DATE 07/11/2008 Columbia County Bu This Permit Must Be Prominently Posted o		PERMIT 000027166
APPLICANT JACOB KIRSCH	PHONE 344-4817	00002/100
ADDRESS 484 NW TURNER AVE	LAKE CITY	FL 32055
OWNER SOUTHEAST DEVELOPERS	PHONE 755-2082	<u> </u>
ADDRESS 199 SW BUTTERCUP DR	LAKE CITY	FL 32024
CONTRACTOR JACOB KIESCH	PHONE 344-4817	
LOCATION OF PROPERTY 90 W, L 247, L CALLAHAN AVE.	TR ON HOPE HENRY, TR MORNING	GL
TR ON BUTTERCUP, 4TH ON RI	GHT	
TYPE DEVELOPMENT SFD,UTILITY EST	TIMATED COST OF CONSTRUCTION	123200.00
HEATED FLOOR AREA 1914.00 TOTAL AREA	A 2464.00 HEIGHT _	STORIES 1
FOUNDATION CONC WALLS FRAMED RO	OOF PITCH 8/12 FI	LOOR SLAB
LAND USE & ZONING RSF-2	MAX. HEIGHT	20
Minimum Set Back Requirments: STREET-FRONT 25.00	REAR 15.00	SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE X PP	DEVELOPMENT PERMIT NO.	
PARCEL ID 15-4S-16-03023-548 SUBDIVISION	N ROLLING MEADOWS	
LOT 48 BLOCK PHASE UNIT	TOTAL ACRES 0	.51
000001636 CBC1253775	a Vin	
Culvert Permit No. Culvert Waiver Contractor's License Num	ber Applicant/Owner	-/Contractor
CULVERT 08-0057 BK	JH	Y
Driveway Connection Septic Tank Number LU & Zoning	g checked by Approved for Issuan	ce New Resident
COMMENTS: PLAT REQUIRES MFE AT 109 FT., ELEVATION COM	NFIRMATION LETTER	
REQUIRED AT SLAB		
	Check # or C	Cash 12110
FOR BUILDING & ZONIN	G DEPARTMENT ONLY	(footer/Slab)
Temporary Power Foundation	Monolithic	(Iooten Stab)
date/app. by	date/app. by	date/app. by
Under slab rough-in plumbing Slab	Sheathing	/Nailing
date/app. by	date/app. by	date/app. by
Framing Rough-in plumbing about date/app. by	ove slab and below wood floor	
Electrical rough-in Heat & Air Duct	¥	date/app. by
	-	date/app. by
date/app. by	Peri. beam (Lint	A STANDARD CONTRACTOR OF THE ACCUST OF THE A
Permanent power C.O. Final	Peri. beam (Lint	date/app. by
Permanent power C.O. Final date/app. by	date/app. by Peri. beam (Lint Culvert	el)
Permanent power C.O. Final	date/app. by Culvert ate/app. by Pool	date/app. by date/app. by
Permanent power date/app. by C.O. Final date/app. by M/H tie downs, blocking, electricity and plumbing date/app. Reconnection Pump pole	Peri. beam (Lint date/app. by Culvert ate/app. by Pool Utility Pole	date/app. by date/app. by date/app. by
Permanent power C.O. Final	Peri. beam (Lint date/app. by Culvert ate/app. by Pool by Utility Pole date/app. by	date/app. by date/app. by date/app. by
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Permanent power C.O. Final	Peri. beam (Lint date/app. by Culvert ate/app. by Pool by Utility Pole date/app. by Re-roof ate/app. by Striction Surchard FIRE FEE \$ 0.00 WAST	date/app. by date/app. by date/app. by date/app. by g date/app. by E FEE \$ 12.32

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

A. 4 -

The state of the s	a remit Application
For Office Use Only Application # 0 801-52 Date R	eceived 1-11-08 By (1) Permit # (636) 27166
Application Approved by - Zoning Official 12 Date	18.01.08 plans 1 25 TH
Prood Zone Development Permit N/A Zoning	RSF-2 Land Hea Plan Man Colon Red J. O.
Comments 12+ requires MIPE of 101 H. Elevation	Confirmation Letter Regular
NOC (DEH Deed or PA Site Plan Sta	te Road Info □ Parent Parcel # □ Development Perm
	FOY 586 - 18 4 - COMP
Name Authorized Person Signing Permit 2006 K.	rsch Phone 386-344-4817
	14 1 1 1 1 2 7 7 C C C
Owners Name Southeast Developers	Phone 386-755-2082
Maddless 1 1 300 Sufferwall 1 (1) of it	D 37x211
Contractors Name Sucob Kirsch - Compas	5 Builders Phone 386 - 344-4817
Address 19 100 (VINET ave, #10),	akr /itu El 32000
Fee Simple Owner Name & Address Southeast	Druelopeis Group, lake (in F)
Bonding Co. Name & Address_N/H	
Architect/Engineer Name & Address Nicholas Paul	Geisler, 1758 NW Brown H. L.C. F
Mortgage Lenders Name & Address (Olombia Bank	173 NW hillshore ST Lake City
Circle the correct power company - FL Power & Light - Clay Property ID Number 15-45-16-63-073-5-5-0	12,300
TOTAL TITLE TOTAL	Let YX Di-
Driving Directions from Usgo/441 - go usgo Wes	Lot 48 Block Unit Phase
Sw callahan ave, tL, follow to R	olling Market Follow to
Follow Morning Glory dr. to SW Buttercop do	Colling Meadows Sub. TR.
Type of Construction S. F. D.	·
Total Acreage Lot Size Do you need a Culve	umber of Existing Dwellings on Property
Actual Distance of Structure from Property Lines - Front	
Total Building Heleft /0 /s	
	TOTAL 2 UIU ROOF FIRE
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuance of a permit and Il laws regulating construction in this jurisdiction.	stallations as indicated. I certify that no work or
Il laws regulating construction in this jurisdiction.	trial all work be performed to meet the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information ompliance with all applicable laws and regulating construction.	mation is accurate and all work will be done in
	and zoning.
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF WICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTI	OF COMMENCMENT MAY RESULT IN YOU PAYING
ENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE O	F COMMENCEMENT.
	1 1 2.1
wner Builder or Authorized Person by Notarized diatter FLORIDA	Contractor Signature
TATE OF FLORIDA Michelle Fischer	Contractors License Number (BC 1753775
OUNTY OF COLUMBIA Commission # DD598374	Competency Card Number
worn to (or affirmed) and subscribed Selber ANTIC BONDING CO., INC.	
day of 2008	Michelle Nochop)
ersonally known or Produced Identification	Notary Signature (Revised Sept. 2006
47	(Nevised Sept. 2000
	STIN LESS MESSAY 1.18.08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

<u>YOU ARE HEREBY NOTIFIED</u> as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's License Number (BC 1253775 Columbia County Competency Card Number Competency Card Number Competency Card Number Or Produced Identification or Produced Identification

SEAL:

Page 2 of 2 (Both Pages must be submitted together.)

State of Florida Notary Signature (For the Contractor)

Notary Public State of Florida

My Commission DD578349 Expires 09/17/2010

Matthew Rocco

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001636

APPLICANT ADDRESS 484 NW TURNER AVE SUITE 101 LAKE CITY FL 32055 OWNER SOUTHEAST DEVELOPERS PHONE 755-2082 ADDRESS 199 SW BUTTERCUP DR LAKE CITY FL 32024 CONTRACTOR ACOB KIESCH PHONE 344-4817 LOCATION OF PROPERTY 90 W, L 247, L CALLAHAN AVE, TR ON HOPE HENRY, TR MORNING GLORY, TR ON BUTTERCUP, 4TH ON RIGHT SUBDIVISION/LOT/BLOCK/PHASE/UNIT ROLLING MEADOWS 48 SIGNATURE INSTALLATION REQUIREMENTS X Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4: 1 slope and poured with a 4 inch thick reinforced concrete slab. INSTALLATION NOTE: Turnouts will be required as follows: a) a majority of the current and existing driveway turnouts are paved, or; b) the driveway to be served will be paved or formed with concrete. Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts. Culvert installation shall conform to the approved site plan standards. Department of Transportation Permit installation approved standards. Other	DATE 0//1	1/2008 PARCEL ID #	15-45-16-03023-548	
OWNER SOUTHEAST DEVELOPERS PHONE 1755-2082 ADDRESS PHONE 1755-2082 ADDRESS PHONE ACOB KIESCH PHONE ALAKE CITY FL 1755-2082 CONTRACTOR ACOB KIESCH PHONE 1755-2082 ADDRESS PHONE 1755-2082 ALAKE CITY PHONE 1755-2082 ADDRESS PHONE 1755-2082 ALAKE CITY PHONE 1755-2082 ADDRESS PHONE 1755-2082 ALAKE CITY PL 1755-2082 ALAKE CITY PHONE 1755-2082 ALAKE CITY PHONE 1755-2082 ALAKE CITY PL 1755-2082 ALAKE CITY PHONE 1755-2082 ALAKE CITY PL 1755-2082 ALAKE CITY PHONE 340-44-4817 ALAKE CITY PHONE 340-44-4817 ALAKE CITY PL 1755-2082 ALAKE CITY PHONE 340-44-4817 ALAKE CITY PHONE 340-44-4817 ALAKE CITY PL 1755-2082 ALAKE CITY PHONE 340-44-4817 ALAKE ALAKE ALAKE ALAKE CITY PHONE 340-44-4817 ALAKE ALAKE ALAKE CITY PHONE ALAKE ALAKE ALAKE CITY ALAKE ALAKE ALAKE ALAKE ALAKE ALAKE ALAKE CITY ALAKE ALAKE ALAKE ALAKE ALAKE ALAKE ALAKE ALAKE ALAKE CITY ALAKE ALAKE ALAKE ALAKE ALAKE CITY ALAKE ALAKE	APPLICANT	JACOB KIRSCH	PHONE 344	1-4817
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Department of Transportation Permit installation approved standards. Other		 a) a majority of the current and exist b) the driveway to be served will be presented that the concrete or paved driveway, which 	ting driveway turnouts are pav paved or formed with concrete ed a minimum of 12 feet wide never is greater. The width sha	e. or the width of the
Other		Culvert installation shall conform to t	he approved site plan standar	ds.
		Department of Transportation Permit	installation approved standar	ds.
		Other		
ALL PROPER CATEGORY PERMITS CHANGE PROPERTY OF THE CONTROL OF THE				
	ALL BRODER CO	EETV DEGUIDEMENTS SHOW B 22 224	LOWER	<i>4</i> 1 6

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



Project Name:

Climate Zone:

Address:

Owner:

2.

3.

4.

City, State:

Compass Builders - Kezia

Spec House

7. Glass type 1 and area: (Label reqd. by 13-104.4.5 if not default)

(or Single or Double DEFAULT) 7a. (Dble Default) 294.3 ft2

7b.

North

New construction or existing

Single family or multi-family

Number of Bedrooms

5. Is this a worst case?6. Conditioned floor area (ft²)

b. SHGC:

Number of units, if multi-family

(or Clear or Tint DEFAULT)

Lot: \Sub: Rolling Meadows, Plat:

Lake City, FL 32025- 1pt 48

Compass Builders

Cap: 42.0 kBtu/hr

Cap: 42.0 kBtu/hr

HSPF: 7.70

SEER: 13.00

Permitting Office: Columbia County

000155

Permit Number: 27103

Jurisdiction Number:

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

New

Single family

Description Area

(Clear) 294.3 ft²

Builder:

12. Cooling systems

a. Central Unit

13. Heating systems

a. Electric Heat Pump

b. N/A

c. N/A

b. N/A

8. Floor types a. Slab-On-Grade Edge Insulation b. N/A c. N/A 9. Wall types a. Frame, Wood, Exterior b. Frame, Wood, Adjacent R=13.0, 1487.7 ft ² R=13.0, 220.0 ft ²	c. N/A 14. Hot water systems a. Electric Resistance Cap: 50.0 gallons EF: 0.90
c. N/A d. N/A e. N/A 10. Ceiling types a. Under Attic b. N/A c. N/A	c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan,
a. Sup: Unc. Ret: Unc. AH: Garage Sup. R=6.0, 50.0 ft b. N/A Glass/Floor Area: 0.15 Total as-built protated by Total base protections.	PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) points: 25305 points: 25788
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: 1-9-08 1 hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT: DATE:	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes. BUILDING OFFICIAL: DATE:

EnergyGauge® (Version: FLRCPB v4.5.2)

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE					AS-	BUI	LT				
GLASS TYPES .18 X Condition Floor Ar		SPM =	Points	Type/SC		rhang Len	Hgt	Area X	SPI	иx	SOF:	= Points
.18 1914.	0	18.59	6405.0	1.Double, Clear	W	1.5	10.0	105.0	38	3.52	0.98	3959.0
				2.Double, Clear	W	1.5	10.0		38	8.52	0.98	150.0
				3.Double, Clear	N	1.5	10.0	40.0	19	9.20	0.98	754.0
				4.Double, Clear	W	1.5	10.0	6.0	38	3.52	0.98	226.0
				5.Double, Clear	W	1.5	10.0	16.0	38	3.52	0.98	603.0
				6.Double, Clear	N	1.5	8.0	20.0	19	9.20	0.97	371.0
				7.Double, Clear	E	1.5	8.0	30.0	42	2.06	0.96	1208.0
				8.Double, Clear	E	7.5	10.0	13.3	42	2.06	0.59	332.0
				9.Double, Clear	E	1.5	10.0	30.0	42	2.06	0.98	1234.0
				10.Double, Clear	S	1.5	8.0	30.0	35	5.87	0.92	993.0
				As-Built Total:				294.3				9830.0
WALL TYPES	Area X	BSPM	= Points	Туре		R-	Value	e Area	Х	SPI	M =	Points
Adjacent	220.0	0.70	154.0	1. Frame, Wood, Exterior			13.0	1487.7		1.50	11	2231.5
Exterior	1487.7	1.70	2529.1	2. Frame, Wood, Adjacent			13.0	220.0		0.60		132.0
Base Total:	1707.7		2683.1	As-Built Total:				1707.7				2363.5
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	Х	SPI	M =	Points
Adjacent	20.0	2.40	48.0	1.Exterior Insulated				20.0		4.10		82.0
Exterior	20.0	6.10	122.0	2.Adjacent Insulated				20.0		1.60		32.0
Base Total:	40.0		170.0	As-Built Total:				40.0				114.0
CEILING TYPES	Area X	BSPM	= Points	Туре	F	R-Valu	ie A	Area X S	SPM	X S	CM =	Points
Under Attic	1914.0	1.73	3311.2	1. Under Attic		;	30.0	2050.0	1.73 X	(1.00		3546.5
Base Total:	1914.0		3311.2	As-Built Total:				2050.0				3546.5
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	Х	SPI	л =	Points
Slab 2 Raised	240.0(p) 0.0	-37.0 0.00	-8880.0 0.0	1. Slab-On-Grade Edge Insu	lation		5.0	240.0(p	-	36.20		-8688.0
Base Total:			-8880.0	As-Built Total:				240.0				-8688.0
INFILTRATION	Area X	BSPM	= Points					Area	х	SPN	/ =	Points
	1914.0	10.21	19541.9					1914.0)	10.21	1	19541.9

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

	BASE		AS-BUILT	
Summer Ba	se Points:	23231.2	Summer As-Built Points:	26708.0
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credi Component Ratio Multiplier Multiplier Multipli (System - Points) (DM x DSM x AHU)	
23231.2	0.3250	7550.2	(sys 1: Central Unit 42000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6 26708 1.00 (1.09 x 1.147 x 1.00) 0.260 0.950 26708.0 1.00 1.250 0.260 0.95	8247.6

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

BASE		AS-I	BUILT		
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area		verhang nt Len	Hgt Area X	WPM X W	OF = Points
.18 1914.0 20.17 6949.0	1.Double, Clear	W 1.5	10.0 105.0	20.73 1.0	1 2188.0
ž a	2.Double, Clear	W 1.5	10.0 4.0	20.73 1.0	01 83.0
	3.Double, Clear	N 1.5	10.0 40.0	24.58 1.0	00 983.0
	4.Double, Clear	W 1.5	10.0 6.0	20.73 1.0	125.0
	5.Double, Clear	W 1.5	10.0 16.0	20.73 1.0	1 333.0
6	6.Double, Clear	N 1.5	8.0 20.0	24.58 1.0	00 491.0
	7.Double, Clear	E 1.5	8.0 30.0	18.79 1.0	2 574.0
	8.Double, Clear	E 7.5	10.0 13.3	18.79 1.2	C-17
,	9.Double, Clear	E 1.5	10.0 30.0	18.79 1.0	
	10.Double, Clear	S 1.5	8.0 30.0	13.30 1.0	415.0
*	As-Built Total:		294.3		6065.0
WALL TYPES Area X BWPM = Points	Туре	R-\	/alue Area	X WPM	= Points
Adjacent 220.0 3.60 792.0	1. Frame, Wood, Exterior	1	3.0 1487.7	3.40	5058.2
Exterior 1487.7 3.70 5504.5	2. Frame, Wood, Adjacent	1	13.0 220.0	3.30	726.0
Base Total: 1707.7 6296.5	As-Built Total:		1707.7		5784.2
DOOR TYPES Area X BWPM = Points	Туре		Area	X WPM	= Points
Adjacent 20.0 11.50 230.0	1.Exterior Insulated		20.0	8.40	168.0
Exterior 20.0 12.30 246.0	2.Adjacent Insulated		20.0	8.00	160.0
Base Total: 40.0 476.0	As-Built Total:		40.0		328.0
CEILING TYPES Area X BWPM = Points	Туре	R-Value	Area X W	PM X WCM	= Points
Under Attic 1914.0 2.05 3923.7	1. Under Attic	3	0.0 2050.0 2	2.05 X 1.00	4202.5
Base Total: 1914.0 3923.7	As-Built Total:		2050.0		4202.5
FLOOR TYPES Area X BWPM = Points	Туре	R-V	/alue Area	X WPM	= Points
Slab 240.0(p) 8.9 2136.0 Raised 0.0 0.00 0.0	Slab-On-Grade Edge Insulation	1	5.0 240.0(p	7.60	1824.0
Base Total: 2136.0	As-Built Total:		240.0		1824.0
INFILTRATION Area X BWPM = Points			Area	X WPM	= Points
1914.0 -0.59 -1129.3			1914.0	-0.59	-1129.3

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT
Winter Base	Points:	18651.9	Winter As-Built Points: 17074
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heatin Component Ratio Multiplier Multiplier Multiplier Point (System - Points) (DM x DSM x AHU)
18651.9	0.5540	10333.2	(sys 1: Electric Heat Pump 42000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Gar(AH),R6.0 17074.4 1.000 (1.069 x 1.169 x 1.00) 0.443 0.950 8976.9 17074.4 1.00 1.250 0.443 0.950 8976.9

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	E	BASE				AS-BUILT						
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	x	Tank X Ratio	Multiplier	X Credit Multiplie	= Total r
3		2635.00		7905.0	50.0	0.90	3		1.00	2693.56	1.00	8080.7
					As-Built To	otal:						8080.7

	CODE COMPLIANCE STATUS												
		BAS	SE.							AS-	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
7550		10333		7905		25788	8248		8977		8081		25305

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.8

The higher the score, the more efficient the home.

Spec House, Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

1.	New construction or existing	New	_ 12	2.	Cooling systems		
2.	Single family or multi-family	Single family	_	a.	Central Unit	Cap: 42.0 kBtu/hr	
3.	Number of units, if multi-family	1	15 <u>0.110</u> 03			SEER: 13.00	200
4.	Number of Bedrooms	3		b.	N/A		
5.	Is this a worst case?	No					
6.	Conditioned floor area (ft2)	1914 ft²	_	c.	N/A		
7.	Glass type 1 and area: (Label reqd.)	by 13-104.4.5 if not default)					_
a.	U-factor:	Description Area	13	3.	Heating systems		_
	(or Single or Double DEFAULT)	7a. (Dble Default) 294.3 ft ²			Electric Heat Pump	Cap: 42.0 kBtu/hr	
b.	SHGC:	(_		· ·	HSPF: 7.70	
	(or Clear or Tint DEFAULT)	7b. (Clear) 294.3 ft ²		b.	N/A	110111111	_
8.	Floor types	(Clour) 271.3 It	_		2002		
a.	Slab-On-Grade Edge Insulation	R=5.0, 240.0(p) ft		C.	N/A		
b.	N/A	,	_	-			-
c.	N/A		14	1	Hot water systems		
9.	Wall types				Electric Resistance	Cap: 50.0 gallons	
a.	Frame, Wood, Exterior	R=13.0, 1487.7 ft ²				EF: 0.90	
b.	Frame, Wood, Adjacent	R=13.0, 220.0 ft ²		b.	N/A	21.0.70	_
c.	N/A						_
d.	N/A		_	C.	Conservation credits		_
e.	N/A				(HR-Heat recovery, Solar		-
10.	Ceiling types		-		DHP-Dedicated heat pump)		
	Under Attic	R=30.0, 2050.0 ft ²	15		HVAC credits	PT,	
b.	N/A				(CF-Ceiling fan, CV-Cross ventilation,		_
c.	N/A				HF-Whole house fan,		
11.	Ducts		-		PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 50.0 ft			MZ-C-Multizone cooling,		
	N/A		_		MZ-H-Multizone heating)		
					6		
I ce	rtify that this home has complie	ed with the Florida Energy	Efficience	cy	Code For Building		
Con	struction through the above ene	ergy saving features which	will be i	inst	talled (or exceeded)	OF THE STATE	A
in th	is home before final inspection	Otherwise, a new EPL I	Display C	ard	will be completed	13/00 / No.	B
base	ed on installed Code compliant	features	John C	uru	oc completed	5 3	8
			Data				FI
Juli	der Signature:		Date:			13	
Add	ress of New Home:		City/FL	Ziţ	o:	TRUS	Ø
	ore me to the state of		4			W. W.	

