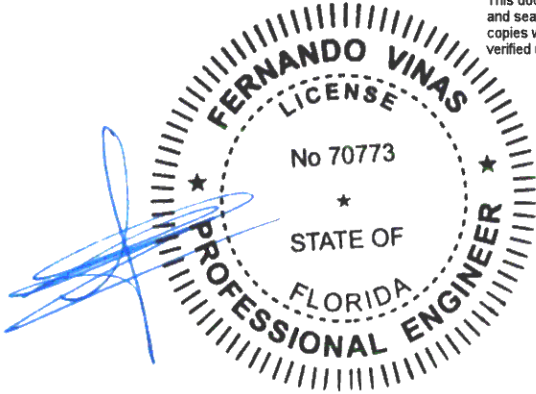


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Alpine, an ITW Company
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www.alpineitw.com



06/07/2022

COA#0-278
Florida Certificate of Product Approval #FL1999



Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	Job Number: B55647a
Job Description: WENTWORTH RESIDENCE	
Address: FL	

Job Engineering Criteria:
Design Code: FBC 7th Ed. 2020 Res.
IntelliVIEW Version: 21.01.03A
JRef #: 1XG68570009
Wind Standard: ASCE 7-16 Wind Speed (mph): 120
Design Loading (psf): 37.00
Building Type: Closed

This package contains general notes pages, 40 truss drawing(s) and 8 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	157.22.1646.56257	A1 34'4" Steardown Hip	2	157.22.1646.50820	A2 34'4" Steardown Hip
3	157.22.1646.46587	A2a 34'4" Steardown Hip	4	157.22.1646.40060	A3 34'4" Steardown Hip
5	157.22.1646.38380	A3a 34'4" Steardown Hip	6	157.22.1646.36850	A4 34'4" Steardown Hip Girder
7	157.22.1646.35253	A4a 34'4" Steardown Hip	8	157.22.1646.33490	A5 34'4" Steardown Hip Girder
9	157.22.1646.31590	B1 31' Common Girder	10	157.22.1646.29873	B2 31' Common
11	157.22.1646.28030	C1-DG 25' Gable	12	157.22.1646.22463	C2 25' Common
13	157.22.1646.16860	C3 25' Common	14	157.22.1646.13243	C3a 25' Common
15	157.22.1646.04583	C3s 24'6"8 Common	16	157.22.1646.02840	C4-SDG 25' Gable
17	157.22.1645.54880	D1-SDG 21'4" Gable	18	157.22.1645.53050	D2 21'4" Scissor
19	157.22.1645.51337	D2a 21'4" Scissor	20	157.22.1645.49297	D2b 21'4" Scissor
21	157.22.1645.47550	E1-DG 13'2" Gable	22	157.22.1645.45883	E2 13'2" Common
23	157.22.1645.44127	E3 21'3"8 Common	24	157.22.1645.42200	G1-DG 12'6" Gable
25	157.22.1645.10797	V1 27'10"7 Valley	26	157.22.1645.09300	V2 23'10"7 Valley
27	157.22.1645.07930	V3 19'10"7 Valley	28	157.22.1645.06520	V4 15'10"7 Valley
29	157.22.1645.04970	V5 11'10"7 Valley	30	157.22.1645.03403	V6 7'10"7 Valley
31	157.22.1644.59550	V7 7'10"7 Valley	32	157.22.1645.26103	PB1 6'4" Common
33	157.22.1645.22167	PB2 6'4" Common	34	157.22.1645.40640	JA 11'3" Hip Jack Girder
35	157.22.1645.39003	JB 8'5"1 Hip Jack Girder	36	157.22.1645.36633	JC 8' End Jack
37	157.22.1645.34277	JD 6' End Jack	38	157.22.1645.32680	JE 5'11"11 Jack
39	157.22.1645.30523	JF 3'11"11 Jack	40	157.22.1645.28207	JG 1'11"11 Jack
41	A12030ENC160118		42	GABRST160118	
43	GBLLETIN0118		44	PB160160118	
45	PB180160118		46	REPCHRD1014	
47	VALTN160118		48	A12015ENC160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

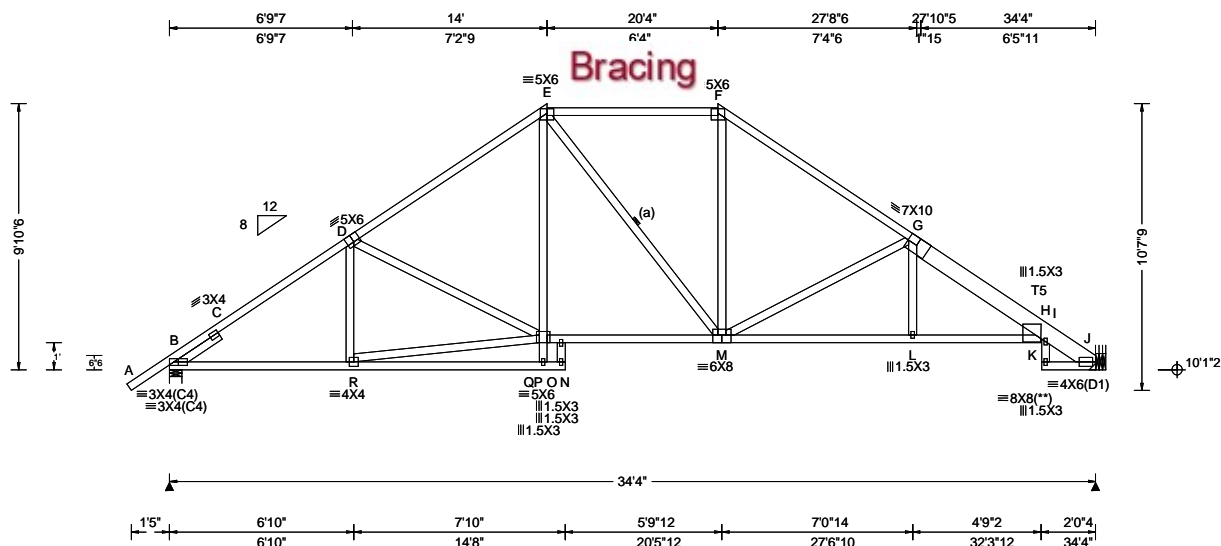
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 125491 FROM: RJL	HIPS Qty: 6	Ply: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A1 34'4" Stepped Hip	Cust: R 857 JRRef: 1XG68570009 T26 DrwNo: 157.22.1646.56257 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.157 L 999 360 VERT(CL): 0.297 L 999 240 HORZ(LL): 0.137 K - - HORZ(TL): 0.260 K - - Creep Factor: 2.0 Max TC CSI: 0.545 Max BC CSI: 0.515 Max Web CSI: 0.988 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1410 - / - / /748 /13 /174 J 1312 - / - / /685 /2 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.7 J Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 246 -2066 F - G 188 -1681 C - D 132 -1879 G - H 148 -2257 D - E 189 -1671 H - I 57 -653 E - F 197 -1289 I - J 55 -720

Lumber

Top chord: 2x4 SP #1; T5 2x8 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

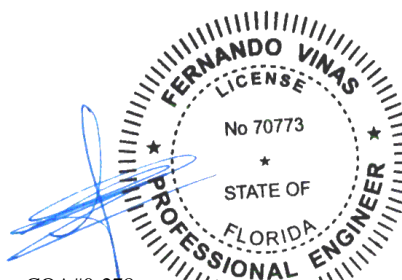
(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



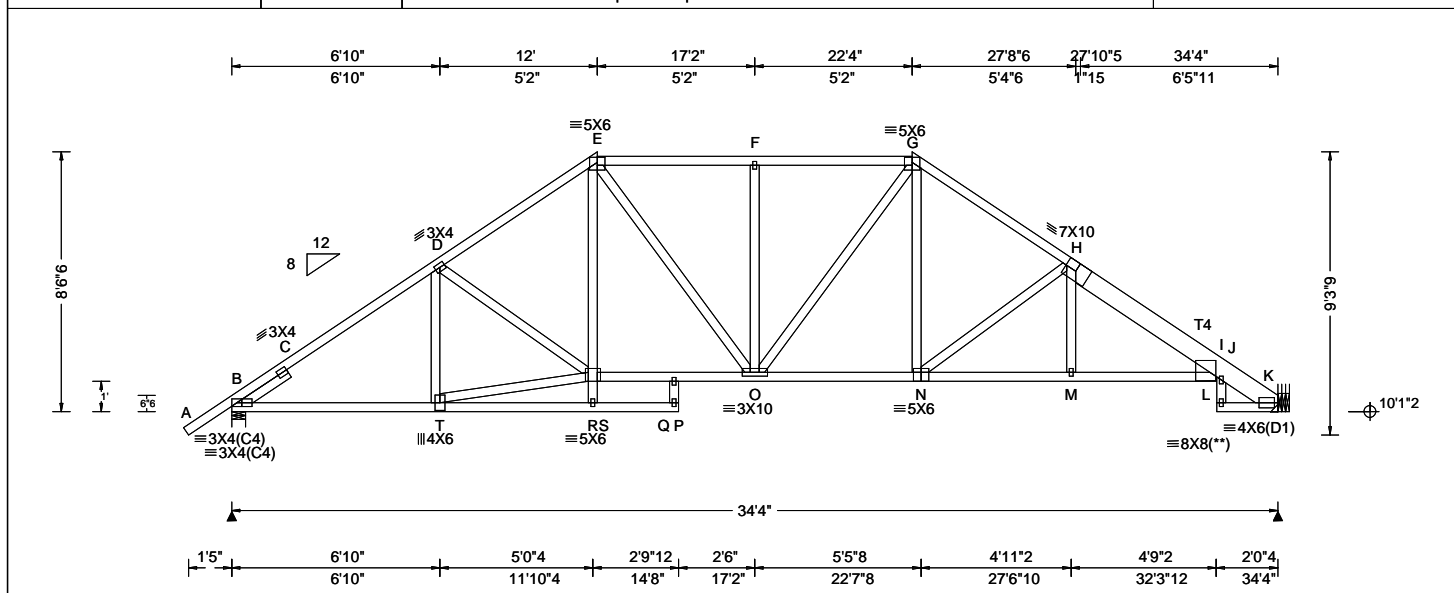
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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125499 FROM: RJL	HIPS Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A2 34'4" Stepped Hip	Cust: R 857 JRRef: 1XG68570009 T36 DrwNo: 157.22.1646.50820 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.167 M 999 360 VERT(CL): 0.307 M 999 240 HORZ(LL): 0.146 L - - HORZ(TL): 0.270 L - - Creep Factor: 2.0 Max TC CSI: 0.570 Max BC CSI: 0.462 Max Web CSI: 0.580 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1445 - / - / 743 / 17 / 152 K 1368 - / - / 680 / 5 / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.7 K Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 305 -2141 G - H 252 -1948 C - D 188 -1921 H - I 213 -2336 D - E 256 -1878 I - J 75 -683 E - F 275 -1665 J - K 74 -752 F - G 275 -1665

