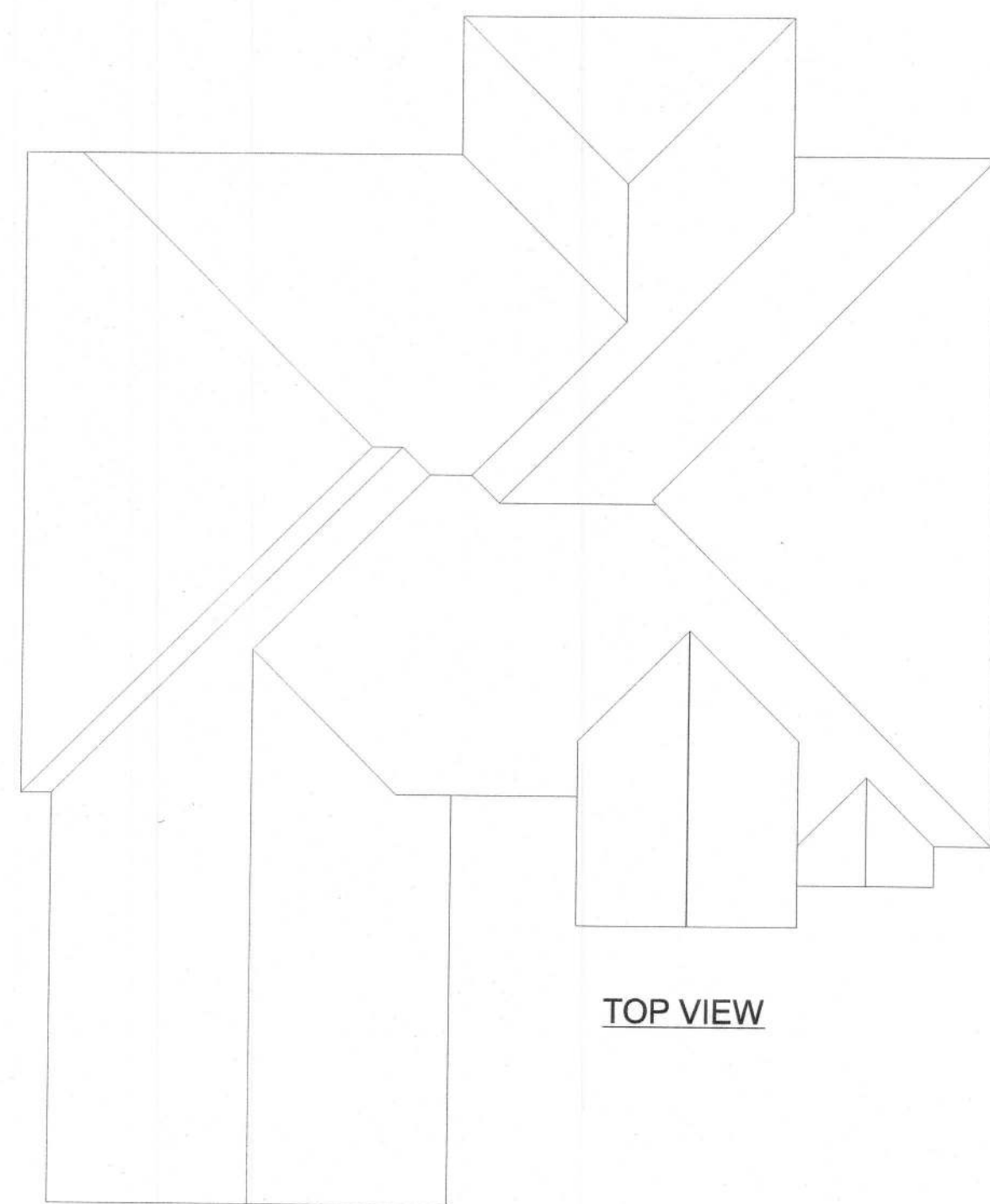
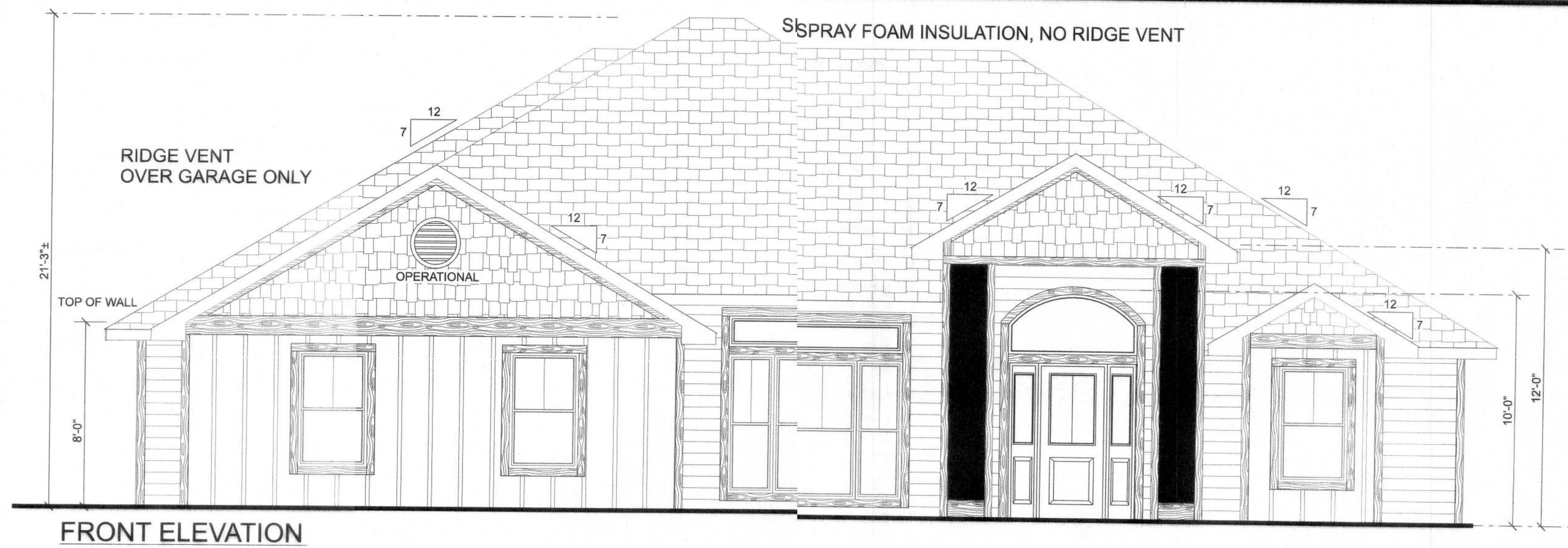
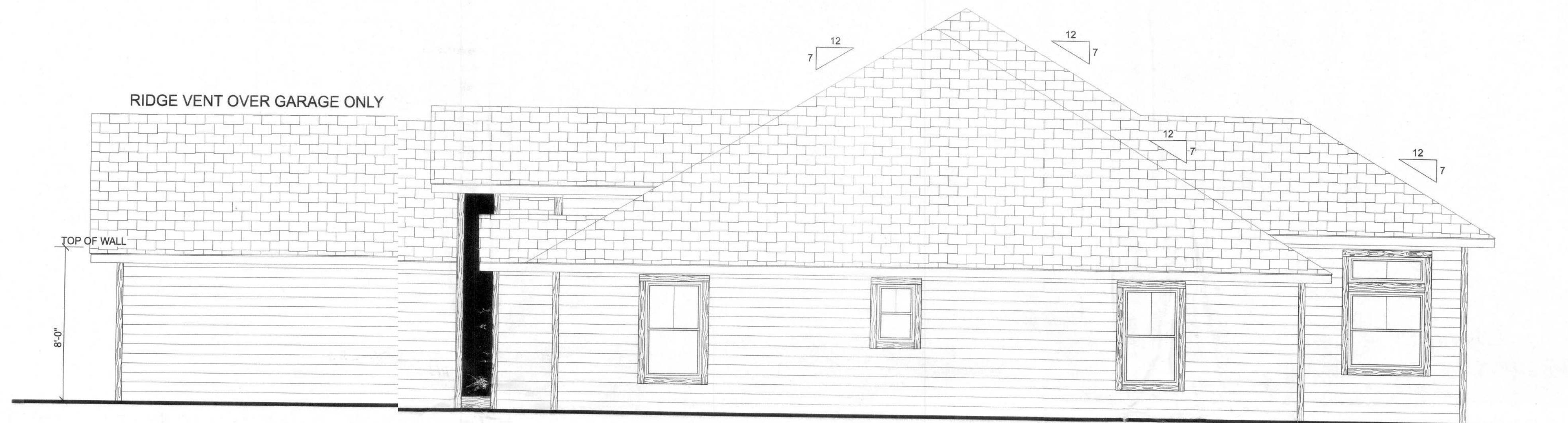




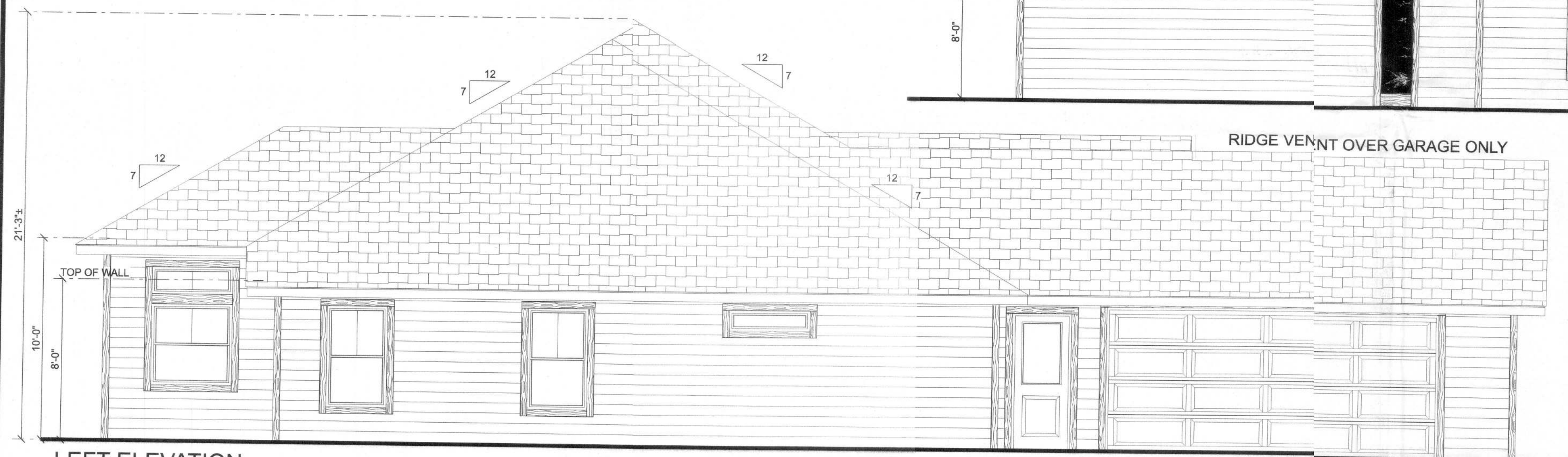
TOP VIEW



FRONT ELEVATION



RIGHT ELEVATION



LEFT ELEVATION



REAR ELEVATION

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



ERKINGER
CONSTRUCTION
CROUP

MICHAEL AND MAUREEN
FLANDERS

ADDRESS:
171 NW Ambleside Dr.
Lot #21 Ph I Cobblestone S/D,
Lake City, Florida 32055

PARCEL ID:
24-3S-1602275-121(0119)

