DATE 03/24/2011

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000029270

APPLICANT ROB SIMPSON PHONE 407.277.1028

ADDRESS 4881 DISTRIBUTION CT. ORLANDO FL 32822

OWNER BARBARA SWEENEY PHONE 352.318.1530

ADDRESS 918 SW CR 778 FT. WHITE FL 32038

CONTRACTOR DAVID DAVIS PHONE 407.277.1028

LOCATION OF PROPERTY 441-S TO C-778,TR JUST BEFORE STERLING TERR ON L.(CORNER OF STERLING & C-778.

TYPE DEVELOPMENT POOL ENLOSURE ESTIMATED COST OF CONSTRUCTION 4001.00

HEATED FLOOR AREA _____ TOTAL AREA _____ HEIGHT ___ STORIES ___

FOUNDATION _____ WALLS _____ ROOF PITCH _____ FLOOR _____

LAND USE & ZONING A-3 MAX. HEIGHT _____

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 16-7S-17-10006-102 SUBDIVISION SUMMER ACRES

LOT 2 BLOCK ____ PHASE ____ UNIT ____ TOTAL ACRES 10.54

_____ ___ SCC1311501 _____

Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor

EXISTING BLK JLW ___

Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: ACCESSORY USE. NOC on file.

Check # or Cash 2178

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by Pool _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

Pump pole _____ date/app. by Utility Pole _____ date/app. by M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by RV _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 25.00 CERTIFICATION FEE $ 0.00 SURCHARGE FEE $ 0.00

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ _____

FLOOD DEVELOPMENT FEE $ _____ FLOOD ZONE FEE $ _____ CULVERT FEE $ _____ **TOTAL FEE** 75.00

INSPECTORS OFFICE _____ CLERKS OFFICE _____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

"270" 28547 =

Columbia County Building Permit Application

For Office Use Only Application # 11-03-03 Date Received 3-2-11 By LH Permit # 29270

Zoning Official BLK Date 10.03.11 Flood Zone X Land Use A-3 Zoning A-3

FEMA Map # N/A Elevation N/A MFE N/A River N/A Plans Examiner T.C. Date 3-9-11

Comments

☑ NOC ☑ EH ☐ Deed or PA ☑ Site Plan ☐ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☑ Letter of Auth. from Contractor ☐ F W Comp. letter

IMPACT FEES: EMS Fire Corr Road/Code

School = TOTAL Accessory Structure

Septic Permit No. N/A Fax

Name Authorized Person Signing Permit Rob Simpson Phone 407.277.1028

Address 4881 Distribution Ct, Orlando, fl 32822

Owners Name Barbara Sweeney Phone 352-318-1530

911 Address 918 SW County Rd 778 fort White fl 32038

Contractors Name Custom Screens - David Davis Phone 4072771028

Address 4881 Distribution Ct. Orl, fl 32822

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Davis & Cleaton 601 N. Orlando Ave

Mortgage Lenders Name & Address MHd 327

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy

Property ID Number 16-7S-17-10006-102 Estimated Cost of Construction 3200 @ 40 (A)

Subdivision Name Summer Acres Lot 2 Block Unit Phase

Driving Directions 44' south @ 778, just before, Sterling on the left (corner of sterling and 778)

Number of Existing Dwellings on Property 1

Construction of pool enclosure (34' x 21') 700# Total Acreage 10.54 Lot Size

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height

Actual Distance of Structure from Property Lines - Front See site Plans + Copy of Bld permit applic. Side Side Rear

Number of Stories Heated Floor Area Total Floor Area Roof Pitch

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. CODE: Florida Building Code 2007 with 2009 Supplements and the 2008 National Electrical Code.

JW spoke w/ Rob on 3.10.11

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

(Owners Must Sign All Applications Before Permit Issuance.)

X Barbara J. Sweeney
Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.** N/A

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Contractor's Signature (Permitee)

Contractor's License Number SCC131150241
Columbia County
Competency Card Number 148

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 13 **day of** Dec. **20** 10.

Personally known ✓ **or Produced Identification** _____

Lani Rae Minson
State of Florida Notary Signature (For the Contractor)

SEAL:

Notary Public State of Florida
Lani Rae Minson
My Commission EE028160
Expires 10/12/2014

4881 Distribution Court • Orlando, Florida 32822 (407) 277-1028 • FAX (407) 277-1674

Columbia County, Florida
12/13, 2010

I, David Davis, give power of attorney to
Rob Simpson for the purpose of
Obtaining a building permit to construct a pool enclosure on
Lot 2 in Summer Acres subdivision.
Address: 918 SW CR 778

The foregoing instrument was acknowledged before me this
13 day of Dec., 2010 by David Davis.

Notary Public State of Florida
Terry Davis
My Commission EE017283
Expires 10/12/2014

Davis & Cleaton Engineering, Inc.

601 North Orlando Avenue
Suite #112
Maitland, FL 32751
(407)539-2353
FAX (407)539-2334

103 West Wisconsin Avenue
Suite #104
DeLand, FL 32720
(386)738-7475
FAX (386)738-7781

September 17, 2010

To Whom It May Concern:

This letter is to serve as authorization for the following contractor to use my sealed engineering set of design criteria and details for the design of aluminum structures.

Custom Screens
4881 Distribution Ct.
Orlando, FL 32822
Phone: 407-277-1028
Fax: 407-277-1674
Contractor Name: David Davis
License Number: SCC131150241

Please note that I require only one signed and sealed letter per job and that I furnish the contractor with two complete sets of signed and sealed engineering. Please be advised that any additional signed and sealed copies are treated by me as separate jobs and therefore add cost to the contractor.

In accordance with Florida Statute 489.113(9), this sealed engineering set is intended to be used as a reference in conjunction with the contractor's own site-specific design drawing. The contractor's drawing is not required to be sealed by me as the engineer of record as per FS 489.113(9). It is only required to be in compliance with what is set forth in my sealed design set.

If there are any questions, please contact us.

Sincerely,

Frank A. Cleaton, Jr., P.E.
FL P.E. License # 35816

4881 Distribution Court • Orlando, Florida 32822 (407) 277-1028 • FAX (407) 277-1674

Columbia Co., Florida
3/23, 2011

I, David Davis, give power of attorney to
Rob Simpson for the purpose of
Obtaining a building permit to construct a pool enclosure on
Lot 2 in Summer Acres subdivision.
Address: 918 SW CR 778

The foregoing instrument was acknowledged before me this
23 day of March, 2011 by David Davis.

Notary Public State of Florida
Terry Davis
My Commission EE017283
Expires 10/12/2014

Columbia County Property Appraiser

DB Last Updated: 2/17/2011

2010 Tax Year

Tax Collector | Tax Estimator | Property Card | Parcel List Generator | Interactive GIS Map | Print

Parcel: 16-7S-17-10006-102

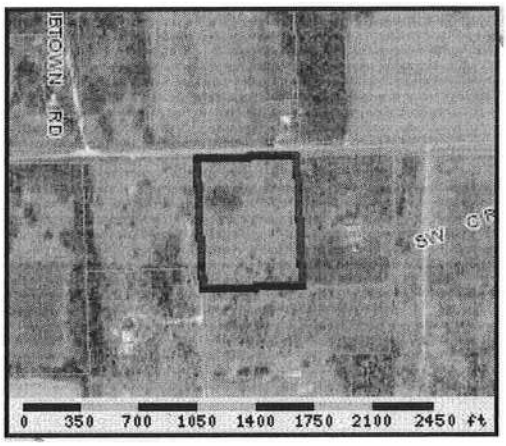
<< Next Lower Parcel | Next Higher Parcel >>

Search Result: 1 of 1

Owner & Property Info

Owner's Name	SWEENEY KENNETH C & BARBARA J		
Mailing Address	16975 HWY 278 WILLISTON, SC 29853		
Site Address	918 SW COUNTY ROAD 778		
Use Desc. (code)	VACANT (000000)		
Tax District	3 (County)	**Neighborhood**	16717
Land Area	10.540 ACRES	**Market Area**	02
Description	NOTE: This description is not to be used as the Legal Description for this parcel in any legal transaction.		

AKA LOT 2 SUMMER'S ACRES UNR: COMM NW COR, RUN S 80.15 FT TO S R/W CR-778, RUN E ALONG R/W 698.08 FT FOR POB, CONT E 587.02 FT, S 782.02 FT, W 587.02 FT, N 782.02 FT TO POB. ORB 983-2580 & WD 1197-2087



Property & Assessment Values

2010 Certified Values		
Mkt Land Value	cnt: (0)	$51,388.00
Ag Land Value	cnt: (1)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$51,388.00
Just Value		$51,388.00
Class Value		$0.00
Assessed Value		$51,388.00
Exempt Value		$0.00
Total Taxable Value		Cnty: $51,388 Other: $51,388 | Schl: $51,388

2011 Working Values

NOTE:
2011 Working Values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Show Working Values

Sales History

Show Similar Sales within 1/2 mile

Sale Date	OR Book/Page	OR Code	Vacant / Improved	Qualified Sale	Sale RCode	Sale Price
7/13/2010	1197/2087	WD	V	Q	01	$130,000.00
5/21/2003	983/2580	WD	V	Q		$48,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value

NOTICE OF COMMENCEMENT

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number 16-7S-17-10006-102

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this **NOTICE OF COMMENCEMENT.**

1. Description of property *(legal description)*: Lot 2 Sumner Acres
   a) Street *(job)* Address: 918 SW County Rd 778 High Springs
2. General description of improvements: _____
3. Owner Information
   a) Name and address: Kenneth Sweeney 918 SW County Rd 778, High Springs 32669
   b) Name and address of fee simple titleholder (if other than owner) _____
   c) Interest in property Owner
4. Contractor Information
   a) Name and address: Custom Screens 4881 Distribution Ct. Orlando, FL 32822
   b) Telephone No.: _____ Fax No. (Opt.) _____
5. Surety Information
   a) Name and address: _____
   b) Amount of Bond: _____
   c) Telephone No.: _____
6. Lender
   a) Name and address: _____
   b) Phone No. _____

Inst 201112003159 Date:3/2/2011 Time:3:05 PM
DC,P DeWitt Cason,Columbia County Page 1 of 1 B 1210 P 1723

7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
   a) Name and address: _____
   b) Telephone No.: _____ Fax No. (Opt.) _____

8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
   a) Name and address: _____
   b) Telephone No.: _____ Fax No. (Opt.) _____

9. Expiration date of Notice of Commencement **(the expiration date is one year from the date of recording unless a different date is specified)**: _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. Kenneth C Sweeney
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager

KENNETH C SWEENEY
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 23rd day of November, 2010, by: Kenneth Sweeney as owner (type of authority, e.g. officer, trustee, attorney fact) for Kenneth Charles Sweeney (name of party on behalf of whom instrument was executed).

Personally Known ____ OR Produced Identification ✓ Type DL #S500503533[illegible]

Notary Signature Christina Wood Notary Stamp or Seal:

CHRISTINA WOOD
MY COMMISSION # EE 008413
EXPIRES: July 13, 2014
Bonded Thru Notary Public Underwriters

—AND—

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Kenneth C Sweeney
Signature of Natural Person Signing (in line #10 above.)

· Columbia County Building Permit Application Ck# - 3358

For Office Use Only Application # 1010-15 Date Received 10/7/10 By LH Permit # 28997 / 18852

Zoning Official BLK Date 10.10.10 Flood Zone X Land Use A-3 Zoning A-3

FEMA Map # N/A Elevation N/A MFE Waived River N/A Plans Examiner T.C. Date 10-15-10

Comments

☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan ☑ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter

IMPACT FEES: EMS Fire Corr Road/Code

School = TOTAL N/A Suspended - ☒ V.F.

Septic Permit No. 10-438 Fax (386) 454-4971 (call first)

Name Authorized Person Signing Permit Kara Sutton Phone (352) 318-9864

Address PO BOX 1198 Newberry, FL 32669

Owners Name Kenneth and Barbara Sweeney Phone (352) 316-3463

911 Address 918 SW CR 778 High Springs, FL 32643

Contractors Name Sutton Family Homes and Management Consulting Inc (KARA SUTTON) Phone (352) 318-9864

Address PO BOX 1198 Newberry, FL 32669

Fee Simple Owner Name & Address Same as above

Bonding Co. Name & Address None

Architect/Engineer Name & Address Juris Luzins (352) 224-8673

Mortgage Lenders Name & Address None

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 11-75-17-10006-102 Estimated Cost of Construction $511,000

Subdivision Name Summers Acres Lot 2 Block Unit Phase

Driving Directions 441 South, (R) 778, Property is on the (Left) 2nd Lot past SW Marynik Drive (or) 2nd Lot back from Scrub town Rd

Number of Existing Dwellings on Property 0

Construction of Single Family Dwelling. Total Acreage 10.5 Lot Size 10.5 Acre

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 24'-6"

Actual Distance of Structure from Property Lines - Front 230' Side 185' Side 235' Rear 470'+

Number of Stories 1 Heated Floor Area 3244 ± Total Floor Area 5152 ± Roof Pitch 7/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. **CODE:** Florida Building Code 2007 with 2009 Supplements and the 2008 National Electrical Code.

1441.52

Jw left message for KARA 10.19.10

CERTIFICATE OF COMPLETION

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 16-7S-17-10006-102 **Building permit No.** 000029270

Permit Holder DAVID DAVIS

Owner of Building BARBARA SWEENEY

Location: 918 SW CR 778, FT WHITE, FL 32038

Date: 03/29/2011

Troy Cier

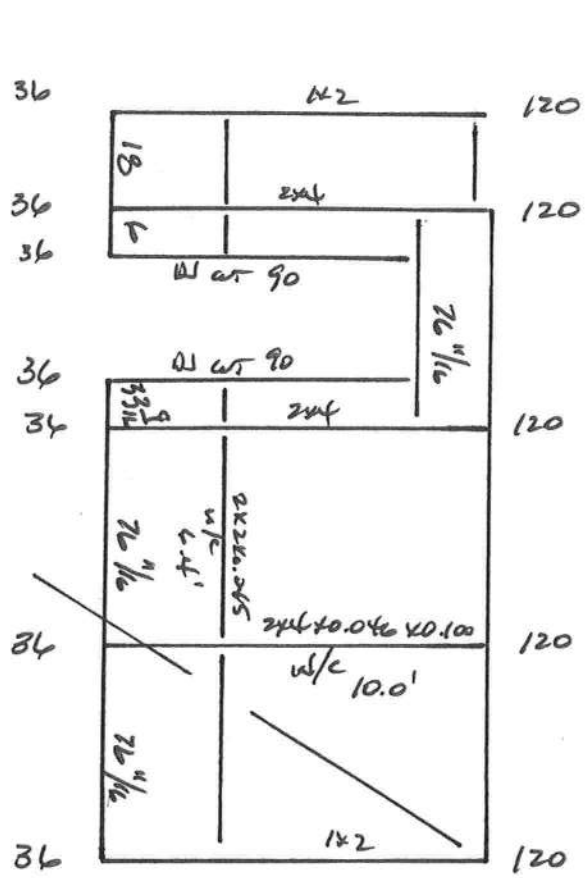
Building Inspector

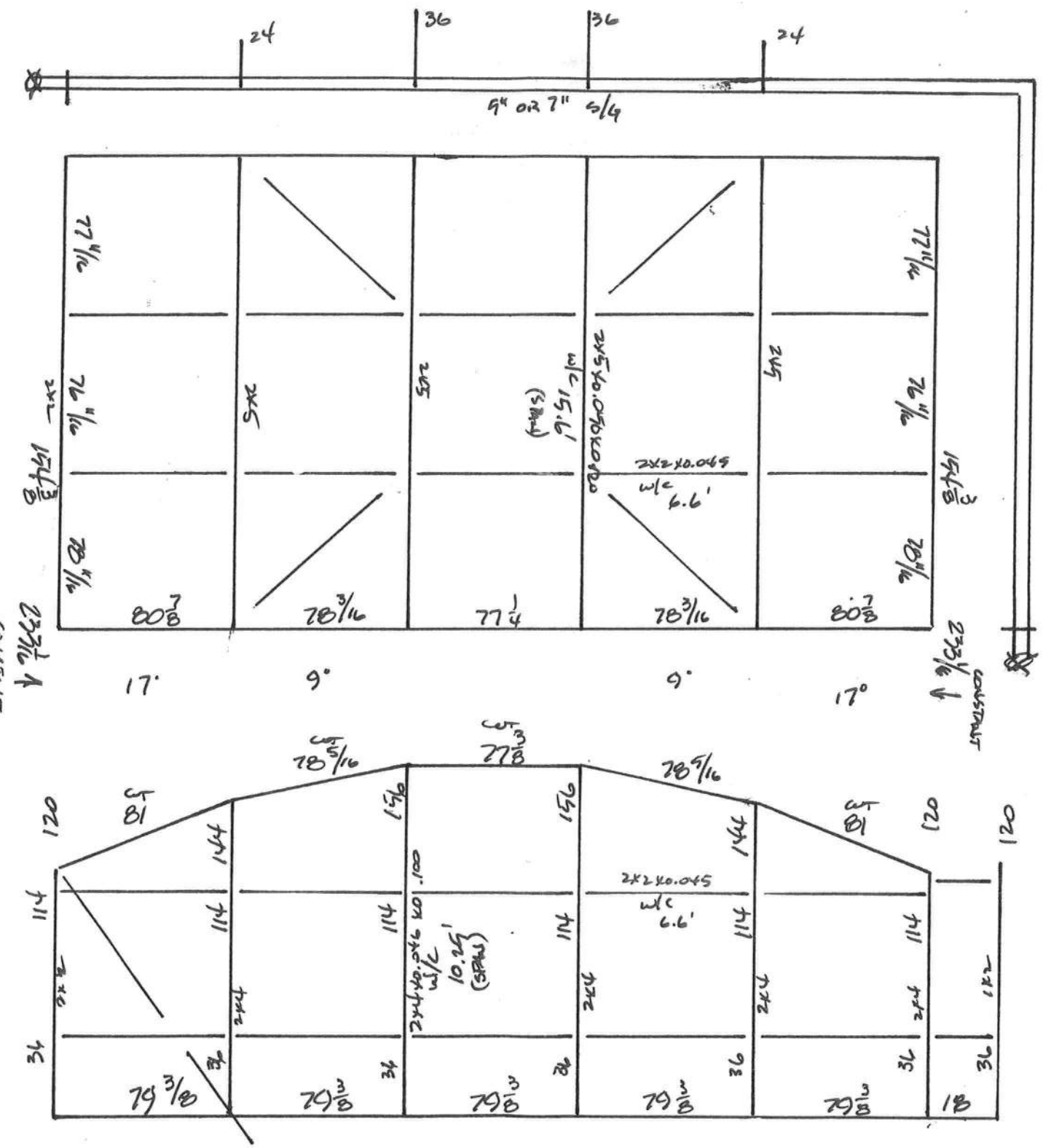
POST IN A CONSPICUOUS PLACE
(Business Places Only)

KB's House 2@20 / 2@37 > 2x3 LAT. VALUE w/c = 1.16'

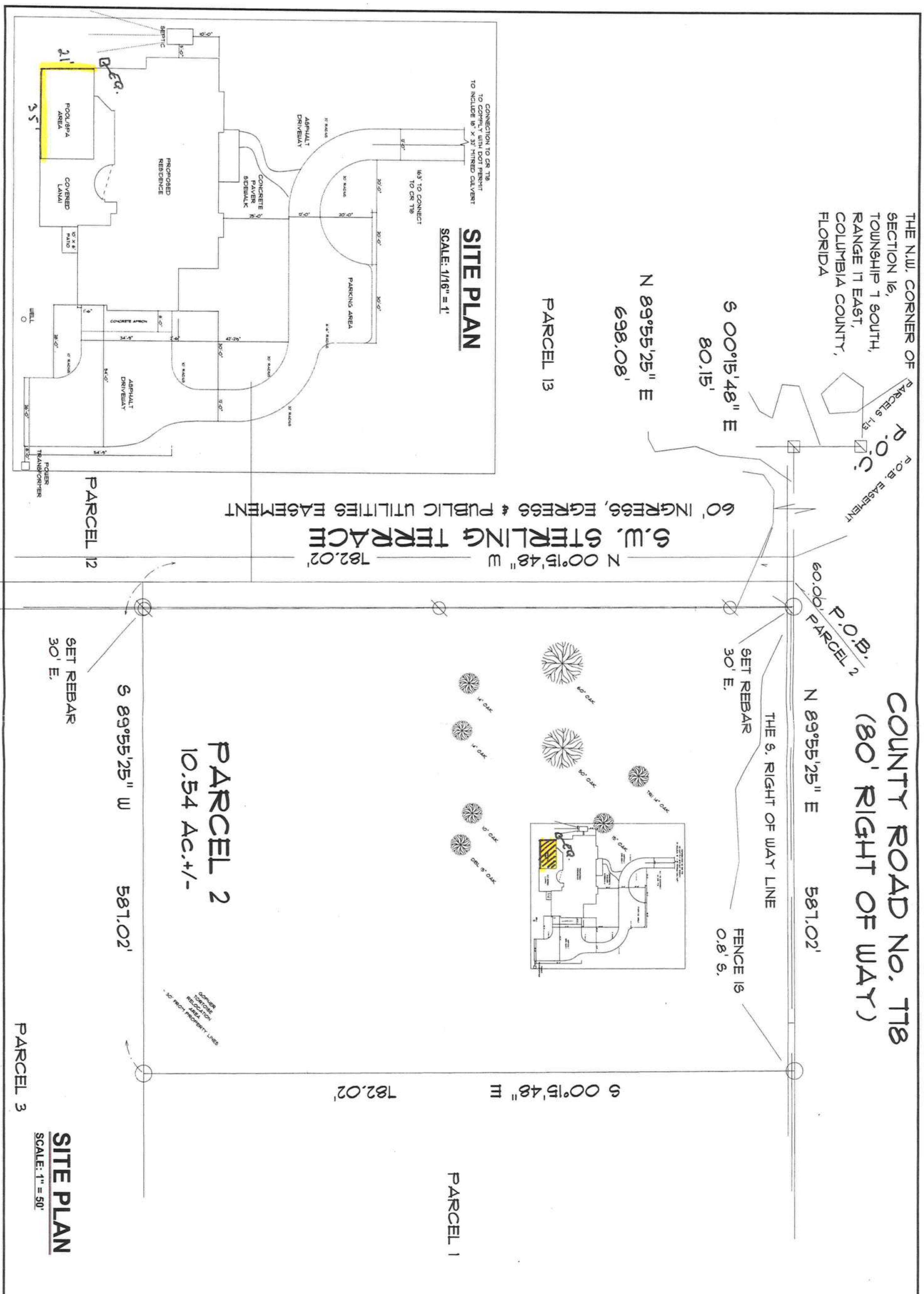
KB's FW 4@ 42½ 2x3 LAT. & VERT. VALUE = 2.5'

CABLE's 2@ 14'8"





SWEENEY
918 SW CR 778
HIGH SPRINGS
"SUMMER ACRES"



CUSTOMER:	KEN AND BARBARA SWEENEY
CONTRACTOR:	SUTTON FAMILY HOMES & MANAGEMENT CONSULTING, INC.
SUBDIVISION:	SUMMER'S ACRES
LOT:	LOT 2

SHEET NO.	6
PLAN NAME:	KEN & BARBIE'S DREAM HOUSE
GARAGE POSITION:	GARAGE RIGHT
SCALE:	1/4" = 1'-0" UNLESS OTHERWISE NOTED

AREA SUMMARY	
MAIN LIVING:	3244 SF
GARAGE/MECH:	948 SF
ENTRY:	160 SF
LANAI:	800 SF
TOTAL:	5152 SF

*ALL WORK SHALL COMPLY WITH APPLICABLE LOCAL CODES
*THESE PLANS AND NOTES ARE THE PROPERTY AND SOLE RESPONSIBILITY OF SUTTON FAMILY HOMES AND MANAGEMENT CONSULTING, INC. USE OF THESE PLANS WITHOUT THE WRITTEN CONSENT OF SUTTON FAMILY HOMES AND MANAGEMENT CONSULTING, INC. IS PROHIBITED.
*THESE PLANS ARE SUBJECT TO MODIFICATION AS NECESSARY TO MEET CODE REQUIREMENTS OR TO FACILITATE MECHANICAL INSTALLATIONS OR TO INCORPORATE DESIGN IMPROVEMENTS. THE BUILDER RESERVES THE RIGHT TO MAKE ANY CHANGES FOR ANY REASON, AT ANY TIME, PROVIDING THEY COMPLY WITH THE CODE.
*THE SUB-CONTRACTOR SHALL COMPARE AND COORDINATE ALL DRAWINGS, WHEN A DISCREPANCY OR ERROR OR OMISSION EXISTS, THE SUB-CONTRACTOR SHALL COMPLY WITH THE CODE AND CONTACT THE BUILDER IN WRITING FOR PROPER ADJUSTMENT.

REVISION DATE:
08/06/2010
08/13/2010
08/21/2010
09/16/2010
09/29/2010-FINAL

DESIGNED BY:
SUTTON FAMILY HOMES AND MANAGEMENT CONSULTING, INC.
PO BOX 1198
NEWBERRY, FL 32669
PH (352) 318-9684
FAX (352) 472-8686

SOFTPLAN

PAGE CONTENT:
SITE PLAN

General Notes & Design Statement

1) This structure has been designed in accordance with the requirements of the 2007 FBC – Residential, Chapter 3, Section R301.2.1.1, with the 2009 supplements. Wind loading requirements were met in accordance with 2007 FBC-Building, Chapter 20 (Light Metal Alloys) - Table 2002.4 (DESIGN PRESSURES FOR ALUMINUM SCREEN ENCLOSURES).
   a. I=0.77 for 110 MPH or higher \ I=0.87 for 100 MPH
   b. Building Category = I (in accordance with 2007 FBC-Building Chapter 16 table 1604.5).
   c. Applicable Internal Pressure Coefficient: ± 0.00
   d. Design Pressure for Exterior Components & Cladding: +14 PSF
   e. Wind Speed is based on a 3 second gust wind speed. Wind speed and category are defined in the title block.

2) This structure is designed to be attached to block and wood frame structures of adequate structural capacity. The contractor shall verify that the host structure and foundation to which the aluminum structure attaches to is in good condition and meets all requirements of sufficient strength as set forth by the engineer of record. If there is a question of structural capacity then the owner, at his own expense, shall hire an architect or engineer to verify the structural capacity of the host structure.

3) The screen enclosures designed with this set of engineering connection details are not required to be sealed by an engineer in accordance with F.S. 489.113(9), provided they meet the following requirements: (1) Pool enclosures shall not exceed 16' in average height or a combined height and span of 50'. (2) Solid roofs (composite roofs or aluminum pan roofs) shall not exceed 16' in span. Structures that exceed the aforementioned requirements are to be signed and sealed by Frank Cleaton, JR, P.E.

4) The user of this generic engineering package is required to have ***only one*** signed and sealed letter of authorization from Frank Cleaton, JR, P.E. per job, allowing for the usage of this set of engineering. Copies of said letter can be made, provided they are used in conjunction with an individual signed and sealed letter.

5) Where super gutter is specified, the two available sizes of 5 in. and 7 in. can be used interchangeably, unless otherwise noted.

6) Screen wall and screen roofs are based on a 60% screen mesh density with a maximum density weave of 20x20 threads per square inch.

7) Actual wall thickness of members shall not be less than 0.040 in. Alloy of members is given in the "Member Definition List."

8) Concrete shall be a minimum 28 day compressive strength of 2500 PSI, and be in accordance with the requirements of ACI 318. Reinforcing steel shall have a minimum yield strength of 40,000 PSI (grade 40) and be provided with cover in accordance with ACI 318. If existing concrete slab/footing are deemed adequate, it may be incorporated into a new slab/footing by observing the following procedure:
   a. Clean and scabble all connecting edges
   b. Drill and epoxy embed #3 (MIN) reinforcing bar placed @ 24" O.C. mid depth. The rebar should be embedded a minimum of 4" (using Rawl R-KEA Epoxy Acrylate Resin System or approved equal), leaving 8" exposed to be incorporated into the new slab/footing.

9) Doors and their locations are incidental to the structural integrity of the design.

10) Knee Brace lengths are given by their horizontal and vertical displacements and not the actual cut length.

11) Screen Frame wall with guardrail are designed as follows:
   a. 20 lbs/LF in any direction at the top of the rail and will transfer loads to the supports of the structure.
   b. 200 lbs concentrated load at any point in any direction along the top
   c. A 1.33 stress increase is allowed per 1607.7.1.3 for working stress (allowable stress) design.

12) Screen mesh continuity along frame members is incidental to the structural integrity of the design, therefore each panel is not required to be fastened in place independent of other panels around it.

13) When using Self-Drilling TEK screws in lieu of S.M.S., longer screws must be used to compensate for drill head so that the required embedment length of threaded parts of the screw are in positive contact with the host or connected material. TEK screws and S.M.S. are used interchangeably. Aluminum Surfaces in contact with lime-mortar, concrete, or other masonry materials, shall be protected with alkali-resistant coatings such as heavy-bodied bituminous paint or water-white methacrylate lacquer.

