

FLOOR PLAN SCALE: 1/4 IN. = 1 FT.

ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion—resistant wire mesh, wit h 1 / 8 inch (3.2 mm) minimum to 1 inch (6.4 mm) maximum openings.

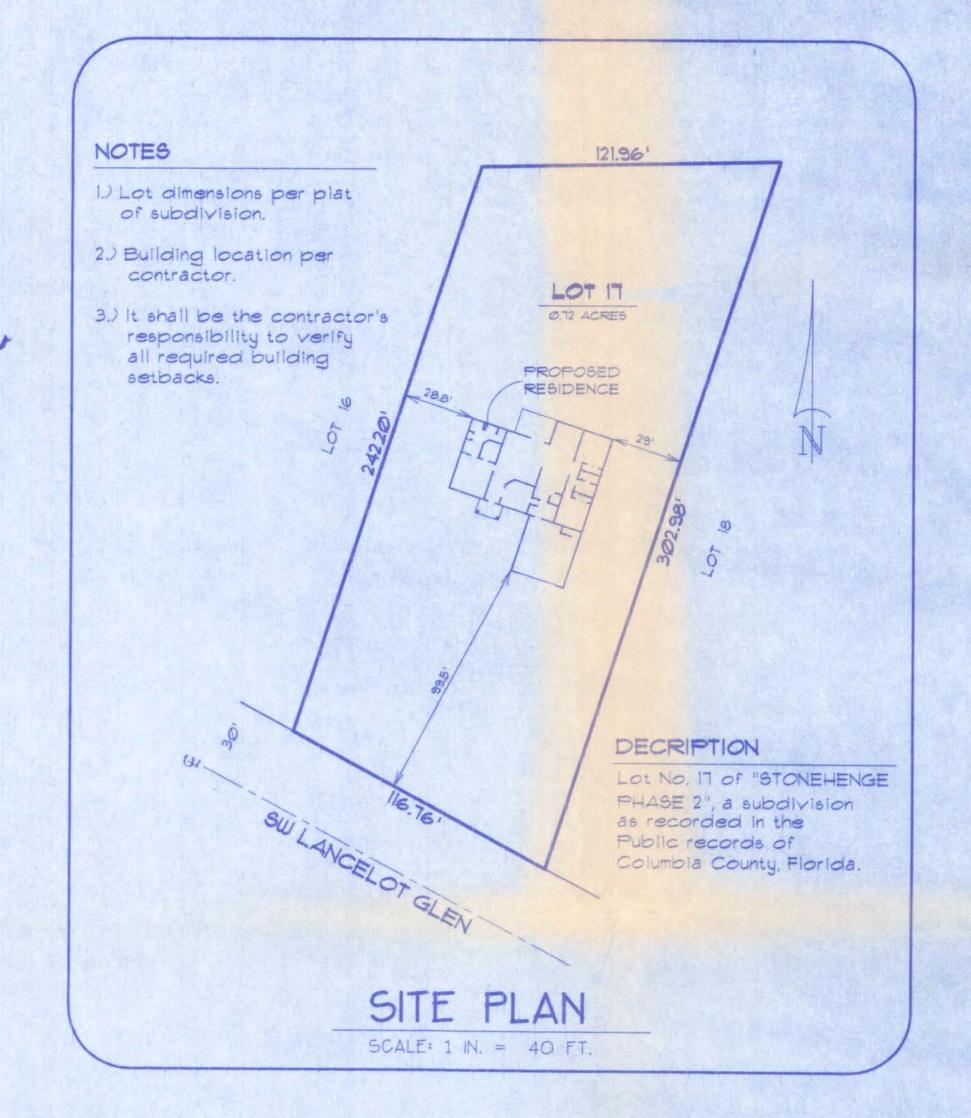
The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper partian of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

Index to Sheets

SHEET A-1	 SITE PLAN + FLOOR PLAN + ELEVATIONS
SHEET A-2	 ELEVATIONS + GEN. NOTES
SHEET A-3	 FOUNDATION + SECTIONS
SHEET A-4	 ELECTRICAL
SHEET S-1	 WIND ENGINEERING

Model Home

STONEHENGE PHASE 2 - LOT 17





FRONT ELEVATION SCALE: 1/4 IN. = 1 FT.

