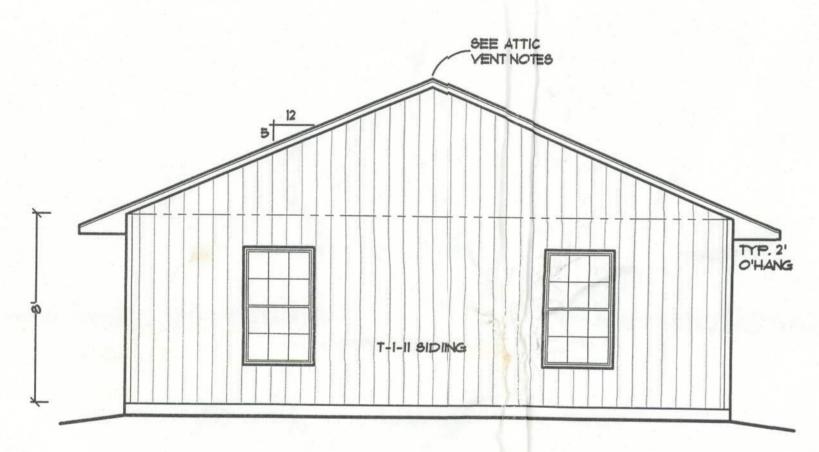
Code Tor Seven Sev

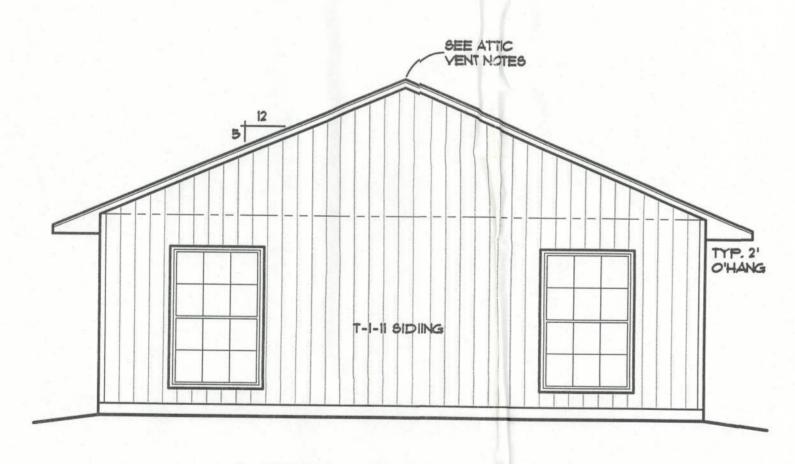
SEE ATTIC VENT NOTES METAL ROOFING T-1-II SIDING

REAR ELEVATION SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION

SCALE: 1/4 IN. = 1 FT.



