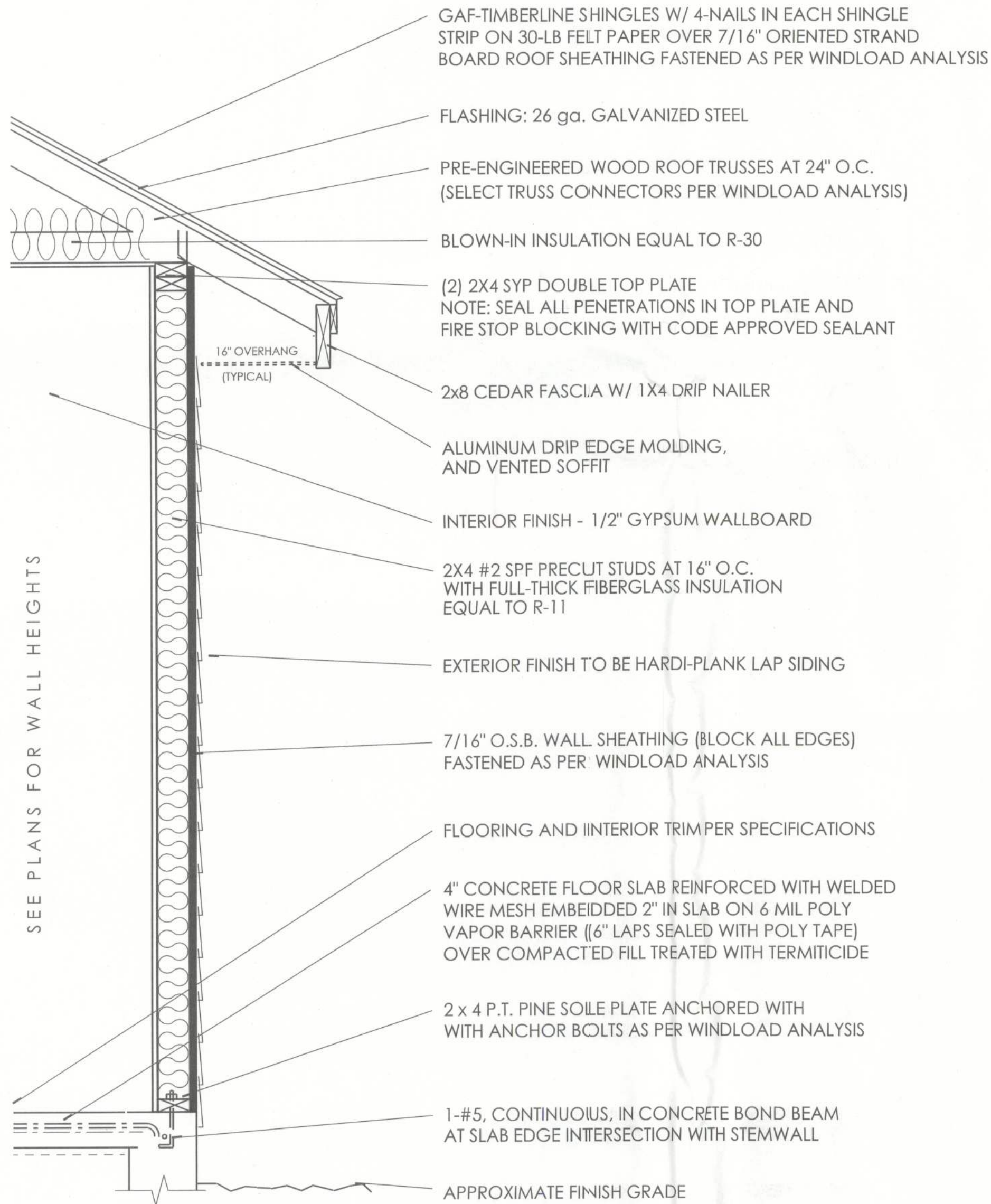


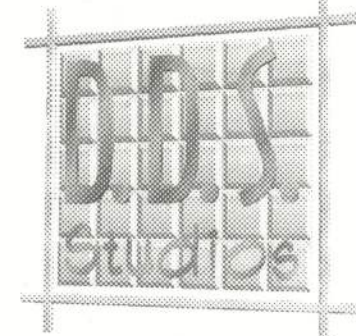
ALL DRAWINGS NOT TO BE SCALED. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

Design Criteria		
FL Building Code	Residential	2010
FL Electrical	Nat Elec Code	2008
FL Plumbing	FL	2010
FL Mechanical	FL	2010
Wind load Design	ASCE 7-10 - 6.12.2.1	2011
Wind load Design	FBC 1606.1.7	FBC 2010
Roof Live Load	20 PSF	
Floor Live Load	40 PSF	
Floor Live Load BRM	30 PSF	

Product	FL Code#	Version
Windows	FL12250-R10	2010
Exterior Doors	FL15220	2010
Garage Door	FL14092-R5	2010
Soffit	FL12198-R2	2010
Tamko Shingles	NOA 11 0720.03	2011
Hardie Siding	FL13192-R2	2010



TYPICAL WALL SECTION
SCALE: 3/4" = 1'0"



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EXTERIOR ELEVATIONS

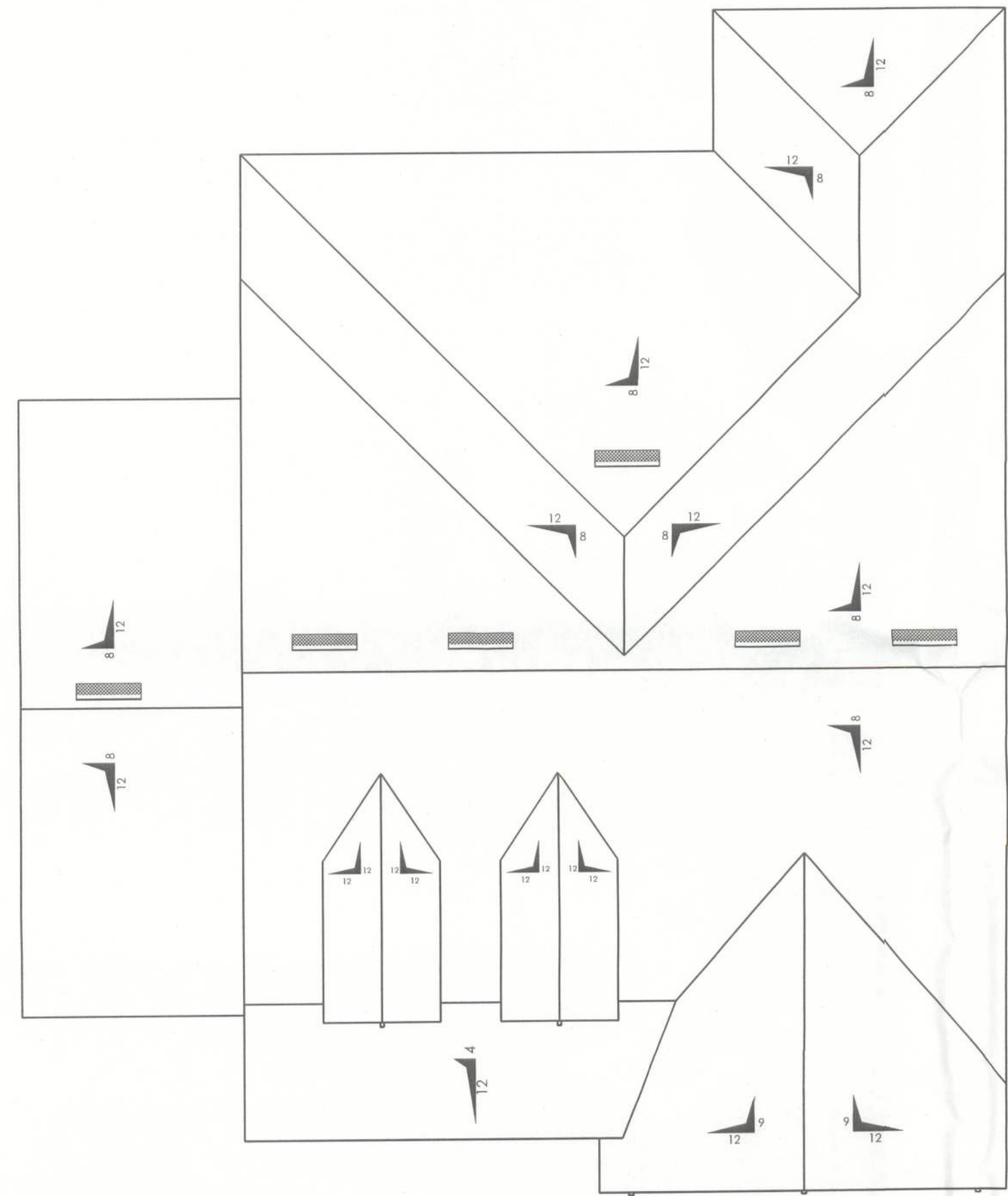
SHEET NUMBER

1 of 4

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

PROJECT NUMBER

010708



ROOF PLAN
SCALE: 1/8" = 1'

ROOF PLAN NOTES

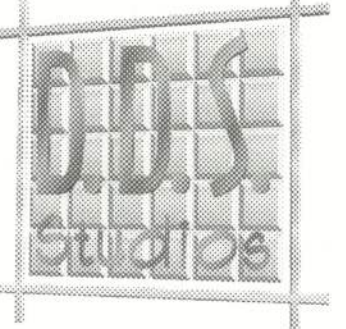
- R-1 ALL OVERHANG 20"
- R-2 PROVIDE ATTIC VENTILATION IN ACCORDANCE WITH CODE REQUIREMENTS
- R-3 SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY PLATE AND HEEL HEIGHTS
- R-4 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR



LEFT ELEVATION
SCALE: 1/4" = 1'



RIGHT ELEVATION
SCALE: 1/4" = 1'



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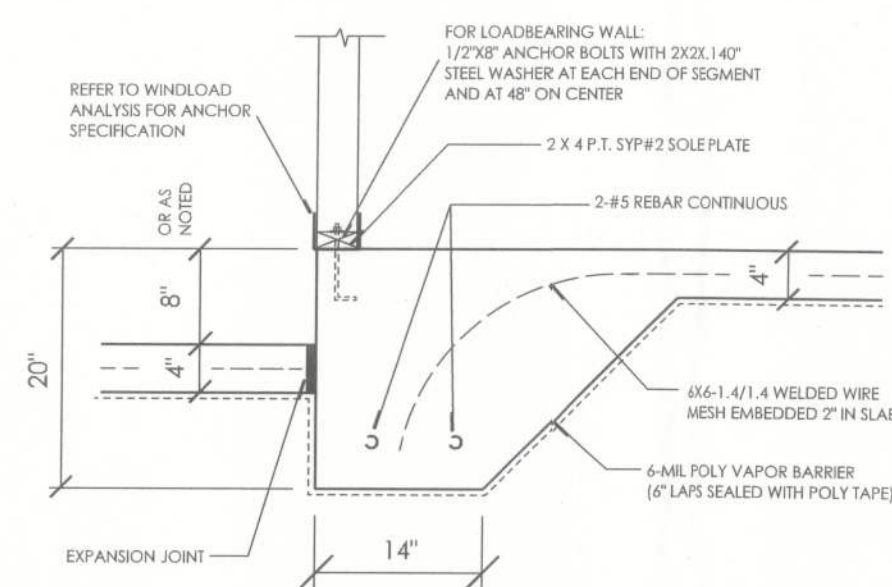
Daniel Shaheen
Daniel Shaheen