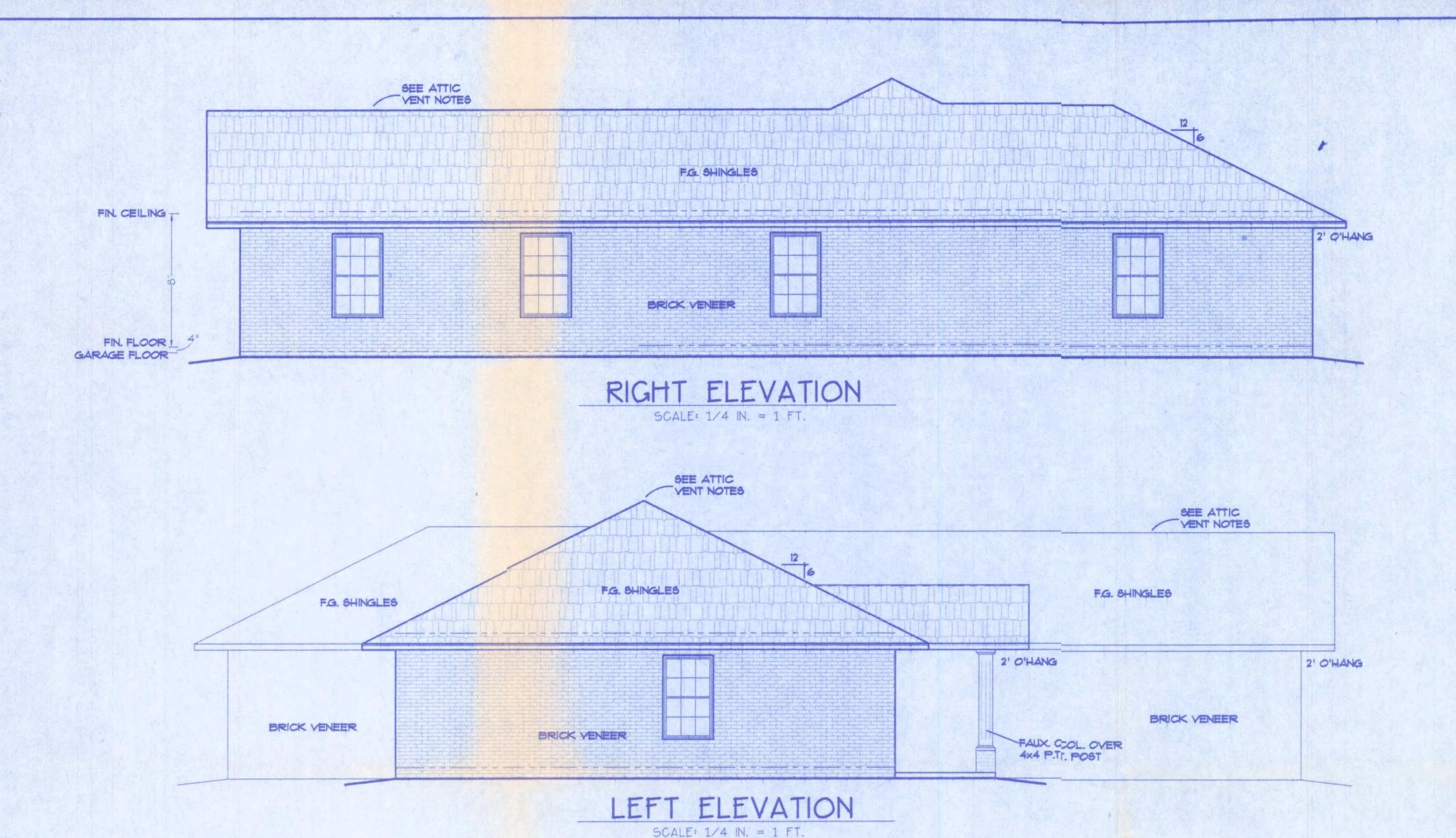
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: LOT 17 - STONEHENGE PH 2 Job No.: 607072

FILE: 06-011 DATE: 2-6-06	JONATHAN PERRY CONST.	SHEET: 1 OF 4 CAD FILE: OGD11
DRAWN: T A D CHECK: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services 192 SW Sagewood Gin., Lake City, FL 32024 Phone (386) 755-5891	REV: 12-9-04 REV:



SEE ATTIC

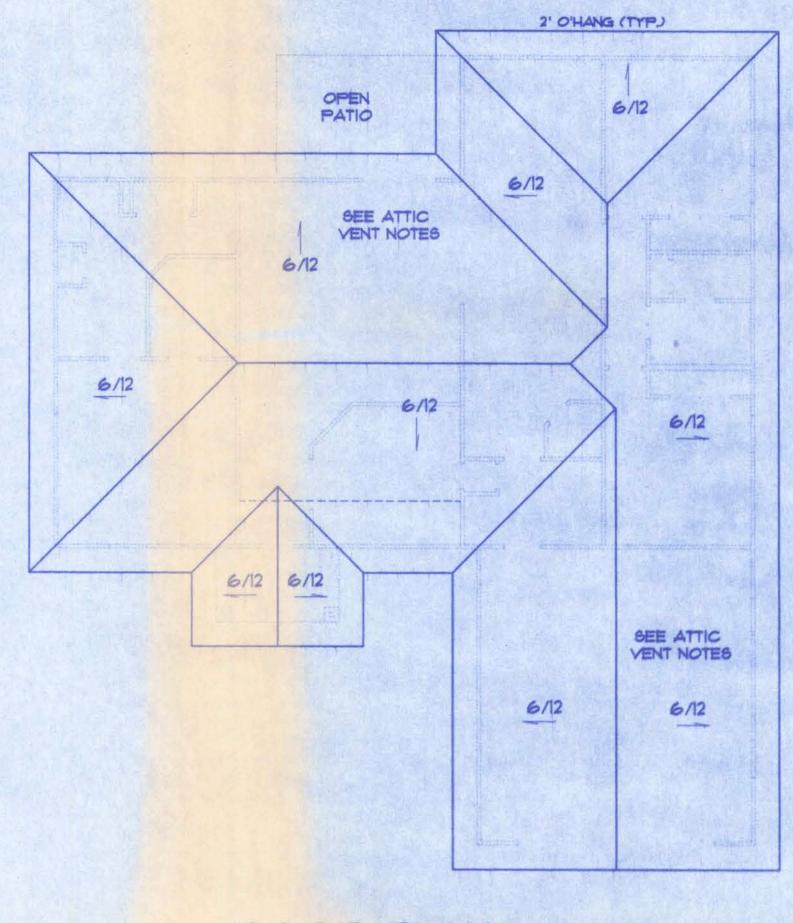
VENT NOTES

F.G. SHINGLES

BRICK VENEER

FIN CEILING

FIN. FLOOR



SCALE: 1/8 IN. = 1 FT.

GENERAL NOTES

- 1.) See 'Wind Load Detail Sheet S-1' and Wind Engineer's Notes for II G.) The Truss Manufactuer shall prepare Shop Drawings indicating data pertaining to Wind Design and compliance w/ Florida Building Code.

 Truss placement, Girder locations, Truss-to-Truss Connections
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- The Truss Manufactuer shall prepare Shop Drawings indicating Truss placement. Girder locations. Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

ATTIC VENTILATION

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The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not mare than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

SCALE: 1/4 IN. = 1 FT.

REAR ELEVATION

F.G. SHINGLES

APPROX. RIDGE

HT .= 17'-0"

BRICK VENEER

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LOT 17 - STONEHENGE PH 2

2' O'HANG

Job No.: 602072

FILE:

OG-O11

DATE:

2-G-OG.