Lumber

Top chord: 2x4 SP #1; T4 2x8 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

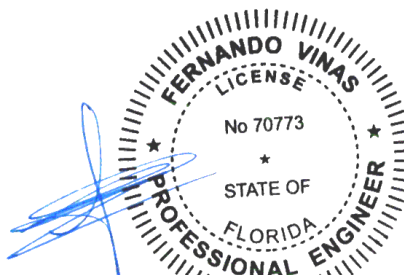
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



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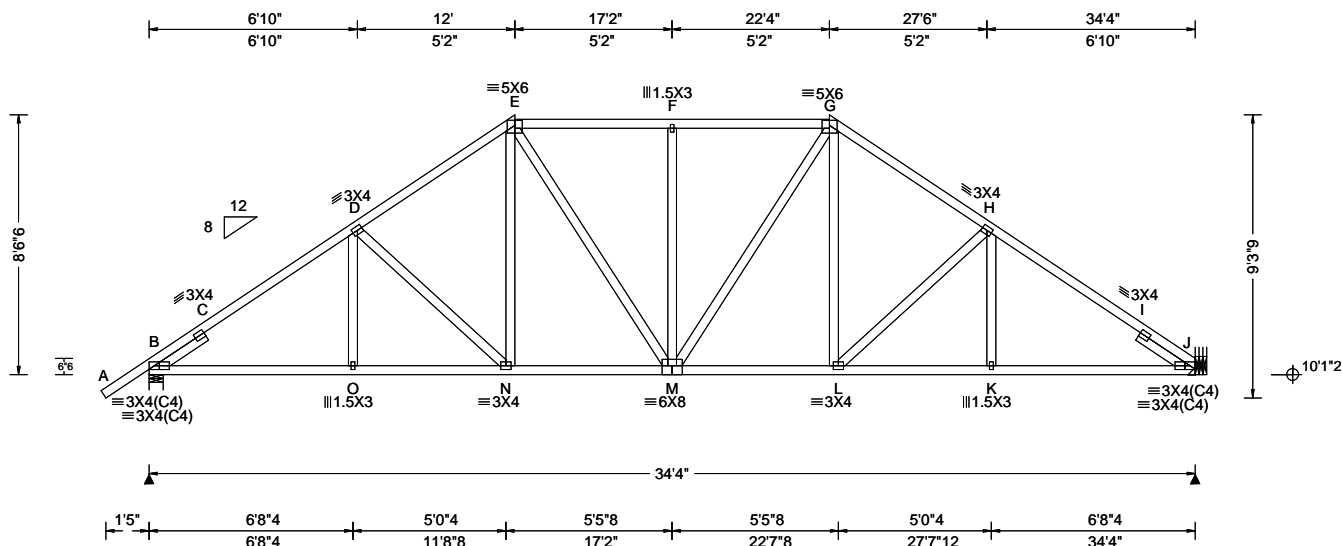
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125493 FROM: RJL	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A2a 34'4" Stepped Hip	Cust: R 857 JRef: 1XG68570009 T38 DrwNo: 157.22.1646.46587 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.083 F 999 360 VERT(CL): 0.148 F 999 240 HORZ(LL): 0.041 J - - HORZ(TL): 0.073 J - - Creep Factor: 2.0 Max TC CSI: 0.348 Max BC CSI: 0.402 Max Web CSI: 0.429 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1515 - / - / - / 743 - / 152 J 1416 - / - / - / 680 - / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 J Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 302 -2187 F - G 262 -1563 C - D 187 -2033 G - H 244 -1782 D - E 243 -1768 H - I 192 -2070 E - F 262 -1563 I - J 302 -2178

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Hangers / Ties

(J) Hanger Support Required, by others

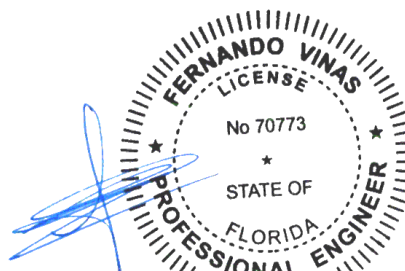
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	1625 -104	M - L	1401 -63
O - N	1623 -104	L - K	1667 -100
N - M	1394 -72	K - J	1669 -99

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
N - E	389 0	G - L	417 0

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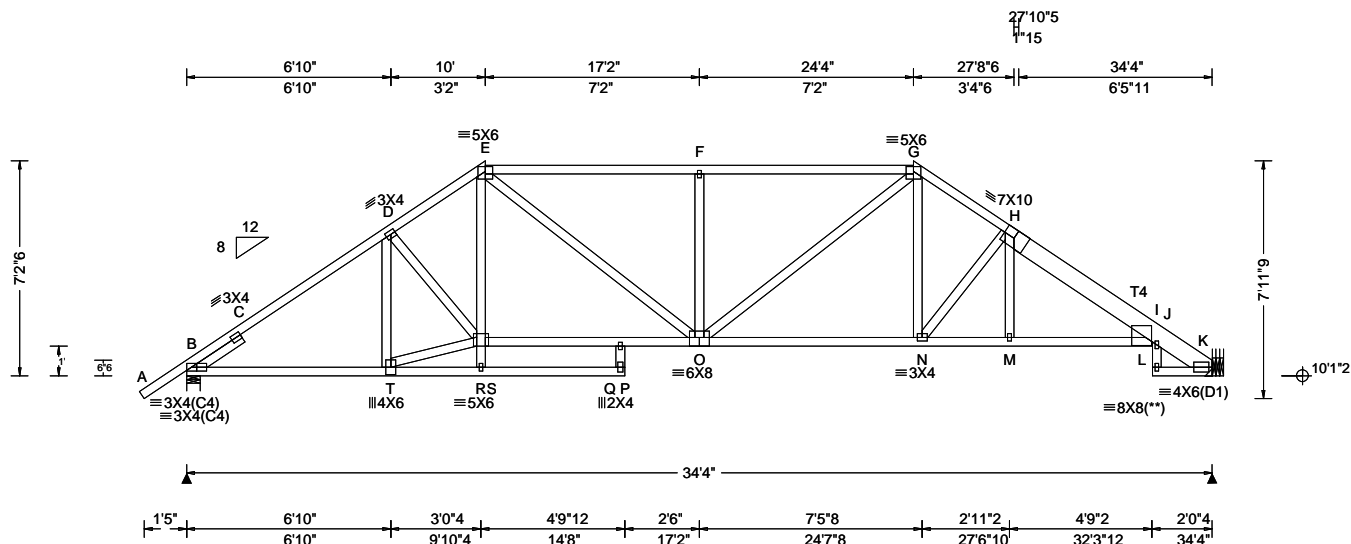
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125501 FROM: RJL	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A3 34'4" Stepped Hip	Cust: R 857 JRRef: 1XG68570009 T17 DrwNo: 157.22.1646.40060 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.175 N 999 360 VERT(CL): 0.323 N 999 240 HORZ(LL): 0.150 L - - HORZ(TL): 0.278 L - - Creep Factor: 2.0 Max TC CSI: 0.573 Max BC CSI: 0.562 Max Web CSI: 0.596 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1440 - / - / /735 /20 /130 K 1374 - / - / /672 /8 - /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.7 K Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 357 -2168 G - H 316 -2144 C - D 246 -1905 H - I 280 -2327 D - E 324 -2033 I - J 93 -687 E - F 376 -2070 J - K 93 -755 F - G 376 -2070

Lumber

Top chord: 2x4 SP #1; T4 2x8 SP SS Dense;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

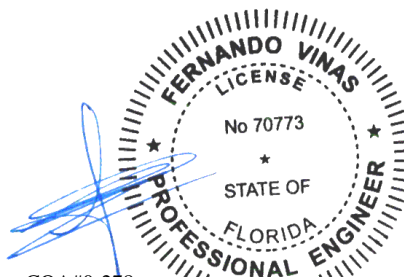
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