EXTERIOR ELEVATIONS
ROOF PLAN

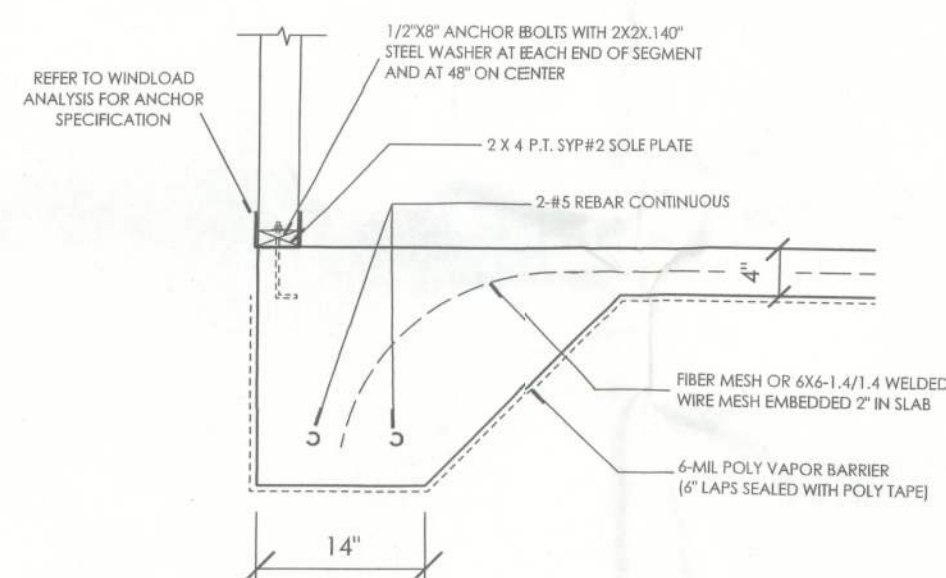
SHEET NUMBER
2 of 4

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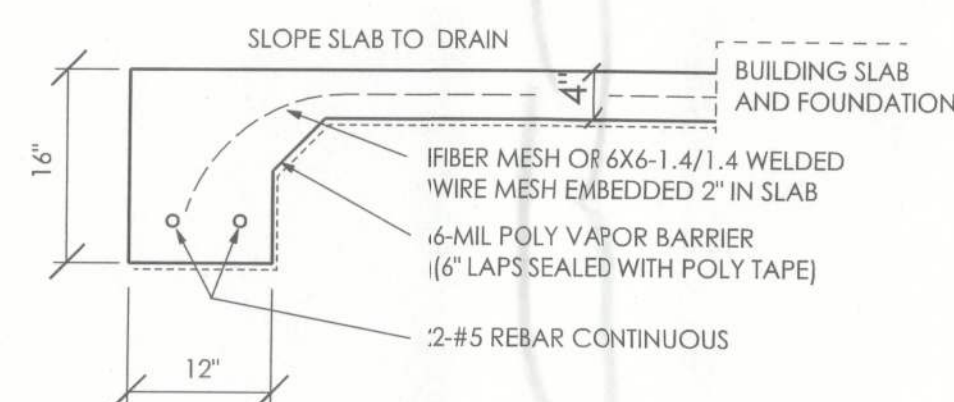
PROJECT NUMBER
010708



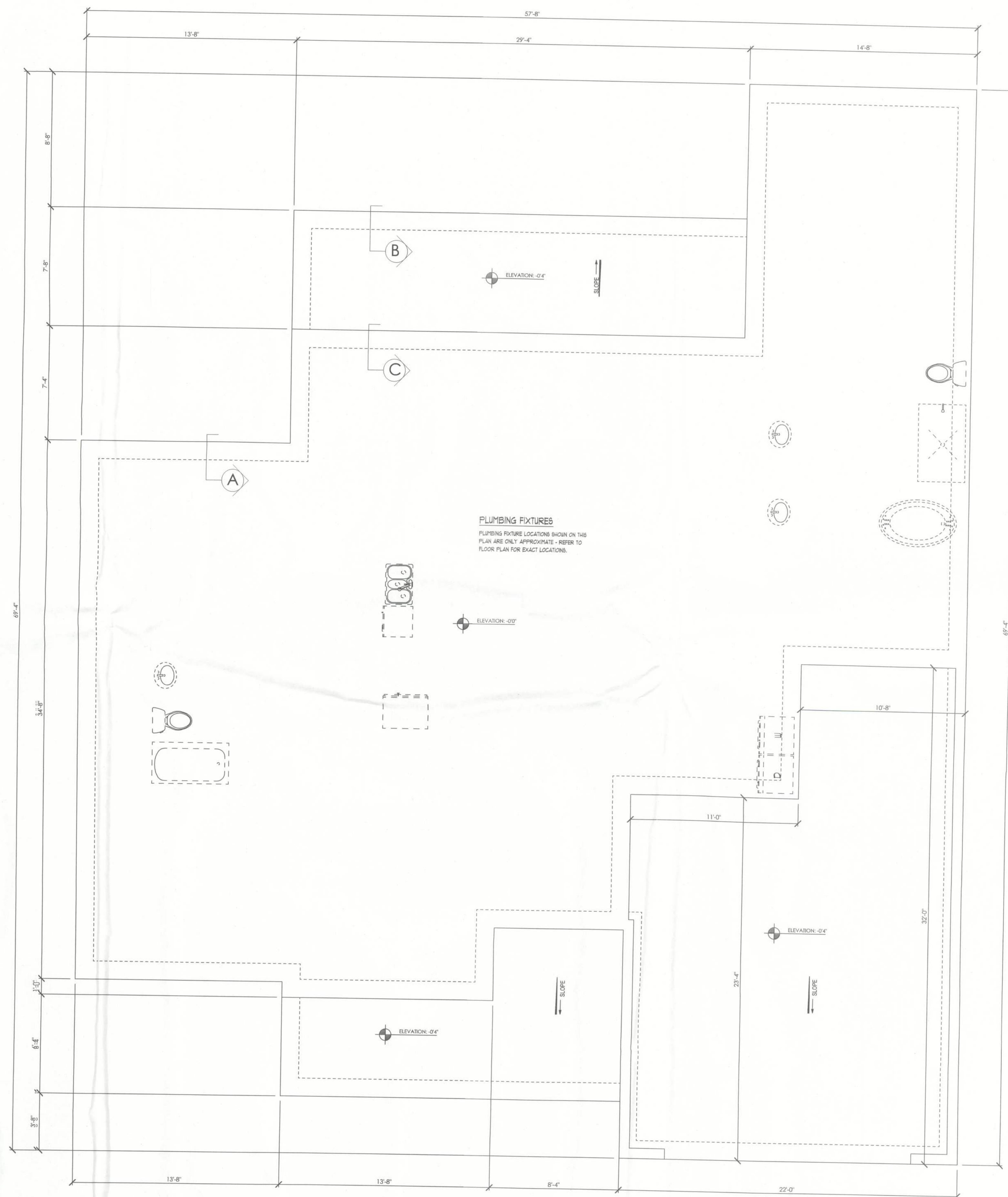
TYPICAL STEPPED SLAB / SECTION C
SCALE: 3/4" = 1'



TYPICAL MONOLITHIC SLAB / SECTION A
SCALE: 3/4" = 1'



TYPICAL PORCH SLAB / SECTION B
SCALE: 3/4" = 1'



FOUNDATION PLAN



ALL DRAWINGS NOT TO BE SCALED. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

ELECTRICAL PLAN NOTES

INSTALLATION SHALL BE PER 2008 NATL. ELECTRIC CODE.

ELECTRICAL SERVICE PANEL SHALL BE GROUNDED TO FOUNDATION STEEL WITH #4 BARE COPPER WIRE.

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUFACTURER'S SPECIFICATIONS.

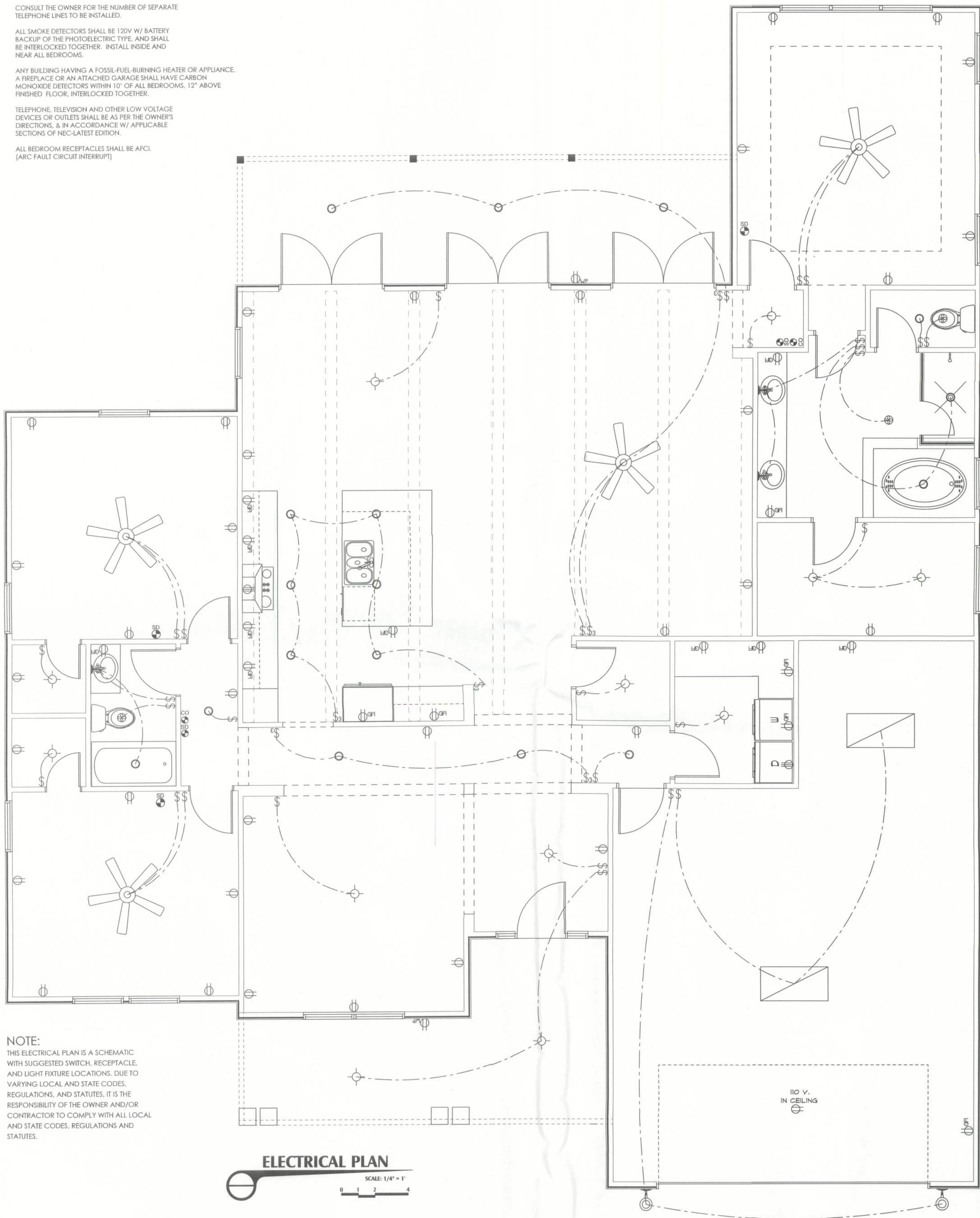
CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