14) Unless otherwise noted, concrete anchors shall be placed no less than 2-1/2" from the edge of slab/footing or 5x the embedment of the fastener.

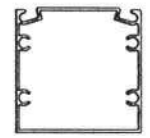
15) Any notes or design specifications inside the engineering details can be superseded by a site-specific design sealed by the Frank Cleaton, JR, P.E.

16) Minimum edge distance and center-to-center distances for fasteners are as follows:

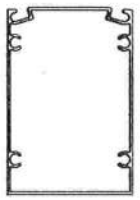
C-1022 Low Carbon Steel S.M.S. & Self-drilling (TEK) screws			
Fastener	Nominal Diameter	Minimum Edge Distance	Minimum Center to Center Distance
#10	0.188	5/16"	1/2"
#12	0.219	3/8"	5/8"
#14	0.25	1/2"	7/8"

1x2 O.B.
(1x2x0.044 Open Back)
6005-T5

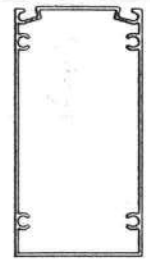
1x3 O.B.
1x3x0.044 Open Back
6005-T5



2x2 H
(2x2x0.044 Hollow)
6005-T5



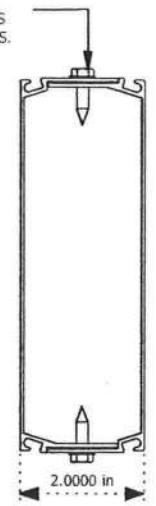
2x3 H
(2x3x0.045 Hollow)
6005-T5



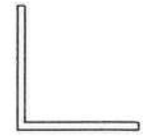
2x4 H
(2x4x0.050 Hollow)
6005-T5

Self-Mating Beam Half-Shell
6005-T5 alloy

SMB Naming Convention	
Label Name	Proper Name
2x4 SMB	2x4x0.046x0.100 Self-Mating Beam
2x5 SMB	2x5x0.050x0.116 Self-Mating Beam
2x6 SMB	2x6x0.050x0.120 Self-Mating Beam
2x7 SMB	2x7x0.055x0.120 Self-Mating Beam
2x8 SMB	2x8x0.072x0.224 Self-Mating Beam
2x9 SMB	2x9x0.072x0.224 Self-Mating Beam
2x10 SMB	2x10x0.092x0.389 Self-Mating Beam

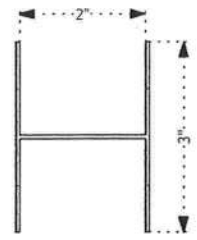


Self-Mating Beam Fastened Together
(6005-T5 Alloy)



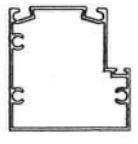
2x2 Angle
(2x2x0.125 Angle)
6005-T5

U-Channel / Receiver Channel
1x2x0.055 U-Channel
6005-T5



H-Channel
2x3x0.062 H-Channel
6005-T5

1x2 Angle
(1x2x0.062 Angle)
6005-T5



Door Jamb
(2x2x0.046 Door Jamb)
6005-T5

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DATE 7/25/11

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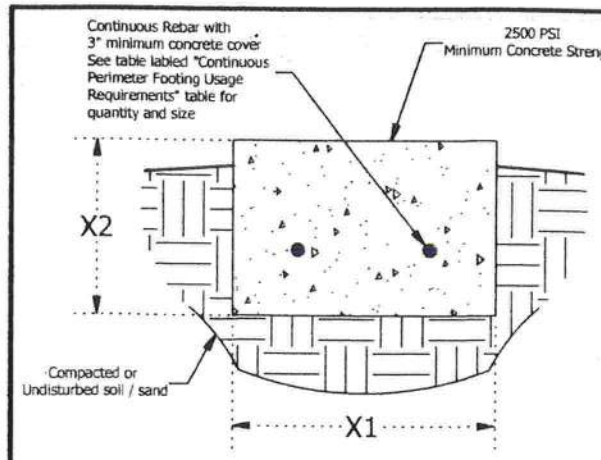
Sheet: D1

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



Continuous Perimeter Footing

Continuous Perimeter Footing Usage Requirements

Footer Size		Rebar Size & Qty		100-130 MPH Exposure B		100-130 MPH Exposure C	
x1	x2	#3	#5	Max Beam Span	Max Upright Height	Max Beam Span	Max Upright Height
8"	8"	2	1	15.2'	12.1'	12.1'	10.4'
12"	8"	2	1	23.0'	16.0	19.2'	14.8'
12"	12"	3	2	30.0'	17.2'	26.3'	16.0'
12"	16"	3	2	36.0'	22.2'	31.2'	16.9'
12"	18"	3	2	38.0'	23.3'	34.0'	17.8'
16"	18"	4	3	44.2'	29.5'	40.0	22.5'
16"	24"	4	3	57.6'	35.4'	48.0'	27.4'
20"	24"	5	4	69.1'	39.7'	57.6'	32.5'

Continuous Perimeter Footing Notes:

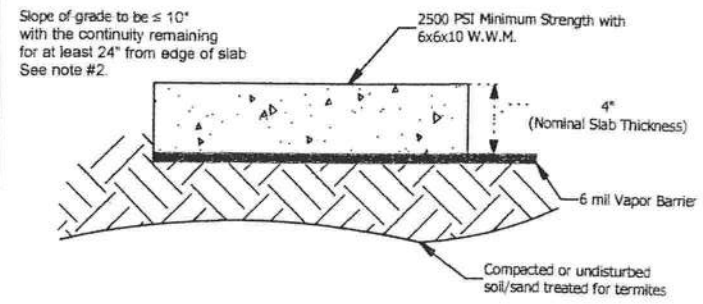
1) "x1" and "x2" dimensions may be switched to accomodate for field conditions.

2) If the span capabilities and the upright height capabilities are in conflict, the larger footer is to be used

3) Reinforcement bars to be grade 60 steel

4) Footer size is determined by using the average beam span or upright height of the adjoining roof or wall respectively

5) For side walls (non-bearing walls), use the width of the adjoining purlin for the "Max Beam Span" (which would is typically no more than 8' wide).



Typical Mono-Slab Foundation

Mono-Slab Turndown Footing Notes:

1) "x1" and "x2" dimensions may be switched to accomodate for field conditions.

2) Slope of grade to be ≤ 10° with the continuity remaining for at least 24" from edge of slab. Under these conditions the slab may be 4" above grade. If the grade is greater than 10° then a minimum 12" x 12" footer with (2) #5 rebars is to be used with a minimum of 12" below grade.

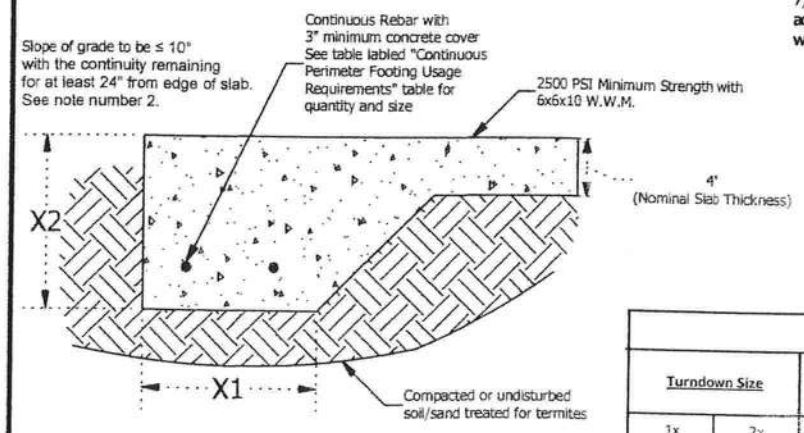
3) Row number 1 in the "Mono-Slab with Turndown Footing Requirements" table represents a monolithic slab with no turndown footing.

4) If the span capabilities and the upright height capabilities are in conflict, the larger footer is to be used

5) Reinforcement bars to be grade 60 steel

6) Footer size is determined by using the average beam span or upright height of the adjoining roof or wall respectively

7) To determine "span" of side walls (non-bearing walls), use the width of the adjoining purlin for the "Max Beam Span" (which would is typically no more than 8' wide).



Turndown Footing Detail

Typical 4" Mono-Slab

Tributary Load Width of Columns	Max. Projection
4'	18.35'
5'	14.67'
6'	12.23'
7'	10.49'
8'	9.18'

* 12'-0" Max. Upright Height

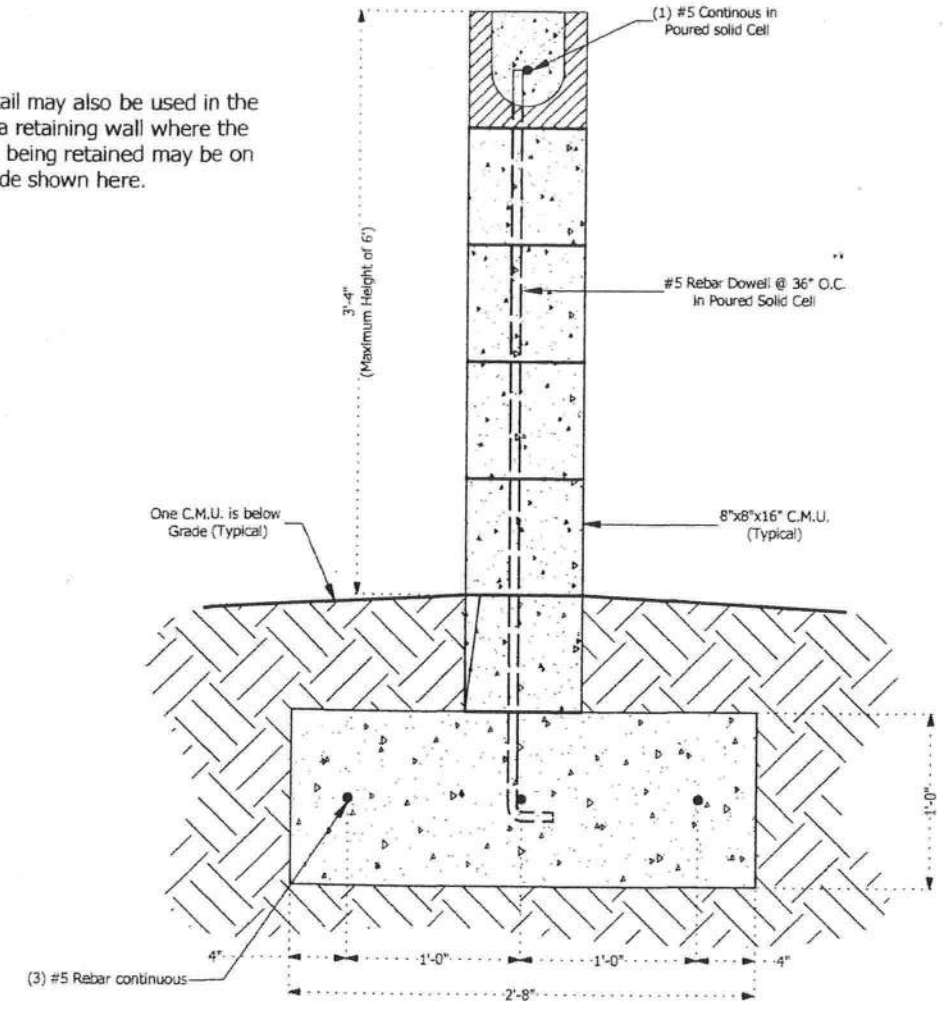
Mono Slab with Turndown Footing Requirements

Turndown Size		Rebar Size & Qty		Tributary Load Width of Columns	Max. Projection	Max. Upright Height
1x	2x	#4	#5	4'	41.05'	16.0'
8"	8"	2	1	5'	36.72'	16.0'
				6'	33.52'	16.0'
				7'	31.03'	16.0'
				8'	29.03'	16.0'

Mono Slab with Turndown Footing Requirements

Turndown Size		Rebar Size & Qty		Tributary Load Width of Columns	Max. Projection	Max. Upright Height
1x	2x	#4	#5	4'	55.08'	16.0'
12"	12"	3	2	5'	49.26'	16.0'
				6'	44.97'	16.0'
				7'	41.63'	16.0'
				8'	38.94'	16.0'

Note:
This detail may also be used in the case of a retaining wall where the material being retained may be on either side shown here.



Knee Wall / Retaining Wall Section
SCALE: 1" = 1'-0"

If this box is circled then the foundation for the screen enclosure or screen room is pre-existing and has been verified by the contractor that it meets structural requirements and is in good condition. See Note #2 in Design Statement.

Existing Foundation

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DATE 1/25/11

Sheet: D2

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.

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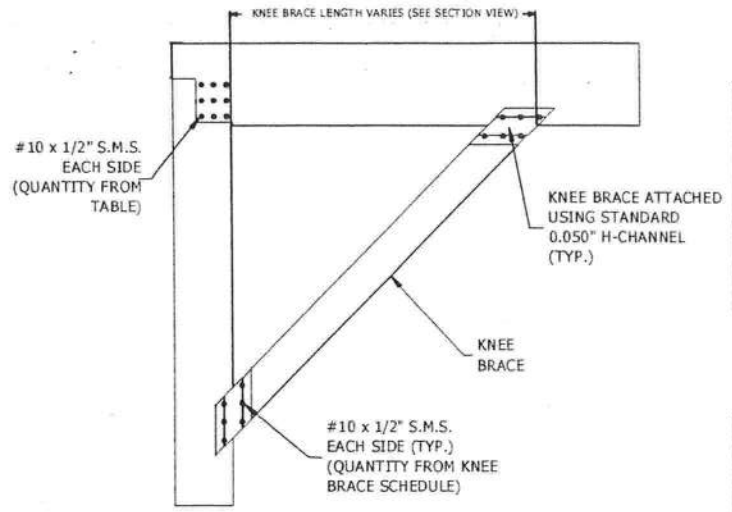


BEAM TO COLUMN

Number of #10 S.M.S. Required

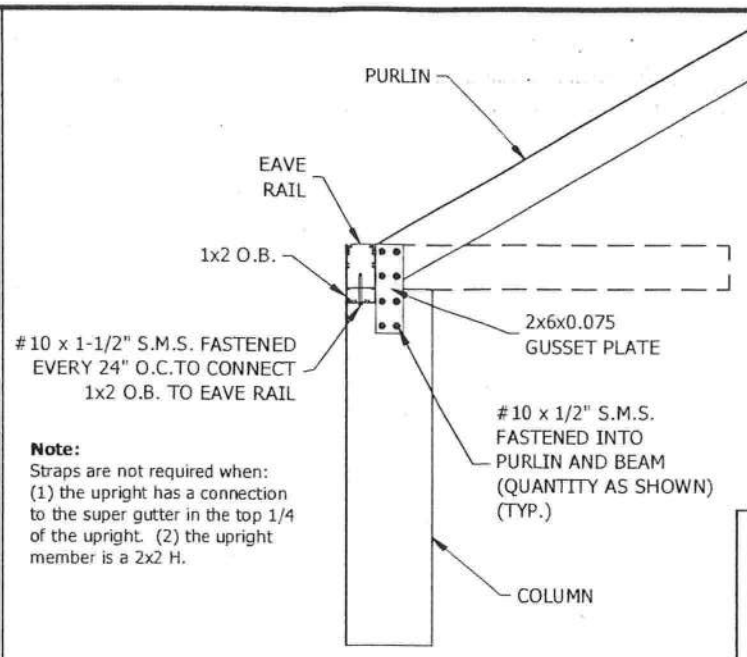
Upright Size \ Beam Size	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	2	4	6	6	8	10	12
2x5	3	6	9	9	12	15	18
2x6	4	8	12	12	16	20	24
2x7	4	8	12	12	16	20	24
2x8	5	10	15	15	20	25	30
2x9	6	12	18	18	24	30	36
2x10	7	14	21	21	28	35	42

Number of #14 S.M.S. Required

Upright Size \ Beam Size	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	N/A	2	2	3	4	5	6
2x5	2	2	4	4	6	6	8
2x6	2	4	6	8	10	12	14
2x7	3	4	8	8	10	12	14
2x8	4	6	10	10	12	14	16
2x9	5	6	12	12	14	16	18
2x10	6	8	14	14	16	18	20

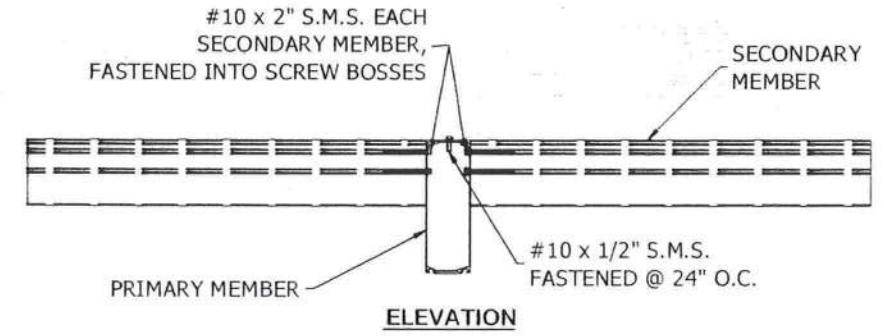
Knee Brace Schedule

Size	Length	Qty Per Flange
2x2x0.044	Up to 2'	2
2x3x0.050	2' to 4'	3
2x4x0.050	4' to 6'	4
2x4x 0.048x 0.100 SMB	6' to 7'	4
2x6x 0.050x 0.120 SMB	7' to 8'	6



Note:
Straps are not required when: (1) the upright has a connection to the super gutter in the top 1/4 of the upright. (2) the upright member is a 2x2 H.

PURLIN TO COLUMN



ELEVATION

Internal Fastening Connection

BEAM SCHEDULE

Beam Member Type \ Tributary Width	4'	5'	6'	7'	8'
	Effective Span (Ft.)				
2x4 SMB	20.4	18.9	17.3	16.0	14.9
2x5 SMB	27.3	24.4	22.3	20.6	19.3
2x6 SMB	31.8	28.9	26.4	24.4	22.8
2x7 SMB	36.2	33.3	30.4	28.1	26.3
2x8 SMB	47.1	43.7	41.2	39.1	37.3
2x9 SMB	55.9	51.9	48.8	46.4	44.4
2x10 SMB	60.0	59.9	56.4	53.5	51.2
2x5 TFB	28.5	26.0	23.7	22.0	20.5
2x7 TFB	38.5	35.7	33.4	30.9	28.9
2x9 TFB	51.1	47.5	44.7	42.4	40.6

Notes:
1) Spans may be interpolated but not extrapolated
2) For anything smaller than a 2x4 (i.e. a 2x2 or 2x3) use the purlin table to determine spans.

Purlin Schedule (up to 130 MPH / Exposure B)

Member \ Tributary Width (ft)	4	4.5	5	5.5	6	6.5	7	7.5	8
	Effective Span (ft)								
2x2x0.044	8.0	8.0	8.0	8.0	8.0	8.0	7.9	7.7	7.5
2x3x0.050	13.7	13.1	12.8	12.4	12.0	11.7	11.4	11.1	10.8
2x3x0.060	15.3	14.7	14.3	13.8	13.4	12.9	12.4	11.9	11.4
2x4x0.050	17.0	17.3	16.8	16.1	15.8	15.1	14.4	13.7	13.0

Chair Rail Schedule

Member \ Tributary Width	4'	5'	6'	7'	8'
	Effective Span (Ft.)				
2x2x0.045	8.5	7.8	7.6	7.1	6.8
2x3x0.050	11.0	10.4	9.7	9.1	8.7
2x3x0.060	12.7	11.8	11.1	10.6	10.1
2x4x0.050	13.6	12.7	11.7	11.2	10.5
2x5x0.050	15.0	13.4	12.2	11.3	10.6

Column Schedule

Column Member Type \ Tributary Width	4.0'	4.5'	5.0'	5.5'	6.0'	6.5'	7.0'	7.5'	8.0'
	Effective Height (Ft.)								
2x2 H	6.2	5.8	5.5	5.1	4.8	4.6	4.2	3.9	3.3
2x3 H	8.1	7.8	7.3	6.9	6.7	6.4	6.1	5.9	5.5
3x2x0.070 H	9.7	9.1	8.8	8.4	8.0	7.8	7.2	7.0	6.4
2x4 H	10.7	10.0	9.7	9.2	8.8	8.6	7.9	7.7	7.0
2x4 SMB	13.9	13.1	12.4	11.8	11.3	10.9	10.5	10.1	9.8
2x5 SMB	17.2	16.2	15.3	14.6	14.0	13.5	13.0	12.5	12.1
2x6 SMB	19.6	18.5	17.6	16.7	16.0	15.4	14.8	14.3	13.9
2x7SMB	22.0	20.7	19.7	18.8	18.0	17.3	16.6	16.1	15.5
2x8 SMB	30.3	28.9	27.4	26.1	25.0	24.0	23.2	22.4	21.7
2x9 SMB	36.0	34.6	33.2	31.6	30.3	29.1	28.0	27.1	26.2
2x10 SMB	41.5	39.9	38.5	37.0	35.4	34.0	32.6	31.7	30.7
2x5 TFB	18.2	17.1	16.3	15.5	14.8	14.2	13.7	13.3	12.8
2x7 TFB	24.2	22.8	21.6	20.6	19.7	18.9	18.2	17.6	17.1
2x9 TFB	32.8	30.9	29.3	27.9	26.7	25.7	24.8	23.9	23.2

This table applies to bearing and non-bearing walls for horizontal wind loads in design. Heights may be interpolated but not extrapolated

Carry Beam Schedule for Screen Enclosure Roofs

Member Type \ Screen Roof Tributary Width	8'	12'	20'	24'	30'	34'	40'	44'	48'	52'
	Effective Span (Ft.)									
2x4 SMB	16.4	14.1	11.7	10.9	9.6	9.1	8.4	8.0	7.7	7.3
2x5 SMB	20.1	17.4	14.7	13.7	12.3	11.5	10.7	10.2	9.8	9.3
2x6 SMB	22.4	20.4	17.2	15.8	14.1	13.2	12.2	11.7	11.2	10.7
2x7 SMB	25.4	23.4	19.5	18.3	16.4	15.3	14.2	13.5	12.9	12.4
2x8 SMB	31.6	28.7	24.1	22.7	20.9	19.6	18.2	17.3	16.5	15.9
2x9 SMB	34.3	31.6	26.6	25.0	22.7	21.3	19.7	18.8	18.1	17.3
2x10 SMB	42.7	39.3	33.0	31.0	28.6	27.5	25.9	24.7	23.6	22.7
2x5 TFB	24.1	22.2	18.5	17.4	15.6	14.5	13.5	12.8	12.3	11.8
2x7 TFB	30.0	27.3	22.9	21.6	19.9	18.6	17.3	16.4	15.7	15.1
2x9 TFB	36.3	33.4	28.1	26.4	24.3	23.4	22.0	21.0	20.1	19.3

Spans may be interpolated but not extrapolated

Notes:
1) Beam member size is chosen by the span between the knee braces.

2) If there are no knee braces then the span is chosen by the distance of the beam length minus half the depth of each upright. (i.e. - Beam Length - (half depth of Upright 1 + half depth of upright 2)).

3) Knee Brace Length shall not exceed what is given in the Knee Brace schedule (i.e. 8' in horizontal or vertical distance) unless the site-specific layout has been approved and sealed by a licensed professional engineer.

4) Upright member size is chosen by the span from the ground to the bottom of the knee brace.

5) Chair rails that are attached to uprights through the web of the upright into every available screw boss of the chair rail, may be used as Guard Rails in screen enclosures provided that they are set at a height of between 18" & 48" and that they span no greater than what the Chair Rail Schedule' identifies for each respective member.

6) The span tables shown are calculated for exposure B type terrain. If exposure C is needed, then use the multipliers as follows: With a height range of [0'-15': multiply span by 0.83 or fasten Self-Mating Beams together @ 12" O.C.], [15'-20': multiply span by 0.78 or fasten Self-Mating Beams together @ 12" O.C.], [20'-25': multiply span by 0.74 or fasten Self-Mating Beams together @ 8" O.C.], [25'-30': multiply span by 0.71 or fasten Self-Mating Beams together @ 6" O.C.].

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DATE 1/25/11

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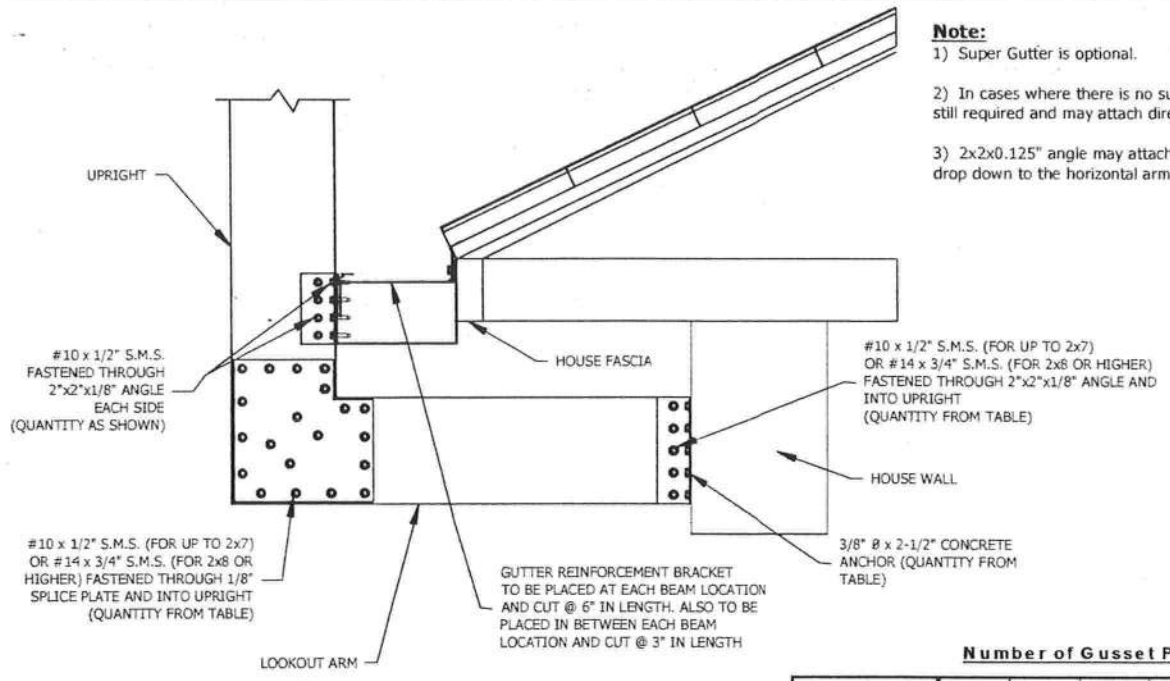
ENGINEER - FL. LIC. 35816

Sheet: D3

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



Note:

1) Super Gutter is optional.

2) In cases where there is no super gutter, the 2x2x0.125" angle is still required and may attach directly to the fascia.

3) 2x2x0.125" angle may attach to either the vertical upright or drop down to the horizontal arm from the fascia or super gutter.

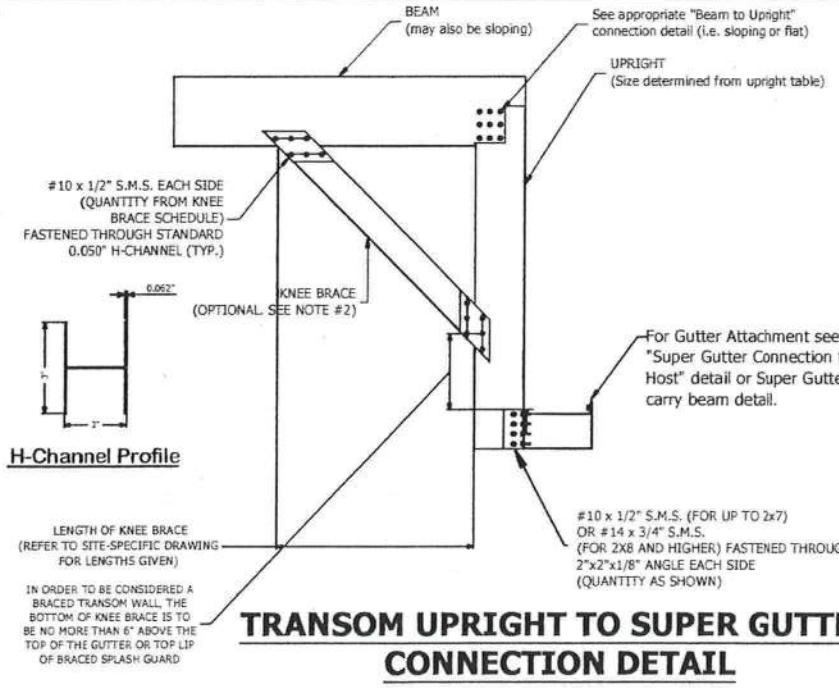
LOOKOUT ARM CONNECTION DETAIL

Number of Gusset Plate Screws

Upright Size	2x4	2x5	2x6	2x7	2x8	2x9	2x10
Qty of Screws	16	18	20	24	26	30	30

Number of Fasteners to Angle Clip

Upright Size	2x4	2x5	2x6	2x7	2x8	2x9	2x10
Qty of Screws	3	4	5	6	7	8	9



H-Channel Profile

Knee Brace Schedule

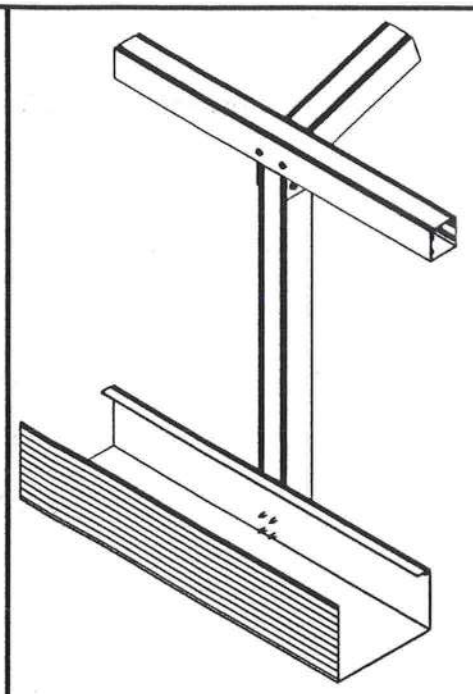
Size	Length	Qty Per Flange
2x2x0.044	Up to 2'	2
2x3x0.050	2' to 4'	3
2x4x0.050	4' to 6'	4
2x4x 0.048x 0.100 SMB	6' to 7'	4
2x6x 0.050x 0.120 SMB	7' to 8'	6

Note:

1) Super Gutter may be 5" or 7".