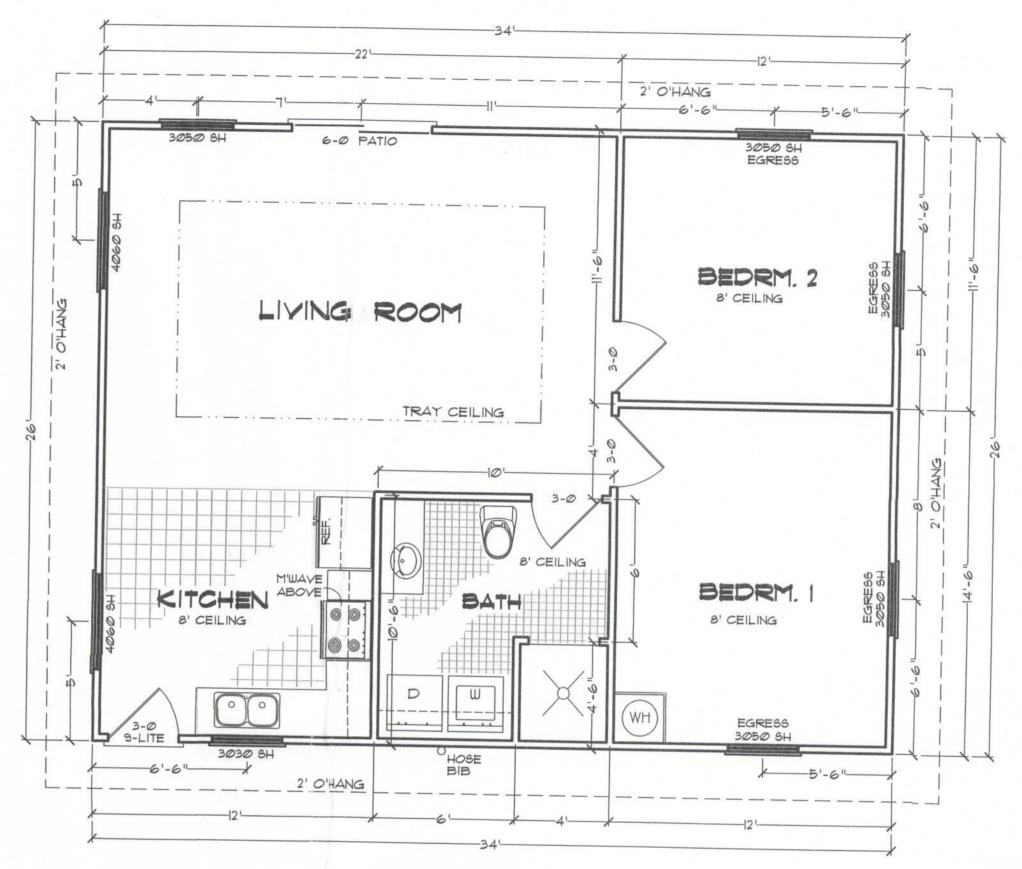
SCALE: 1/4 IN. = 1 FT

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion—resistant wire mesh, wit h 1 / 8 inch (3.2 mm) minimum to ¼ inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

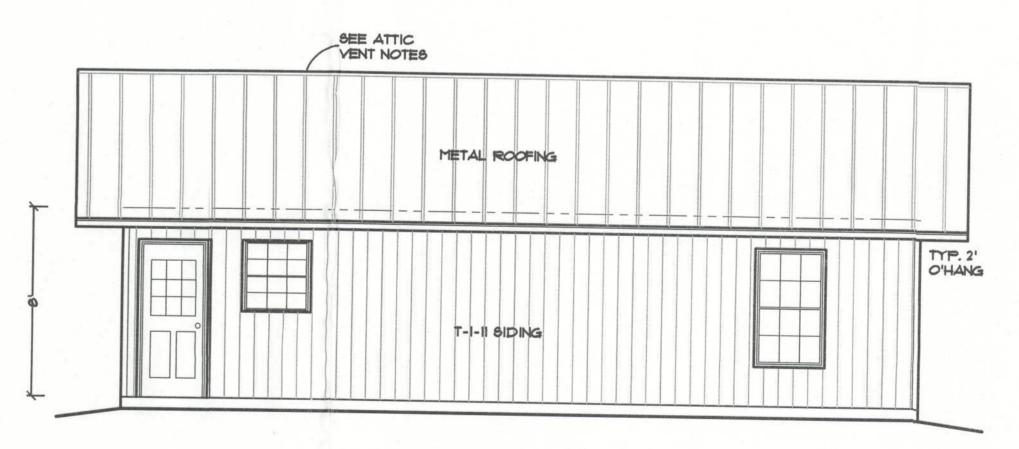
Marks Residence



FLOOR PLAN

884 SQ. FEET LIVING AREA

SCALE: 1/4 IN. = 1 FT.