ANY BUILDING HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE OR AN ATTACHED GARAGE SHALL HAVE CARBON MONOXIDE DETECTORS WITHIN 10' OF ALL BEDROOMS, 12" ABOVE FINISHED FLOOR, INTERLOCKED TOGETHER.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

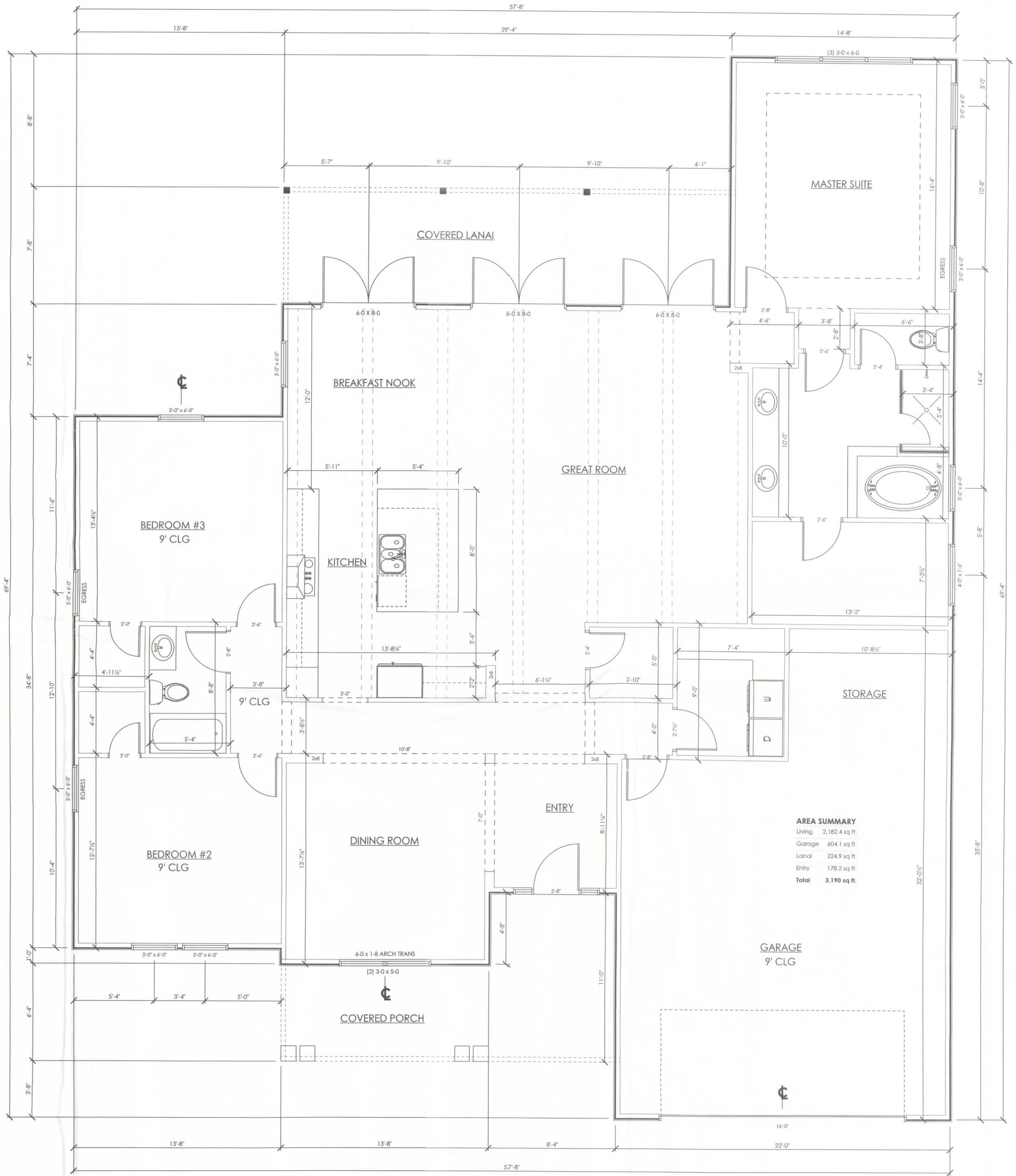
ALL BEDROOM RECEPTACLES SHALL BE AFCI. (ARC FAULT CIRCUIT INTERRUPT)



ELECTRICAL PLAN
SCALE: 1/4" = 1'

ELECTRICAL SERVICE PROVIDED BY
CLAY ELECTRIC COOP

NOTE:
THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGGESTED SWITCH, RECEPTACLE, AND LIGHT FIXTURE LOCATIONS. DUE TO VARYING LOCAL AND STATE CODES, REGULATIONS, AND STATUTES, IT IS THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND STATE CODES, REGULATIONS AND STATUTES.

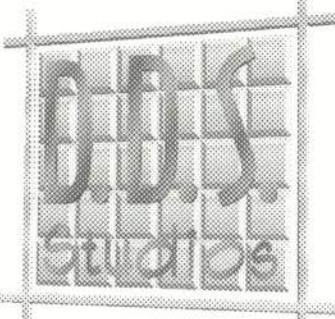


FLOOR PLAN
SCALE: 1/4" = 1'

ALL CEILINGS 10' UNLES OTHERWISE NOTED

AREA SUMMARY

Living	2,182.4 sq. ft.
Garage	604.1 sq. ft.
Lanai	224.9 sq. ft.
Entry	178.2 sq. ft.
Total	3,190 sq. ft.



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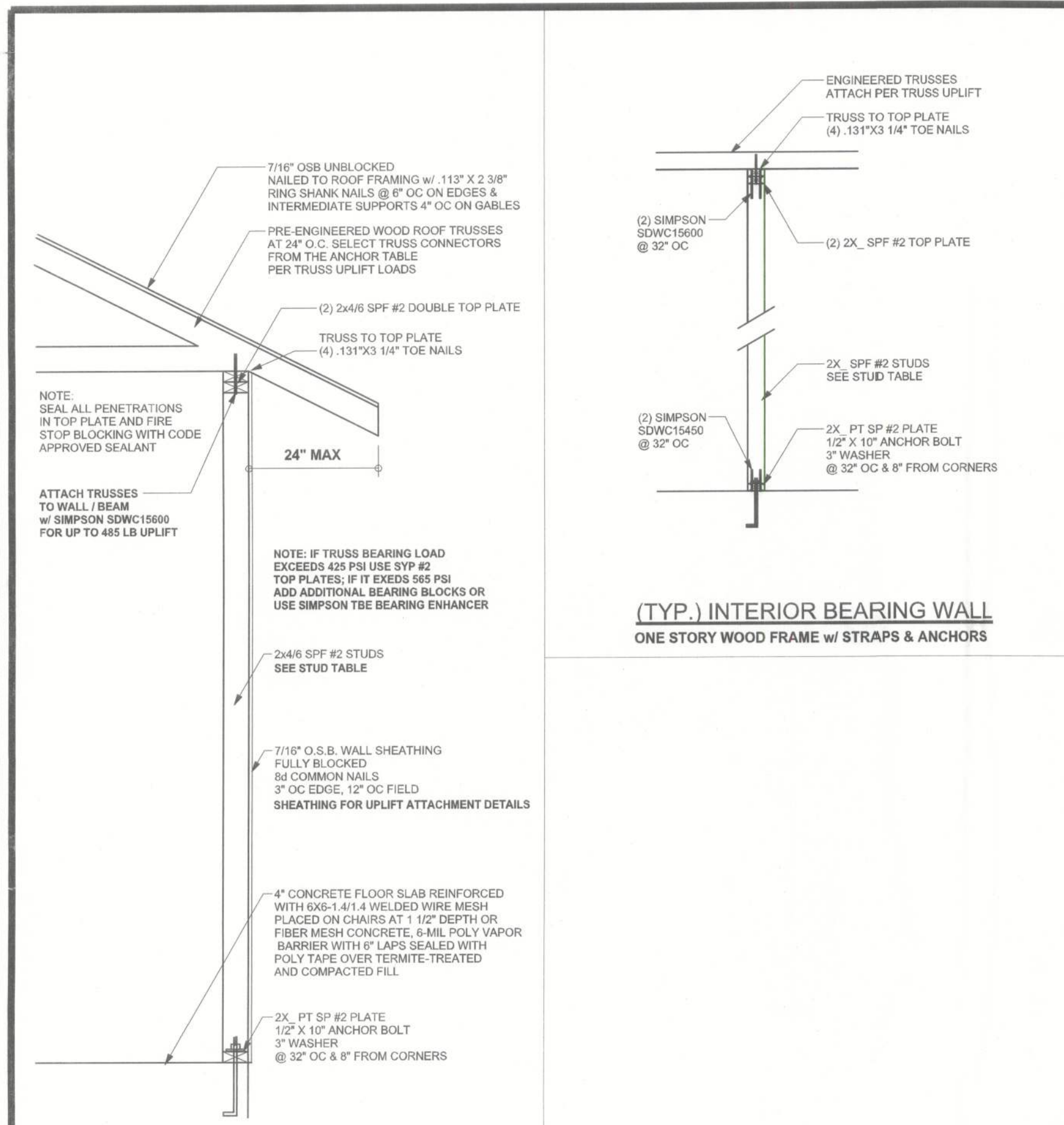
Dan Shaheen
Daniel Shaheen