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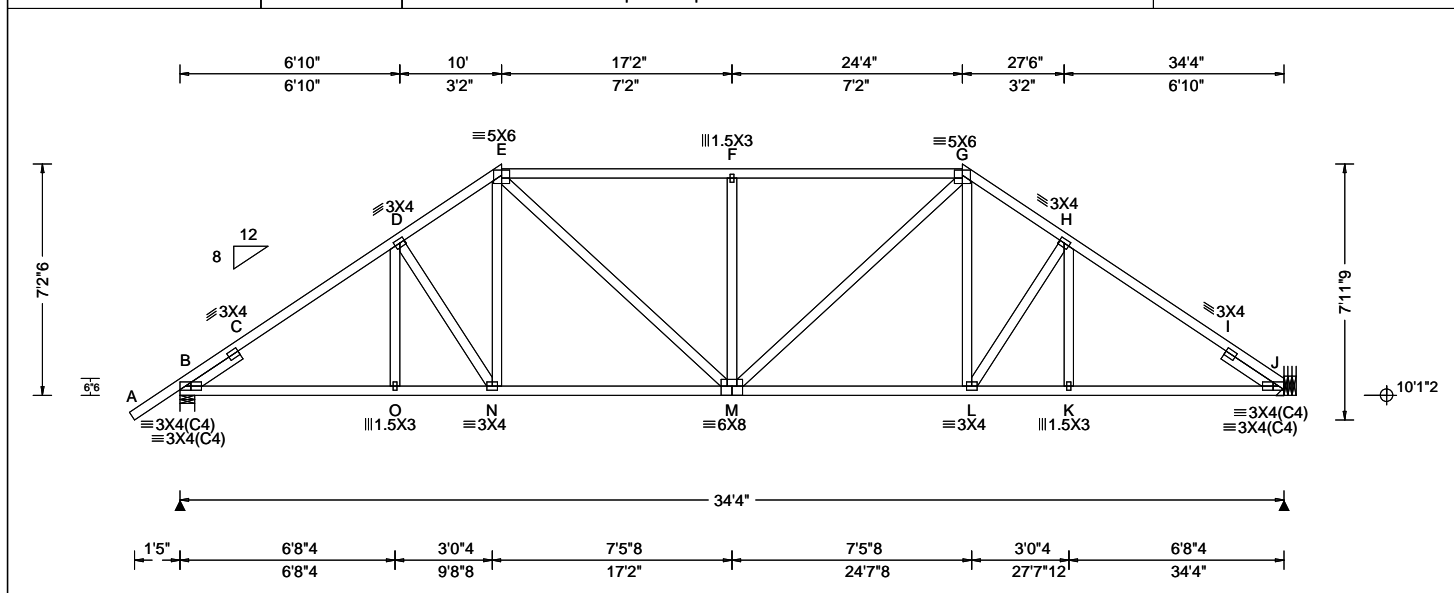
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125495 FROM: RJL	HIPS Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A3a 34'4" Stepped Hip	Cust: R 857 JRRef: 1XG68570009 T37 DrwNo: 157.22.1646.38380 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.097 F 999 360 VERT(CL): 0.172 F 999 240 HORZ(LL): 0.042 J - - HORZ(TL): 0.075 J - - Creep Factor: 2.0 Max TC CSI: 0.493 Max BC CSI: 0.509 Max Web CSI: 0.429 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1531 - / - / /735 - / /130 J 1432 - / - / /672 - / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 J Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 353 - 2231 F - G 341 - 1904 C - D 245 - 2050 G - H 295 - 1911 D - E 292 - 1884 H - I 249 - 2089 E - F 341 - 1904 I - J 350 - 2220

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

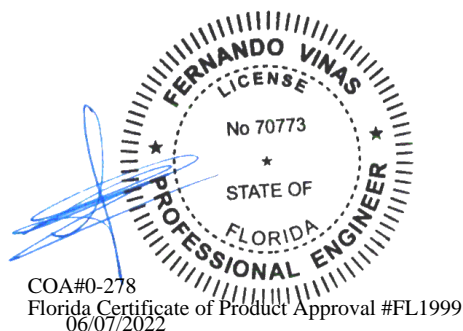
Hangers / Ties
(J) Hanger Support Required, by others

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	1635 - 150	M - L	1542 - 125
O - N	1634 - 150	L - K	1680 - 146
N - M	1529 - 134	K - J	1682 - 146

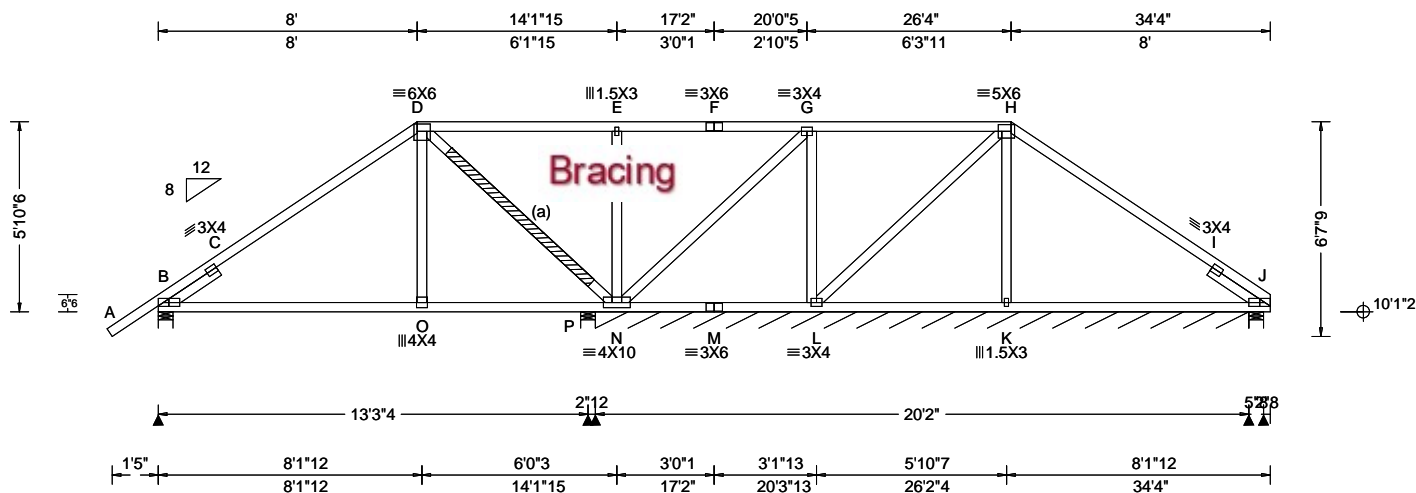
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
N - E	375 0	M - G	497 - 110
E - M	515 - 114	G - L	415 0
F - M	229 - 484		



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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125522 FROM: RJL	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A4 34'4" Stepped Hip Girder	Cust: R 857 JRef: 1XG68570009 T8 DrwNo: 157.22.1646.36850 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.096 C 999 360 VERT(CL): 0.176 C 888 240 HORZ(LL): 0.074 C - - HORZ(TL): 0.136 C - - Creep Factor: 2.0 Max TC CSI: 0.906 Max BC CSI: 0.741 Max Web CSI: 0.576 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1281 -/- /- /- /55 -/ P 354 -/- /- /71 -/- P* 109 -/- /- /5 -/- J 771 -/- /- /47 -/ K -105 Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 P Brg Wid = 5.5 Min Req = 1.5 P Brg Wid = 241 Min Req = - J Brg Wid = 5.5 Min Req = 1.5 Bearings B, P, P, & J are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" oc.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 57 plf at -1.58 to 57 plf at 8.00
TC: From 28 plf at 8.00 to 28 plf at 12.06
TC: From 57 plf at 12.06 to 57 plf at 34.33
BC: From 20 plf at 0.00 to 20 plf at 8.03
BC: From 10 plf at 8.03 to 10 plf at 12.06
BC: From 20 plf at 12.06 to 20 plf at 34.33
PLB: From 40 plf at 5.42 to 40 plf at 8.00
PLB: From 40 plf at 26.33 to 40 plf at 28.92
TC: 372 lb Conc. Load at 8.03
TC: 194 lb Conc. Load at 10.06, 12.06
BC: 722 lb Conc. Load at 8.03
BC: 144 lb Conc. Load at 10.06, 12.06

Plating Notes

All plates are 3X4(C4) except as noted.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads and reactions based on MWFRS.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Maximum Top Chord Forces Per Ply (lbs)

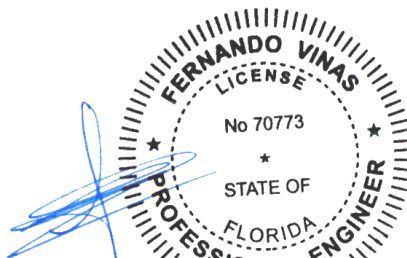
Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	306 -2093	H - I	90 -840
C - D	73 -1573	I - J	558 -1327

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	1221 -44	L - K	622 -57
O - N	2496 -79	K - J	614 -57

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - O	1074 0	N - G	25 -420
D - N	31 -1578	L - H	33 -445
E - N	150 -603	K - H	387 0



COA#0-278
Florida Certificate of Product Approval #FL1999
06/07/2022

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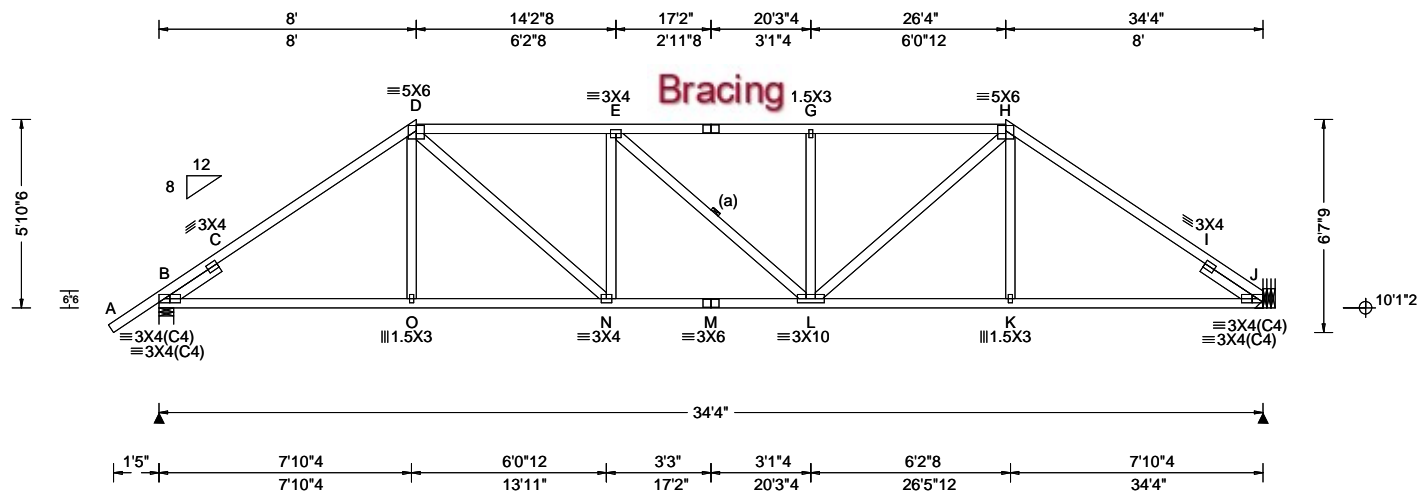
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125497 FROM: RJL	HIPS Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A4a 34'4" Stepped Hip	Cust: R 857 JRRef: 1XG68570009 T19 DrwNo: 157.22.1646.35253 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.108 I 999 360 VERT(CL): 0.193 I 999 240 HORZ(LL): 0.044 J - - HORZ(TL): 0.079 J - - Creep Factor: 2.0 Max TC CSI: 0.614 Max BC CSI: 0.571 Max Web CSI: 0.268 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1502 - / - / 725 / 24 / 108 J 1404 - / - / 662 / 12 / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.8 J Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 454 - 2288 F - G 419 - 2115 C - D 313 - 1991 G - H 420 - 2116 D - E 415 - 2098 H - I 314 - 2020 E - F 419 - 2115 I - J 471 - 2308