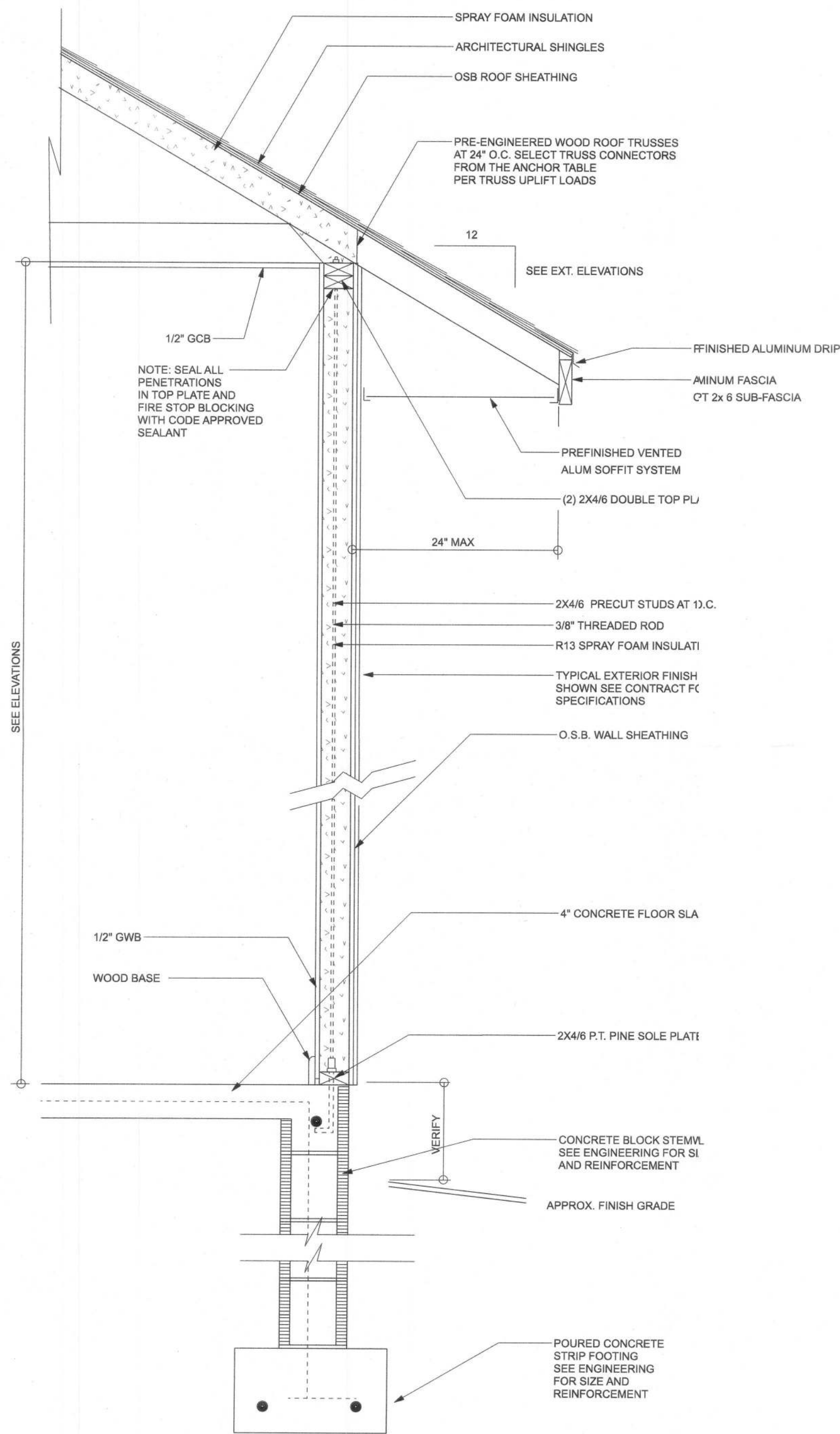
PRINTED DATE:
Wednesday, June 29, 2022

DRAWN BY: MATTHEW A. ERKINGER Sr
STRUCTURAL BY: EVAN BEARNSLEY

DRAWING NUMBER

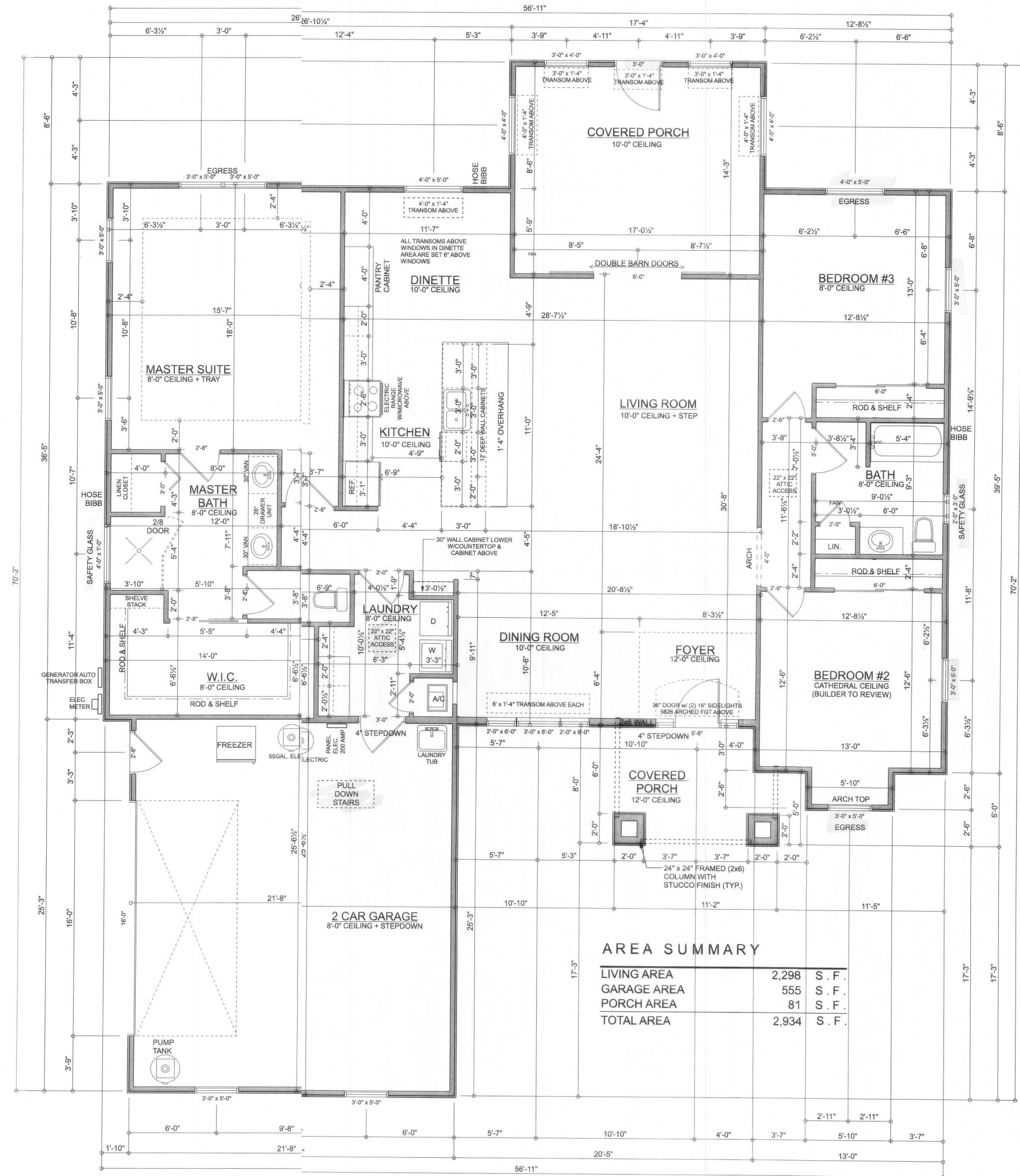
A1

OF 3 SHEETS



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA

SCALE: 1\"/>



FLOOR PLAN
SCALE: 1/4\"/>

AREA SUMMARY

LIVING AREA	2,298	S. F.
GARAGE AREA	555	S. F.
PORCH AREA	81	S. F.
TOTAL AREA	2,934	S. F.

ERKINGER
CONSTRUCTION
GROUP

MICHAEL AND MAUREEN
FLANDERS

ADDRESS:
11 NW Ambleside Dr.
Lot 61 Ph. 1 Cobblestone S/D,
Lake City, Florida 32055

PARCEL ID:
24-IS-16-02275-121(8119)

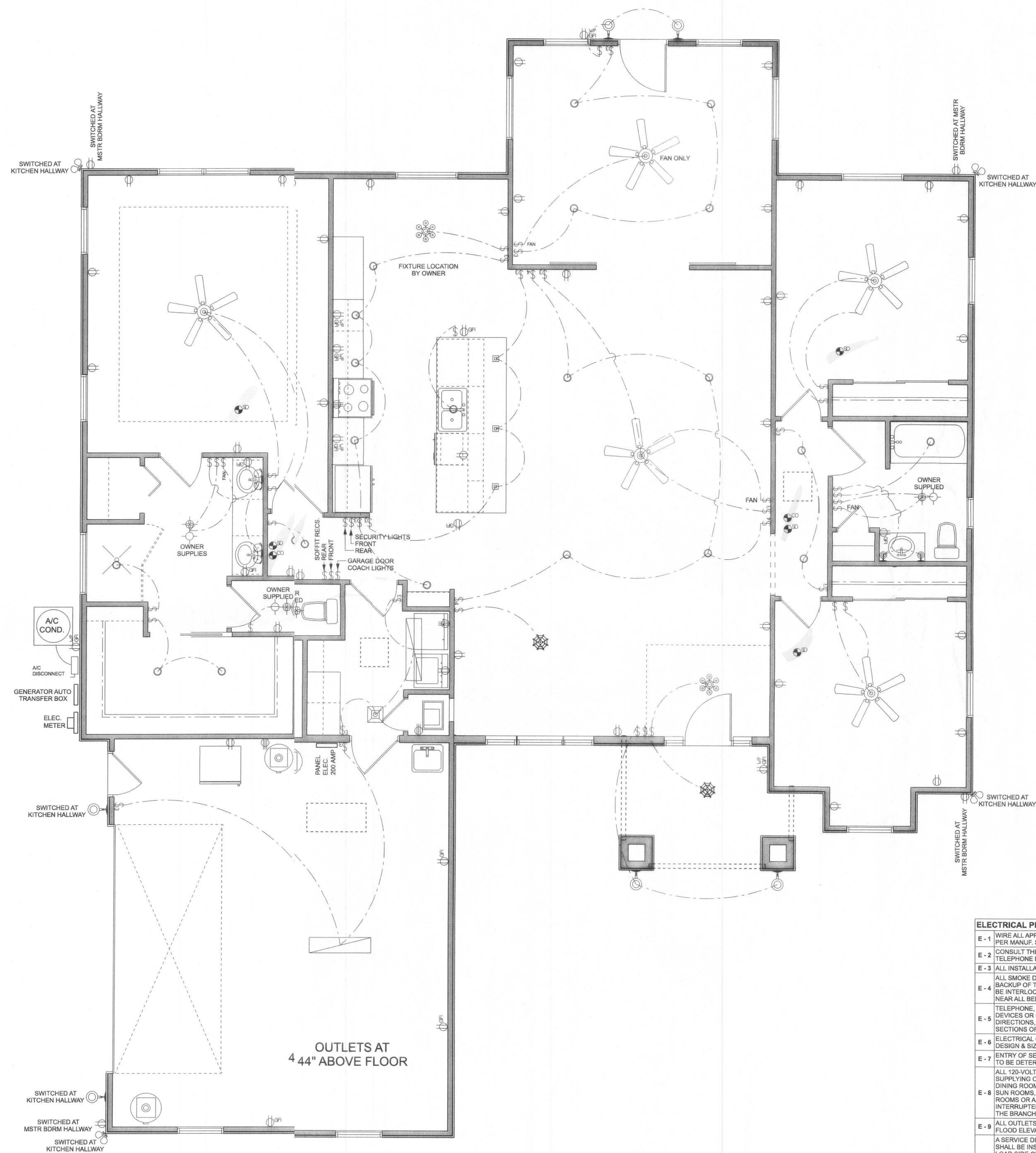
PRINTED DATE:
Wednesday, June 29, 2022

DRAWN BY: STRUCTURAL BY:
Matthew A. Erkinger Sr. Evan Beamsley

DRAWING NUMBER

A2

OF 3 SHEETS



ELECTRICAL LEGEND	
ELECTRICAL	SYMBOL
fan - Bath fan light	
light - can light	
light - exterior spotlights	
light - fluorescent 1 x 4	
outlet	
outlet - 220v	
outlet - WP	
outlet - gfi	
switch	
switch - 3 way	
switch - 4 way	
ceiling fan 5 bladed 02	
ceiling shade square	
chandelier 01	
chandelier 02	
pendant cube	
wall mounted 02 2 lights	
wall mounted 03 1 light	
wall mounted 11 1 light	
AC Disconnect	
detector - CO	
detector - smoke	

ELECTRICAL PLAN NOTES:

- E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E-3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E-4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E-6 ELECTRICAL CONTRY SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E-8 ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENs, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
- E-9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION.
- E-10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
- E-11 CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
- E-12 ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.
- E-13 A MINIMUM OF 75% OF PERMANENTLY INSTALLED LAMPS OR LIGHTING FIXTURES SHALL BE HIGH EFFICACY FBC EC SEC. R404.1.

ERKINGER
CONSTRUCTION
GROUPMICHEL AND MAUREEN
FLANDERSADDRESS:
171 NW Ambleside Dr.
Lot #1 Ph. I Cobblestone S/D,
Lac City, Florida 32055PARCEL ID:
24-3-16-02275-121(8119)PRINTED DATE:
Wednesday, June 29, 2022DRAWN BY: Matthew A. rkinger Sr
STRUCTURAL BY: Evan Beamsley