2) The Knee Brace (K.B.) is optional and is only used to qualify a transom wall as being braced to the host structure. When the transom wall is braced to the host structure, requirements for diagonal bracing in the roof change accordingly. See the detail labeled "Diagonal Roof Bracing Plan" for diagonal roof bracing rules.

TRANSOM UPRIGHT TO SUPER GUTTER CONNECTION DETAIL



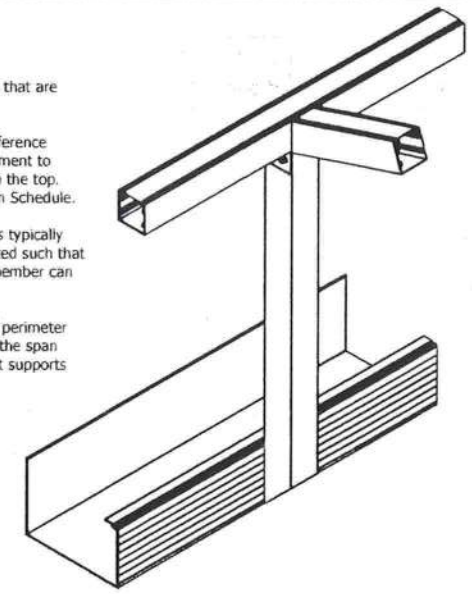
Notes:

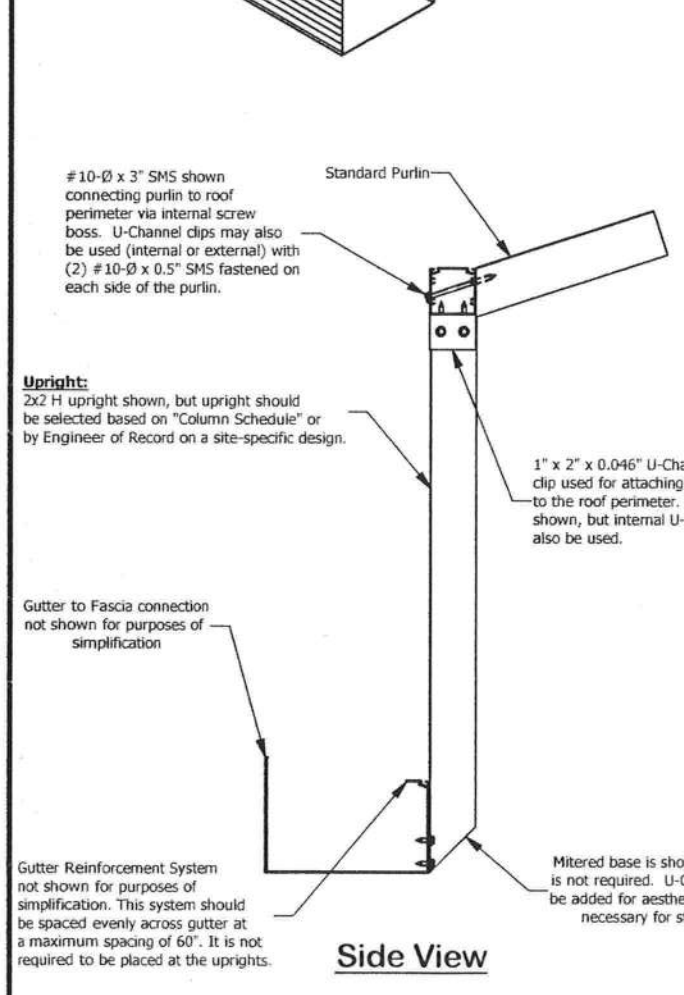
1) This connection is only good for non-load bearing transom walls that are 60" in height or less from the top lip of the super gutter.

2) For "non-load bearing" transom walls that are taller in height, reference the "Transom Upright to Super Gutter Connection Detail" for attachment to the super gutter and the "Purlin to Column" detail for attachment to the top. Upright member size would also be determined by using the Column Schedule.

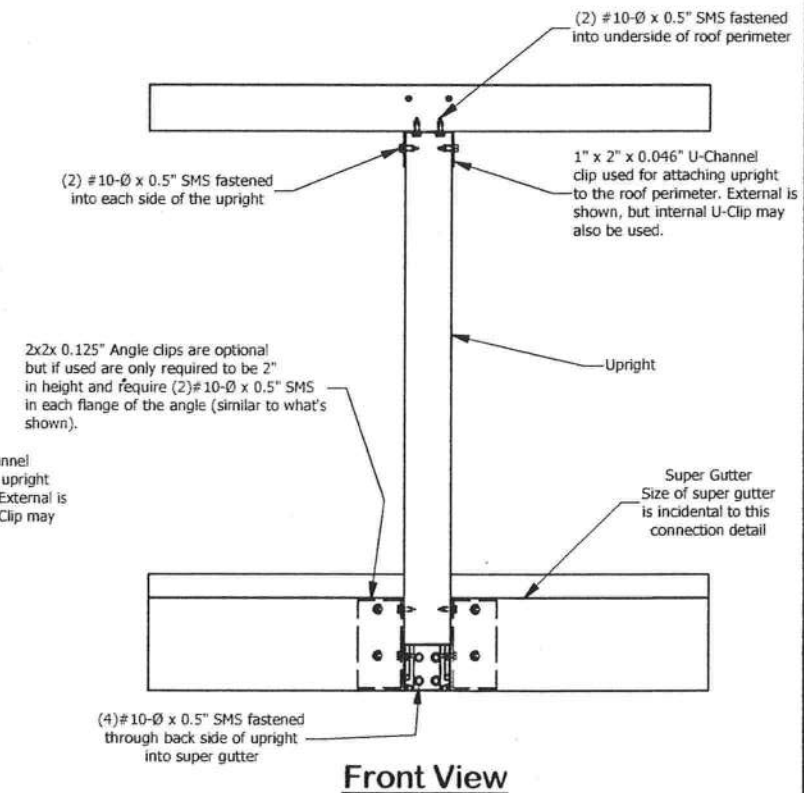
2) The upright is shown lining up with a purlin in this detail. This is typically how it's installed, but is not required. The uprights need to be spaced such that the span between them does not exceed what the roof perimeter member can span according to the "Beam Schedule" table.

3) This type of connection is used to break up the span of the roof perimeter member. If the roof perimeter member is large enough to support the span on its own (i.e. a 2x4 SMB spanning a 12' distance) then the upright supports are not necessary and this detail is not required.





Side View



Front View
(From Inside Cage - Without Purlin Shown)

NON-LOAD BEARING TRANSOM WALL CONNECTION

DAVIS & CLEATON ENGINEERING, INC.
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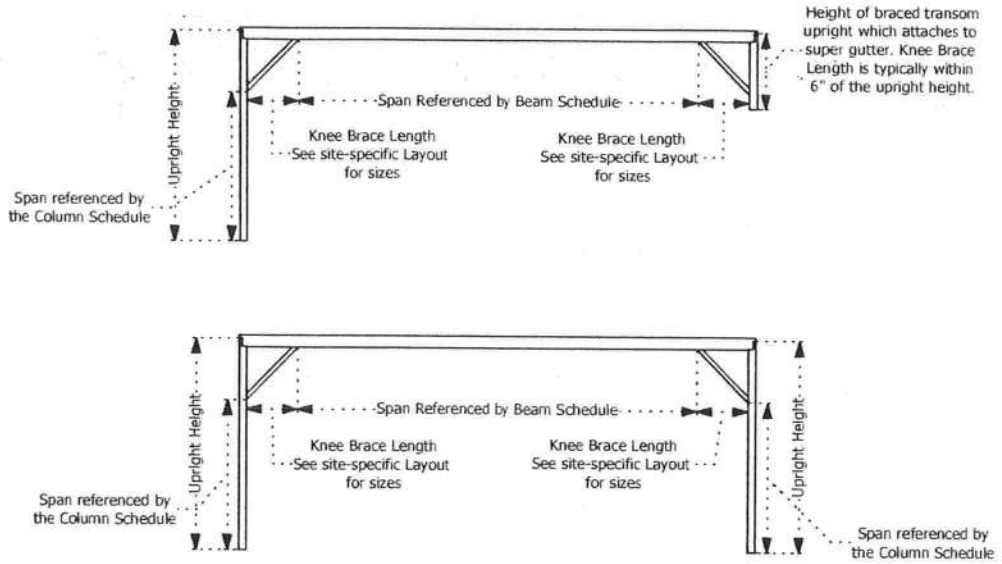
DATE 1/25/11

FRANK CLEATON, JR, P.E.
ENGINEER - FL. LIC. 35816

Sheet: D4

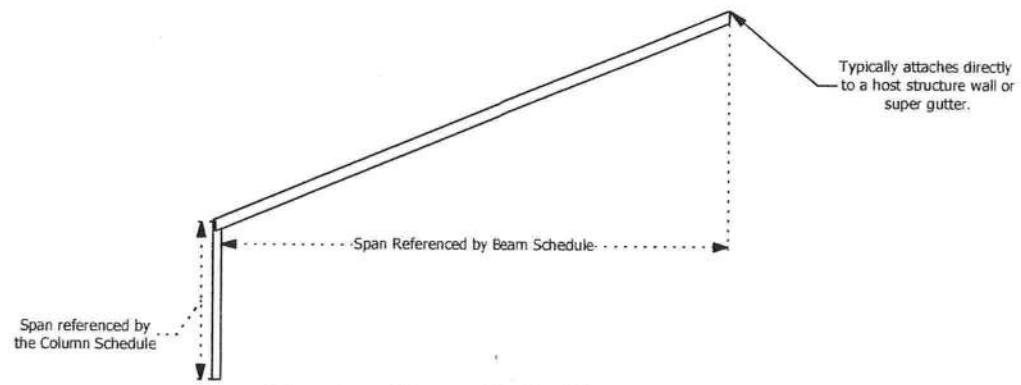
Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



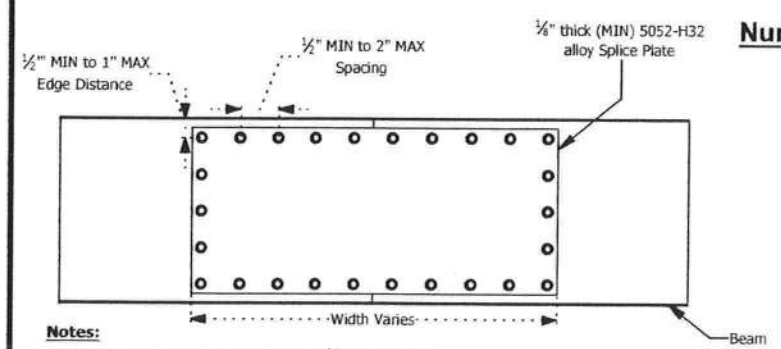
Flat Beam Span Definitions

SCALE: 3/32" = 1'-0"



Sloping Beam Definition

SCALE: 3/32" = 1'-0"



Notes:

1) Splice plate locations are to be located ¼" the distance of the beam.
2) Splice plate locations on both sides of the beam may be in the same location or on opposite ends of each other.
3) Splice plates may be on the inside or outside of the beam.

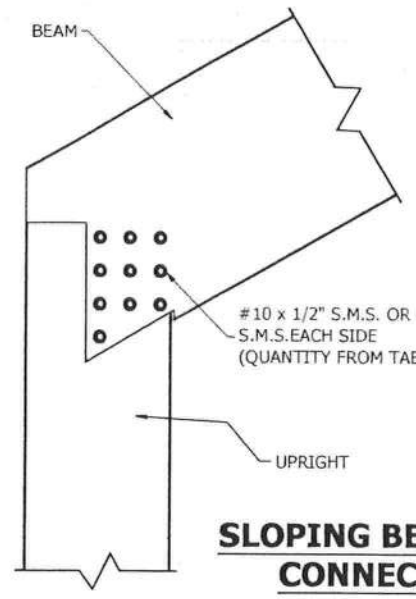
Number of Screws Required

Beam Size	# Screws
2x4-2x6	12
2x7	16
2x8	18
2x9	20
2x10	22

Length of Splice Plate

Beam Size	Length
2x4	8 in
2x5	10 in
2x6	12 in
2x7	14 in
2x8	16 in
2x9	18 in
2x10	18 in

Straight In-Line Splice Connection



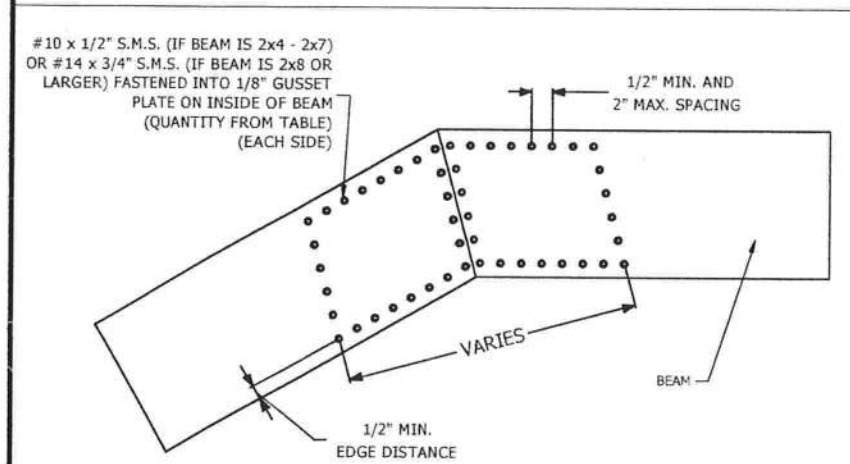
SLOPING BEAM TO UPRIGHT CONNECTION DETAIL

Quantity of #10 Fasteners Required

Upright size \ Beam Size	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	N/A	3	5	7	9	11	13
2x5	N/A	4	7	10	13	16	19
2x6	N/A	4	8	12	16	20	24
2x7	N/A	4	9	14	18	22	27
2x8	N/A	4	9	14	18	22	27
2x9	N/A	4	9	14	19	25	31
2x10	N/A	4	9	14	20	27	34

Quantity of #14 Fasteners Required

Upright size \ Beam Size	2x4	2x5	2x6	2x7	2x8	2x9	2x10
2x4	N/A	3	5	7	7	9	11
2x5	N/A	3	6	9	10	13	16
2x6	N/A	3	6	9	9	12	16
2x7	N/A	3	7	11	12	15	19
2x8	N/A	3	6	10	11	15	19
2x9	N/A	3	6	12	13	17	22
2x10	N/A	3	7	12	14	19	25



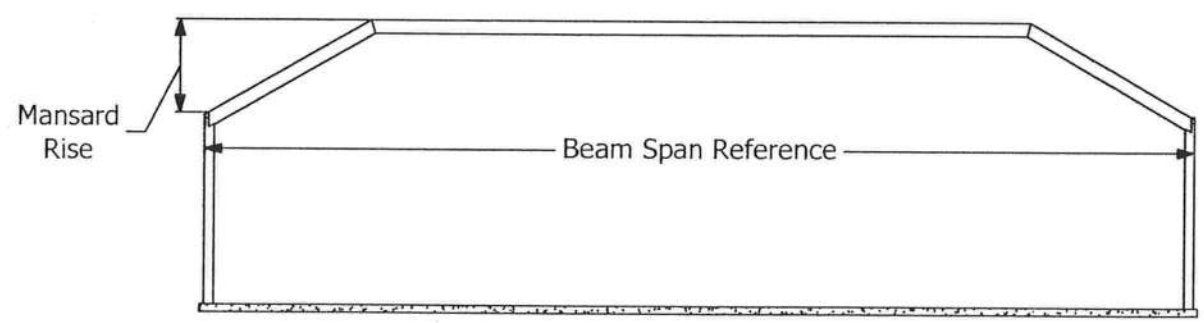
MANSARD SPLICE DETAIL

Number of Screws Required

Beam Size	# Screws
2x4-2x6	12
2x7	16
2x8	18
2x9	20
2x10	22

Length of Splice Plate

Beam Size	Length
2x4	8 in
2x5	10 in
2x6	12 in
2x7	14 in
2x8	16 in
2x9	18 in
2x10	18 in



Mansard Beam Span Definition

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DATE 1/25/11

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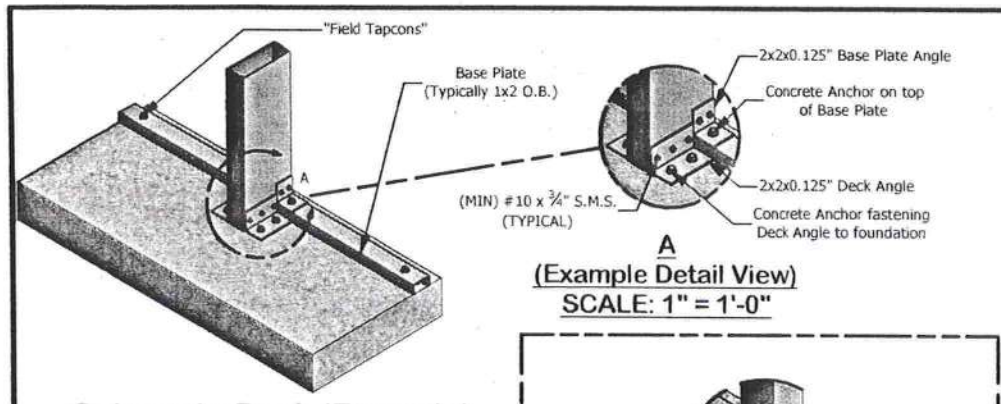
Sheet: D5

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



A
(Example Detail View)
SCALE: 1" = 1'-0"

Column to Deck (Example)
SCALE: 3/4" = 1'-0"

Notes:

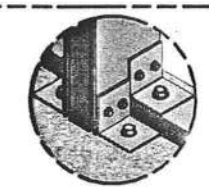
1) "Field Anchors" are to be ¼" Ø tapcon with a 1 ¼" minimum embedment into concrete foundation. Max Spacing is 48" O.C.

2) Unless specified on a sealed site-specific drawing, minimum edge distance from the center of the tapcon to the edge of the concrete deck is 2 ½" or 5 times the embedment depth; which ever is greater. Minimum embedment depth is 1 ¼" into concrete foundation.

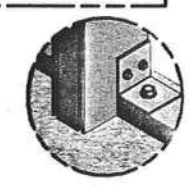
3) Quantity of fasteners for both sheet metal screws and tapcons is to match what is shown in the detail views below. Pattern of sheet metal screws may be different than what is shown as long as Sheet metal screws maintain a miniumum spacing of ¾" and a maximum spacing of 1 ¾" with a minimum ⅜" edge distance. Tapcon patterns may not differ from what is shown.

4) Sheet metal screws are to be a minimum of #10 Ø x ¾". The larger S.M.S. may be used, but the quantity is to remain the same.

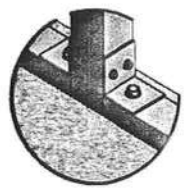
5) Columns may sit on top of brick pavers but the tapcons must penetrate through the bricks and into the concrete foundation beneath with the minimum embedment of 1 ¼".



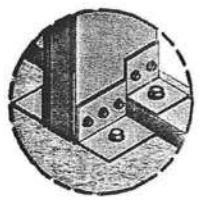
2x4 Column (1/4" Anchors)
SCALE: 1 1/2" = 1'-0"



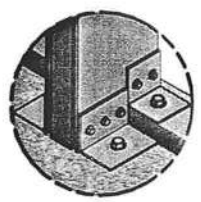
2x4 Column (3/8" Anchors)
SCALE: 1 1/2" = 1'-0"



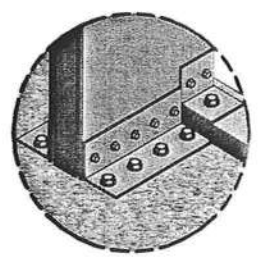
2x2 Column (1/4" Anchors)
SCALE: 1 1/2" = 1'-0"



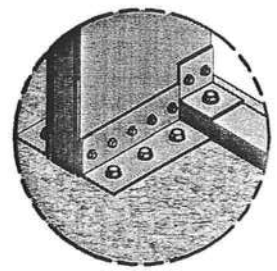
2x5 Column (1/4" Anchors)
SCALE: 1 1/2" = 1'-0"



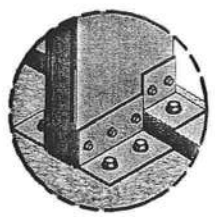
2x5 Column (3/8" Anchor)
SCALE: 1 1/2" = 1'-0"



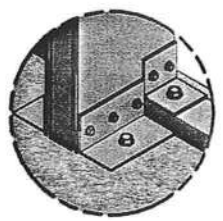
2x8 Column (1/4" Anchors)
SCALE: 1 1/2" = 1'-0"



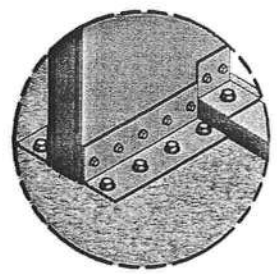
2x8 Column (3/8" Anchors)
SCALE: 1 1/2" = 1'-0"



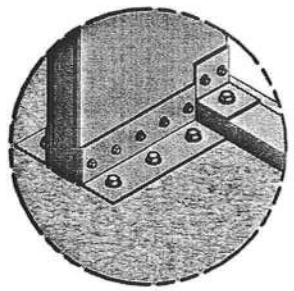
2x6 Column (1/4" Anchors)
SCALE: 1 1/2" = 1'-0"



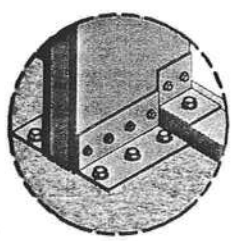
2x6 Column (3/8" Anchors)
SCALE: 1 1/2" = 1'-0"



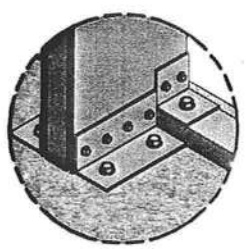
2x9 Column (1/4" Anchors)
SCALE: 1 1/2" = 1'-0"



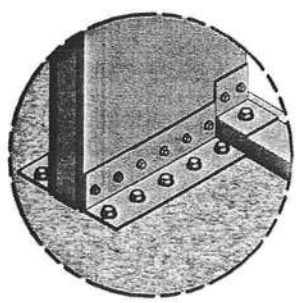
2x9 Column (3/8" Anchors)
SCALE: 1 1/2" = 1'-0"



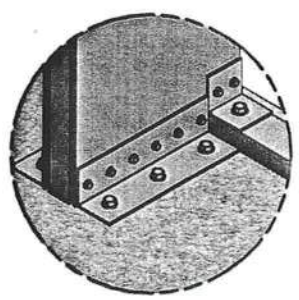
2x7 Column (1/4" Anchor)
SCALE: 1 1/2" = 1'-0"



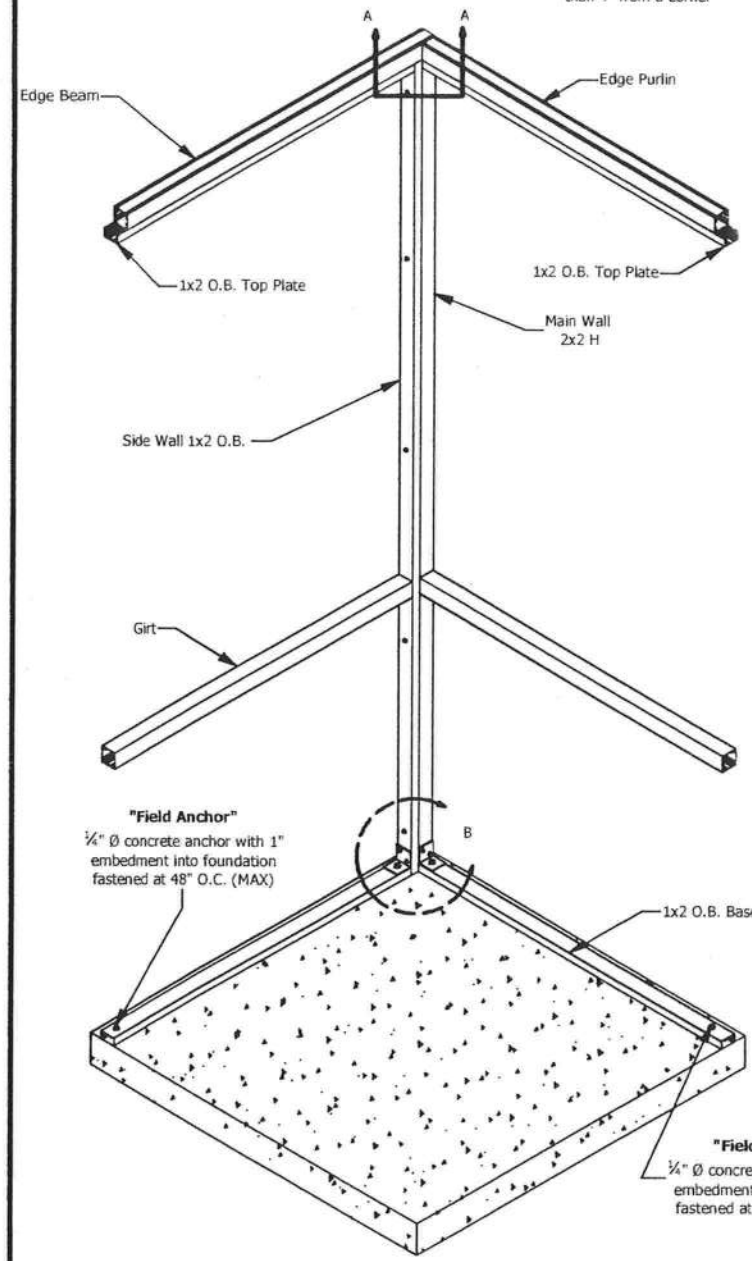
2x7 Column (3/8" Anchors)
SCALE: 1 1/2" = 1'-0"

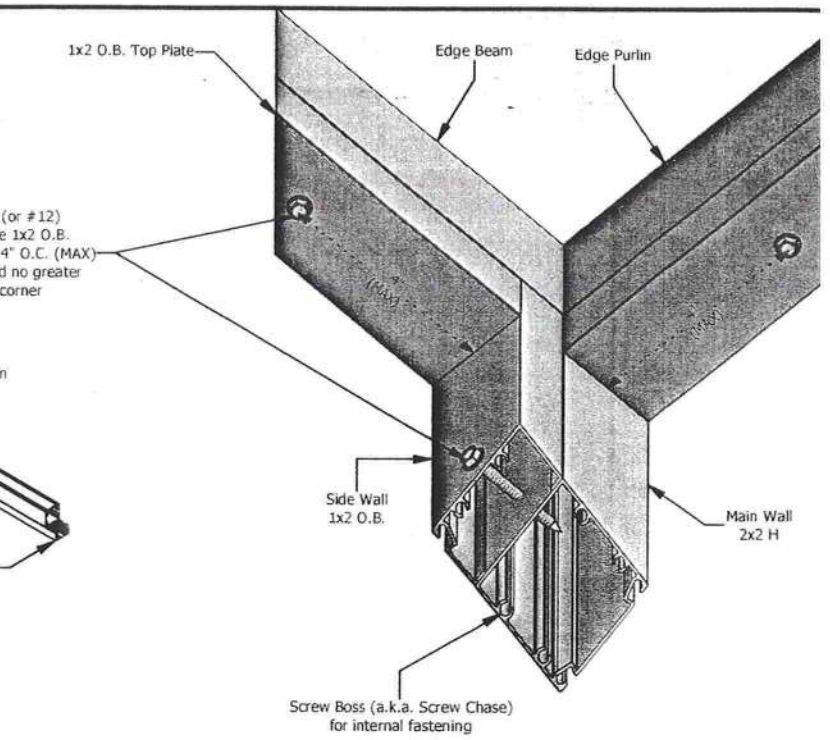


2x10 Column (1/4" Anchors)
SCALE: 1 1/2" = 1'-0"