PREPARED BY:

T A D

PREPARED BY:

TIM DELBENE

Drafting + Technical Services

192 SW Sagewood Gin, Lake City, FL 32024

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SHEET:

2 OF 4

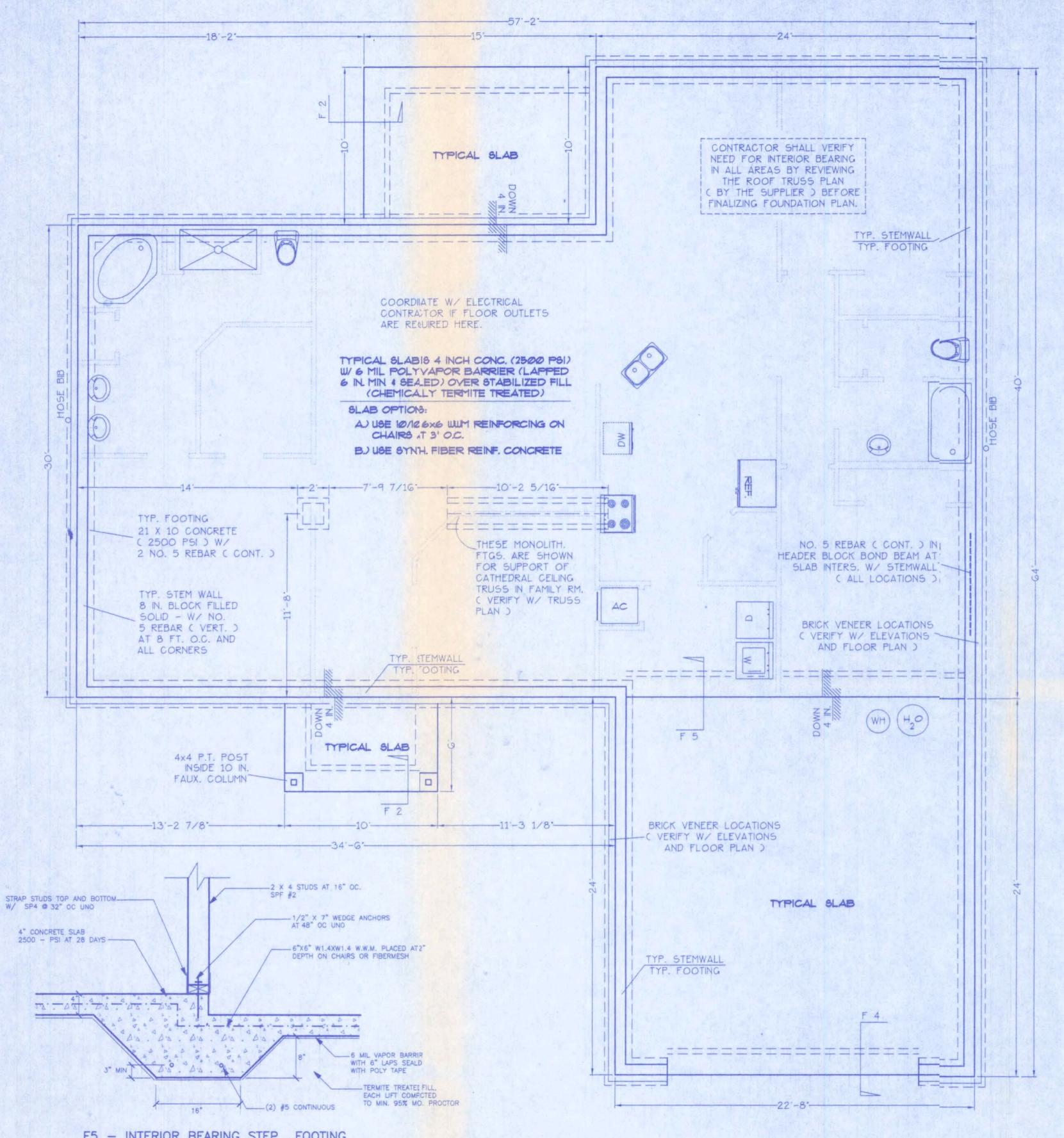
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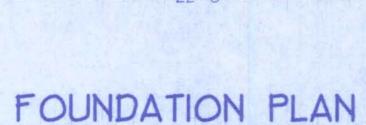
A-2



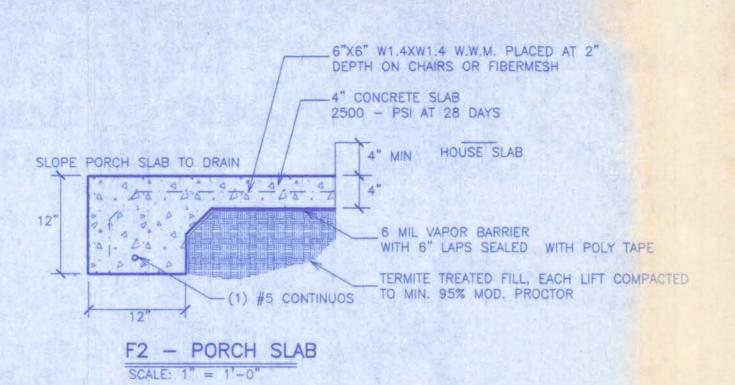
F5 - INTERIOR BEARING STEP FOOTING

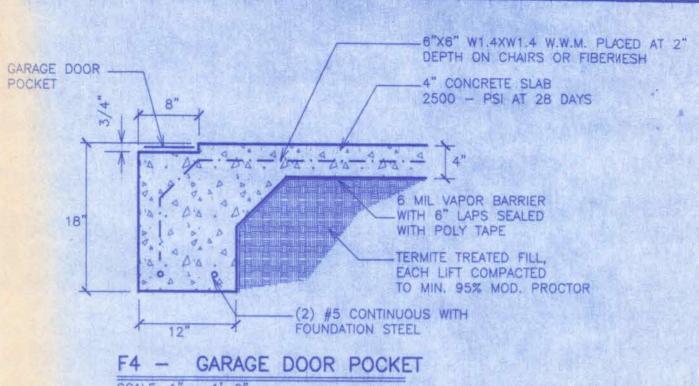
FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN C BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBLITY OF THE CONTRACTOR / OWNER.



SCALE: 1/4 IN. = 1 FT.