DRAWING NUMBER

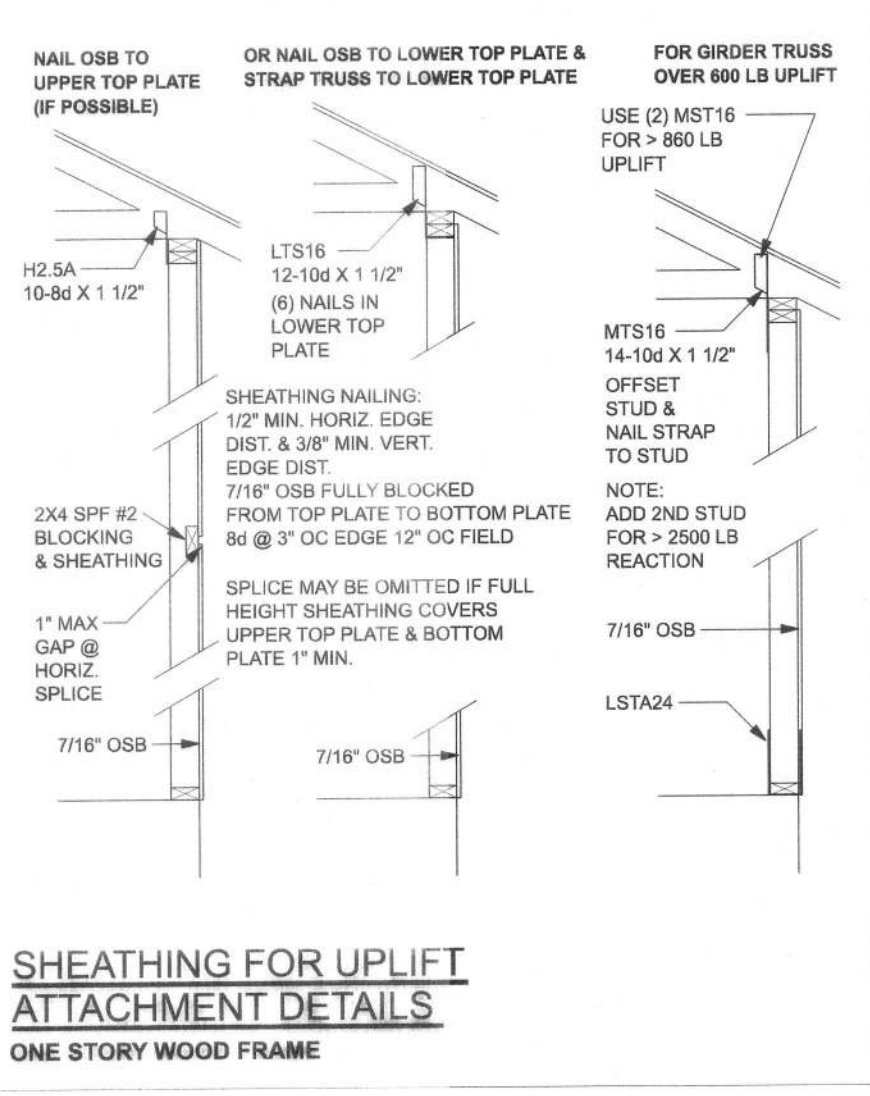
A3

OF 3 SHEETS

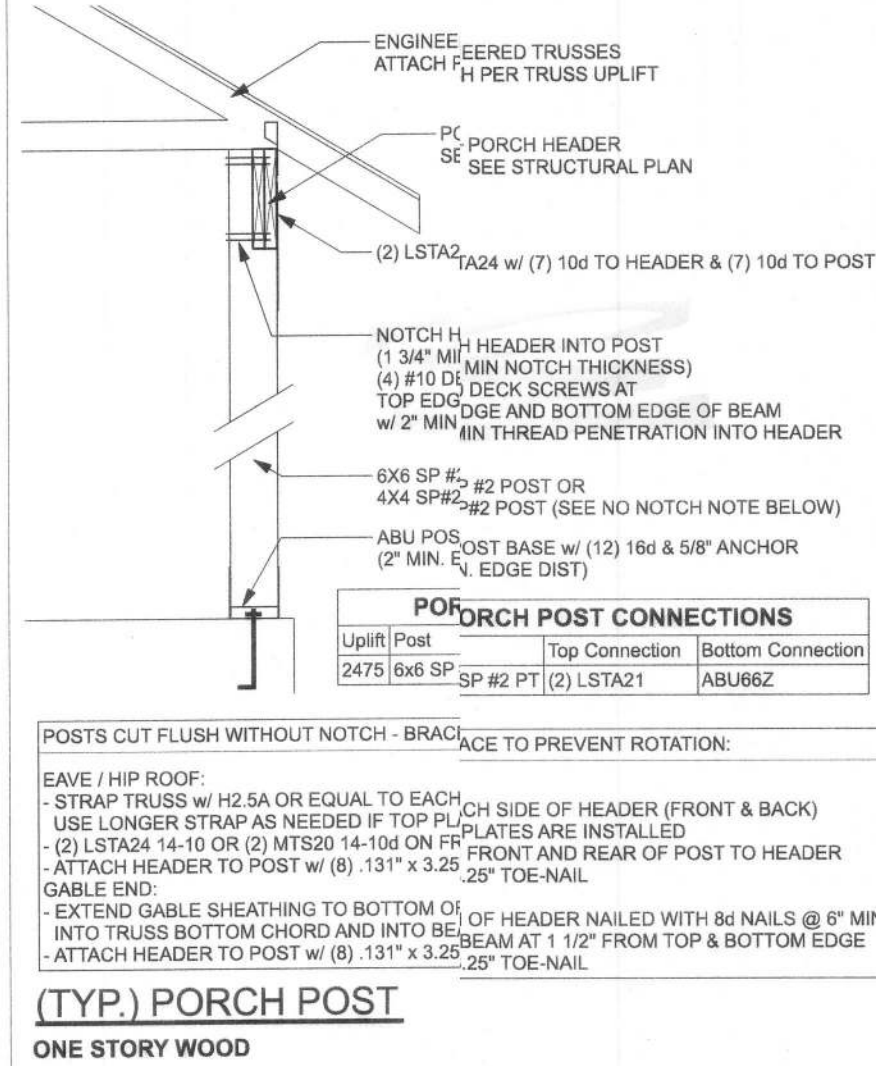
ROOF SHEATHING FASTENING TABLE (RAFT / TRUSS SG = 0.49)

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along edge	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSR-01 (2.38" x 0.113")	6" oc	6" oc

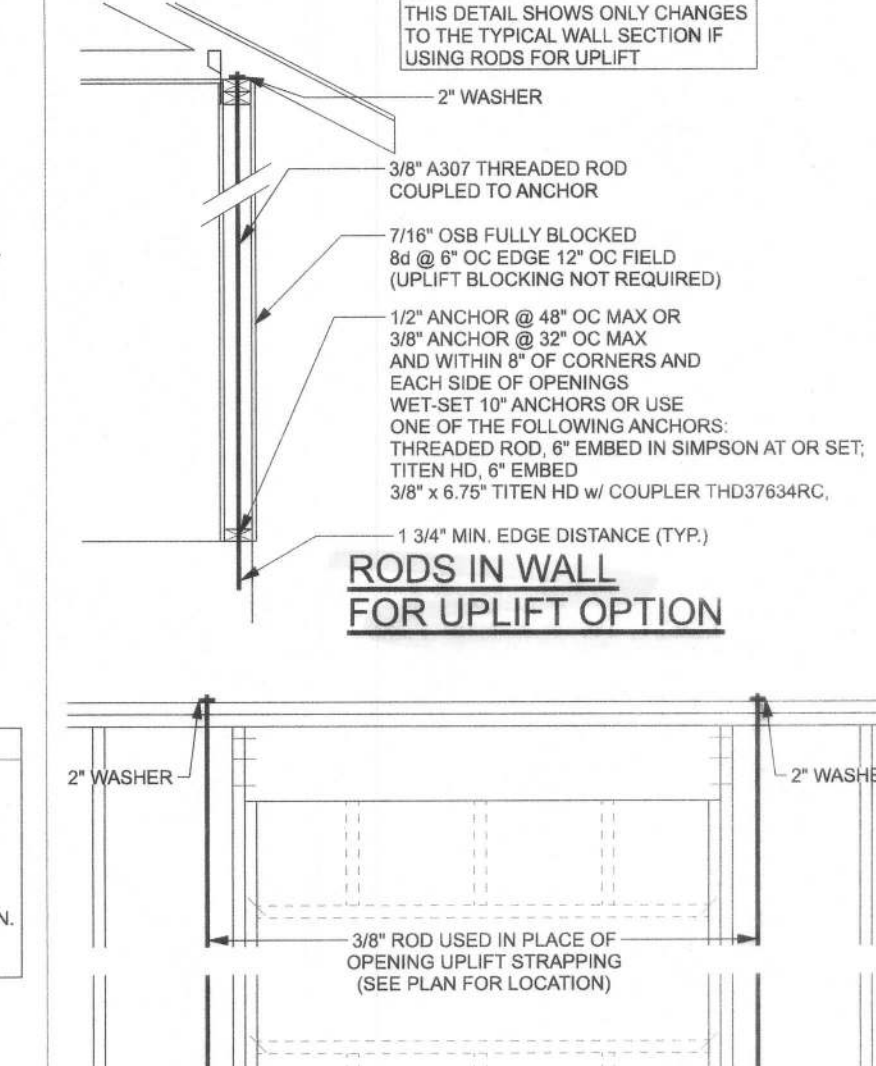
Note: For sheathing located a minimum of 4 feet from the perimeter of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 in on center along panel edges and 6 inches on center along intermediate supports in the panel field. This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturers product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME



(TYP.) PORCH POST ONE STORY WOOD



RODS IN WALL FOR UPLIFT OPTION

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8dX1 1/2"	4-8dX1 1/2"
575	495	H2.5A	5-8dX1 1/2"	5-8dX1 1/2"
1340	1015	H10A	9-10dX1 1/2"	9-10dX1 1/2"
720	620	LTST2-20	6-10dX1 1/2"	6-10dX1 1/2"
1000	860	MTS12-30	7-10dX1 1/2"	7-10dX1 1/2"
1450	1245	HTS20-30	12-10dX1 1/2"	12-10dX1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1840	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemm	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titan HD
4235	3840	HTTA	18-16dX2 1/2"	1/2"x12" Titan HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titan HD
4235	3840	HTTA	18-16dX2 1/2"	1/2"x12" Titan HD
Uplift SP	Uplift SPF	Stud Bases @ Stemm	To Stud	Anchor
2200	ABU44		12-16d	5/8"x12" Drill & Epoxy
2300	ABU66		12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Stud Bases @ Mono	To Post	Anchor
2200	ABU44		12-16d	5/8"x7" Drill & Epoxy
2300	ABU66		12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

STUD HEIGHT	STUD DEFLECTION LIMIT	STUD SPACING
10'-0" TO 11'-0"	1/240	16" OC
11'-0" TO 12'-0"	1/240	16" OC
12'-0" TO 13'-0"	1/240	16" OC
13'-0" TO 14'-0"	1/240	16" OC
14'-0" TO 15'-0"	1/240	16" OC
15'-0" TO 16'-0"	1/240	16" OC
16'-0" TO 17'-0"	1/240	16" OC
17'-0" TO 18'-0"	1/240	16" OC
18'-0" TO 19'-0"	1/240	16" OC
19'-0" TO 20'-0"	1/240	16" OC

GRADE & SPECIES TABLE	Fb	E
2x8 SP #2	925	1.4
2x10 SP #2	800	1.4
2x12 SP #2	750	1.4
GLB 24F-V3 SP	2800	1.9
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	2950	2.0
PSL PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS-TO-TRUSS CONNECTIONS, UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS AND PROVIDE FOOTINGS FOR INTERIOR UPLIFT AND REACTION LOADS. THE TRUSS DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS DESIGNER'S REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTIONS EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT THROUGH REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, $f_y = 40$ KSI. ALL LAP SPICES 4" DB (25' FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-88, UN.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE, NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A 3/8" ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

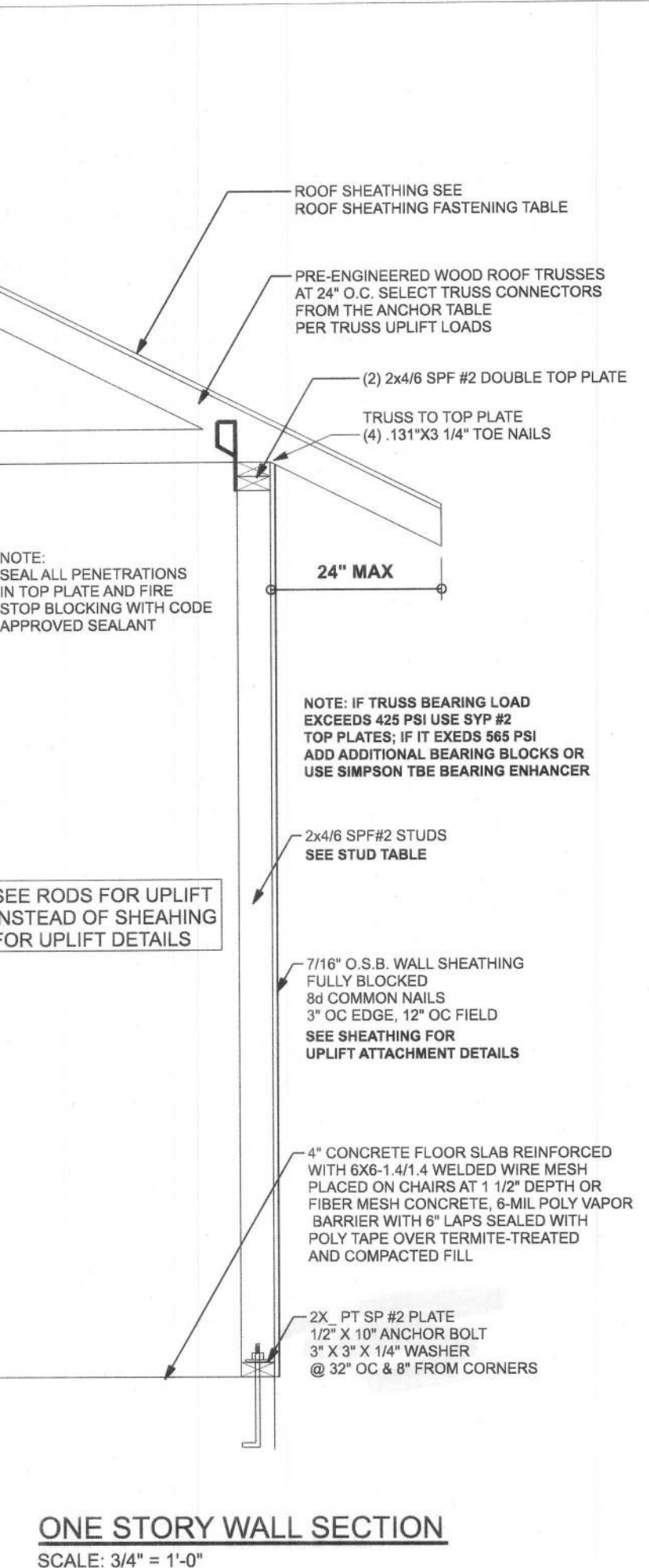
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

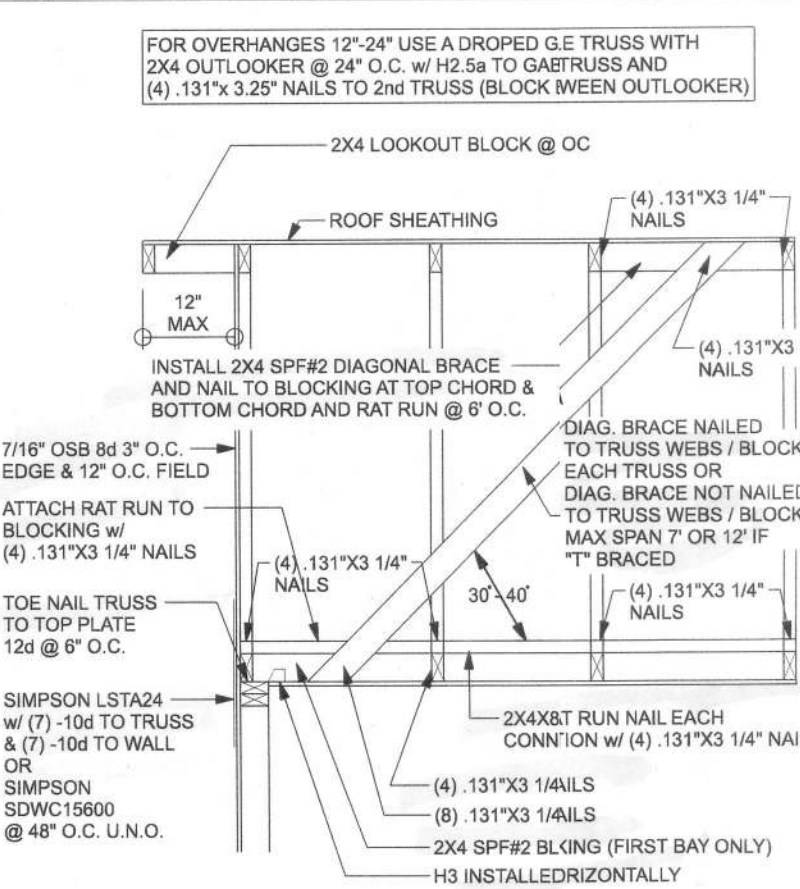
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, TRUSS-TO-TRUSS CONNECTIONS, UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