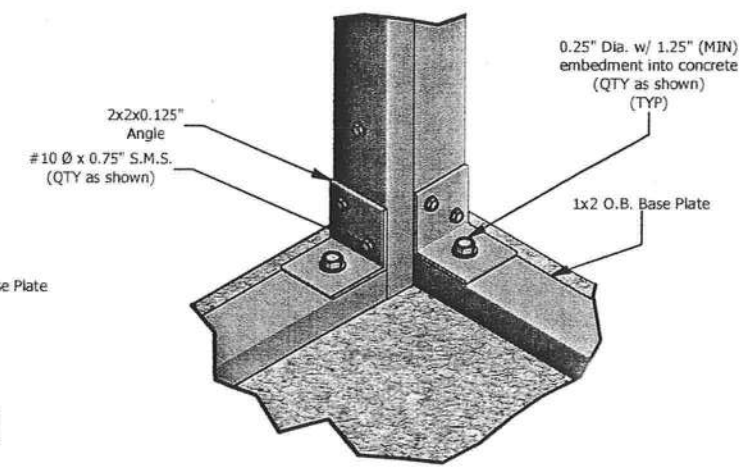


2x10 Column (3/8" Anchors)
SCALE: 1 1/2" = 1'-0"





SECTION: A-A
SCALE: 1:3



B
SCALE: 1:5

Main Wall To Side Wall Connection

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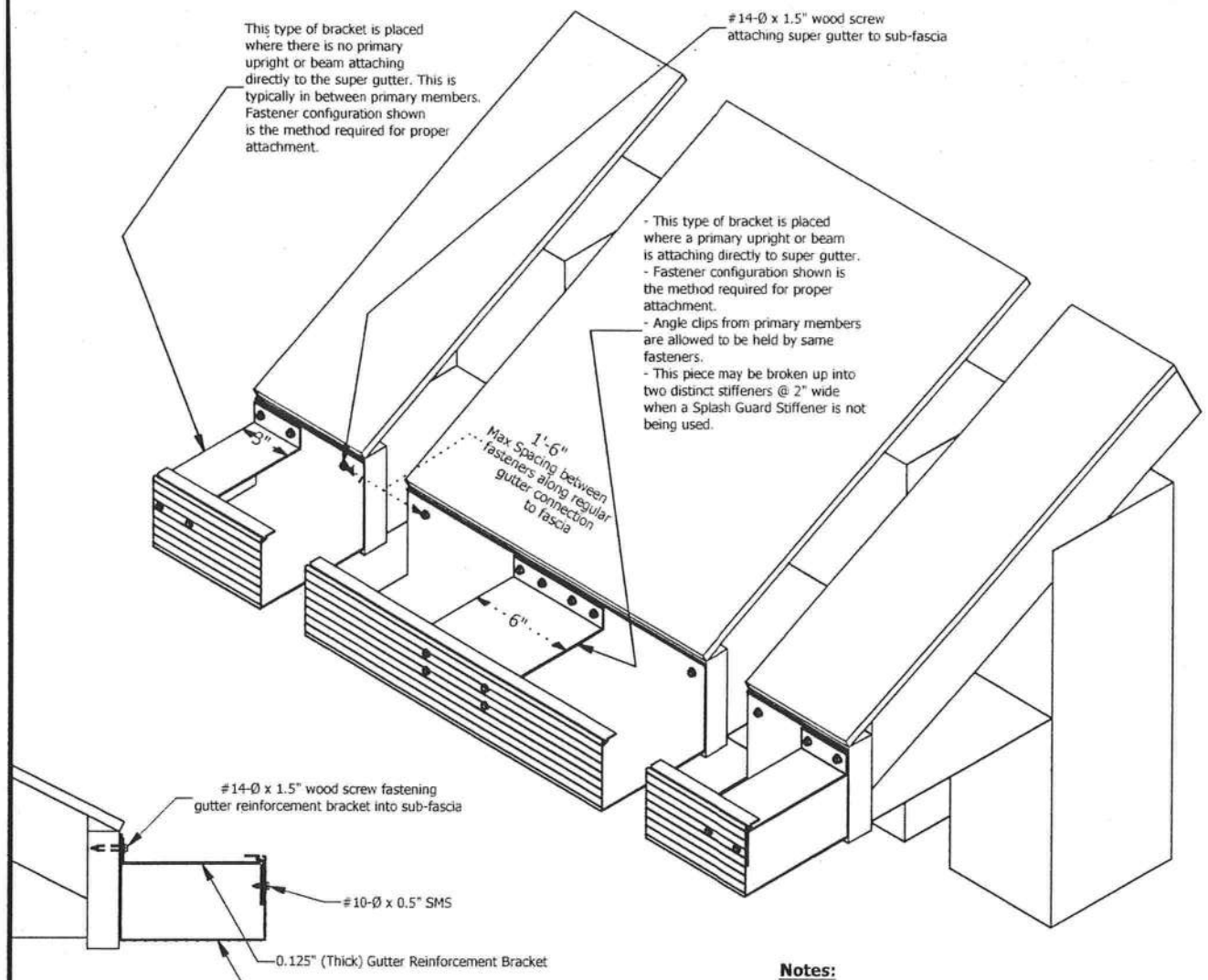
ENGINEER - FL. LIC. 35816

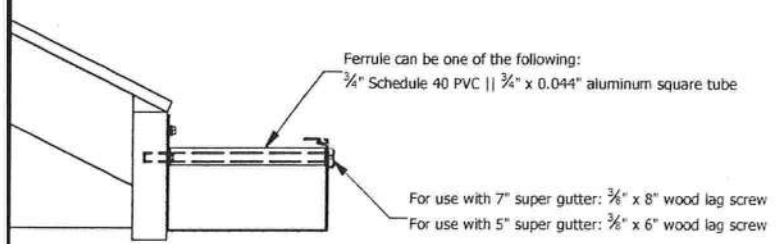
Sheet: D6

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



Standard Gutter Stiffener
(as shown in Isometric view)



Alternate Gutter Stiffener

Notes:

1) Fascia shown is a plumb cut fascia. If the fascia is square cut (perpendicular to the roof pitch) then the super gutter may have something behind it (i.e. "L-Angle" or wood wedge) to keep it plumb. This component is incidental to the structural intention of this design.

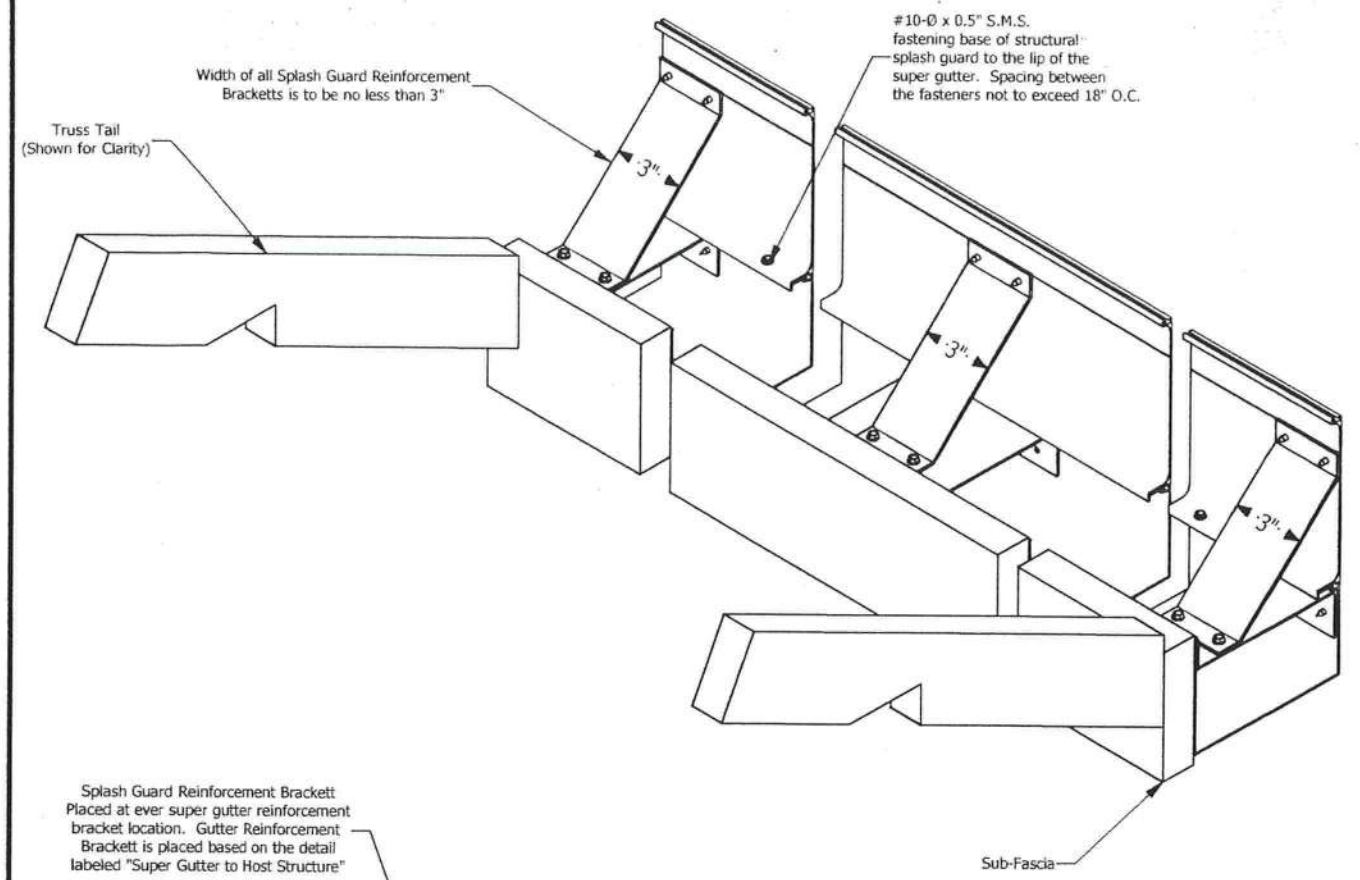
2) For a run of super gutter that non-load bearing, the max spacing of gutter reinforcement brackets is 60".

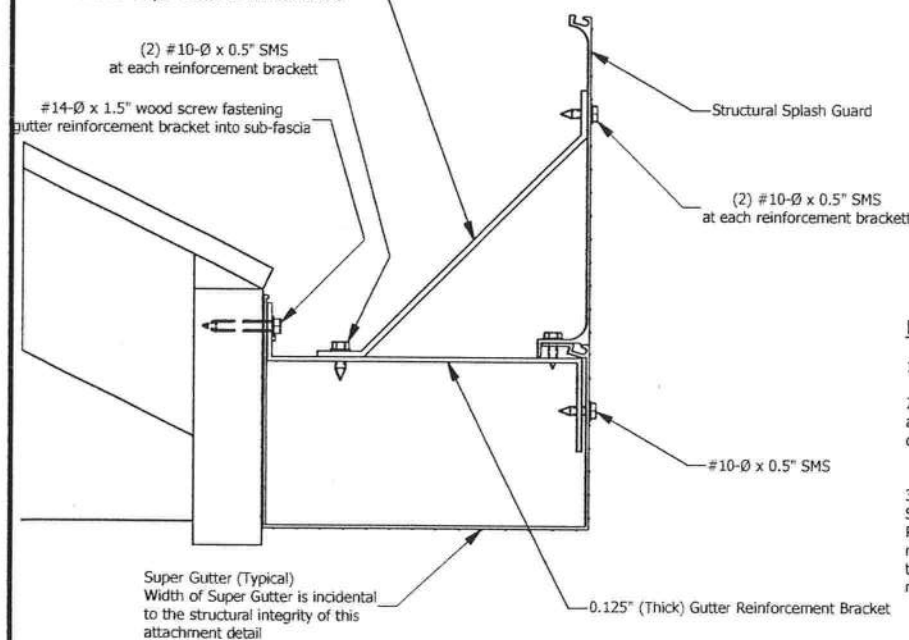
3) For a run of super gutter that has a structural or load bearing component attaching to it and there is a continuous load across the super gutter without primary load points (i.e. beam or upright) then the maximum spacing between the gutter reinforcement brackets is 48".

4) Gutter may be attached to a conventional wood framed wall in the same manner that is shown in this detail. If the wall is a C.M.U. then replace the #14-Ø wood screws with 0.25"-Ø x 1.25" Tapcons (or equivilant). This connection shall also remain the same for a super gutter attaching to a carry beam where the wood lag screws are replaced by #10-Ø x 0.75" S.M.S.

5) When the super gutter is being used for structural purposes (i.e. structural support members attach to it) then the fasteners that attach to the fascia must penetrate into the sub-fascia at a minimum of 1 in. embedment.

Super Gutter Connection to Host Structure





Notes:

1) All notes from the detail labeled "Super Gutter to Host Structure" carry over here.

2) The Structural Splash Guard is an option used to create a greater area for attachment of structural members as well as provide splash-over protection for certain types of roofs.

3) If the contractor circles this note item, then it is not their intention to use the Structural Splash Guard for structural purposes and may omit the Splash Guard Reinforcement Bracket from their installation. If this is the intention, then they may not use the Structural Splash Guard to provide support for frame members of any type. Alternatively, the contractor may attach the Structural Splash Guard to frame members that are already fully supported in order to install the splash guard.

Structural Splash Guard Connection

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DATE 1/25/11

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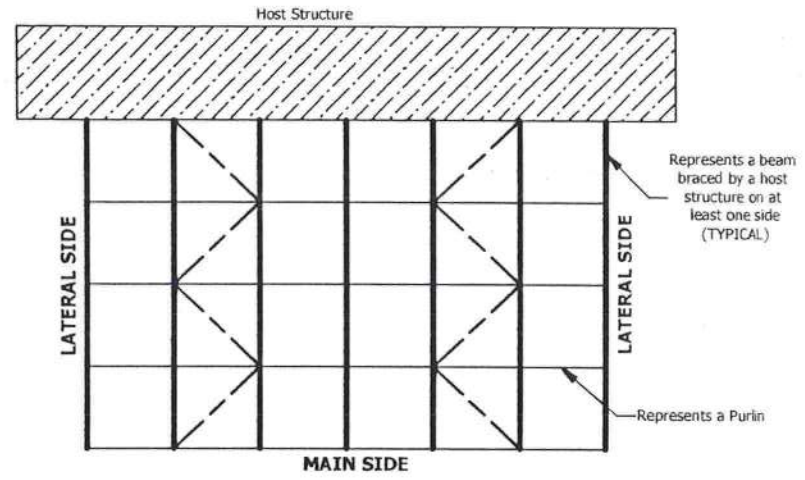
Sheet: D7

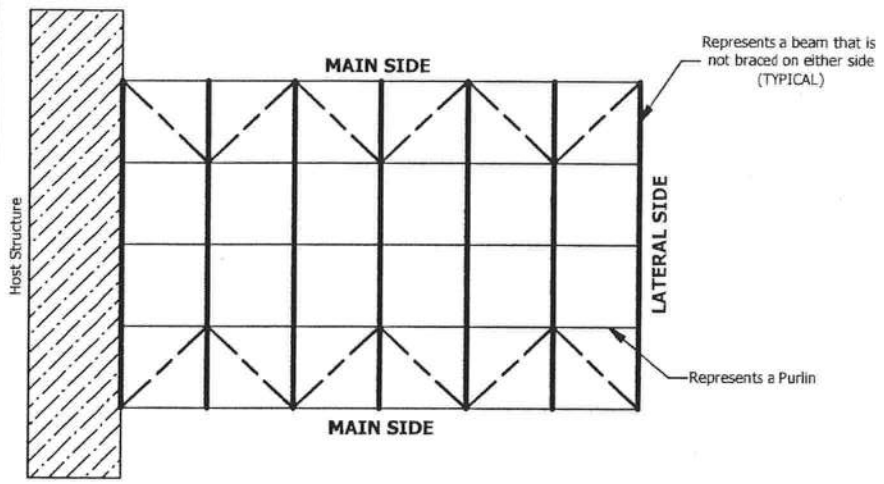
Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



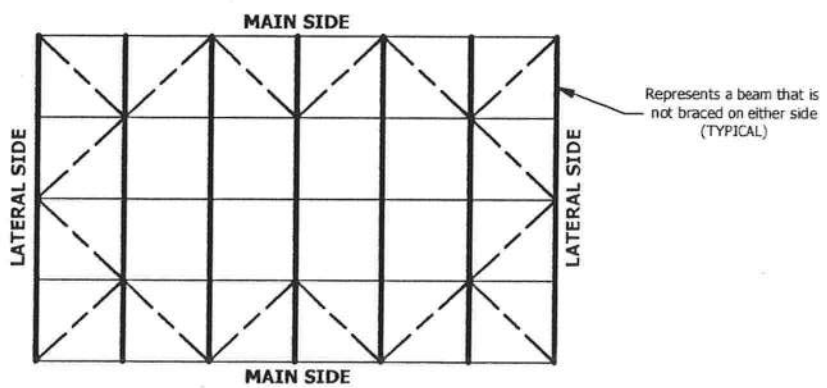
Diagonal Bracing: Braced on Main Side

SCALE: 1/16" = 1'-0"



Diagonal Bracing: Braced On Lateral Side

SCALE: 1/16" = 1'-0"



Diagonal Bracing: All Sides Unbraced

SCALE: 1/16" = 1'-0"

Notes:

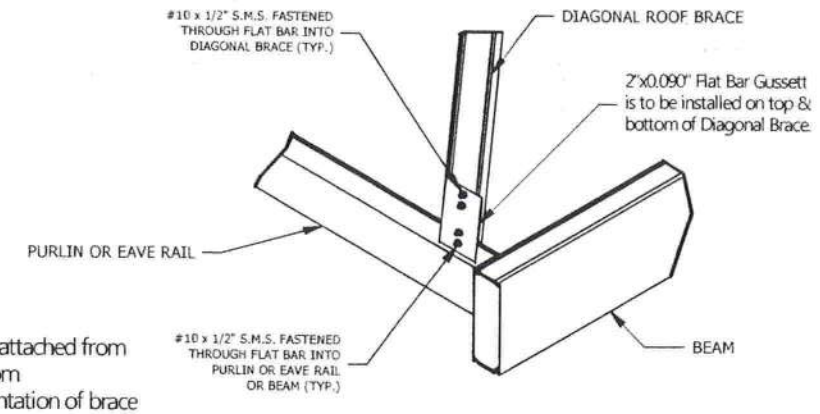
1) Roof bracing may be placed in either the first or second bay on each side of the enclosure where applicable.

2) The orientation (i.e. direction) of each individual diagonal brace is incidental to the structual integrity of the design.

3) If there are an odd number of panels, a brace in the center may be omitted.

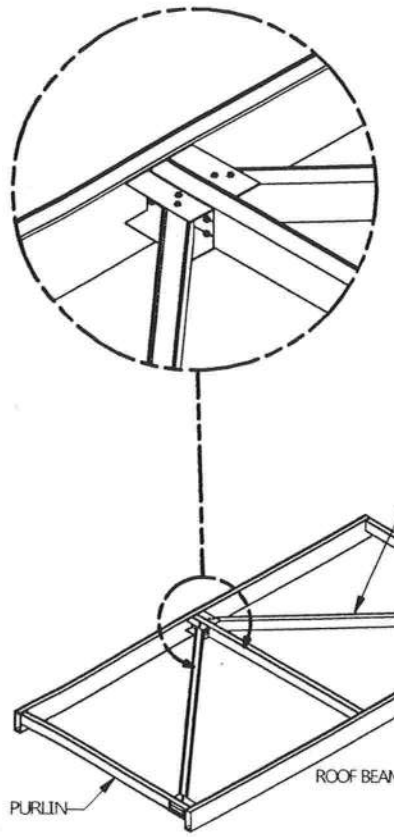
4) Any enclosure that is braced on both a "main side" and a "lateral side" does not require roof bracing.

4) In order for a side (main side or lateral side) to be considered "braced", the beam or row of purlins must terminate into the host structure or be supported by a screen wall no greater than 12" above the host structure. If the wall height is greater than 12" there must be a knee brace going from the roof member down to the upright with the bottom of that knee brace no higher than 6" above the host structure.

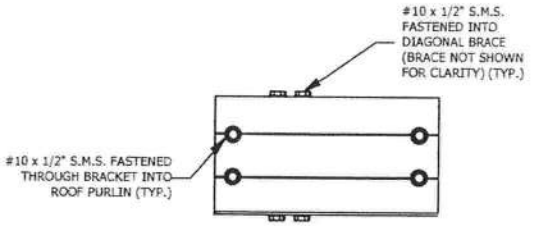


Note:
Roof bracing may be attached from purlin-to-purlin or from beam-to-beam. Orientation of brace is incidental to design.

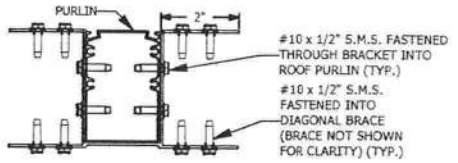
Diagonal Brace Gussett Attachment



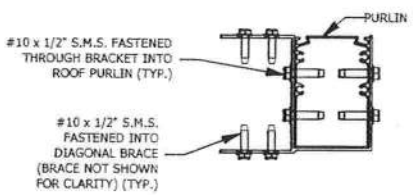
Note:
Roof bracing may be attached from purlin-to-purlin or from beam-to-beam. Orientation of brace is incidental to design.



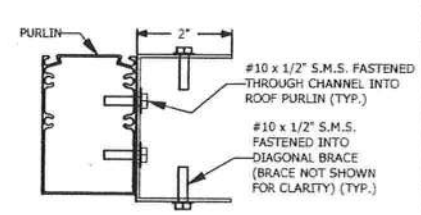
SIDE VIEW (TYP.)



FRONT VIEW (DOUBLE SIDED BRACKET)



FRONT VIEW (SINGLE SIDED BRACKET)



ALTERNATE DIAGONAL BRACE CONNECTION (C-CHANNEL)

Diagonal Brace Channel Bracket

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DATE 1/25/11

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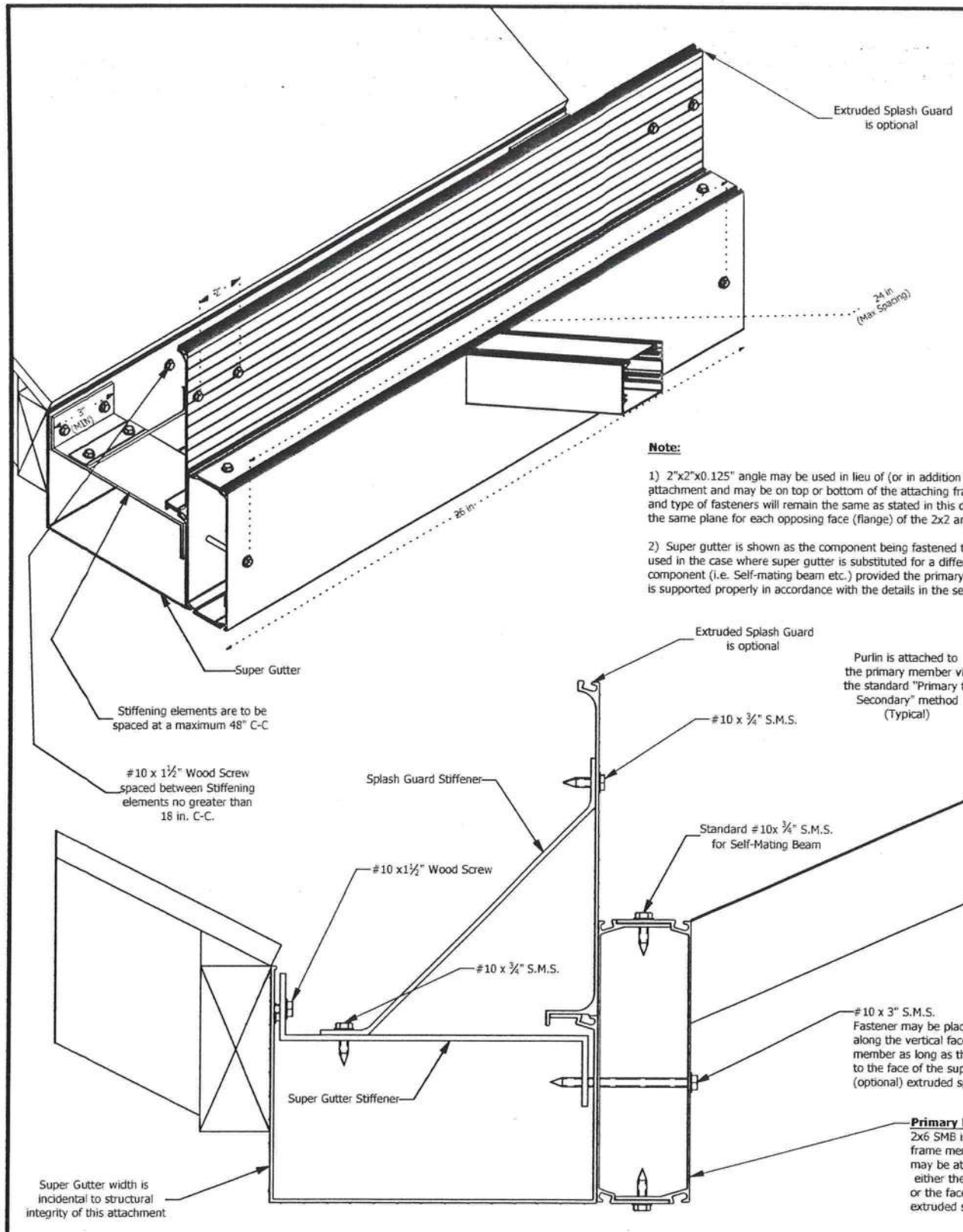
Sheet: D8

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.

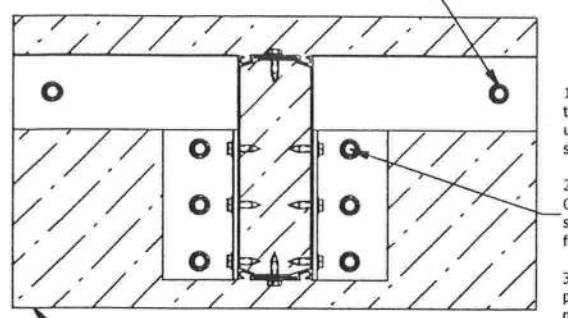


Note:

1) 2"x2"x0.125" angle may be used in lieu of (or in addition to) the #10 x 3" S.M.S. attachment and may be on top or bottom of the attaching frame member. The spacing and type of fasteners will remain the same as stated in this detail. Each fastener will be on the same plane for each opposing face (flange) of the 2x2 angle.

2) Super gutter is shown as the component being fastened to, but this detail can be used in the case where super gutter is substituted for a different aluminum structural component (i.e. Self-mating beam etc.) provided the primary structural component is supported properly in accordance with the details in the sealed engineering package.

Parallel Frame Member to Super Gutter

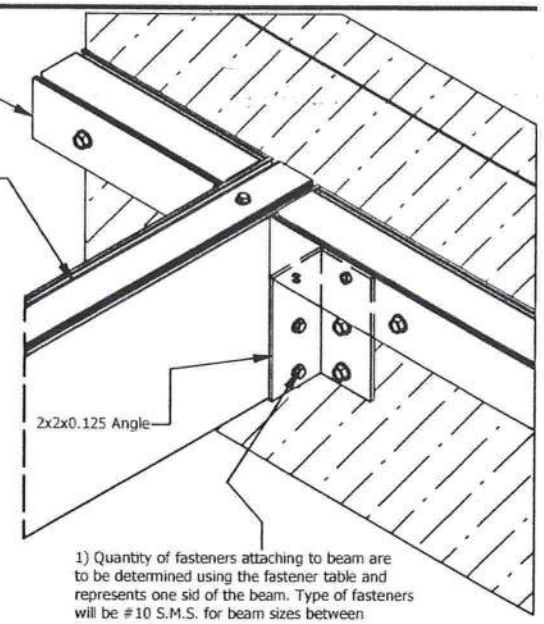


Roof Purlin is typical but may be ommited. Purlin is to be attached through the web of the beam into its internal screw bosses. Member sizes may be as follows: 1x2 O.B., 2x2 H, 2x3 H, 2x4 H, 2x5 H

1) Type & quantity of fasteners attaching to support structure are to be determined using the fastener table and represents one side of the beam.

2) Fasteners shall not be less than 0.75" Center-to-Center from each other nor shall they be greater than 2" Center-to-Center from each other.

3) Fasteners may be arranged in any pattern provided the minimum QTY from the table is met along with the above mentioned spacing requirements.