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCPB v4.5.2)

Residential System Sizing Calculation

Spec House

Lake City, FL 32025-

Summary
Project Title:
Compass Builders - Kezia

Lot 48

Code Only Professional Version

Climate: North

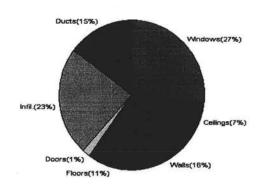
1/10/2008

				1/10/200	0
Location for weather data: Gaine	sville - Def	aults: Latite	ude(29) Altitude(152 ft.) Temp Ran	ge(M)	
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	77F) Humidity difference(54gr.)	J-(/	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	35547	Btuh	Total cooling load calculation	49767	Btuh
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	118.2	42000	Sensible (SHR = 0.75)		31500
Heat Pump + Auxiliary(0.0kW)	118.2	42000	Latent		10500
			Total (Electric Heat Pump)		42000

WINTER CALCULATIONS

Winter Heating Load (for 1914 soft)

Load component			Load	
Window total	294	sqft	9474	Btuh
Wall total	1708	sqft	5608	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	2050	sqft	2416	Btuh
Floor total	240	sqft	3925	Btuh
Infiltration	204	cfm	8270	Btuh
Duct loss			5336	Btuh
Subtotal		- 1	35547	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			35547	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1914 sqft)

Load component			Load	
Window total	294	sqft	19977	Btuh
Wall total	1708	sqft	3435	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	2050	sqft	3395	Btuh
Floor total			0	Btuh
Infiltration	179	cfm	3325	Btuh
Internal gain			3780	Btuh
Duct gain		- 1	6405	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			40708	Btuh
Latent gain(ducts)			1330	Btuh
Latent gain(infiltration)			6528	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occ	upants/othe	r)	1200	Btuh
Total latent gain			9059	Btuh
TOTAL HEAT GAIN			49767	Btuh

Letent Internal(2%) Int.Gain(8%) Ducts(16%) Windows(4D%) Infil.(20%) Doors(4%)



Version 8 For Florida residences only

EnergyGauge® System Sizing PREPARED BY: _ DATE:

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Kezia

Professional Version

Lake City, FL 32025-

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	105.0	32.2	3380 Btul
2	2, Clear, Metal, 0.87	W	4.0	32.2	129 Btu
3	2, Clear, Metal, 0.87	N	40.0	32.2	1288 Btu
4	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btul
5	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btul
6	2, Clear, Metal, 0.87	N	20.0	644 Btul	
7	2, Clear, Metal, 0.87	E	30.0	966 Btul	
8	2, Clear, Metal, 0.87	E	429 Btul		
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btul
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btul
	Window Total		294(sqft)		9474 Btul
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1488	3.3	4886 Btul
2	Frame - Wood - Adj(0.09)	13.0	220	3.3	722 Btul
	Wall Total		1708		5608 Btul
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btul
2	Insulated - Adjacent		20	12.9	259 Btul
	Door Total		40		518Btul
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2050	1.2	2416 Btul
	Ceiling Total		2050		2416Btul
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	240.0 ft(p)	16.4	3925 Btul
	Floor Total	-	240	27	3925 Btul
			Envelope Su	ıbtotal:	21941 Btuh
Infiltration	Туре	ACH X Vol	ume(cuft) walls(sqf	t) CFM=	
	Natural	0.80	15312 1708	204.2	8270 Btul
Ductload			(D	LM of 0.177)	5336 Btul
All Zones		Sens	sible Subtotal Al	I Zones	35547 Btul

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kezia Code Only Professional Version

Climate: North

1/10/2008

Lake City, FL 32025-

Subtotal Sensible	35547 Btuh
Ventilation Sensible	0 Btuh
Total Btuh Loss	35547 Btuh

EQUIPMENT

1. Electric Heat Pump

#

42000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8 For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Spec House

Project Title: Compass Builders - Kezia

Professional Version

Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	105.0	32.2	3380 Btuh
2	2, Clear, Metal, 0.87	W	4.0	32.2	129 Btuh
3	2, Clear, Metal, 0.87	N	40.0	32.2	1288 Btuh
4	2, Clear, Metal, 0.87	W	6.0	32.2	193 Btuh
5 6	2, Clear, Metal, 0.87	W	16.0	32.2	515 Btuh
6	2, Clear, Metal, 0.87	N	20.0	32.2	644 Btuh
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btuh
9	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
10	2, Clear, Metal, 0.87	S	30.0	32.2	966 Btuh
	Window Total		294(sqft)		9474 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1488	3.3	4886 Btuh
2	Frame - Wood - Adj(0.09)	13.0	220	3.3	722 Btuh
	Wall Total		1708		5608 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40	ANDERSON	518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	2050	1.2	2416 Btuh
	Ceiling Total		2050		2416Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	240.0 ft(p)	16.4	3925 Btuh
	Floor Total		240		3925 Btuh
		Z	one Envelope Su	ubtotal:	21941 Btuh
Infiltration	Туре	ACH X Volu	ume(cuft) walls(sqf	t) CFM=	
	Natural	0.80	15312 1708	204.2	8270 Btuh
Ductload	Average sealed, Supply(R6.	0-Attic), Return	n(R6.0-Attic) (D	LM of 0.177)	5336 Btuh
Zone #1		Sens	sible Zone Subto	otal	35547 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

1. Electric Heat Pump

Project Title: Compass Builders - Kezia Code Only Professional Version

Climate: North

1/10/2008

Sensible	35547 Btul
Sensible	0 Btuh 35547 Btuh
	n Sensible n Loss

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (Frame types - metal, wood or insulated metal)

#

(U - Window U-Factor or 'DEF' for default) (HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



42000 Btuh

Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Kezia

Code Only Professional Version

Climate: North

Lake City, FL 32025-

1/10/2008

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

Component Loads for Whole House

	Type*		Over	hang	Win	dow Are	a(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hat	Gross		Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	105.0	0.0	105.0	29	80	8349	Btuh
2	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	4.0	0.0	4.0	29	80	318	Btuh
3	2, Clear, 0.87, None, N, N	N	1.5ft	10ft.	40.0	0.0	40.0	29	29	1158	Btuh
4	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	6.0	0.0	6.0	29	80	477	Btuh
5	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	16.0	0.0	16.0	29	80	1272	Btuh
6	2, Clear, 0.87, None, N, N	N	1.5ft	8ft.	20.0	0.0	20.0	29	29	579	Btuh
7	2, Clear, 0.87, None, N, N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btuh
8	2, Clear, 0.87, None, N, N	E	7.5ft	10ft.	13.3	5.8	7.5	29	80	768	Btuh
9	2, Clear, 0.87, None, N, N	E	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385	Btuh
10	2, Clear, 0.87, None,N,N Excursion	S	1.5ft	8ft.	30.0	30.0	0.0	29	34	869 1415	Btuh Btuh
	Window Total				294 (sqft)				19977	Btuh
Walls	Туре		R-Va	alue/U	I-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	148	37.7		2.1	3103	Btuh
2	Frame - Wood - Adj			13.0/	0.09	22	0.0		1.5	332	Btuh
	Wall Total					170	08 (sqft)			3435	Btuh
Doors	Туре				T-5		(sqft)		НТМ	Load	
1	Insulated - Exterior						0.0		9.8	196	Btuh
2	Insulated - Adjacent						0.0		9.8		6 Btuh
	Door Total						10 (sqft)		5.0		Btuh
Ceilings	Type/Color/Surface		R-Va	alue			(sqft)		нтм	Load	Dian
1	Vented Attic/DarkShingle			30.0			50.0		1.7		Btuh
	Ceiling Total			30.0		77 100 100 100 100 100 100 100 100 100 1			1.7		
Floors	Type		D \/-				0 (sqft)		1174	3395	Dlun
			R-Va				ze		HTM	Load	
1	Slab On Grade			5.0	240 (ft(p))			0.0		0	Btuh
	Floor Total					240	.0 (sqft)			0	Btuh
						E	nvelope :	Subtota	t	27199	Btuh
nfiltration	Туре		Δ	CH	Volum	e(cuft)	wall area	(saft)	CFM=	Load	
	SensibleNatural			0.70	Volulli	15312	wali area 1708	(sqit)	204.2	3325	Btuh
Internal	- Control tatalar		2001:5								blun
		,	Occup				ccupant	F	Appliance	Load	
gain				6		X 23	0 +		2400	3780	Btuh
						S	ensible E	nvelope	Load:	34303	Btuh
Duct load							(DGI	of 0.1	87)	6405	Btuh
						Ser	nsible Lo	ad All	Zones	40708 I	3tuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kezia Code Only Professional Version Climate: North

1/10/2008

WHOLE HOUSE TOTALS

		*	
	Sensible Envelope Load All Zones	34303	Btuh
	Sensible Duct Load	6405	Btuh
	Total Sensible Zone Loads	40708	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	40708	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6528	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1330	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	9059	Btuh
	TOTAL GAIN	49767	Btuh

EQUIPMENT		
1. Central Unit	#	42000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)



Version 8 For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Spec House

Project Title: Compass Builders - Kezia Code Only Professional Version

Climate: North

Lake City, FL 32025-

9

Summer Temperature Difference: 17.0 F

1/10/2008

Component Loads for Zone #1: Main

Reference City: Gainesville (Defaults)

	Type*		Over	hang	Win	dow Area	a(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	105.0	0.0	105.0	29	80	8349	Btuh
2	2, Clear, 0.87, None, N, N	W	1.5ft	10ft.	4.0	0.0	4.0	29	80	318	Btuh
3	2, Clear, 0.87, None,N,N	N	1.5ft	10ft.	40.0	0.0	40.0	29	29	1158	Btuh
4	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	6.0	0.0	6.0	29	80	477	Btuh
5	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	16.0	0.0	16.0	29	80	1272	Btuh
6	2, Clear, 0.87, None,N,N	N	1.5ft	8ft.	20.0	0.0	20.0	29	29	579	Btuh
7	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	Btuh
8	2, Clear, 0.87, None,N,N	E	7.5ft	10ft.	13.3	5.8	7.5	29	80	768	
9	2, Clear, 0.87, None,N,N	E	1.5ft	10ft.	30.0	0.0	30.0	29	80	2385	Btuh
10	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	30.0	30.0	0.0	29	34		Btuh
	Window Total				294 (sqft)				18562	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	148	7.7		2.1	3103	Btuh
2	Frame - Wood - Adj			13.0/	0.09	220	0.0		1.5	332	Btuh
	Wall Total					170	8 (sqft)		(A-14)	3435	Btuh
Doors	Туре					Area		american de la constanta	HTM	Load	
1	Insulated - Exterior					20			9.8	196	Btuh
2	Insulated - Adjacent					20	10.00		9.8	0.00	Btuh
	Door Total					4	0 (sqft)		3.5	392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area	THE RESERVE OF THE PARTY OF THE	-	HTM	Load	
1	Vented Attic/DarkShingle			30.0		205			1.7	3395	Btuh
	Ceiling Total			5710		205	0 (sqft)		1000	3395	Btuh
Floors	Туре		R-Va	alue		Siz	the state of the s	-	HTM	Load	
1	Slab On Grade			5.0		24	10 (ft(p))		0.0	0	Btuh
	Floor Total			-				0.0			Btuh
	Tioor Total		-			240.0 (sqft)					
						Z	one Enve	elope Si	ubtotal:	25784	Btuh
nfiltration	Туре		A	CH	Volum	ne(cuft) v	wall area	(sqft)	CFM=	Load	
	SensibleNatural			0.70		15312	1708	, , ,	178.6	3325	Btuh
Internal		(Occup	pants		Btuh/oc	cupant		Appliance	Load	
gain			0.85	6		X 23			2400	3780	Btul
						Se	ensible E	Envelop	e Load:	32889	Btuh
Duct load	Average sealed, Supply	(R6.0-	Attic),	Retur	n(R6.0	-Attic)		(DGM	of 0.187)	6141	Btul
							Sensib	le Zone	Load	39029	Btuh

The following window Excursion will be assigned to the system loads.

Windows	July excursion for System 1	1415 Btuh
	ACCEPTED TO A CONTROL OF THE CONTRO	n Subtotal: 1415 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Project Title:

Code Only Professional Version

Lake City, FL 32025-

Compass Builders - Kezia

Climate: North

1/10/2008

	Sensible Excursion Load	1679 Btuh
Duct load		264 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kezia

Code Only Professional Version Climate: North

1/10/2008

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	34303	Btul
	Sensible Duct Load	6405	Btuh
	Total Sensible Zone Loads	40708	Btu
	Sensible ventilation	0	Btul
	Blower	0	Btul
Whole House	Total sensible gain	40708	Btu
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	6528	Btu
	Latent ventilation gain	0	Btu
	Latent duct gain	1330	Btu
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btu
	Latent other gain	0	Btu
	Latent total gain	9059	Btu
	TOTAL GAIN	49767	Btu

EQUIPMENT		
1. Central Unit	#	42000 Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

Residential Window Diversity

MidSummer

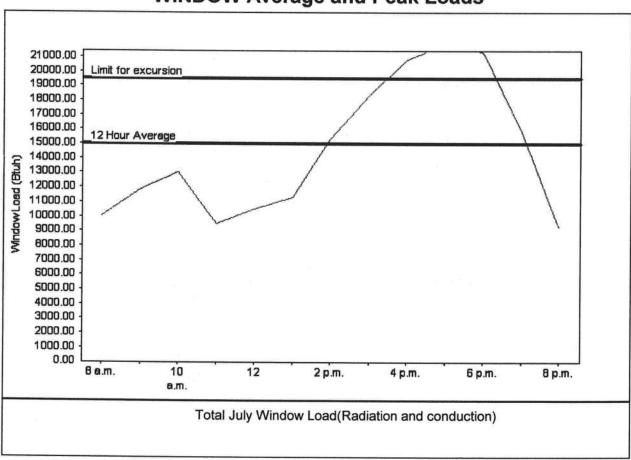
Spec House Lake City, FL 32025-

Project Title: Compass Builders - Kezia Code Only Professional Version Climate: North

1/10/2008

Weather data for: Gainesville - Defi	aults		
Summer design temperature	92 F	Average window load for July	14983 Btu
Summer setpoint	75 F	Peak window load for July	21892 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	19478 Btu
Latitude	29 North	Window excursion (July)	2414 Btuh

WINDOW Average and Peak Loads



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

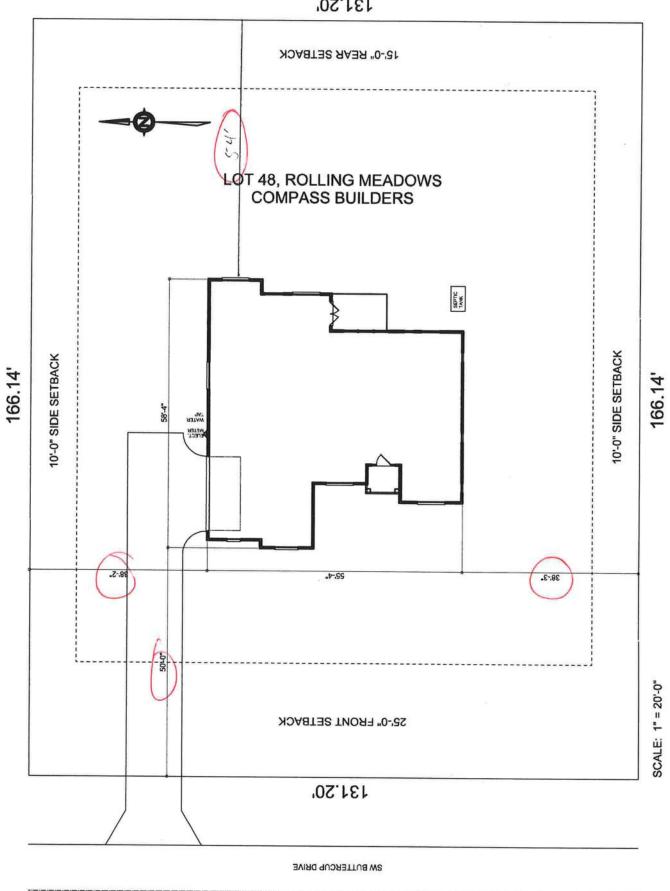
EnergyGauge® System Sizing for Florida residences only

PREPARED BY:

DATE:

EnergyGauge® FLRCPB v4.5.2





15-4S-16-03023-548 LOT 48 ROLLING MEADOWS S/D. WD 1063-1963.WD 1067-2447. CWD 1104-327

COLUMBIA COUNTY 9-1-1 ADDRESSING / **GIS DEPARTMENT**

P. O. Box 1787, Lake City, FL 32056-1787 Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

ADDRESS ASSIGNMENT DATA

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

Residential or Other Structure on Parcel Number: 15-4S-16-03023-548

Address Assignment: 199 SW BUTTERCUP DR, LAKE CITY, FL, 32024

Note: LOT 48 ROLLING MEADOWS S/D

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

Corporate Warranty Deed

This Indenture, made this December 4, 2006 A.D.

Burbach Investment Group, LLC, a Florida Limited Liability Company, whose post office address is: 507 W. Duval Street, Lake City, FL 32055; Grantor and Southeast Developers Group, Inc., a Florida Corporation whose post office address is: 197 SW Waterford Court, Lake City, Florida 32025

Witnesseth, that the said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00), to it in hand paid by the said Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said Grantee forever, the following described land, situate, lying and being in the County of Columbia, State of Florida, to wit:

Lots 7, 48, 50, 52, and 54, ROLLING MEADOWS, according to the Plat thereof, recorded in Plat Book 8, Pages 45 and 46, of the Public Records of Columbia County, Florida.

Subject to taxes for the current year, covenants, restrictions and easements of record, if any.

Parcel Identification Number: R03023-507, R03023-548, R03023-550, R03023-552, and R03023-554

And the said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2006.

In Witness Whereof, the said Grantor has caused this instrument to be executed in its name by its duly authorized officer and caused its corporate seal to be affixed the day and year first above written.

> **Burbach Investment Group, LLC** a Florida Limited Liability Company

> > Thomas P Cady, Manager

Signed and Sealed in Our Presence:

MELINDA WEAVER

State of County of

#06-0389

Matt Rocco

Sierra Title, LLC,

Prepared by & Return to:

Lake City, Florida 32025

Florida Columbia

The foregoing instrument was acknowledged before me this 4th day of December, 2006, by Thomas P Cady, Manager of Burbach Investment Group, LLC, a Florida Limited Liability Company, on behalf of the company. as identification.

He is personally known to me or has produced

Notary Public Notary Printed Name:

My Commission Expires::

619 SW Baya Drive, Suite 102 Inst:2006028778 Date:12/06/2006 Time:12:42

1400.00

Doc Stamg-Deed :

DC,P.DeWitt Cason,Columbia County B: 1104 P:327

Notary Public State of Florida Matthew Rocco

My Commission DD578349

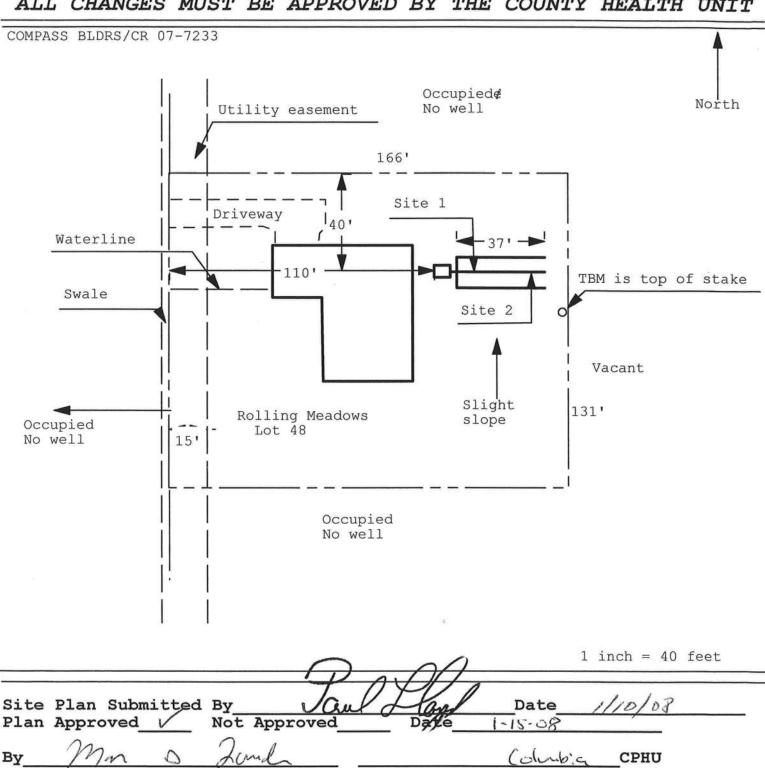
Expires 09/17/2010

(Seal)

18-00 97

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 080 - 57

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Notes:

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
I. EXTERIOR DOORS			
A. SWINGING	Jeld-Wen	Exterior Swinging door	FL- 498-RI
B. SLIDING		70.00	1 - 110 - 1
C. SECTIONAL/ROLL UP	ROYNOR	Paynor	FL-4867
D. OTHER	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		112 1181
2. WINDOWS	<u> </u>		
A. SINGLE/DOUBLE HUNG	MI- Products	Single Hung Window	FL-5108
B. HORIZONTAL SLIDER		3	
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	James Hardie	Hardi Plant Siding	FL-889-RI
B. SOFFITS	Kaycan	Aluminum Soffit	FL- 4957
C. STOREFRONTS			1 - 1137
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS	 		
A. ASPHALT SHINGLES	Elk Roofing	Asphalt shingles	F1 556 D2
B. NON-STRUCT METAL	1	15 photo Shringtes	FL-586-RZ
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A WOOD CONNECTORS	SIMPSON Strong to	Truss STIAPS	C1 - 1/211 81
B. WOOD ANCHORS	1	3/14/51	FL- 474-RI
C. TRUSS PLATES			
D. INSULATION FORMS		<u> </u>	
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
A	***************************************		

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobaite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE DATE

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Applicant	Plans Examine	NTS: Two (2) complete sets of plans containing the following:
Ø∕	0	All drawings must be clear, concise and drawn to scale ("Optional"
8	0	Designers name and signature on decomposition of the same and signature of
4	0	architect or engineer, official seal shall be affixed. Site Plan including:
		a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
à	0	Wind-load Engineering Summary, calculations and any details required
		 a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.
		 c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.
		e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including: a) All sides
	0	-,

d		Alternative size with the transfer of the tran
4	0	d) Location, size and height above roof of chimneys.
t e		e) Location and size of skylights
Z	0	f) Building height
H	u	e) Number of stories
п,		Floor Plan including:
N. C.	0	a) Rooms labeled and dimensioned.
7		b) Shear walls identified.
	0	c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
P		 d) Show safety glazing of glass, where required by code.
Y		e) Identify egress windows in bedrooms, and size.
Y	0	 f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
þ	0	 g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
h		h) Must show and identify accessibility requirements (accessible bathroom)
/_		Foundation Plan including:
		 a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
a 0 0	0	b) All posts and/or column footing including size and reinforcing
		c) Any special support required by soil analysis such as piling
		d) Location of any vertical steel.
	0.000	Roof System;
þ	0	a) Truss package including:
ĺ.		 Truss layout and truss details signed and sealed by Fl. Pro. Eng. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
4	0	b) Conventional Framing Layout including:
C		Rafter size, species and spacing
		2. Attachment to wall and uplift
		Ridge beam sized and valley framing and support details
		 Roof assembly (FBC 106.1.1.2)Roofing systems, materials.
		manufacturer, fastening requirements and product evaluation with
		wind resistance rating)
_		Wall Sections including:
		a) Masonry wall
		All materials making up wall
		 Block size and mortar type with size and spacing of reinforcement Lintel tie-beam sizes and reinforcement
		 Lintel, tie-beam sizes and reinforcement Gable ends with rake beams showing reinforcement or gable truss
		and wall bracing details
		All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation shall be
		designed by a Windload engineer using the engineered roof truss plans.
		Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with resistance rating)
		7. Fire resistant construction (if required)
		8. Fireproofing requirements
		 Shoe type of termite treatment (termiticide or alternative method) Slab on grade
120		a. Vapor retarder (6mil. Polyethylene with joints lapped 6

inches and sealed)

b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports

11. Indicate where pressure treated wood will be placed

12. Provide insulation R value for the following:

		b) Wood frame wall
		All materials making up wall
		2. Size and species of studs
		Sheathing size, type and nailing schedule
		4. Headers sized
		5. Gable end showing balloon framing detail or gable truss and wall
		ninge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer fastening
		requirements and product evaluation with wind resistance esting)
		o. The resistant construction (if applicable)
		9. Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)11. Slab on grade
		a. Vapor retarder (6Mil. Polyethylene with joints lapped 6
		inches and sealed
		b. Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports
		12. Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following: a. Attic space
		b. Exterior wall cavity
		c. Crawl space (if applicable)
		c) Metal frame wall and roof (designed, signed and sealed by Florida Prof
		Engineer of Architect)
		Floor Framing System:
4		a) Floor truss package including layout and details, signed and sealed by Florida
d	0	Registered Professional Engineer b) Floor joist size and spacing
Т	0	c) Girder size and spacing
4	Ö	d) Attachment of joist to girder
ф		e) Wind load requirements where applicable
00000		Plumbing Fixture layout
		Electrical layout including:
		a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
0		b) Ceiling fans
5		c) Smoke detectors
	0	d) Service panel and sub-panel size and location(s)
1	0	e) Meter location with type of service entrance (overhead or underground)
#	0	1) Appliances and HVAC equipment
#		g) Arc Fault Circuits (AFCI) in bedrooms
4		h) Exhaust fans in bathroom
d		HVAC information
<u>d</u>		a) Energy Calculations (dimensions shall match plans)
4	0	b) Manual J sizing equipment or equivalent computation
		c) Gas System Type (LP or Natural) Location and BTU demand of equipment Disclosure Statement for Owner Builders
Φ/		*** Notice Of Commencement Required Before Any Inspections Will Be Done
OX.		Private Potable Water

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)



Project Information for:

L265561

Builder:

Compass Builders

Lot:

48-1

Subdivision:

Rolling Meadows

County: Truss Count: Baker 45

Design Program: MiTek 20/20 6.3 **Building Code:**

FBC2004/TPI2002

Truss Design Load Information:

Gravity: Roof (psf): 42.0 Wind:

Wind Standard: ASCE 7-02

Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for the Structural Engineering: Jacob C. Kirsch Florida License No. CBC1253775

Address: 196 Southwest Huntsview Way Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

No.