STRUCTURAL SHEATHING VENT -15 LB. FELT PAPER PRE-ENGINEERED WOOD FIBERGLASS SHINGLES TRUSSES - 24 IN. O.C.-(DESIGN BY SUPPLIER) TRUSS ANCHOR - EA. TRUSS PER STRUCTURAL ENGINEER R-30 F.G. INSULATION-1/2 IN. GYP. BOARD-ALUM. DRIP EDGE DBL. 2 X 4 PLATE--2 X FASCIA W/ VINYL TRIM STUD/PLATE ANCHORS VENTED VINYL SOFFIT IF REQUIRED (24 IN. OVERHANG) PER STRUCTURAL ENGINEER -R-13 F.G. INSULATION INTERIOR FINISHES -STRUCTURAL SHEATHING PER OWNER 1/2 IN. GYP. BOARD-BRICK VENEER - ANCHORED 2 X 4 WOOD - W/ GALV. MTL. STRAPS AT STUDS - 16 IN. O.C. 16 IN. O.C. EA. WAY # 5 REBAR CONT. IN CMU HEADER BLOCK ANCHOR BOLTS - SIZE / TYPE + BOND BEAM AT SLAB INTERS. W/STEMWALL SPACING PER STRUCT. ENGN'R. # 5 REBAR (VERT.) SPACED PER STRUCT, ENG'R. SGL. 2 X 4 P.T. PLATE -AND AT ALL CORNERS. STANDARD ACI STUD/SILL ANCHOR -HOOK AT TOP + BOTTOM. GRADE 40 STEEL. IF REQUIRED 8 X 8 X 16 CONC. BLOCK STEMWALL PER STRUCT. ENGN'R. W/ CELLS FILLED SOLID 4 INCH CONCRETE SLAB (2500 _ GRADE PSI MIN.) OVER 6 MIL. POLY VAPOR BARRIER AND CHEMICALLY TERMITE-TREATED COMPACTED FILL C SEE SLAB OPTIONS NOTE BELOW) CONCRETE FOOTING (2500 PSI) ___ SIZE PER STRUCT. ENG'R. 6 MIL POLY VAPOR BARRIER W/ WITH 2 - # 5 REBAR (CONT.) ALL SEAMS + PENETRATIONS-TAPED (LAP SEAMS G INCHES)

SLAB OPTIONS:

OPTION 1 - Use 6x6 10/10 WWM reinforcing on chair supports at 3' O.C.

OPTION 2 - Use Synthetic Fiber reinforced

concrete.

WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

DESIGN WALL SECTION

NON-STRUCTURAL DATA

SCALE: 1/2 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

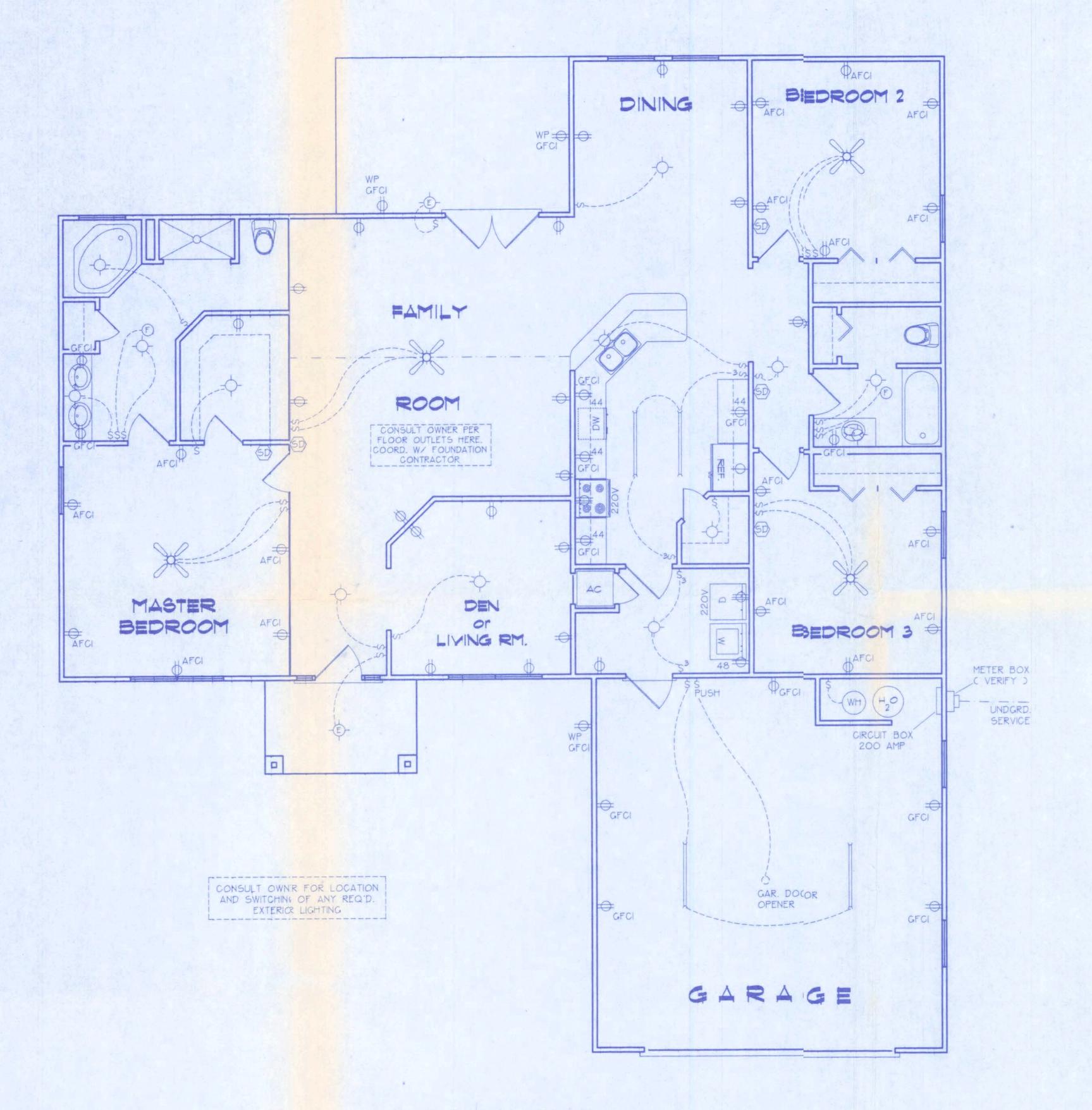
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LOT 17 - STONEHENGE PH 2