Note:

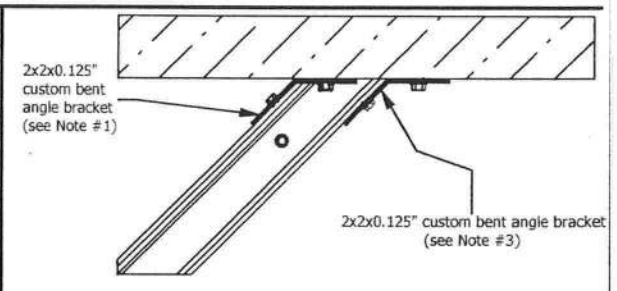
1) U-channel may be used in lieu of 2x2 angle clips on either side of the beam as long as the thickness of the U-Channel being used is greater than or equal to the thickness of the beam to which it is attaching.

2) If there is a purlin attaching to the beam and its presence provides minimal room to achieve the minimum fastener requirements then an additional 2x2x0.125" angle may be used going from the beam to the purlin with the fasteners penetrating the purlin and the angle to attach to the host structure.

Beam to Support Structure

Beam to Host Structure Fastener Requirements

	Minimum Quantity of Specified Fasteners (One side)					
	Fasteners to Beam		Fasteners to C.M.U. Host Structure		Fasteners to Wood Host Structure	
Beam Size	#10 S.M.S.	#14 S.M.S.	0.25"Ø Tapcons	0.375"Ø Tapcons	#10 Wood Screw	#14 Wood Screw
2x2 H	2	2	1	1	2	1
2x3 H	2	2	1	1	2	1
2x4 H	2	2	1	1	2	2
2x4 SMB	2	2	1	1	2	2
2x5 SMB	3	3	1	1	3	2
2x6 SMB	3	3	2	1	3	3
2x7 SMB	4	4	3	2	4	4
2x8 SMB	5	5	4	3	5	5
2x9 SMB	5	5	4	3	5	5
2x10 SMB	6	6	5	3	6	6
	N/A	N/A	1.25"	1.25"	1"	1"
	Minimum Embedment Depths (inches)					



Angled Beam to Support Structure (Plan View)

Notes:

1) Acute side of beam may be notched to allow for the 2x2x0.125" custom bent angle bracket to slide in between the beam and support structure. The fabrication method to achieve this would be to simply attach the angle bracket to the beam prior to installing the beam half-shell, provided the acute side of the beam is installed first. This would allow for the fasteners to be installed into the support structure prior to the installation of the obtuse side of the beam.

2) If the fabrication method shown here cannot be achieved, then a secondary 2x2x0.125" angle must be installed beneath the beam. Type and Qty of fasteners should reference the row labled "2x2 H" in the "Fastener Requirements" table.

3) A custom bent 2x2x0.125" angle must always be installed on the obtuse side of the beam.

4) All other notes and specifications from the "Straight Beam to Support Structure" detail apply to this detail.

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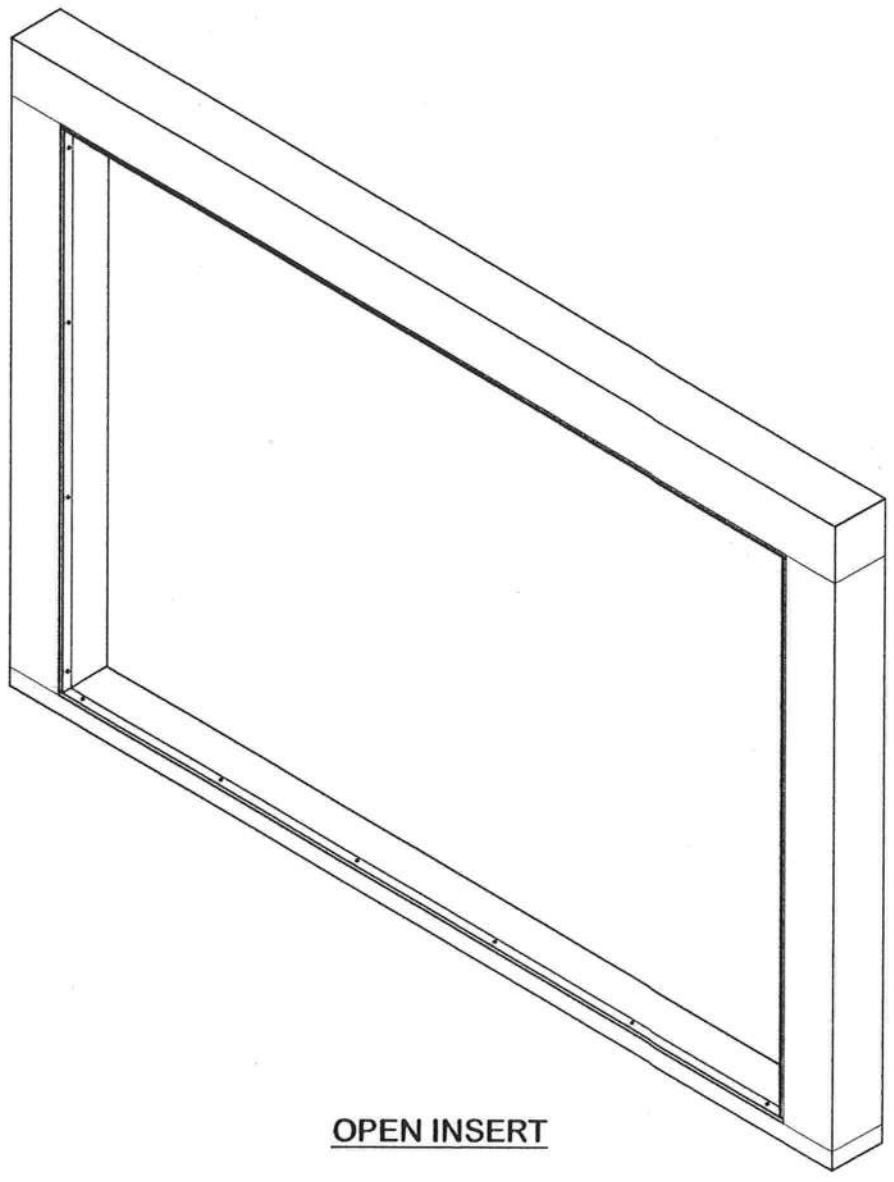
Sheet: D9

Wind Zone: 130 MPH
Exposure: B

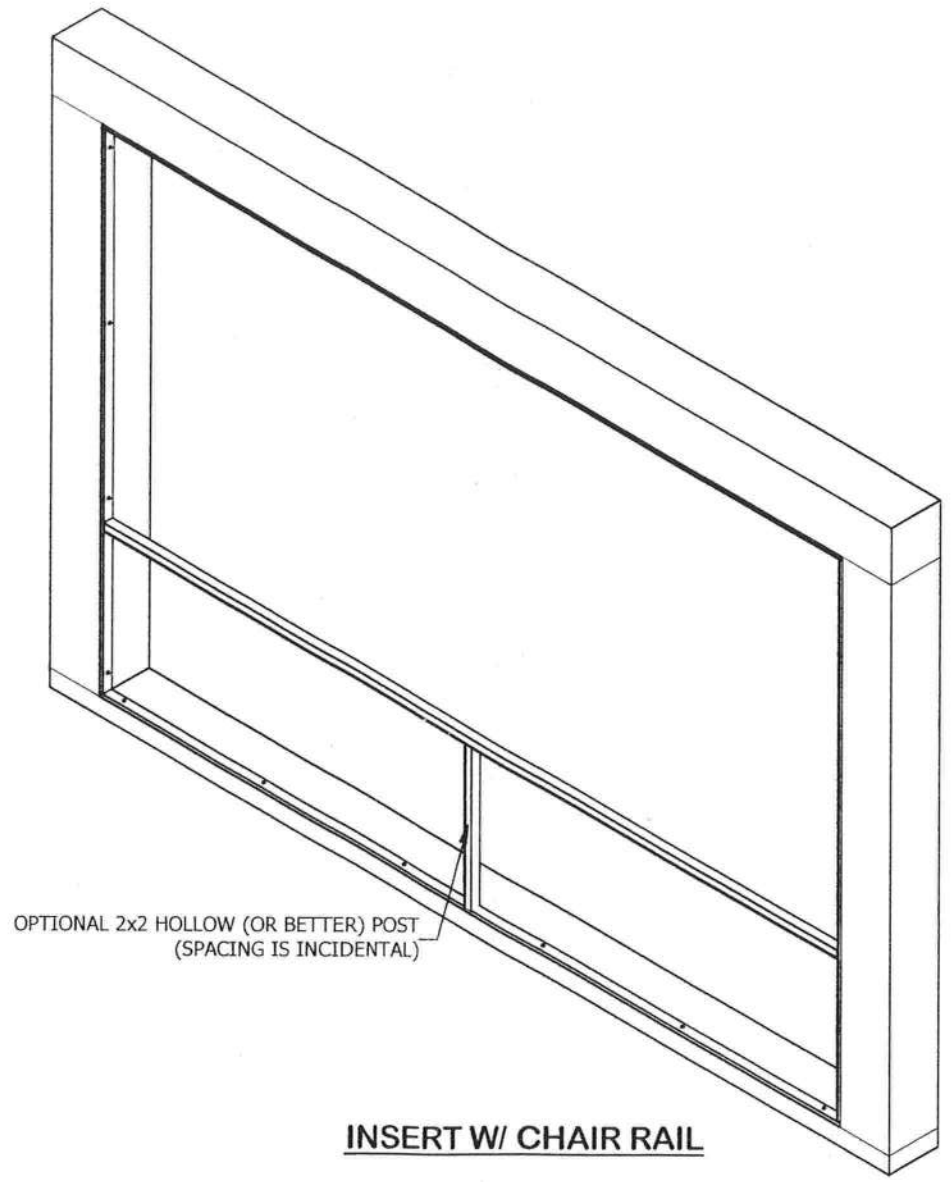
Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.

Screen Insert Notes:

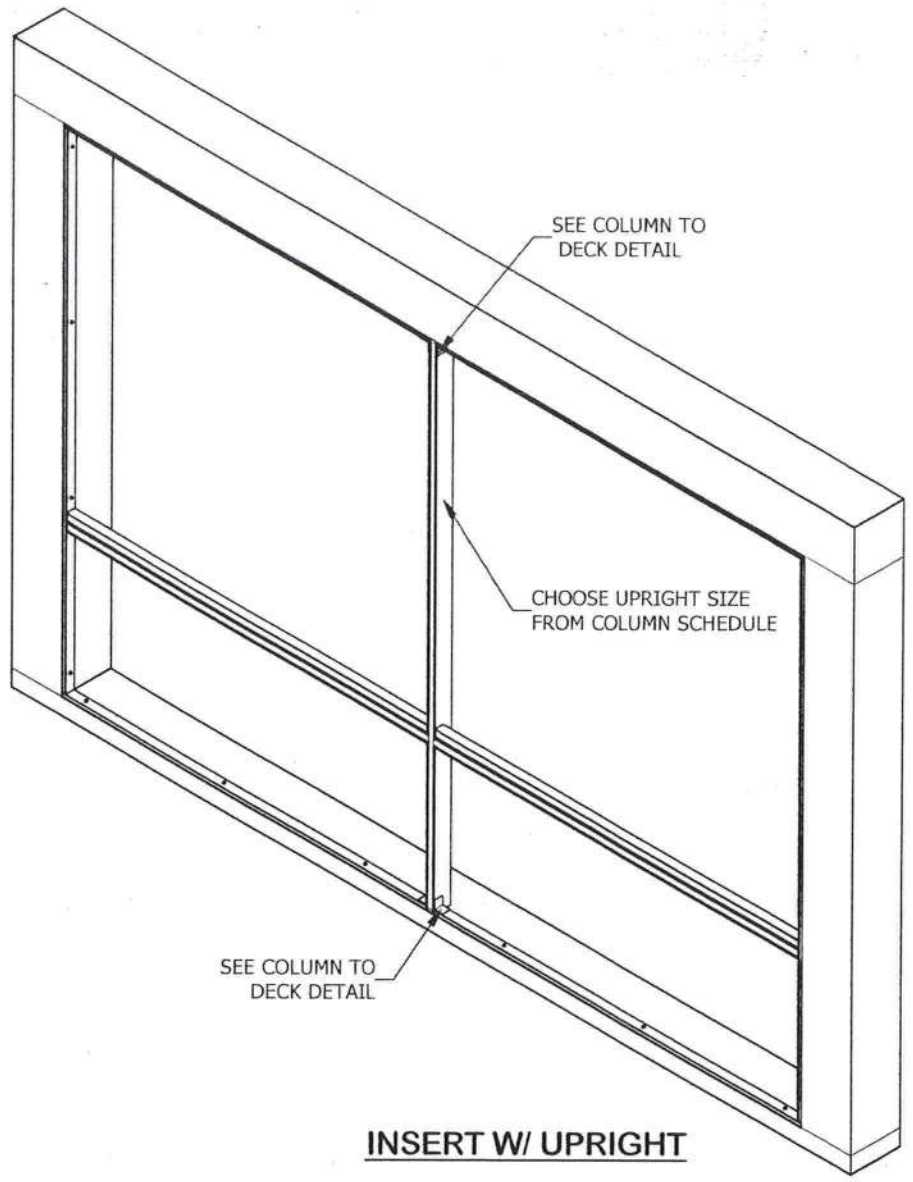
1) Inserts are framed in by 1"x2" O.B., ½" x 2" O.B., or 1"x3" O.B. members.
2) Framing members are fastened to a C.M.U. using 1/4" Ø concrete anchors with a minimum 1¼" embedment.
3) Framing members are fastened to a wood framed wall using a (MIN) #10 wood screw with a (MIN) 1" embedment.
4) Framing members that are fastened to an aluminum framed member will use a (MIN) #10 S.M.S.
5) Framing members are fastened @ max. 36" O.C., regardless of C.M.U., wood, or to another aluminum frame member.
6) There shall be a fastener within 4" of every 90° corner or upright in either direction but not required to be in both directions.
7) Inserts may be as large as desired, the type of insert used is chosen by the designer and is incidental to structural integrity



OPEN INSERT



INSERT W/ CHAIR RAIL



INSERT W/ UPRIGHT

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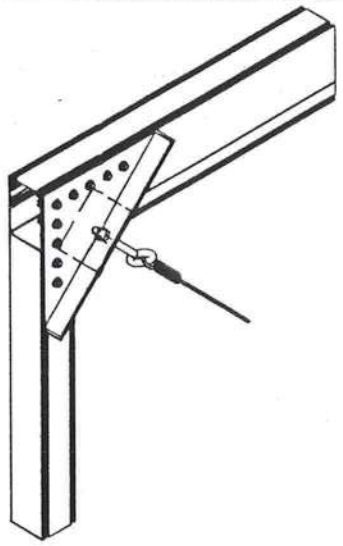
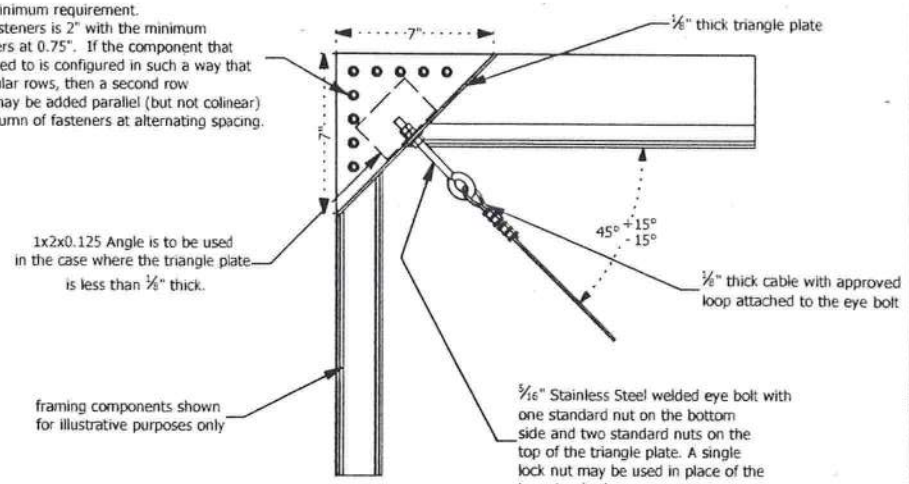


#10 Ø x 0.75" S.M.S.
Quantity shown is the minimum requirement.
Max spacing between fasteners is 2" with the minimum spacing between fasteners at 0.75". If the component that the cable is being attached to is configured in such a way that prevents two perpendicular rows, then a second row or column of fasteners may be added parallel (but not colinear) to the default row or column of fasteners at alternating spacing.



Triangle Plate Top Attachement

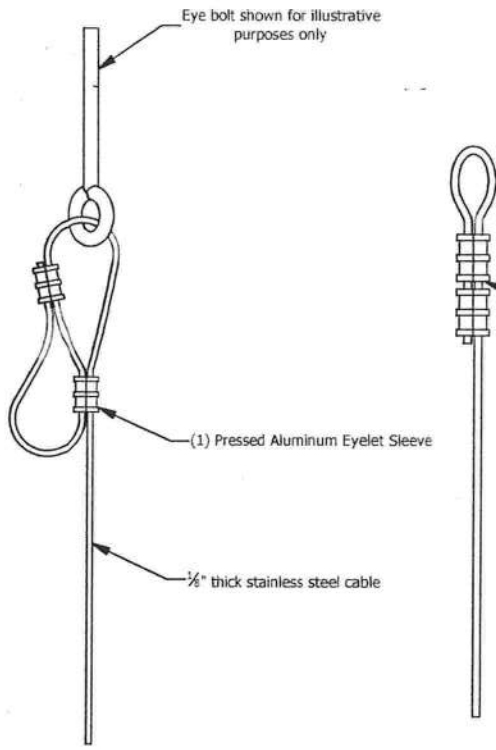
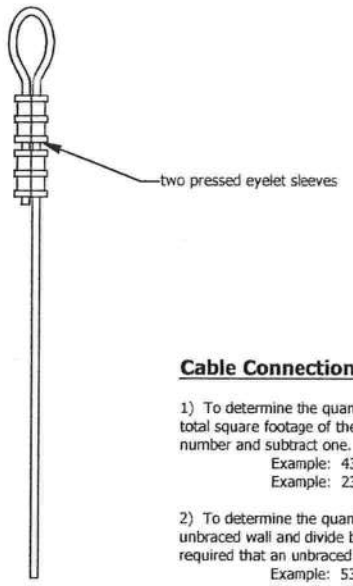
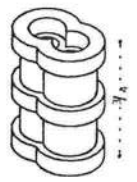


Figure 8 Loop



Single Loop



Pressed Eyelet Sleeve
(Shown after being mechanically pressed)

Cable Connection Notes:

1) To determine the quantity of cables needed for a screen wall braced on one side by a host structure take the total square footage of the braced wall and divide by 250. Round the calculated value to the closest whole number and subtract one. It is intended to not have a cable on a wall that is less than 250 S.F.
Example: 430 S.F. / 250 = 1.72 --> rounds to 2 --> 2-1 = 1 cable
Example: 230 S.F. / 250 = 0.92 --> rounds to 1 --> 1-1 = 0 cables

2) To determine the quantity of cables needed for an unbraced screen wall, take the total square footage of the unbraced wall and divide by 250. Round the calculated value to the closest whole number and multiply by 2. It is required that an unbraced screen wall have an equal amount of cables opposing each other.
Example: 535 S.F. / 250 = 2.14 --> rounds to 2 --> 2*2 = 4 cables (2 pairs of opposing cables)
Example: 780 S.F. / 250 = 3.12 --> rounds to 3 --> 3*2 = 6 cables (3 pairs of opposing cables)

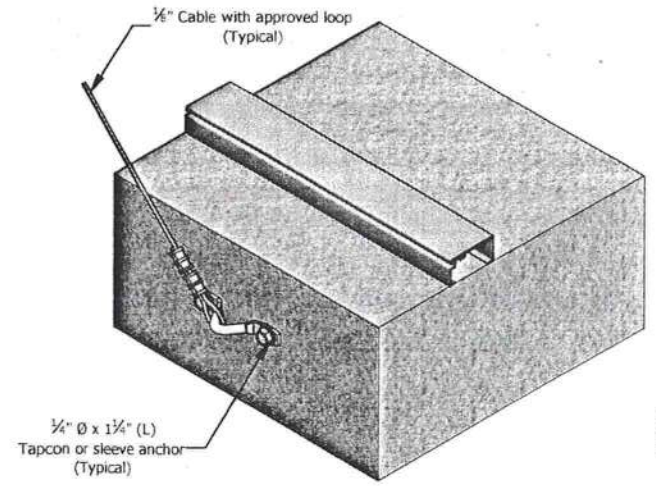
3) Any one of the approved cable-to-deck attachments shown may be used. Field conditions will dictate the type of attachment used and may change dynamically from the original design intent. This has no bearing on the design or the structural integrity of the enclosure and therefore is allowed to be done, as long as one of the approved attachment details is utilized.

4) Cables may attach through pavers as long as the minimum concrete embedment of 1-¼" is achieved by using a longer tapcon.

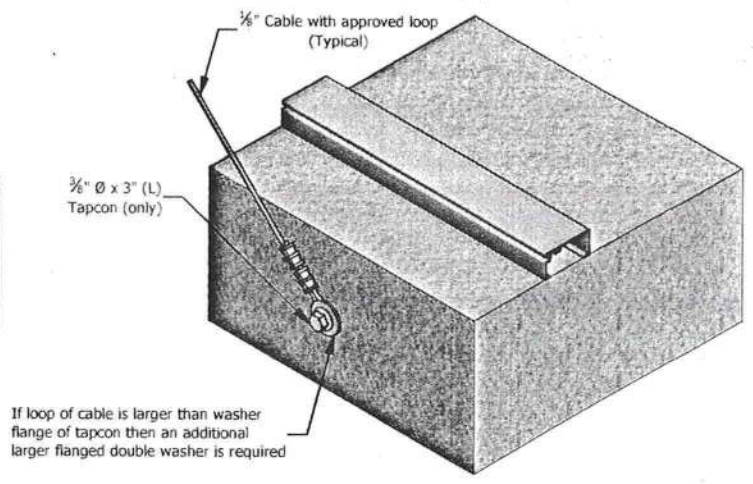
5) Minimum concrete edge distance for all concrete fasteners is 2½".

6) Cables should be at a 45° angle to the vertical uprights (±15°).

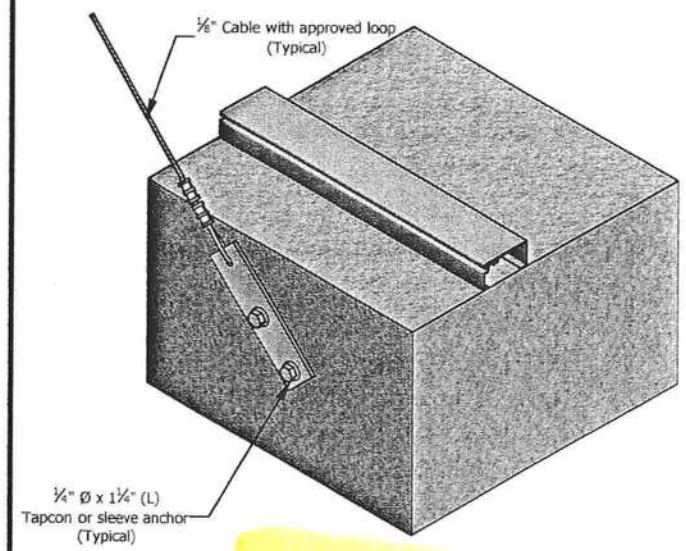
Cable Attachment Details (1 of 2)



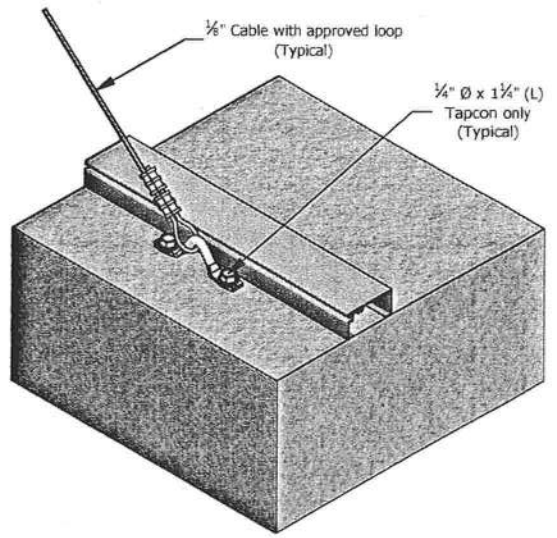
Camelback to Side of Deck



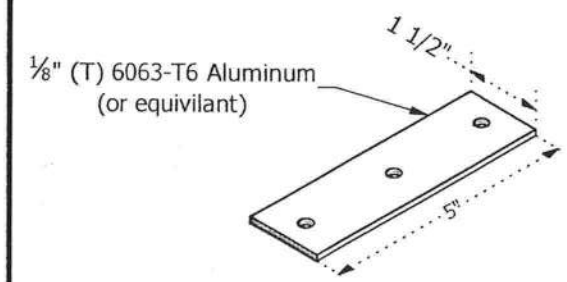
Single Concrete Fastener to Side of Deck



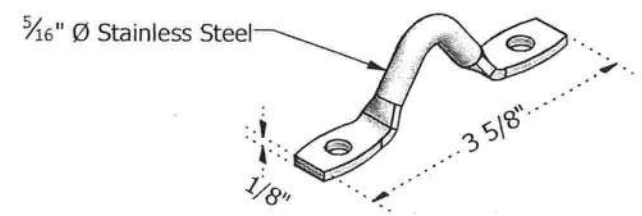
Flat Bar to Side of Deck



Camelback to Top of Deck



Flat Bar Plate



Camelback Clip Bracket

Cable Attachment Details (2 of 2)

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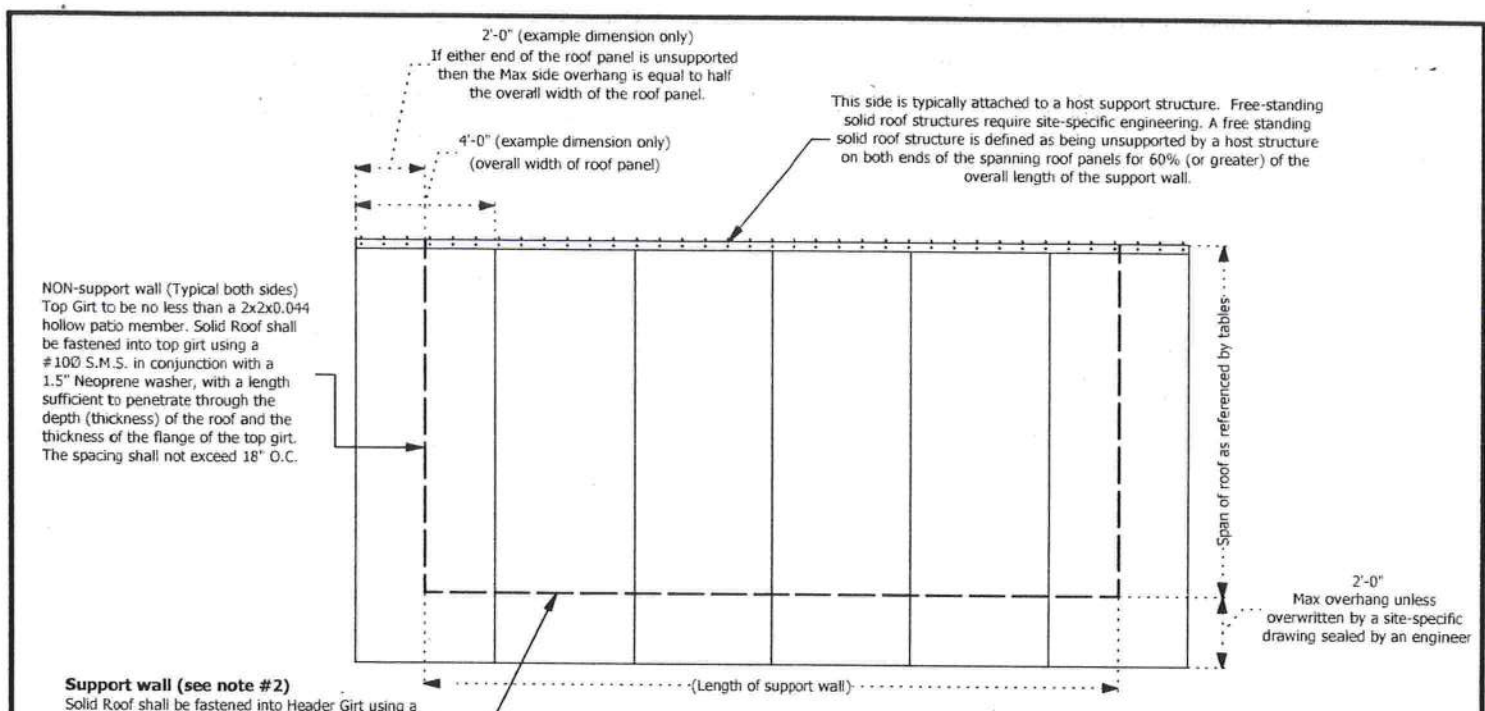
Sheet: D11	Wind Zone: 130 MPH Exposure: B	Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.	FRANK CLEATON, JR, P.E. ENGINEER - FL. LIC. 35816



Solid Aluminum Roof Plan View
SCALE 3/16" = 1'-0"

Notes:

1) The plan view in this detail shows an Elite composite roof but the same rules apply to an aluminum pan roof unless otherwise noted.

