



Project Information for: L281329

Address : 1109 SW BLUFF DR.
FORT WHITE, FL
County: COLUMBIA
Truss Count: 4
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

James H. Norton Florida Registered Building Contractor License No. RB0031780
Address: Norton Home Improvement Company, Inc. Route 28 Box 388A Lake City, Florida 32025

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1976478	T01	6/24/08
2	J1976479	T01G	6/24/08
3	J1976480	T02	6/24/08
4	J1976481	T02G	6/24/08



Job	Truss	Truss Type	Qty	Ply	NORTON HOMES - NEWSOME RES.
L281329	T01	COMMON	32	1	J1976478
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jun 24 11:53:47 2008 Page 1

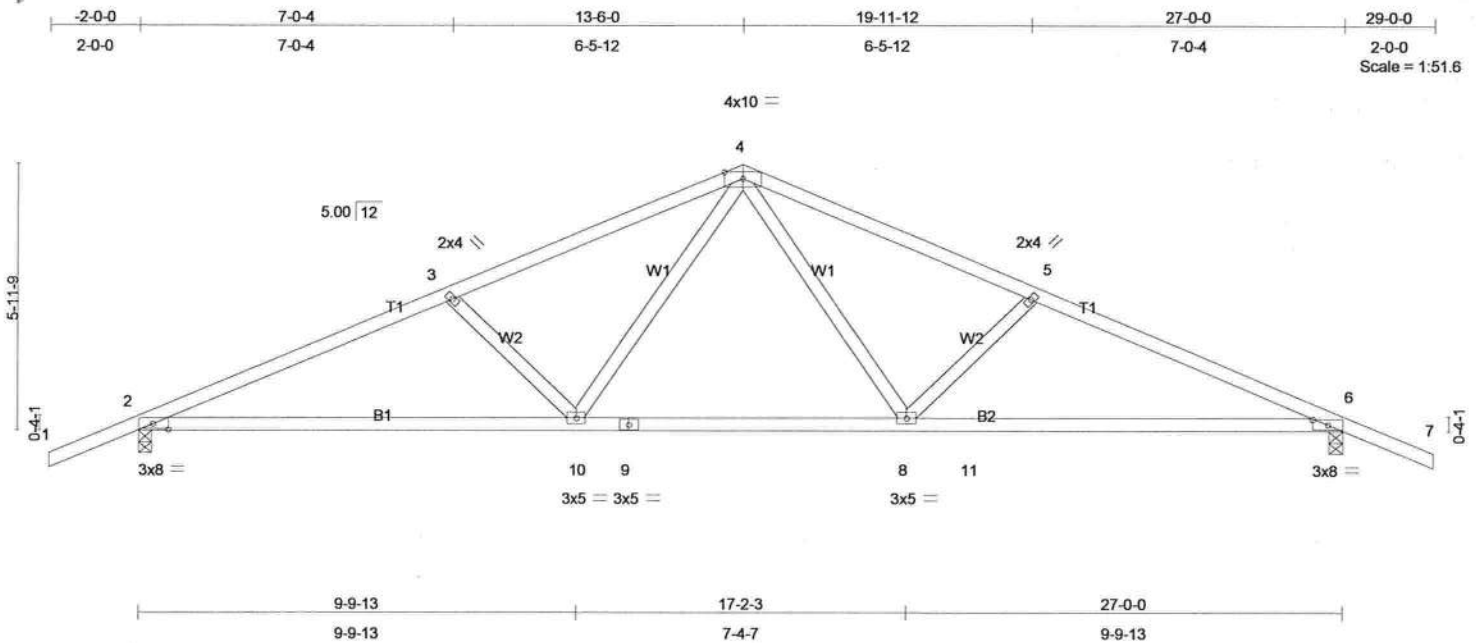


Plate Offsets (X,Y): [2:0-4-2,0-1-8], [6:0-4-2,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.48	Vert(LL)	0.36	8-10	>895	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.91	Vert(TL)	-0.51	8-10	>624	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.09	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 121 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-6 oc bracing.