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

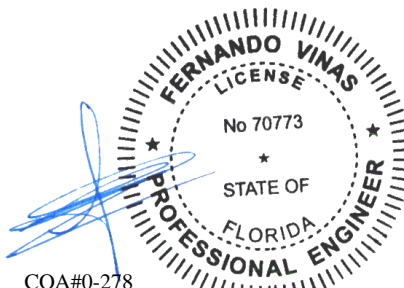
Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278

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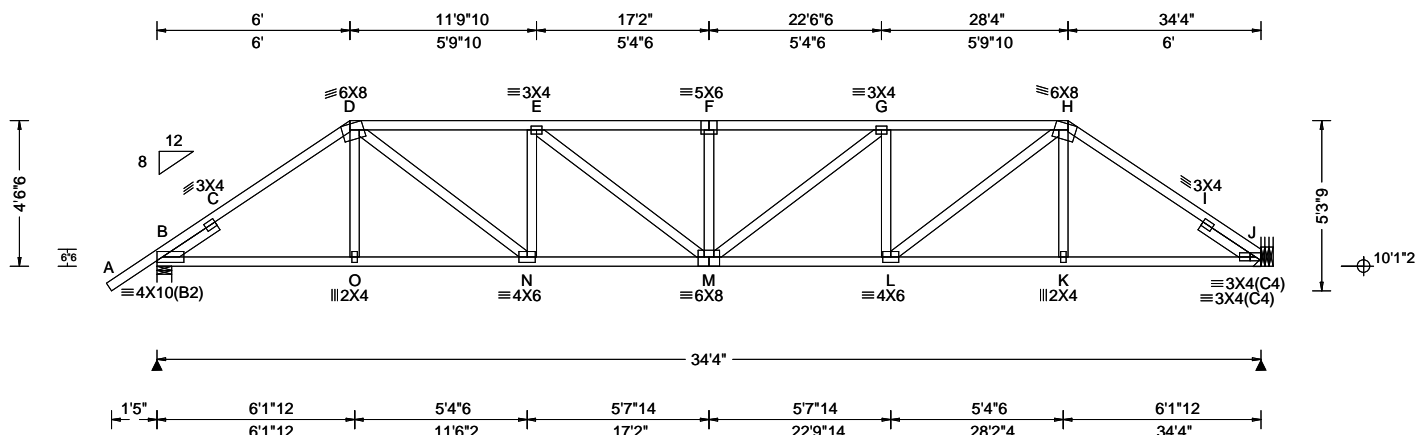
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125518 FROM: RJL	HIPS Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: A5 34'4" Stepped Hip Girder	Cust: R 857 JRRef: 1XG68570009 T20 DrwNo: 157.22.1646.33490 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.43 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.218 F 999 360 VERT(CL): 0.410 F 993 240 HORZ(LL): 0.066 J - - HORZ(TL): 0.124 J - - Creep Factor: 2.0 Max TC CSI: 0.833 Max BC CSI: 0.781 Max Web CSI: 0.556 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 2308 -/- /- /- /99 -/- J 1578 -/- /- /- /29 -/- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 2.7 J Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 187 -3433 F - G 112 -3702 C - D 144 -3368 G - H 76 -3071 D - E 152 -3770 H - I 58 -2359 E - F 112 -3702 I - J 155 -2417

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Special Loads

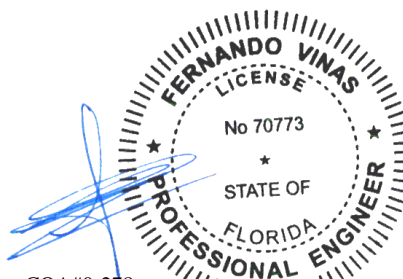
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 57 plf at -1.58 to 57 plf at 6.00
TC: From 28 plf at 6.00 to 28 plf at 12.06
TC: From 57 plf at 12.06 to 57 plf at 34.33
BC: From 20 plf at 0.00 to 20 plf at 6.03
BC: From 10 plf at 6.03 to 10 plf at 12.06
BC: From 20 plf at 12.06 to 20 plf at 34.33
TC: 317 lb Conc. Load at 6.03
TC: 141 lb Conc. Load at 8.06,10.06,12.06
BC: 338 lb Conc. Load at 6.03
BC: 106 lb Conc. Load at 8.06,10.06,12.06

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

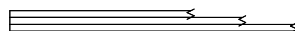


COA#0-278
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06/07/2022

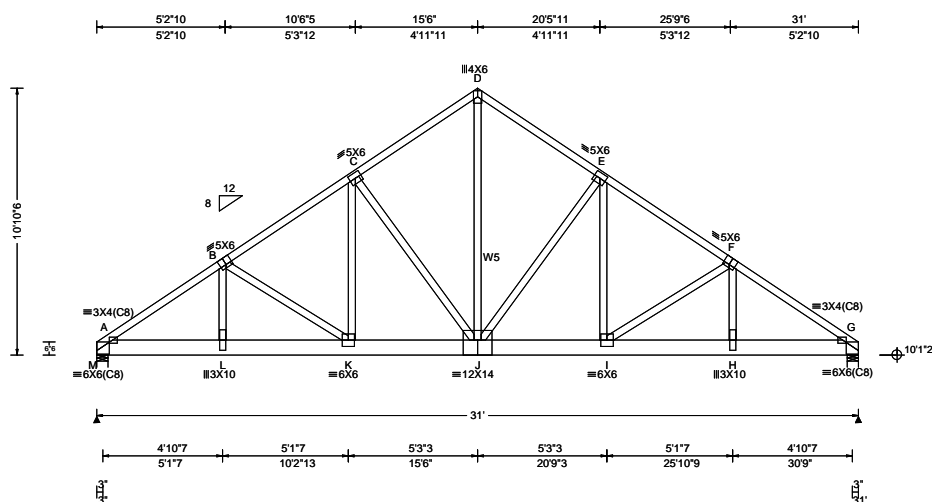
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125540 FROM: RJL	COMN Ply: 3 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: B1 31' Common Girder	Cust: R 857 JRRef: 1XG68570009 T12 DrwNo: 157.22.1646.31590 SSB / FV 06/06/2022
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3 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.79 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.10 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.157 I 999 360 VERT(CL): 0.302 I 999 240 HORZ(LL): 0.048 B - - HORZ(TL): 0.092 B - - Creep Factor: 2.0 Max TC CSI: 0.443 Max BC CSI: 0.352 Max Web CSI: 0.613 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 10730 -/- /- /- /42 -/- G 10730 -/- /- /- /42 -/- Wind reactions based on MWFRS M Brg Wid = 5.5 Min Req = 3.6 G Brg Wid = 5.5 Min Req = 3.6 Bearings M & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3; W5 2x4 SP #1;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 3.50" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Loading

Girder supports 34'-4" span to BC one face and 2'-0" span to TC/BC split opposite face.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

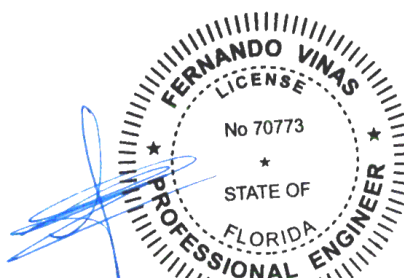
Chords	Tens.Comp.	Chords	Tens. Comp.
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A - L	4265 -17	J - I	3490 -14
L - K	4248 -17	I - H	4248 -17
K - J	3490 -14	H - G	4265 -17

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
------	------------	------	-------------

L - B	900 0	J - E	7 -1391
B - K	4 -865	E - I	1609 0
K - C	1609 0	I - F	4 -865
C - J	7 -1391	F - H	900 0
D - J	3459 0		



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06/07/2022

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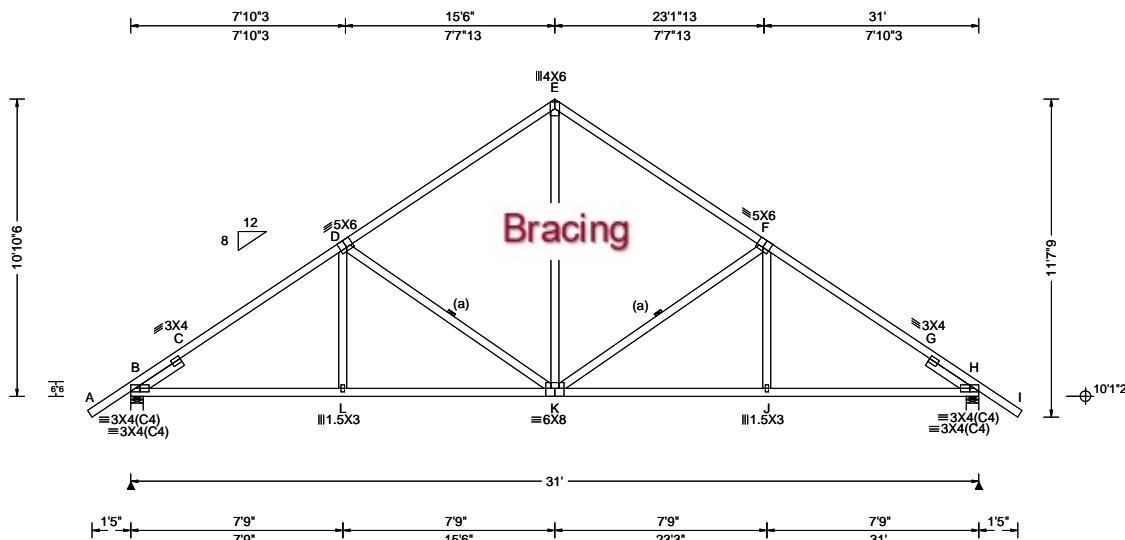
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125438 FROM: RJL	COMN Ply: 1 Qty: 5	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: B2 31' Common	Cust: R 857 JRRef: 1XG68570009 T9 DrwNo: 157.22.1646.29873 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.27 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.10 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.072 K 999 360 VERT(CL): 0.122 K 999 240 HORZ(LL): 0.037 H - - HORZ(TL): 0.063 H - - Creep Factor: 2.0 Max TC CSI: 0.610 Max BC CSI: 0.490 Max Web CSI: 0.671 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1449 - / - /678 - /199 H 1449 - / - /678 - /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.7 H Brg Wid = 5.5 Min Req = 1.7 Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 273 -2106 E - F 103 -1323 C - D 46 -1909 F - G 45 -1909 D - E 103 -1323 G - H 273 -2106