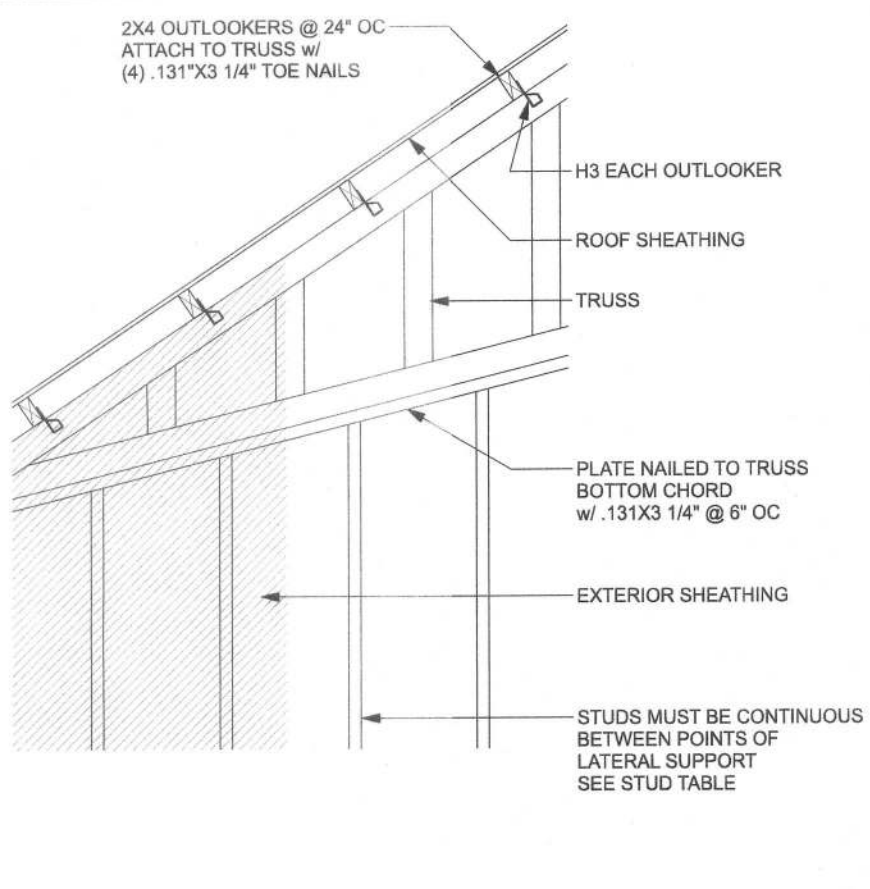
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



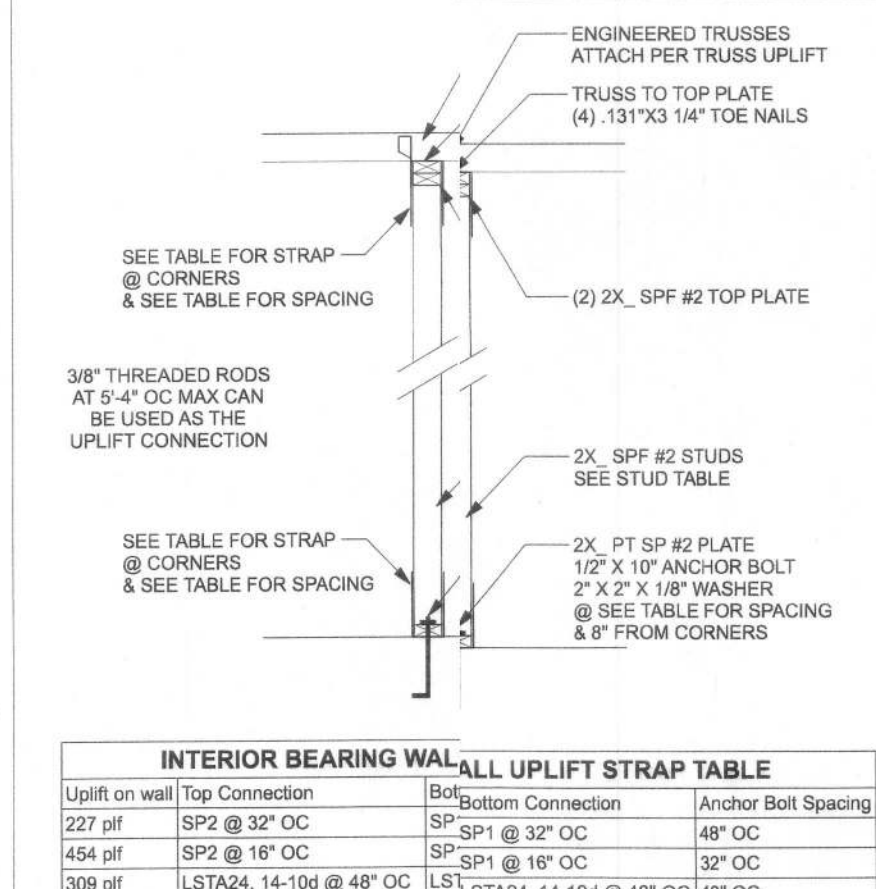
ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"



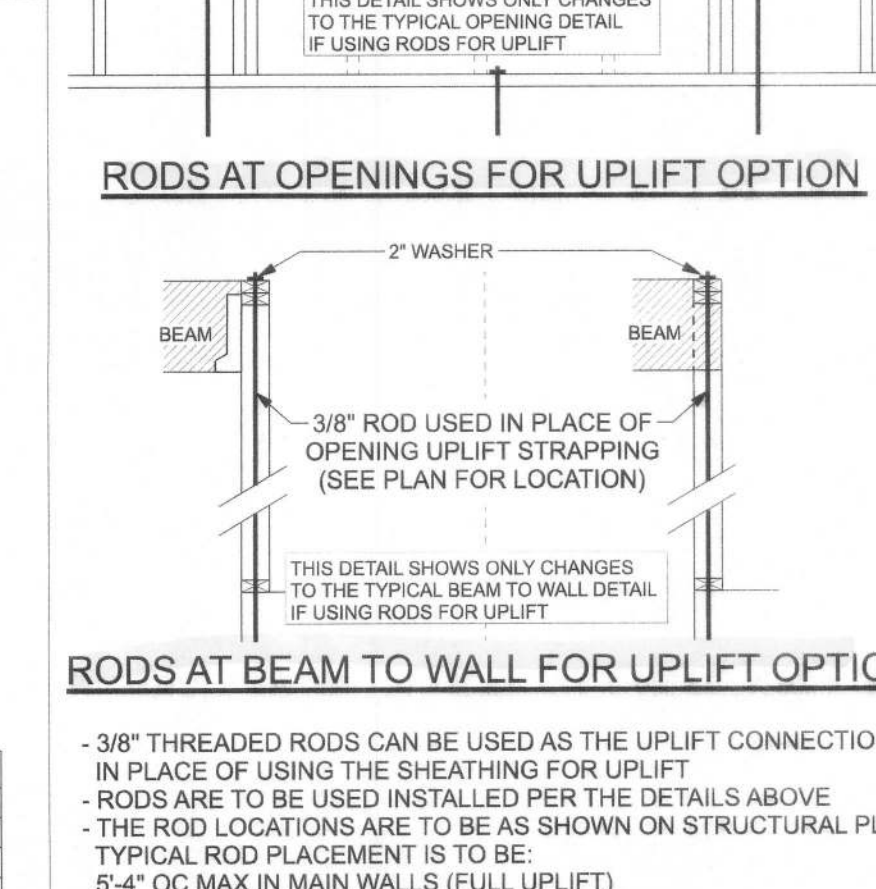
SPACE RAT RUN & DIAGONAL BRACE 6" x 6" O. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, ETC, ENCLOSED



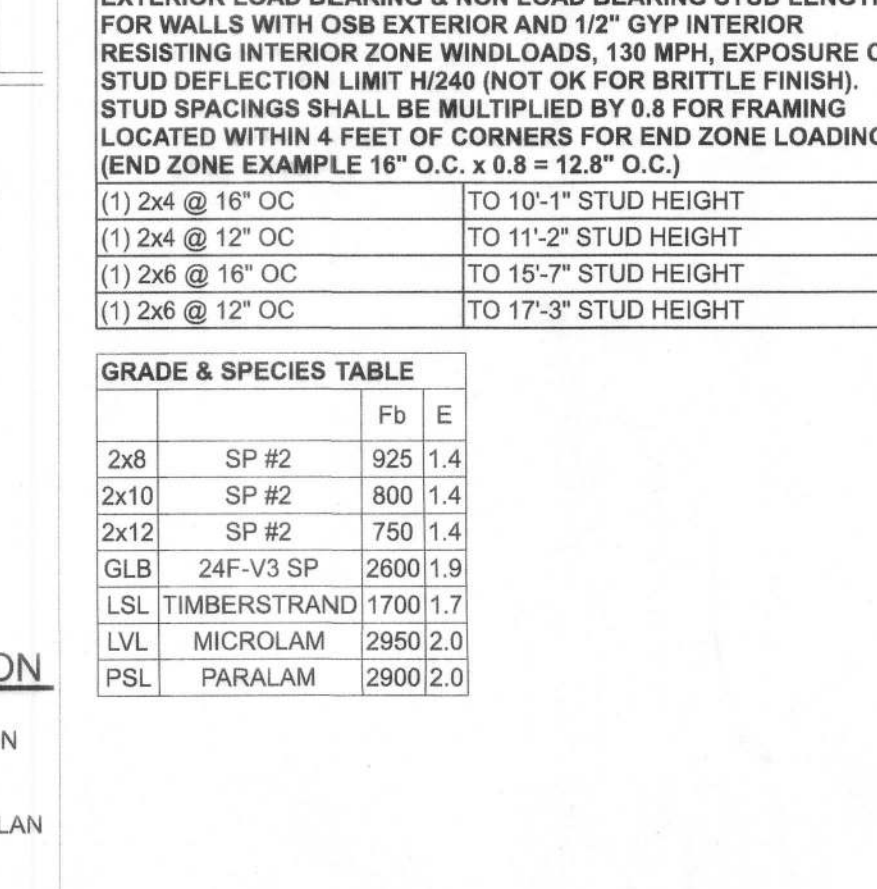
(TYP.) GABLE BRACING DETAIL WOOD FRAME



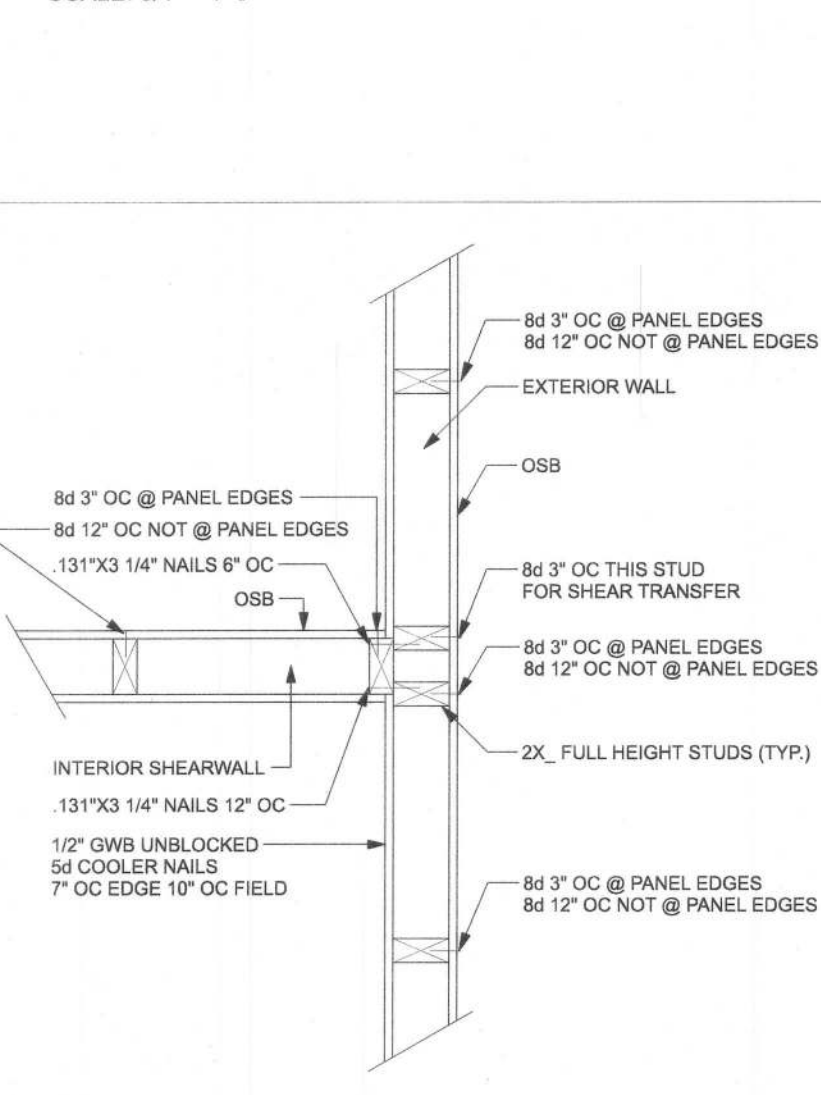
(TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME



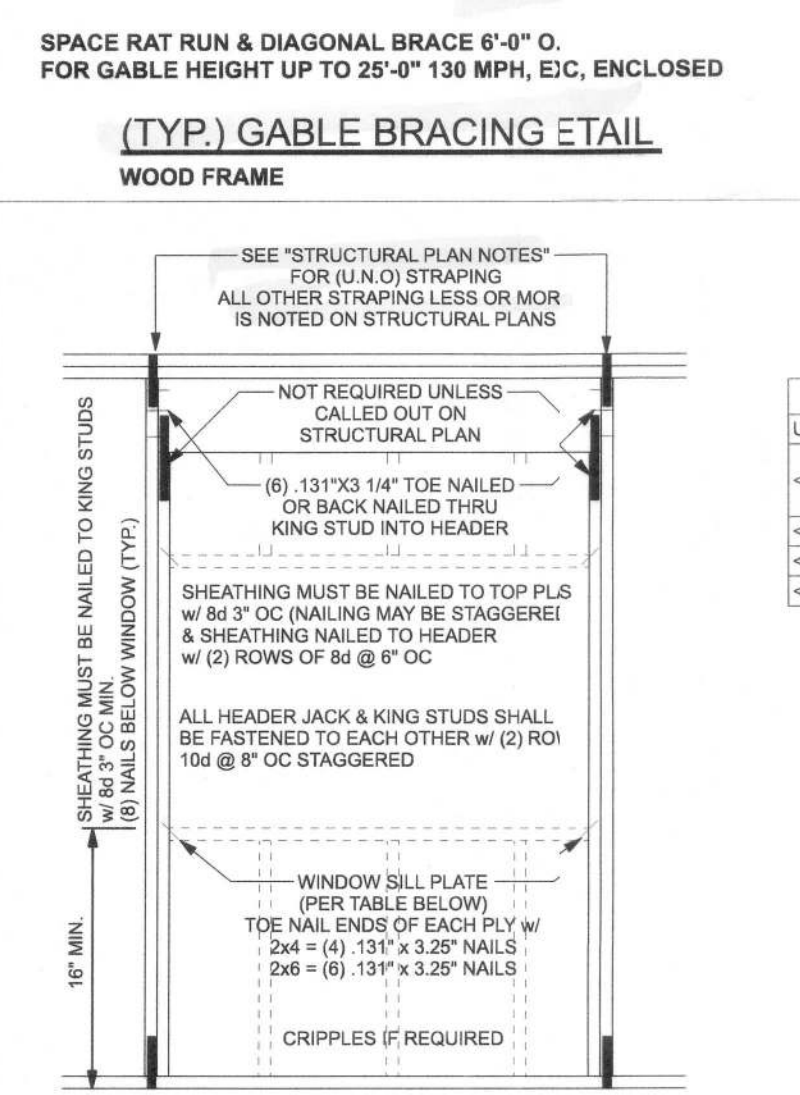
RODS AT BEAM TO WALL FOR UPLIFT OPTION



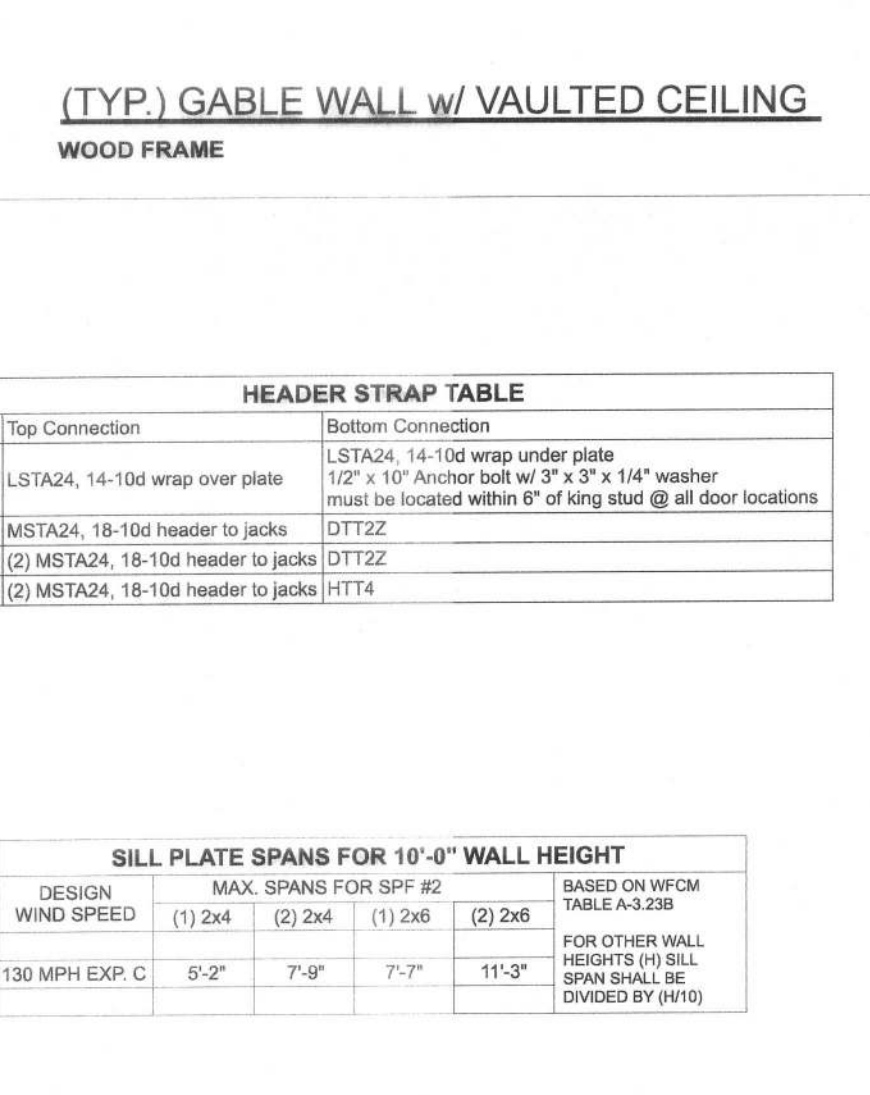
RODS FOR UPLIFT INSTEAD OF SHEATHING



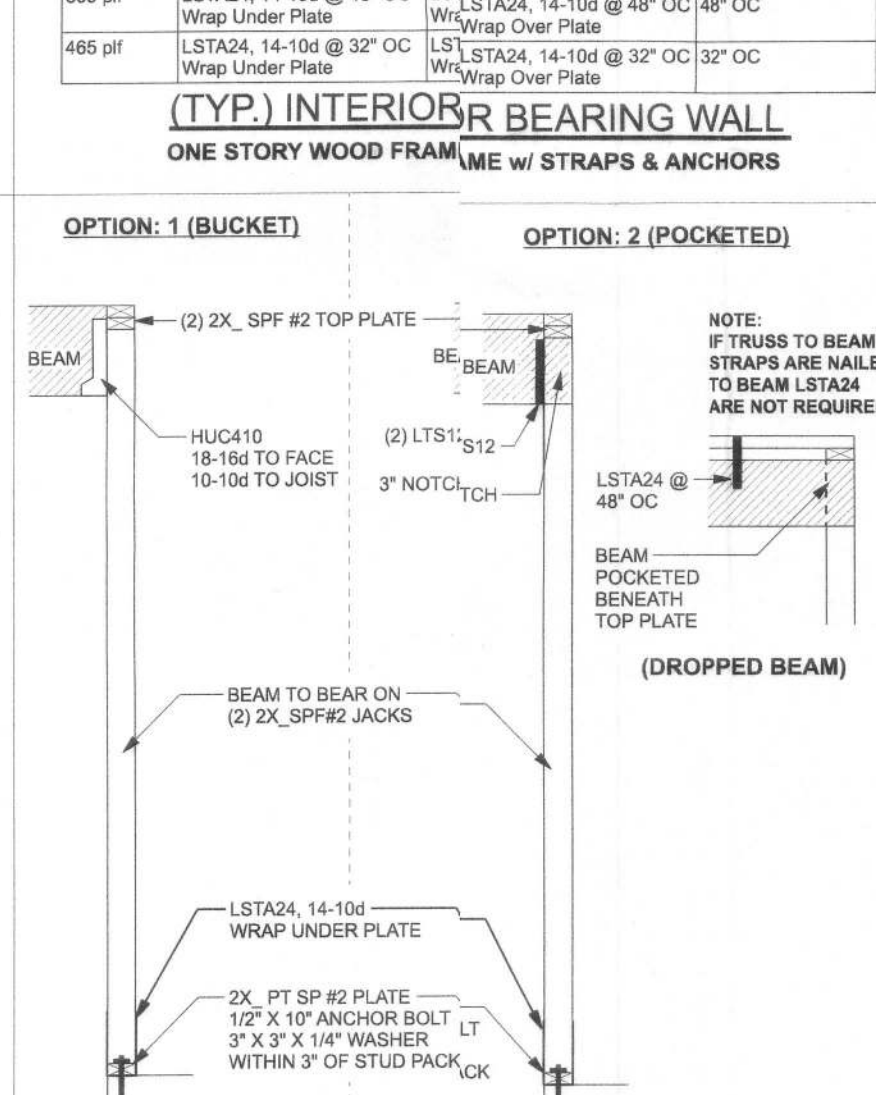
(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



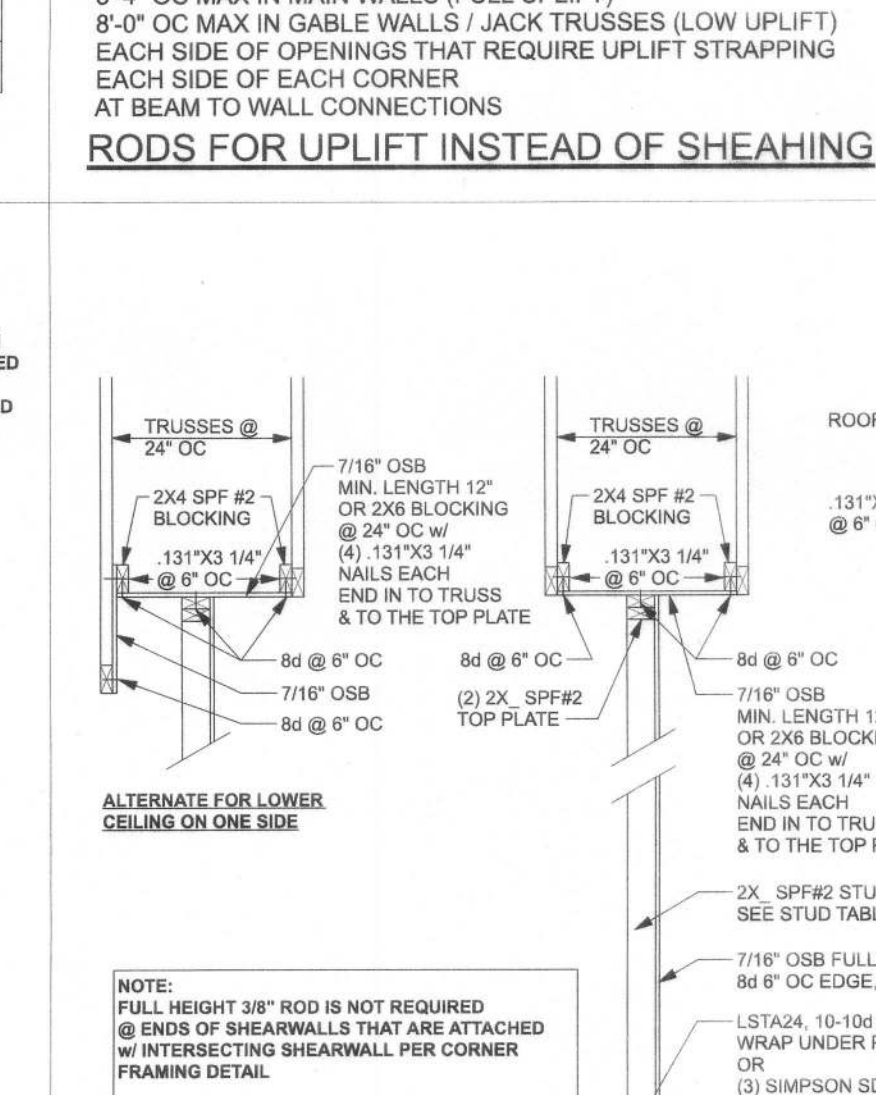
TYPICAL HEADER STRAPIN DETAIL ONE STORY WOOD FRAME w/ STRAPS & AHORS



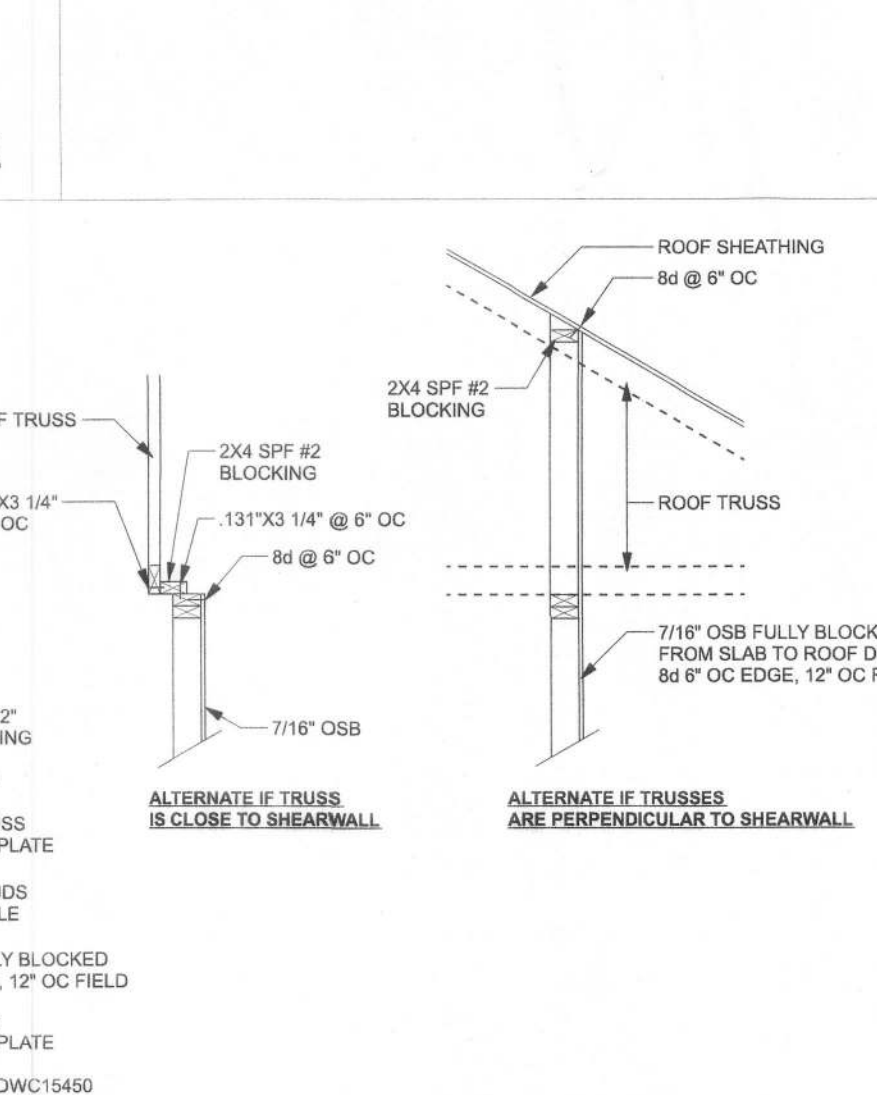
(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS



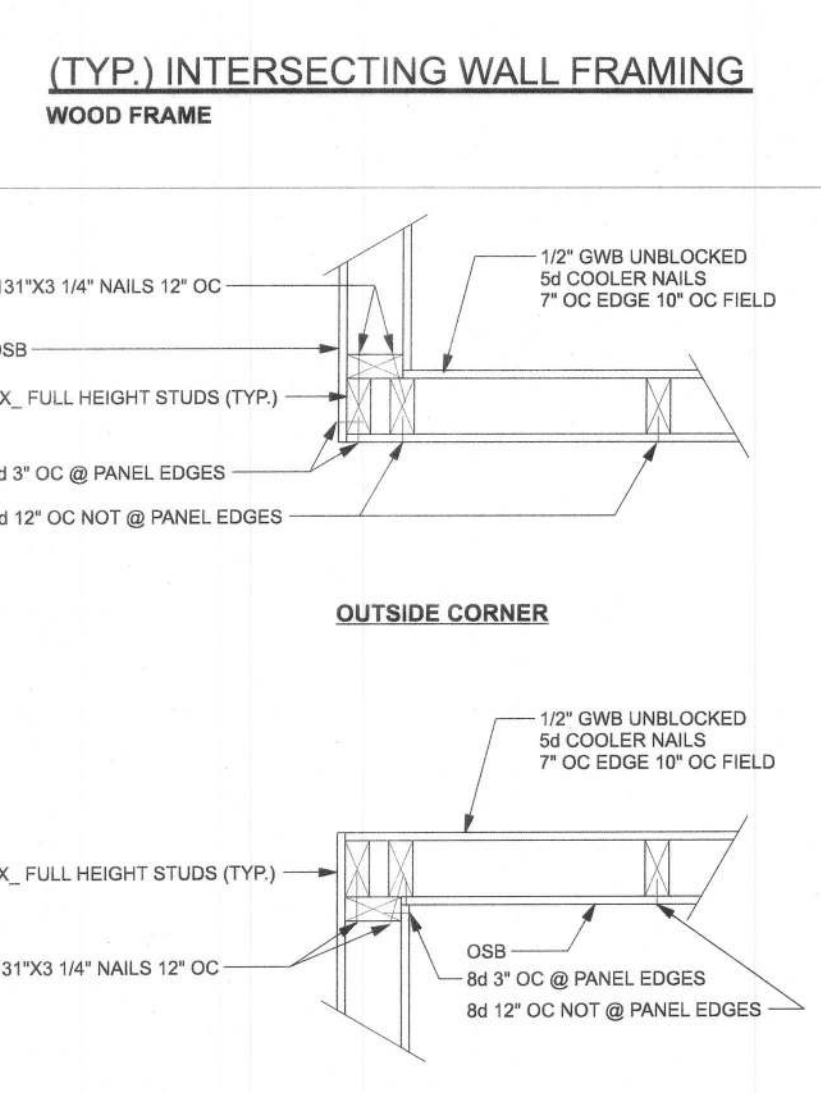
(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS



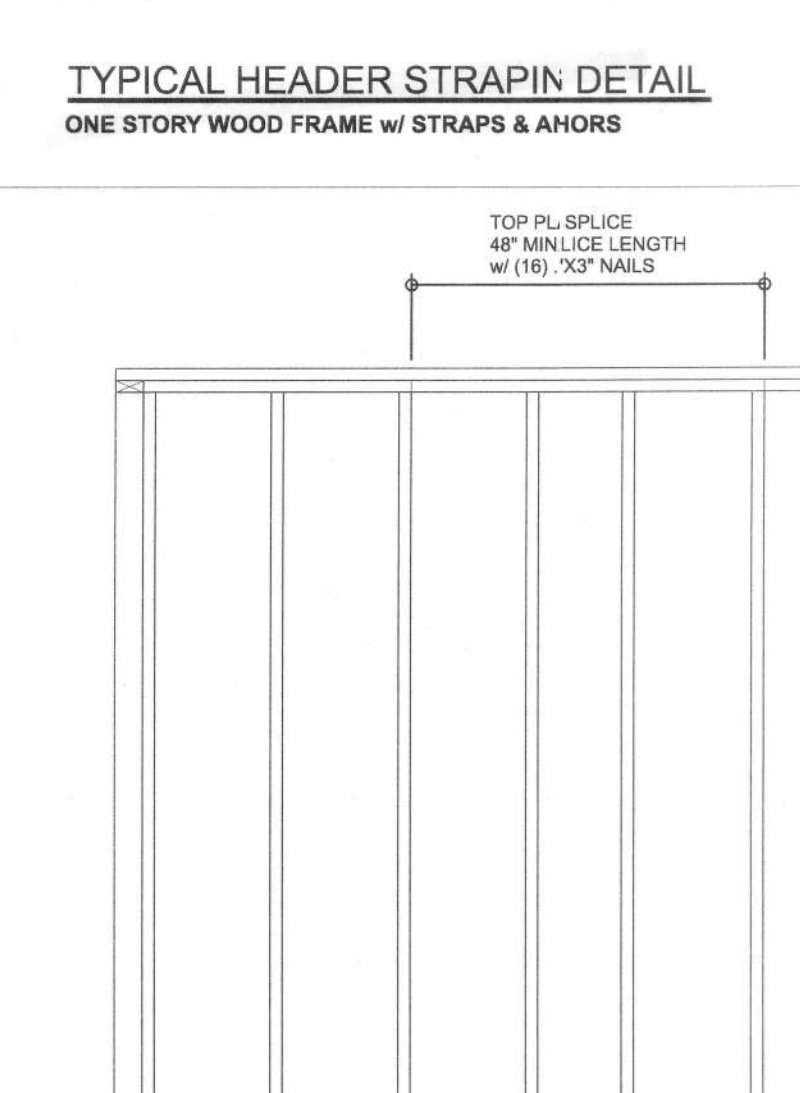
(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS



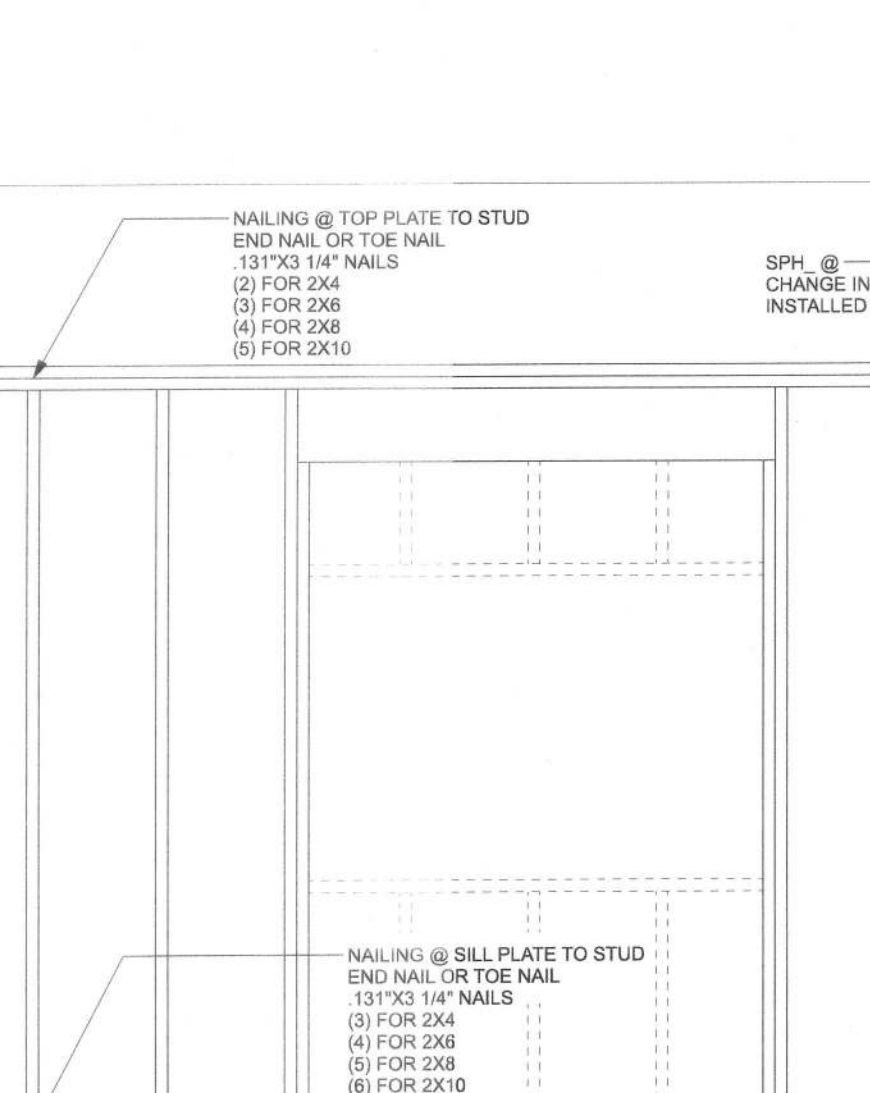
(TYP.) BEAM TO WALL WOOD FRAME w/ STRAPS & ANCHORS