REACTIONS (lb/size) 2=1220/0-3-8, 6=1247/0-3-8
Max Horz 2=96(load case 6)
Max Uplift 2=-353(load case 6), 6=-360(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2353/1310, 3-4=-2083/1216, 4-5=-2146/1252, 5-6=-2416/1346, 6-7=0/40
BOT CHORD 2-10=-1049/2096, 9-10=-636/1494, 8-9=-636/1494, 8-11=-1082/2154, 6-11=-1082/2154
WEBS 3-10=-355/307, 4-10=-358/691, 4-8=-419/797, 5-8=-356/307

JOINT STRESS INDEX

2 = 0.68, 3 = 0.33, 4 = 0.36, 5 = 0.33, 6 = 0.68, 8 = 0.59, 9 = 0.56 and 10 = 0.59

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida RES No. 34888
1409 Coastal Bay Blvd
Boynton Beach, FL 33435

June 24, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	NORTON HOMES - NEWSOME RES.	J1976479
L281329	T01G	GABLE	2	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jun 24 11:53:49 2008 Page 1

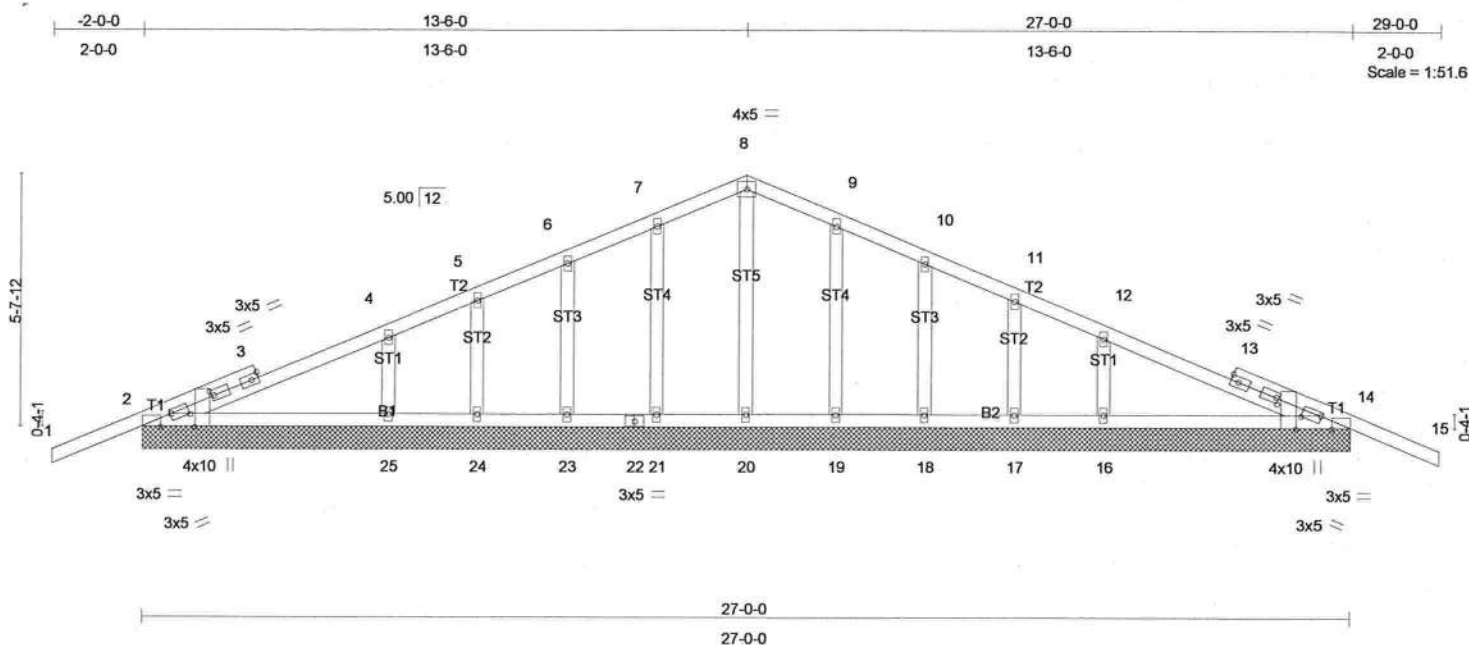


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-2-13,Edge], [2:0-4-12,Edge], [3:0-0-13,0-1-8], [13:0-0-13,0-1-8], [14:0-4-12,Edge], [14:0-3-8,Edge], [14:0-2-13,Edge]

LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	2-0-0	TC 0.49	Vert(LL)	-0.01	15	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.16	Vert(TL)	-0.01	15	n/r	90		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.17	Horz(TL)	0.01	14	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 140 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=518/27-0-0, 14=518/27-0-0, 20=352/27-0-0, 21=245/27-0-0, 23=284/27-0-0, 24=90/27-0-0, 25=589/27-0-0, 19=245/27-0-0, 18=284/27-0-0, 17=90/27-0-0, 16=589/27-0-0