FLOOR PLAN
ELECTRICAL PLAN

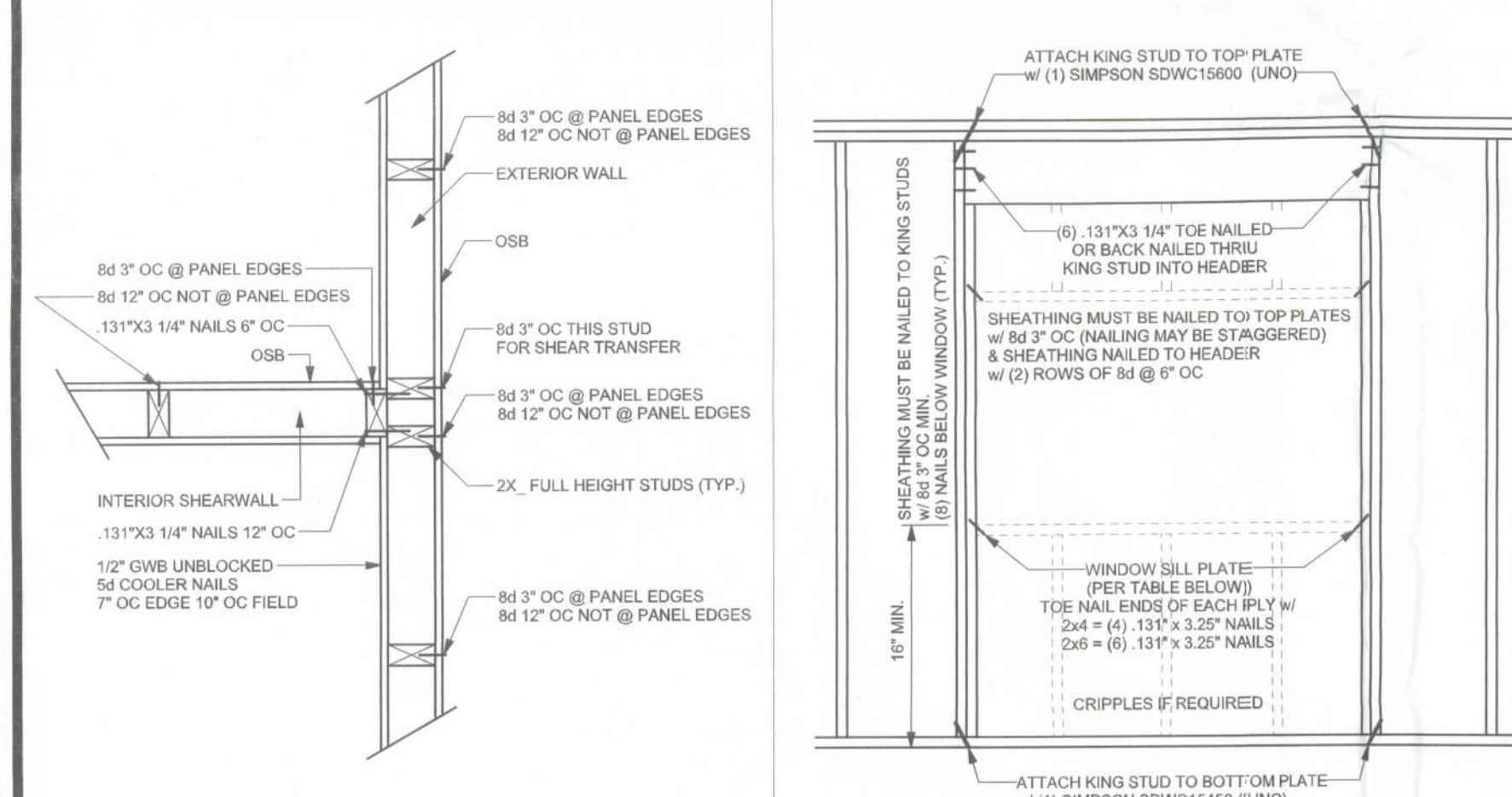
SHEET NUMBER
4 of 4

All work shall comply with the standard building code, and all applicable local codes and ordinances.
Contractor shall verify all dimensions prior to commencing construction.

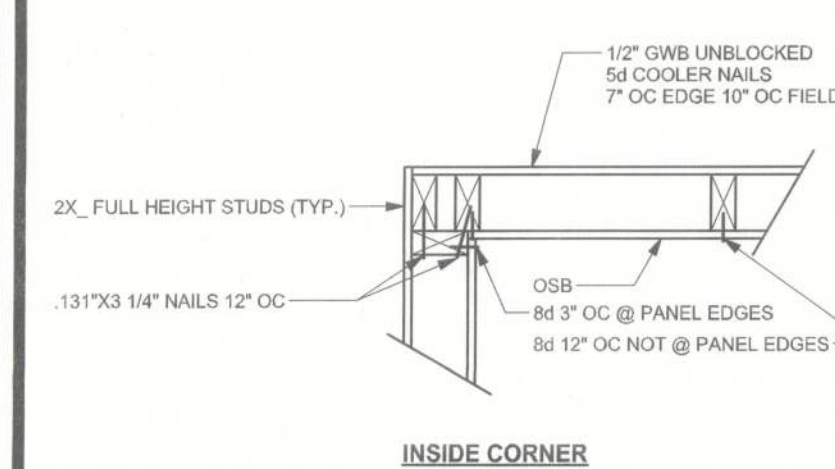
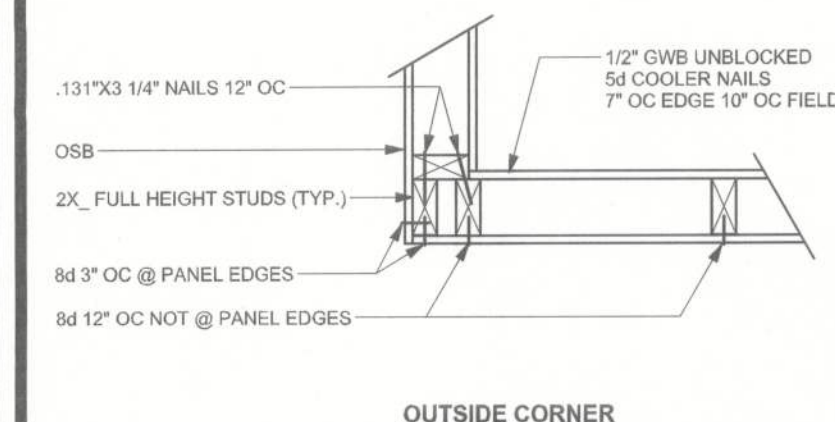
PROJECT NUMBER
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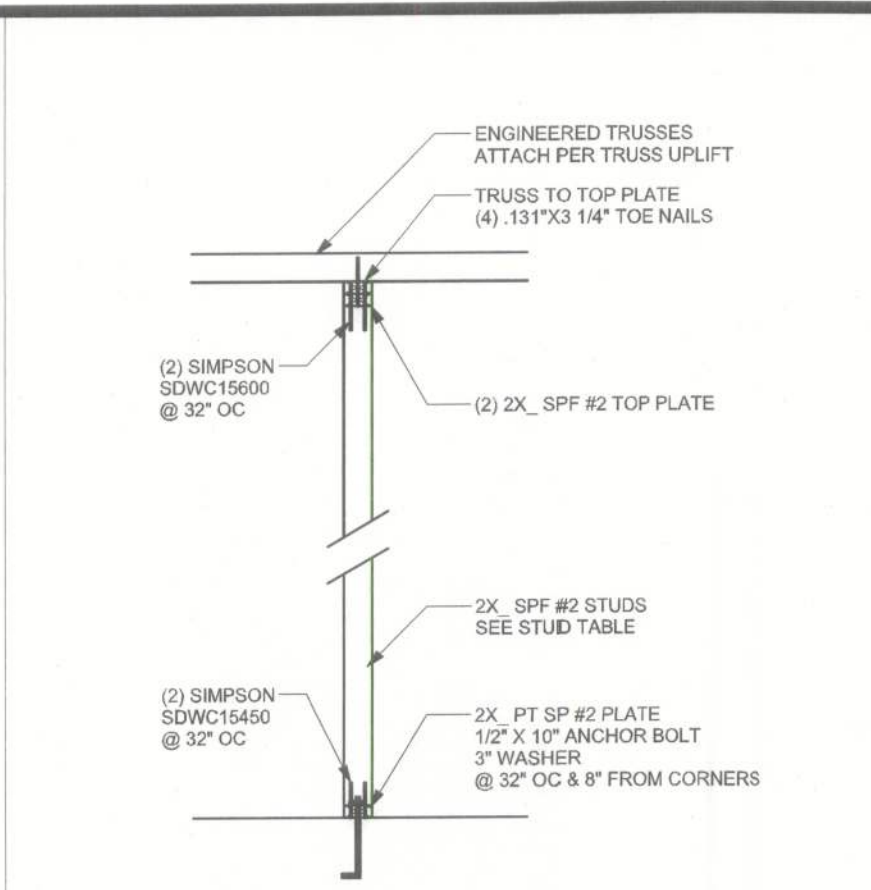
ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



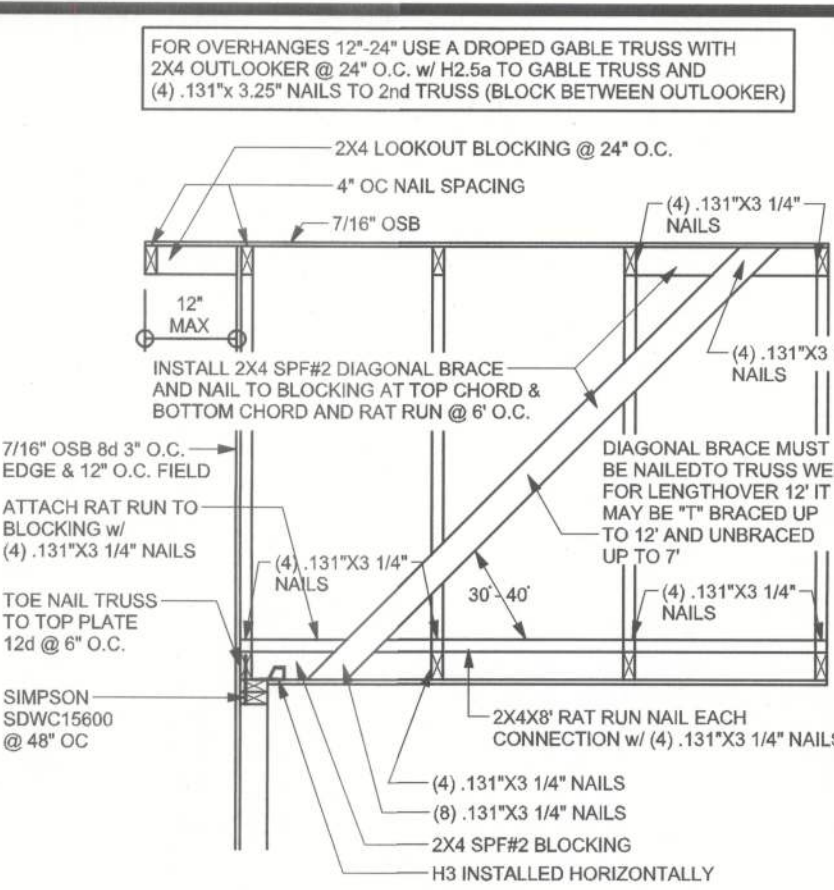
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