FRONT ELEVATION

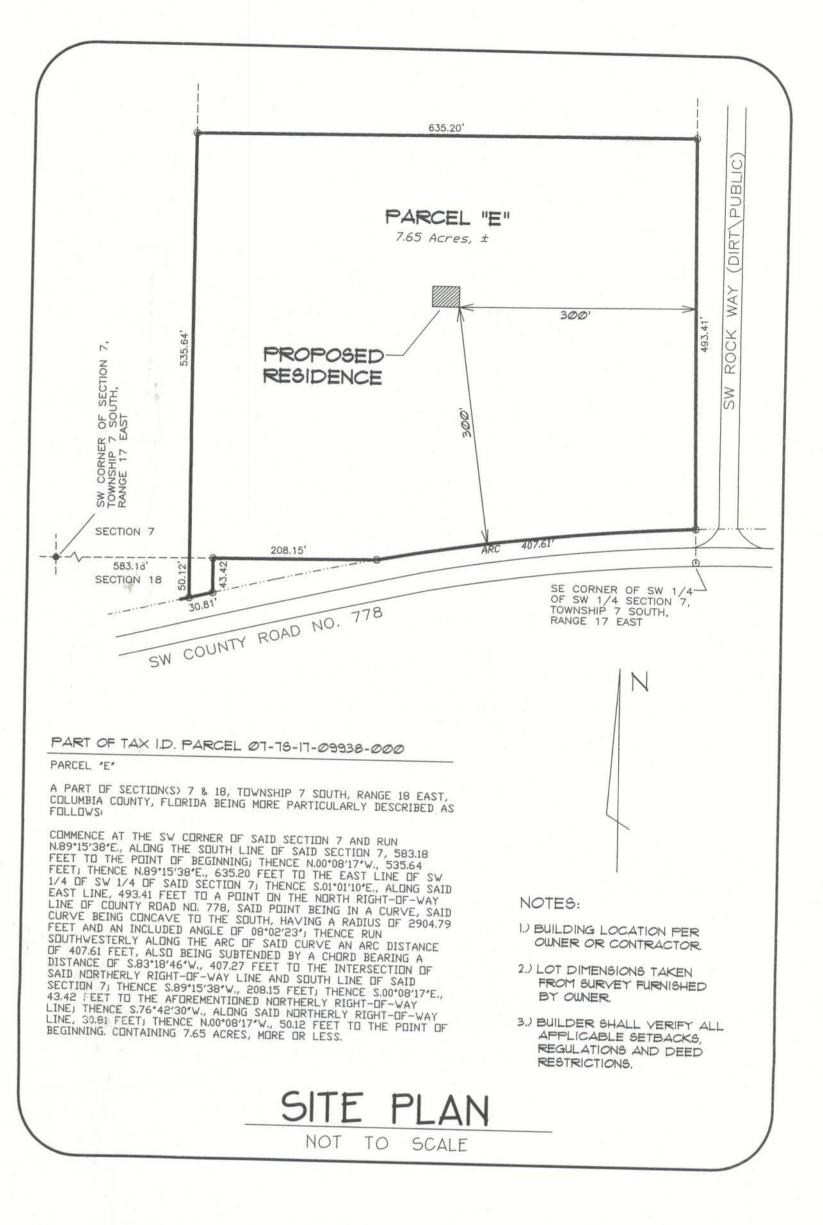
SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2010, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

ocation:_______ Job No.: 130 20 13 70 156 9 5 1

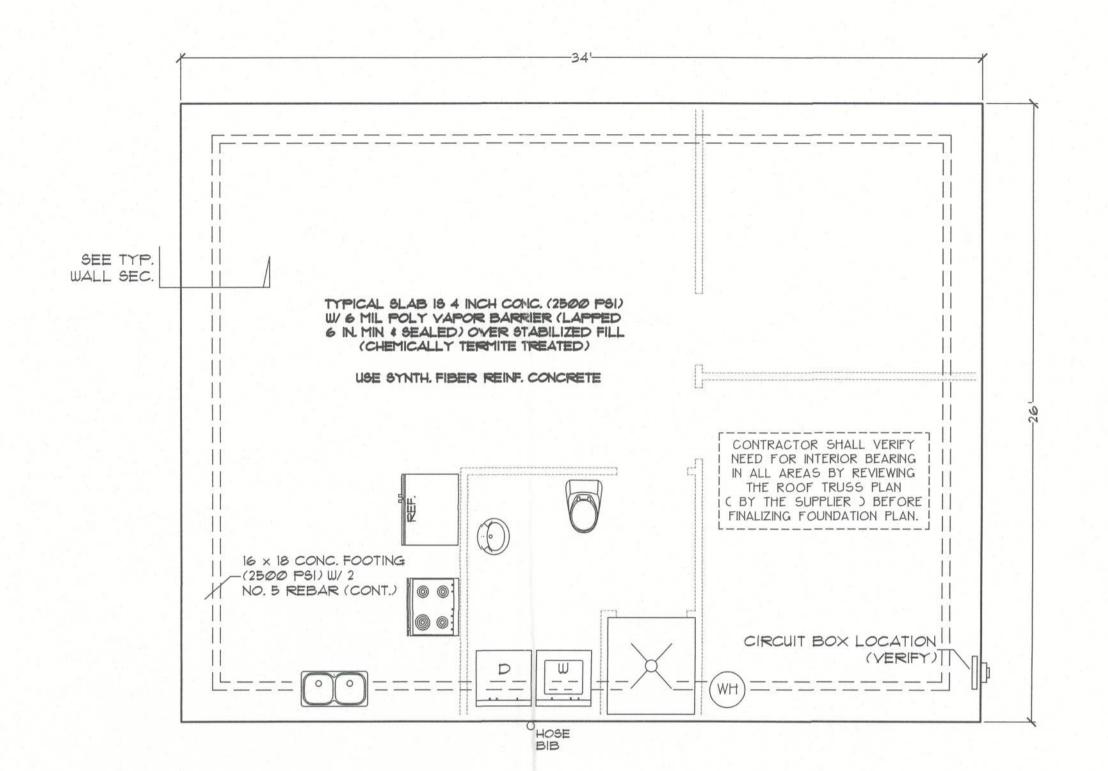


GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- G.) The Truss Manufactuer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 3.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.
- 9.) This Residence will have no heating / cooling system installed.