602072

FILE: 06-011 DATE: 2-6-06	JONATHAN PERRY CONST.	SHEET: 3 OF 4 CAD TILE: CGO11
TAD	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK:	192 SW Sagewood Gln. Lake City, FL 32024 Phone (386) 755-5891	REV:



ELECTRICAL PLAN NOT TO SCALE

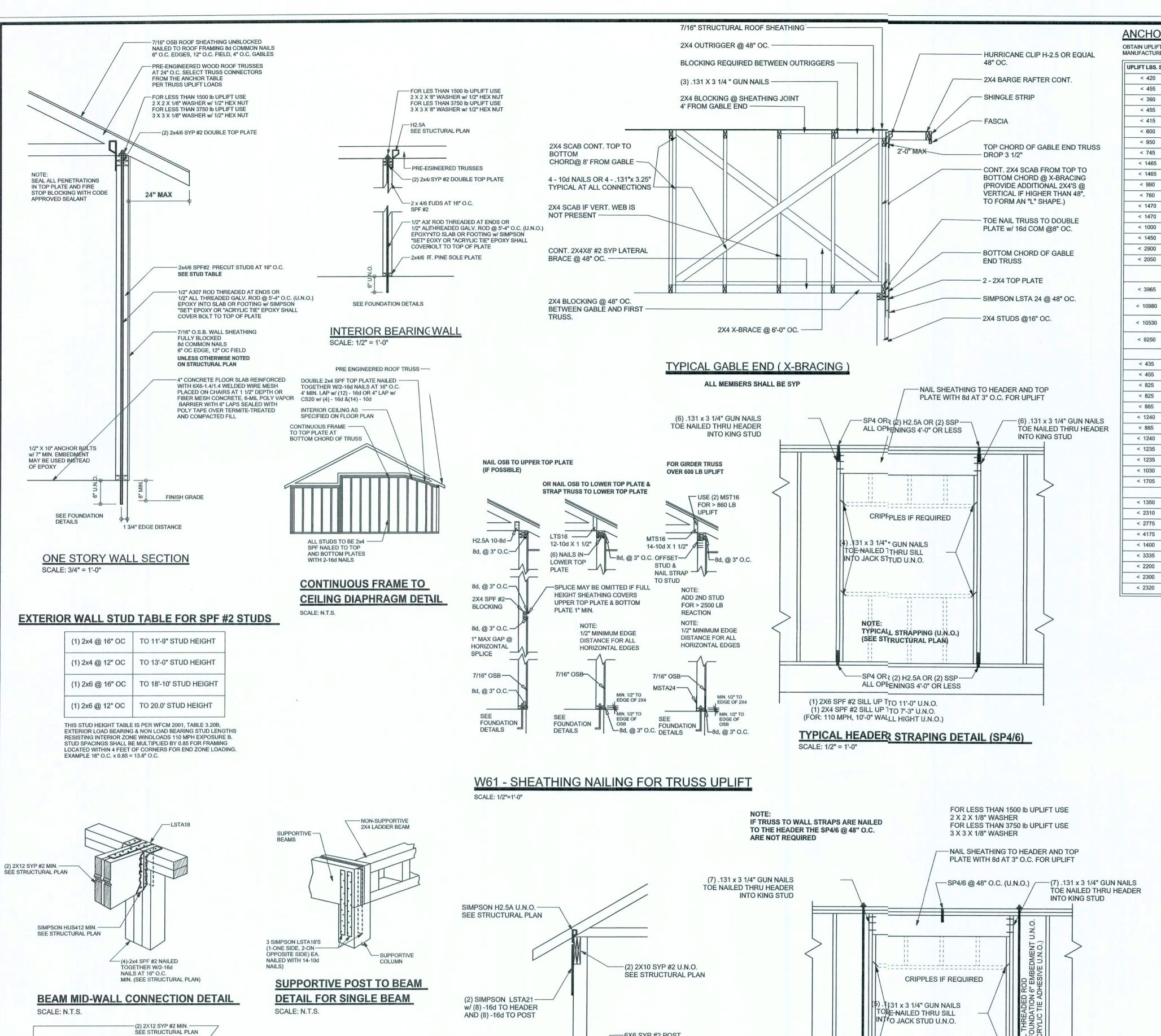
ELECTRICAL SYMBOL LEGEND

	= FLOURESCENT LIGHTING FIXTURE.
	= CEILING LIGHT FIXTURE
-É-	= EXTERIOR LIGHTING
ş	= LIGHT SWITCH.
\$ ₃	= THREE-WAY SWITCH.
ф	= 110 V. DUPLEX OUTLET.
42	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
⇔ GFCI	= GROUND FAULT CIRC. OUTLET
Φ ^{AFCI}	= ARC FAULT CIRC. OUTLET
ф	= 110 V. SINGLE REGEPTAGLE OUTLET.
€220V	= 220 VOLT OUTLET (4 WIRE)
X	= FAN LOGATION C CEILING 3
•	= FAN LOCATION C EXHAUST 3
(SD)	= SMOKE DETECTOR

ELECTRICAL PLAN NOTES

- -WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- -CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- -ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- -ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- -TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS. + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- -ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- -ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
 TO BE DETERMINED BY POWER COMPANY.

3	FILE:	JONATHAN	SHEET:
	06-011 DATE:		CAD FILE:
١	2-6-06		06011
0	DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
1	CHECK: T A D	192 SW Sagewood Gln., Lake City, FL 32024 Phone (386) 755-5891	REV:



-6X6 SYP #2 POST

SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

ANCHOR BOLT

LSTA24

NAIL THRU 2x4 INT BEAM W/4-16d

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

- SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

SUPPORTIVE BEAM ----

3-1/2" P.T.