Drwg. #

J1925226

J1925227

J1925228

J1925229

J1925230

J1925231

J1925232

J1925233

J1925234

J1925235

J1925236

J1925237

J1925238

J1925239

Notes:

Drwg. #

No.

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Date

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

1/10/08

- 1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
- 2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
- 3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 section 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T22

T23

T24

T25

T26

T27

T28

T29

T30

T31

T32

T33

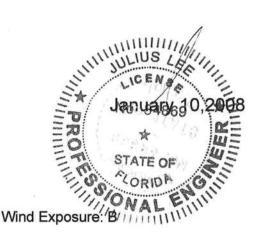
T34

T35

1	J1925195	CJ01	1/10/08	32
2	J1925196	CJ02	1/10/08	33
3	J1925197	CJ03	1/10/08	34
4	J1925198	EJ01	1/10/08	35
5	J1925199	EJ02	1/10/08	36
6	J1925200	EJ03	1/10/08	37
7	J1925201	HJ01	1/10/08	38
8	J1925202	HJ02	1/10/08	39
9	J1925203	P01	1/10/08	40
10	J1925204	P02	1/10/08	41
11	J1925205	T01	1/10/08	42
12	J1925206	T02	1/10/08	43
13	J1925207	T03	1/10/08	44
14	J1925208	T04	1/10/08	45
15	J1925209	T05	1/10/08	
16	J1925210	T06	1/10/08	
17	J1925211	T07	1/10/08	
18	J1925212	T08	1/10/08	
19	J1925213	T09	1/10/08	
20	J1925214	T10	1/10/08	
21	J1925215	T11	1/10/08	
22	J1925216	T12	1/10/08	
23	J1925217	T13	1/10/08	
24	J1925218	T14	1/10/08	
25	J1925219	T15	1/10/08	
26	J1925220	T16	1/10/08	
27	J1925221	T17	1/10/08	
28	J1925222	T18	1/10/08	
29	J1925223	T19	1/10/08	
30	J1925224	T20	1/10/08	
31	J1925225	T21	1/10/08	

Truss ID

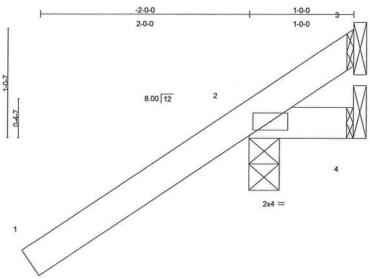
Date



Truss Truss Type Qty Ply 00 J1925195 L265561 CJ01 **JACK** 6 1 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:45 2008 Page 1



LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Matr	rix)						Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=270/0-3-8, 4=5/Mechanical, 3=-100/Mechanical

Max Horz 2=119(load case 6)

Max Uplift 2=-312(load case 6), 3=-100(load case 1)

Max Grav 2=270(load case 1), 4=14(load case 2), 3=161(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/61, 2-3=-90/111

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.32

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 Ib uplift at joint 2 and 100 lb uplift at joint 3. Continued on page 2

January 10,2008

Scale = 1:10.4

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



1.ab	Truss	Truss Type	Qty	Ply	00
L265561	CJ01	JACK	6	1	J1925195
					Job Reference (optional)
Builders First S	ource, Jacksonville	Florida 32244 6.	300 s Feb 15 2006	MiTek Ir	ndustries, Inc. Thu Jan 10 17:38:45 2008 Page 2

LOAD CASE(S) Standard

1

January 10,2008

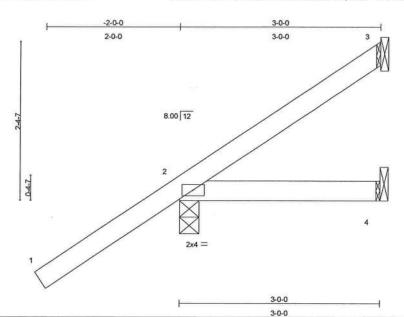


 Job
 Truss
 Truss Type
 Qty
 Ply
 0 0
 J1925196

 L265561
 CJ02
 JACK
 2
 1
 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:46 2008 Page 1



LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES** GRIP (loc) in TCLL 20.0 Plates Increase 1.25 TC 0.33 Vert(LL) -0.002-4 >999 360 MT20 244/190 TCDL 7.0 1.25 BC 0.06 Lumber Increase Vert(TL) -0.012-4 >999 240 BCLL 10.0 * Rep Stress Incr YES WB 0.00 -0.00Horz(TL) 3 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 14 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical

Max Horz 2=179(load case 6)

Max Uplift 3=-33(load case 7), 2=-207(load case 6)

Max Grav 3=34(load case 4), 2=258(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-72/19

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.27

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 207 lb uplift at joint 2. Continued on page 2

Truss Design Engineer Florida PE No. 34889 1 100 Coastal Ray Blvd Boynton Besch. FL 33439

January 10,2008

Scale = 1:16.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



*Job	Truss	Truss Type	Qty	Ply	00
L265561	CJ02	JACK	2	1	J19251
	0002	UNOIL		25	Job Reference (optional)
Builders First S	Source, Jacksonville,	Florida 32244 6	.300 s Feb 15 2006	MiTek Ir	ndustries, Inc. Thu Jan 10 17:38:46 2008 Page 2

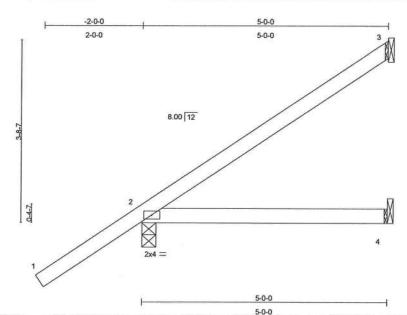
LOAD CASE(S) Standard

January 10,2008



Builders First Source, Jacksonville Florida 32244

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 20 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=101/Mechanical, 2=302/0-3-8, 4=24/Mechanical

Max Horz 2=240(load case 6)

Max Uplift 3=-103(load case 6), 2=-188(load case 6)

Max Grav 3=101(load case 1), 2=302(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-90/43

BOT CHORD 2

2-4=0/0

JOINT STRESS INDEX

2 = 0.30

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 3 and 188 lb uplift at joint 2. Continued on page 2

Hollins Less Truse Design Engineer Florida PE No. 34869 1 109 Ceastal Bay Blvd Boynton Beach, FL 33435

January 10,2008

航 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265561	CJ03	JACK	2	1		J1925197
Will Ver20-EXSENT				0.00	Job Reference (optional)	

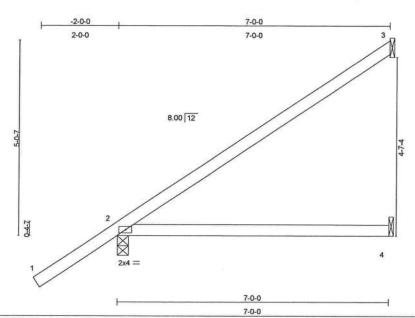
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:46 2008 Page 2

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	0 0	520, 550
L265561	EJ01	MONO TRUSS	13	1		J1925198
4.4.	THE CANADA				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:47 2008 Page 1



LOADING (psf) SPACING 2-0-0 CSI DEFL **PLATES** in (loc) I/defl L/d GRIP TCLL 20.0 Plates Increase 1.25 TC 0.45 Vert(LL) -0.11>730 360 MT20 2-4 244/190 7.0 TCDL Lumber Increase 1.25 BC 0.33 Vert(TL) -0.20240 2-4 >417 **BCLL** 10.0 YES WB 0.00 Rep Stress Incr Horz(TL) -0.00n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 27 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS

(lb/size) 3=164/Mechanical, 2=357/0-3-8, 4=34/Mechanical

Max Horz 2=218(load case 6)

Max Uplift 3=-110(load case 6), 2=-124(load case 6)

Max Grav 3=164(load case 1), 2=357(load case 1), 4=102(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-138/72

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.34

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 3 and 124 lb uplift at joint 2.

Truse Cesian Engineer Florida PE No. 24888 1100 Coastal Bay Blvri Boynton Beach, Ft. 20426

LOAD CASE(S) Standard

January 10,2008

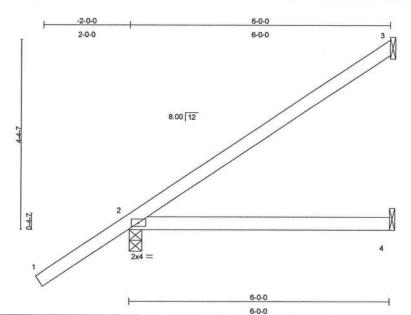
Scale = 1:28.0

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 683 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265561	EJ02	MONO TRUSS	4	1		J1925199
					Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.06	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.10	2-4	>673	240	L. SIEPLIE INNERSIV	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)				0.000.000.00000		Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=133/Mechanical, 2=329/0-3-8, 4=29/Mechanical

Max Horz 2=271(load case 6)

Max Uplift 3=-139(load case 6), 2=-187(load case 6)

Max Grav 3=133(load case 1), 2=329(load case 1), 4=87(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-114/58

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.32

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 Ib uplift at joint 3 and 187 lb uplift at joint 2. Continued on page 2

January 10,2008

Scale = 1:25.0

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

Awaring - verify design parameters and record to the component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265561	EJ02	MONO TRUSS	4	1		J1925199
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:48 2008 Page 2

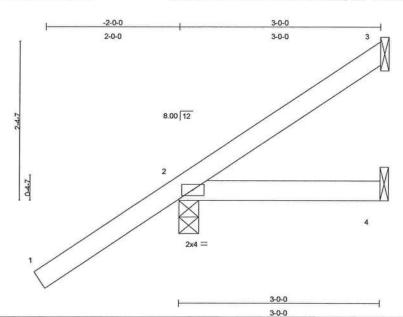
LOAD CASE(S) Standard



Job Truss Type Ply Truss 0.0 Qty J1925200 L265561 **EJ03** MONO TRUSS 1 Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:48 2008 Page 1



LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP in TCLL 20.0 Plates Increase 1.25 TC 0.33 Vert(LL) -0.002-4 >999 360 MT20 244/190 TCDL 7.0 1.25 BC 0.06 Lumber Increase Vert(TL) -0.01 2-4 >999 240 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.003 n/a n/a **BCDL** Code FBC2004/TPI2002 5.0 (Matrix) Weight: 14 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracina.

REACTIONS (lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical

Max Horz 2=179(load case 6)

Max Uplift 3=-33(load case 7), 2=-207(load case 6)

Max Grav 3=34(load case 4), 2=258(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-72/19

BOT CHORD

2-4=0/0

JOINT STRESS INDEX

2 = 0.27

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 207 lb uplift at joint 2. Continued on page 2

January 10,2008

Scale = 1:16.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	2000 - 1000 CO
L265561	EJ03	MONO TRUSS	4	1		J1925200
	35500 ON 1800				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:48 2008 Page 2

LOAD CASE(S) Standard



Job Truss Truss Type Ply Qty 00 J1925201 L265561 HJ01 MONO TRUSS 1 Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:49 2008 Page 1 -2-9-15 4-0-0 8-5-13 2-9-15 4-0-0 4-5-13 Scale = 1:24.9 3x6 = 5.66 12 0-4-7 6 5 3x6 = 8-5-13 4-0-0 4-5-13 LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plates Increase 1.25 TC 0.55 Vert(LL) -0.016-7 >999 360 244/190 MT20 TCDL 7.0 Lumber Increase 1.25 BC 0.19 Vert(TL) -0.04240 6-7 >999 BCLL 10.0 * Rep Stress Incr NO WB 0.12 Horz(TL) 0.00 5 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 40 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. 2 X 4 SYP No.3 **WEBS BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=194/Mechanical, 2=392/0-4-15, 5=150/Mechanical

Max Horz 2=285(load case 5)

Max Uplift 4=-196(load case 5), 2=-231(load case 5), 5=-38(load case 5)

Max Grav 4=194(load case 1), 2=392(load case 1), 5=163(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/65, 2-3=-338/0, 3-4=-108/62 2-7=-132/265, 6-7=-132/265, 5-6=0/0

BOT CHORD

3-6=-292/146, 3-7=0/162

JOINT STRESS INDEX

2 = 0.47, 3 = 0.12, 6 = 0.08 and 7 = 0.11

NOTES

WEBS

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 4, 231 lb uplift at joint 2 and 38 lb uplift at joint 5.

Julius Les Truse Design Engineer Flonda PE No. 34869 1109 Ceastal Bay Blvd Boynton Beach, FL 33435

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

Builders FirstSource

` Job	Truss	Truss Type	Qty	Ply	00	
L265561	HJ01	MONO TRUSS	1	1		J1925201
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:49 2008 Page 2

NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

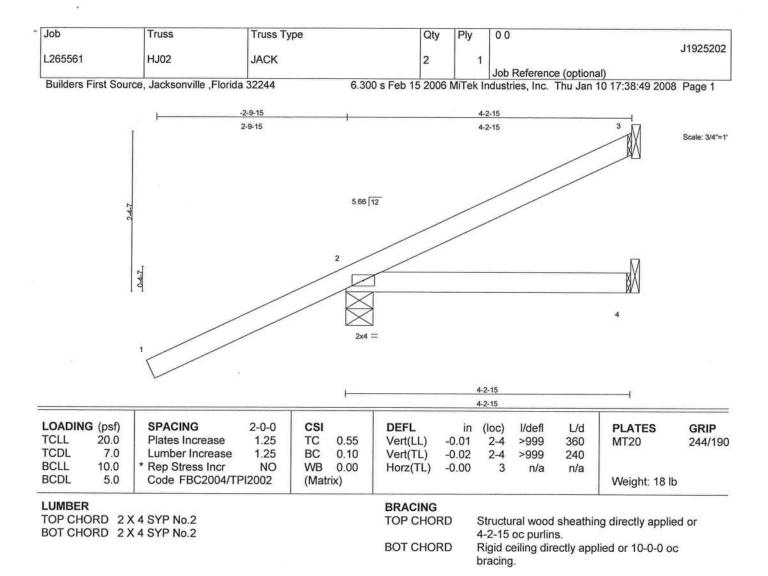
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-4=-115(F=-30, B=-30), 2=0(F=5, B=5)-to-5=-21(F=-6, B=-6)





REACTIONS (lb/size) 3=13/Mechanical, 2=281/0-4-15, 4=14/Mechanical

Max Horz 2=127(load case 5)

Max Uplift 3=-4(load case 6), 2=-252(load case 5)

Max Grav 3=45(load case 3), 2=281(load case 1), 4=54(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-49/19

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.28

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 3 and 252 lb uplift at joint 2.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). Continued on page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



1	Job	Truss	Truss Type	Qty	Ply	00	
	L265561	HJ02	JACK	2	1		J1925202
L						Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:49 2008 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

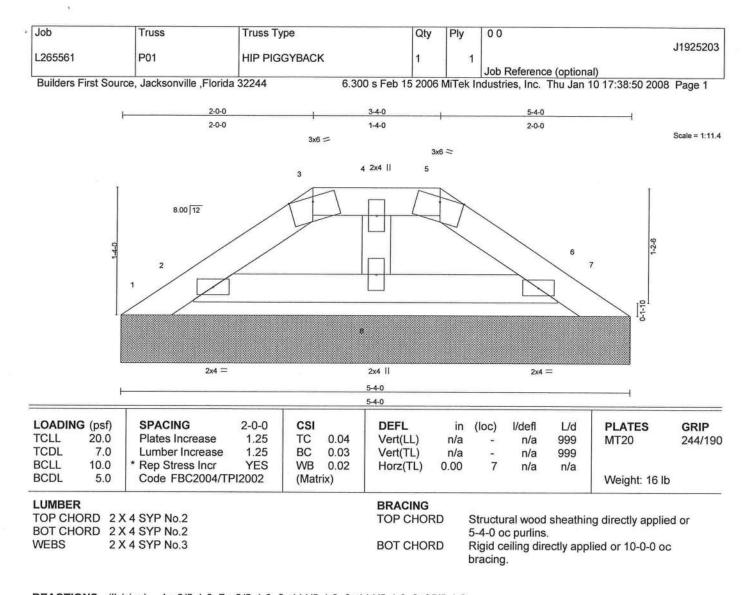
Uniform Loads (plf)

Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-3(F=26, B=26)-to-3=-57(F=-2, B=-2), 2=0(F=5, B=5)-to-4=-11(F=-0, B=-0)

Julius Lee Trues Design Engineer Florida PE No. 24869 1169 Coastal Bay Blvd





REACTIONS (lb/size) 1=-3/5-4-0, 7=-3/5-4-0, 2=111/5-4-0, 6=111/5-4-0, 8=95/5-4-0

Max Horz 1=-33(load case 4)

Max Uplift 1=-44(load case 4), 7=-28(load case 11), 2=-79(load case 5), 6=-63(load

case 4), 8=-16(load case 5)

Max Grav 1=51(load case 5), 7=31(load case 4), 2=138(load case 10), 6=138(load case 11), 8=95(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-53/54, 2-3=-69/52, 3-4=-36/59, 4-5=-36/59, 5-6=-69/52, 6-7=-15/30

BOT CHORD

2-8=-14/36, 6-8=-14/36

WEBS

4-8=-71/69

JOINT STRESS INDEX

2 = 0.14, 3 = 0.02, 4 = 0.04, 5 = 0.02, 6 = 0.14 and 8 = 0.04

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Truse Design Engineer Florida PE No. 34865 1 100 Coastal Bay Blvd Boynton Beach, FL 33436

3) Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	P01	HIP PIGGYBACK	1	1		J1925203
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:50 2008 Page 2

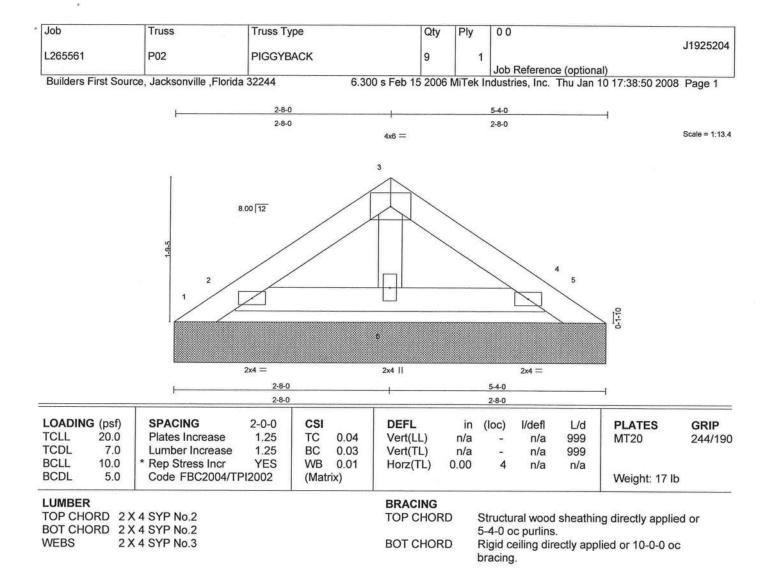
NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 28 lb uplift at joint 7, 79 lb uplift at joint 2, 63 lb uplift at joint 6 and 16 lb uplift at joint 8.
- 8) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida PE No. 34889 1109 Coestel Bay Blvd Boynton Beach, FL 38435





REACTIONS (lb/size) 1=-25/5-4-0, 5=-25/5-4-0, 2=133/5-4-0, 4=133/5-4-0, 6=92/5-4-0

Max Horz 1=-45(load case 4)

Max Uplift 1=-29(load case 4), 5=-25(load case 1), 2=-69(load case 6), 4=-62(load case 7)

case /)

Max Grav 1=43(load case 5), 5=22(load case 7), 2=133(load case 1), 4=133(load case 1), 6=92(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-59/57, 2-3=-48/34, 3-4=-48/34, 4-5=-18/28

BOT CHORD

2-6=-8/29, 4-6=-8/29

WEBS

3-6=-68/29

JOINT STRESS INDEX

2 = 0.13, 3 = 0.02, 4 = 0.13 and 6 = 0.02

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

her

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 Continued on page 2

January 10,2008

🚵 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	Control of the Contro
L265561	P02	PIGGYBACK	9	1		J1925204
	isheavect	CT of Survival and Associate Statement Color			Job Reference (optional)	

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NOTES

4) Gable requires continuous bottom chord bearing.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1, 25 lb uplift at joint 5, 69 lb uplift at joint 2 and 62 lb uplift at joint 4.

7) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard



Job Ply Truss Truss Type Qty 00 J1925205 L265561 T01 **GABLE** 1 Job Reference (optional) Builders First Source, Jacksonville ,Florida 32244 6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:52 2008 Page 1 -2-0-0 5-9-0 11-6-0 13-6-0 2-0-0 5-9-0 Scale = 1:27.9 3x6 = 7 8.00 12 10 3x6 / 3x6 > 11 0-4-7 14-7 5x8 || 5x8 || 11-6-0 11-6-0 Plate Offsets (X,Y): [2:0-3-8,Edge], [7:0-3-0,Edge], [12:0-3-8,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL L/d **PLATES** GRIP (loc) I/defl in 20.0 TCLL Plates Increase 1.25 TC 0.28 Vert(LL) -0.0313 n/r 120 MT20 244/190 1.25 TCDL 7.0 Lumber Increase BC 0.06 Vert(TL) -0.0413 n/r 90 10.0 BCLL Rep Stress Incr YES WB 0.03 Horz(TL) 0.00 12 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 66 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or BOT CHORD 2 X 4 SYP No.2 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc OTHERS 2 X 4 SYP No.3 **BOT CHORD** bracing. REACTIONS (lb/size) 2=244/11-6-0, 12=244/11-6-0, 17=74/11-6-0, 18=94/11-6-0, 19=68/11-6-0, 16=74/11-6-0, 15=94/11-6-0, 14=68/11-6-0 Max Horz 2=-130(load case 4) Max Uplift 2=-213(load case 6), 12=-228(load case 7), 17=-16(load case 5), 18=-113(load case 6), 19=-29(load case 7), 15=-114(load case 7), 14=-24(load case 6) Max Grav 2=244(load case 1), 12=244(load case 1), 17=74(load case 1), 18=96(load case 10), 19=83(load case 2), 16=74(load case 1), 15=96(load case 11), 14=83(load case 2) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-2=0/61, 2-3=-79/78, 3-4=-72/90, 4-5=-49/75, 5-6=-36/118, 6-7=-30/108,

7-8=-30/108, 8-9=-36/118, 9-10=-33/52, 10-11=-31/49, 11-12=-57/37, 12-13=0/61

BOT CHORD 2-19=-9/135, 18-19=-9/135, 17-18=-9/135, 16-17=-9/135, 15-16=-9/135,

14-15=-9/135, 12-14=-9/135

6-17=-62/25, 5-18=-78/110, 4-19=-62/48, 8-16=-62/0, 9-15=-78/110, 10-14=-62/43

JOINT STRESS INDEX

WEBS

2 = 0.45, 3 = 0.00, 3 = 0.16, 4 = 0.03, 5 = 0.06, 6 = 0.03, 7 = 0.13, 8 = 0.03, 9 = 0.06, 10 = 0.03, 11 = 0.00, 11 = 0.16, 12 = 0.000.45, 14 = 0.03, 15 = 0.06, 16 = 0.02, 17 = 0.02, 18 = 0.06 and 19 = 0.03

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T01	GABLE	1	1		J1925205
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:52 2008 Page 2

NOTES

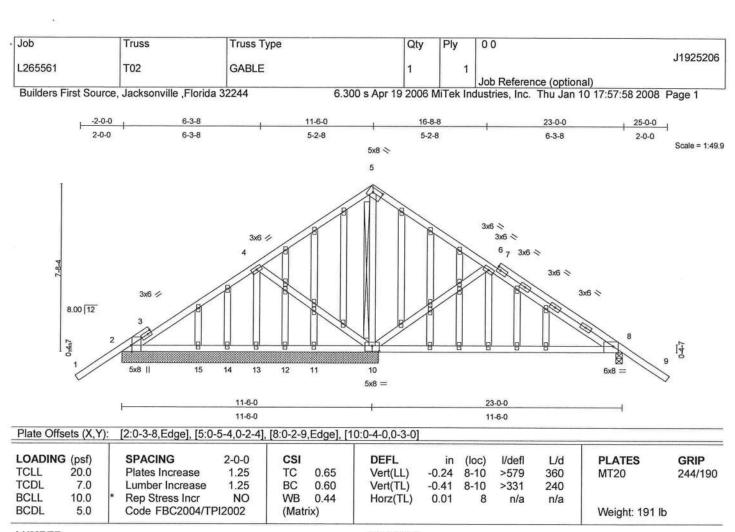
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 228 lb uplift at joint 12, 16 lb uplift at joint 17, 113 lb uplift at joint 18, 29 lb uplift at joint 19, 114 lb uplift at joint 15 and 24 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Florida FE No. 34888 1100 Coastel Bay Blvd





L	U	М	в	Ε	R	

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 **OTHERS** 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 5-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

2=656/11-9-8, 10=1822/11-9-8, 8=400/0-3-8, 11=-48/11-9-8, 12=38/11-9-8, REACTIONS (lb/size) 13=34/11-9-8, 14=-92/11-9-8, 15=195/11-9-8

Max Horz 2=263(load case 5)

Max Uplift 2=-405(load case 6), 10=-843(load case 6), 8=-276(load case 7), 11=-150(load case 2), 13=-12(load case 7), 14=-92(load case 1), 15=-81(load case 7)

Max Grav 2=675(load case 10), 10=1822(load case 1), 8=457(load case 11), 12=133(load case 2), 13=34(load case 2), 14=64(load case 7), 15=195(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-24/128, 2-3=-403/155, 3-4=-256/145, 4-5=-152/510, 5-6=-150/550, 6-7=-172/154,

7-8=-349/158, 8-9=0/62

BOT CHORD 2-15=-166/213, 14-15=-166/213, 13-14=-166/213, 12-13=-166/213, 11-12=-166/213,

10-11=-166/213, 8-10=-36/257

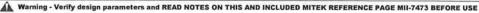
4-10=-581/434, 5-10=-1061/484, 6-10=-563/405

and Jesian Engineer PE No. 34868 Destal Bay Blvd. Destal Bay Blvd.

January 10,2008

Continued on page 2

WEBS



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9	Job	Truss	Truss Type	Qty	Ply	0 0
	L265561	T02	GABLE	1	1	J1925206
9				2		Job Reference (optional)

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JOINT STRESS INDEX

2 = 0.81, 3 = 0.00, 3 = 0.52, 4 = 0.39, 5 = 0.99, 6 = 0.39, 7 = 0.00, 7 = 0.33, 7 = 0.33, 7 = 0.33, 8 = 0.57, 10 = 0.84, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.00, 34 = 0.34, 35 = 0.34, 36 = 0.00 and 36 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 405 lb uplift at joint 2, 843 lb uplift at joint 10, 276 lb uplift at joint 8, 150 lb uplift at joint 11, 12 lb uplift at joint 13, 92 lb uplift at joint 14 and 81 lb uplift at joint 15.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-114(F=-60), 5-7=-141(F=-87), 7-9=-54, 2-8=-10

Julius Lee Truss Design Engineer Florida PE No. 24899 1109 Coestal Bay Blyd Boyoton Basen Et 22425





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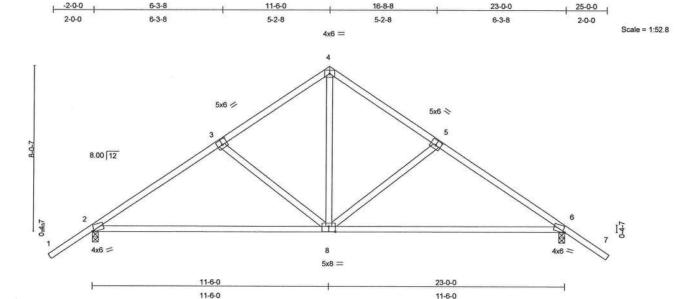


Plate Offsets (X,Y): [2:0-1-1,Edge], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-1-1,Edge], [8:0-4-0,0-3-4]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.26	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.48	2-8	>570	240	10-10-20-20-2	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.25	· Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 113 lb	

 IBA	BE	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-11-12 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=847/0-3-8, 6=847/0-3-8

Max Horz 2=-212(load case 4)

Max Uplift 2=-251(load case 6), 6=-251(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1011/440, 3-4=-744/390, 4-5=-744/390, 5-6=-1011/440, 6-7=0/62

BOT CHORD 2-8=-162/761, 6-8=-162/761

WEBS 3-8=-286/252, 4-8=-237/511, 5-8=-286/252

JOINT STRESS INDEX

2 = 0.83, 3 = 0.41, 4 = 0.43, 5 = 0.41, 6 = 0.83 and 8 = 0.82

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Flonds PE No. 24869 1 109 Ceastel Bay Blvd Boynton Beach, FL 93435

January 10,2008

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Job	Truss	Truss Type	Qty	Ply	00	
L265561	T03	COMMON	6	1		J1925207
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:54 2008 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint 6.

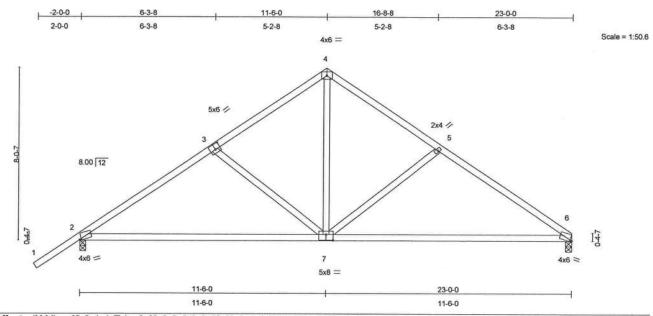
LOAD CASE(S) Standard

Julius Lee Trues Cesign Engineer Flonda PE No. 24869 1109 Ceastel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T04	COMMON	3	1		J1925208
				1 6	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:55 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.26	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.50	6-7	>544	240	111111111111111111111111111111111111111	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	P12002	(Mat	rix)						Weight: 110 lb	

BRACING

TOP CHORD

BOT CHORD

1	H	IN	A	R	F	R
-	u	ш	"	Ь	_	,

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

REACTIONS (lb/size) 6=721/0-3-8, 2=853/0-3-8 Max Horz 2=239(load case 5)

Max Uplift 6=-151(load case 7), 2=-252(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1023/458, 3-4=-756/409, 4-5=-760/411, 5-6=-1015/472

BOT CHORD 2-7=-265/770, 6-7=-291/788

3-7=-286/251, 4-7=-263/515, 5-7=-306/281 **WEBS**

JOINT STRESS INDEX

2 = 0.83, 3 = 0.41, 4 = 0.42, 5 = 0.33, 6 = 0.83 and 7 = 0.83

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Structural wood sheathing directly applied or

Rigid ceiling directly applied or 10-0-0 oc

5-10-5 oc purlins.

bracing.

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters show for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occur. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0 0	10000 200 50000
L265561	T04	COMMON	3	1		J1925208
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:55 2008 Page 2

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 6 and 252 lb uplift at joint 2.

LOAD CASE(S) Standard



		1-		La constitución de la constituci						
Job	Truss	Truss Ty	pe	Qty	Ply	0.0				J1925209
L265561	T05	GABLE		1		1 Joh	Referenc	e (optiona	al)	
Builders First Sc	urce, Jacksonville ,F	orida 32244	6.30	0 s Feb 15 200	06 MiTek	Indust	ries, Inc.	Thu Jan	10 17:38:56 2008	Page 1
-2-	0-0	6-10-0				13-8	-0		15-8-0	
2-	0-0	6-10-0				6-10	-0		2-0-0	Scale: 3/8"=
		8.00 12	5	3x6 = 7	9		20			
946-14	3x6 %	4		9		B	10	3x6 ×	12	0.4-7
Plate Offsets (X	5x8], [7:0-3-0,Edg	e], [12:0-3-8,Edg	13-8-0 13-8-0 ge]				5.	(8	13
COADING (psf) FCLL 20.0 FCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increas Lumber Increa * Rep Stress Inc Code FBC200	se 1.25 or YES	CSI TC 0.27 BC 0.05 WB 0.02 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.02 -0.03 0.00	(loc) 13 13 12	I/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 79 lb	GRIP 244/19
BOT CHORD 2	2 X 4 SYP No.2 2 X 4 SYP No.2 2 X 4 SYP No.3	z.		BRACING TOP CHO	RD RD	6-0-0 c	c purlins eiling dir		ng directly applie	ed or
N N	16=94/13- Max Horz 2=155(loa Max Uplift 2=-194(loa 18=-97(loa 14=-66(loa Max Grav 2=245(loa 18=60(loa	8-0, 15=59/13- d case 5) ad case 6), 12= ad case 6), 19= ad case 6) d case 10), 12= d case 10), 19=	-71(load case 7	-8-0 7), 17=-14(load), 15=-98(load) 11), 17=94(load)	ad case d case 7 ad case	5),), 1),	13-8-0,		130	
ORCES (Ib) - OP CHORD OT CHORD	Maximum Compres 1-2=0/61, 2-3=-94/ 7-8=-15/122, 8-9=- 2-19=-18/157, 18-1 14-15=-18/157, 12-	92, 3-4=-81/10 13/138, 9-10=- 9=-18/157, 17-	6, 4-5=-35/92, 5 23/82, 10-11=-3	4/59, 11-12=-	46/45, 1	2-13=0	0/61	ulius L russ D lorida 100 C oyntor	esign Engi PE No. 248 nastal Ray 1 Deach, Fi	neer sou Blvd _ 3343

JOINT STRESS INDEX

WEBS

2 = 0.44, 3 = 0.00, 3 = 0.16, 4 = 0.06, 5 = 0.05, 6 = 0.04, 7 = 0.15, 8 = 0.04, 9 = 0.05, 10 = 0.06, 11 = 0.00, 11 = 0.16, 12 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.0.44, 14 = 0.05, 15 = 0.05, 16 = 0.03, 17 = 0.03, 18 = 0.05 and 19 = 0.05

6-17=-79/23, 5-18=-54/90, 4-19=-124/88, 8-16=-79/0, 9-15=-54/90, 10-14=-124/91

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T05	GABLE	1	1		J1925209
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:56 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 2, 211 lb uplift at joint 12, 14 lb uplift at joint 17, 97 lb uplift at joint 18, 71 lb uplift at joint 19, 98 lb uplift at joint 15 and 66 lb uplift at joint 14.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L265561	Т06	GABLE	1	1		J1925210
	19.000		12		Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:58:55 2008 Page 1

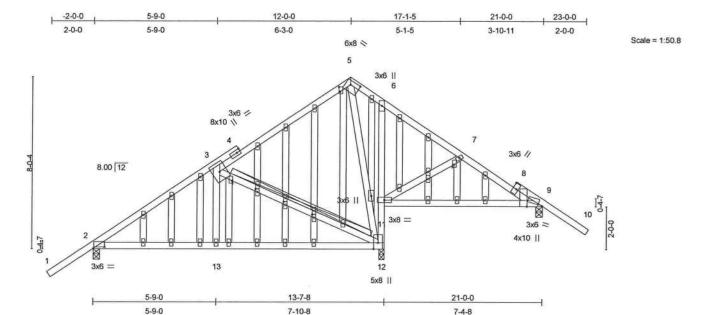


Plate Offsets (X,Y): [2:0-6-3,0-0-10], [5:0-5-4,0-2-4], [7:0-1-8,0-0-12], [8:Edge,0-2-0], [9:0-2-6,0-0-3], [9:0-0-4,Edge], [25:0-2-11,0-0-12], [26:0-1-12,0-1-0], [29:0-1-12,0-1-0], [32:0-1-12,0-1-0], [36:0-1-8,0-0-4]

LOADIN	G (psf)	1	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.84	Vert(LL)	0.10	9-11	>857	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.14	12-13	>999	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mati	rix)						Weight: 194 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

6-12 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 5-7-8 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

2 X 4 SYP No.3 - 3-12 T-Brace:

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=687/0-3-8, 9=731/0-3-8, 12=1295/0-3-0

Max Horz 2=266(load case 5)

Max Uplift 2=-452(load case 6), 9=-757(load case 7), 12=-894(load case 6) Max Grav 2=687(load case 1), 9=734(load case 11), 12=1295(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-836/628, 3-4=-301/371, 4-5=-148/314, 5-6=-181/226, 6-7=-132/278,

7-8=-322/490, 8-9=-502/597, 9-10=-80/131

BOT CHORD 2-13=-452/650, 12-13=-453/649, 11-12=-382/467, 6-11=-152/115, 9-11=-329/338

WEBS 3-13=0/217, 3-12=-585/506, 7-11=-383/483, 5-12=-701/590 esion Chaineer PE No. 34868 Pestal Bay Blvd n Besch, ft 36436

JOINT STRESS INDEX

2 = 0.63, 3 = 0.62, 4 = 0.00, 4 = 0.52, 5 = 1.00, 6 = 0.25, 7 = 0.34, 7 = 0.66, 8 = 0.00, 8 = 0.68, 9 = 0.88, 9 = 0.57, 11 = 0.88, 12 = 0.30, 12 = 0.30, 13 = 0.68, 13 = 0.68, 14 = 0.88, 1413 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.16, 23 = 0.34, 24 = 0.34, 250.34, 25 = 0.71, 26 = 0.40, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.40, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.40, 32 = 0.34, 3334 = 0.34, 35 = 0.34, 36 = 0.46, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34 and 42 = 0.34

January 10,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek conner Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



- Job	Truss	Truss Type	Qty	Ply	00	
L265561	T06	GABLE	1	1		J1925210
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:58:55 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 452 lb uplift at joint 2, 757 lb uplift at joint 9 and 894 lb uplift at joint 12.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

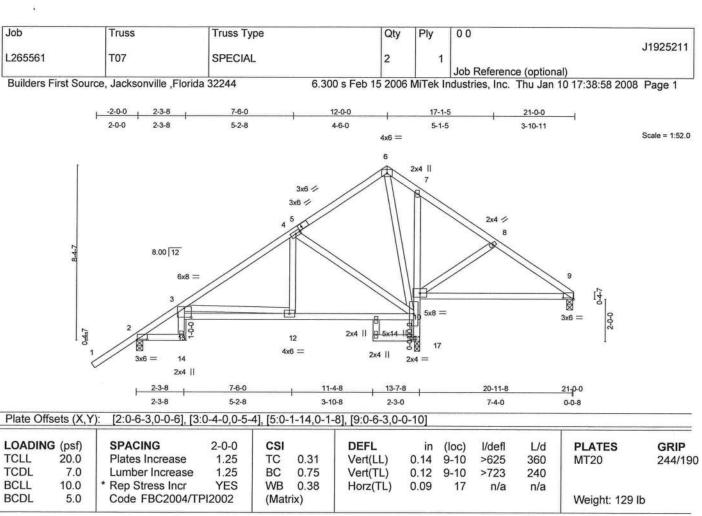
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-141(F=-87), 5-6=-141(F=-87), 6-10=-114(F=-60), 2-12=-10, 9-11=-10

Julius Lee Truse Design Engineer Flonda PE No. 248e9 1100 Coestal Bay Blyd Boynton Beach, Et. 22426





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-14 2 X 4 SYP No.3, 15-16 2 X 4 SYP No.3

7-17 2 X 4 SYP No.3

WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

2 X 4 SYP No.3

REACTIONS (lb/size) 9=79/0-3-8, 2=470/0-3-8, 17=896/0-3-0

Max Horz 2=246(load case 5)

Max Uplift 9=-180(load case 4), 2=-152(load case 6), 17=-343(load case 6) Max Grav 9=160(load case 11), 2=473(load case 10), 17=896(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-434/56, 3-4=-350/69, 4-5=-153/115, 5-6=-151/197, 6-7=-173/238,

7-8=-153/290, 8-9=-175/191

BOT CHORD 2-14=-106/276, 13-14=-4/66, 3-13=0/118, 12-13=-310/880, 11-12=-46/232,

11-17=-896/698, 10-11=-323/424, 7-10=-158/81, 9-10=-139/124

WEBS 4-12=-6/252, 3-12=-650/292, 8-10=-204/256, 4-11=-417/266, 6-11=-334/222

JOINT STRESS INDEX

2 = 0.64, 3 = 0.62, 4 = 0.41, 5 = 0.63, 6 = 0.27, 7 = 0.37, 8 = 0.33, 9 = 0.61, 10 = 0.62, 11 = 0.24, 12 = 0.24, 13 = 0.00, 14 = 0.000.72, 15 = 0.33, 16 = 0.33 and 17 = 0.33

 Unbalanced roof live loads have been considered for this design. Continued on page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the abilding code. For general guidance regarding storage, delivery, et and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T07	SPECIAL	2	1		J1925211
					Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 9, 152 lb uplift at joint 2 and 343 lb uplift at joint 17.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonda FE No. 34869 1109 Ceastal Bay Blvd Bovnton Besch, FL 33435



Job	Truss	Truss Type	Qty	Ply	00	
L265561	тов	SPECIAL	3	1		J1925212
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:59 2008 Page 1

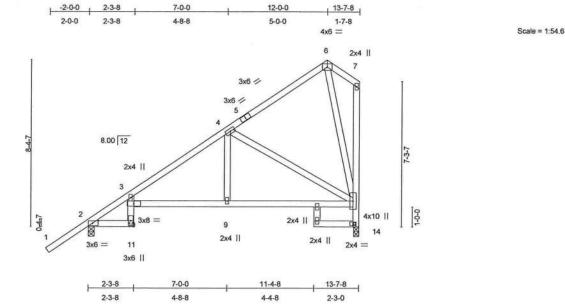


Plate Of	fsets (X, Y	′): [2:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	0.18	9-10	>897	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.23	9-10	>691	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.12	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 90 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-11 2 X 4 SYP No.1D, 12-13 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 *Except*

7-14 2 X 4 SYP No.2

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 9-10-15 oc

bracing.

REACTIONS (lb/size) 2=557/0-3-8, 14=417/0-3-0

Max Horz 2=306(load case 6)

Max Uplift 2=-155(load case 6), 14=-157(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-569/22, 3-4=-555/160, 4-5=-131/12, 5-6=-104/33, 6-7=-27/44,

8-14=-417/273, 7-8=-41/30

BOT CHORD 2-11=-243/374, 10-11=-12/72, 3-10=-18/89, 9-10=-325/444, 8-9=-325/444

WEBS 4-9=-23/254, 4-8=-471/346, 6-8=-169/152

JOINT STRESS INDEX

2 = 0.55, 3 = 0.66, 4 = 0.19, 5 = 0.23, 6 = 0.27, 7 = 0.17, 8 = 0.34, 9 = 0.18, 10 = 0.75, 11 = 0.28, 12 = 0.00, 13 = 0.00 and 14 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Colffice designed for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee Truse Design Engineer Flonda PE No. 34869 1 100 Ceastel Bay Blyd Boynton Beach, FL 33436

January 10,2008

Warming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0 0	
L265561	тов	SPECIAL	3	1		J1925212
			-		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:38:59 2008 Page 2

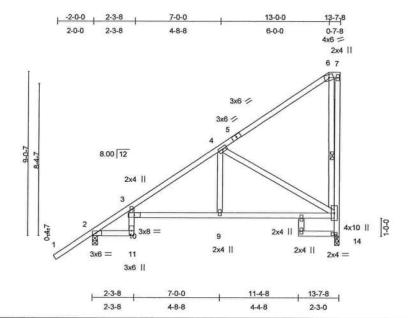
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2 and 157 lb uplift at joint 14.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	100000000000000000000000000000000000000
L265561	Т09	SPECIAL	3	1		J1925213
		0. 201.12		,	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:00 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.18	9-10	>898	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.23	9-10	>706	240	1000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.12	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0					Weight: 93 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

3-11 2 X 4 SYP No.1D, 12-13 2 X 4 SYP No.3

2 X 4 SYP No.3 *Except* **WEBS**

7-14 2 X 4 SYP No.2

BRACING

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 9-7-10 oc

1 Row at midpt

bracing.

7-14

REACTIONS (lb/size) 2=557/0-3-8, 14=417/0-3-0

Max Horz 2=348(load case 6)

Max Uplift 2=-131(load case 6), 14=-193(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-568/0, 3-4=-558/89, 4-5=-128/0, 5-6=-73/52, 8-14=-417/322,

7-8=-199/159, 6-7=-10/6

BOT CHORD 2-11=-257/373, 10-11=-9/72, 3-10=-15/84, 9-10=-346/450, 8-9=-346/450

WEBS 4-9=-20/255, 4-8=-491/373, 6-8=-314/362

JOINT STRESS INDEX

2 = 0.53, 3 = 0.66, 4 = 0.19, 5 = 0.26, 6 = 0.52, 7 = 0.32, 8 = 0.32, 9 = 0.18, 10 = 0.73, 11 = 0.28, 12 = 0.00, 13 = 0.00 and 14 = 0.28, 12 = 0.00, 13 = 0.00 and 14 = 0.28, 12 = 0.00, 13 = 0.00, 13 = 0.00, 14 = 0.00, 15 == 0.00

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Repride adequate drainage to prevent water ponding.