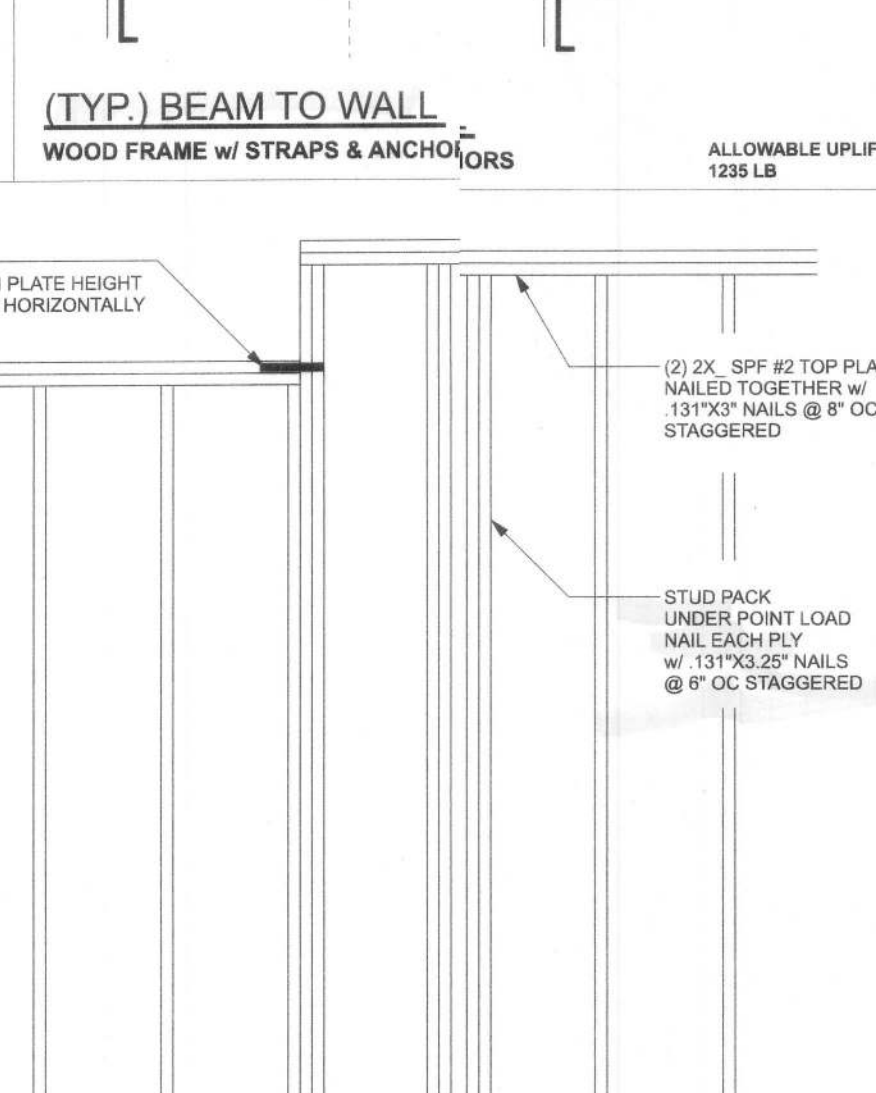
(TYP.) CORNER FRAMING WOOD FRAME



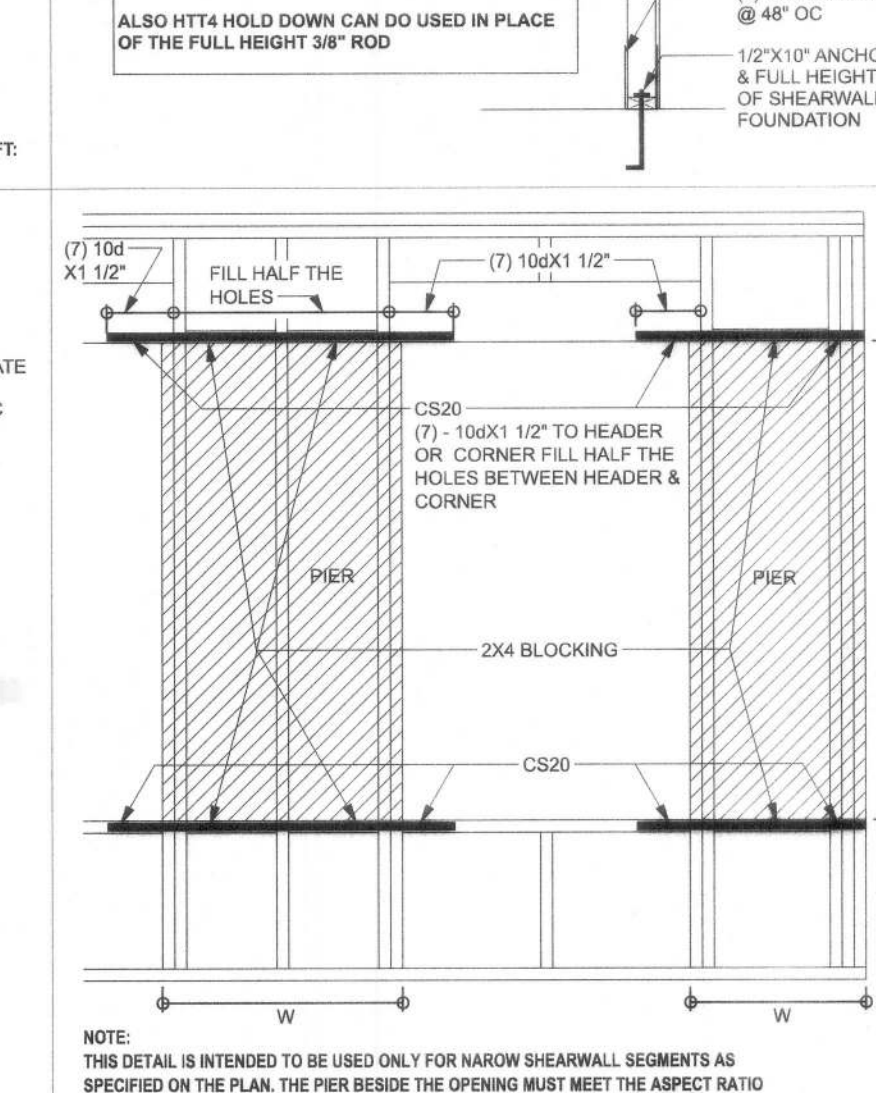
(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



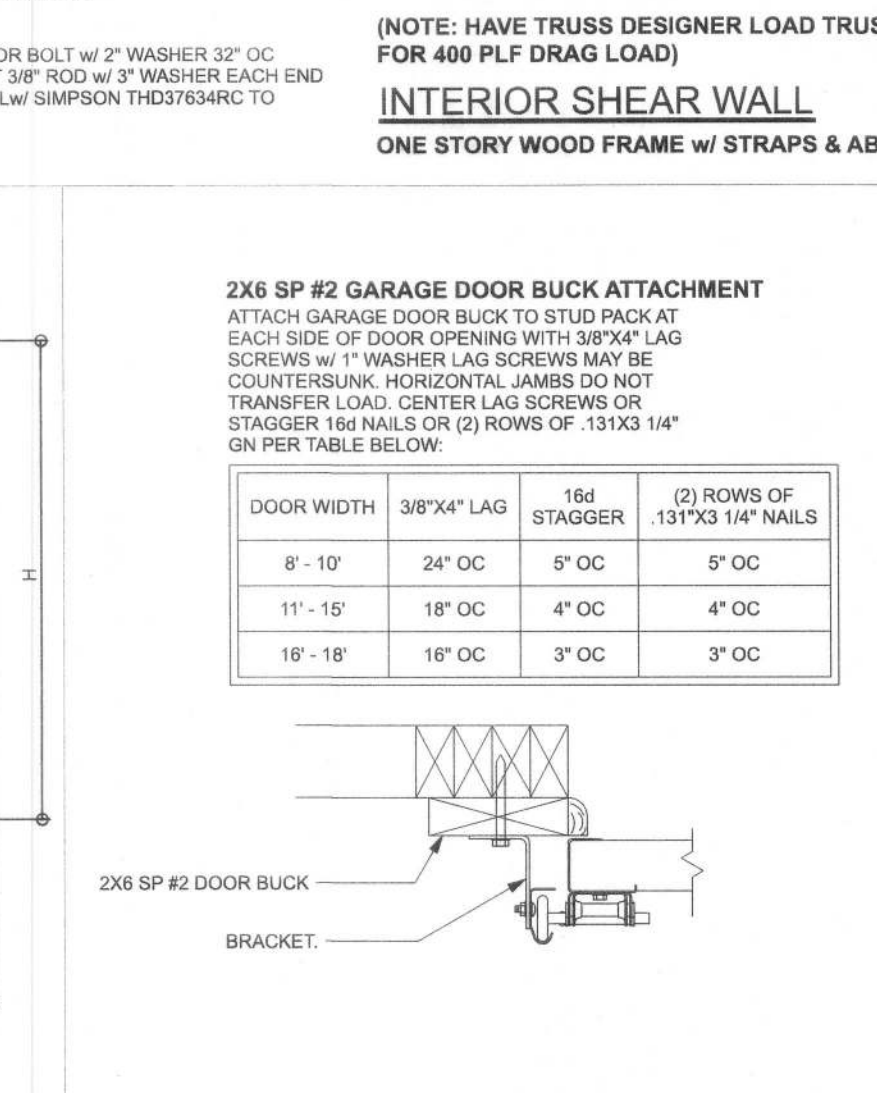
(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME



(TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME

DESIGN CRITERIA & LOADS:	7TH EDITION
BUILDING CODE	FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
BASIC WIND SPEED	130 MPH
WIND EXPOSURE	C
TOPOGRAPHIC FACTOR	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	40 PSF LIVE LOAD
ROOFS OTHER THAN SLEEPING ROOM	30 PSF LIVE LOAD
SLEEPING ROOMS	40 PSF LIVE LOAD
ROOF LOADING	20 PSF LIVE LOAD
FLAT OR < 4:12	12 PSF LIVE LOAD
4:12 TO < 12:12	12 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)	WIND AREA (FT ²)	ZONE 1 INTERIOR	ZONE 2 END & FROM ALL OUTSIDE CORNER
0 - 20	+25.0(Vsust) -27.9(Vsust)	+25.0(Vsust) -34.2(Vsust)	-27.9(Vsust) -37.9(Vsust)
20 - 40	+22.6(Vsust) -25.5(Vsust)	+22.6(Vsust) -31.8(Vsust)	-25.5(Vsust) -35.5(Vsust)
40 - 60	+20.1(Vsust) -23.0(Vsust)	+20.1(Vsust) -29.3(Vsust)	-23.0(Vsust) -33.0(Vsust)
60 - 80	+17.6(Vsust) -20.5(Vsust)	+17.6(Vsust) -26.8(Vsust)	-20.5(Vsust) -30.5(Vsust)
80 - 100	+15.1(Vsust) -18.0(Vsust)	+15.1(Vsust) -24.3(Vsust)	-18.0(Vsust) -28.0(Vsust)
100 - 120	+12.6(Vsust) -15.5(Vsust)	+12.6(Vsust) -21.8(Vsust)	-15.5(Vsust) -25.5(Vsust)
120 - 140	+10.1(Vsust) -13.0(Vsust)	+10.1(Vsust) -19.3(Vsust)	-13.0(Vsust) -23.0(Vsust)
140 - 160	+7.6(Vsust) -10.5(Vsust)	+7.6(Vsust) -16.8(Vsust)	-10.5(Vsust) -20.5(Vsust)
160 - 180	+5.1(Vsust) -8.0(Vsust)	+5.1(Vsust) -14.3(Vsust)	-8.0(Vsust) -18.0(Vsust)
180 - 200	+2.6(Vsust) -5.5(Vsust)	+2.6(Vsust) -11.8(Vsust)	-5.5(Vsust) -15.5(Vsust)

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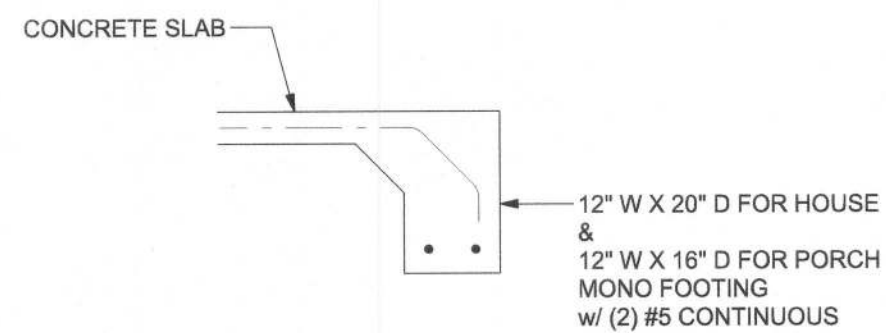
386.754.5419

disowaydesign@gmail.com

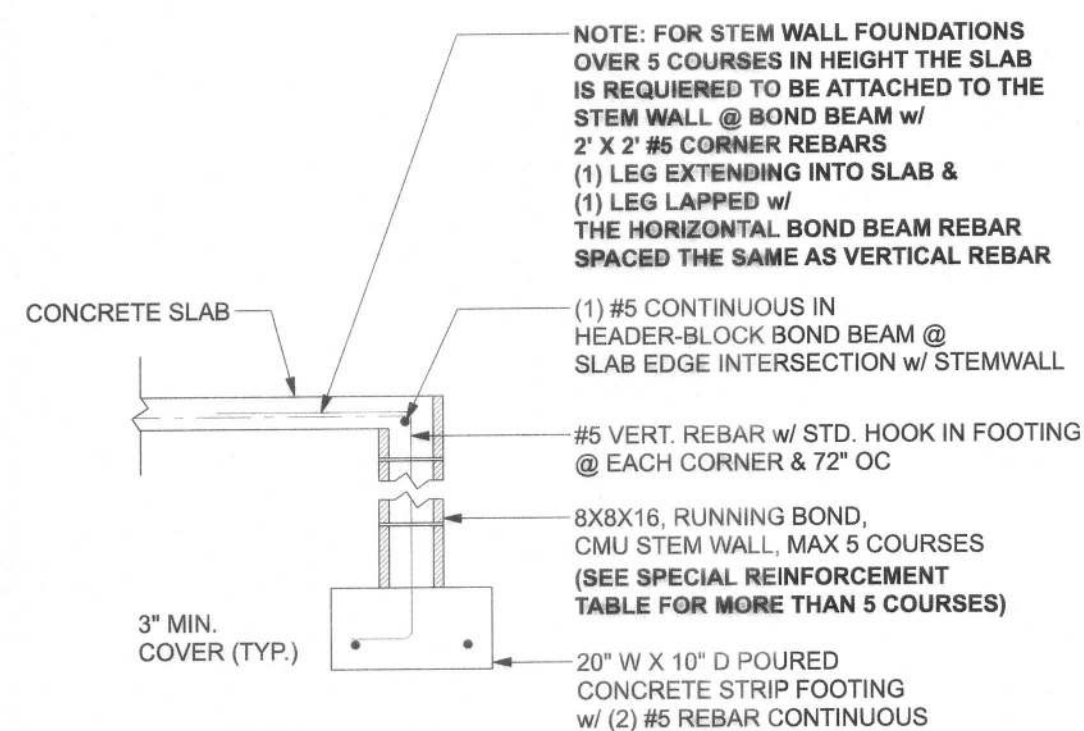
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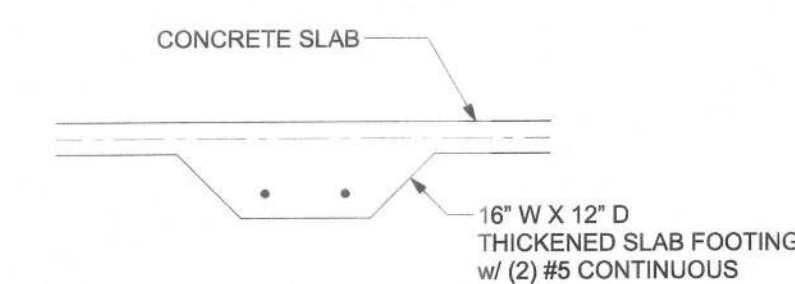
OF 3 SHEETS



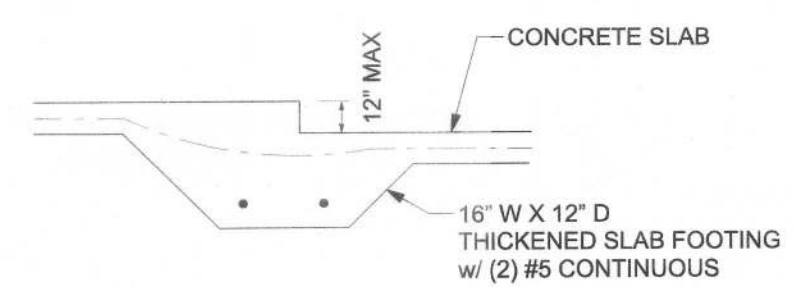
F1 S-2 OPTIONAL MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



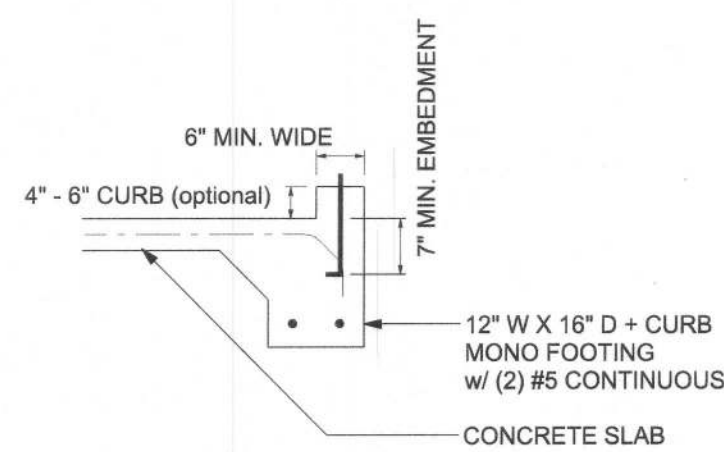
F1 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