2) There are two types of walls: Support Walls and Non-Support Walls. The Support Wall will consist of a Header Girt supporting the solid roof that can either run continuously across the entire length of the wall with intermittent support from columns (See "Independent Post to Carry Beam" detail) or be broken up to span between the support columns via being internally fastened into screw bosses through the web of the support columns (see "Primary Member to Secondary Member" detail). Non-Support Walls (A.K.A side walls) are not used to support the roof and therefore are not required to maintain a Header Girt.

Note:
The Elite roof span tables were produced in accordance with the allowable loads given in the Florida Product Approval of the Elite Aluminum Corporation's Composite Roof Panel. The Florida Product Approval number for this roof panel product is FL-#7561-R1.

Aluminum Pan (3" Riser) Span Table

Pan Type	3"x12"x 0.024"	3"x12"x 0.030	3"x 12" x 0.050
Wind Zone	Effective Span (Ft)		
100	11.0	11.9	15.1
110	10.8	11.7	13.7
120	9.8	10.9	12.8
123	9.5	10.8	12.6
130	9.2	10.5	12.2
140-1	8.8	9.5	11.6
140-2	8.8	9.5	11.6
150	8.4	9.0	11.0

Carry Beam / Header Girt Schedule for Solid Roofs Only

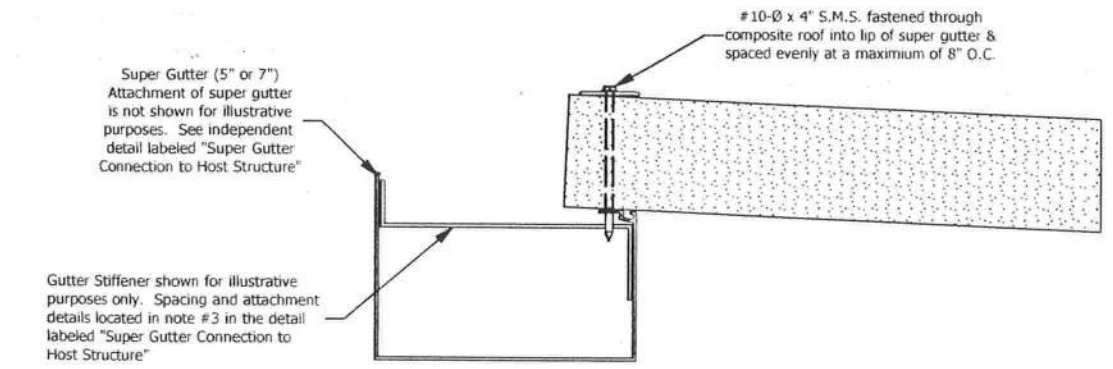
Beam Size	Composite Roof Span	4	5	6	7	8	9	10	11	12	14	16	20
		Effective Span (Ft)											
	2x2 H	7.6	7.1	6.5	6.0	5.8	5.6	5.4	5.2	5.0	4.6	4.3	4.0
	2x3 H	10.7	9.9	9.1	8.4	8.2	7.8	7.5	7.3	6.9	6.4	6.0	5.6
	2x4 H	11.6	10.7	9.9	9.1	8.9	8.5	8.2	7.9	7.5	7.0	6.5	6.1
	2x5 H	14.4	13.4	12.5	12.0	11.4	11.1	10.6	10.3	9.9	9.1	8.6	8.1
	2x4 SMB	12.4	11.6	10.9	10.2	9.5	9.0	8.6	8.2	7.8	7.2	6.7	6.3
	2x5 SMB	15.4	13.2	13.5	12.5	11.7	11.0	10.5	10.0	9.5	8.9	8.3	7.8
	2x6 SMB	18.0	16.6	15.1	13.9	13.1	12.5	11.7	11.2	10.6	9.9	9.2	8.8
	2x7 SMB	20.1	17.9	16.4	15.2	14.1	13.4	12.7	12.1	11.6	10.7	10.0	9.4
	2x8 SMB	25.5	23.6	22.3	21.2	20.2	19.6	18.8	18.1	17.2	15.9	14.9	14.0
	2x9 SMB	28.0	25.9	24.5	23.3	22.1	21.0	19.8	18.9	18.0	16.8	15.6	14.8
	2x10 SMB	34.8	32.4	30.4	28.9	27.7	26.7	25.7	25.0	24.2	23.0	21.9	20.9

Elite Aluminum Corporation Composite Roof Span Table (1# Core Density Foam) (Florida Product Approval #FL7561-R1)

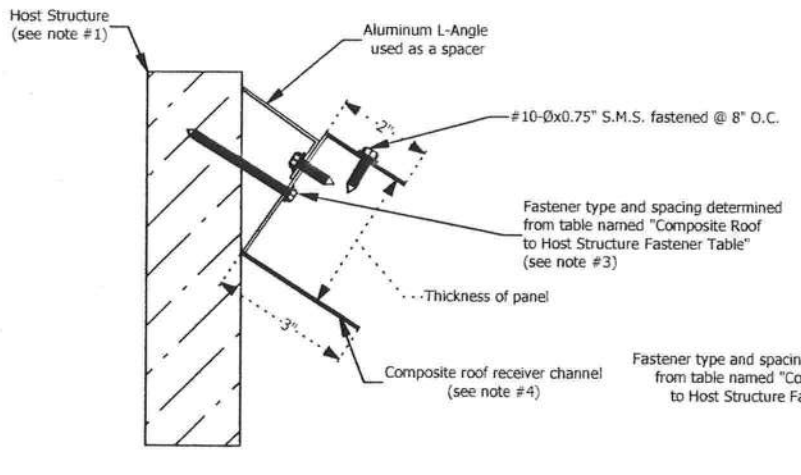
Wind Zone & Category	Roof and Skin Thickness	3"x 48"x 0.024"	3"x 48"x 0.030"	4"x 48"x 0.024"	4"x 48"x 0.030"	6"x 48"x 0.024"	6"x 48"x 0.030"
	100 B	21'-1"	24'-9"	23'-1"	26'-7"	28'-7"	33'-3"
	100 C, 110 B	19'-5"	22'-9"	21'-3"	24'-5"	26'-5"	30'-6"
	110 C, 120 B	18'-2"	21'-1"	19'-7"	22'-6"	24'-6"	28'-3"
	120 C, 130 B	15'-6"	19'-3"	18'-2"	20'-10"	22'-6"	26'-8"
	130 C, 140B	12'-7"	15'-0"	14'-0"	16'-3"	17'-5"	20'-2"

Elite Aluminum Corporation Composite Roof Span Table (2# Core Density Foam) (Florida Product Approval #FL7561-R1)

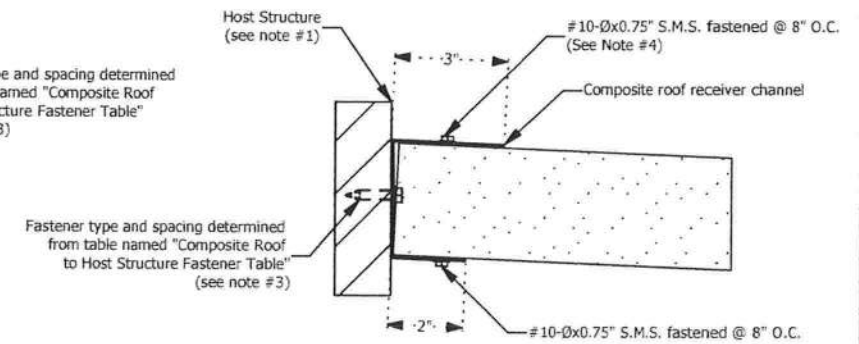
Wind Zone & Category	Roof and Skin Thickness	3"x 48"x 0.024"	3"x 48"x 0.030"	4"x 48"x 0.024"	4"x 48"x 0.030"	6"x 48"x 0.024"	6"x 48"x 0.030"
	100 B	23'-1"	27'-2"	25'-4"	29'-3"	31'-6"	36'-4"
	100 C, 110 B	21'-2"	24'-11"	23'-4"	26'-11"	29'-0"	33'-5"
	110 C, 120 B	19'-7"	23'-2"	21'-8"	25'-1"	28'-6"	31'-1"
	120 C, 130 B	18'-1"	21'-2"	19'-11"	22'-9"	24'-8"	28'-3"
	130 C, 140B	14'-0"	16'-3"	15'-3"	17'-8"	18'-3"	22'-1"



Composite Roof to Top of Super Gutter



Receiver Attachment at a Steep Pitch



Composite Roof to Face of Host Structure

Composite Roof to Host Structure Fastener Requirements

Roof Thickness	Maximum Spacing of Specified Fasteners (inches)					
	Fasteners to Beam		Fasteners to C.M.U. Host Structure		Fasteners to Wood Host Structure	
	#10 S.M.S.	#14 S.M.S.	1/4"Ø Tapcons	3/8"Ø Tapcons	#10 Wood Screw	#14 Wood Screw
3"	8	12	18	24	8	12
4"	6	8	12	18	6	8
5"	4	6	8	12	4	6
6"	4	6	8	12	4	6
	N/A	N/A	1.25"	1.25"	1"	1"
	Minimum Embedment Depths (inches)					

Notes:

1) The term host structure is used synonymously with any structural component that the composite roof can attach to. This is including, but not limited to, the face of a carry beam, house fascia, house wall, and the face of super gutter.

2) The type of fastener that is attaching to the host structure is to be determined by the "Composite Roof to Host Structure Fastener Requirements".

3) Sheet Metal Screws and Wood Lag Screws (only) that attach into the host structure through the receiver channel must contain a 1.5" washer. If a washer is not used, then the type of fasteners mentioned in this note are to be doubled-up. They should be installed in a vertical pattern, evenly spaced between the top and bottom flange of the receiver channel.

4) The fasteners that attach the composite roof to the long flange of the receiver channel are only required where there is sufficient room for installation. For example, if the composite roof is installed under a house overhang to the house wall; or when the roof is in a steep pitch and the bottom is inaccessible. Sufficient room is defined as 24" of space between the top of the composite roof and the bottom of the object covering that area. Anything less than that would constitute insufficient room and therefore not require that the top flange of the receiver channel be fastened.

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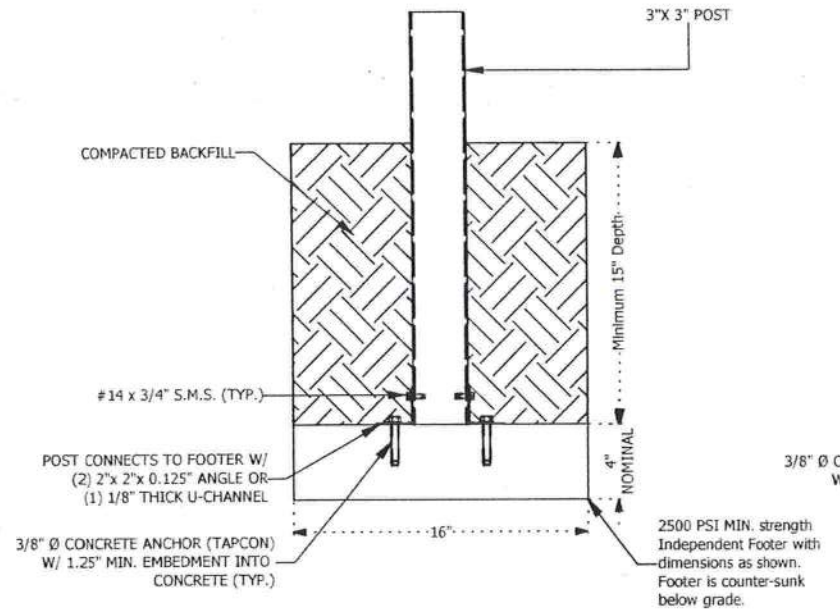
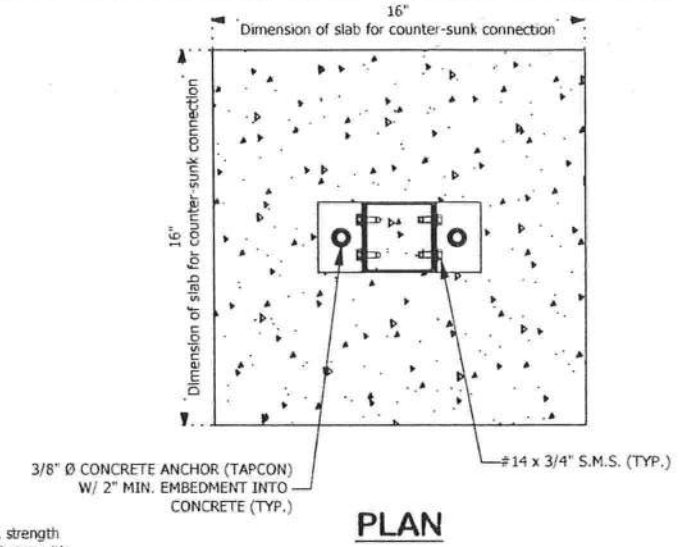
Sheet: D12

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



Counter-Sunk Post (Elevation)



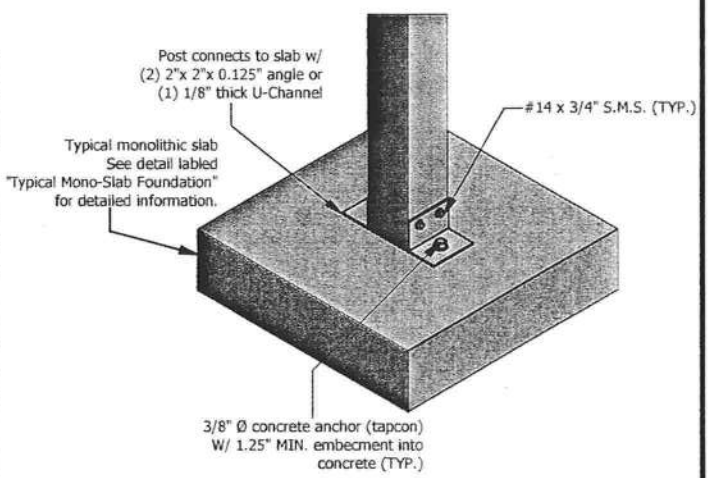
PLAN

Required Pier Footing

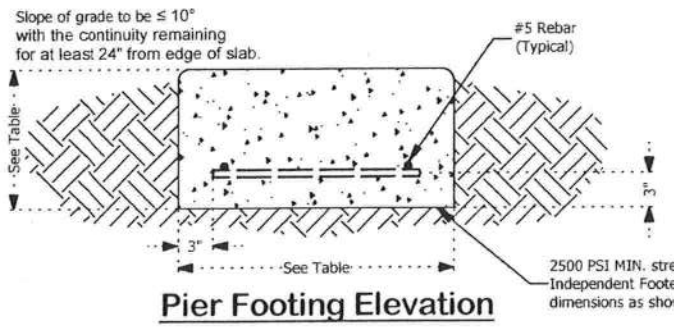
Isolated Footing Dimensions	Allowable Uplift Rating (Lbs.)	Maximun Attributable Roof Area in Ft.^{2}	Required #5 Rebar
1-0" x 1'-0" x 1'-0"	150	4.00	1 Each Way
1-6" x 1'-6" x 1'-6"	1,709	13.47	2 Each Way
2-0" x 2'-0" x 1'-9"	1,050	27.56	3 Each Way
2-3" x 2'-3" x 2'-0"	1,519	39.06	4 Each Way
2-6" x 2'-6" x 2'-3"	2,156	56.25	5 Each Way
2-9" x 2'-9" x 2'-6"	2,836	76.56	6 Each Way
3-0" x 3'-0" x 2'-9"	3,713	95.06	7 Each Way
3-3" x 3'-3" x 3'-0"	4,901	132.25	8 Each Way

To Determine Total Uplift Rating:

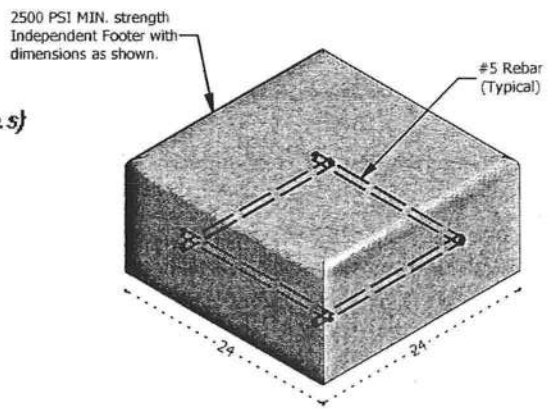
*Maximun Attributable Roof Area (Ft.^{2}) * 37PSF = Actual Uplift Rating (Lbs)*



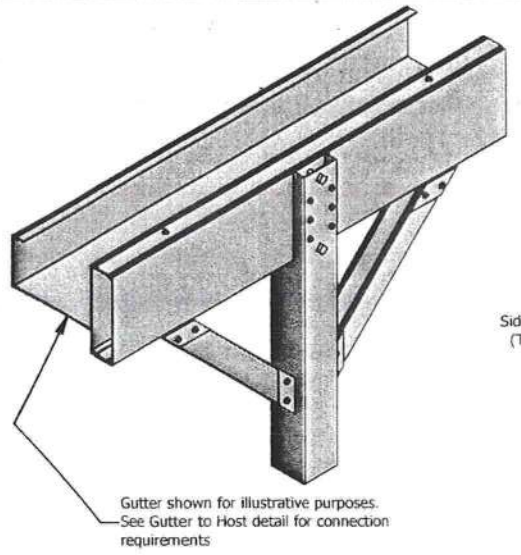
Independent Post to Monlithic Slab



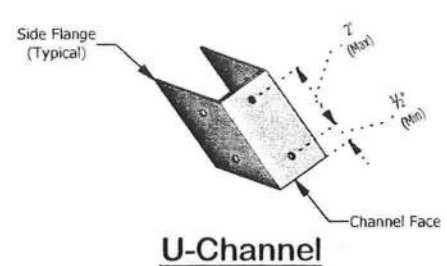
Pier Footing Elevation



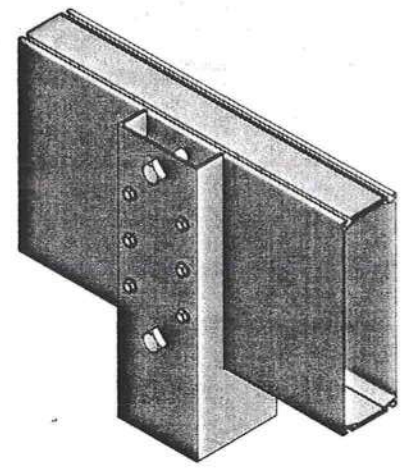
Pier Footing Isometric



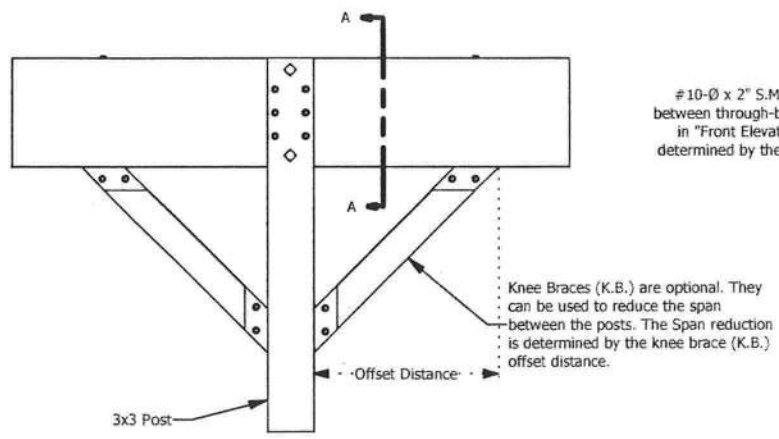
General Isometric



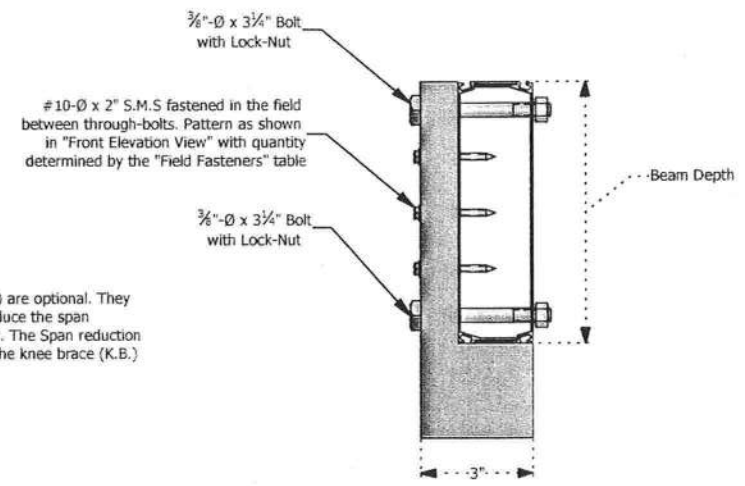
U-Channel



Section A-A Isometric



Front Elevation



Section: A - A

Field Fasteners	
Beam Size	Qty of field fasteners
2x4	0
2x5	2
2x6	4
2x7	6
2x8	6
2x9	8
2x10	8

Knee Brace Schedule		
Size	Length	Qty Per Flange
2x2x0.044	Up to 2'	2
2x3x0.050	2' to 4'	3
2x4x0.050	4' to 6'	4
2x4x 0.048x 0.100 SMB	6' to 7'	4
2x6x 0.050x 0.120 SMB	7' to 8'	6

Notes:

1) The notch in the 3x3 post is shown as a side notch but a centered notch is also allowable. In that case the field fasteners would be required to be installed on both sides of the notch and the through bolts would be required to penetrate both sides of the notch through the beam.

2) The (optional) knee braces are shown being attached with U-Channel but H-Channel is also acceptable. The size of the knee brace and the quantity of fasteners is given in the "Knee Brace Schedule".

3) Height and type of 3x3 post is determined by the appropriate tables called "Column Schedule for Solid Roof / Screen Enclosure Combo". In the event that there is no solid roof or no screen enclosure then take the minimum span given in the table for the non-existent component.

Independent Post to Beam Connection

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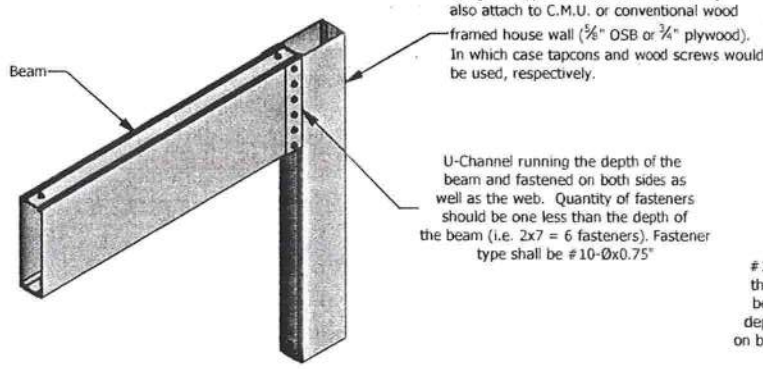
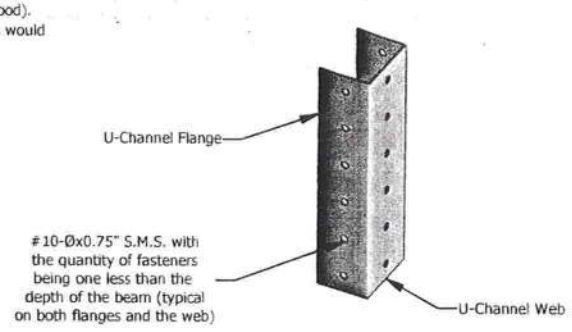
Sheet: D13

Wind Zone: 130 MPH
Exposure: B

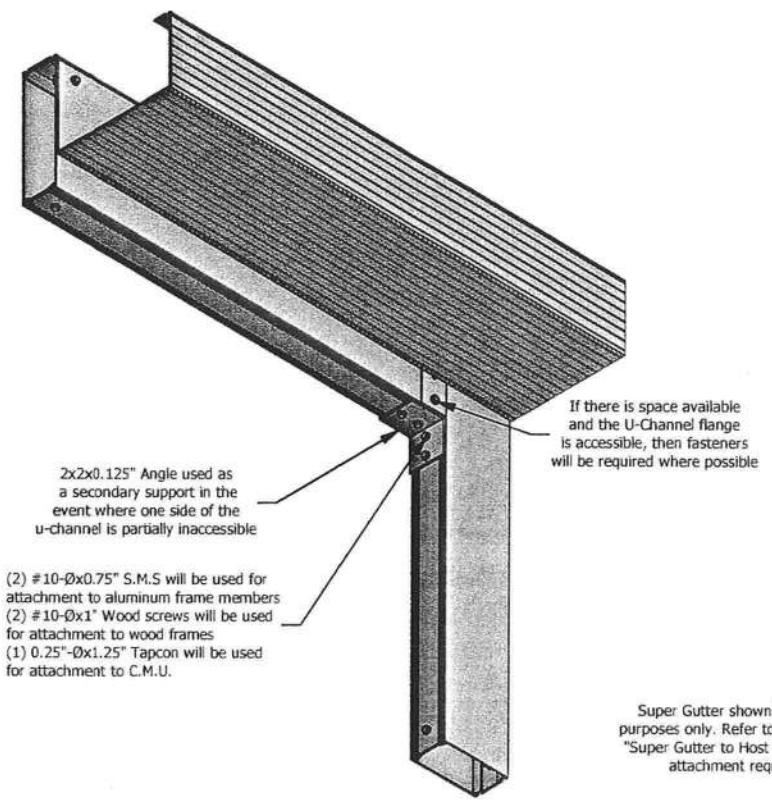
Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



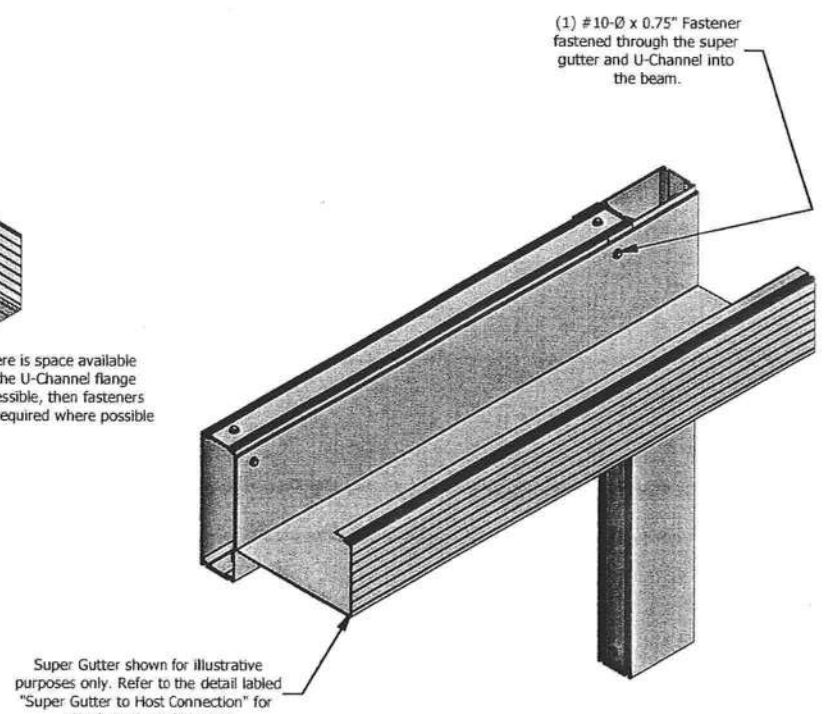
Standard Beam to U-Channel Connection
(Shown from Isometric Top)



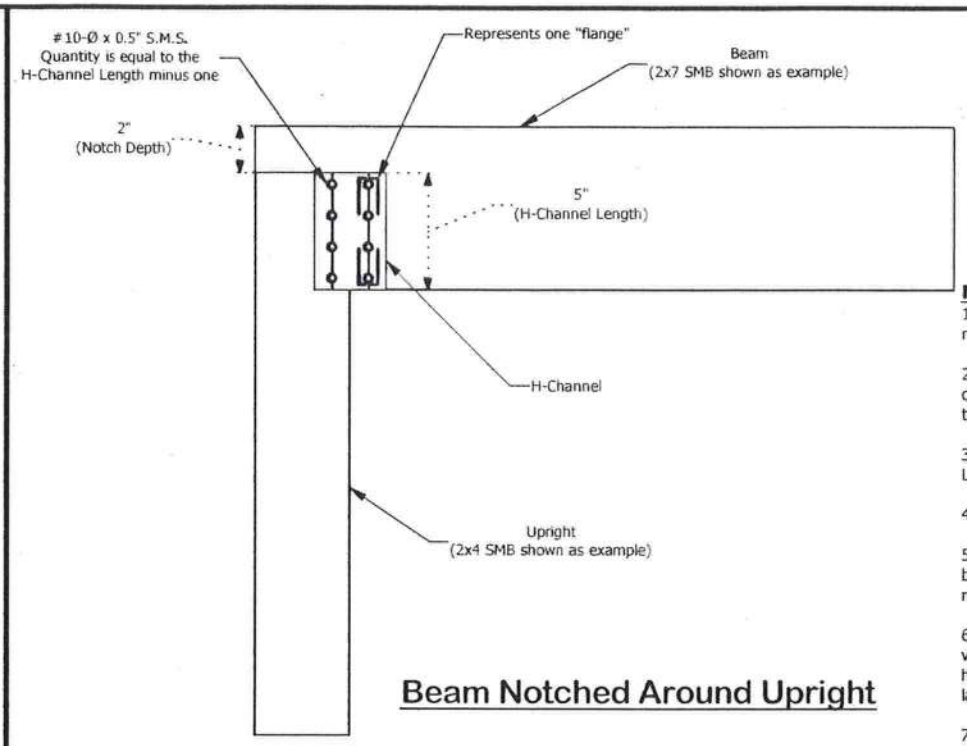
U-Channel
(isolated for clarity)



Beam to U-Channel w/ Gutter
(Shown from Isometric Bottom)



Beam to U-Channel w/ Gutter
(Shown from Isometric Top)



Beam Notched Around Upright

Notes:

1) H-Channel Length will be equal to the beam length minus the notch depth.