January 10,2008

Scale = 1:59.9

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T09	SPECIAL	3	1		J1925213
					Job Reference (optional)	

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NOTES

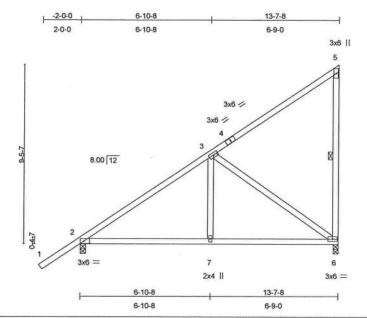
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 2 and 193 lb uplift at joint 14.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	2222-02000-20-20-01-12
L265561	T10	MONO TRUSS	6	1		J1925214
	0.77-00				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:00 2008 Page 1



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.04	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.08	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 80 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS

1 Row at midpt

5-6

REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=358(load case 6)

Max Uplift 6=-205(load case 6), 2=-125(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-525/0, 3-4=-129/0, 4-5=-90/69, 5-6=-146/156

BOT CHORD 2-7=-271/354, 6-7=-271/354 WEBS 3-7=0/229, 3-6=-414/316

JOINT STRESS INDEX

2 = 0.41, 3 = 0.17, 4 = 0.21, 5 = 0.30, 6 = 0.22 and 7 = 0.16

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Florida PE No. 24869 1199 Coastal Bay Blvd Boynton Beach, FL 22435

January 10,2008

Scale = 1:57.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265561	T10	MONO TRUSS	6	1		J1925214
			"		Job Reference (optional)	

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NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 205 lb uplift at joint 6 and 125 lb uplift at joint 2.

LOAD CASE(S) Standard

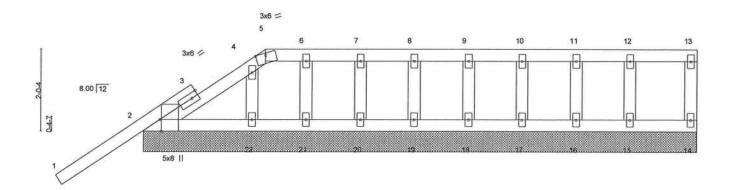


Truss	Truss Type	Qty	Ply	00
T11	GABLE	1	1	J1925215
	0,1522	9.50		Job Reference (optional)
	T11	T11 GABLE	T11 GABLE 1	T11 GABLE 1 1

-2-0-0 3-0-0 13-7-8

10-7-8

Scale = 1:26.7



13-7-8 13-7-8

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.00	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 66 lb	

LUMBERTOP CHORD2 X 4 SYP No.2TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.BOT CHORD2 X 4 SYP No.3BOT CHORDRigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 14=45/13-7-8, 2=241/13-7-8, 22=84/13-7-8, 21=86/13-7-8, 20=85/13-7-8, 19=85/13-7-8, 18=85/13-7-8, 17=86/13-7-8, 16=83/13-7-8, 15=94/13-7-8

Max Horz 2=166(load case 6)

Max Uplift 14=-30(load case 4), 2=-211(load case 6), 22=-25(load case 7), 21=-73(load case 4), 20=-49(load case 4), 19=-54(load case 4), 18=-53(load case 4), 17=-53(load case 4), 16=-53(load case 4)

15=-55(load case 4)
Max Grav 14=45(load case 1), 2=241(load case 1), 22=94(load case 2), 21=86(load case 1), 20=85(load case 1), 19=85(load case 1), 18=85(load case 1

17=86(load case 1), 16=83(load case 1), 15=94(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-48/12, 3-4=-24/24, 4-5=-26/10, 5-6=-4/6, 6-7=-4/6, 7-8=-4/6,

8-9=-4/6, 9-10=-4/6, 10-11=-4/6, 11-12=-4/6, 12-13=-4/6, 13-14=-37/32

BOT CHORD 2-22=-6/4, 21-22=-6/4, 20-21=-6/4, 19-20=-6/4, 18-19=-6/4, 17-18=-6/4, 16-17=-6/4,

15-16=-6/4, 14-15=-6/4

4-22=-73/44, 6-21=-71/75, 7-20=-72/59, 8-19=-72/61, 9-18=-72/61, 10-17=-72/61,

11-16=-70/60, 12-15=-81/67

Julius Lee Truse Cesion Engineer Plorida PE No. 34865 1 109 Coesial Bay Blvd Boynton Beach, FL 93406

Continued on page 2

WEBS

2-0-0

3-0-0



Job	Truss	Truss Type	Qty	Ply	00	W/ TO SECTION OF THE SECTION OF T
L265561	T11	GABLE	1	1		J1925215
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	The state of the s				Job Reference (optional)	

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JOINT STRESS INDEX

2 = 0.44, 3 = 0.00, 3 = 0.15, 4 = 0.04, 5 = 0.04, 6 = 0.04, 7 = 0.03, 8 = 0.03, 9 = 0.03, 10 = 0.03, 11 = 0.03, 12 = 0.04, 13 = 0.07, 14 = 0.04, 15 = 0.04, 16 = 0.03, 17 = 0.03, 18 = 0.03, 19 = 0.03, 20 = 0.03, 21 = 0.04 and 22 = 0.03

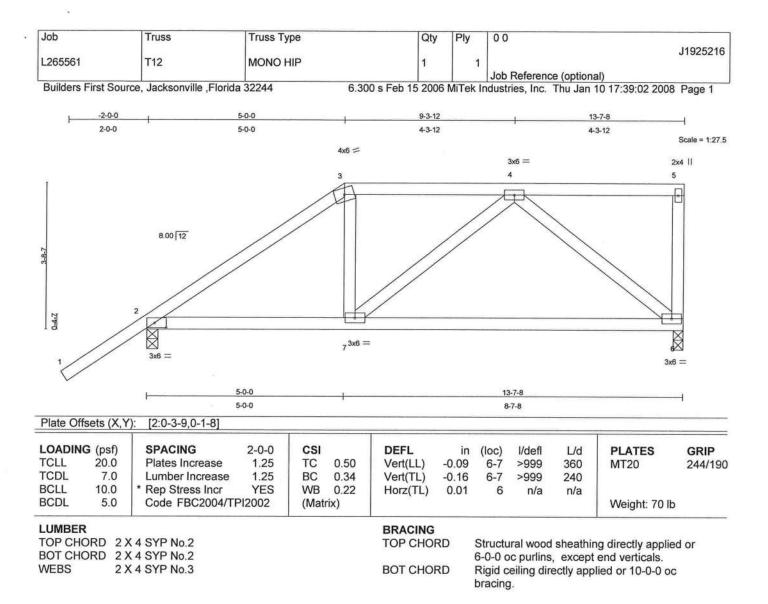
NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 14, 211 lb uplift at joint 2, 25 lb uplift at joint 22, 73 lb uplift at joint 21, 49 lb uplift at joint 20, 54 lb uplift at joint 19, 53 lb uplift at joint 18, 53 lb uplift at joint 17, 53 lb uplift at joint 16 and 55 lb uplift at joint 15.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda PE No. 34868 1109 Cessiel Bay Blvd Boynton Besch, FL 23435





REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=177(load case 6)

Max Uplift 6=-117(load case 4), 2=-177(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-567/216, 3-4=-402/233, 4-5=-53/4, 5-6=-106/83

BOT CHORD 2-7=-236/400, 6-7=-237/358

WEBS 3-7=-1/146, 4-7=-10/132, 4-6=-422/301

JOINT STRESS INDEX

2 = 0.39, 3 = 0.34, 4 = 0.14, 5 = 0.54, 6 = 0.62 and 7 = 0.09

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Florida PE No. 34868 1109 Coastal Bay Blvd Boynton Beach, FL 33435

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



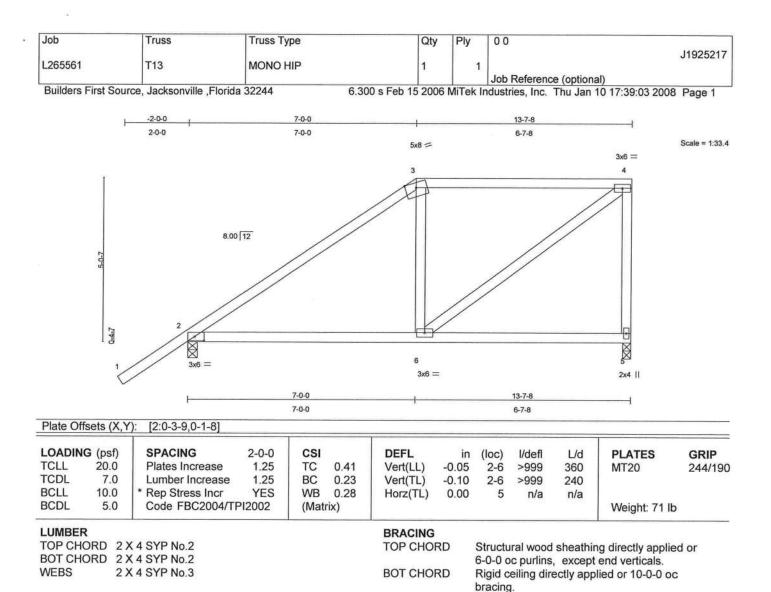
Job	Truss	Truss Type	Qty	Ply	0 0	
L265561	T12	MONO HIP	1	1	J	J1925216
					Job Reference (optional)	

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 6 and 177 lb uplift at joint 2.

LOAD CASE(S) Standard





REACTIONS (lb/size) 5=417/0-3-0, 2=557/0-3-8

Max Horz 2=220(load case 6)

Max Uplift 5=-118(load case 5), 2=-177(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-517/167, 3-4=-340/224, 4-5=-386/276

BOT CHORD 2-6=-230/345, 5-6=-21/30

WEBS 3-6=-155/202, 4-6=-258/387

JOINT STRESS INDEX

2 = 0.46, 3 = 0.68, 4 = 0.63, 5 = 0.53 and 6 = 0.22

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

January 10,2008

A Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	0.0	
L265561	T13	MONO HIP	1	1		J1925217
		Inco Toronto Incon			Job Reference (optional)	

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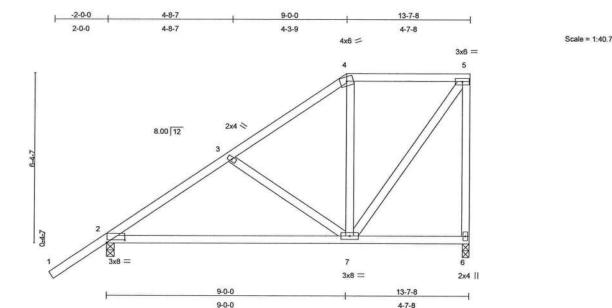
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 5 and 177 lb uplift at joint 2.

LOAD CASE(S) Standard





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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.13	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.23	2-7	>706	240	3173	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	roceteven benings:					Weight: 83 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=263(load case 6)

Max Uplift 6=-119(load case 5), 2=-170(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-544/153, 3-4=-332/104, 4-5=-220/145, 5-6=-402/276

BOT CHORD 2-7=-308/391, 6-7=-8/9

WEBS 3-7=-204/192, 4-7=-109/150, 5-7=-237/363

JOINT STRESS INDEX

2 = 0.70, 3 = 0.10, 4 = 0.42, 5 = 0.31, 6 = 0.24 and 7 = 0.36

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Florida PE No. 34869 1 199 Coastal Bay Blvd Boynton Beach, Et. 33435

January 10,2008

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Job	Truss	Truss Type	Qty	Ply	00	
L265561	T14	MONO HIP	1	1		J1925218
		IIIONO TIII			Job Reference (optional)	*

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 6 and 170 lb uplift at joint 2.

LOAD CASE(S) Standard





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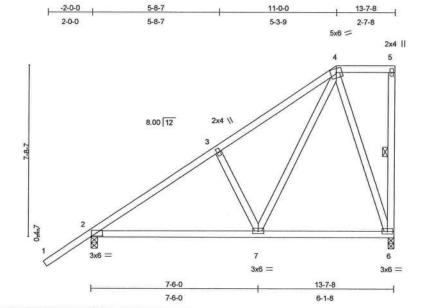


Plate Of	fsets (X,)	(): [2:0-6-3,0-0-6]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.06	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.11	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)	,					Weight: 87 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS 1 Row at midpt 5-6

REACTIONS (lb/size) 6=417/0-3-0, 2=557/0-3-8

Max Horz 2=305(load case 6)

Max Uplift 6=-146(load case 6), 2=-154(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-537/75, 3-4=-404/137, 4-5=-6/0, 5-6=-30/40 **BOT CHORD** 2-7=-294/375, 6-7=-100/124

3-7=-250/255, 4-7=-193/330, 4-6=-382/316 **WEBS**

JOINT STRESS INDEX

2 = 0.62, 3 = 0.14, 4 = 0.31, 5 = 0.14, 6 = 0.23 and 7 = 0.27

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall hearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

January 10,2008

Scale = 1:48.8

航 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265561	T15	MONO HIP	1	1		J1925219
110000010000000000000000000000000000000					Job Reference (optional)	

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NOTES

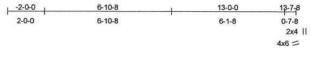
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 6 and 154 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	0 0	100 5.7 (5.8) (5.4)
L265561	T16	MONO HIP	1	1		J1925220
		Control of the Contro			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:05 2008 Page 1



Scale = 1:58.9

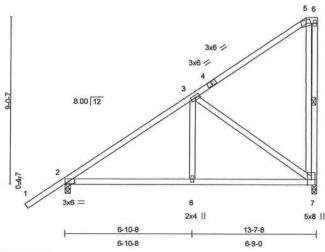


Plate Of	fsets (X,)	(): [2:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.08	2-8	>999	240	100000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	And reconstitution (1998)					Weight: 91 lb	

1	1	п	M	IR	E	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING

WEBS

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

BOT CHORD

bracing.

1 Row at midpt

5-7

REACTIONS (lb/size) 7=417/0-3-0, 2=557/0-3-8

Max Horz 2=348(load case 6)

Max Uplift 7=-193(load case 6), 2=-131(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/62, 2-3=-522/0, 3-4=-129/0, 4-5=-76/55, 5-6=-11/8, 6-7=-195/162

BOT CHORD

2-8=-266/349, 7-8=-266/349

WEBS

3-8=0/230, 3-7=-398/297, 5-7=-322/362

JOINT STRESS INDEX

2 = 0.40, 3 = 0.17, 4 = 0.19, 5 = 0.53, 6 = 0.39, 7 = 0.17 and 8 = 0.16

NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Champearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

Trues Design Engineer Florida PE No. 24899 1 100 Chastal Bay Blyd Boynton Beach, FL 33435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	522.50
L265561	T16	MONO HIP	1	1		J1925220
					Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 7 and 131 lb uplift at joint 2.

LOAD CASE(S) Standard

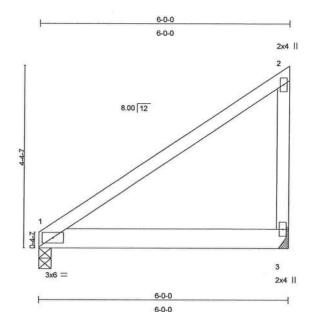


 Job
 Truss
 Truss Type
 Qty
 Ply
 0 0
 J1925221

 L265561
 T17
 MONO TRUSS
 1
 1
 1
 Job Reference (optional)

Builders First Source, Jacksonville , Florida 32244

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Simpson HTU26

LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.06	1-3	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.12	1-3	>554	240	100000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	,					Weight: 30 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=613/0-3-8, 3=613/Mechanical

Max Horz 1=124(load case 5)

Max Uplift 1=-135(load case 5), 3=-219(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-86/69, 2-3=-154/110

BOT CHORD 1-3=0/0

JOINT STRESS INDEX

1 = 0.07, 2 = 0.07 and 3 = 0.06

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 lb uplift at joint 1 and 219 lb uplift at joint 3.
- 5) Girder carries tie-in span(s): 11-8-0 from 0-0-0 to 6-0-0
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Trues Design Engineer Florida PE No. 34869 1100 Ceastal Bay Blyd Boynton Beach, Ft. 33436

January 10,2008

Scale = 1:26.1

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building occ. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0.0	2000
L265561	T17	MONO TRUSS	1	1		J1925221
	100000				Job Reference (optional)	

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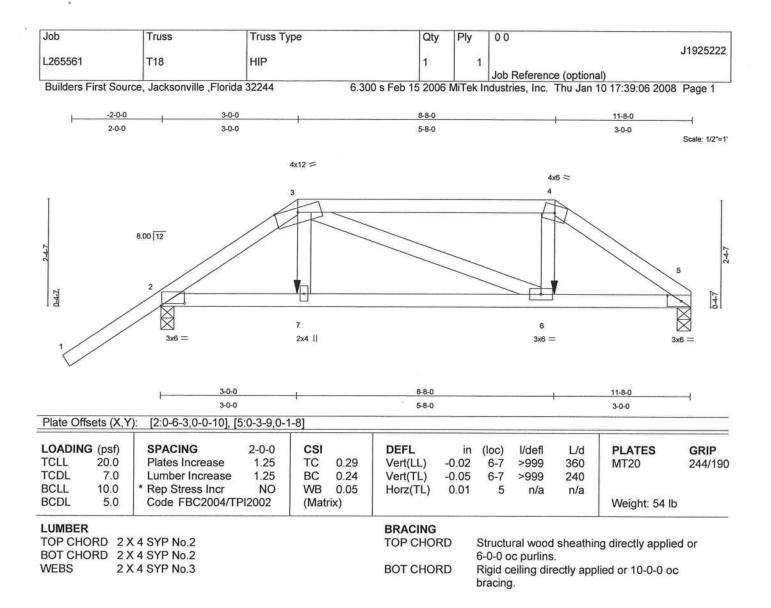
LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-161(F=-151), 1-2=-54





REACTIONS (lb/size) 5=432/0-3-8, 2=576/0-3-8

Max Horz 2=92(load case 5)

Max Uplift 5=-132(load case 3), 2=-218(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-648/214, 3-4=-551/218, 4-5=-679/226

BOT CHORD 2-7=-209/499, 6-7=-208/504, 5-6=-173/545

WEBS 3-7=0/159, 3-6=-70/63, 4-6=-1/164

JOINT STRESS INDEX

2 = 0.66, 3 = 0.63, 4 = 0.43, 5 = 0.34, 6 = 0.10 and 7 = 0.11

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

 *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Less Truss Design Engineer Florida PE No. 34869 1199 Ceastal Bay Blvd Boynton Beach, FL 93435

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265561	T18	HIP	1	1		J1925222
		N 2001			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:06 2008 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 5 and 218 lb uplift at joint 2.
- 7) Girder carries hip end with 3-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-64(F=-10), 4-5=-54, 2-7=-10, 6-7=-12(F=-2), 5-6=-10

Concentrated Loads (lb)

Vert: 7=-48(F) 6=-48(F)

dulius Lee Truss Design Engineer Florida PE No. 34888 1 109 Coastal Bay Blyd



lob	Truss	Truss Type	Qty	Ply	0.0	
265561	T19	MONO HIP	1	1		J1925223
		77.255.25.25.25.25.	'		Job Reference (option	al)
Builders First	Source, Jacksonville,	Florida 32244	6.300 s Feb 15 200	6 MiTek I	Industries, Inc. Thu Jan	10 17:39:06 2008 Page 1
V.	-2-0-0	5-0-0			11-8-0	
	2-0-0	5-0-0			6-8-0	
			4x12 =		10.00	3x6 = Scale = 1:27.1
						6x8
19			3			4
1						
			MAK			
	8.00	12		//		Simpson HTU
-8-7						
7						11 1
	,	//				5-0-0
	2/					
7-4-7						
			100			
	3x6 =		6			5
1/			2x4			3x6 =
	6 3				nia de carrar	
	I	5-0-0 5-0-0			11-8-0 6-8-0	
					300	
late Offsets	(X,Y): [2:0-3-9,0-1-	-81				

LUMBER

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

20.0

7.0

10.0

5.0

OTHERS

2 X 4 SYP No.3

BRACING

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

L/d

360

240

n/a

I/defl

>999

>999

n/a

PLATES

Weight: 61 lb

MT20

GRIP

244/190

Rigid ceiling directly applied or 10-0-0 oc

bracing.

5-6

5-6

7

in (loc)

-0.03

-0.05

-0.01

REACTIONS (lb/size) 2=487/0-3-8, 7=351/Mechanical

SPACING

Plates Increase

Rep Stress Incr

Lumber Increase

Code FBC2004/TPI2002

Max Horz 2=177(load case 6)

Max Uplift 2=-168(load case 6), 7=-100(load case 5)

2-0-0

1.25

1.25

YES

CSI

TC

BC

WB

(Matrix)

0.30

0.16

0.24

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-446/166, 3-4=-59/42, 5-7=-77/195, 4-7=-171/151

BOT CHORD 2-6=-203/308, 5-6=-200/310 **WEBS**

3-6=0/182, 3-5=-286/180

JOINT STRESS INDEX

2 = 0.48, 3 = 0.74, 4 = 0.22, 5 = 0.71, 6 = 0.13, 7 = 0.00 and 7 = 0.19

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall bearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

lius Lee Use Design Engineer Inda PE No. 34866 09 Geastel Bay Blvd Pynton Beson, FL 33435

January 10,2008

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00	
L265561	T19	MONO HIP	1	1		J1925223
					Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 2 and 100 lb uplift at joint 7.

LOAD CASE(S) Standard



Job	Truss	Truss Type		Qty	Ply	0.0				14005004
L265561	T20	MONO HIP)	1	1					J1925224
						Job	Referenc	e (optiona	1)	
Builders First S	Source, Jacksonville ,Flor	ida 32244	6.30	0 s Feb 15 2006	MiTek	Industr	ies, Inc.	Thu Jan 1	0 17:39:07 2008	Page 1
	-2-0-0		7-0-0			44.6				
	2-0-0		7-0-0			11-8 4-8	1,001			
				4x12	=		-			Scale = 1:33.6
								6x8		
	1			3	_			4		
	1 3x6 =	8.00 12		6 2x4	11			3x6 =		on HTU26
			7-0-0	74		11-8	-0			
	1		7-0-0	- 1		4-8-				
Plate Offsets (X,Y): [2:0-3-9,0-1-8]									
I late Ullacta I	1									
LOADING (ps: TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	Plates Increase Lumber Increase * Rep Stress Incr	YES	CSI TC 0.34 BC 0.26 WB 0.24 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.05 -0.11 0.01	(loc) 2-6 2-6 7	I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 66 lb	GRIP 244/190

bracing.