SUPPORTIVE CENTER POST TO BEAMDETAIL

IF BEAM JOINT IS AT ---

4-SIMPSON LSTA18 -

(2-ONE SIDE,2-ON OTHER SIDE)

POST CONNECTION,

LSTA18 ON ONE SIDE

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

< 2320

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
- 2220	- 0200	ADUIDO	10 101		7 202 72

GENERAL NOTES:

IRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND SPECIFICALLY NO	OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDIT BACKFILL HEIGHT, WIN	ONS, FOUNDATION BEARING CAPACITY, GRADE AND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS A REQUIREMENTS FOR T	ID CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 E STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUO BELIEVE THE PLAN OM THE WIND LOAD ENGIN	S LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU S A CONTINUOUS LOAD PATH CONNECTION, CALL ER IMMEDIATELY.
DESIGN, PLACEMENT P	UFACTURER'S SEALED ENGINEERING INCLUDES TRUSS ANS, TEMPORARY AND PERMANENT BRACING DETAILS, ECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

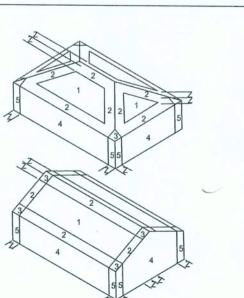
BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRE LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

2-5/8" AB

BUILDING IS NOT IN THE HIGH VELOCITY HURI BUILDING IS NOT IN THE WIND-BORNE DEBRIS 1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II					
 BASIC WIND SPEED = 110 MPH WIND EXPOSURE = B WIND IMPORTANCE FACTOR = 1.0 BUILDING CATEGORY = II 	REGION				
2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II					
3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II					
4.) BUILDING CATEGORY = II					
E) BOOF ANOLE - 40 45 DEODEED					
5.) ROOF ANGLE = 10-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT = N/A	(ENCLOSED B	UILDING)		
8.) COMPONENTS AND CLADDING DESIGN W	IND PRESSURI	ES (TABI	E R30	1.2(2))	
		I man			
**	Zone	Effective 10	Wind A	100	
	1	19.9 -21	8 18 1		



2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors Wors (Zone	st Cas	е	21.8	-29.1
8x7 Gar	age D	oor	19.5	-22.9
16x7 Ga	rage [Door	18.5	-21.0

DESIGN LOADS

OOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	

10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

REVISIONS

SOTTPLYN

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INDLOAD ENGNEER: Mark Disosway,

PE No.53915, PCB 868, Lake City, FL

Stated dimensions supercede scaled

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dimensions. Refe all questions to Mark Disosway, f.E. for resolution.

Do not proceed without clarification.

32056, 386-754-5419

Jonathan Perry

Spec House Lot 17

ADDRESS: Lot 17Stonehenge S/D

Stonehenge S/D Phase I

Phase II Colimbia County, Florida Mark Disosway P.E. P.0. Box 868

Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (336) 269 - 4871

PRNTED DATE:

February 21, 2006 DRAWN BY: STRUCTURAL BY David Disosway

FINALS DATE 21 / Feb / 0€

> JOBNUMBER: 602072 DRAVING NUMBER

> > **S-1** OF 3 SHEETS

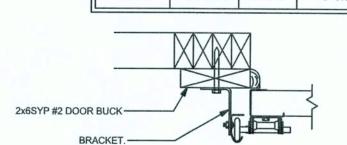
GRADE & SPECIES TABLE

18 - 16d

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

TYPICAL STRAPPING (U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

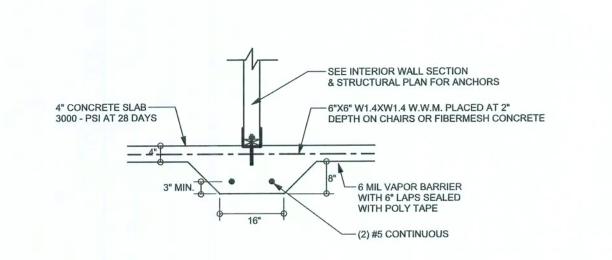
(SEE STRUCTURAL PLAN)

(1) 2X6 SPF #22 SILL UP TO 7'-6" U.N.O.

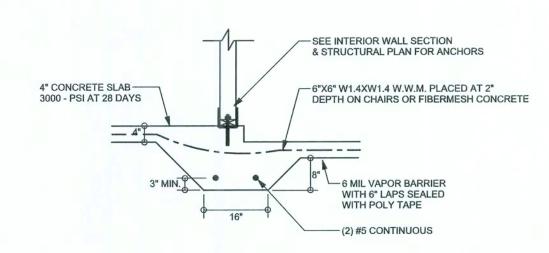
(2) 2X4 SPF #22 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #22 SILL UP TO 5'-1" U.N.O.

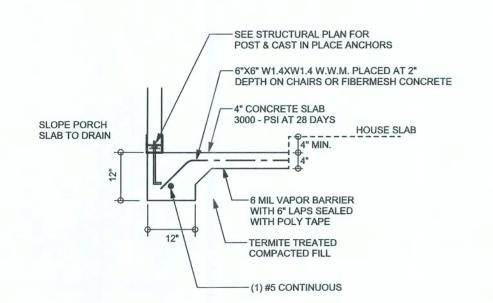
(FOR: 120 MPH, 1₁₀'-0" WALL HEIGHT U.N.O.)



