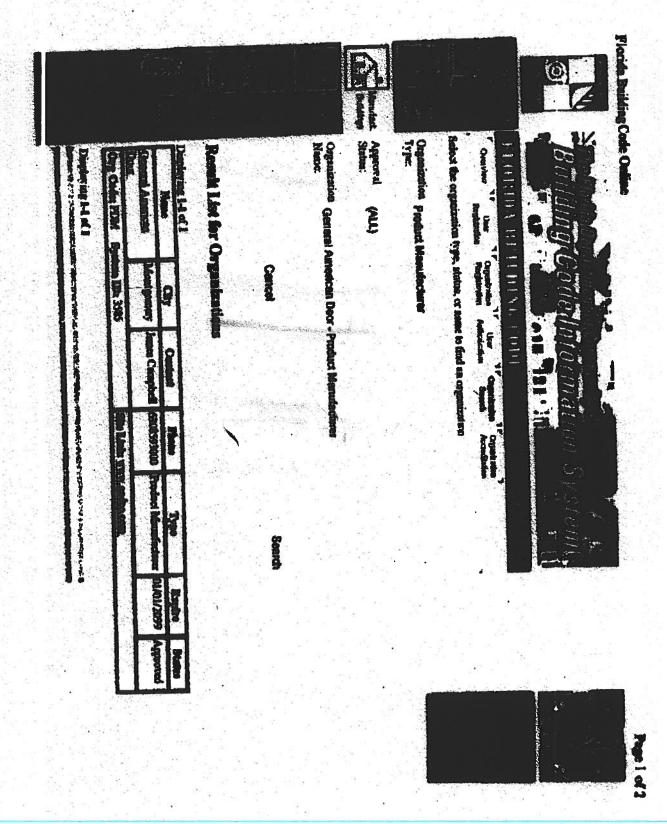
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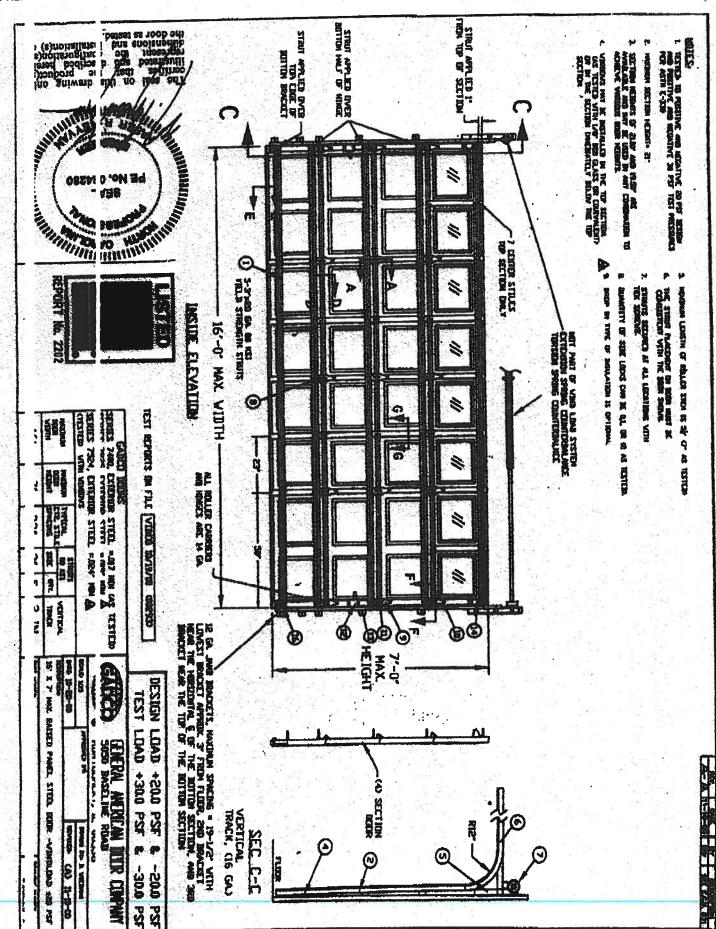
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