HEET: MARKS 13-002 1 OF 2 RESIDENCE AD FILE: 2-2-13 13-002 DRAWN: PREPARED BY: TIM DELBENE TAD Drafting + Technical Services CHECK: REV: REV: Phone (386) 755-5891

1



FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AIND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBLITY OF THE CONTRACTOR / OWNER.

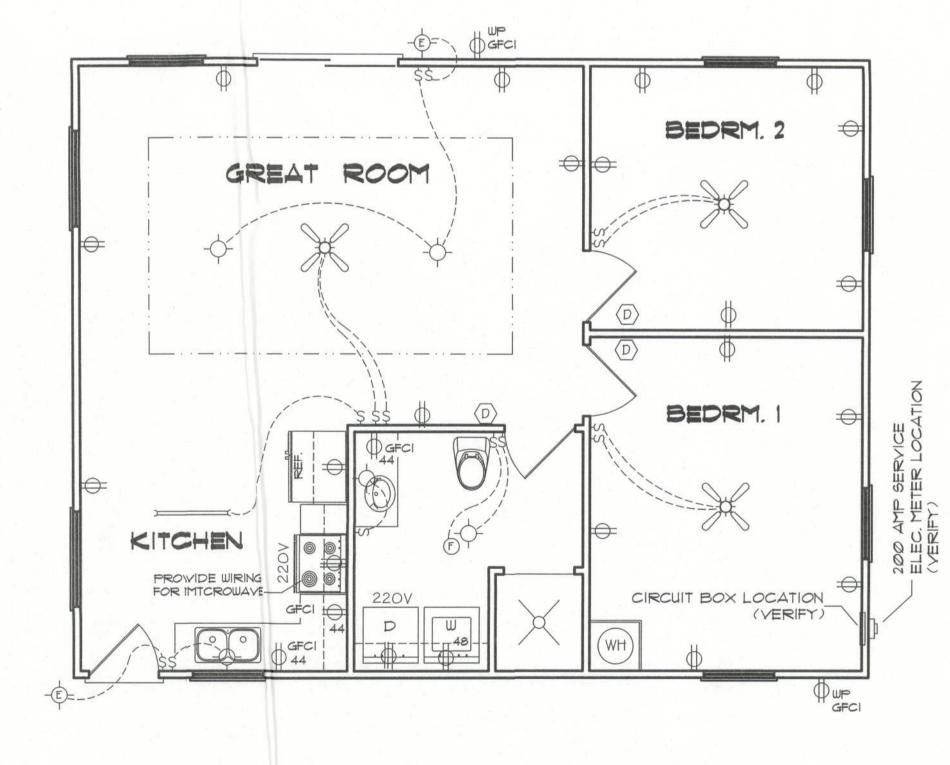
FOUNDATION PLAN SCALE: 1/4 IN. = 1 FT.