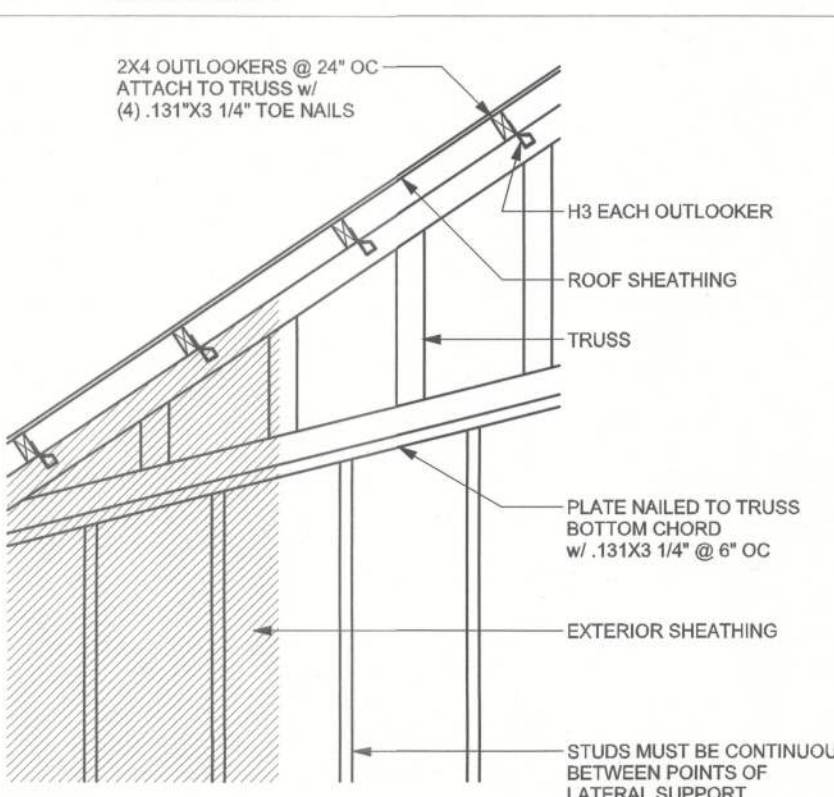
(TYP.) CORNER FRAMING
WOOD FRAME



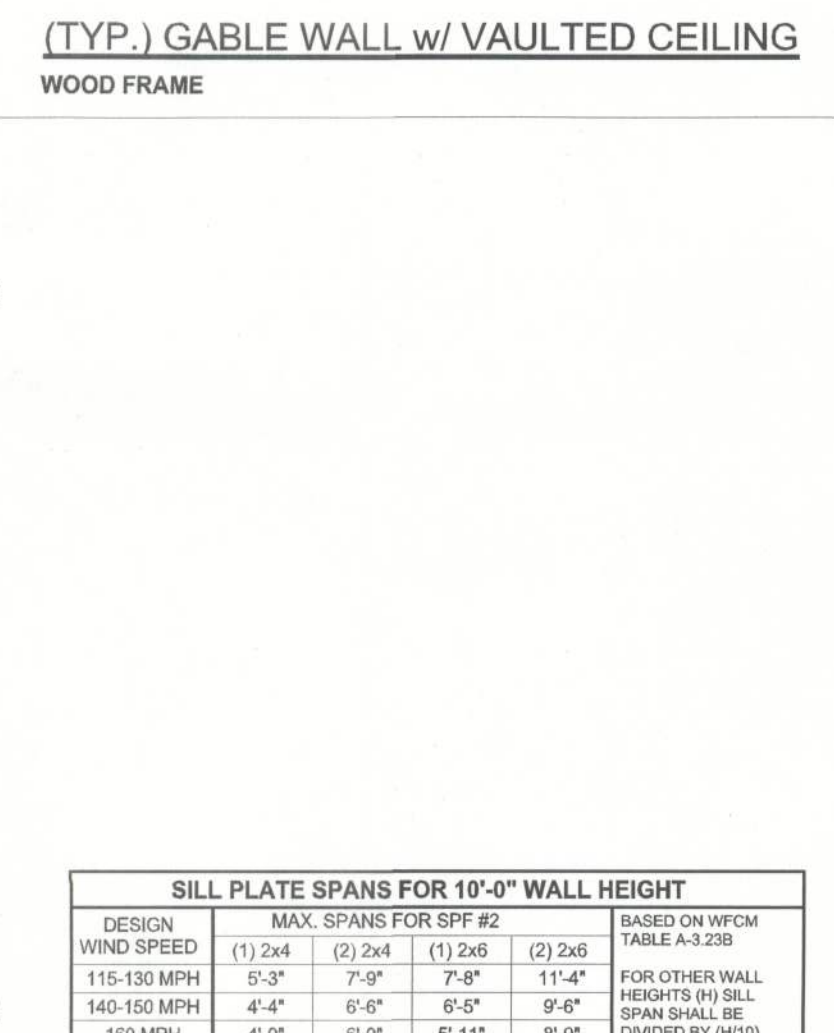
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



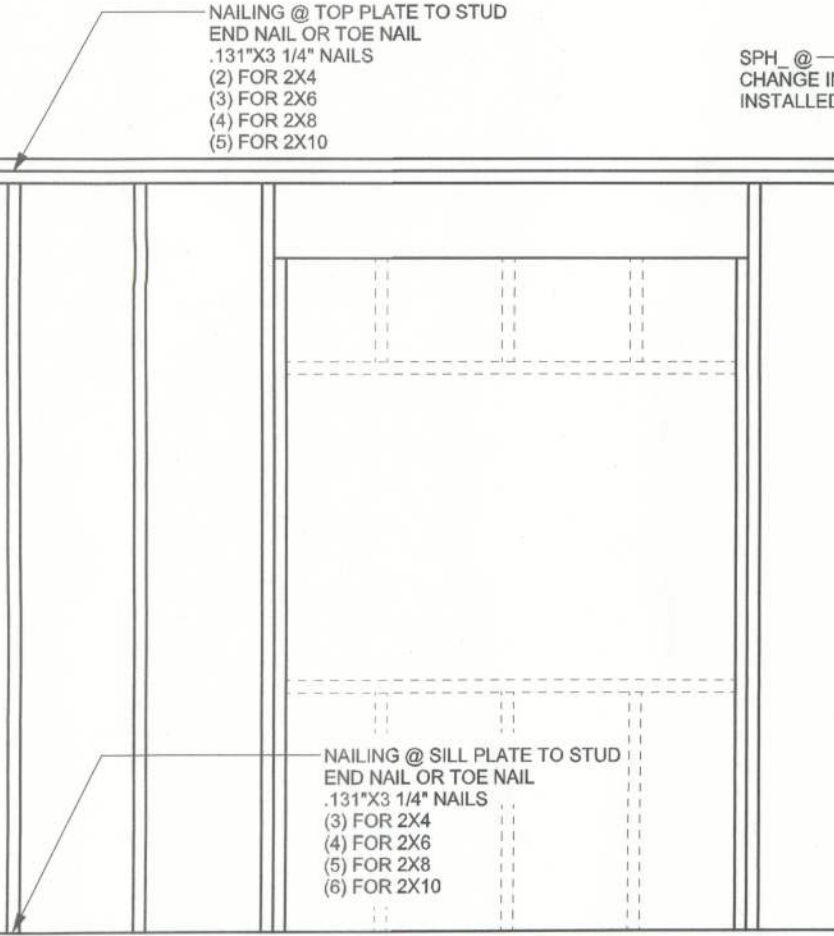
(TYP.) GABLE BRACING DETAIL
WOOD FRAME



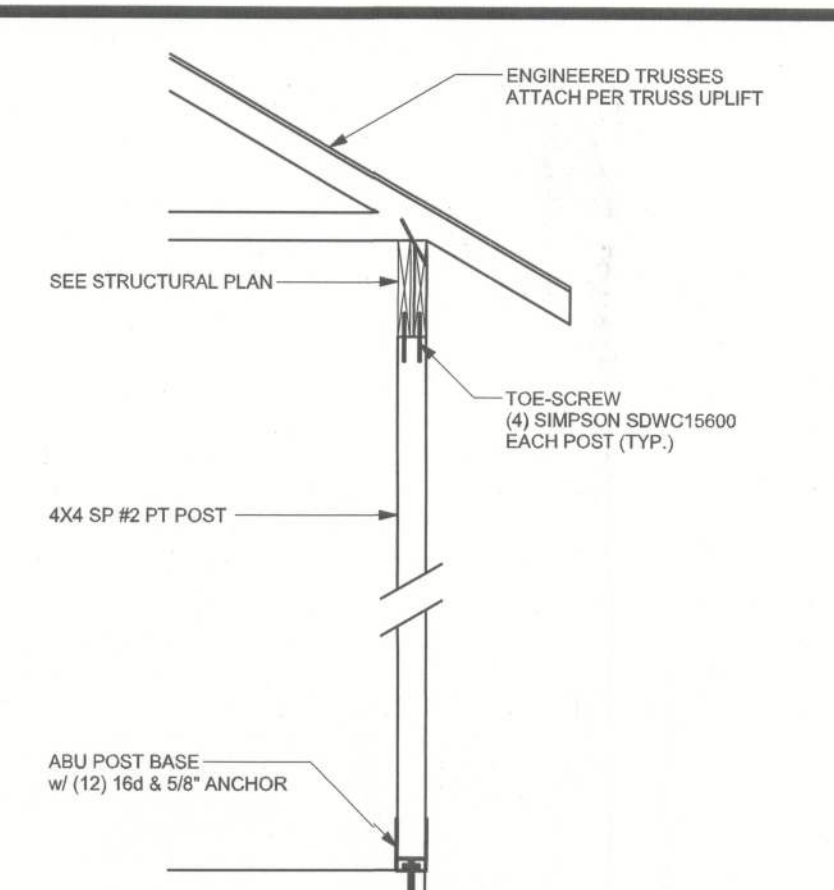
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



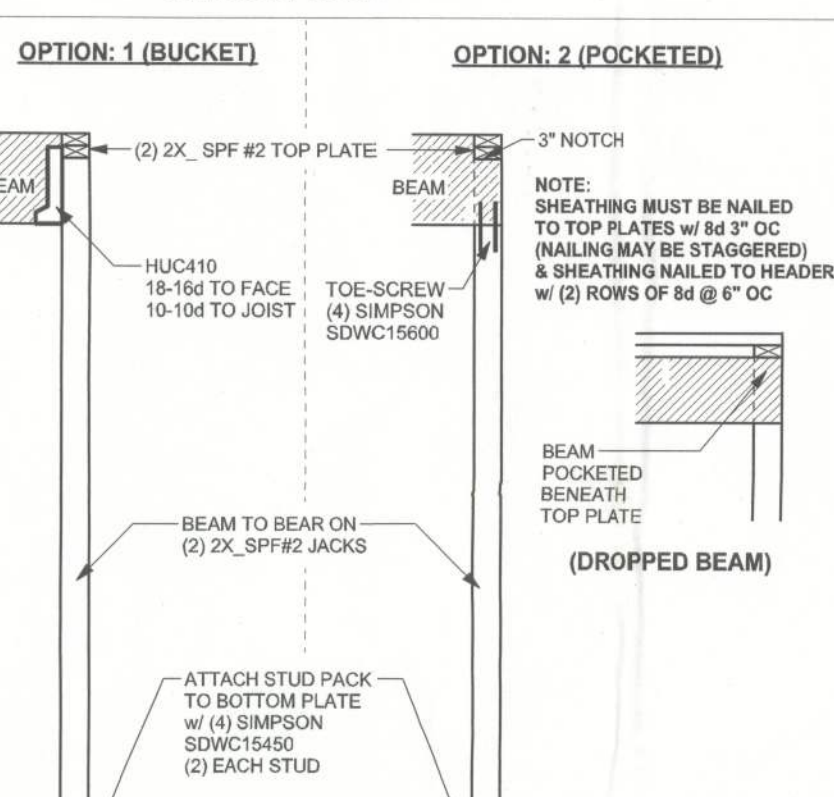
TYPICAL HEADER STRAPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