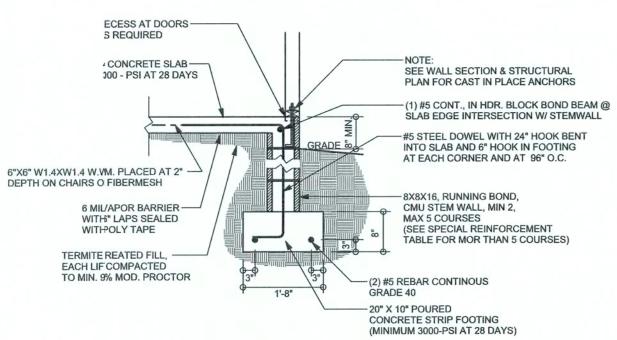
F2 INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



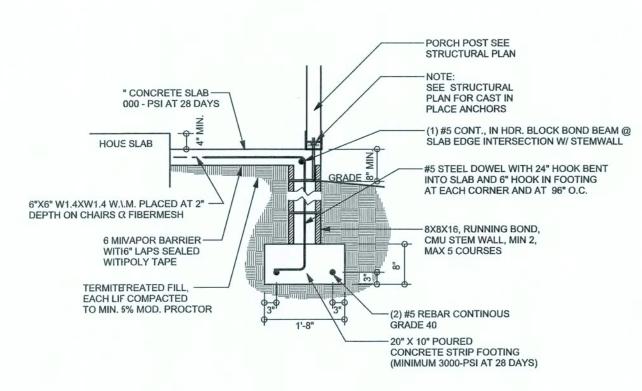
F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



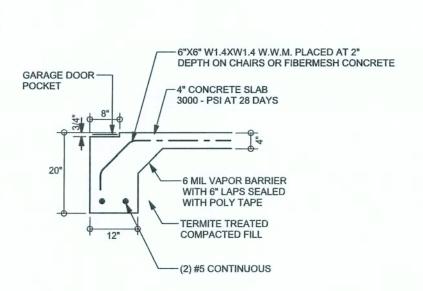
F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



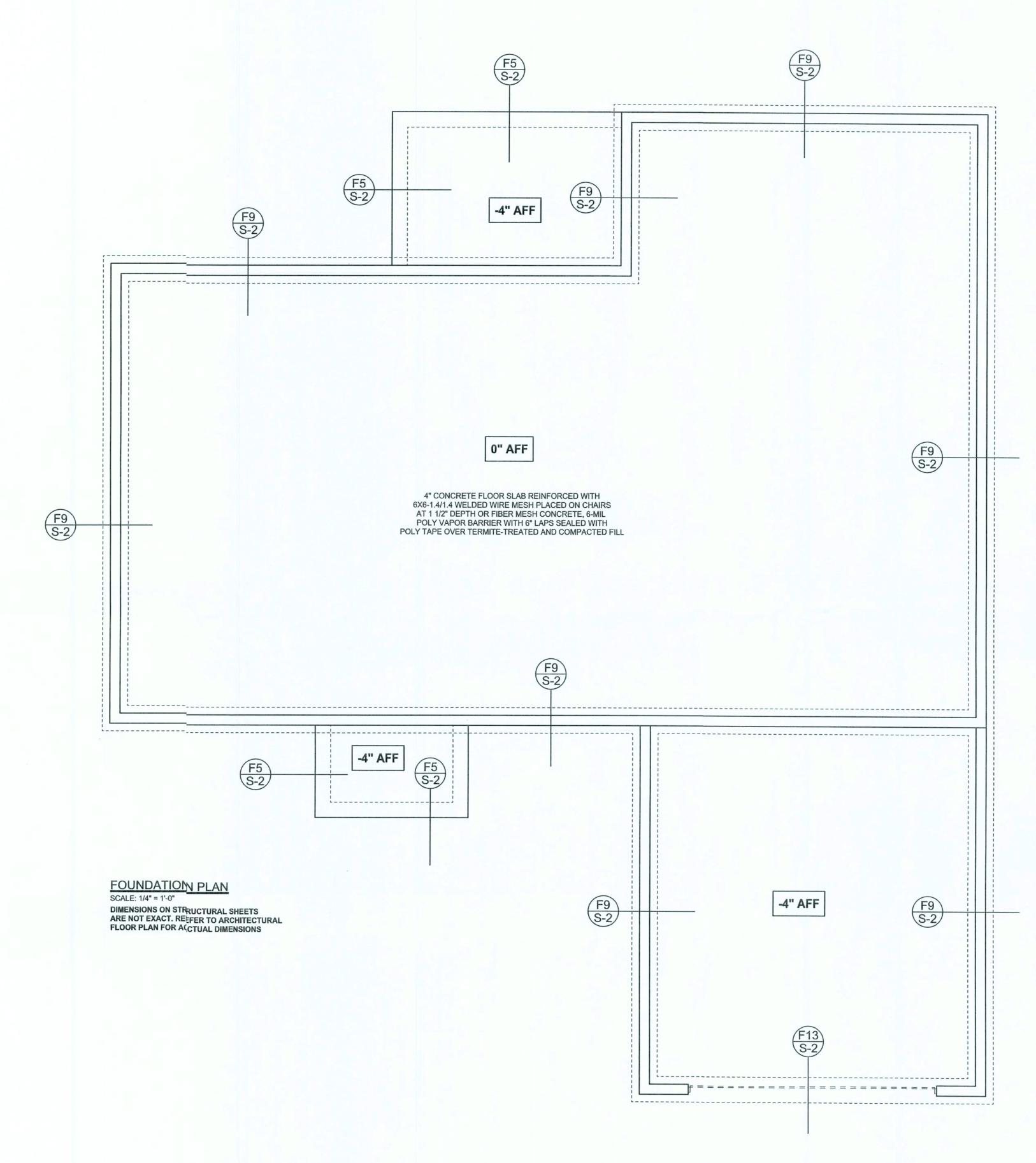
F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING SCALE: 1/2" = 1'-0"



F13 ALT. STEM WALL GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"



REVISIONS

SOFTPIAN ARGITECTURAL DESIGN SOFTWAR

WINDLOAD INGINEER: Mark Disosway, PE No.53915 POB 868, Lake City, FL 32056, 386-7;4-5419

Stated dimensions supercede scaled dimensions. Lefer all questions to Mark Disoswy, P.E. for resolution. Do not proced without clarification.

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CERTIFICATON: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering.