REACTIONS (lb/size) 2=487/0-3-8, 7=351/Mechanical

Max Horz 2=220(load case 6)

Max Uplift 2=-163(load case 6), 7=-101(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-388/93, 3-4=-5/14, 5-7=-177/269, 4-7=-82/85

BOT CHORD 2-6=-169/235, 5-6=-167/237

2 X 4 SYP No.3

WEBS 3-6=0/205, 3-5=-349/247

JOINT STRESS INDEX

2 = 0.48, 3 = 0.97, 4 = 0.10, 5 = 0.17, 6 = 0.14, 7 = 0.00 and 7 = 0.19

NOTES

OTHERS

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Chall bearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

ulius Les russ Cesign Engineer 100 Salett 199 Gnestel Bay Blvd 199 Gnestel By Blvd

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	Hele on Websita
L265561	T20	MONO HIP	1	1		J1925224
	1.77				Job Reference (optional)	

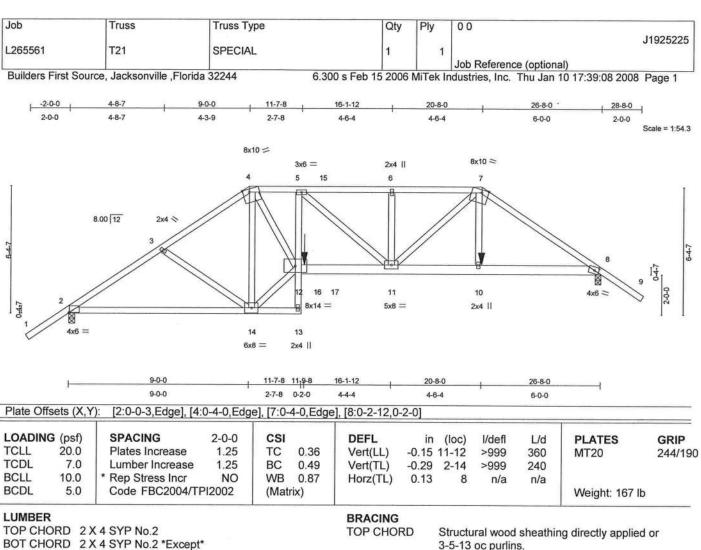
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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 2 and 101 lb uplift at joint 7.

LOAD CASE(S) Standard





5-13 2 X 4 SYP No.3, 8-12 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3

3-5-13 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

REACTIONS

BOT CHORD

(lb/size) 2=1549/0-3-8, 8=1754/0-3-8

Max Horz 2=161(load case 5)

Max Uplift 2=-502(load case 5), 8=-628(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-2274/780, 3-4=-2075/765, 4-5=-3055/1226, 5-15=-2929/1212,

6-15=-2929/1212, 6-7=-2930/1211, 7-8=-2660/1018, 8-9=0/65

2-14=-721/1807, 13-14=-37/60, 12-13=-53/4, 5-12=-144/222, 12-16=-1226/3115,

16-17=-1221/3101, 11-17=-1222/3100, 10-11=-804/2141, 8-10=-800/2129

3-14=-169/116, 4-14=-1322/565, 4-12=-1062/2630, 5-11=-248/46, 6-11=-502/365,

7-11=-510/1114, 7-10=-137/350, 12-14=-811/2162

JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.51, 5 = 0.45, 6 = 0.33, 7 = 0.66, 8 = 0.82, 10 = 0.33, 11 = 0.51, 12 = 0.64, 13 = 0.43 and 14 = 0.78

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

8) Prayide adequate drainage to prevent water ponding.

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265561	T21	SPECIAL	1	1		J1925225
					Job Reference (optional)	

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NOTES

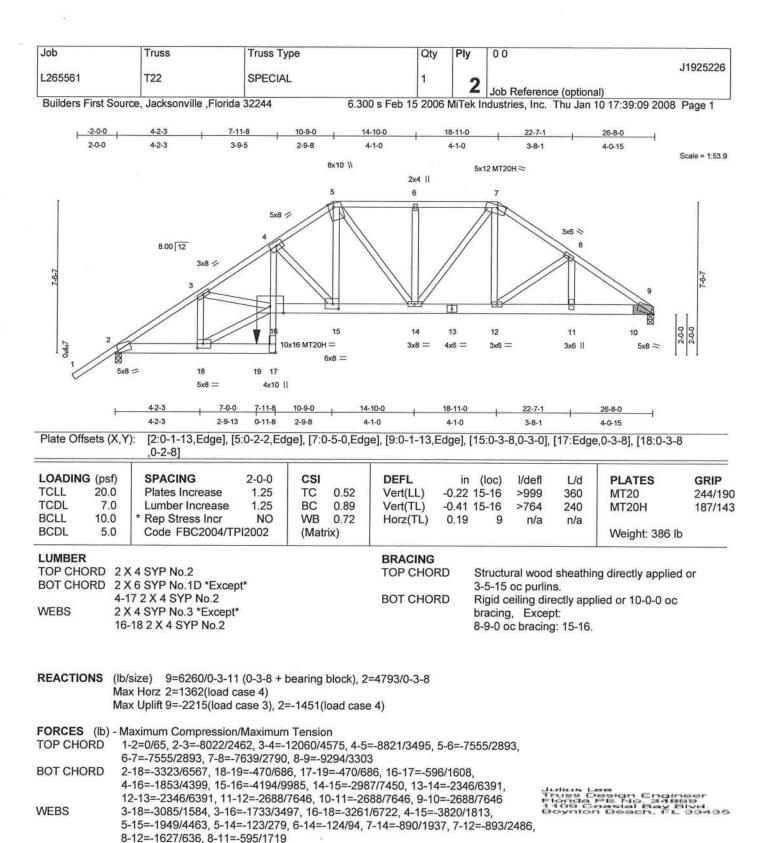
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 2 and 628 lb uplift at joint 8.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 - Uniform Loads (plf)
 - Vert: 1-4=-54, 4-15=-54, 7-15=-104(F=-50), 7-9=-54, 2-13=-10, 12-17=-10, 10-17=-19(F=-9), 8-10=-10
 - Concentrated Loads (lb)
 - Vert: 10=-288(F) 16=-613(F)

Julius Les Truse Design Engineer Pionda PE No. 24869 1160 Coastel Ray Blyd Boynton Respirit 1944





JOINT STRESS INDEX

2 = 0.72, 3 = 0.86, 4 = 0.72, 5 = 0.93, 6 = 0.33, 7 = 0.95, 8 = 0.64, 9 = 0.82, 9 = 0.00, 9 = 0.00, 10 = 0.00, 10 = 0.00, 10 = 0.00, 11 = 0.27, 12 = 0.79, 13 = 0.90, 14 = 0.96, 15 = 0.60, 16 = 0.68, 17 = 0.78 and 18 = 0.74

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T22	SPECIAL	1			J1925226
	,	0. 201.2		2	Job Reference (optional)	

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NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc, 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) 2 X 6 SYP No.1D bearing block 12" long at jt. 9 attached to back face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners per block. Bearing is assumed to be SYP.
- 4) Unbalanced roof live loads have been considered for this design.
- 5) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 6) Provide adequate drainage to prevent water ponding.
- 7) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2215 lb uplift at joint 9 and 1451 lb uplift at joint 2.
- 11) Girder carries tie-in span(s): 33-4-0 from 22-6-8 to 26-8-0; 25-8-0 from 8-0-0 to 22-6-8

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

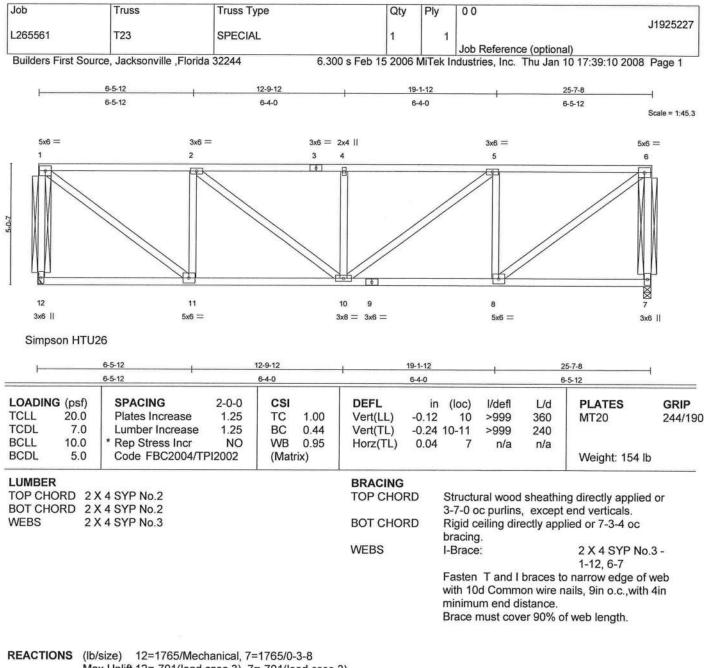
Vert: 1-5=-54, 5-7=-54, 7-9=-54, 2-17=-10, 11-16=-384(F=-374), 9-11=-507(F=-497)

Concentrated Loads (lb)

Vert: 19=-1765(F)

Julius Lee Truss Design Engineer Flonda FE No. 24869 1 100 Gessis! Bay Blvd Bovnton Beach, Et. 22426





Max Uplift 12=-701(load case 3), 7=-701(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-12=-1686/729, 1-2=-1867/741, 2-3=-2445/970, 3-4=-2445/970, 4-5=-2445/970,

5-6=-1867/741, 6-7=-1686/729

BOT CHORD 11-12=-30/75, 10-11=-741/1867, 9-10=-741/1867, 8-9=-741/1867, 7-8=-30/75 WEBS

1-11=-889/2239, 2-11=-1217/632, 2-10=-286/722, 4-10=-724/419, 5-10=-286/722,

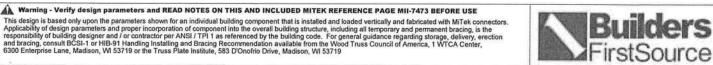
5-8=-1217/632, 6-8=-889/2239

JOINT STRESS INDEX

1 = 0.70, 2 = 0.42, 3 = 0.32, 4 = 0.33, 5 = 0.42, 6 = 0.70, 7 = 0.39, 8 = 0.70, 9 = 0.60, 10 = 0.68, 11 = 0.70 and 12 = 0.39

January 10,2008

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	0 0	Ya Wata Tanan
L265561	T23	SPECIAL	1	1		J1925227
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:10 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 701 lb uplift at joint 12 and 701 lb uplift at joint 7.
- 6) Girder carries hip end with 0-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

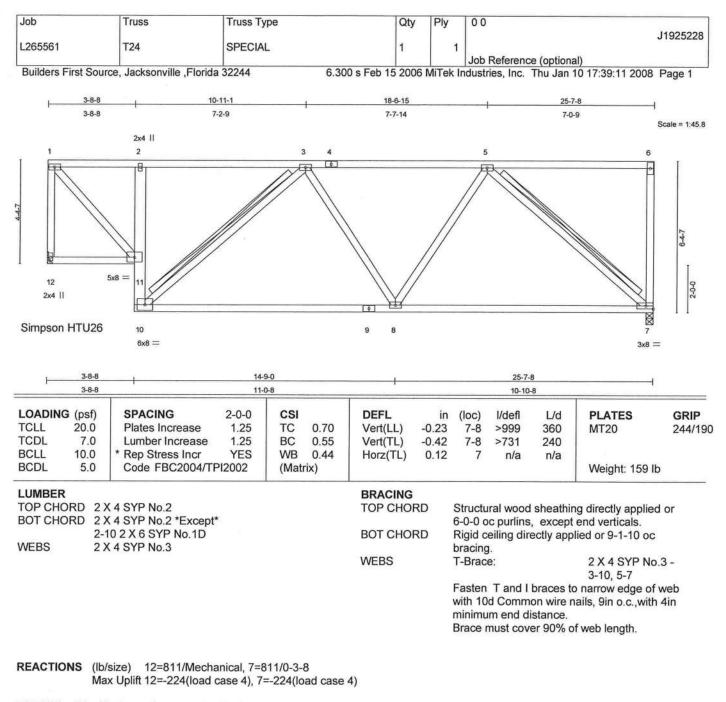
LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-6=-118(F=-64), 7-12=-22(F=-12)

Julius Lee Truss Design Engineer Florida PE No. 24865 1109 Coastal Bay Blyd Boynton Beach, Ft. 25436





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-12=-726/386, 1-2=-629/321, 2-3=-478/236, 3-4=-815/393, 4-5=-815/393,

5-6=-43/9, 6-7=-167/116

BOT CHORD 11-12=-13/25, 10-11=-169/400, 2-11=-296/215, 9-10=-458/865, 8-9=-458/865,

7-8=-378/704

WEBS 1-11=-452/887, 3-10=-513/294, 3-8=-94/122, 5-8=-28/308, 5-7=-891/492

Julius Lee Trues Cesian Engineer Flonda PE No. 24869 1169 Ceastal Bay Blyd Goynton Beach, FL 33435

JOINT STRESS INDEX

1 = 0.57, 2 = 0.33, 3 = 0.42, 4 = 0.25, 5 = 0.42, 6 = 0.34, 7 = 0.57, 8 = 0.42, 9 = 0.52, 10 = 0.68, 11 = 0.56 and 12 = 0.70

January 10,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty Ply 00
L265561	T24	SPECIAL	J1925228
The state of the s			Job Reference (optional)
Builders First S	Source Jacksonville	Florida 32244	6 300 s Feb 15 2006 MiTek Industries Inc. Thu Ian 10 17:30:11 2009 Dags 2

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 12 and 224 lb uplift at joint 7.

LOAD CASE(S) Standard



L265561	T25						14005555
		SPECIAL	1	1			J1925229
Builders First Source	, Jacksonville ,Florida	32244 6	300 s Feb 15 2006 MiT	Job Reference		7-20-12 2000	Dogg 1
Duliders First Source	, Jacksonville ,i londa	32244 0.	300 S FED 13 2000 WILL	ek maustries, mc.	Thu Jan 10 17	7.39.12 2008	Page 1
———	10-11-0		18-7-0		25-7-8		
	10-11-0 2x4		7-8-0		7-0-8		Scale: 1/4"=1"
1	2	3 4		5		6	
12 2x4	ix8 = 11						2-0-0
Cimpon UTU26	84		(4)			<u> </u>	1 1
Simpson HTU26	10 4x10 =		9 8			7	
	4x10 —					3x8 =	
3-8-8		14-9-0 11-0-8		25-7-8 10-10-8			
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 * BCDL 5.0	SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TP	2-0-0 CSI 1.25 TC 0.65 1.25 BC 0.51 YES WB 0.48	DEFL Vert(LL) -0.2 Vert(TL) -0.4 Horz(TL) 0.1	n (loc) I/defl 4 7-8 >999 2 7-8 >720	360 M 240 n/a	PLATES 1T20 Veight: 173 I	GRIP 244/190
2-10	SYP No.2 SYP No.2 *Except* 2 X 6 SYP No.1D SYP No.3	1	BRACING TOP CHORD BOT CHORD WEBS	Structural wood 6-0-0 oc purlins Rigid ceiling dir bracing. T-Brace: Fasten T and I with 10d Comm minimum end d Brace must cov	ectly applied 2 3 braces to narron wire nails, istance.	verticals. or 10-0-0 oc X 4 SYP No -10, 3-8, 5-7 rrow edge of 9in o.c.,with	5.3 - 6-7, f web

Max Uplift 12=-224(load case 4), 7=-224(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-12=-731/387, 1-2=-476/243, 2-3=-393/194, 3-4=-669/323, 4-5=-669/323,

5-6=-33/6, 6-7=-164/114

BOT CHORD 11-12=-9/17, 10-11=-168/398, 2-11=-300/218, 9-10=-376/709, 8-9=-376/709,

7-8=-311/577

WEBS 1-11=-408/801, 3-10=-461/264, 3-8=-88/115, 5-8=-26/295, 5-7=-809/447

Truss Design Engineer Florida PE No. 34860 1100 Crastal Bay Blvd Governor Beach Et 23435

JOINT STRESS INDEX

1 = 0.59, 2 = 0.33, 3 = 0.44, 4 = 0.24, 5 = 0.44, 6 = 0.31, 7 = 0.53, 8 = 0.44, 9 = 0.54, 10 = 0.95, 11 = 0.54 and 12 = 0.61





Job	Truss	Truss Type	Qty	Ply	00	
L265561	T25	SPECIAL	1	1		J1925229
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:12 2008 Page 2

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 12 and 224 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida PE No. 34868 1199 Coastal Bay Blyd



Job	Truss	Truss Type		Qty	Ply	0.0		14005000
_265561	T26	SPECIAL		1	1			J1925230
Builders First S	Source, Jacksonville ,F	Florida 32244	6 300 s Feb 15	2006	MiTek In	Job Reference (optiondustries, Inc. Thu Ja	n 10 17:30:13 2	008 Page 1
randoro i not c	ourse, suchostivine ,i	iorida ozzara	0.000 3 1 05 10	2000	WII I CK II	idustries, mc. Thu Ja	11 10 17.59.15 2	ooo rage i
	2-0-0	9-1-8	16-3-0	-		23-4-0	25-7-8	
	2-0-0	7-1-8	7-1-8			7-1-0	2-3-8	020000000000
8.0	00 12 5x6 =	2x4	3x6 =	3x6 =	=	4x12 =	=	Scale = 1:5
	. 2	3	4	5		6		
2.0.7	2x4						3x6 × 7	9-6-7

10 Simpson HTU26 3x6 II 3x8 = 2x4 || 9-1-8 16-3-0 25-7-8 23-4-0 9-1-8 7-1-0 2-3-8 1 /2/ 2/

12

3x8 =

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	12-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.22	12-13	>999	240	120012-0220	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 204 lb	

LU	184		_	
	IIVI	к	-	ĸ

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

13

3x8 =

5-10 2 X 4 SYP No.3

WEBS

2 X 4 SYP No.3 *Except*

1-13 2 X 4 SYP No.2, 7-8 2 X 4 SYP No.2

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing. Except:

T-Brace:

2 X 4 SYP No.3 -

5-11

WEBS

T-Brace:

2 X 4 SYP No.3 -5-12, 6-9, 7-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=811/0-3-8, 13=811/Mechanical

Max Horz 13=-40(load case 4)

Max Uplift 8=-261(load case 4), 13=-264(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-2=-52/61, 2-3=-742/428, 3-4=-742/428, 4-5=-742/428, 5-6=-740/438,

6-7=-245/139, 1-13=-73/65, 7-8=-804/398

BOT CHORD 12-13=-117/230, 11-12=-374/744, 10-11=0/101, 5-11=-397/274, 9-10=-2/30,

2-12=-346/709, 3-12=-399/286, 5-12=-7/25, 6-11=-387/749, 6-9=-677/393,

9-11=-98/191, 7-9=-335/702, 2-13=-823/481

Continued on page 2

WEBS

January 10,2008

🚵 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T26	SPECIAL	1	1		J1925230
	1	J. 2011.2	ļ		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:13 2008 Page 2

JOINT STRESS INDEX

1 = 0.48, 2 = 0.59, 3 = 0.33, 4 = 0.27, 5 = 0.35, 6 = 0.78, 7 = 0.54, 8 = 0.33, 9 = 0.87, 10 = 0.31, 11 = 0.50, 12 = 0.68 and 13 = 0.61

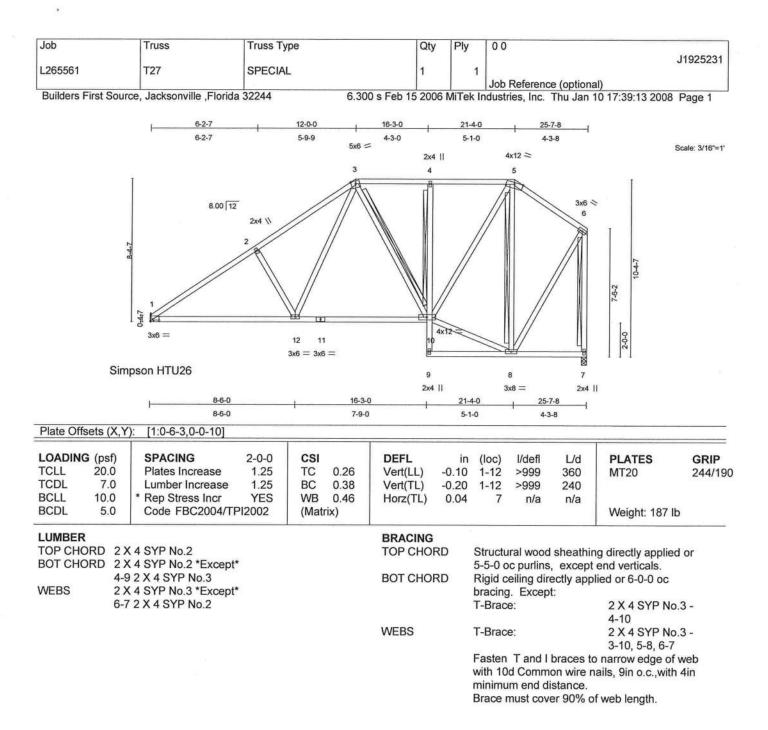
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 8 and 264 lb uplift at joint 13.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34869 1100 Ceastal Bay Blyd Boynton Beach Ft 13446





REACTIONS (lb/size) 1=811/Mechanical, 7=811/0-3-8

Max Horz 1=217(load case 5)

Max Uplift 1=-156(load case 6), 7=-151(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1218/536, 2-3=-1055/585, 3-4=-599/415, 4-5=-598/417, 5-6=-413/244,

6-7=-789/422

BOT CHORD 1-12=-577/938, 11-12=-343/647, 10-11=-343/647, 9-10=0/64, 4-10=-259/155,

8-9=-21/4, 7-8=-6/8

WEBS 2-12=-282/286, 3-12=-229/380, 3-10=-115/108, 5-10=-305/599, 5-8=-569/318,

8-10=-132/311, 6-8=-265/573

Truse Cesian Engineer Florida FE No. 24925 1 199 Cestal Bay Blvd Boynton Beach, FL 33435

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T27	SPECIAL	1	1		J1925231
		0. 202			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:14 2008 Page 2

JOINT STRESS INDEX

1 = 0.61, 2 = 0.33, 3 = 0.44, 4 = 0.33, 5 = 0.71, 6 = 0.50, 7 = 0.33, 8 = 0.61, 9 = 0.35, 10 = 0.54, 11 = 0.25 and 12 = 0.46

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 1 and 151 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34865 1169 Guestel Bay Blyd Boyston Besch, Et 33435



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T28	SPECIAL	4	1		J1925232
					Job Reference (optional)	

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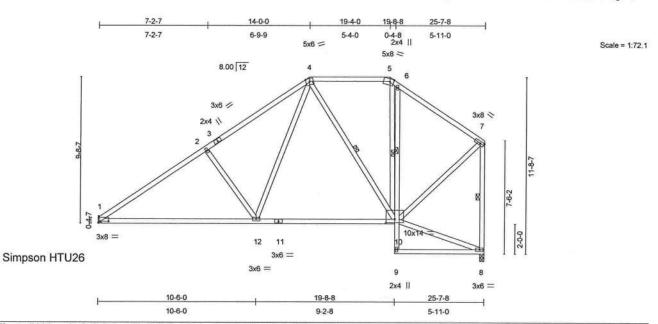


Plate Offsets (X,Y): [1:0-8-3,0-1-2], [5:0-3-0,Edge], [10:0-7-0,0-2-12]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.22	1-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.44	1-12	>699	240	National Contraction of the Cont	20000000000000
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)					30.317.363	Weight: 181 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Excep

2 X 4 SYP No.2 *Except* 6-9 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 *Except* 7-8 2 X 4 SYP No.2

BRACING

BOT CHORD

TOP CHORD Structural wood sheathing directly applied or

5-3-11 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc

bracing. Except:

1 Row at midpt

6-10

WEBS

1 Row at midpt

4-10, 7-8, 5-10

REACTIONS (lb/size) 1=811/Mechanical, 8=811/0-3-8

Max Horz 1=254(load case 5)

Max Uplift 1=-162(load case 6), 8=-151(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1182/515, 2-3=-962/512, 3-4=-938/541, 4-5=-402/343, 5-6=-497/440,

6-7=-574/322, 7-8=-788/421

BOT CHORD 1-12=-545/901, 11-12=-266/559, 10-11=-266/559, 9-10=0/84, 6-10=-171/217,

8-9=-29/0

WEBS 2-12=-336/337, 4-12=-243/445, 4-10=-336/193, 7-10=-230/541, 8-10=-13/43,

5-10=-176/128

Julius Lee Trues Cesian Engineer Flonda PE No. 34869 1100 Ceastal Bay Blvd Goynton Beach, FL 33435

JOINT STRESS INDEX

1 = 0.65, 2 = 0.33, 3 = 0.44, 4 = 0.59, 5 = 0.56, 6 = 0.45, 7 = 0.71, 8 = 0.35, 9 = 0.63, 10 = 0.29, 11 = 0.35 and 12 = 0.47

NOTES

Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T28	SPECIAL	4	1		J1925232
	1.20	0. 20%			Job Reference (optional)	

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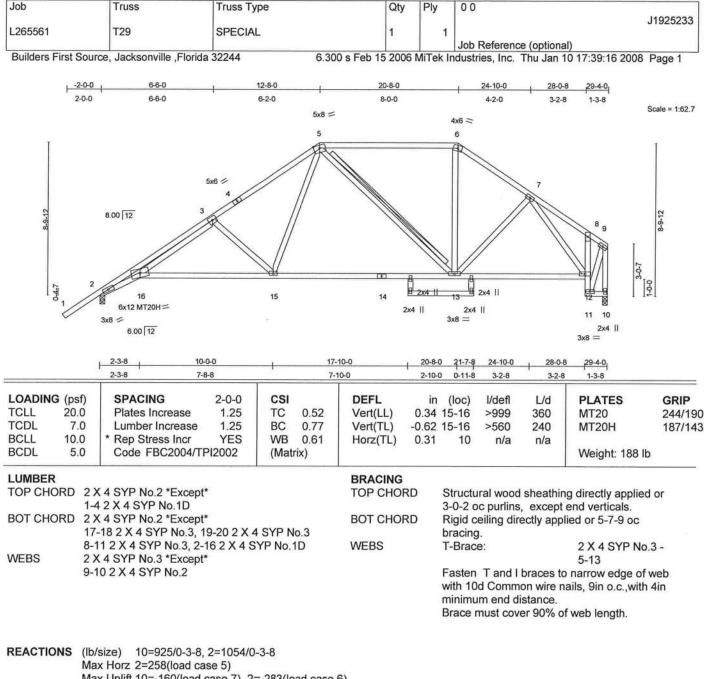
NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 1 and 151 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truse Cesign Engineer Florida PE No. 24869 1109 Ceastel Bay Blyd Boydoo Beer F. 19445





Max Uplift 10=-160(load case 7), 2=-283(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-3518/1483, 3-4=-1359/687, 4-5=-1190/713, 5-6=-782/527,

6-7=-973/553, 7-8=-513/308, 8-9=-401/192, 9-10=-1005/473

15-16=-673/1355, 14-15=-359/908, 13-14=-359/908, 12-13=-318/720,

11-12=-598/302, 8-12=-103/117, 10-11=-12/25, 2-16=-1335/3076

3-15=-403/335, 5-13=-251/166, 6-13=-59/269, 7-13=-111/179, 9-11=-351/731,

3-16=-726/1900, 5-15=-202/492, 7-12=-585/251

JOINT STRESS INDEX

BOT CHORD

WEBS

2 = 0.77, 3 = 0.74, 4 = 0.25, 5 = 0.58, 6 = 0.75, 7 = 0.41, 8 = 0.41, 9 = 0.56, 10 = 0.43, 11 = 0.69, 12 = 0.52, 13 = 0.56, 14 = 0.560.50, 15 = 0.48, 16 = 0.91, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T29	SPECIAL	1	1		J1925233
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:16 2008 Page 2

NOTES

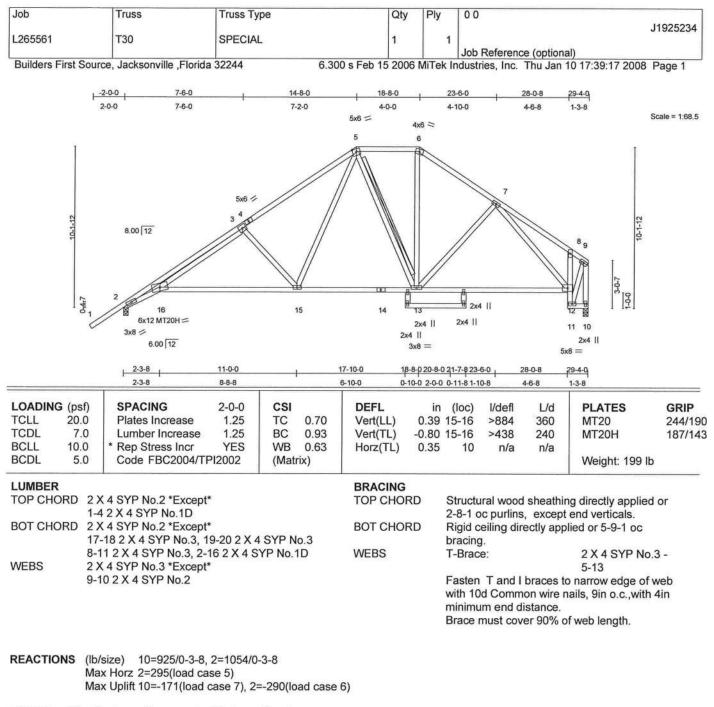
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 3x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 160 lb uplift at joint 10 and 283 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34869 1189 Coastel Bay Blvd





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-3521/1409, 3-4=-1291/671, 4-5=-1276/706, 5-6=-716/518,

6-7=-933/553, 7-8=-562/329, 8-9=-406/178, 9-10=-1022/459

BOT CHORD 15-16=-626/1284, 14-15=-257/778, 13-14=-257/778, 12-13=-322/762,

11-12=-595/324, 8-12=-143/174, 10-11=-11/25, 2-16=-1258/3081 3-15=-458/389, 5-13=-251/183, 6-13=-175/282, 7-13=-108/171, 9-11=-367/745,

3-16=-685/1969, 5-15=-285/519, 7-12=-544/234

Julius Late Truss Cesign Engineer Florida FE No. 34869 1100 Caastal Bay Blvd Goynton Beach, Ft. 33485

JOINT STRESS INDEX

WEBS

2 = 0.77, 3 = 0.76, 4 = 0.61, 5 = 0.63, 6 = 0.31, 7 = 0.41, 8 = 0.63, 9 = 0.57, 10 = 0.42, 11 = 0.70, 12 = 0.69, 13 = 0.62, 14 = 0.28, 15 = 0.46, 16 = 0.91, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T30	SPECIAL	1	1		J1925234
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:17 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) All plates are 3x6 MT20 unless otherwise indicated.

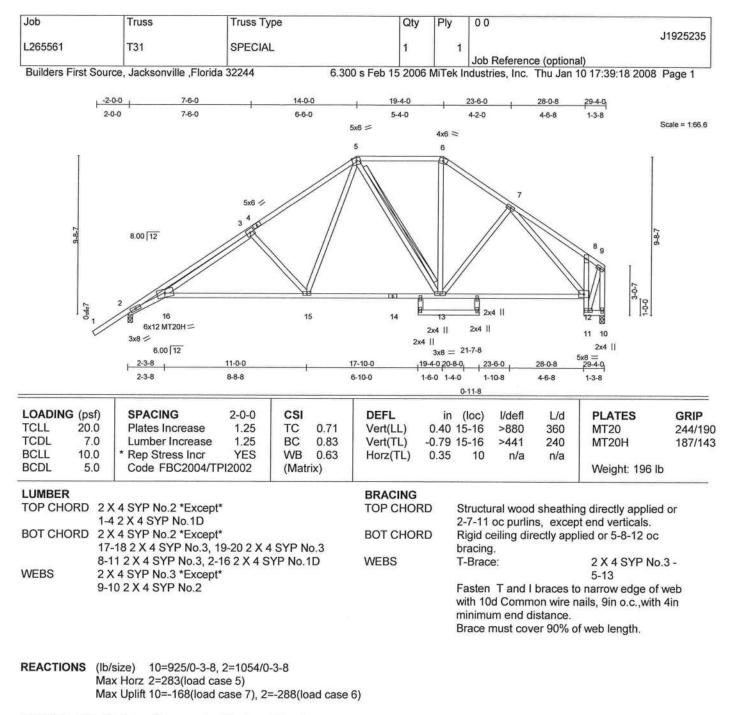
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 10 and 290 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Trues Cesign Engineer Florida PE No. 24805 1109 Cesstel Bay Blvd Boynon Baser E. 13446





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-3520/1425, 3-4=-1289/672, 4-5=-1274/703, 5-6=-735/519.

6-7=-940/561, 7-8=-566/343, 8-9=-401/180, 9-10=-1018/464

BOT CHORD 15-16=-622/1277, 14-15=-286/816, 13-14=-286/816, 12-13=-320/758, 11-12=-607/329, 8-12=-161/190, 10-11=-11/25, 2-16=-1272/3080

3-15=-438/370, 5-13=-239/166, 6-13=-149/280, 7-13=-85/148, 9-11=-375/754,

3-16=-704/1975, 5-15=-261/496, 7-12=-540/217

JOINT STRESS INDEX

WEBS

2 = 0.77, 3 = 0.76, 4 = 0.54, 5 = 0.56, 6 = 0.37, 7 = 0.41, 8 = 0.58, 9 = 0.58, 10 = 0.42, 11 = 0.71, 12 = 0.57, 13 = 0.57, 14 = 0.570.27, 15 = 0.48, 16 = 0.91, 17 = 0.33, 18 = 0.33, 19 = 0.33 and 20 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T31	SPECIAL	1	1		J1925235
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:18 2008 Page 2

NOTES

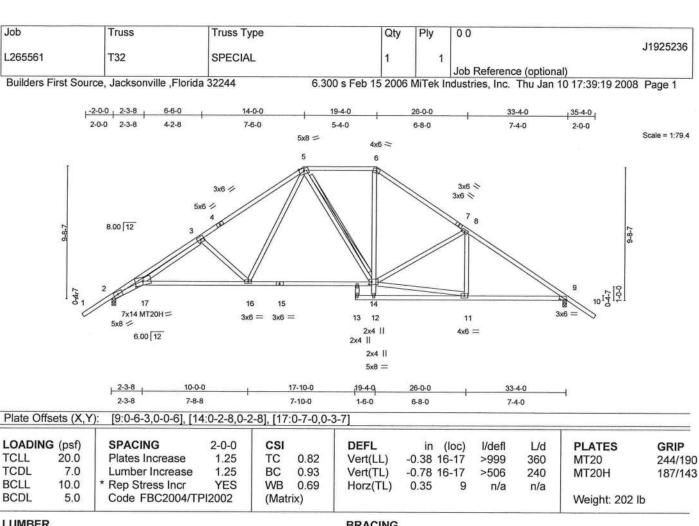
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 3x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 168 lb uplift at joint 10 and 288 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Trues Cesign Engineer Florida PE No. 34868 1189 Ceastal Bay Blvd.





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 *Except*

13-18 2 X 4 SYP No.3, 2-17 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

2-2-0 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc

bracing.

WEBS

T-Brace:

2 X 4 SYP No.3 -

5-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 9=1184/0-3-8, 2=1183/0-3-8

Max Horz 2=-260(load case 4)

Max Uplift 9=-304(load case 7), 2=-305(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/60, 2-3=-4051/1270, 3-4=-1636/752, 4-5=-1521/784, 5-6=-1028/631,

6-7=-1308/659, 7-8=-1325/627, 8-9=-1617/693, 9-10=0/62

16-17=-516/1616, 15-16=-180/1030, 14-15=-180/1030, 12-13=0/0, 11-12=-3/33, 9-11=-361/1246, 2-17=-914/3549

3-17=-465/2140, 3-16=-479/366, 5-16=-229/551, 5-14=-157/181, 12-14=0/127,

6-14=-162/407, 8-14=-289/270, 8-11=-85/121, 11-14=-361/1231

BOT CHORD

WEBS

2 = 0.83, 3 = 0.83, 4 = 0.27, 5 = 0.65, 6 = 0.56, 7 = 0.48, 8 = 0.41, 9 = 0.74, 11 = 0.51, 12 = 0.33, 13 = 0.33, 14 = 0.60, 15 = January 10,2008 Continued by -5265217 = 0.78 and 18 = 0.33

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	5000550005
L265561	T32	SPECIAL	1	1		J1925236
	ALTERNA .			1	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:19 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

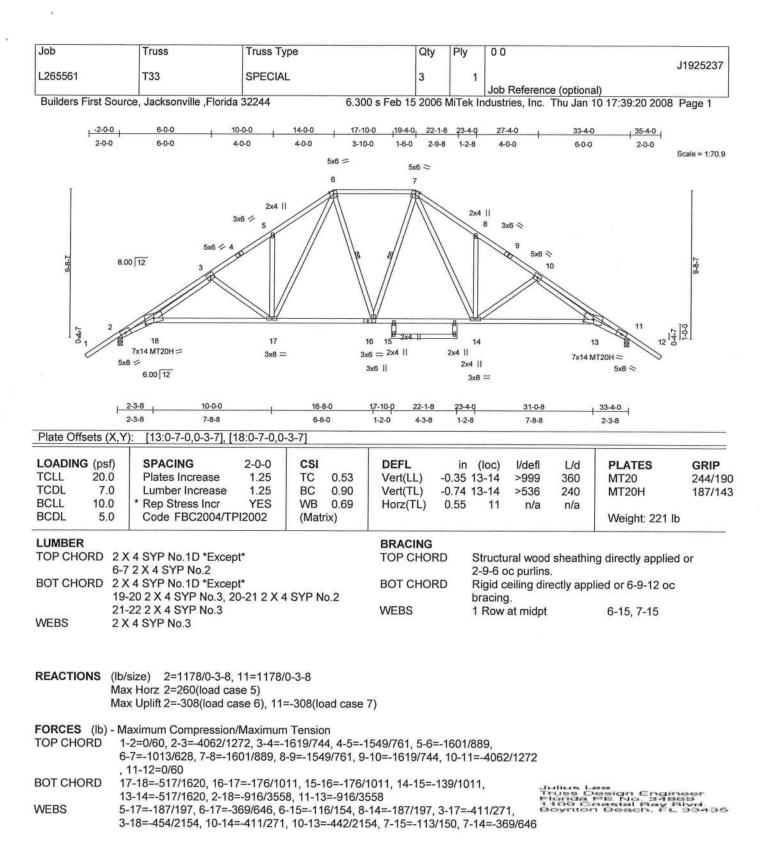
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 304 lb uplift at joint 9 and 305 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1109 Coasiel Bay Blvd Boynton Beach, Ft. 23435





JOINT STRESS INDEX

2 = 0.84, 3 = 0.84, 4 = 0.29, 5 = 0.33, 6 = 0.38, 7 = 0.38, 8 = 0.33, 9 = 0.29, 10 = 0.84, 11 = 0.84, 13 = 0.76, 14 = 0.72, 15 = 0.34, 16 = 0.36, 17 = 0.72, 18 = 0.76, 19 = 0.33, 20 = 0.33, 21 = 0.33 and 22 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T33	SPECIAL	3	1		J1925237
LLCGGGGT	1.00	OI LOW IL	Ů		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:20 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 7) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 308 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Lee Trues Cesign Engineer Flonds PE No. 24869 1100 Cessiel Bay Blvd Boyston Baser E. 1944



Job Truss Truss Type Qty Ply 00 J1925238 L265561 T34 SPECIAL Job Reference (optional) Builders First Source, Jacksonville , Florida 32244 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 18:07:07 2008 Page 1 6-0-0 14-0-0 35-4-0 17-10-0 19-4-0, 22-1-8 23-4-0 27-4-0 33-4-0 6-0-0 4-0-0 4-0-0 3-10-0 1-6-0 2-9-8 1-2-8 4-0-0 6-0-0 2-0-0 Scale = 1:67.7 5x6 = 5x6 = 5 8.00 12 6 2x4 II 3x6 / 3x6 > 16 15 13 12 7x14 MT20H= 3x6 = 2x4 | I2x4 || 3x8 = 7x14 MT20H = 2x4 || 3x6 II 5x8 > 6.00 12 3x8 =Simpson HTU26 10-0-0 16-8-0 1,7-10-0 23-4-0 31-0-8 33-4-0 7-8-8 6-8-0 1-2-0 4-3-8 1-2-8 7-8-8 2-3-8 Plate Offsets (X,Y): [12:0-7-0,0-3-7], [17:0-7-0,0-3-7] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl L/d **PLATES GRIP** (loc) in 20 0 TCLL Plates Increase 1.25 TC 0.83 Vert(LL) -0.41 16-17 >977 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.94 Vert(TL) -0.84 16-17 >470 240 MT20H 187/143 BCLL 10.0 Rep Stress Incr YES WB 0.77 Horz(TL) 0.61 10 n/a n/a BCDL Code FBC2004/TPI2002 50 (Matrix) Weight: 217 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 2-1-4 BOT CHORD 2 X 4 SYP No.2 *Except* oc purlins. 18-19 2 X 4 SYP No.3, 20-21 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 2-2-0 oc bracing.

1-17 2 X 4 SYP No.1D, 10-12 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.3 WEBS 1 Row at midpt

5-14, 6-14

REACTIONS

(lb/size) 1=1053/Mechanical, 10=1182/0-3-8

Max Horz 1=-287(load case 4)

Max Uplift 1=-210(load case 6), 10=-309(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4328/1622, 2-3=-1641/776, 3-4=-1568/793, 4-5=-1620/918, 5-6=-1021/639,

6-7=-1609/902, 7-8=-1557/774, 8-9=-1628/757, 9-10=-4113/1303, 10-11=0/60

BOT CHORD 16-17=-575/1661, 15-16=-179/1020, 14-15=-179/1020, 13-14=-148/1017, 12-13=-528/1629,

1-17=-1265/3824, 10-12=-944/3606

4-16=-184/192, 5-16=-394/663, 5-14=-118/156, 7-13=-188/198, 2-16=-440/313, 2-17=-746/2389, 9-13=-413/272, 9-12=-460/2196, 6-14=-114/149, 6-13=-371/648

JOINT STRESS INDEX

1 = 0.90, 2 = 0.63, 3 = 0.30, 4 = 0.34, 5 = 0.40, 6 = 0.39, 7 = 0.34, 8 = 0.31, 9 = 0.65, 10 = 0.87, 12 = 0.77, 13 = 0.74, 14 = 0.35, 15 = 0.74, 14 = 0.74, 14 = 0.74, 14 = 0.74, 15 = 0.0.37, 16 = 0.76, 17 = 0.81, 18 = 0.34, 19 = 0.34, 20 = 0.34 and 21 = 0.34

NOTES

WEBS

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is Continued for Ge Cofor members and forces, and for MWFRS for reactions specified

January 10,2008

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265561	T34	SPECIAL	1	1		J1925238
		0.20%	1.		Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 18:07:07 2008 Page 2

NOTES

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

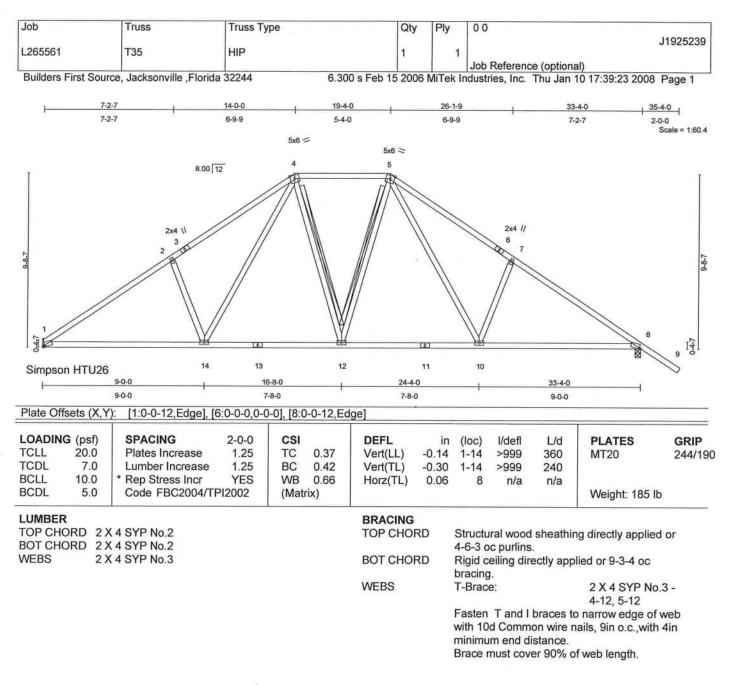
5) All plates are MT20 plates unless otherwise indicated.

- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34869 1100 Coestal Bay Blyd Governo Desch Per 1944





REACTIONS (lb/size) 1=1054/Mechanical, 8=1182/0-3-8

Max Horz 1=-286(load case 4)

Max Uplift 1=-210(load case 6), 8=-309(load case 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-1621/749, 2-3=-1482/830, 3-4=-1460/859, 4-5=-924/631, 5-6=-1352/827, TOP CHORD

6-7=-1460/798, 7-8=-1602/722, 8-9=0/62

BOT CHORD 1-14=-418/1261, 13-14=-162/916, 12-13=-162/916, 11-12=-138/913,

10-11=-138/913, 8-10=-387/1238

2-14=-336/345, 4-14=-326/477, 4-12=-120/173, 5-12=-123/178, 5-10=-282/447,

7-10=-320/323

JOINT STRESS INDEX

WEBS

1 = 0.80, 2 = 0.33, 3 = 0.50, 4 = 0.59, 5 = 0.59, 6 = 0.50, 7 = 0.33, 8 = 0.80, 10 = 0.47, 11 = 0.32, 12 = 0.50, 13 = 0.32 and 14 January 10,2008 Continued on page 2

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0 0	
L265561	T35	HIP	1	1		J1925239
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:39:23 2008 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 3x6 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1 and 309 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Flonda PE No. 34868 1100 Ceastal Bay Blvd Bovnton Beach, Ft. 33436

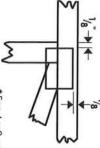


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless securely seat. plates to both sides of truss and Dimensions are in inches. Apply dimensions indicate otherwise



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 × 4

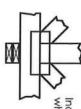
dimension is the length parallel perpendicular to slots. Second The first dimension is the width

LATERAL BRACING



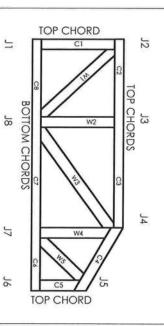
Indicates location of required continuous lateral bracing.