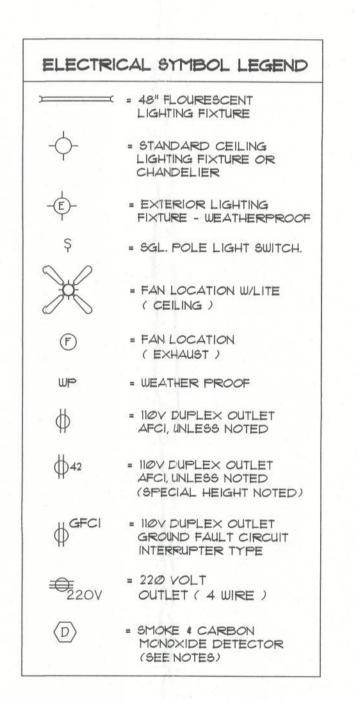
ELECTRICAL PLAN NOTES

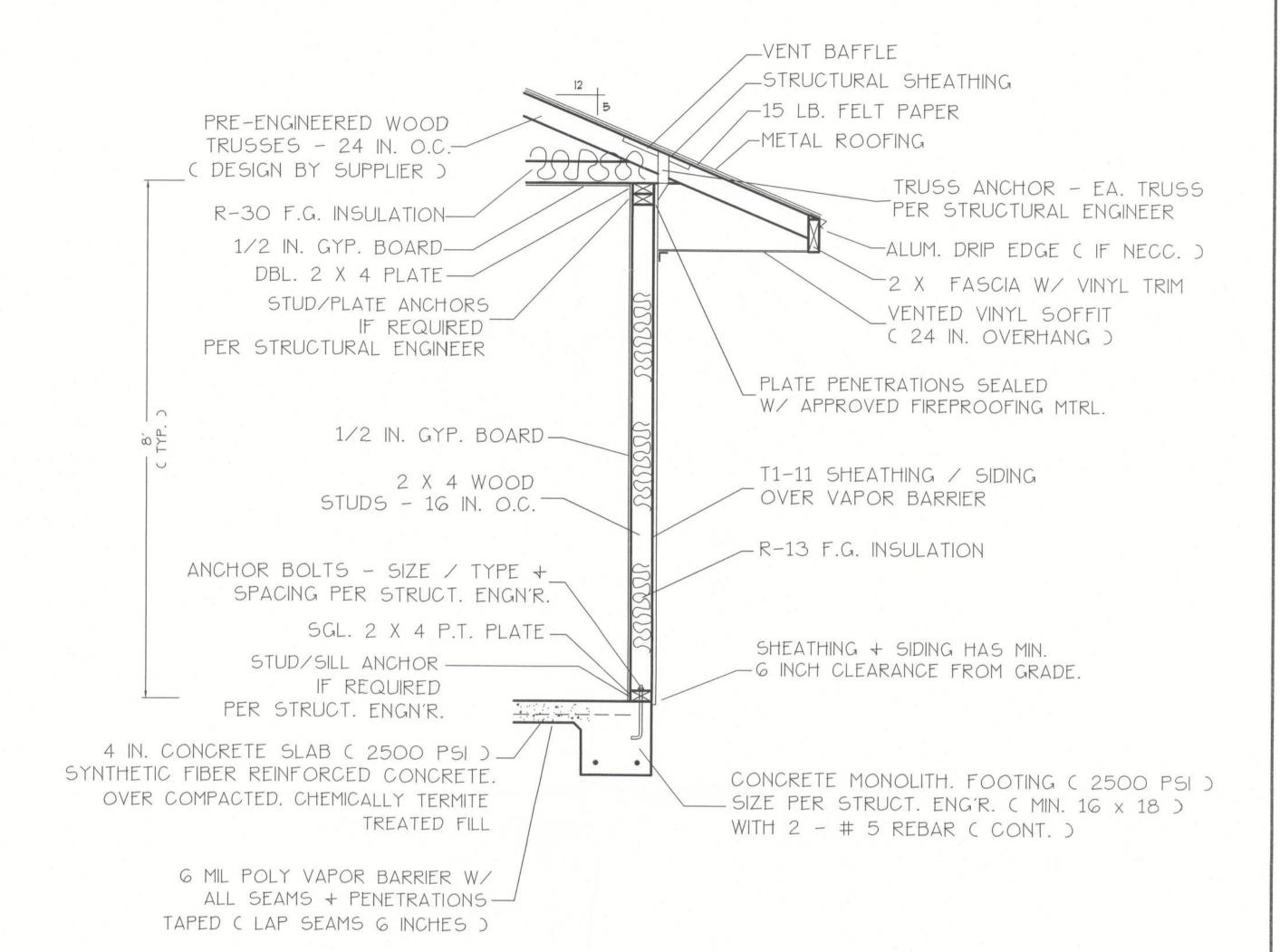
- -ALL INSTALLATIONS ARE PER NAT'L. ELECTRIC CODE (NEC) 2008.
- -ALL RECEPTACLES, UNLESS NOTED OTHERWISE, SHALL BE ARC FAULT CIRCUIT INTERRUPTER (AFCI) TYPE. ALSO, RECEPTACLES, UNLESS NOTED, SHALL BE TAMPER RESISTANT.
- -GROUNDING OF ELECTRICAL SYSTEM SHALL BE BY "UFER" STYLE GROUNDING METHOD TO REINFORCING ROD IN CONCRETE FOUNDATION FOOTING (NEC 250.52 - GROUNDING ELECTRODES).
- -WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- -ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- -ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)

TO BE DETERMINED BY POWER COMPANY.