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LIMITATION:This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Jonathan Perry

Spec House
Lot 17
Stonehange S/D Phase II

ADDRESS: Lot17 Stonehenge S/D Phase II Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
February 21, 2006

DRAWN FY: STRUCTURAL BY

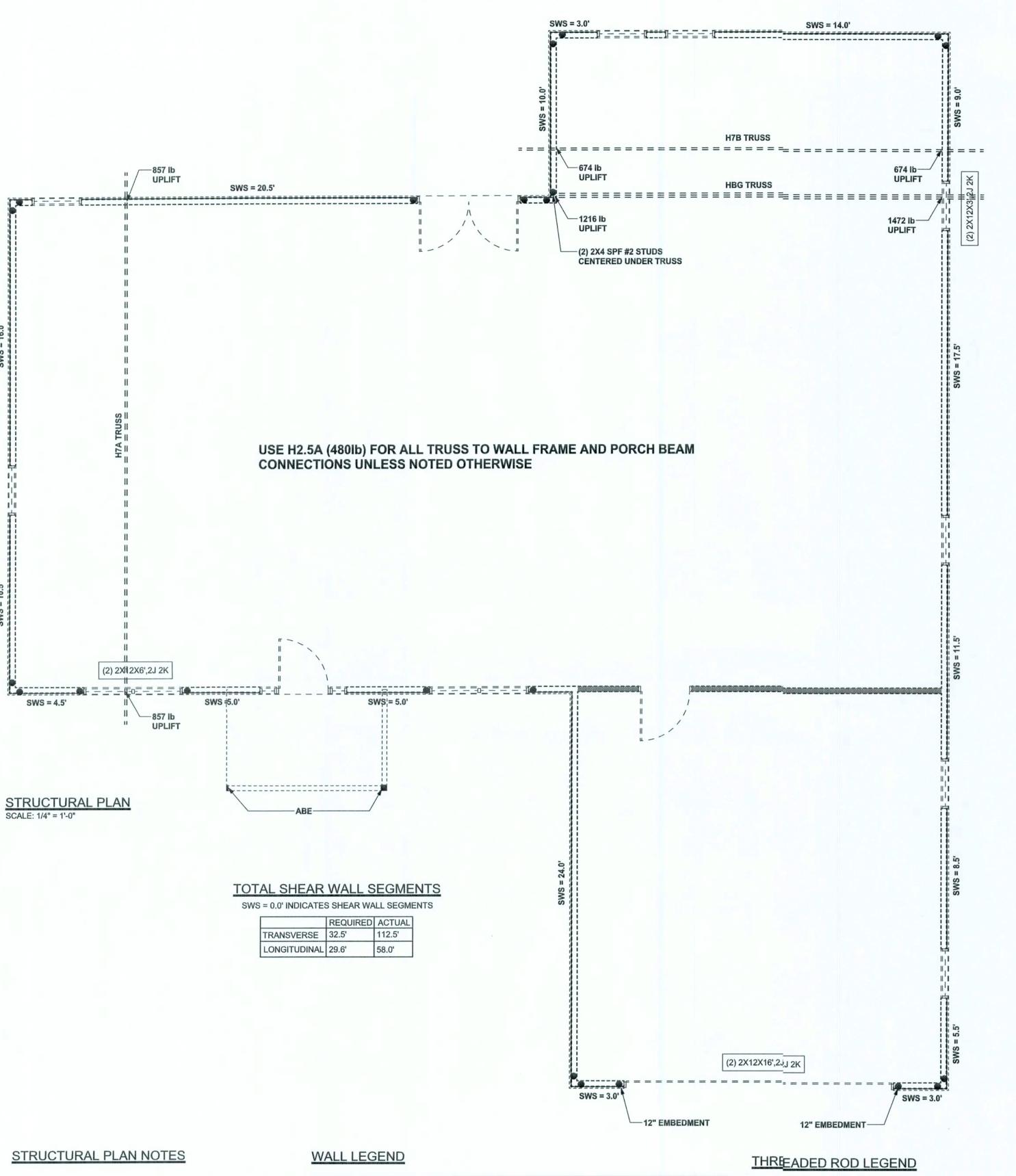
David Disosway

FINALS DATE:

JOB NUMBER: 602072

DFAWING NUMBER

S-2
OF 3 SHEETS



MSTA30, 10-10d (1700lb)
(5) NAILS EACH SIDE OF STUD
(OR STRAP STUD TO HEADER 20-10d)

LTT20B, 10-16d (1750lb)

1/2" ANCHOR w/ 6" EMBEDMENT U.N.O., SIMPSON AT (MAY BE RECESSED BELOW FINISHED FLOOR)

ALTERNATE WALL TIE CONNECTION WHERE
THREADED ROD CANNOT BE PLACED IN WALL.
SCALE: 1/2" = 1'-0"

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, PDB 868, Lake City, FL 32056, 386-7545419

REVISIONS

SCFTPIAN

DIMENSIONS: Stated dimensions supercede scaled dimensions. Rear all questions to Mark Disosway,P.E. for resolution. Do not proceed without clarification.

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MARK DISOSWAY
P.E. 53915

A SEAL

Jorathan Perry

Spec House
Lot 17
Stonehenge S/D Phase II

ADDRESS:
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Phase II Cdumbia County, Florida

MarkDisosway P.E. P.O. Box 868 Lake Cty, Florida 32056 Phone:(386) 754 - 5419 Fax: (386) 269 - 4871

RINTED DATE:
Fetruary 21, 2006

DRAWN BY STRUCTURAL BY:
David Disosway

FINALS DA'E: 21 / Feb / I6

CONNECTIONS, WALL, & HEADER DESIGN IS BASED

(JOB #6-064)

ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO.

JOE NUMBER: 602072 DRAWING NUMBER

S-30F 3 SHEETS

SN-1 ALL LOAD BEARING FRAME WALL & PORCHEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.I.O.)

SN-2

ALL LOAD BEARING FRAME WALL HEADERS
SHALL HAVE (1) JACK STUD & (1) KING STUI
EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4

PERMANENT TRUSS BRACING IS TO BE INSALLED AT LOCATIONS AS SHOWN ON THE SEALED TFJSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED ER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSB2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

SWS = 0.0'

SWS = 0.0'

2ND FLOOR EXTERIOR

IBW

SEE DETAILS ON SHEET S-1

2ND FLOOR INTERIOR BEARING WALLS
SEE DETAILS ON SHEET S-1

INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

(2) 2X12X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.)

NUMBER OF KING STUDS (FULL LENGTH)

NUMBER OF JACK STUDS (UNDER HEADER)

SPAN OF HEADER

SIZE OF HEADER MATERIAL

NUMBER OF PLIES IN HEADER