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

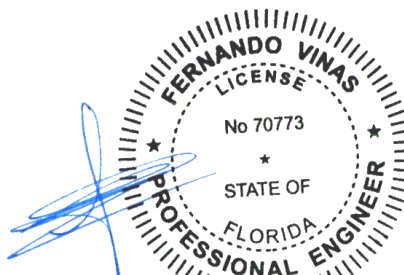
(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



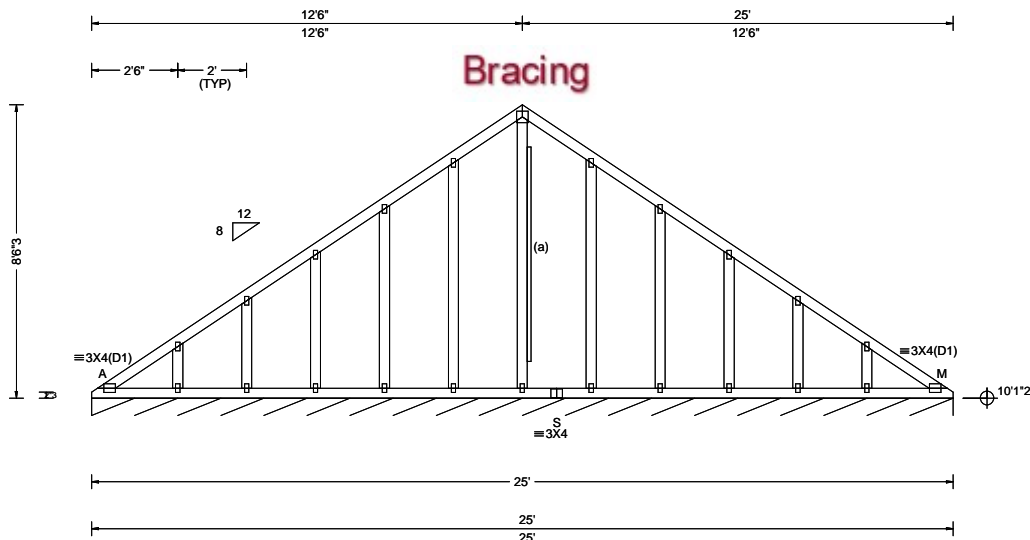
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06/07/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125462 FROM: RJL	GABL Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: C1-DG 25' Gable	Cust: R 857 JRef: 1XG68570009 T3 DrwNo: 157.22.1646.28030 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 H 999 360 VERT(CL): 0.003 H 999 240 HORZ(LL): -0.002 D - - HORZ(TL): 0.003 F - - Creep Factor: 2.0 Max TC CSI: 0.075 Max BC CSI: 0.044 Max Web CSI: 0.119 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A* 111 /- /- /44 /3 /7 Wind reactions based on MWFRS A Brg Wid = 300 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) 1x4 #3SRB SPF-S or better "L" reinforcement.
80% length of web member. Attach with 8d Box or
Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers
and cladding load not to exceed 5.00 PSF one face
and 24.0" span opposite face. Top chord must not be
cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C
member design.

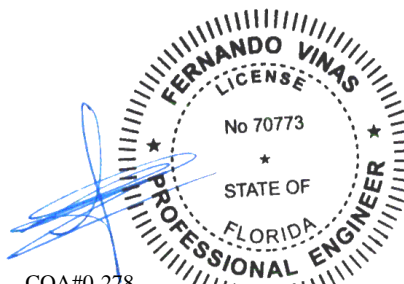
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, &
GABRST160118 for gable wind bracing and other
requirements.

Note: The Project Engineer shall provide for endwall
stability per the Florida Building Code. The top of the
wall below this truss shall be laterally braced as
specified by the Project Engineer. This truss will not
provide lateral support of the end wall.

Bottom chord to be laterally braced for out of
plane wind loads



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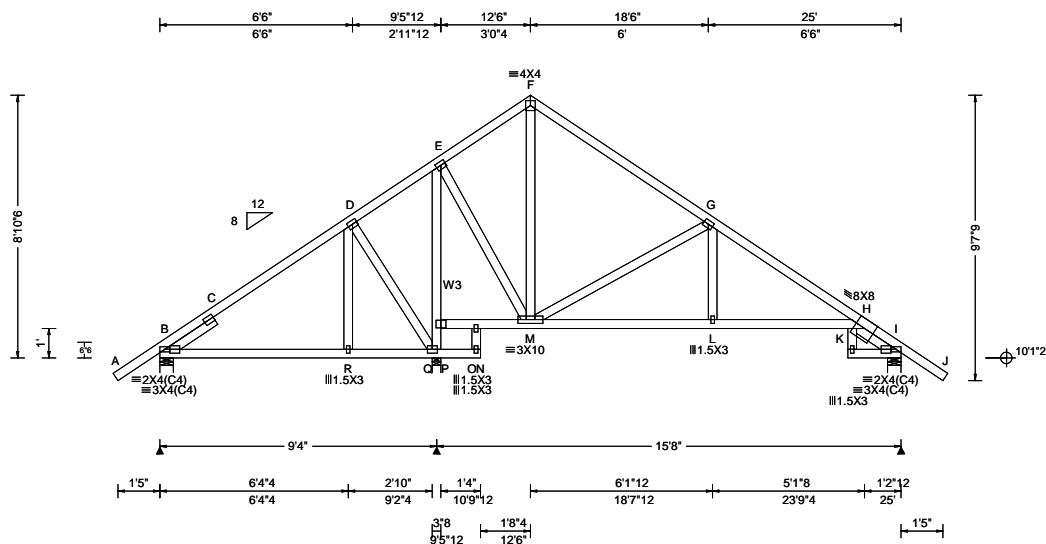
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125459 FROM: RJL	COMN Ply: 1 Qty: 8	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: C2 25' Common	Cust: R 857 JRRef: 1XG68570009 T2 DrwNo: 157.22.1646.22463 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.174 K 999 360 VERT(CL): 0.323 K 578 240 HORZ(LL): 0.083 K - - HORZ(TL): 0.155 K - - Creep Factor: 2.0 Max TC CSI: 0.930 Max BC CSI: 0.326 Max Web CSI: 0.600 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 392 - / - /140 /54 /165 Q 1189 - / - /669 - / - I 612 - / - /379 /34 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 Q Brg Wid = 3.5 Min Req = 1.5 I Brg Wid = 5.5 Min Req = 1.5 Bearings B, Q, & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W3 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.503'

Plating Notes

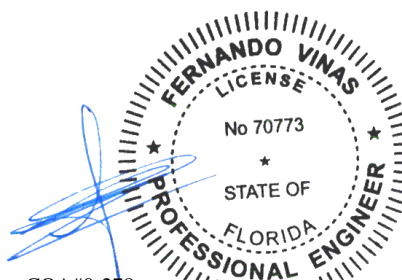
All plates are 3X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



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Glenview, IL 60025

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 7.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.171 K 999 360	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.275 K 999 240	B 1285 -/- /- /557 /140 /165
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.031 D - -	H 1285 -/- /- /557 /140 -/-
Des Ld: 37.00	EXP: B Kzt: NA		HORZ(TL): 0.050 D - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 5.5 Min Req = 1.5
Soffit: 0.00	TCDL: 4.2 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.331	H Brg Wid = 5.5 Min Req = 1.5
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.457	Bearings B & H are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.343	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: Any	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE	VIEW Ver: 21.01.03A.0805.15	B - C 281 - 1841 E - F 226 - 1755
	Wind Duration: 1.60			

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	57 plf at	-1.58 to	57 plf at	26.58
BC: From	20 plf at	0.00 to	20 plf at	6.50
BC: From	60 plf at	6.50 to	60 plf at	18.50
BC: From	20 plf at	18.50 to	20 plf at	25.00

Wind loading based on both gable and hip roof types.



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The drawing illustrates a roof truss system with the following components and dimensions:

- Roof Slope:** 12/8 (12 vertical, 8 horizontal).
- Top Chord Members:**
 - Left side: 3X4 (C) from A to B, 1.5X3 (D) from B to E.
 - Right side: 3X4 (G) from H to F, 1.5X3 (F) from F to E.
- Bottom Chord Members:**
 - Left side: 2X4 (C4) / 3X4 (C4) from A to B.
 - Right side: 2X4 (C4) / 3X4 (C4) from H to G.
 - Center: 1.5X3 (B2) from E to J, 2X4 (J) from J to B, 2X4 (I) from I to G, 3X4 from G to F, 3X4 from F to E, 3X4 from E to D, 8X8 from D to B, 8X8 from G to I.
- Dimensions:**
 - Horizontal (Top):** 6'4" (4 segments), 12'6" (1 segment), 18'7" (2 segments), 25' (1 segment).
 - Horizontal (Bottom):** 1'5" (1 segment), 6'6" (2 segments), 2'3" (2 segments), 11'7" (1 segment), 18'3" (1 segment), 1'6" (1 segment), 6'6" (1 segment), 25' (1 segment).
 - Vertical (Left):** 8'10" (1 segment), 1'5" (1 segment), 1'6" (1 segment).
 - Vertical (Right):** 8'7" (1 segment), 10'1" (1 segment).