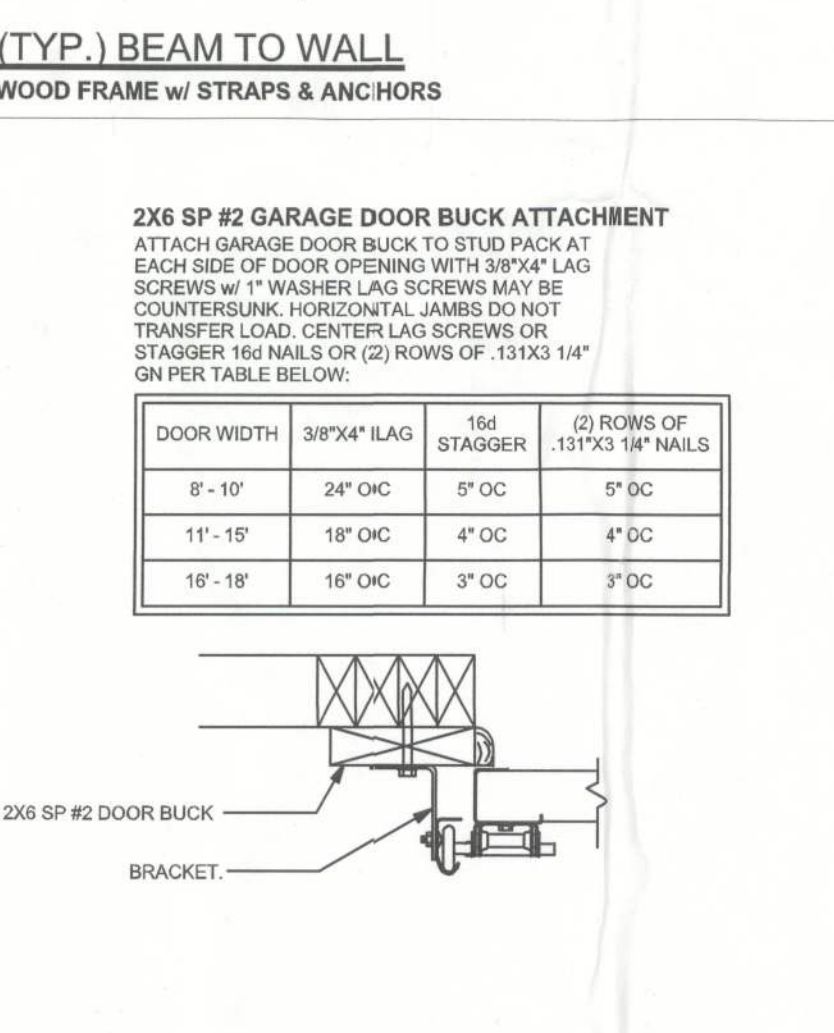
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT, P-LANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, $F_y = 85$ KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 2'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 15 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI, ALL LAP SPLICES 40" DB (20" FOR #5 BARS); UNB. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-VSP, $F_b = 2481$ E = 1800K; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS @ 12" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: 3-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls $F_m = 1500$ psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia, (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B4. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 12" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT 1/640 IN/FT OR FOR SOME BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

GRADE & SPECIES TABLE

	Fb (psi)	E (10 ⁶ psi)
2x8 SP #2	925	1.6
2x10 SP #2	800	1.6
2x12 SP #2	750	1.6
GLB 24F-V3 SP	2400	1.8
LSL TIMBERSTRAND	1700	1.7
LVL MICROLAM	1600	1.9
PSL PARALAM	2900	2.0

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	SPF UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 750	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2300	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10800	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS30	18-6d		
< 1705	< 1705	CS16	28-6d		
STUD ANCHORS*					TO STUDS
< 1350	< 1305	LT119		8-16d	12" AB
< 2310	< 2310	LT131		18-10d, 1 1/2"	12" AB
< 2775	< 2570	HD2A		2-5/8" BOLTS	5/8" AB
< 4175	< 3895	HTT16		18-16d	5/8" AB
< 1400	< 1400	PAHD42		16-16d	
< 3335	< 3335	HPAHD22		16-16d	
< 2200	< 2200	ABU44		12-16d	12" AB
< 2300	< 2300	ABU66		12-16d	12" AB
< 2320	< 2320	ABU88		18-16d	2-5/8" AB

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES THE RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)

2) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY

3) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY

4) RISK CATEGORY = II, (MRI = 700 YR)

5) ROOF ANGLE = 7-45 DEGREES

6) MEAN ROOF HEIGHT = <30 FT

7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (R2)	1	2	3	4
1	39	43			
2	39	43			
3	39	43			
4	43	46			
5	43	46			

DESIGN LOADS	FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
	ROOF	20 PSF (FLAT OR <4:12)
		16 PSF (4:12 TO <12:12)
		12 PSF (12:12 AND GREATER)
	STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY	1500 PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

REVISIONS	



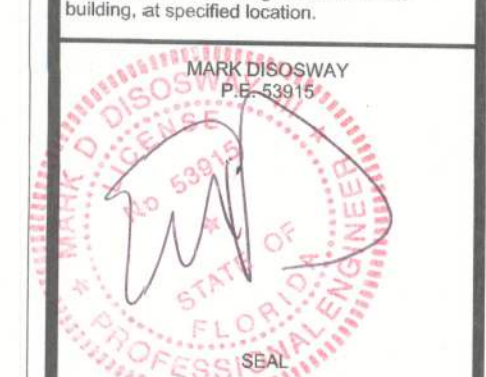
WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Aaron Simque Homes, Inc.

Bristol Model

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Mark Disoway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: Friday, June 27, 2014

DRAWN BY: STRUCTURAL BY:

FINALS DATE: 27Jun14

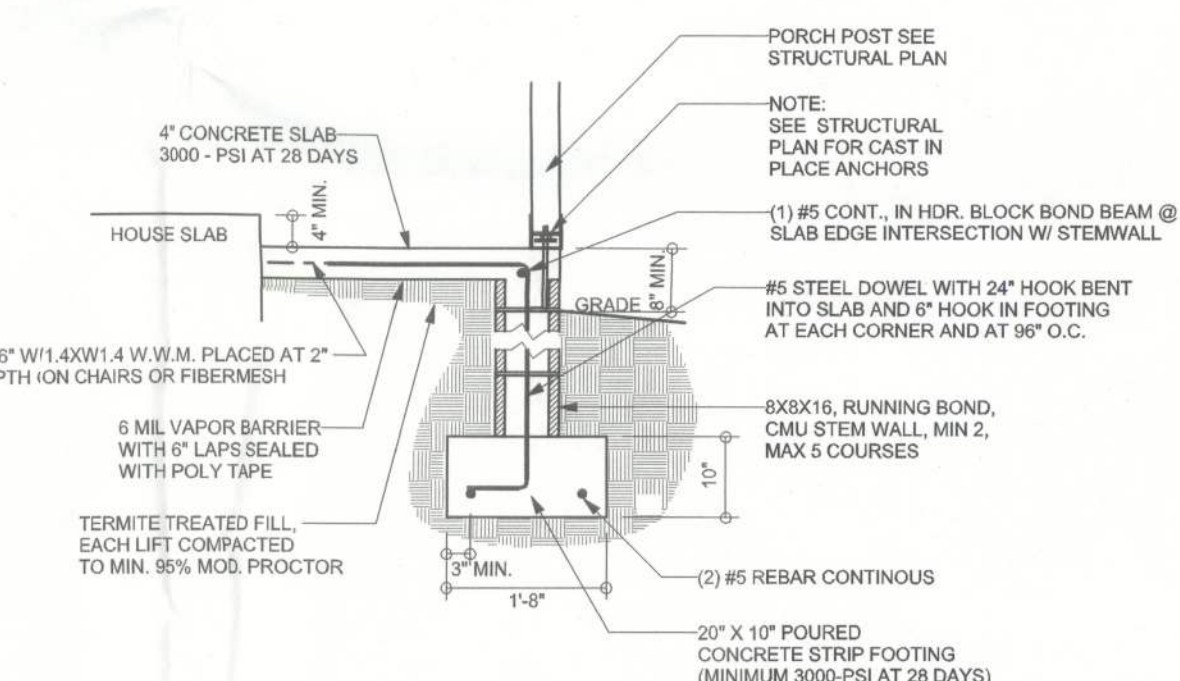
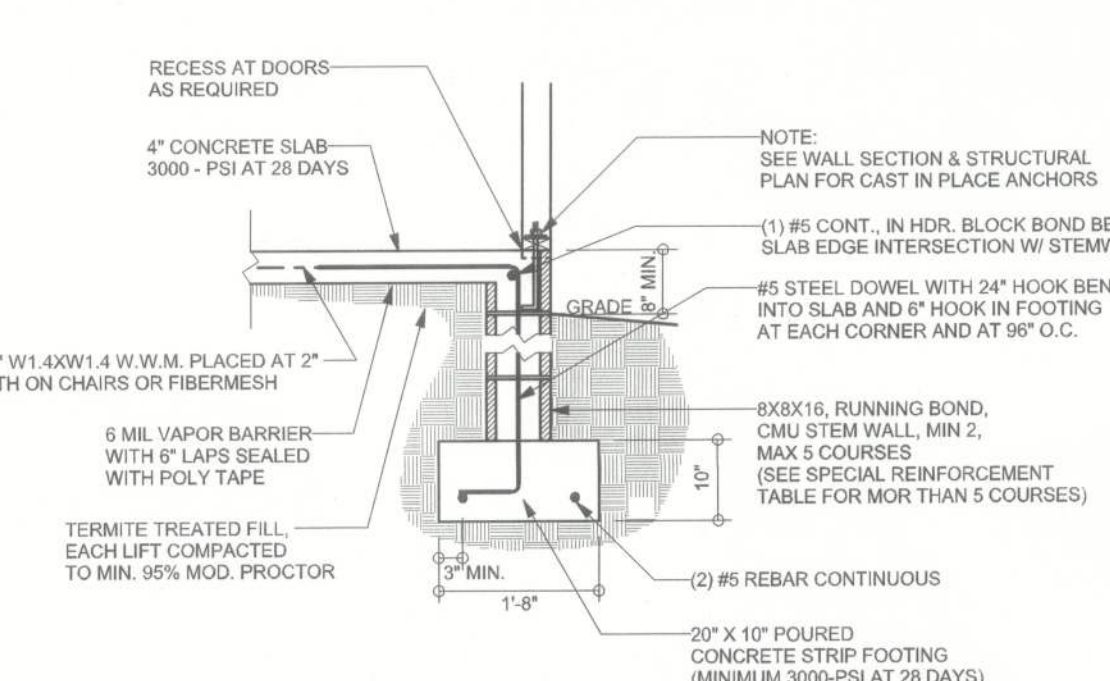
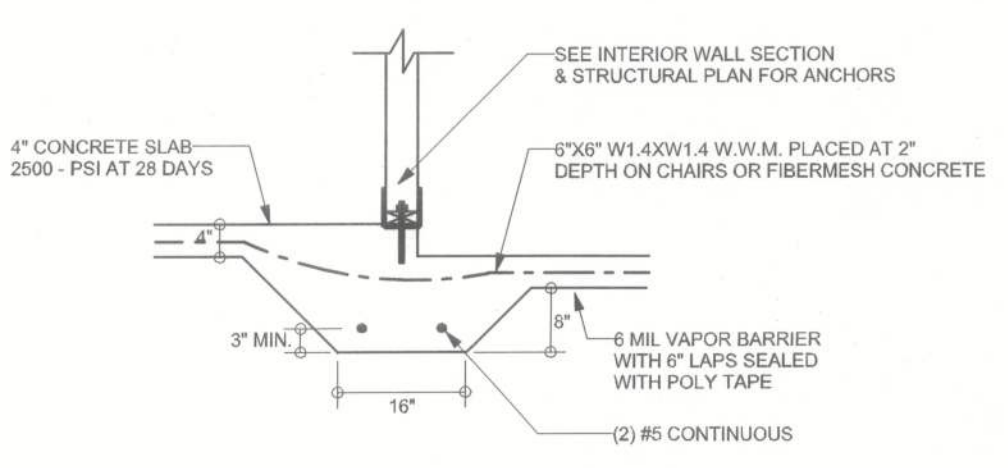
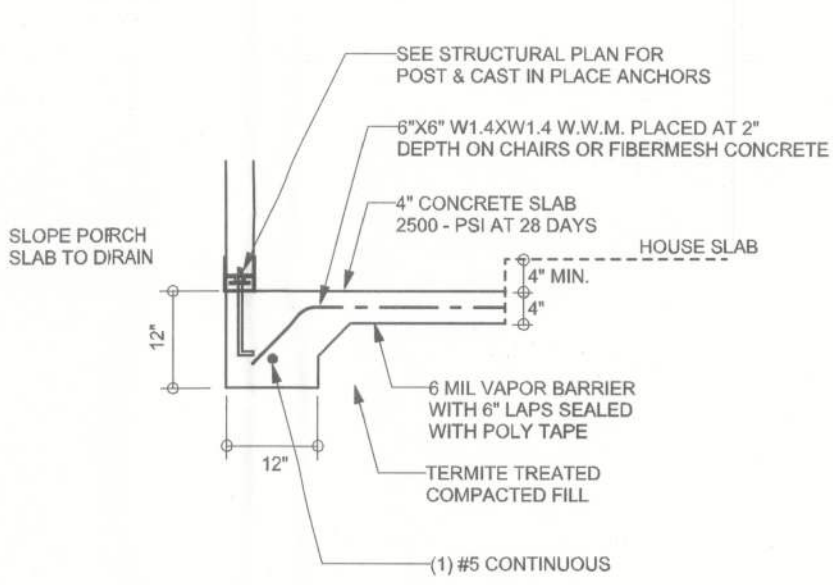
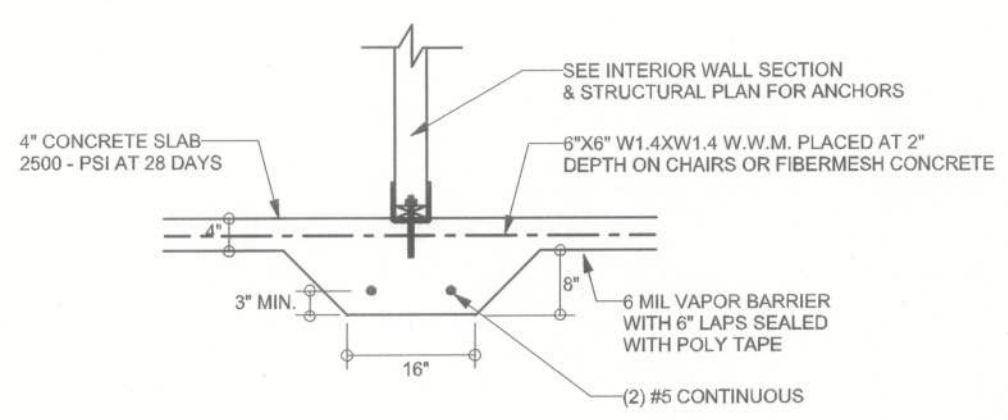
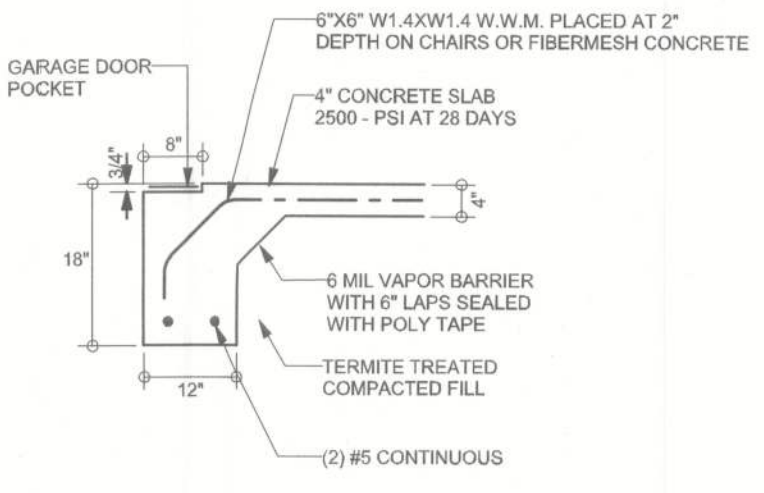
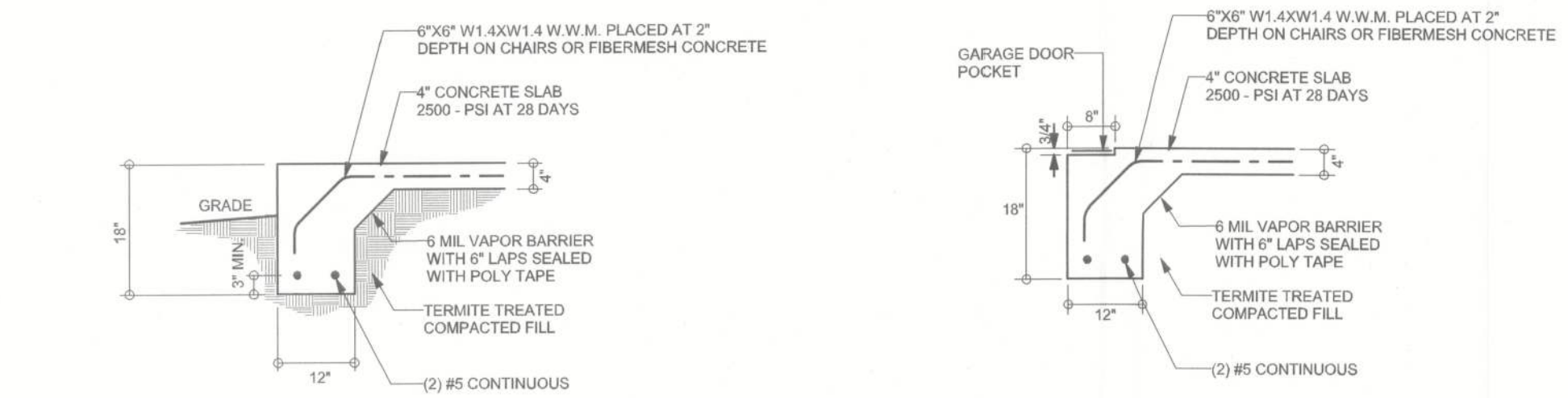
JOB NUMBER: 140555