Max Horz 2=106(load case 6)

Max Uplift 2=-308(load case 6), 14=-324(load case 7), 20=-68(load case 6), 21=-135(load case 6), 23=-155(load case 6), 24=-72(load case 6), 25=-290(load case 6), 19=-133(load case 7), 18=-156(load case 7), 17=-69(load case 7), 16=-295(load case 7)

Max Grav 2=526(load case 10), 14=526(load case 11), 20=352(load case 1), 21=249(load case 10), 23=284(load case 1), 24=90(load case 1), 25=591(load case 10), 19=249(load case 11), 18=284(load case 1), 17=90(load case 1), 16=591(load case 11)

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Continued on page 2

June 24,2008

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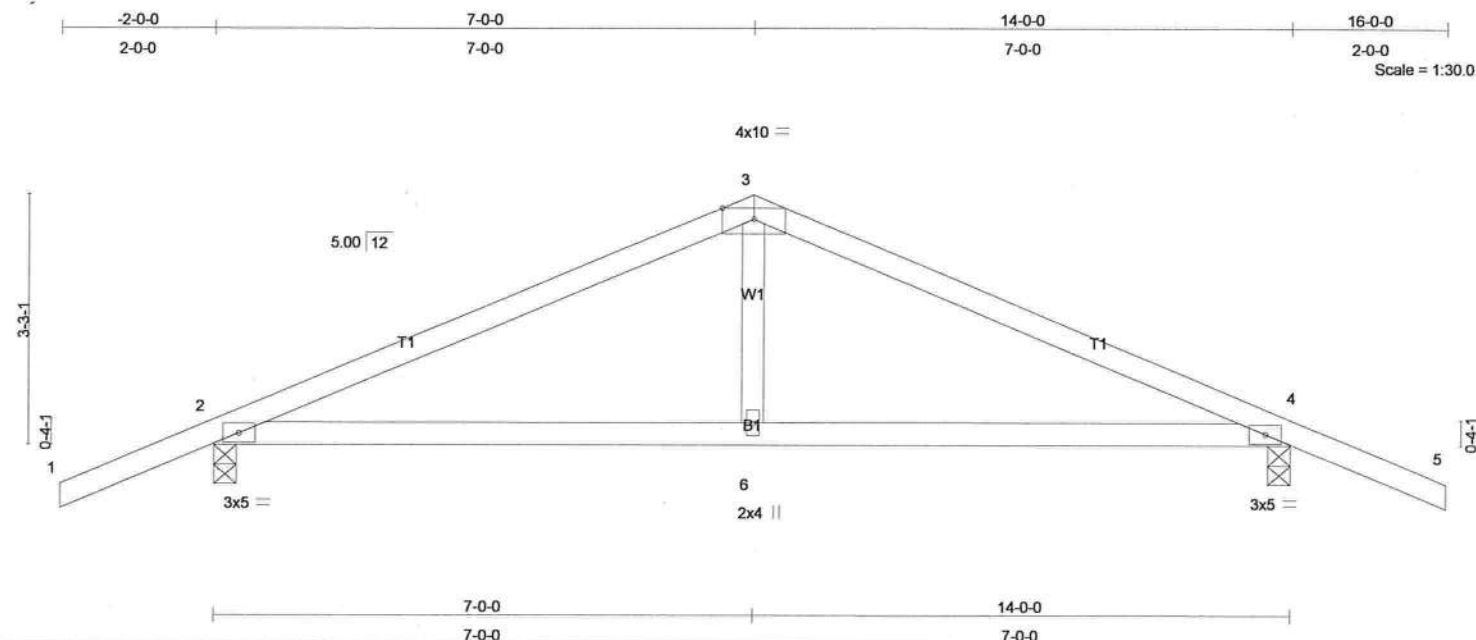
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Job	Truss	Truss Type	Qty	Ply	NORTON HOMES - NEWSOME RES.	J1976480
L281329	T02	COMMON	6	1	Job Reference (optional)	

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6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jun 24 11:53:50 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.40	Vert(LL)	0.15	2-6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.27	Vert(TL)	-0.09	2-6	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.08	Horz(TL)	-0.02	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 54 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-11-2 oc bracing.