Lumber				C - D	230 - 1770	F - G	232 - 1783
Top chord: 2x4 SP #1;				D - E	229 - 1762	G - H	276 - 1844
Bot chord: 2x4 SP #1; B2 2x6 SP #1;				Maximum Bot Chord Forces Per Ply (lbs)			
Webs: 2x4 SP #3;				Chords	Tens.Comp.	Chords	Tens. Comp.
Lt Slider: 2x4 SP #3; block length = 1.958'				B - J	1415 - 179	I - H	1431 - 183
Rt Slider: 2x4 SP #3; block length = 1.958'				Maximum Web Forces Per Ply (lbs)			
Special Loads				Webs	Tens.Comp.	Webs	Tens. Comp.
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)				J - E	899 - 96	E - I	924 - 101
TC: From	57 plf at	-1.58 to	57 plf at 25.00				
BC: From	20 plf at	0.00 to	20 plf at 6.50				
BC: From	60 plf at	6.50 to	60 plf at 18.50				
BC: From	20 plf at	18.50 to	20 plf at 25.00				



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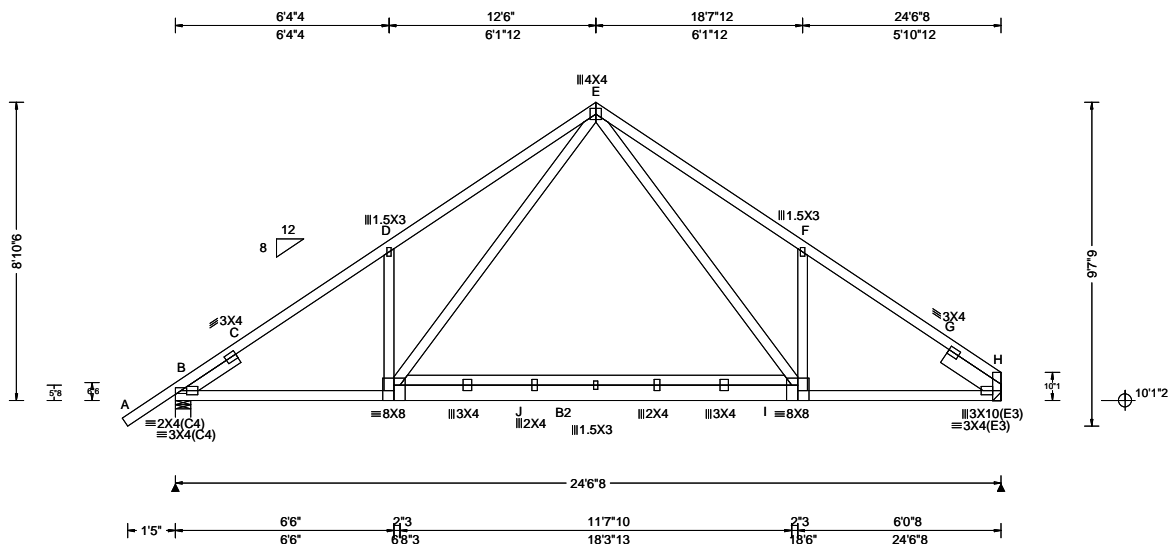
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SEQN: 125456 FROM: RJL	COMN Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: C3s 24'6"8 Common	Cust: R 857 JRRef: 1XG68570009 T6 DrwNo: 157.22.1646.04583 SSB / FV 06/06/2022
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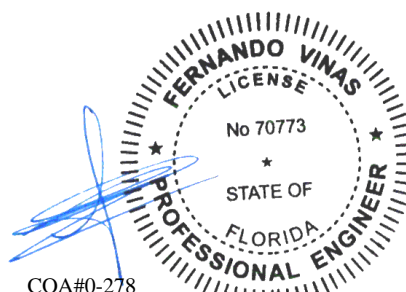
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.168 J 999 360 VERT(CL): 0.270 J 999 240 HORZ(LL): 0.031 D - - HORZ(TL): 0.049 D - - Creep Factor: 2.0 Max TC CSI: 0.332 Max BC CSI: 0.453 Max Web CSI: 0.342 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1278 - / - / /553 /139 /151 H 1173 - / - / /482 /124 - / Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 H Brg Wid = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 280 - 1829 E - F 225 - 1721 C - D 228 - 1751 F - G 227 - 1737 D - E 225 - 1742 G - H 257 - 1792

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x6 SP #1; block length = 1.958'

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 57 plf at -1.58 to 57 plf at 24.54
BC: From 20 plf at 0.00 to 20 plf at 6.50
BC: From 60 plf at 6.50 to 60 plf at 18.50
BC: From 20 plf at 18.50 to 20 plf at 24.54

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

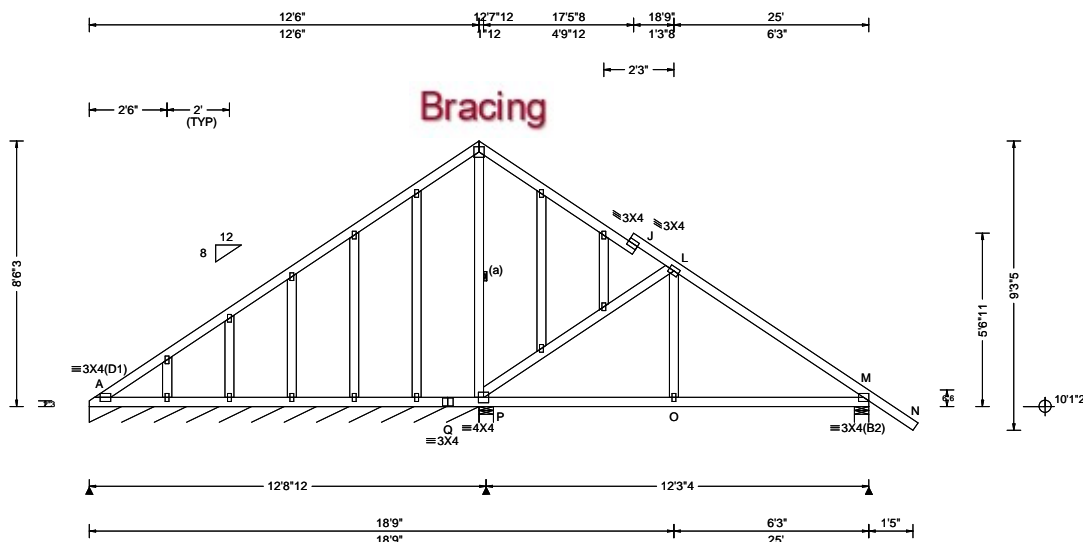


COA#0-278
Florida Certificate of Product Approval #FL1999
06/07/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125474 FROM: RJL	GABL Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: C4-SDG 25' Gable	Cust: R 857 JRef: 1XG68570009 T5 DrwNo: 157.22.1646.02840 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.048 H 999 360 VERT(CL): 0.095 H 999 240 HORZ(LL): 0.036 W - - HORZ(TL): 0.070 W - - Creep Factor: 2.0 Max TC CSI: 0.539 Max BC CSI: 0.273 Max Web CSI: 0.207 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL A* 90 - / - / 45 / 11 / 16 P 1132 - / - / 419 - / - M 639 - / - / 386 / 72 - Non-Gravity Wind reactions based on MWFRS A Brg Wid = 150 Min Req = - P Brg Wid = 5.5 Min Req = 1.5 M Brg Wid = 5.5 Min Req = 1.5 Bearings A, P, & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

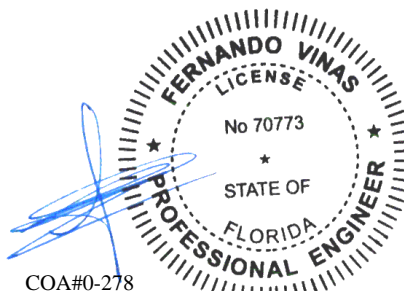
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



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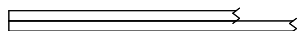
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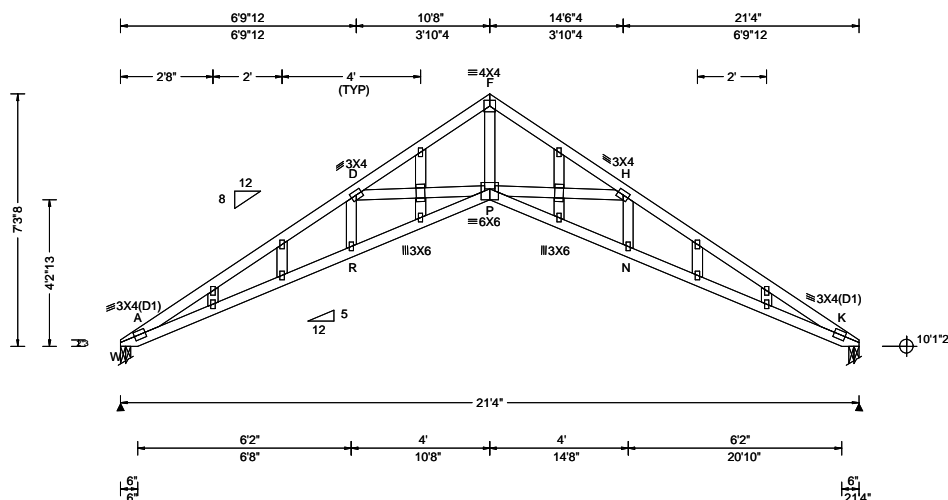


155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125485 FROM: RJL	GABL Qty: 1	Ply: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: D1-SDG 21'4" Gable	Cust: R 857 JRef: 1XG68570009 T7 DrwNo: 157.22.1645.54880 SSB / FV 06/06/2022
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.135 P 999 360 VERT(CL): 0.238 P 999 240 HORZ(LL): 0.141 K - - HORZ(TL): 0.250 K - - Creep Factor: 2.0 Max TC CSI: 0.239 Max BC CSI: 0.395 Max Web CSI: 0.470 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL W 1048 - / - / /515 /35 /157 K 1048 - / - / /515 /35 - Wind reactions based on MWFRS W Brg Wid = 3.5 Min Req = 1.5 K Brg Wid = 3.5 Min Req = 1.5 Bearings W & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - D 225 - 1751 F - H 127 - 1284 D - F 125 - 1284 H - K 223 - 1751

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

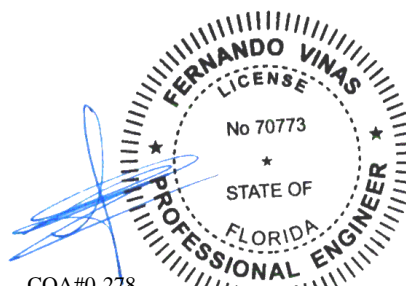
Truss designed to support 1-4-0 top chord outlookers
and cladding load not to exceed 5.00 PSF one face
and 24.0" span opposite face. Top chord must not be
cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C
member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, &
GABRST160118 for gable wind bracing and other
requirements.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - R	1555 -148	P - N	1534 -130
R - P	1534 -139	N - K	1555 -140

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - P	142 -380	P - H	143 -380
F - P	1234 -103		