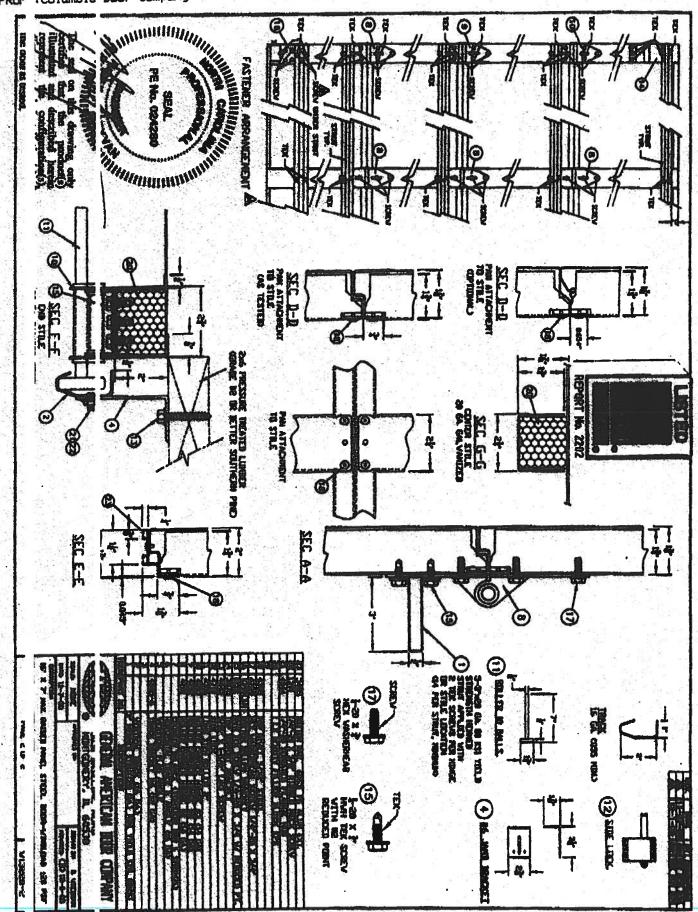
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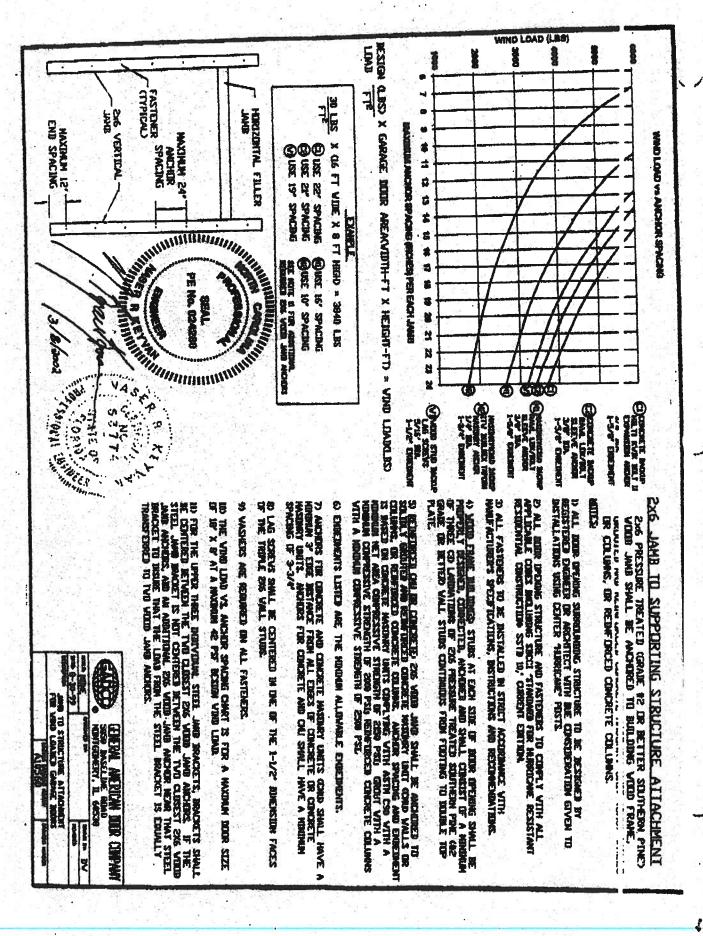
FAX NO. :386-754-9993