REACTIONS (lb/size) 2=555/0-3-8, 4=555/0-3-8
Max Horz 2=-64(load case 7)
Max Uplift 2=-367(load case 6), 4=-367(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-687/987, 3-4=-687/987, 4-5=0/40
BOT CHORD 2-6=-760/568, 4-6=-760/568
WEBS 3-6=-417/235

JOINT STRESS INDEX

2 = 0.71, 3 = 0.79, 4 = 0.71 and 6 = 0.17

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=12ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Continued on page 2

June 24, 2008

Job	Truss	Truss Type	Qty	Ply	NORTON HOMES - NEWSOME RES.
L281329	T02G	GABLE	2	1	J1976481
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Jun 24 11:53:51 2008 Page 1

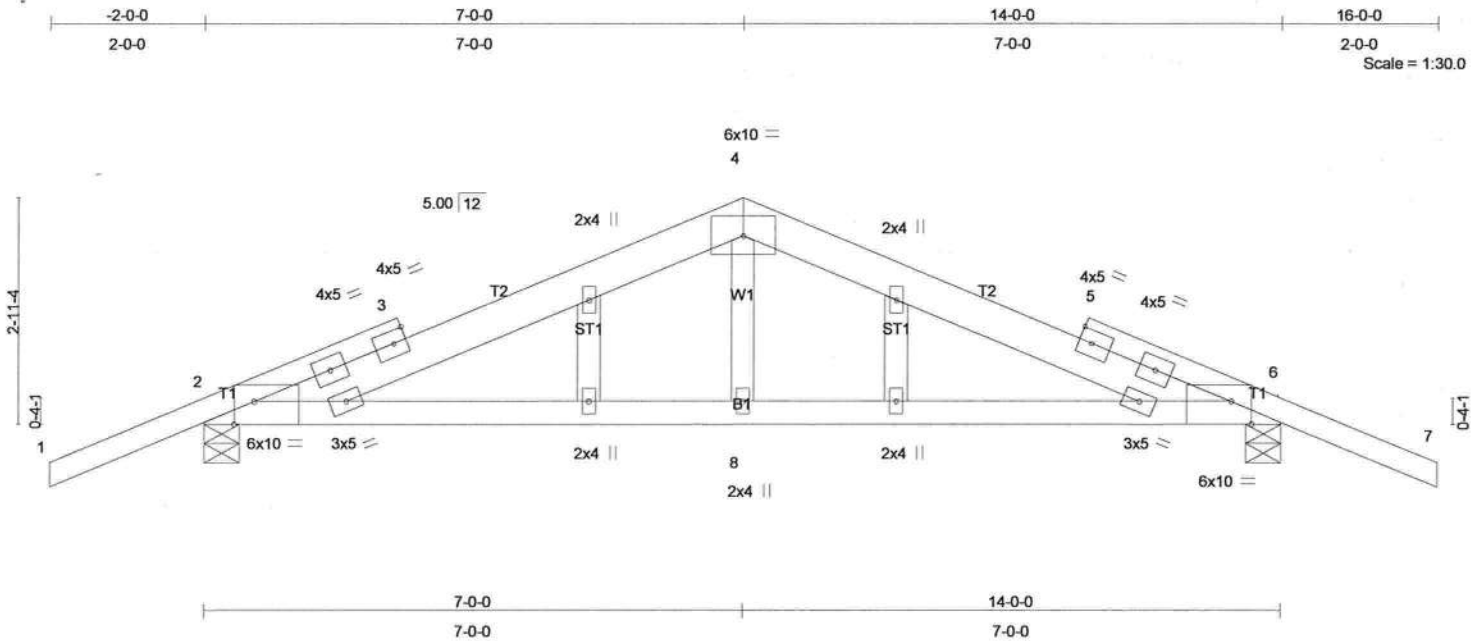


Plate Offsets (X,Y): [2:0-3-2,Edge], [6:0-3-2,Edge]

LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.72	Vert(LL)	0.18	2-8	>920	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.34	Vert(TL)	-0.10	2-8	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.07	Horz(TL)	-0.04	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 75 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except*
T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or
6'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 4'-6" oc
bracing.