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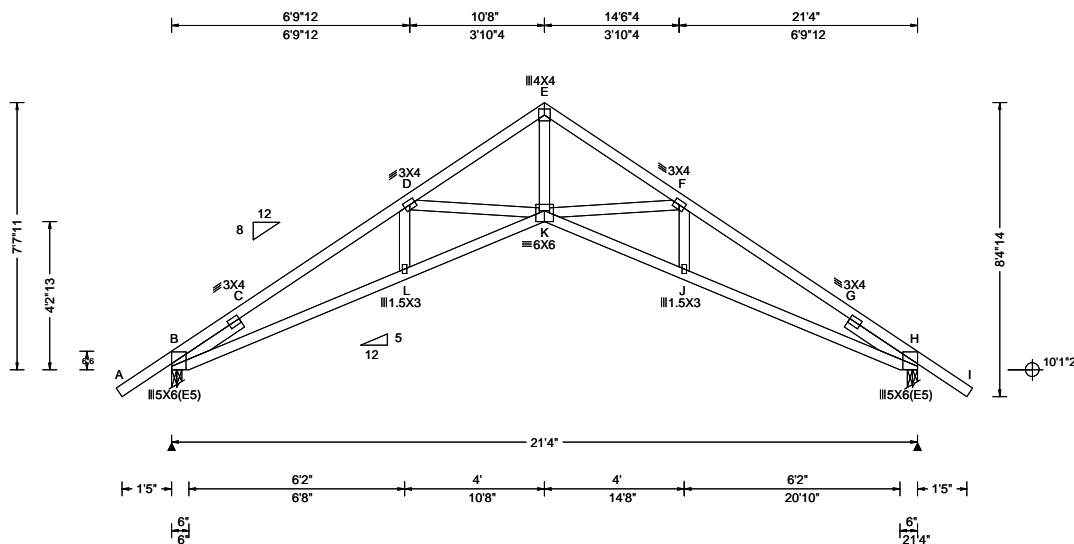
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125476 FROM: RJL	SCIS Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: D2 21'4" Scissor	Cust: R 857 JRef: 1XG68570009 T24 DrwNo: 157.22.1645.53050 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.145 K 999 360 VERT(CL): 0.274 K 935 240 HORZ(LL): 0.150 H - - HORZ(TL): 0.285 H - - Creep Factor: 2.0 Max TC CSI: 0.468 Max BC CSI: 0.518 Max Web CSI: 0.667 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 922 - / - / - /489 /5 /146 H 922 - / - / - /489 /5 /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 H Brg Wid = 3.5 Min Req = 1.5 Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 267 -2483 E - F 36 -1810 C - D 97 -2367 F - G 106 -2367 D - E 41 -1810 G - H 275 -2483

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 2.360'
Rt Slider: 2x4 SP #3; block length = 2.360'

Wind

Wind loads based on MWFRS with additional C&C member design.

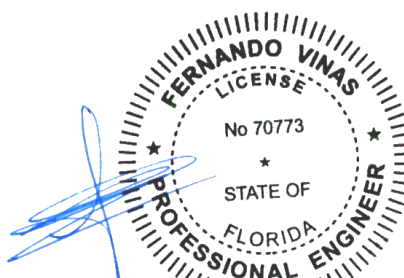
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
B - L	2073	0	K - J	2073	0
L - K	2073	0	J - H	2073	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - K	170 -464	K - F	168 -464
E - K	1752 0		



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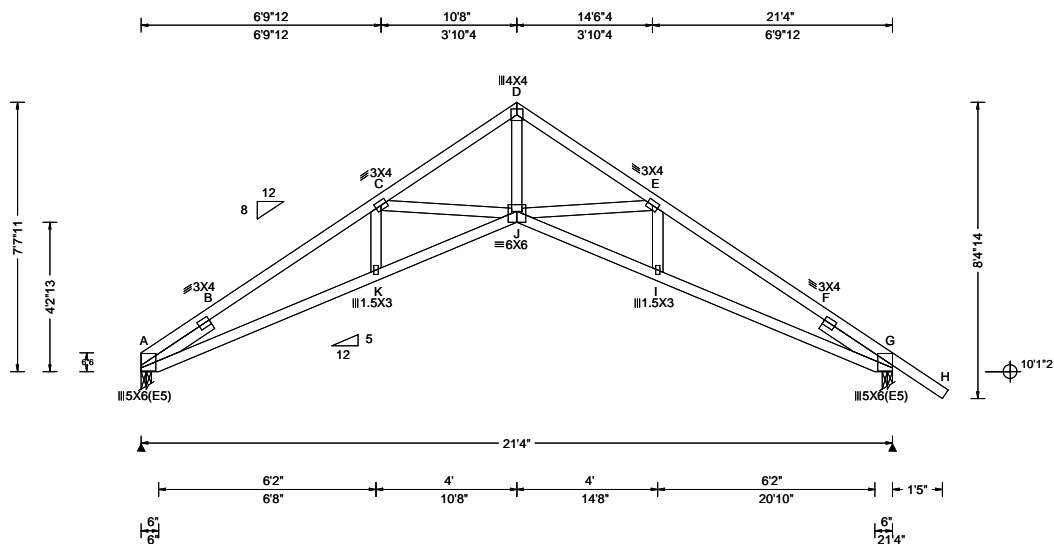
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125479 FROM: RJL	SCIS Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: D2a 21'4" Scissor	Cust: R 857 JRef: 1XG68570009 T25 DrwNo: 157.22.1645.51337 SSB / FV 06/06/2022
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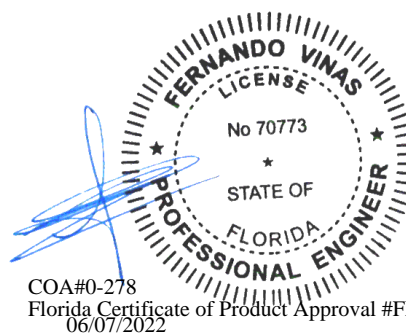
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.143 J 999 360 VERT(CL): 0.275 J 931 240 HORZ(LL): 0.149 G - - HORZ(TL): 0.286 G - - Creep Factor: 2.0 Max TC CSI: 0.512 Max BC CSI: 0.522 Max Web CSI: 0.674 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL A 834 -/- /- /431 -/- /134 G 925 -/- /- /489 /5 -/ Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 3.5 Min Req = 1.5 Bearings A & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 325 -2548 D - E 45 -1821 B - C 121 -2398 E - F 115 -2379 C - D 52 -1823 F - G 280 -2491

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 2.360'
Rt Slider: 2x4 SP #3; block length = 2.360'

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
A - K	2107 -5	J - I	2083 -4
K - J	2105 -2	I - G	2084 -6

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - J	179 -485	J - E	169 -464
D - J	1770 0		

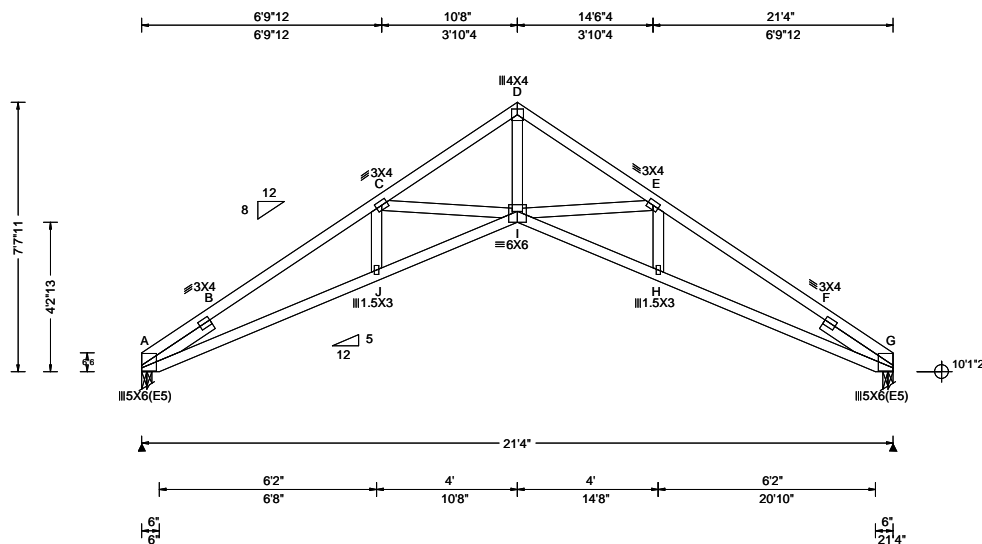


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North Building, 4th Floor
Glenview, IL 60025

SEQN: 125482 FROM: RJL	SCIS Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: D2b 21'4" Scissor	Cust: R 857 JRef: 1XG68570009 T27 DrwNo: 157.22.1645.49297 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.141 I 999 360 VERT(CL): 0.276 I 927 240 HORZ(LL): 0.147 G - - HORZ(TL): 0.288 G - - Creep Factor: 2.0 Max TC CSI: 0.512 Max BC CSI: 0.522 Max Web CSI: 0.681 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL A 837 -/- /- /431 -/- /113 G 837 -/- /- /431 -/- /- Non-Gravity A Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 3.5 Min Req = 1.5 Bearings A & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 355 -2556 D - E 119 -1835 B - C 184 -2410 E - F 182 -2410 C - D 113 -1835 F - G 358 -2556

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 2.360'
Rt Slider: 2x4 SP #3; block length = 2.360'

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

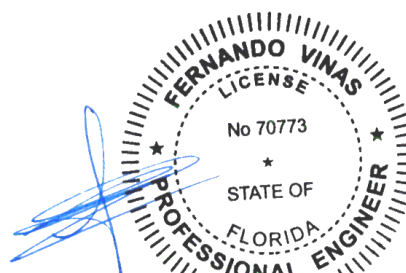
Chords	Tens.Comp.	Chords	Tens. Comp.
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A - J	2117 -109	I - H	2116 -97
J - I	2116 -108	H - G	2117 -98

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
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C - I	170 -485	I - E	172 -485
D - I	1788 -76		