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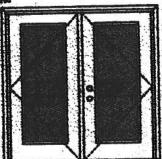


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WOOD-EDGE STEEL DOCRS

AFFROMED ANDAI



Units of other sizes are covered by this report as long as the games used do no exceed $30^{\circ} \times 63^{\circ}$.

Double Door

+40.5/-40.5

Hurricane protective system (shutters) is REQUIRED.

IN ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MAGE 12-02 and MAD-WL-MA0041-02.

REAL LATION DEVAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MAI 192-02.

APPROVED DOOR STYLES:









1/2 SLASS:











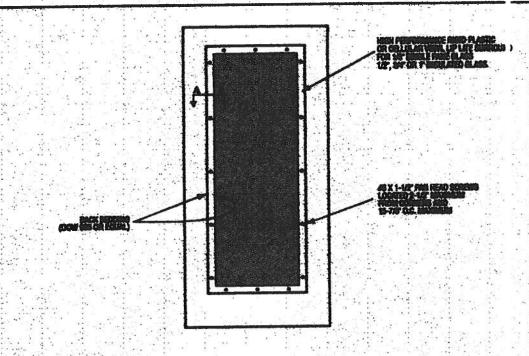


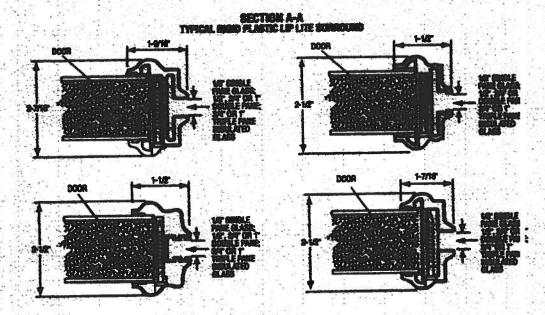


ly door object 6-pend; 6-pend with acrost; fly









Name 25 ; 2012 Or made department of product in productions



Administration and the second



WOOD-EDGE STEEL DOCRS

MOVED BOOR STYLES: 24 OLASS:

















CHIEFED TEST REPORTS:

MCTL 210-1807-7, 8, 9, 10, 11, 12; MCTL 210-1884-5, 6, 7, 8; MCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Milami-Dade BCCO PA202.

Evaluation report MCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both siles constructed from v cod Top and rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel, interior certly of steb filled with rigid polyarethane form core. Sleb glazed with insulated glass mounted in a placetic tip the surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

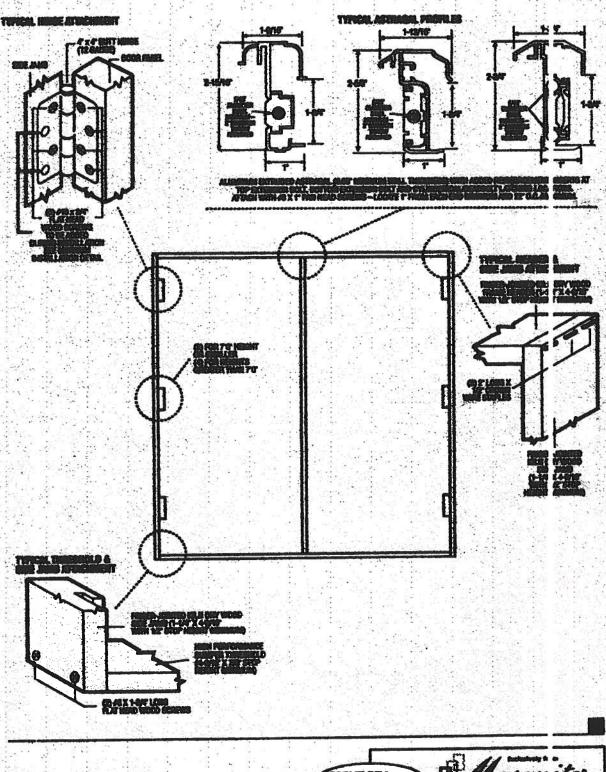
TESTED IN CORDANCE WITH I-DAGE BCCO PAZGZ

COMPANY SAME CITY, STATE

State of Florida, Professional Engineer Kurt Balthazor, P.E. — License Number 66588