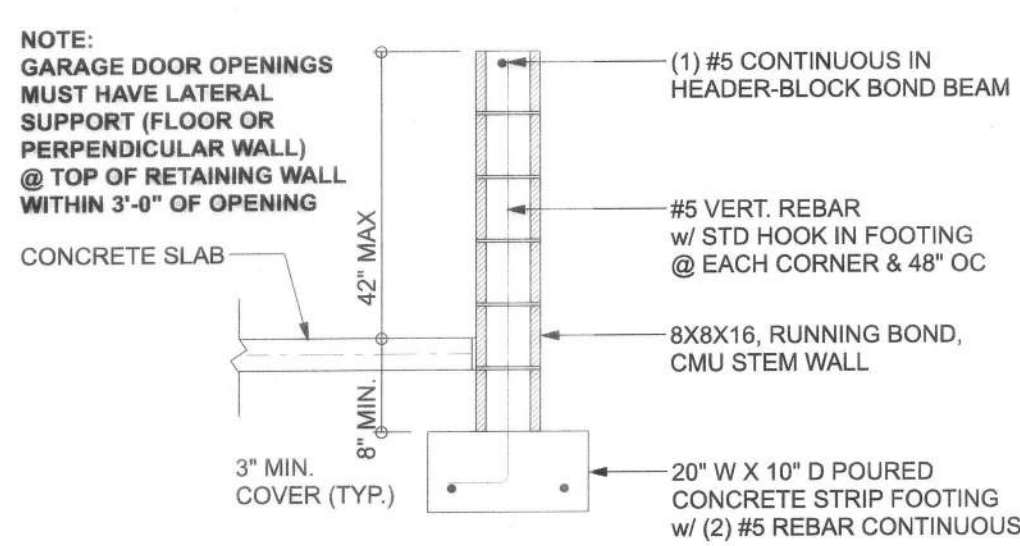
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



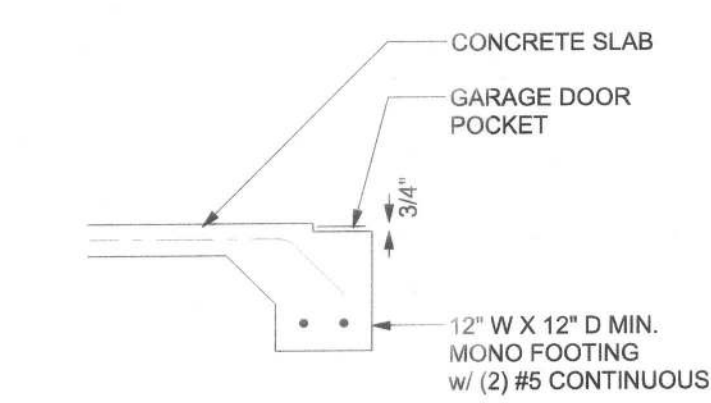
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



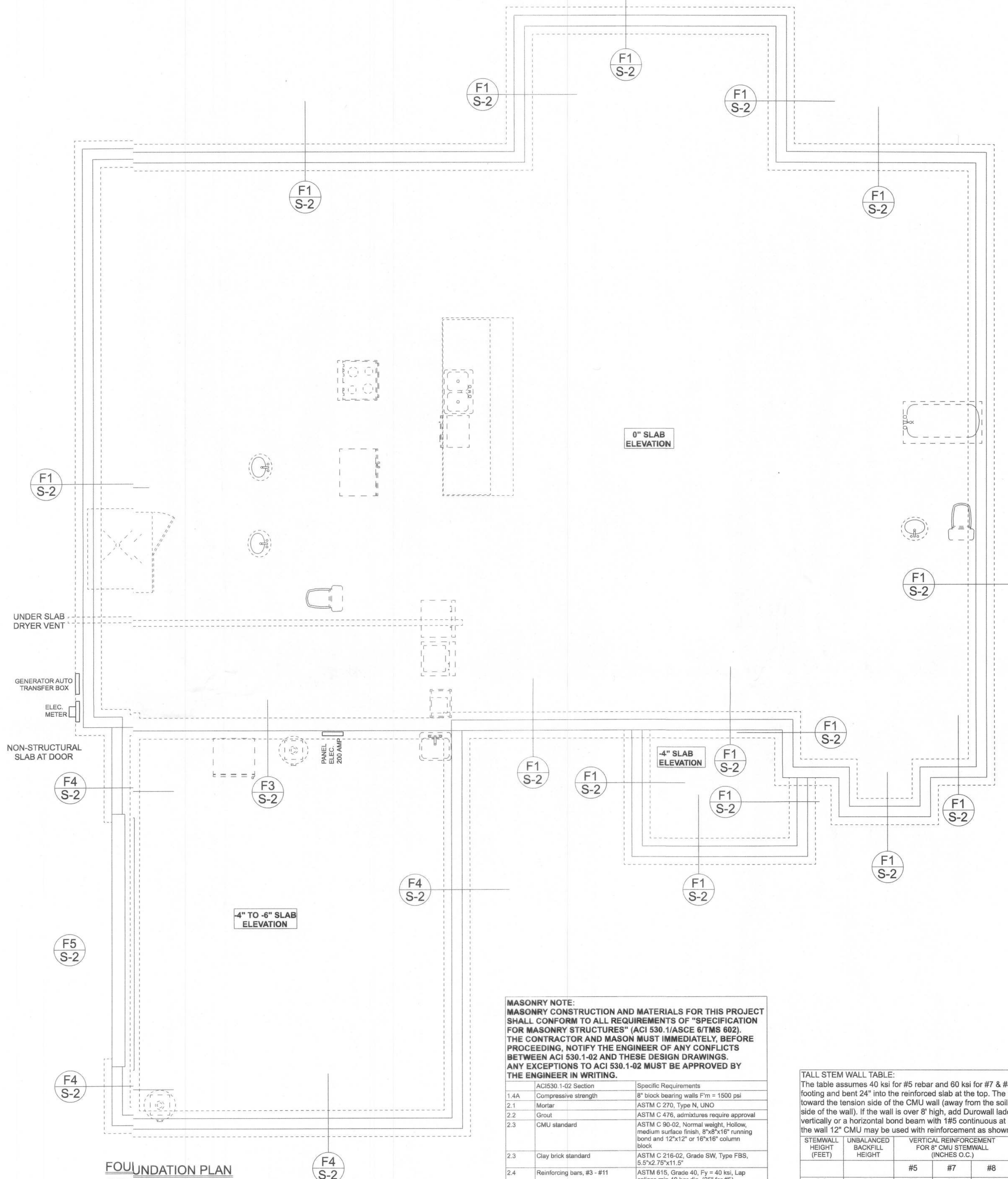
F4 S-2 OPTIONAL MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ #6-1/4" 4 WELDED WIRE MESH PLACED ON CHAIRS @ 1'-0" DEPTH OR FIBER MESH CONCRETE, 5-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL.

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
2.1	Mortar: ASTM C 270, Type N, UNO
2.2	Grout: ASTM C 476, admixtures require approval
2.3	CMU standard: ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard: ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11: ASTM 615, Grade 40, $F_y = 40$ ksi, Lap splices min 40 bar dia. (25" for #8)
2.4F	Coating for corrosion protection: Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A625, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection: Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories: Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints: Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL

ERKINGER CONSTRUCTION GROUP
MICHAEL AND MAUREEN FLANDERS
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1711 W. Lakeview Dr.
Lake City, Florida 32055
Parcel ID# 24-35-16-02275-121(119)
Columbia County

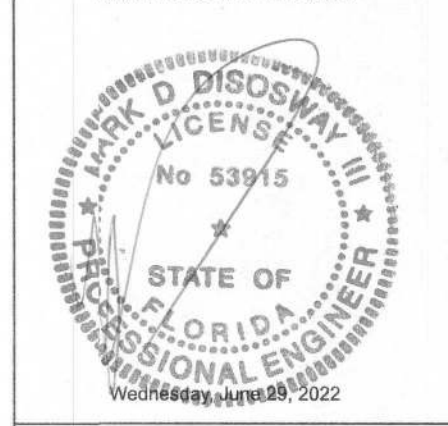
DIMENSIONS:
Stated dimensions supersede standard dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

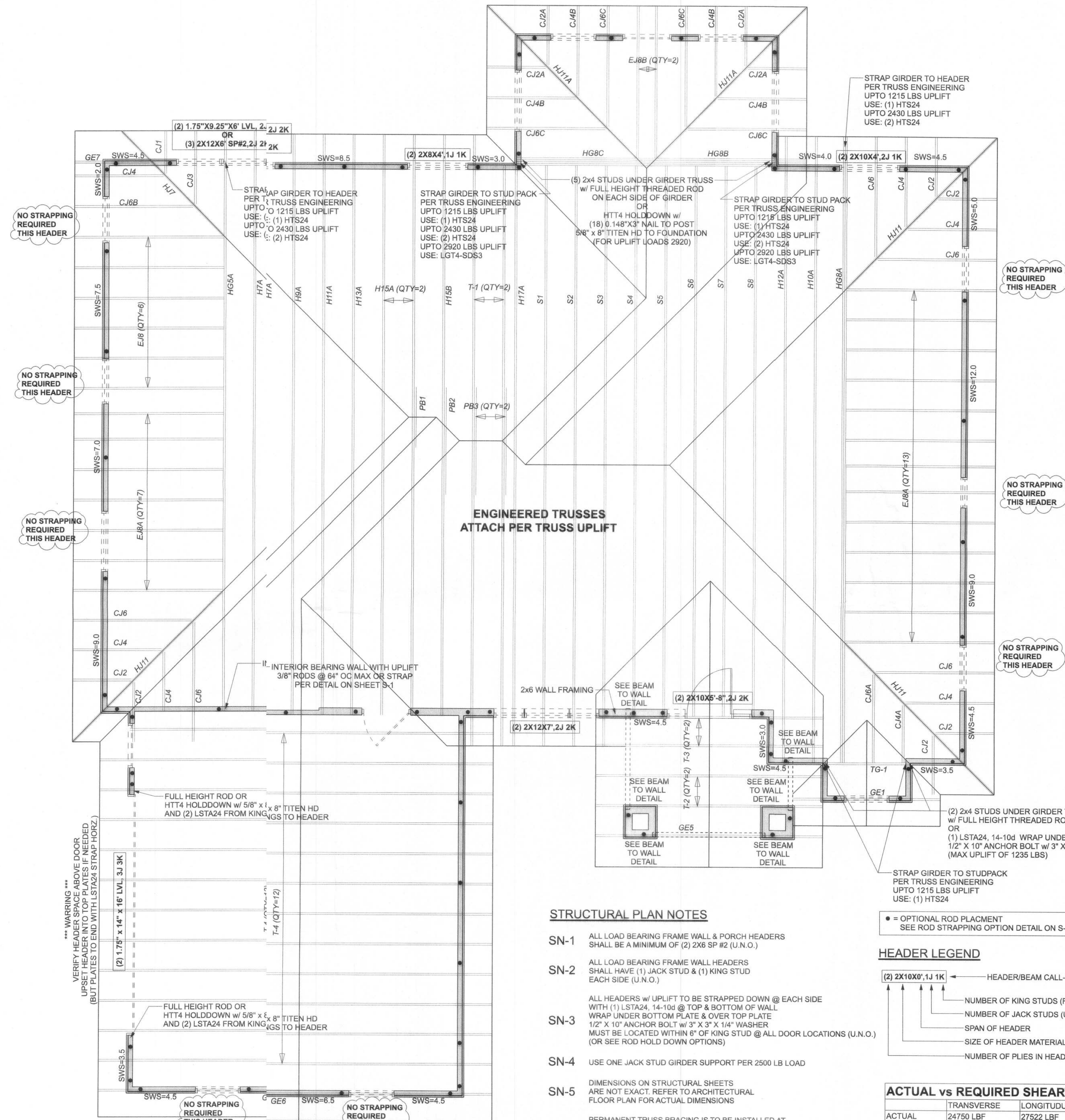
LIMITATION: This design is valid for one building at specified location.

MARK DISOWAY P.E. 53915



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JOB NUMBER:
220767
S-2
OF 3 SHEETS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

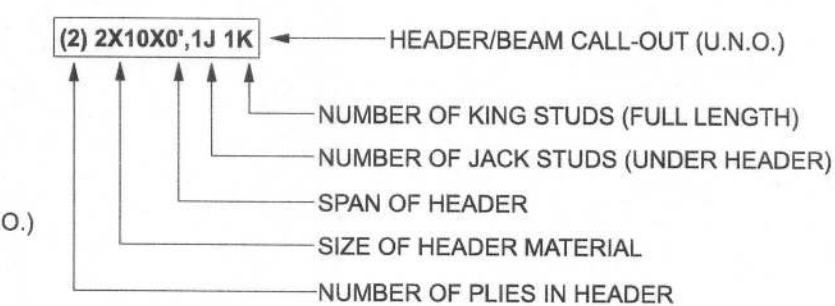
STRUCTURAL PLAN NOTES

- SN-1** ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2** ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3** ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL. WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER. MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.) (OR SEE ROD HOLD DOWN OPTIONS)
- SN-4** USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5** DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6** PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. SEMINOLE TRUSSES, INC JOB # B55557a 06-15-2022

• = OPTIONAL ROD PLACEMENT
SEE ROD STRAPPING OPTION DETAIL ON S-1

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	24750 LBF	27522 LBF
REQUIRED	19639 LBF	23644 LBF

**ERKINGER
CONSTRUCTION GROUP**

**MICHAEL AND MAUREEN
FLANDERS**

ADDRESS:
171 NW Ambleside Dr.
Lake City, Florida 32055
Parcel ID# 24-35-16-02275-121(6119)
Gadsden County

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
220767

S-3

(F 3 SHEETS