2) Knee braces may be used in addition to this connection but are optional. For information about a knee brace connection, please see the detail labeled "Knee Brace Connection".

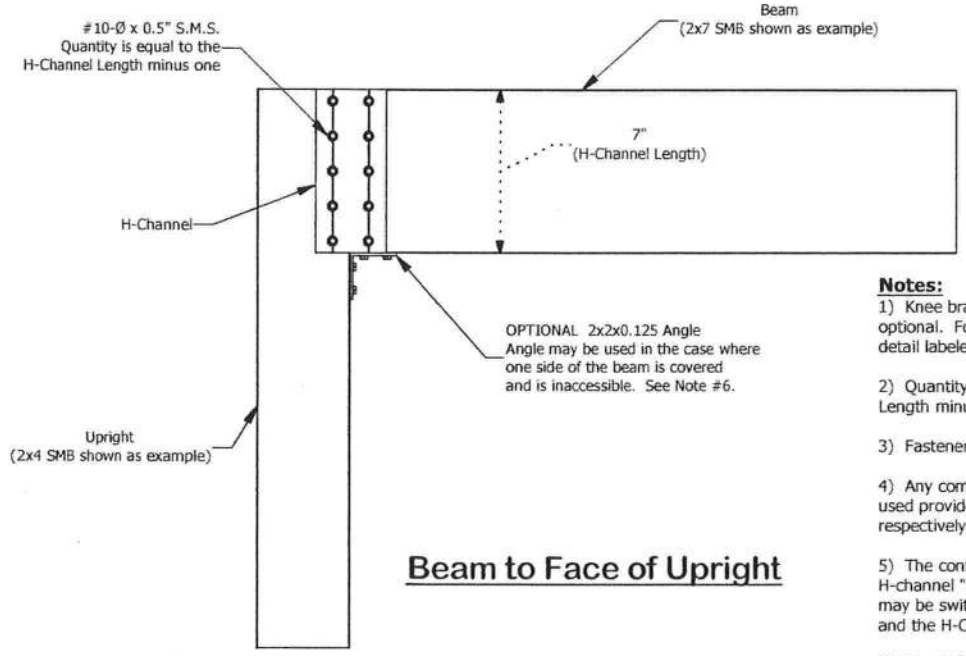
3) Quantity of screws on each flange must be equal to the H-Channel Length minus one (as shown in the detail).

4) Fasteners in H-Channel may be #14-Ø x 0.5" also.

5) Any combination of frame members for the upright and beam may be used provided they are in the Beam Schedule and Upright Schedule respectively.

6) H-Channel configuration and frame-notch shown is in a vertical vector with the beam being notched. The vector may also be horizontal with the upright notched ONLY IF the upright depth is larger than the Beam depth.

7) An Optional 2x2x0.125 angle may be used in cases where one side of the beam is inaccessible. See note #6 in the detail below labeled "Beam to Face of Upright".



Beam to Face of Upright

Notes:

1) Knee braces may be used in addition to this connection but are optional. For information about a knee brace connection, please see the detail labeled "Knee Brace Connection".

2) Quantity of screws on each flange must be equal to the H-Channel Length minus one (as shown in the detail).

3) Fasteners in H-Channel may be #14-Ø x 0.5" also.

4) Any combination of frame members for the upright and beam may be used provided they are in the Beam Schedule and Upright Schedule respectively.

5) The configuration shown is the beam butting into the upright with the H-channel "Length" being driven by the beam depth. This configuration may be switched to have the upright butting into the bottom of the beam and the H-Channel "Length" being driven by the upright depth.

6) The 2x2x0.125 Angle is used in lieu of one inaccessible side of beam. Two #10-Øx0.75 fasteners should be used on each flange of the angle bracket. If the inaccessible side is only partially covered, then fasteners should be installed where possible.

Beam to Upright (H-Channel)

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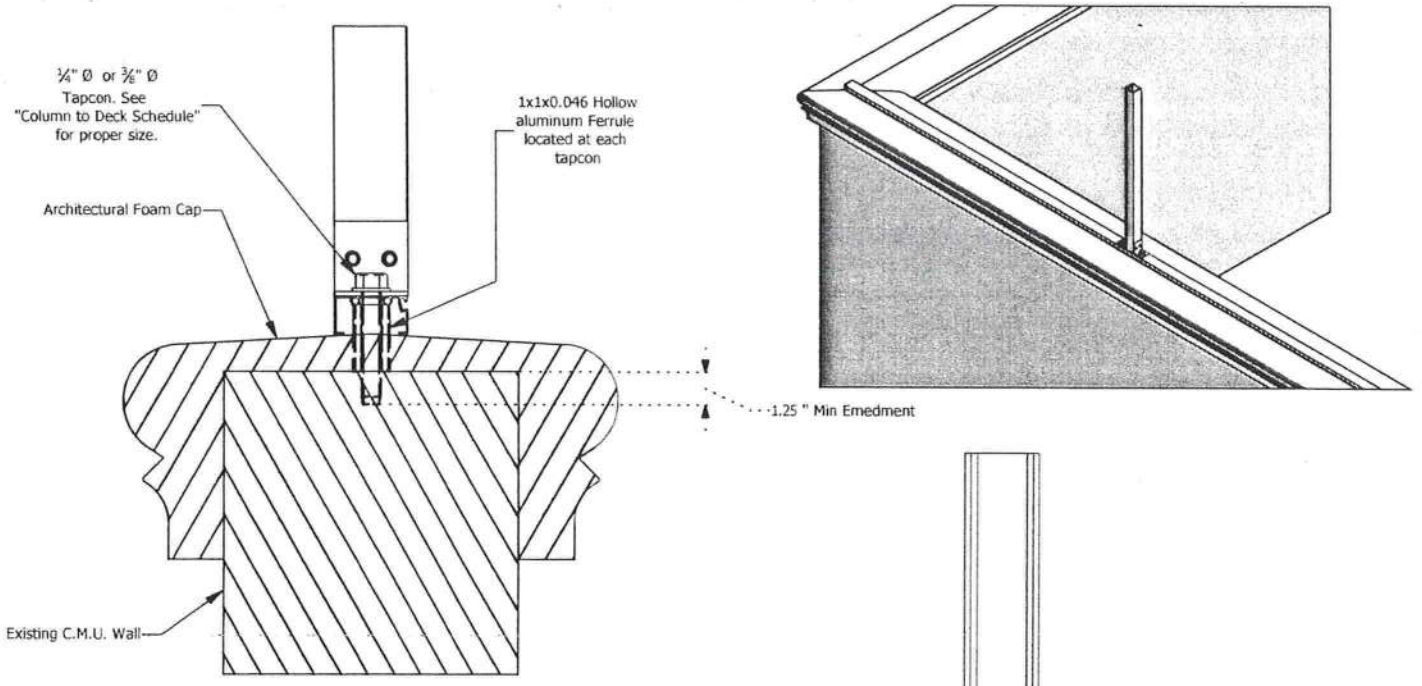
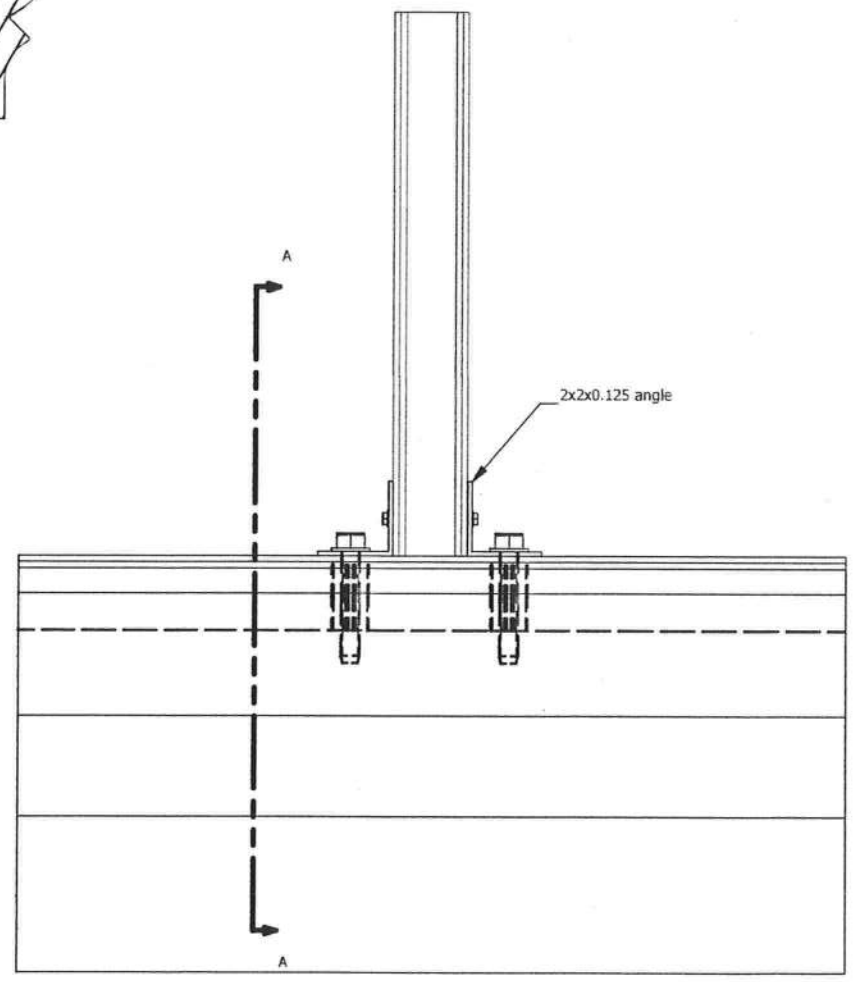
Sheet: D14

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.



SECTION A-A

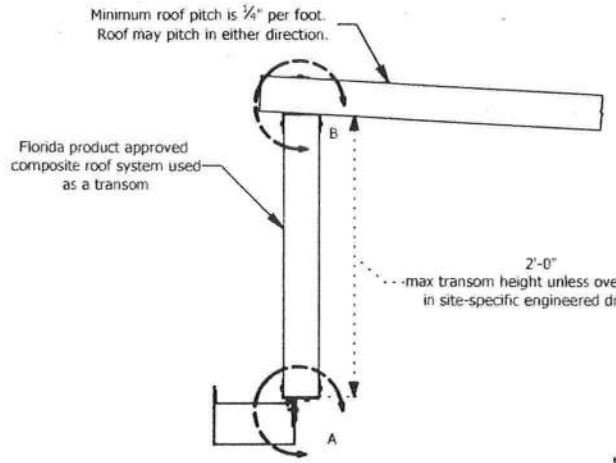


Front View

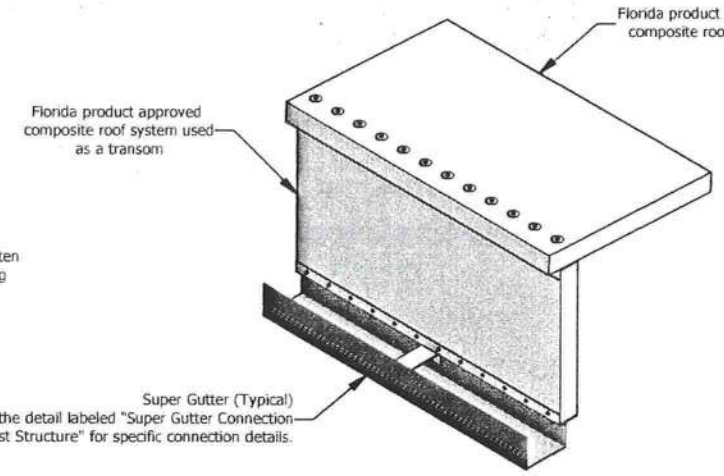
Notes:

1) Tapcons in between each upright may be spaced at 36" O.C. (MAX) with a 1x1x0.046 Hollow aluminum Ferrule support (as shown in the detail).

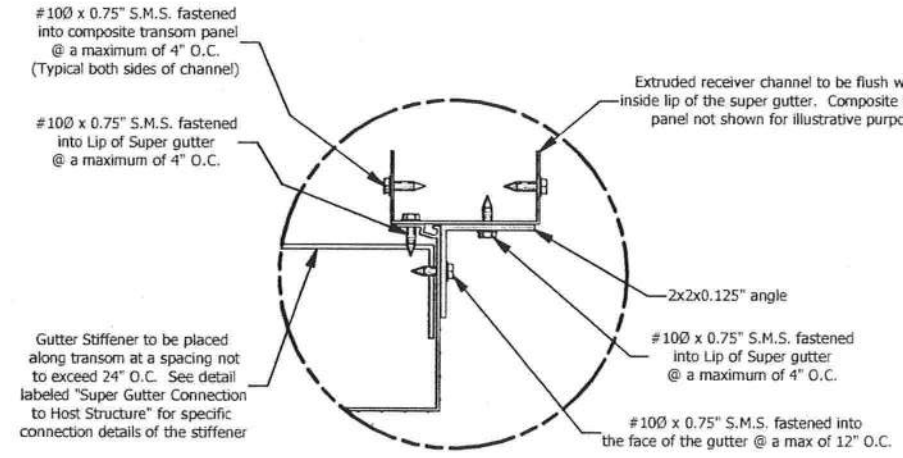
Screen Enclosure Base Attachment to Foam Capped Wall



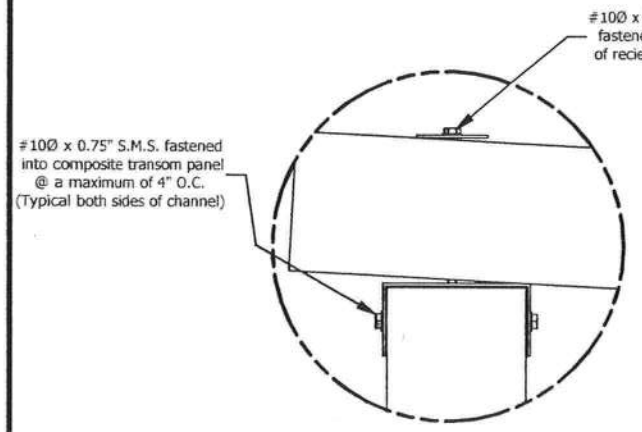
Side Elevation



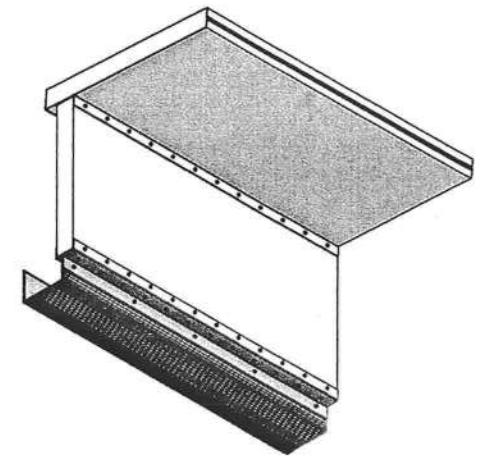
Top Isometric View



DETAIL A



DETAIL B



Bottom Isometric

Composite Panel Transom Detail

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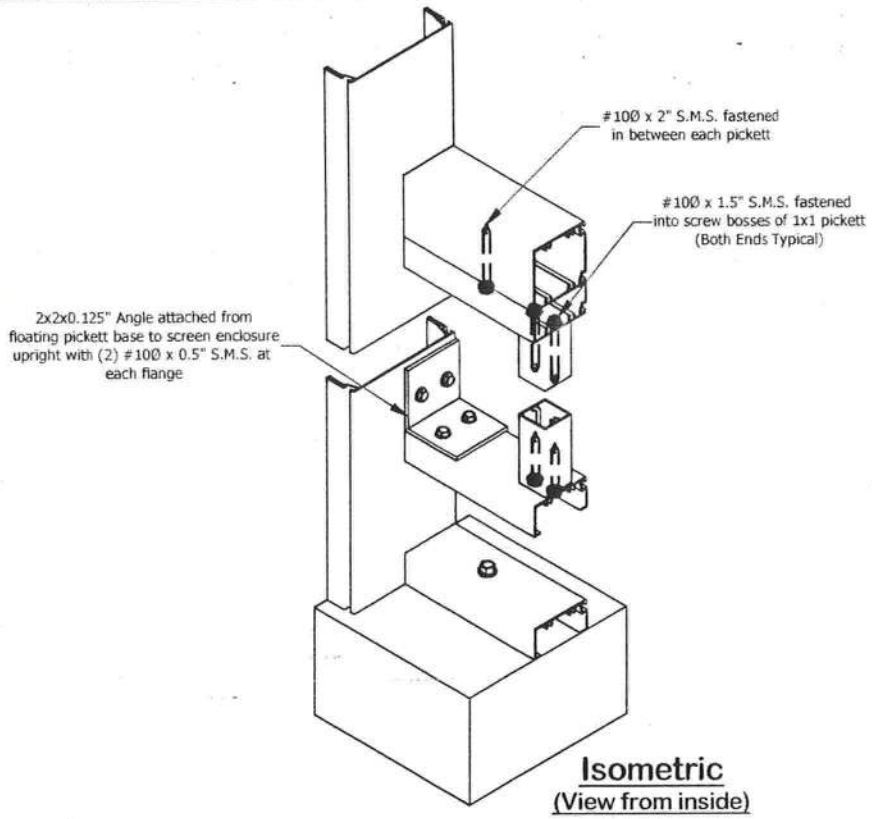
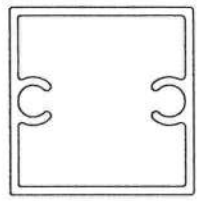
Sheet: D15

Wind Zone: 130 MPH
Exposure: B

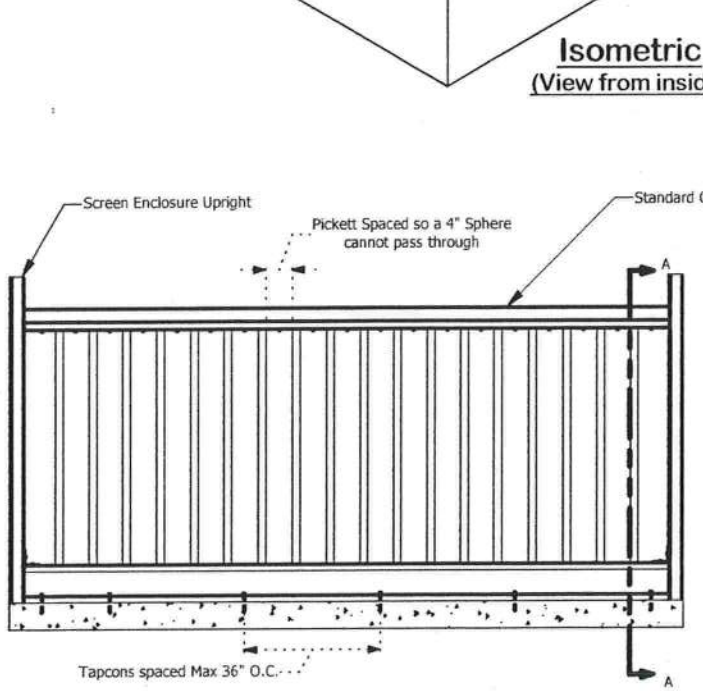
Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.

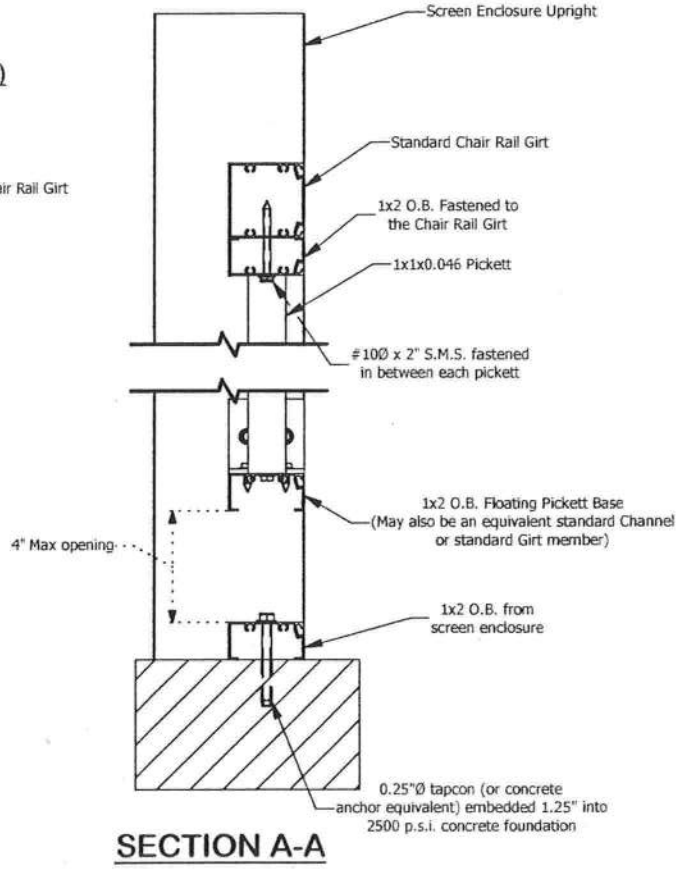


Isometric
(View from inside)



1x1x 0.046 Pickett Profile

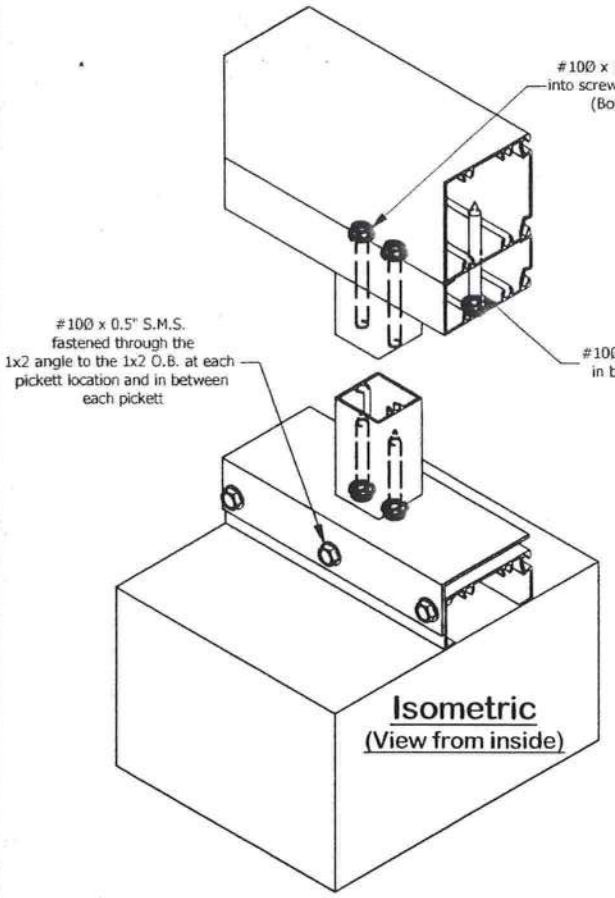




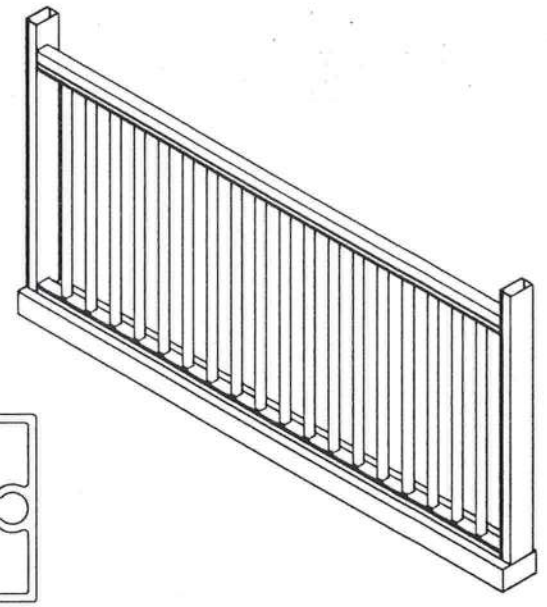
SECTION A-A

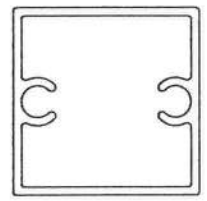
Notes:
1) This method may be incorporated into any aluminum framed screen wall (including framed in openings in a house).
2) The size of the Chair rail (a.k.a. guard rail) may be chosen from the Girt Schedule.
3) Brick Pavers may be under the 1x2 O.B. sole plate as long as the tapcons attach to the concrete footer or deck underneath them meeting the minimum embedment requirements of 1¼".

Detail: Retro-Fit Pickett into Screen Wall (Floating Pickett Base)

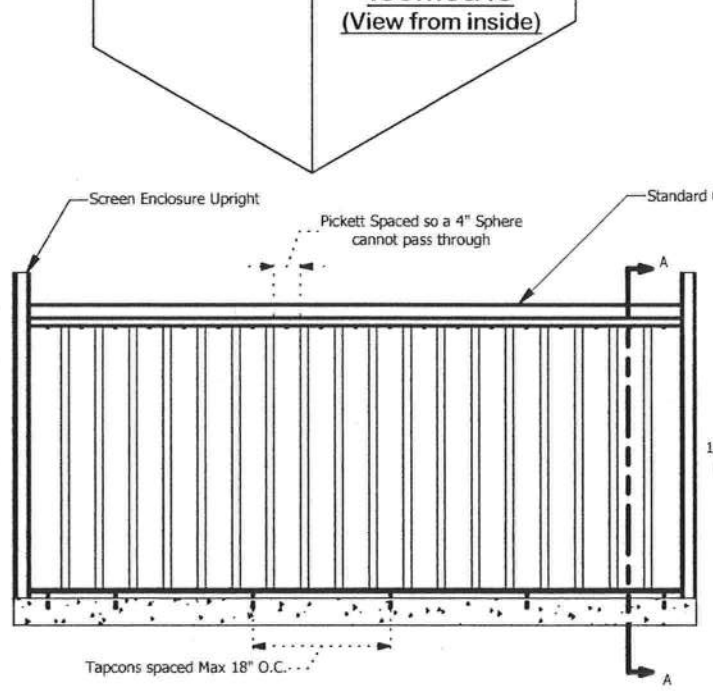


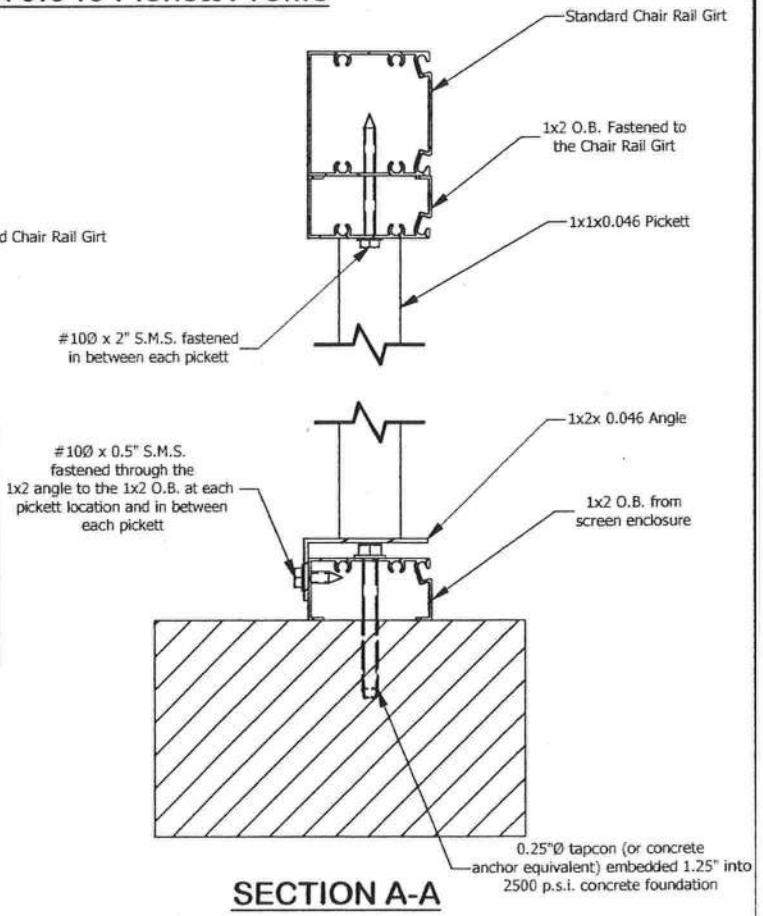
Isometric
(View from inside)





1x1x 0.046 Pickett Profile





SECTION A-A

Notes:
1) This method may be incorporated into any aluminum framed screen wall (including framed in openings in a house).
2) The size of the Chair rail (a.k.a. guard rail) may be chosen from the Girt Schedule.
3) Brick Pavers may be under the 1x2 O.B. sole plate as long as the tapcons attach to the concrete footer or deck underneath them meeting the minimum embedment requirements of 1¼".