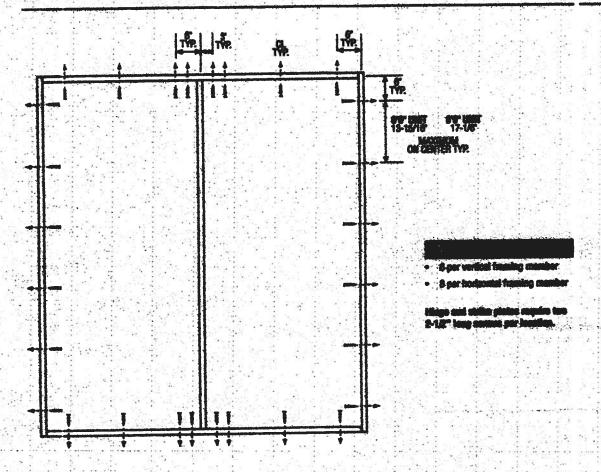
OUTSWING UNITS ! FITH DOUBLE ! COR



- 1.00 ·







Latching Hardware:

Compliance requires that GPADE 2 or botter (ANSMONMA A166.2) cylinderical and desclock hardware be installed.

Malae:

- 1. And for calculations have been carried out with the lowest (sens) featurer enting from the different fasteners being considered for wish is remarked for this and locality of the wood screens or 3/16" Topology.
- The wood servir degle sheer dealgo values come from Table 11.5A of ANSIAF & PA NOS for equitiest place beinder with a ship manifer internate of 1-U/C and publicament of minimum embedment. The 3/16" Tapcon shops wheer dealgo values come from the ITW and ELCO Dade Con by approvals respectively, each with minimum 1-1AC embedment.
- 3. Wood backs by others, must be anchored properly to transfer loads to the structure.



Window

Authorized Signature

ommunity Affairs



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USER: Public User **Product Approval**

Product Approval Menu > Product or Application Search > Application List > Application Detail

Application Type Code Version

日#

Application Status

Archived Comments

FL5108

2004 New

Approved

Product Manufacturer Address/Phone/Email

(717) 365-3300 ext 2101 650 W Market St MI Windows and Doors Gratz, PA 17030

Steven Urich

surich@miwd.com

surich@miwd.com

Technical Representative Address/Phone/Email

Address/Phone/Email Quality Assurance Representative

1 of 9





AAMA **CERTIFICATION PROGRAM**



AUTHORIZATION FOR PRODUCT CERTIFICATION

MI Wi dows & Doors, Inc. P.O. F ox 370 Greatz, PA 17030-0370

Attn: Bit Embey

The product described below is hereby approved for listing in the next lessue of the AAMA Certified Products Directory. he approval is based on successful completion of tests, and the reporting to the Administrator of the results of tests, ac ompanied by related drawings, by an AAMA Accredited Laboratory.

The fi ting below will be added to the next published AAMA Centiled Products Directory.

	SPECIFICATION					
	4AAMMOA 101/LS. 2-57 H-R55*-38-42	RECORD OF PRODUCT TESTED				LABEL ORDER
COMF	WY AND PLANT LOCATION	CODE NO.	SERIES MODEL & PRODUCT DESCRIPTION	MAXIMUM SIZE TESTED		NO.
MA Window All Window	s & Doors, Inc. (Oldsmar, FL) s & Doors, Inc. (Sugma, 110)	MTL-8 MTL-9	185/3195 SH (Fig) (ASTM)	20 x 52	SASH 210° x 27°	Py

- This C artification will expire May 14, 2008, and requires validation until then by continued listing in the current AAMA Certifii d Products Directory.
- Produ t Tested and Reported by: Architectural Testing, Inc.

Report No.: 01-50360.02

Date c Report: June 14, 2004

NOTE: LEASE REVIEW, AND ADVIS : ALI MANEDIATELY IF DATA, / 3 SHOWN, NEEDS CO MECTION.