- -TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION 2008.
- -CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- -LOW YOLTAGE ITEMS (TELEPHONE, CATY, DATA CABLING) IS SHOWN, IF REQUESTED BY OWNER / BUILDER. CONSULT OWNER FOR REQUIREMENTS IF NOT SHOWN ON ELECTRICAL PLAN.
- -ALL SMOKE DETECTORS SHALL BE 120Y W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS. THEY SHALL ALSO PROVIDE CARBON MONOXIDE DETECTION.







WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

DESIGN WALL SECTION

NON-STRUCTURAL DATA

SCALE: 3/4 IN. = 1 FT.

NOT TO SCALE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056,

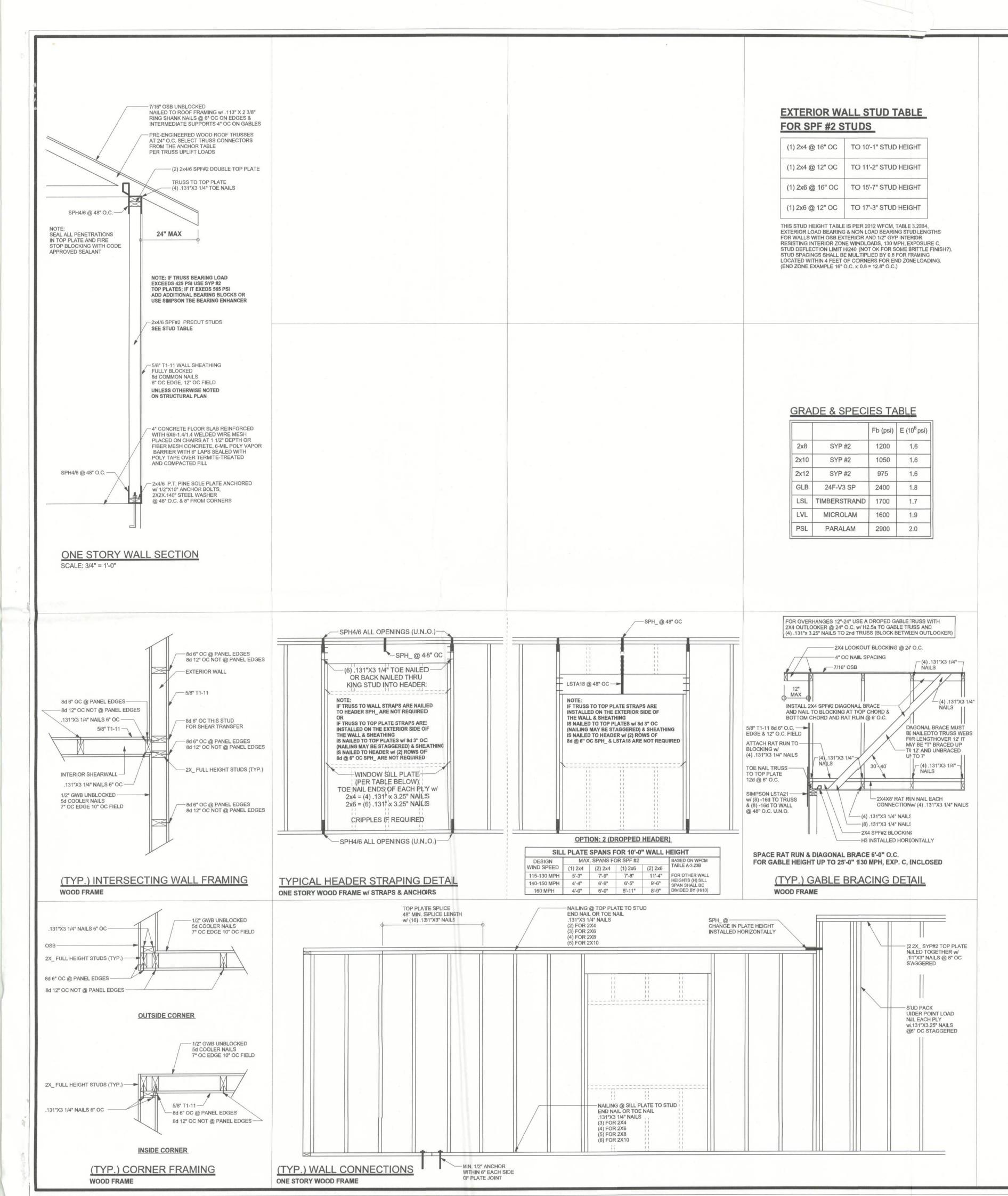
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2010, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

ocation:



FILE: 13-002 DATE: 2-2-13	MARKS RESIDENCE	SHEET: 2 OF 2 CAD FILE: 13-002
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Gln., Lake City, FL 32024 Phone (386) 755-5891	REV:



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 X W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25* FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	NER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE RT OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	TIONS, FOUNDATION BEARING CAPACITY, GRADE AND NO SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	AND CONSTRUCTION TECHNIQUES, WHICH COMPLY JIREMENTS FOR THE STATED WIND VELOCITY AND
	US LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU IITS A CONTINUOUS LOAD PATH CONNECTION, CALL NEER IMMEDIATELY.
DESIGN, PLACEMENT F	NUFACTURER'S SEALED ENGINEERING INCLUDES TRUS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS INECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements				
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi				
2.1	Mortar	ASTM C 270, Type N, UNO				
2.2	Grout	ASTM C 476, admixtures require approve				
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block				
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"				
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)				
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS				
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet me ties not completely embedded in mortar grout, ASTM A153, Class B2, 1.50 oz/ft/ or 304SS				
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.				
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.				

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24		Charles and Allerta	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*		1.00	TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CO					
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS MEAN ROOF HEIGHT	WITH FLA	T, HIPF	PED, OF	R GABI	E ROOF
BUILDING IS NOT IN THE HIGH VELOCITY HU	RRICANE Z	ONE			
BUILDING IS NOT IN THE WIND-BORNE DEBR	RIS REGION	1			
1.) BASIC WIND SPEED = 130 MPH, (3 SEC	GUST, 33 F	T, EXF	P. C)		
2.) WIND EXPOSURE = C, BUILDER MUST FI	IELD VERIF	Y			
3.) TOPOGRAPHIC FACTOR = 1.0, BUILDER	MUST FIE	LD VE	RIFY		
4.) RISK CATEGORY = II, (MRI = 700 YR)					
5.) ROOF ANGLE = 7-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT = N	/A (ENCLO	SED B	UILDING	3)	
8.) COMPONENTS AND CLADDING DESIGN	WIND PRES	SSURE	S (TAB	LE R30	01.2(2))
A B					
	Zone	Effec	tive W	ind A	rea (ft2)
2 2			10		
1	1	39	-43		
2 2 2 2 5	2	39	-68	-	-
3 4	3	39	-100	-	+
5 5	3	00	-100		1
The state of the s	4	43	-46		
12	5	43	-57		
\$ 2	Gara	ge Do	or		T
5 2 3	2010	2010 FBCR, Table R301.2.(4) 8x7 Garage Door			
2 4 /3/ 5					
4 1				37	-42
55	16x7 Garage Door		Door	36	-40
DESIGN LOADS					
FLOOR 40 PSF (ALL OTHER DWELLING ROO	OMS)				
30 PSF (SLEEPING ROOMS)					
30 PSF (ATTICS WITH STORAGE)					
10 PSF (ATTICS WITHOUT STORAG	E, <3:12)				
ROOF 20 PSF (FLAT OR <4:12)					
16 PSF (4:12 TO <12:12)					
12 PSF (12:12 AND GREATER)					
STAIRS 40 PSF (ONE & TWO FAMILY DWELLII	NGS)				
SOIL BEARING CAPACITY 1000PSF					
NOT IN FLOOD ZONE (BUILDER TO VERIFY)					

REVISIONS

VINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS:

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. OPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reser ts common law copyrights and property right i these instruments of service. This document is

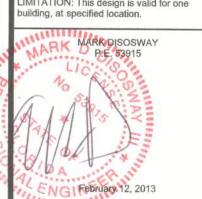
not to be reproduced, altered or copied in any form or manner without first the express writte permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineeri

to the best of my knowledge.

Building Code Residential

LIMITATION: This design is valid for one building, at specified location.

comply with section R301.2.1, 2010 Florida



Marks Resdience

ADDRESS: 2939 SW County Road 778 Ft. White, FL 32038

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: February 12, 2013 DRAWN BY: STRUCTURAL BY FINALS DATE: 12Feb13

> JOB NUMBER: 1302013

> > OF 2 SHEETS

DRAWING NUMBER

TOTAL SHEAR WALL SEGMENTS

REQUIRED ACTUAL

REQUIRED ACTUAL

TRANSVERSE 28.5' 38.0'

TRANSVERSE 28.5' 45.0'

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.)