BEARING



which bearings (supports) occur. Indicates location of joints at

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

ICBO 3907, 4922 96-31, 96-67

BOCA

SBCCI

9667, 9432A

NER

561

WISC/DILHR

960022-W, 970036-N





MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 2 Cut members to bear tightly against each other.
- 4. ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)

S

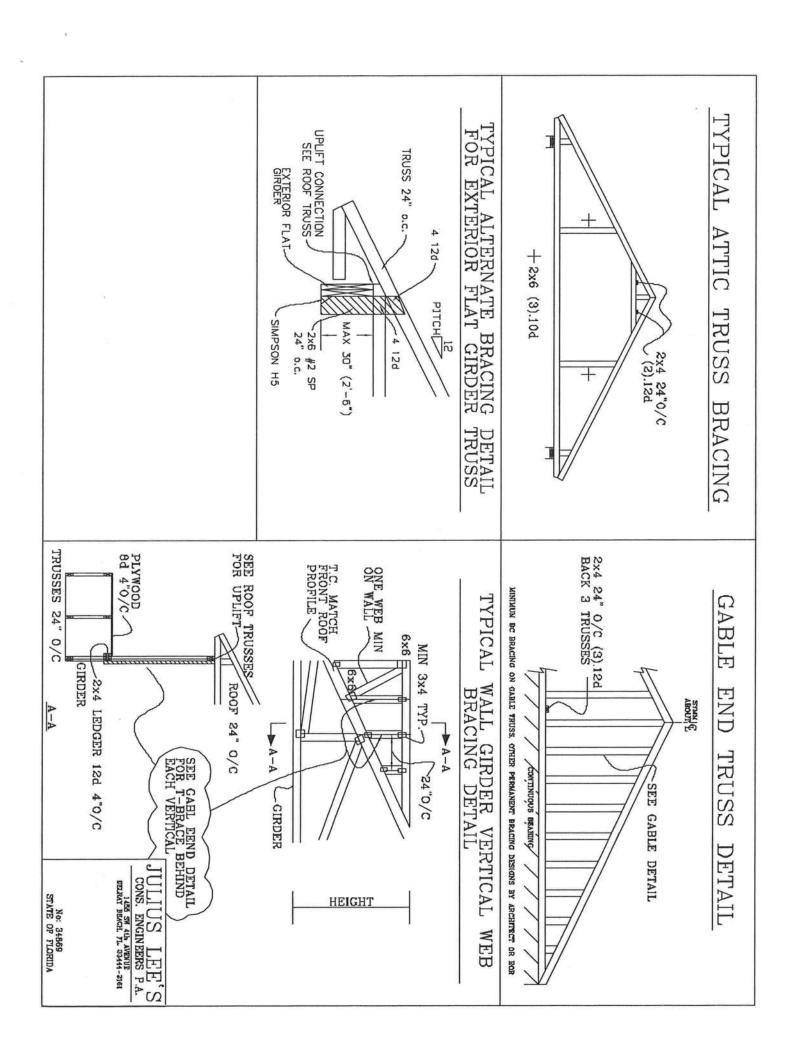
Unless otherwise noted, moisture content of

- 0 Unless expressly noted, this design is not applicable for use with fire retardant or lumber shall not exceed 19% at time of fabrication
- 7 Camber is a non-structural consideration and preservative treated lumber.
- 00 Plate type, size and location dimensions shown indicate minimum plating requirements practice is to camber for dead load deflection. is the responsibility of truss fabricator. General
- 9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Bottom chords require lateral bracing at 10 unless otherwise noted. ft. spacing, or less, if no ceiling is installed
- Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- 13. Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling, erection and installation of trusses.

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DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USSED. CONNECT INACONAL BRACE FOR SAGS AT EACH END. MAY WEB TOTAL LENGTH IS 14°. MAX GABLE VERTICAL LENGTH SPACING SPECIES VERTICAL LENGTH IN TABLE ABOVE. 16" 24" O.C. O.C. O.C. GABLE VERTICAL CONNECT DIAGONAL AT SPF SPF DFL SPF DFL DFL SP SP H ASCE NAOHS STUD STANDARD #1 #2 #3 STUD STANDARD \$1 / #2 STANDARD #1 / #2 STANDARD STANDARD GRADE STANDARD \$1 / #2 STUD STUD STUD BEEM 3 8 #3 BRACE 7-02: GABLE TRUSS NO 130 GROUP SPF #1/#2. DY-L #2. SPF #1/#2. OR BETTER DIAGONAL BRACE: SINGLE OR DOUBLE CUT (AS SHOWN) AT UPPER RND. E 7 5 5 5 6 4 3 1 2 8 MPH 124 > GROUP B BRACE . WIND GROUP A E SPEED, 2X4 æ GROUP B BRACE . REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH 15 ABOUT 18 GROUP A (2) 8 8 8 8 B MEAN 2X4 "L" EX4 MEN OR BETTER CONTINUOUS GROUP B BRACE ** 10, 5, 11.00 6 8 10' 2" HEIGHT, 0 BARING GROUP A ULIUS LEI cons. Engineers DELRAY BEACH, PL 33444-2161 25 25 288 No: 34869 STATE OF FLORIDA ENCLOSED GROUP B BRACE * 12. 9" 12' 4" GROUP A P.A.S (2) 12, 2XB Н ď. MAX. MAX. GROUP B BRACE 11 13' 7" 13 3 **⊕** TOT. 1.00, SPACING Ę. ATTACH EACH 'L' BRACE WITH 104 NAILS. # FOR (1) 'L' BRACE: SPACE WAILS AT 2" O.C. # FOR (2) 'L' BRACES AND 4" O.C. BETWEEN ZONES. ## FOR (2) 'L' BRACES: SPACE WAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4' 0" DUTLOBKERS WITH 2' O' DVERHANG, DR 12" PROVIDE UPLIFT CONNECTIONS FOR 136 FLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. DOUGLAS FIR-LARCH #3 STUD STUD STANDARD MEMBER LENGTH. L' BRACING MUST BE A MINIMUM OF BOX OF WEB SPRUCE-PINE-WR #1 / #2 STANDARD #3 STUD BRACING GROUP SPECIES PLYWOOD OVERHANG. EXPOSURE BOUTHERN PINE CABLE TRUSS DETAIL NOTES: 60 GREATER THAN 4' D', BUT LESS THAN 11' B' GREATER THAN 11' 6" PEAK, SPLICE, AND HEEL PLATES. VERTICAL LENGTH 24.0 GABLE VERTICAL PLATE SIZES PSF DATE REF DRWG MITEK STD CABLE 15 E HT GROUP GROUP HEM-PIR HI & BIR 0 DOUGLAS FIR-LARCH SOUTHERN PINE #3 STUD 11/26/03 B A: ASCE7-02-CAB13015 NO SPLICE 2.5X4 STANDARD AND GRADES: HEM-PIR 224 CLANDALS

DIAGONAL BRACE OFTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIACONAL BRACE FOR BEG! AT EACH END. MAX WEB **GABLE** TOTAL LENGTH IS 14". MAX VERTICAL LENGTH SPACING SPECIES GRADE VERTICAL LENGTH SHOWN IN TABLE ABOVE. 16" 24" O.C. O.C. O.C. CONNECT DIAGONAL AT GABLE VERTICAL SPF SPF SPF DFL DFL DFL SP SP ASCE STUD STANDARD STANDARD STANDARD STANDARD STANDARD STANDARD #2 #2 BEW BRACE 7-02: MAYARMASIK TRASSIS REQURE EXTREME CARE IN ARBIGATING, HANDLING, SUPPING, INSTALLING AND BRACING. REFER TO BESS 1-03 SUILDING COMPONENT SAFETY RESET TO BESS 1-03 SUILDING COMPONENT SAFETY RASETY HANDLING, PUBLISHED BY FPI CRAUSS PLAIE INSTITUTE, 383 POWERED EX SUITE 200, MAISSON, PUBLISHED AND PUBLICATION TO PERFORMING OF AMERICA, 6300 ENTERPRISE LN, MOISON, PI 33719) TOR SAFETY PRACTICES PRIBE TO PERFORMING THESE TWICTIONS, UNLESS OTHER PAIRS INDICATED THE CHORD SHALL HAVE PORPERLY ATTACHED STRUCTURAL PANDLS AND BUTTOM GOOD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. GABLE TRUSS BRACES 130 GROUP A (1) 1X4 "L" BRACE * ZX4 SP OR DIT-L #2 OR BETTER DIAGONAL BRACE, SINGLE OR DOUBLE AT UPPER BND MPH GROUP B WIND (1) 2X4 "L" BRACE . GROUP A SPEED, GROUP B REFER TO 30 ⊢¤ٍ-⊣ **⊢** B. ABOUT E (2) 2X4 "L" BRACE ** GROUP A MEAN CHART ABOVE FOR MAX GABLE VERTICAL LENGTH EX4 #EN OR BETTER CONLINDOR BEVEING GROUP B 8. 0° 7' 10° 8. 9° HEIGHT, 0 (1) 2X6 "L" BRACE • (2) 2X8 "L" ULIUS LEI GROUP A GROUP B GROUP A DELRAY BEACH, PL. 33444-2161 No: 34869 STATE OF FLORIDA 0 ENCLOSED PET S MAX. MAX. GROUP BRACE II TOT. 1.00, SPACING Ħ E. ATTACH EACH 'L' BRACE WITH 104 NAILS. * FOR (1) 'L' BRACE: SPACE WAILS AT 2" O.C. * FOR (2) 'L' BRACES: AND 4" O.C. BETWEEN ZONES. * FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. CABLE END SUPPORTS LOAD FROM 4' 0" DUTLONERS WITH 2' 0" DVERHANG, DR 12" PLYWOOD OVERHANG. PROVIDE UPLATT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD). LIVE LOAD DEPLECTION CRITERIA IS L/240. MEMBER LENGTH. T. BRACING MUST BE A MINIMUM OF BOX OF WEB DOUGLAS FIR-LARCH BRACING SPRUCE-PINE-FIR #1 / #2 STANDARD #3 STUD VERTICAL LENGTH 1288 THAN 4. 0", BUT 1288 THAN 11" 6" CREATER THAN 11" 6" SDUTHERN PINE EXPOSURE CABLE TRUSS DETAIL NOTES: 60 STANDARD 24.0 PEFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. CABLE VERTICAL PLATE SIZES PSF GROUP SPECIES STUD DATE REF -ENG DWG MITEK STD GABLE SO' E HT HEM-PIR HI & BIR GROUP B: GROUP DOUGLAS FIR-LARCH Ω 11/26/03 ASCB7-02-CAB13030 A SOUTHERN PINE #3 STANDARD STANDARD NO SPLICE AND 2.5X4 274 GRADES:



BOT CHORD 2X4 2X4 ながね 222 BETTER BETTER BETTER

PIGGYBACK DETAIL

TYPE

SPANS

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30,

34

38 5

52

REFER TO SEALED DESIGN FOR DASHED PLATES

SPACE PIGGYBACK VERTICALS AT 4' OC MAX. TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

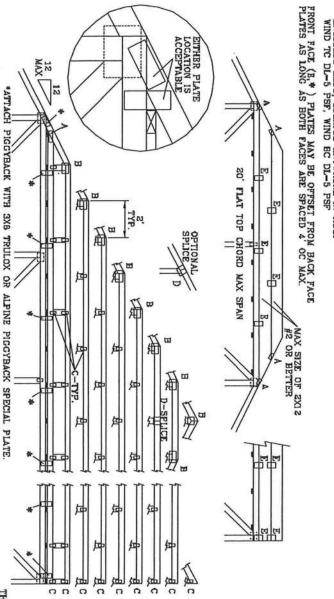
PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
110 MPH WIND, 30 MEAN HGT, ASCE 7-02, CLOSED BLDG,
LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C. WIND TC DL-5 PSF, WIND BC DL-5 PSF
110 MPH WIND, 30 MEAN HGT, FBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL-5 PSF, WIND BC DL-5 PSF

130 MPH WIND, 30' MEAN HCT, ASCE 7-02, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TC DL=6 PSF, WIND BC DL=6 PSF



44	4X8 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY	OTATED T	4X8 OI	Ħ
5X6	5X5	5X6	5 X 4	Ħ
1.5X4	1.5X4	1.5X4	1.5X3	n
536	5X8	5X6	4X8	ᇤ
3X6	2.6X4	2.5X4	2X4	>

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.575" NAILS, EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION 7 S

	WEB BRACING CHART
WEB LENGTH	REQUIRED BRACING
o' To 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS MEMBER OR BETTER AND 80% LENGTH OF
000	MEMBER. ATTACH WITH 8d NAILS AT 4" OC
10' TO 14'	OK LENGTH
	R. ATT

THIS DRAWING REPLACES DRAWINGS 634,016 634.017 & 647.045

STATE OF FLORIDA	5		DELRAY BRACH, TL. 33444-2161	CONS. ENGINEERS P.A.	ひ, 山 山 山 山 山 山 山 山 山
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	50 PSF AT 1.25 DUR. FAC.	1.33	55 PSF	MAX LOADING
		-ENG JL	DRWGMITEK STD PIGGY	DATE 09/12/07	REF PIGGYBACK

MACARHIMSM TRACSES REQUIRE EXTREME EAGE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BACING. REFER TO ESS! 1-IS GUILING COMPONENT SAFETY INFORMATION, PULLALISHED BY THE CRIESS PLATE INSTITUTE, 583 GYOUFRED BS, SUITE 280, MADISON, VI. 33759 AND AFEA ACCOUNT TRACE COUNTING TO FABRICA, 6300 CATERPRISE IM, MADISON, VI. 33759 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OF DIRECTION FOR CAPTER SALL HAVE FOR FRALY ATTACHED TO STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT CELLING.

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VALLEYTRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY INSTALLATION TRUSS

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

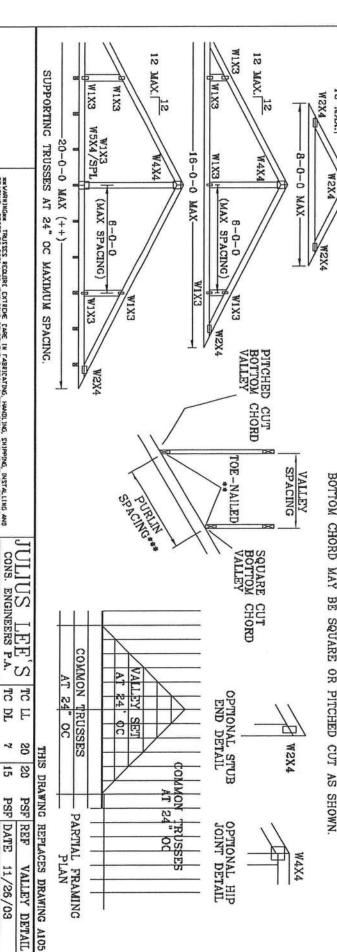
4-0-0

MAX

12 NAX.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



REVERBINGER TRUSSES ECOURE CYTEDE EARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO ESSI I DO GBULDING CEPHONEN SAFETY PEDBANDON, PAUL KOCK COUNCIL PROFINCIA CADD TRUSS COUNCIL PAUL RETURNING. RES DOCTORD DR. SUITE 280, MANISON, VI. 5379% MO VITEA CADD TRUSS COUNCIL OF ARRICA, GAID COUNCIL PAUL RETURNING. PAUL RES DOCTORD DR. SUITE AND RESPONSIVE THESE FUNCTIONS. UNLESS DIFERNIS INCLINES. POERS SAFETY PRACTICES PROFIES A TRICKED THE TRUSK OF THE PAUL RES DIFERNIS FOR THE TRUSK.

DELPAY BEACH, I'L 35444-2161

BC LL BC DL TC DL

32

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PSF PSF

-ENG

PSF DRWG VALTRUSS1103

PSF DATE

11/26/03

No: 34869 STATE OF FLORIDA

SPACING DUR.FAC. 1.25 TOT. LD

24 1.25

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

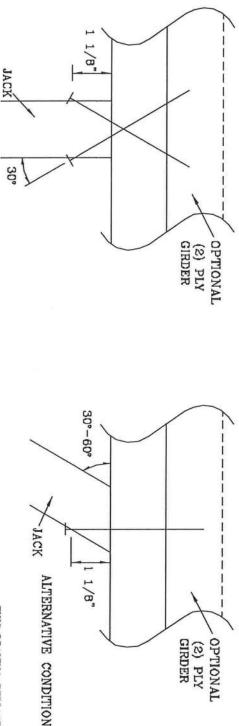
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

5 493# 639# 452# 585# 390#	4 394# 511# 361# 468# 312#	3 296# 383# 271# 351# 234#	2 197# 256# 181# 234# 156#	1 PLY 2 PLIES 1 PLY 2 PLIES 1 PLY	NUMBER OF SOUTHERN PINE DOUGLAS FIR-LARCH HEM-FIR	
5 493 # 639 # 452 # 585# 390 # 507 # 38 4	406#	304#	203#	2 PLIES	-FIR	
384#	307#	230#	154#	1 PLY	SPRUCE	
496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR	

AFFROFRIALE DOMALION LOAD TACLUK.

OPTIONAL

GIRDER (2) PLY



THIS DRAWING REPLACES DRAWING 784040

	WARRUNG TRUSSES REBURE EXTRENT CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACKING. REFER TO BEST 1-93 CHULIUNG CORPORENT SAFETY NETWANTIDNO, PLEISERID Y TP! CRRISS. PALTE INSTITUTE, 98 D'AUCHGU DA, SUITE 200, MANISON, VI. 207191 AND VICA (MODD TRUSS CONTROLLES, I.M. 44018IN, VI. 207191 FR. SAFETY PRACTICES PRIDE TO PERFORMING THESE CINCIPIONS. UNICES OTHERWISE (INDICATED, THE OFFER SHALL HAVE PROPERLY ATTACHED REGING STRUCTURAL PAMICLS AND BOTTON CHORD SHALL HAVE A PROPERLY ATTACHED REGID CEILING.					
No: 34869 STATE OF FLORIDA				DELRAY BEACH, FL 33444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL
	1.00	PSF	PSF	PSF	PSF	PSF
			-ENG JL		DATE	REF
			JL T	DRWG CNTONAIL1103	DATE 09/12/07	TOE-NAIL

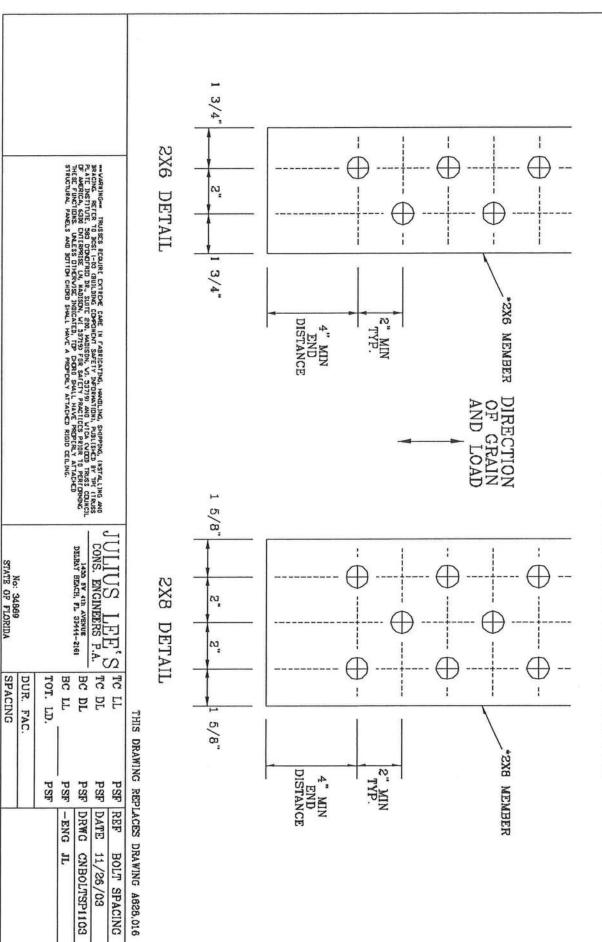
છે. DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



TRULOX CONNECTION DETAI

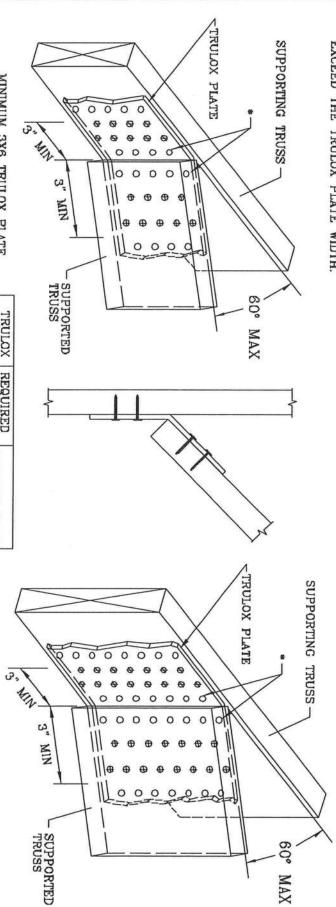
II GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\(\Phi \)).

NAILS MAY BE OMITTED FROM THESE ROWS

THIS DETAIL MAY BE USED WITH SO. PINE. DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE

REQUIRED NAILS PER TRUSS

MAXIMUM LOAD UP OR DOWN

3X6

15 9

#066 350#

JULI cons.

US LEE'S

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

1,152,017

1,159,154 & 1,151,524

REF DATE

-ENG DRWG

IL

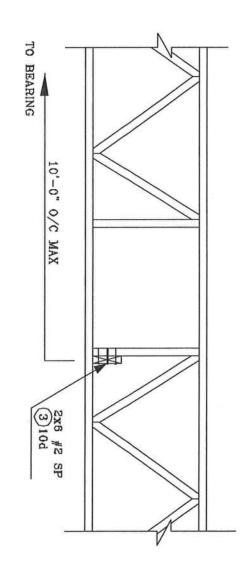
CNTRULOX1103 11/26/03 TRULOX MINIMUM 5X6 TRULOX PLATE

DELRAY BEACH, FL 33444-2383

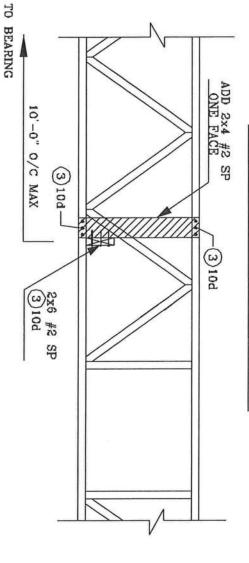
No: 34869 STATE OF FLORIDA

TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND RE 10 JECS) 1-00 (BUILDING COMPINCH SAFTET INFORMATION, PUBLISSED BY TP) (TRUSS IT. 580 D'ONFRID DR., SUITE ROJ, MADISTO, VI. 33739 AND VICA CAUDI TRUSS COLINGIL. 300 CHICRRISE LN, MADISTON, VI. 33739 FCR SAFETY PRACTICES PAIDE 10 PERFORMING INS. UNLESS OTHERVISE (NDICIATED, TOP CARD SMALL HAVE PROPERLY ATTACHED NACES AND STOCK OF THE MAY PROPERLY ATTACHED NACES AND SOTTON CHORD SMALL HAVE PROPERLY ATTACHED RIGID CEILLING.

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



STRONG BACK WITH VERTICAL NOT LINING UP ALTERNATE DETAIL FOR



JULIUS LEE'S cons. engineers p.a.

No: 34869 STATE OF FLORIDA