REACTIONS (lb/size) 2=1094/0-5-8, 6=1094/0-5-8
Max Horz 2=67(load case 6)
Max Uplift 2=-970(load case 6), 6=-970(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-64/86, 2-3=-1417/2124, 3-4=-1337/2072, 4-5=-1337/2072, 5-6=-1417/2124,
6-7=-64/86
BOT CHORD 2-8=-1809/1234, 6-8=-1809/1234
WEBS 4-8=-390/216

JOINT STRESS INDEX

2 = 0.85, 2 = 0.00, 3 = 0.00, 3 = 0.83, 3 = 0.94, 4 = 0.85, 5 = 0.00, 5 = 0.94, 5 = 0.83, 6 = 0.85, 6 = 0.00, 8 = 0.15, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Lee
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Continued on page 2

June 24, 2008

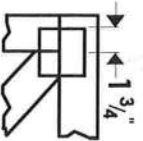
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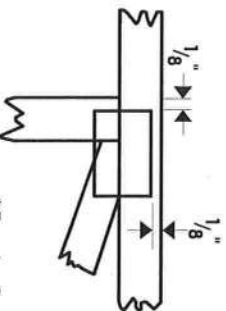


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

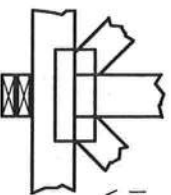
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



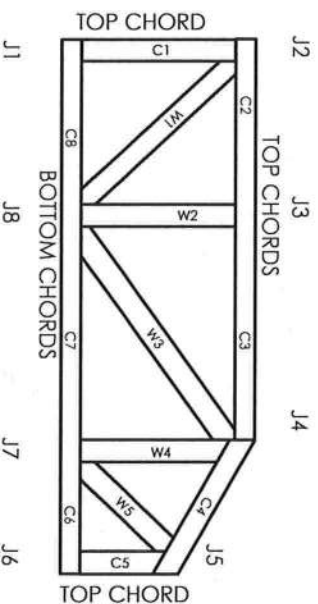
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

General Safety Notes

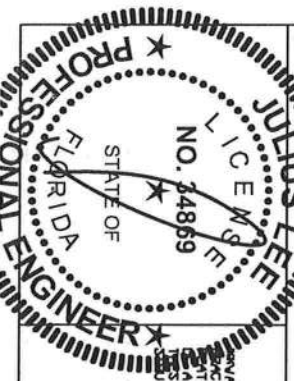
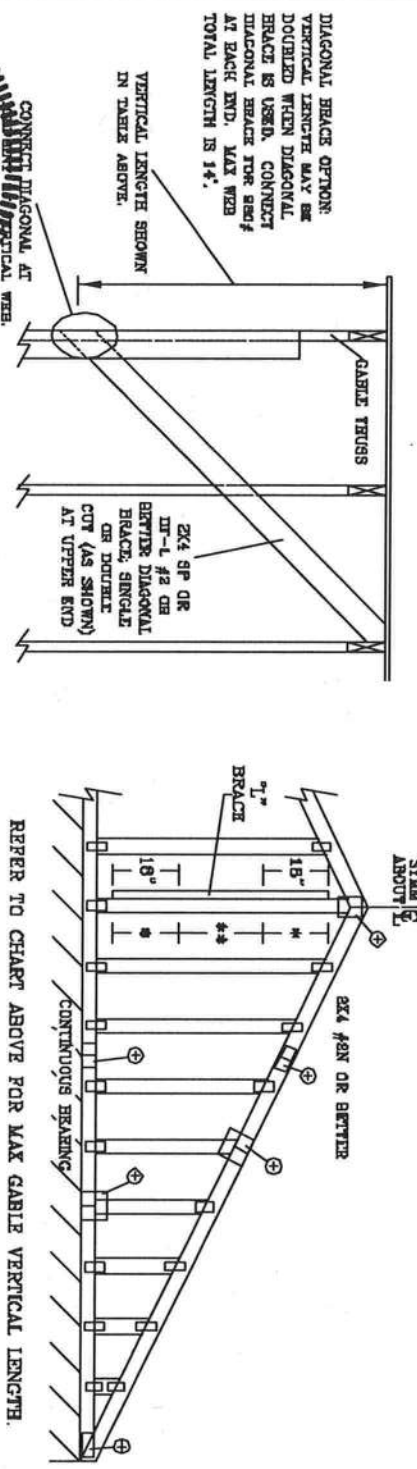
Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.



ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																		
CABLE SPACING	2X4 VERTICAL SPECIES	GRADE	BRACE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE *		(2) 2X8 "L" BRACE **			
					GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B				
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	7' 10"	10' 3"	10' 7"	12' 3"	12' 7"	12' 3"		
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	8' 1"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"			
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"			
	HF	STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	12' 3"		
		#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	12' 3"	12' 3"		
		#2	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	12' 3"	12' 3"		
	SP	#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"		
		STUD	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"		
		STANDARD	3' 0"	3' 10"	3' 10"	6' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"		
	DFL	#1 / #2	3' 8"	6' 4"	6' 4"	7' 6"	7' 8"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	9' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	9' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
16" O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 4"	7' 6"	7' 8"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	9' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	9' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	HF	STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 8"	12' 8"	14' 0"	14' 0"	14' 0"		
		#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	SP	#3	3' 8"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		STUD	3' 8"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		STANDARD	3' 8"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 3"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
12" O.C.	SPF	STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		STANDARD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#1	4' 0"	6' 11"	7' 2"	8' 3"	8' 3"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	SP	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	4' 2"	6' 6"	6' 5"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
DFL	STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"			
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	8' 8"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"			



REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th Avenue
DELRAY BEACH, FL 33444-4661

REF ASCE 7-02-GAB13030
DATE 11/26/03
DWG NOTE STD GABLE 20 x 17
-ENG

CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS FOR 160 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).	
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 8' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH T" BRACE WITH 10d NAILS.	
* FOR (1) T" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) T" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
T" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	
CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO STIFFENING
LESS THAN 4' 0"	1x4 OR 2x4
GREATER THAN 4' 0" BUT LESS THAN 11' 8"	2x4
GREATER THAN 11' 8"	2.5x4
+ REFER TO COMMON TRUSS DESIGN FOR PLANK, SPLICE, AND BEEL PLATES.	

TOP CHORD 2X4 #8 OR BETTER
BOT CHORD 2X4 #12 OR BETTER
WEBS 2X4 #8 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPICES MUST BE STAGGERED SO THAT ONE SPICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED, ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

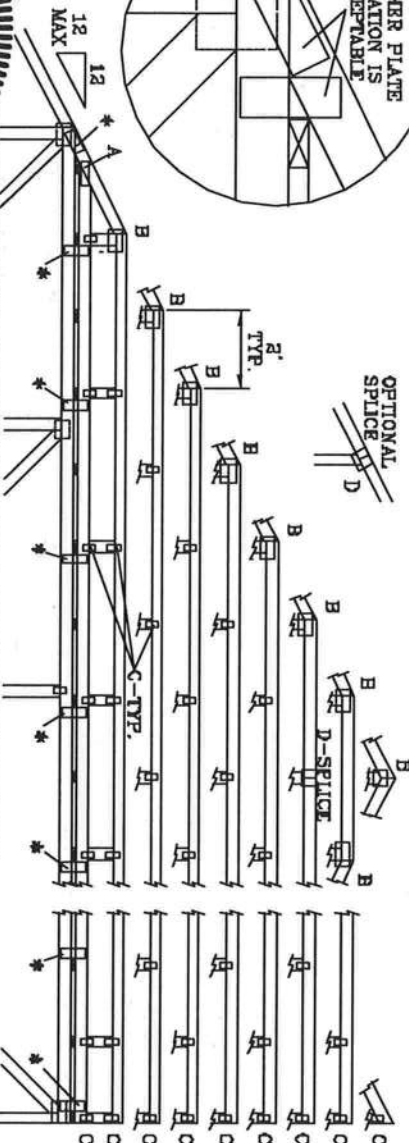
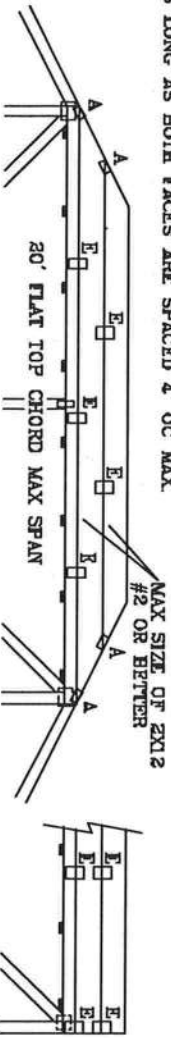
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, ENG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

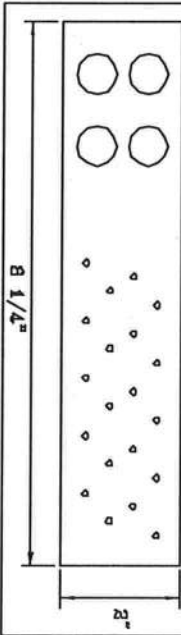


JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X8	6X8	6X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X6	6X6	5X8
E	4X8 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 180 TL FOR TRUSS INFORMATION.

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.

* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

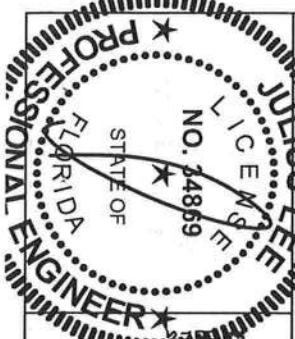


THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

JULIUS LEE'S
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1400 SW 4th AVENUE
ODDWAY BEACH, FL 33444-2161

MAX LOADING
65 PSF AT
1.33 DUR. FAC.
50 PSF AT
1.25 DUR. FAC.
47 PSF AT
1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG/ITER STD PIGGY
-ENG JL



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

No. 34869
STATE OF FLORIDA

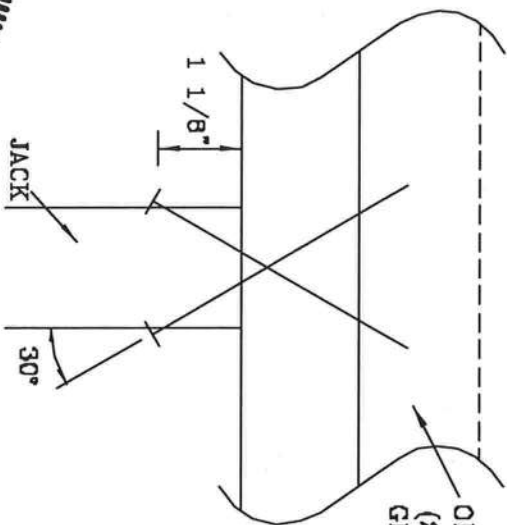
TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI A17.1-2001 SECTION 12.4.1 – EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

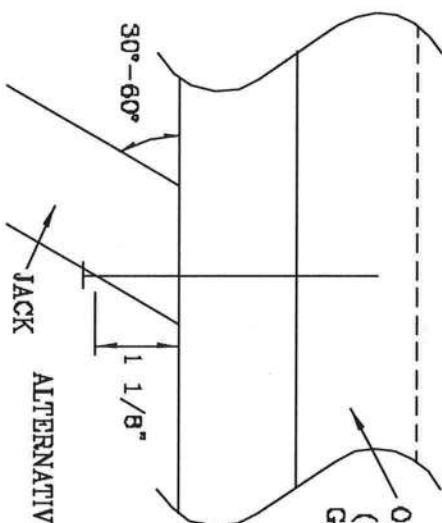
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES
2	187 #	256 #	181 #	234 #	156 #	203 #	154 #	189 #
3	286 #	383 #	271 #	351 #	234 #	304 #	230 #	288 #
4	394 #	511 #	351 #	468 #	312 #	406 #	307 #	397 #
5	493 #	639 #	452 #	585 #	390 #	507 #	384 #	498 #

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



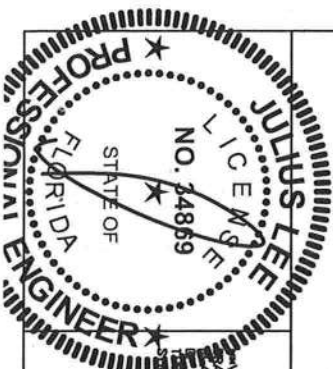
OPTIONAL
(2) PLY
GIRDER



OPTIONAL
(2) PLY
GIRDER

ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040



WARDING. TISSES REQUIRE EXHAUSTIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND REPAIRING. REFERS TO BEST 1-03 CONTINUING COMPETENCY SAFETY INFORMATION, PUBLISHED BY THE CRUISE SHIP INSTITUTE, 7513 JENNIFER DR., SUITE 200, MARLBOROUGH, MA 01759 AND LYCA (A/C) TRUSS COALD, 14000 MARLBOROUGH, 6800 ENTERPRISE LN, MARLBOROUGH, VT 05759 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED MULTIPLE PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TOP CHORD CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1435 ST 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34069
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