DRAWING NUMBER

S-1

OF 3 SHEETS

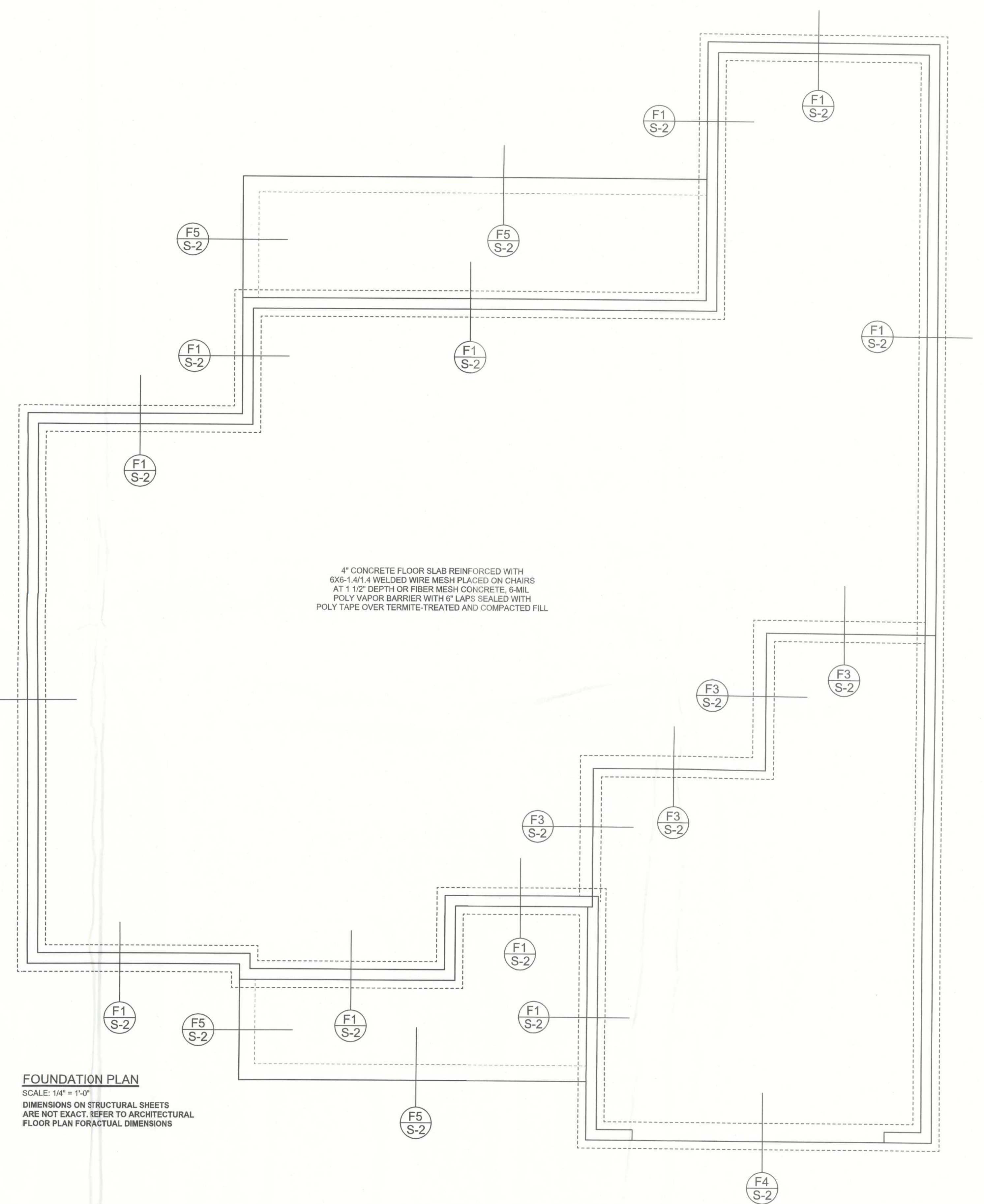
REVISIONS	



TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 166 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

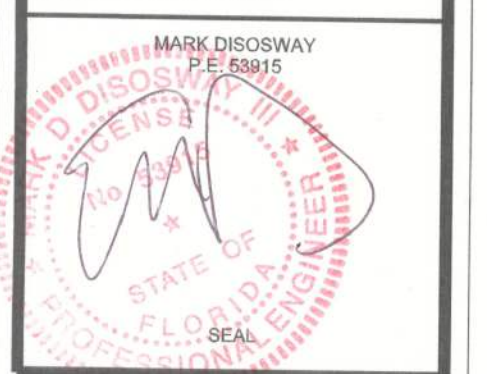
WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
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LIMITATION: This design is valid for one building, at specified location.