Detail: Retro-Fit Pickett into Screen Wall (1x2 Angle Base)

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Sheet: D16

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.

3x3x0.060 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	20.0	19.8	19.6	19.4	19.2	19.0	18.8	18.6	18.4	18.2
10	19.6	19.4	19.2	19.0	18.8	18.6	18.4	18.2	18.0	17.8
12	19.2	19.0	18.8	18.6	18.4	18.2	18.0	17.8	17.6	17.4
14	18.8	18.6	18.4	18.2	18.0	17.8	17.6	17.4	17.2	17.0
16	18.4	18.2	18.0	17.8	17.6	17.4	17.2	17.0	16.8	16.6
18	17.9	17.7	17.5	17.3	17.1	16.9	16.7	16.5	16.3	16.1
20	17.5	17.3	17.1	16.9	16.7	16.5	16.3	16.1	15.9	15.7
22	17.1	16.9	16.7	16.5	16.3	16.1	15.9	15.7	15.5	15.3
24	16.7	16.5	16.3	16.1	15.9	15.7	15.5	15.3	15.1	14.9
26	16.3	16.1	15.9	15.7	15.5	15.3	15.1	14.9	14.7	14.5
Composite Roof Span of 6'										

Heights may be interpolated, but not extrapolated.

3x3x0.060 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	18.9	18.7	18.5	18.3	18.1	17.9	17.7	17.5	17.3	17.1
10	18.2	18.0	17.8	17.6	17.4	17.2	17.0	16.8	16.6	16.4
12	17.5	17.3	17.1	16.9	16.7	16.5	16.3	16.1	15.9	15.7
14	16.8	16.6	16.4	16.2	16.0	15.8	15.6	15.4	15.2	15.0
16	16.2	16.0	15.8	15.6	15.4	15.2	15.0	14.8	14.6	14.4
18	15.5	15.3	15.1	14.9	14.7	14.5	14.3	14.1	13.9	13.7
20	14.8	14.6	14.4	14.2	14.0	13.8	13.6	13.4	13.2	13.0
22	14.1	13.9	13.7	13.5	13.3	13.1	12.9	12.7	12.5	12.3
24	13.4	13.2	13.0	12.8	12.6	12.4	12.2	12.0	11.8	11.6
26	12.7	12.5	12.3	12.1	11.9	11.7	11.5	11.3	11.1	10.9
Composite Roof Span of 10'										

Heights may be interpolated, but not extrapolated.

3x3x0.125 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	20.7	20.6	20.5	20.4	20.3	20.2	20.1	20.0	19.9	19.8
10	20.4	20.3	20.2	20.1	20.0	19.9	19.8	19.7	19.6	19.5
12	20.0	19.9	19.8	19.7	19.6	19.5	19.4	19.3	19.2	19.1
14	19.7	19.6	19.5	19.4	19.3	19.2	19.1	19.0	18.9	18.8
16	19.4	19.3	19.2	19.1	19.0	18.9	18.8	18.7	18.6	18.5
18	19.0	18.9	18.8	18.7	18.6	18.5	18.4	18.3	18.2	18.1
20	18.7	18.6	18.5	18.4	18.3	18.2	18.1	18.0	17.9	17.8
22	18.3	18.2	18.1	18.0	17.9	17.8	17.7	17.6	17.5	17.4
24	18.0	17.9	17.8	17.7	17.6	17.5	17.4	17.3	17.2	17.1
26	17.7	17.6	17.5	17.4	17.3	17.2	17.1	17.0	16.9	16.8
Composite Roof Span of 10'										

Heights may be interpolated, but not extrapolated.

4x4x0.125 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	21.2	21.1	21.0	20.9	20.9	20.8	20.7	20.7	20.6	20.5
10	20.9	20.8	20.8	20.7	20.6	20.5	20.5	20.4	20.3	20.3
12	20.7	20.6	20.5	20.4	20.4	20.3	20.2	20.1	20.1	20.0
14	20.4	20.3	20.3	20.2	20.1	20.0	20.0	19.9	19.8	19.7
16	20.2	20.1	20.0	19.9	19.9	19.8	19.7	19.6	19.6	19.5
18	19.9	19.8	19.8	19.7	19.6	19.5	19.5	19.4	19.3	19.2
20	19.7	19.6	19.5	19.4	19.4	19.3	19.2	19.1	19.1	19.0
22	19.4	19.3	19.3	19.2	19.1	19.0	19.0	18.9	18.8	18.7
24	19.1	19.1	19.0	18.9	18.9	18.8	18.7	18.6	18.6	18.5
26	18.9	18.8	18.8	18.7	18.6	18.6	18.5	18.4	18.3	18.2
Composite Roof Span of 10'										

Heights may be interpolated, but not extrapolated.

3x3x0.060 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	17.8	17.6	17.4	17.2	17.0	16.8	16.6	16.4	16.2	16.0
10	16.8	16.6	16.4	16.2	16.0	15.8	15.6	15.4	15.2	15.0
12	15.5	15.7	15.5	15.3	15.1	14.9	14.7	14.5	14.3	14.1
14	14.9	14.7	14.5	14.3	14.1	13.9	13.7	13.5	13.3	13.1
16	14.0	13.8	13.6	13.4	13.2	13.0	12.8	12.6	12.4	12.2
18	13.0	12.8	12.6	12.4	12.2	12.0	11.8	11.6	11.4	11.2
20	12.0	11.8	11.6	11.4	11.2	11.0	10.8	10.6	10.4	10.2
22	11.1	10.9	10.7	10.5	10.3	10.1	9.9	9.7	9.5	9.3
24	10.1	9.9	9.7	9.5	9.3	9.1	8.9	8.7	8.5	8.3
26	9.1	8.9	8.7	8.5	8.3	8.1	7.9	7.7	7.5	7.3
Composite Roof Span of 14'										

Heights may be interpolated, but not extrapolated.

3x3x0.060 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	16.7	16.5	16.3	16.1	15.9	15.7	15.5	15.3	15.1	14.9
10	15.5	15.3	15.1	14.9	14.7	14.5	14.3	14.1	13.9	13.7
12	14.2	14.0	13.8	13.6	13.4	13.2	13.0	12.8	12.6	12.4
14	13.0	12.8	12.6	12.4	12.2	12.0	11.8	11.6	11.4	11.2
16	11.7	11.5	11.3	11.1	10.9	10.7	10.5	10.3	10.1	9.9
18	10.5	10.3	10.1	9.9	9.7	9.5	9.3	9.1	8.9	8.7
20	9.3	9.1	8.9	8.7	8.5	8.3	8.1	7.9	7.7	7.5
22	8.0	7.8	7.6	7.4	7.2	7.0	6.8	6.6	6.4	6.2
24	6.8	6.6	6.4	6.2	6.0	5.8	5.6	5.4	5.2	5.0
26	5.6	5.4	5.2	5.0	4.8	4.6	4.4	4.2	4.0	3.7
Composite Roof Span of 18'										

Heights may be interpolated, but not extrapolated.

Note:
1) All aluminum frame member tables shown are calculated for exposure B type terrain. If exposure C is needed, then use the multipliers as follows: With a height range of [0'-15': multiply span by 0.83], [15'-20': multiply span by 0.78], [20'-25': multiply span by 0.74], [25'-30': multiply span by 0.71].

3x3x0.092 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	20.9	20.7	20.6	20.5	20.3	20.2	20.1	19.9	19.8	19.7
10	20.6	20.5	20.3	20.2	20.1	19.9	19.8	19.7	19.5	19.4
12	20.3	20.2	20.0	19.9	19.8	19.7	19.5	19.4	19.3	19.1
14	20.0	19.9	19.8	19.6	19.5	19.4	19.2	19.1	19.0	18.9
16	19.8	19.6	19.5	19.4	19.2	19.1	19.0	18.8	18.7	18.6
18	19.5	19.4	19.2	19.1	19.0	18.8	18.7	18.6	18.4	18.3
20	19.2	19.1	19.0	18.8	18.7	18.6	18.4	18.3	18.2	18.0
22	19.0	18.8	18.7	18.6	18.4	18.3	18.2	18.0	17.9	17.8
24	18.7	18.6	18.4	18.3	18.2	18.0	17.9	17.8	17.6	17.5
26	18.4	18.3	18.1	18.0	17.9	17.8	17.6	17.5	17.4	17.2
Composite Roof Span of 6'										

Heights may be interpolated, but not extrapolated.

3x3x0.092 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	20.1	20.0	19.9	19.7	19.6	19.5	19.3	19.2	19.1	18.9
10	19.7	19.5	19.4	19.3	19.1	19.0	18.9	18.8	18.6	18.5
12	19.2	19.1	19.0	18.8	18.7	18.6	18.4	18.3	18.2	18.0
14	18.8	18.6	18.5	18.4	18.2	18.1	18.0	17.8	17.7	17.6
16	18.3	18.2	18.1	17.9	17.8	17.7	17.5	17.4	17.3	17.1
18	17.9	17.7	17.6	17.5	17.3	17.2	17.1	16.9	16.8	16.7
20	17.4	17.3	17.1	17.0	16.9	16.8	16.6	16.5	16.4	16.2
22	17.0	16.8	16.7	16.6	16.4	16.3	16.2	16.0	15.9	15.8
24	16.5	16.4	16.2	16.1	16.0	15.8	15.7	15.6	15.5	15.3
26	16.1	15.9	15.8	15.7	15.5	15.4	15.3	15.1	15.0	14.9
Composite Roof Span of 10'										

Heights may be interpolated, but not extrapolated.

3x3x0.092 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	19.4	19.3	19.1	19.0	18.9	18.7	18.6	18.5	18.4	18.2
10	18.8	18.6	18.5	18.4	18.2	18.1	18.0	17.8	17.7	17.6
12	18.1	18.0	17.9	17.7	17.6	17.5	17.3	17.2	17.1	17.0
14	17.5	17.4	17.2	17.1	17.0	16.8	16.7	16.6	16.5	16.3
16	16.9	16.7	16.6	16.5	16.3	16.2	16.1	15.9	15.8	15.7
18	16.2	16.1	16.0	15.8	15.7	15.6	15.4	15.3	15.2	15.1
20	15.6	15.5	15.3	15.2	15.1	14.9	14.8	14.7	14.5	14.4
22	15.0	14.8	14.7	14.6	14.4	14.3	14.2	14.0	13.9	13.8
24	14.3	14.2	14.1	13.9	13.8	13.7	13.5	13.4	13.3	13.1
26	13.7	13.6	13.4	13.3	13.2	13.0	12.9	12.8	12.6	12.5
Composite Roof Span of 14'										

Heights may be interpolated, but not extrapolated.

3x3x0.092 Column Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width (Ft)	8	10	12	14	16	18	20	22	24	26
Column Tributary Width Spacing (Ft)	Maximum Height (Ft.)									
8	18.7	18.5	18.4	18.3	18.2	18.0	17.9	17.8	17.6	17.5
10	17.9	17.7	17.6	17.5	17.3	17.2	17.1	16.9	16.8	16.7
12	17.1	16.9	16.8	16.7	16.5	16.4	16.3	16.1	16.0	15.9
14	16.2	16.1	16.0	15.8	15.7	15.6	15.4	15.3	15.2	15.1
16	15.4	15.3	15.2	15.0	14.9	14.8	14.6	14.5	14.4	14.2
18	14.6	14.5	14.3	14.2	14.1	13.9	13.8	13.7	13.6	13.4
20	13.8	13.7	13.5	13.4	13.3	13.1	13.0	12.9	12.7	12.6
22	13.0	12.8	12.7	12.6	12.5	12.3	12.2	12.1	11.9	11.8
24	12.2	12.0	11.9	11.8	11.6	11.5	11.4	11.2	11.1	11.0
26	11.3	11.2	11.1	11.0	10.8	10.7	10.6	10.4	10.3	10.2
Composite Roof Span of 18'										

Heights may be interpolated, but not extrapolated.

Carry Beam Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width	8'	10'	12'	14'	16'	18'	20'	22'	24'	26'
Member Type	Effective Span (Ft.)									
2x4 SMB	10.1	9.4	8.8	8.4	8.0	7.7	7.3	6.8	6.2	5.9
2x5 SMB	12.5	11.6	11.0	10.4	9.9	9.5	9.0	8.2	7.7	7.1
2x6 SMB	17.4	16.7	16.4	15.9	15.5	15.1	14.9	14.5	13.8	12.9
2x7 SMB	19.3	18.6	17.9	17.4	17.2	16.7	16.4	15.9	15.5	14.8
2x8 SMB	23.6	23.1	22.5	21.9	21.2	20.9	20.5	20.1	19.4	18.9
2x9 SMB	25.0	24.3	24.0	23.5	22.8	22.0	21.6	21.1	20.5	19.7
2x10 SMB	31.9	31.0	30.3	29.7	29.3	28.7	27.9	27.2	26.4	25.9
2x5 TFB	18.3	17.5	17.1	16.8	16.2	15.8	15.4	14.8	14.5	13.6
2x7 TFB	21.7	21.0	20.5	20.0	19.4	19.1	18.5	18.0	17.5	17.0
2x9 TFB	27.7	26.8	26.1	25.5	25.1	24.6	24.0	23.4	22.7	22.4
Composite Roof Span of 6'										

Carry Beam Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width	8'	10'	12'	14'	16'	18'	20'	22'	24'	26'
Member Type	Effective Span (Ft.)									
2x4 SMB	7.9	7.4	6.9	6.6	6.3	6.1	5.8	5.3	4.9	4.6
2x5 SMB	9.9	9.2	8.6	8.2	7.8	7.5	7.1	6.5	6.1	5.6
2x6 SMB	13.7	13.2	12.9	12.5	12.2	11.9	11.7	11.4	10.9	10.1
2x7 SMB	15.2	14.7	14.1	13.7	13.5	13.2	12.9	12.5	12.2	11.7
2x8 SMB	18.6	18.2	17.7	17.3	16.7	16.5	16.1	15.8	15.3	14.9
2x9 SMB	19.7	19.1	18.9	18.5	18.0	17.4	17.0	16.6	16.1	15.5
2x10 SMB	25.1	24.4	23.9	23.4	23.1	22.6	22.0	21.4	20.8	20.4
2x5 TFB	14.4	13.8	13.4	13.1	12.7	12.5	12.1	11.7	11.4	10.9
2x7 TFB	17.1	16.6	16.1	15.8	15.3	15.0	14.6	14.2	13.8	13.4
2x9 TFB	21.8	21.1	20.6	20.1	19.8	19.4	18.9	18.4	17.9	17.6
Composite Roof Span of 14'										

Carry Beam Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width	8'	10'	12'	14'	16'	18'	20'	22'	24'	26'
Member Type	Effective Span (Ft.)									
2x4 SMB	8.9	8.3	7.8	7.4	7.1	6.8	6.5	6.0	5.5	5.2
2x5 SMB	11.1	10.3	9.7	9.2	8.8	8.4	8.0	7.3	6.8	6.3
2x6 SMB	15.4	14.8	14.5	14.1	13.7	13.4	13.2	12.8	12.2	11.4
2x7 SMB	17.1	16.5	15.8	15.4	15.2	14.8	14.5	14.1	13.7	13.1
2x8 SMB	20.9	20.4	19.9	19.4	18.8	18.5	18.1	17.8	17.2	16.7
2x9 SMB	22.1	21.5	21.2	20.8	20.2	19.5	19.1	18.7	18.1	17.4
2x10 SMB	28.2	27.4	26.8	26.3	25.5	25.4	24.7	24.1	23.4	22.9
2x5 TFB	16.2	15.5	15.1	14.7	14.3	14.0	13.6	13.1	12.8	12.2
2x7 TFB	19.2	18.6	18.1	17.7	17.2	16.5	16.4	15.9	15.5	15.0
2x9 TFB	24.5	23.7	23.1	22.6	22.2	21.8	21.2	20.7	20.1	19.8
Composite Roof Span of 10'										

Carry Beam Schedule for Solid Roof / Screen Enclosure Combo

Screen Roof Tributary Width	8'	10'	12'	14'	16'	18'	20'	22'	24'	26'
Member Type	Effective Span (Ft.)									
2x4 SMB	7.0	6.6	6.2	5.8	5.6	5.4	5.1	4.7	4.3	4.1
2x5 SMB	8.8	8.1	7.7	7.3	7.0	6.6	6.3	5.8	5.4	5.0
2x6 SMB	12.2	11.7	11.5	11.1	10.8	10.6	10.4	10.1	9.6	9.0
2x7 SMB	13.5	13.0	12.5	12.2	12.0	11.7	11.5	11.1	10.8	10.3
2x8 SMB	16.5	16.1	15.7	15.3	14.9	14.6	14.3	14.1	13.6	13.2
2x9 SMB	17.5	17.0	16.7	16.4	16.0	15.4	15.1	14.8	14.3	13.7
2x10 SMB	22.3	21.6	21.2	20.8	20.5	20.1	19.5	19.0	18.5	18.1
2x5 TFB	12.8	12.2	11.9	11.6	11.3	11.1	10.7	10.3	10.1	9.6
2x7 TFB	15.2	14.7	14.3	14.0	13.6	13.4	13.0	12.6	12.2	11.9
2x9 TFB	19.4	18.7	18.2	17.9	17.5	17.2	16.7	16.4	15.9	15.6
Composite Roof Span of 18'										

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Sheet: D17

Wind Zone: 130 MPH
Exposure: B

Design conforms to the FBC 2007 Building and the FBC 2007 Residential, with the 2009 supplements, as well as the ASCE 7-05.

ELITE PANEL SPAN TABLES

Shown for information purposes only

3" x 0024 x 1 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	16.17	15.76	15.03	14.10
20	13.44	13.44	12.22	10.35
30	10.78	10.78	9.41	6.60
40	9.22	9.22	6.60	2.85
50	8.17	8.17	3.79	-
60	7.40	6.39	0.98	-
70	6.81	4.51	-	-
80	6.33	2.64	-	-

3" x 0032 x 1 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	17.50	17.50	16.91	15.96
20	16.64	15.96	14.06	12.16
30	15.17	14.06	11.21	8.36
40	13.69	12.16	8.36	4.56
50	12.22	10.26	5.51	0.76
60	10.75	8.36	2.66	-
70	9.27	6.46	-	-
80	7.80	4.56	-	-

3" x 0024 x 2 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	19.33	18.95	18.31	17.66
20	18.11	17.66	16.36	15.06
30	16.80	16.36	14.41	12.46
40	15.49	15.06	12.46	9.86
50	14.18	13.76	10.51	7.26
60	12.87	12.46	8.57	4.67
70	11.57	11.16	6.62	2.07
80	10.26	9.86	4.67	-

3" x 0030 x 2 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	20.11	20.03	19.42	18.81
20	19.02	18.81	17.58	16.35
30	17.93	17.58	15.73	13.89
40	16.83	16.35	13.89	11.43
50	15.74	15.12	12.05	8.97
60	14.64	13.89	10.21	6.52
70	13.55	12.66	8.36	4.06
80	12.46	11.43	6.52	1.60

4" x 0024 x 1 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	19.00	19.00	17.17	16.53
20	15.01	15.01	15.01	13.95
30	12.50	12.50	12.50	11.38
40	10.97	10.97	10.97	8.80
50	9.92	9.92	9.44	6.22
60	9.13	9.13	7.51	3.64
70	8.52	8.52	5.58	1.07
80	8.02	8.02	3.64	-

4" x 0.032 x 1 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	20.50	20.50	20.11	19.24
20	19.61	19.24	17.49	15.74
30	18.17	17.49	14.87	12.24
40	16.72	15.74	12.24	8.74
50	15.28	13.99	9.62	5.25
60	13.84	12.24	7.00	1.75
70	12.40	10.49	4.38	-
80	10.95	8.74	1.75	-

4" x 0024 x 2 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	21.97	21.97	21.52	20.97
20	20.77	20.77	19.86	18.76
30	19.57	19.57	18.21	16.55
40	18.36	18.36	16.55	14.34
50	17.16	17.16	14.89	12.13
60	15.96	15.96	13.24	9.93
70	14.75	14.75	11.58	7.72
80	13.55	13.55	9.93	5.51

4" x 0030 x 2 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	24.17	24.17	24.17	24.17
20	23.64	23.64	23.41	23.11
30	22.57	22.57	21.90	21.01
40	21.51	21.51	20.39	18.91
50	20.45	20.45	18.88	16.80
60	19.39	19.39	17.37	14.70
70	18.33	18.33	15.86	12.59
80	17.26	17.26	14.35	10.49

6" x 0024 x 1 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	23.00	21.24	21.47	20.85
20	18.06	18.06	18.06	18.06
30	15.13	15.13	15.13	15.13
40	13.34	13.34	13.34	13.34
50	12.10	12.10	12.10	10.91
60	11.17	11.17	11.17	8.43
70	10.44	10.44	10.30	5.95
80	9.85	9.85	8.43	3.47

6" x 0032 x 1 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	24.00	24.00	24.00	23.42
20	23.34	23.21	21.82	20.22
30	22.10	21.63	19.42	17.02
40	20.86	20.05	17.02	13.82
50	19.62	18.47	14.62	10.62
60	18.38	16.89	12.22	7.42
70	17.14	15.30	9.82	4.22
80	15.91	13.72	7.42	1.02

6" x 0024 x 2 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

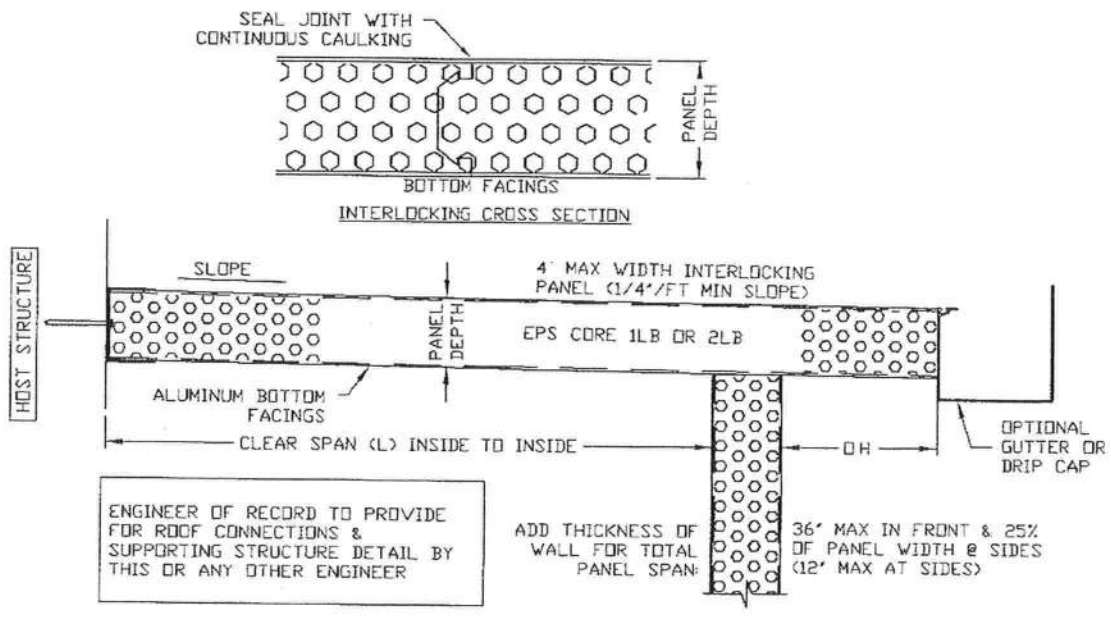
ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	23.93	23.93	23.88	23.60
20	23.20	23.20	23.03	22.46
30	22.47	22.47	22.18	21.33
40	21.75	21.75	21.33	20.20
50	21.02	21.02	20.49	19.07
60	20.29	20.29	19.64	17.94
70	19.57	19.57	18.79	16.81
80	18.84	18.84	17.94	15.68

6" x 0030 x 2 - LB EPS PANELS (ALLOWABLE CLEAR SPAN CHARTS)

ALLOWABLE LOAD (PSF)	MAX ALLOWABLE SPAN (FT)			
	L/80	L/120	L/180	L/240
10	24.00	24.00	24.00	23.84
20	23.65	23.65	23.34	22.84
30	22.94	22.94	22.59	21.85
40	22.23	22.23	21.85	20.85
50	21.53	21.53	21.10	19.86
60	20.82	20.82	20.36	18.87
70	20.11	20.11	19.61	17.87
80	19.40	19.40	18.87	16.88

GENERAL NOTES

1. Composite panels shall be constructed using type 3003-H154 aluminum facings, 1 or 2 PCF ASTM C-578 carpenter brand EPS adhere to aluminum facings with Ashland Chemical 2020D ISO grip Fabrication to be by Elite panel products only in accordance with approved fabrication methods
2. Elite roof panels maintain a UL 1715 (int) class 'B' (ext) rating and are NER-501 approved.
3. This specification has been designed and shall be fabricated in accordance with the requirements of the 2007 Florida Building Code, composite panels comply with Chapter 7 Section 719, Chapter 8 Section 803, Class A interior finish, and Chapter 26 Section 2603. All local building code amendments shall be adhered to as required
4. The designer shall determine by accepted engineering practice the allowable loads for site specific load conditions (including load combinations) using the data from the allowable loads tables and spans in this approval
5. Deflection limits and allowable spans have been listed to meet 2007 FBC including the HVHZ In HVHZ, this product shall be used in structures "not to be considered living areas" per Section 1613 unless impact resistance in accordance to the HVHZ requirements are met
6. Safety factor of 2 0 has been use to develop allowable loads and spans from testing in accordance to the Guidelines for Aluminum Structures Part 1 and conforms to the 2007 FBC Chapter 16 and 20.
7. Testing has been conducted in accordance to ASTM E72-05: Strength Test of Panels for Building Construction
8. Reference test reports: HETI-05-1988, HETI-06-2104, HETI-06-2066, HETI-06-2105, HETI-06-2067, HETI-05-1002, HETI-06-2107, HETI-05-1987, HETI-06-2069, HETI-06-2070, HETI-06-2071, HETI-05-1994, HETI-05-1991, HETI-06-2072, HETI-06-2073, HETI-06-2074, HETI-05-1996, HETI-05-1989, HETI-05-1993, HETI-05-1985, HETI-05-1995, HETI-05-1990, HETI-05-1997, HETI-05-2037, HETI-05-2029, HETI-05-2039, HETI-05-2030, HETI-05-2041, HETI-05-2048, HETI-05-2036, HETI-05-2031, HETI-05-2038, HETI-05-2065, HETI-05-2040, HETI-05-2042
9. Linear interpolation shall be allowed for figures within the tables shown.
10. Panels with fan beams shall be considered equivalent to similar panels without fan beams. Design professionals may include the strength of the fan beam to exceed shown figures as part of site-specific engineering.



EPS ROOF PANEL/ SPAN DESCRIPTION

DO KIM
& ASSOCIATES, LLC
CONSULTING STRUCTURAL ENGINEERS
PO BOX 10039
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Fax: (813) 374-0322

Rev./Date		Description
0	8/21 2006	ISSUED FOR PRODUCT APPROVAL
1	8/26 2008	ISSUED FOR 2007 PRODUCT APPROVAL
△		
△		
△		

Elite Aluminum Corporation
4650 Lyons Technology Parkway
Coconut Creek, FL 33073

EPS FOAM CORE COMPOSITE PANELS
ALUMINUM/ALUMINUM SKIN
FLORIDA STATEWIDE PRODUCT APPROVAL

DRAWN BY:	DYK
CHECKED BY:	DYK
SCALE:	AS SHOWN
DATE:	9/19/06

DO YEON KIM, P.E.
FLA. REG. NUMBER 49497
DO KIM & ASSOCIATES, LLC
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9-3-08

Drawing No. - FL-1001

SHEET 1 OF 1