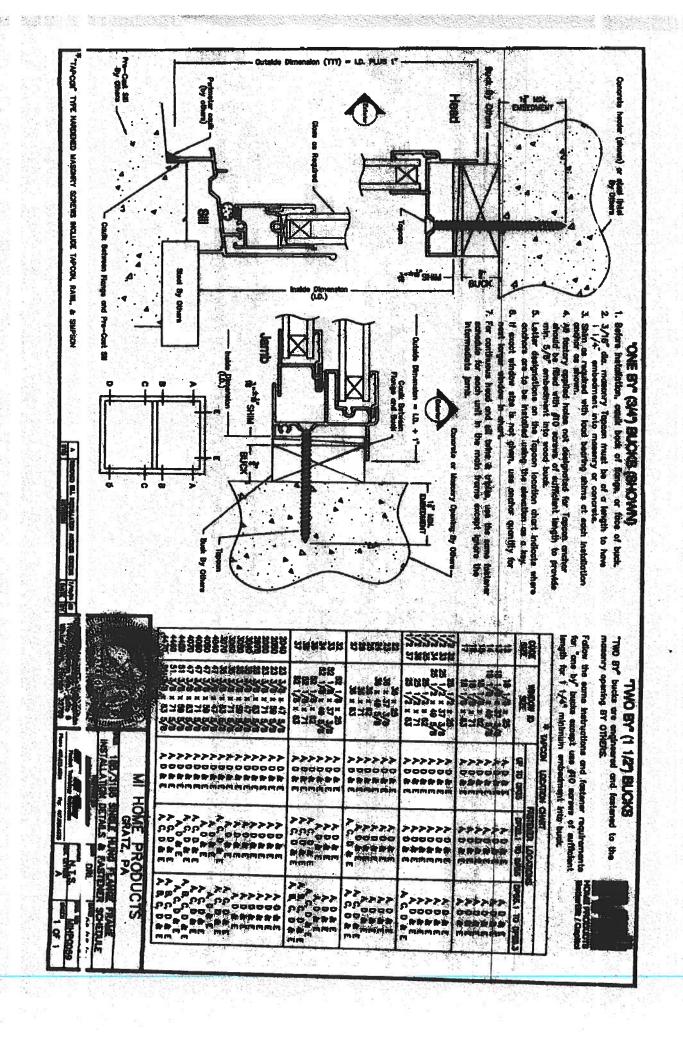
Date: Au just 1, 2005

DC: AAMA JG8/df ACP-04 (Rev 5/03) Validated for Certification:

Authorized for C

American Architectural Manufacturers Association

Laboratories, inc.



New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data geeded, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

¥ 26185

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Section 1: General Information (Treating Company Information)	
Company Name: Company Address: City State Company Business License No. Company Phone No. State Company Phone No. Company Phone No. State Company Phone	
Section 2: Builder Information	
Company Name: Adums From ing Company Phone No.	
Section 3: Property Information	
Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip)	Bulay Ava
Type of Construction (More than one box may be checked) Slab Basement Crawl Approximate Depth of Footing: Outside 77 Type	Other
Section 4: Treatment Information	
Date(s) of Treatment(s)	Masonry Voids
Comments	
Name of Applicator(s) 5700 Branco Certification No. (if required by State law))
The applicator has used a product in accordance with the product label and state requirements. All treatment materials and mater	
Authorized SignatureDate	9-17.07

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Form NPCA-99-B may still be used

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Donald F. Lee & Associates, Inc.

Surveyors & Engineers

140 NW Ridgewood Avenue Lake City, Florida 32055 (386) 755-6166 Fax (386) 755-6167 donald@dfla.com

Monday, September 24, 2007

Revised: 1/18/2008

FROM: Tim Delbene, P.L.S.

TO: Columbia County Building & Zoning Dept.

CC: Adam's Framing

RE: Floor Elevation Check - Lot 2 - Gerald Riggle Development

We have obtained elevations on the finished floor of a house under construction on the above referenced Lot. The elevations are based on Benchmarks set for design surveys of the subject development by Britt Surveying.

Finished Floor Elevation: 113.79°
Garage Floor Elevation: 112.71°
Lowest Adjacent Grade: 110.9°
Highest Adjacent Grade: 111.9°

The 100 year flood elevation for this Lot is 112.0', as shown on the record subdivision plat of Gerald Riggle Development. Minimum Floor Elevation for said lot is 113.50' according to said subdivision plat.

SIGNED:

Timothy A. Delbene, P.L.S. Florida Reg. Cert. No. 5594

DATE: / //8/2008.