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Bristol Model

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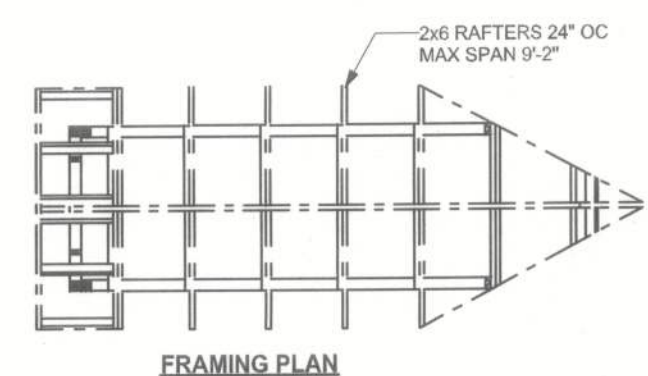
PRINTED DATE:
Friday, June 27, 2014

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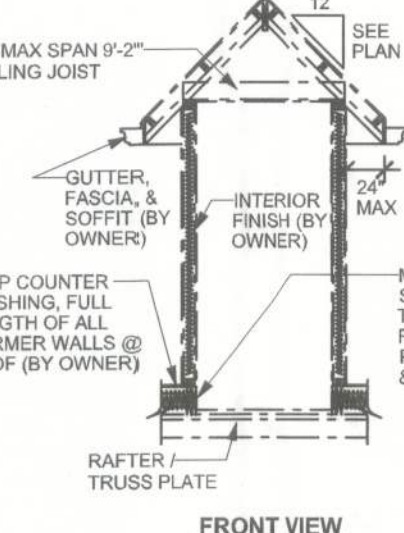
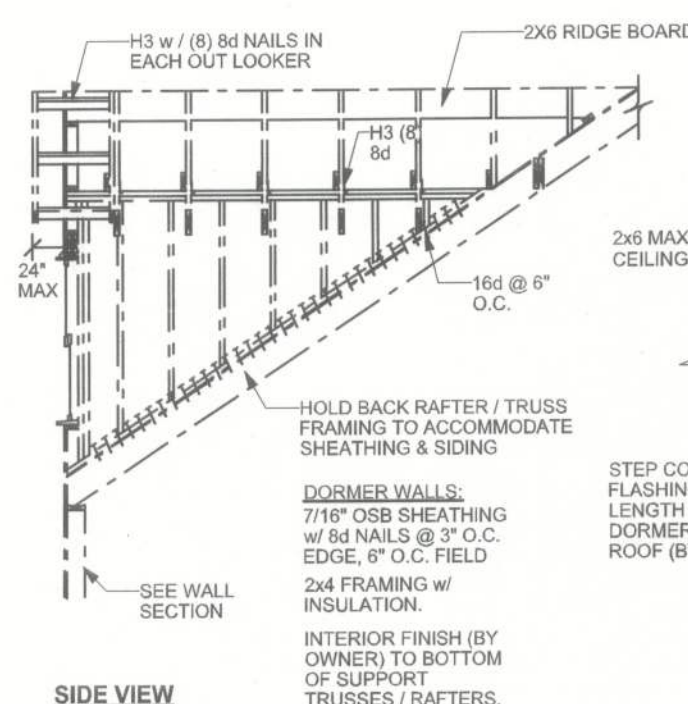
FINALS DATE:
27Jun14

JOB NUMBER:
140555

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S-2
OF 3 SHEETS



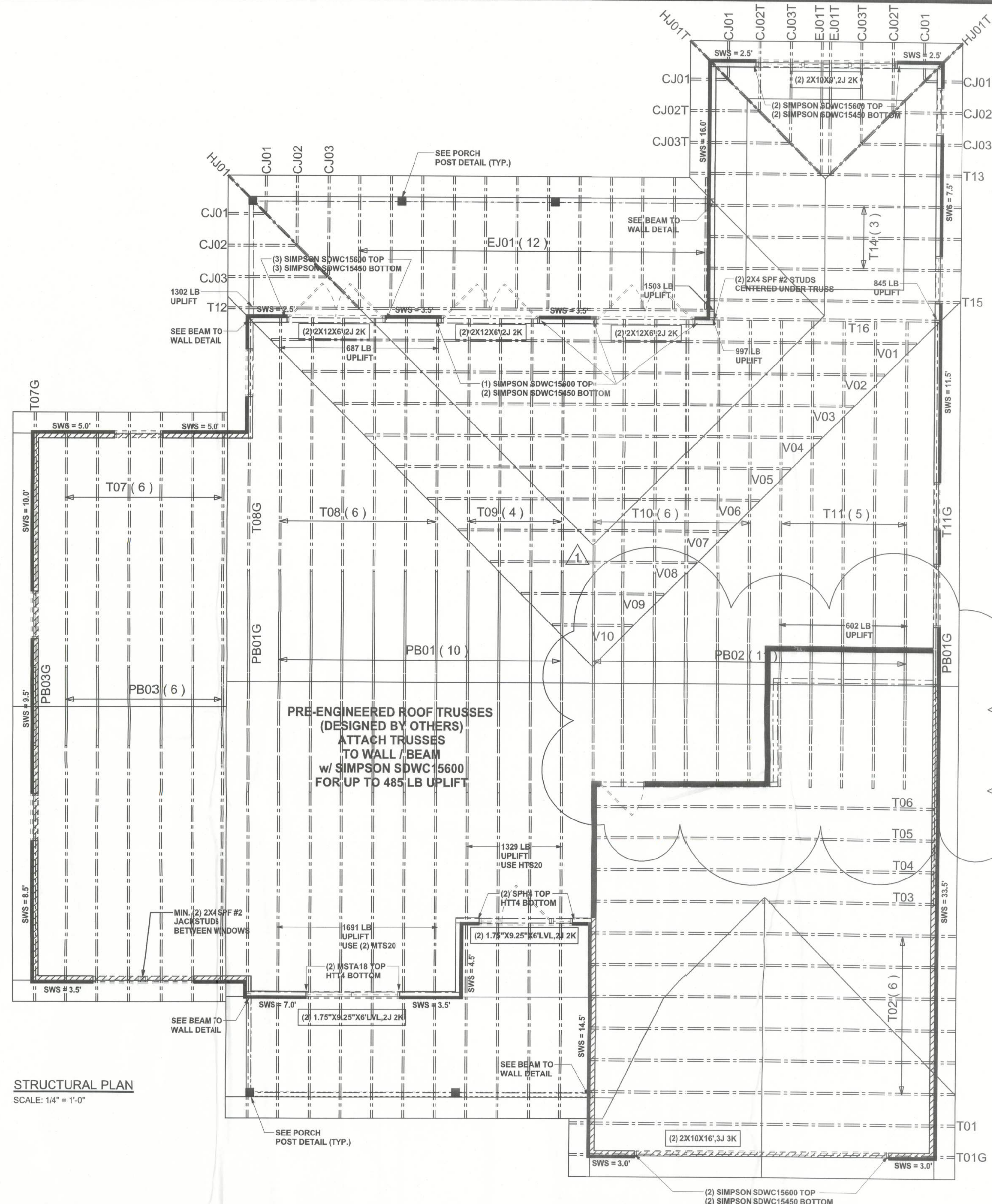
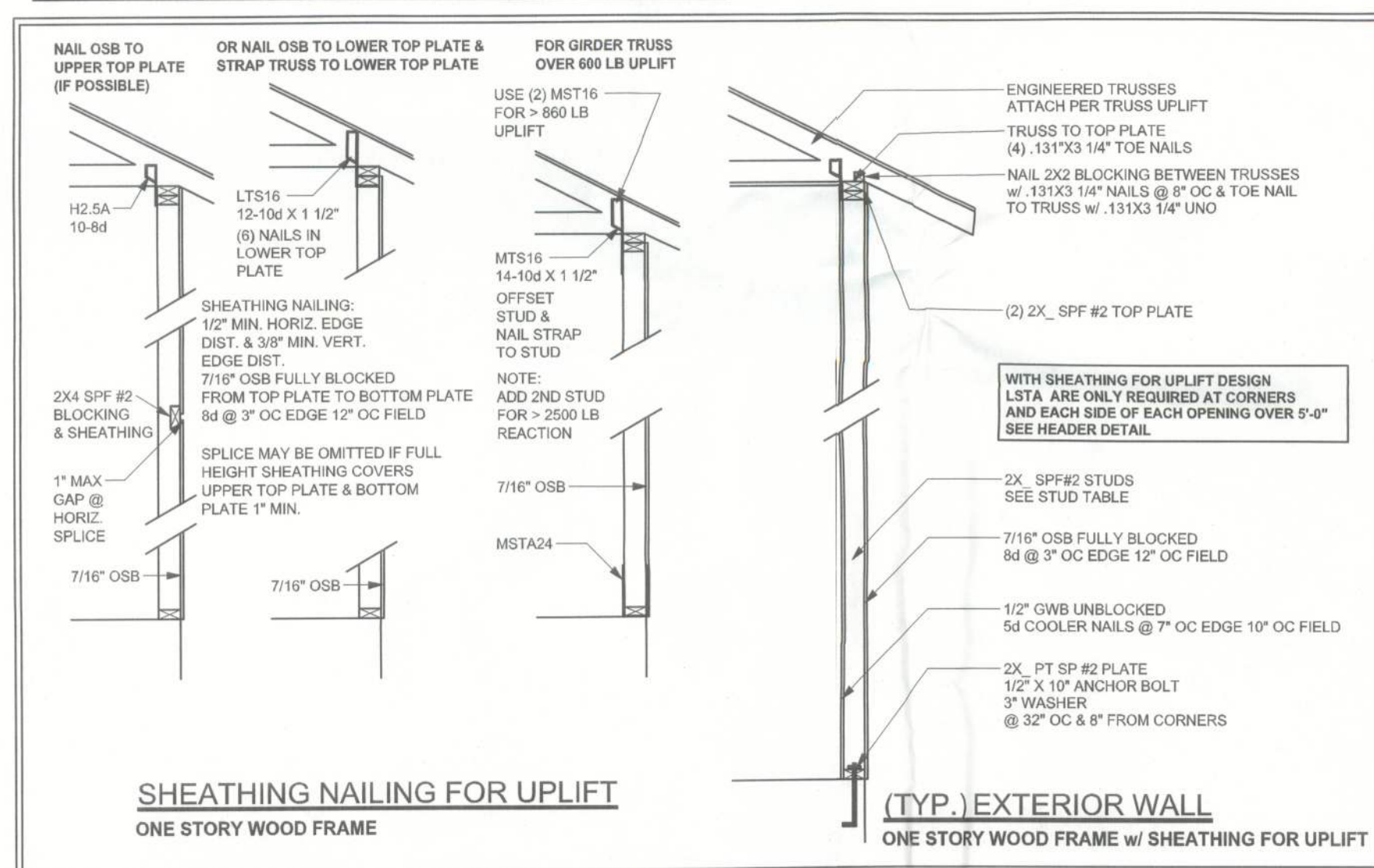
DORMER ROOF:
ROOFING (BY OWNER)
15# FELT OVER 7/16" OSB
TRUSSES / RAFTERS (SEE
ROOF FRAMING PLAN FOR
SIZE & SPACING) TO RIDGE
BOARD (FULL DEPTH OF
RAFTER CUT).
CEILING JOISTS
CEILING FINISH (BY OWNER)



DORMER ANCHORING DETAIL (ON ROOF)

SCALE: N.T.S.

SHEATHING FOR UPLIFT ATTACHMENT DETAILS



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1-03. BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND

- (2) 2X10X0', 1J 1K
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #533042

TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	48.0'	115.5'
LONGITUDINAL	40.0'	44.5'

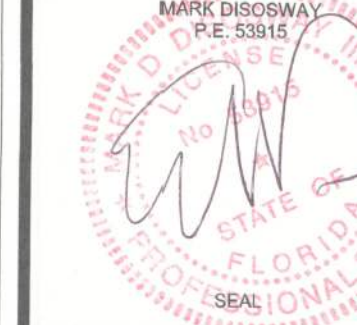
WINDLOAD ENGINEER: Mark Disosway
PE No. 53815, POB 868, Lake City, FL
32056, 386-754-5419

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Homes, Inc.

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