TRULOX CONNECTION DETAIL

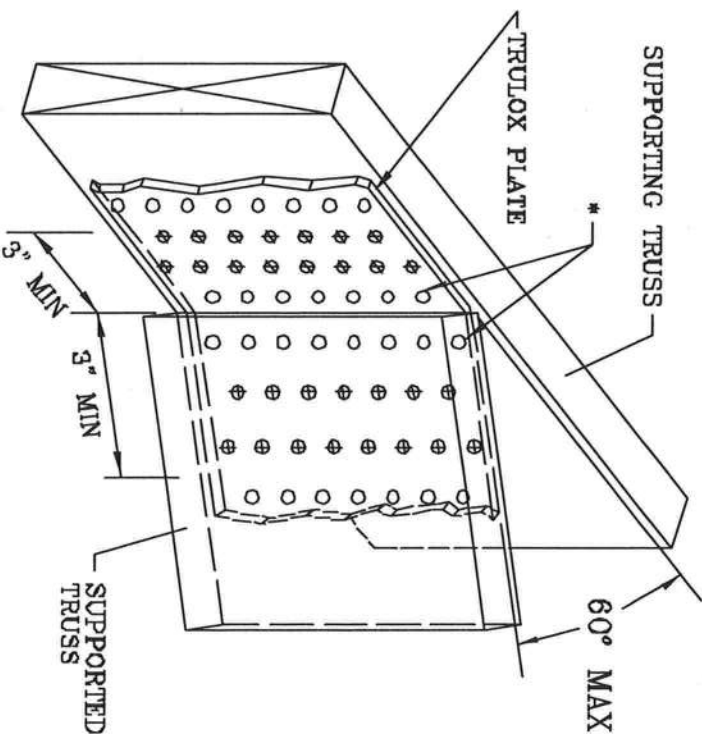
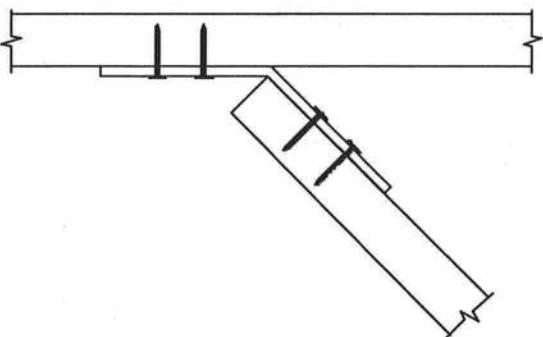
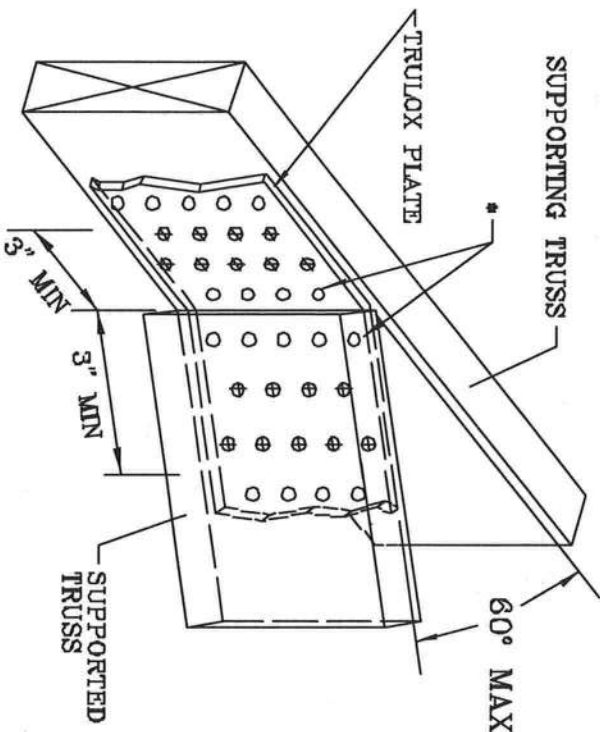
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

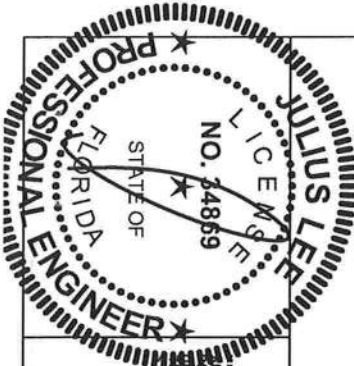


MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350 #
5X6	15	990 #

MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,168,989 1,158,989/R 1,164,844 1,162,217 1,162,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND MAINTAINING. REFER TO DETAIL 1-103, BUILDING DEPARTMENT, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 1000 W. 10TH AVENUE, SUITE 100, DENVER, CO 80202 FOR SAFETY PRACTICES PERTAINING TO THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2001

No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

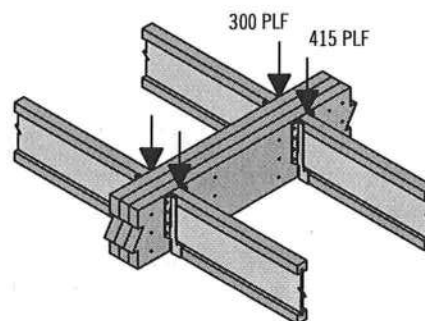
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.