NUMBER OF KING STUDS (FULL LENGTHI)

SPAN OF HEADER

SIZE OF HEADER

NUMBER OF PLIES IN HEADER

SN-4

FERMANEUT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B3, & BCSI-B3, BCSI-

SN-3

PMENSIONS ON STRUCTURAL SHEETS

ELOOR PLAN FOR ACTUAL DIMENSIONS

FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-2 SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SPF #2 U.N.O.

EXTERIOR WALL W/ UPLIFT

INTERIOR LOAD BEARING WALL W/ UPLIFT

INTERIOR LOAD BEARING WALL W/ UPLIFT

INTERIOR LOAD BEARING WALL W/ UPLIFT

INTERIOR WALL W/ UPLIFT

INTERIOR WALL W/ UPLIFT

INTERIOR WALL W/ UPLIFT

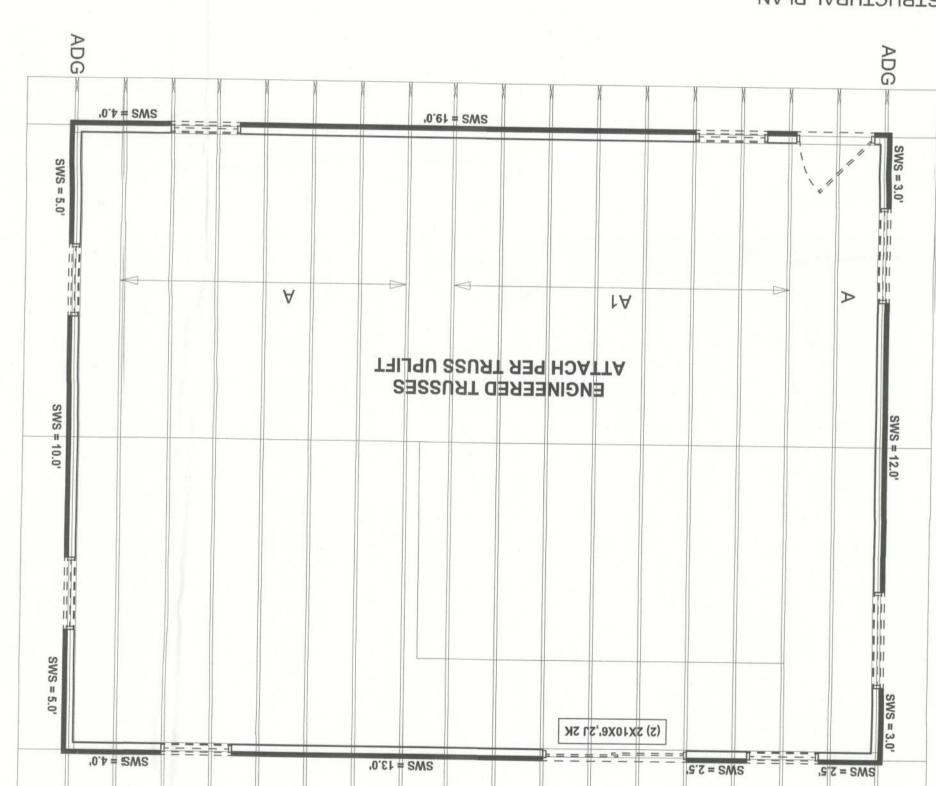
INTERIOR WALL W/ UPLIFT

WALL LEGEND

HEADER LEGEND

STRUCTURAL PLAN NOTES

SCALE: 1/4" = 1'-0"



S-2

12Feb13 1302013 1308 NUMBER:

 February 12, 2013

 DRAWN BY:
 STRUCTURAL BY:

 FINALS DATE:
 STRUCTURAL BY:

PRINTED DATE:

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

ADDRESS: 2939 SW County Road 778 Ft. White, FL 32038

Marks Resdience



LIMITATION: This design is valid for one building, at specified location.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineeri comply with section R301.2.1, 2010 Florida building Code Residential to the best of my knowledge.

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DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

32056, 386-754-5419 PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

RETTE PLOOR SLAB REINFORCED WITH
SRETE FLOOR SLAB REINFORCED WITH

 $1/2^{n} = 1.0^{n}$

OLITHIC FOOTING

—(S) #2 CONTINUOUS

COMPACTED FILL

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GETAERT STIMPST —

MIT VAPOR BARRIER

GEALED

GEA

4" CONCRETE SLAB

DEPTH ON CHAIRS OR FIBERMESH CONCRETE

— 6"X6" W1.4XW1.4 W.W.M. PLACED AT 2"

SOLITIECTURAL DESIGN SOFTWARE

KEVISIONS