COA#0-278
Florida Certificate of Product Approval #FL1999
06/07/2022

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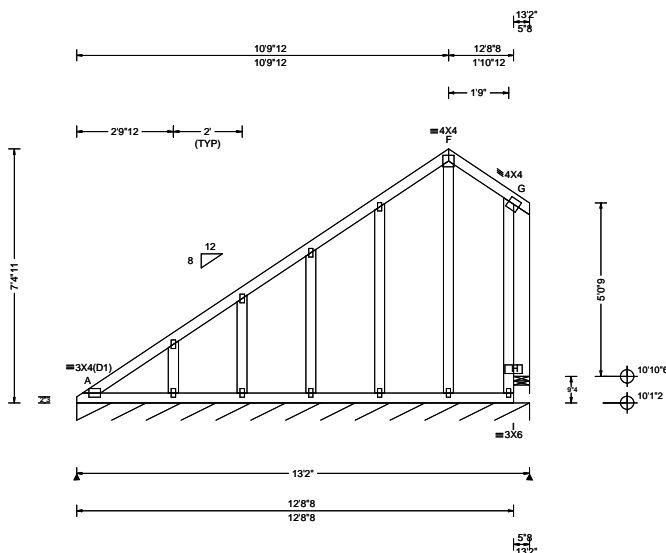
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125531 FROM: RJL	GABL Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: E1-DG 13'2" Gable	Cust: R 857 JRef: 1XG68570009 T13 DrwNo: 157.22.1645.47550 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 360 VERT(CL): 0.003 A 999 240 HORZ(LL): -0.003 E - - HORZ(TL): 0.004 E - - Creep Factor: 2.0 Max TC CSI: 0.078 Max BC CSI: 0.052 Max Web CSI: 0.088 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 102 - / - / 49 / 3 / 13 H 98 - / - / 36 / 31 / - Wind reactions based on MWFRS A Brg Wid = 158 Min Req = - H Brg Wid = 5.5 Min Req = 1.5 Bearings A & H are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x6 SP #1;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

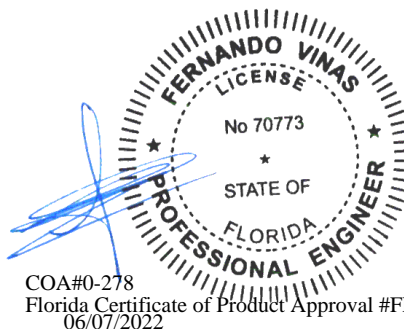
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



COA#0-278
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06/07/2022

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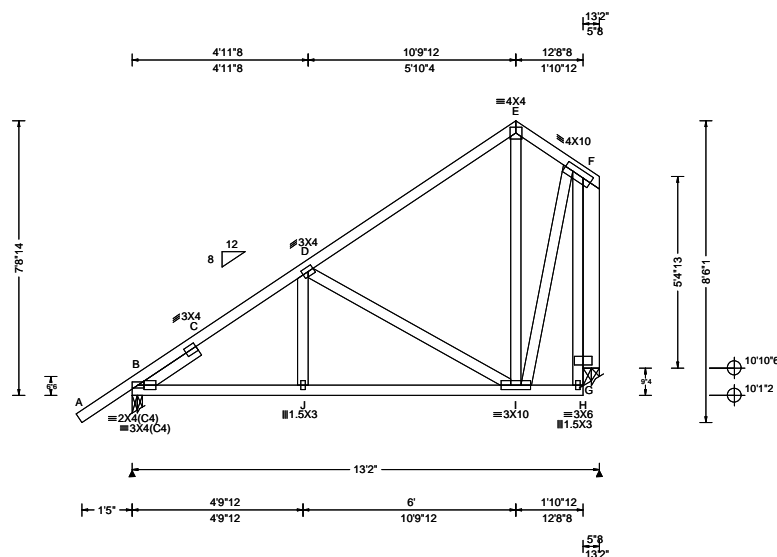
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ALPINE
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125526 FROM: RJL	SPEC Ply: 1 Qty: 3	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: E2 13'2" Common	Cust: R 857 JRRef: 1XG68570009 T16 DrwNo: 157.22.1645.45883 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.009 J 999 360 VERT(CL): 0.017 J 999 240 HORZ(LL): 0.005 C - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.268 Max BC CSI: 0.231 Max Web CSI: 0.344 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 589 -/- /- /329 -/- /133 G 499 -/- /- /282 /29 -/- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 G Brg Wid = 5.5 Min Req = 1.5 Bearings B & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 148 -704 C - D 42 -576

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Bearing Leg: 2x6 SP #1;

Wind

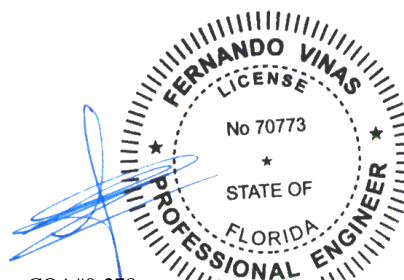
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	461 -167	J - I	458 -167

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
D - I	156 -380	F - G	125 -464
I - F	424 -109		



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06/07/2022

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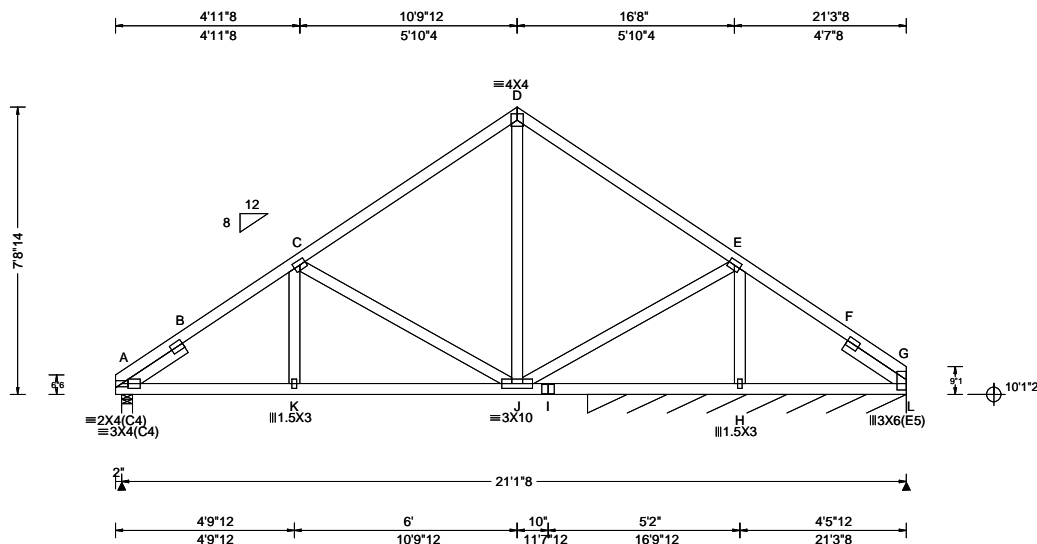
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125533 FROM: RJL	COMN Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: E3 21'3"8 Common	Cust: R 857 JRef: 1XG68570009 T21 DrwNo: 157.22.1645.44127 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.015 F 999 360 VERT(CL): 0.041 F 999 240 HORZ(LL): -0.006 F - - HORZ(TL): 0.026 F - - Creep Factor: 2.0 Max TC CSI: 0.339 Max BC CSI: 0.251 Max Web CSI: 0.319 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 640 -/- /- /332 /4 /115 L* 116 -/- /- /60 /- /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 L Brg Wid = 102 Min Req = - Bearings A & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 89 -879 C - D 86 -482 B - C 41 -792 D - E 86 -485

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

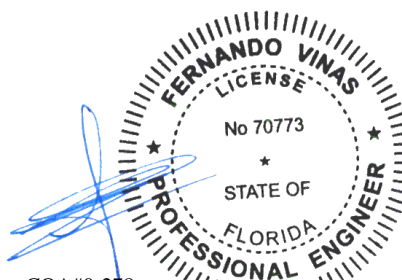
All plates are 3X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



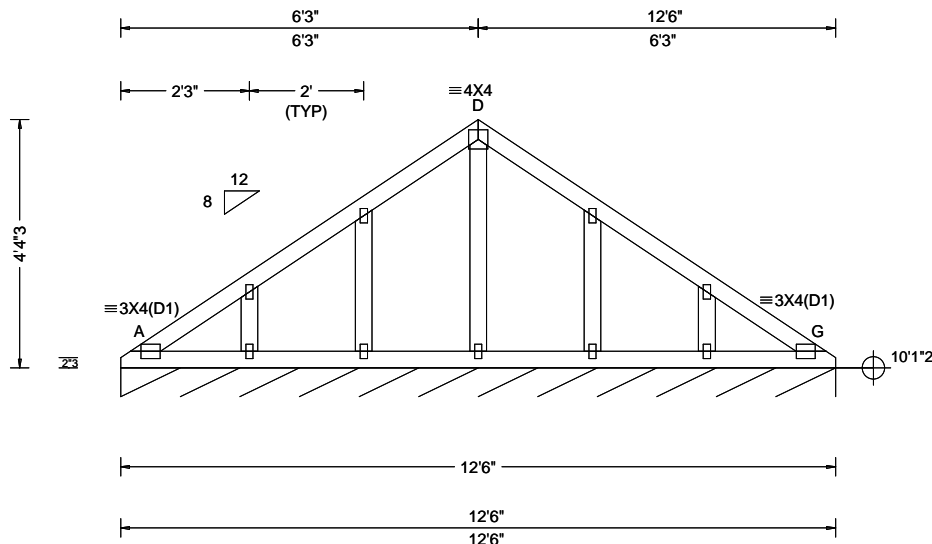
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Florida Certificate of Product Approval #FL1999
06/07/2022

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125431 FROM: RJL	GABL Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: G1-DG 12'6" Gable	Cust: R 857 JRef: 1XG68570009 T35 DrwNo: 157.22.1645.42200 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 360 VERT(CL): 0.001 A 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.063 Max BC CSI: 0.034 Max Web CSI: 0.035 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 100 /- /- /44 /7 /7 Wind reactions based on MWFRS A Brg Wid = 150 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

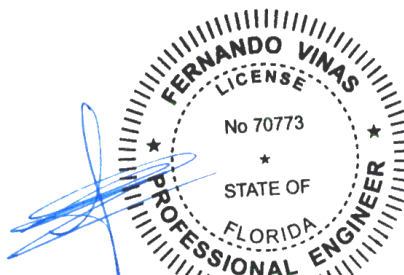
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



COA#0-278

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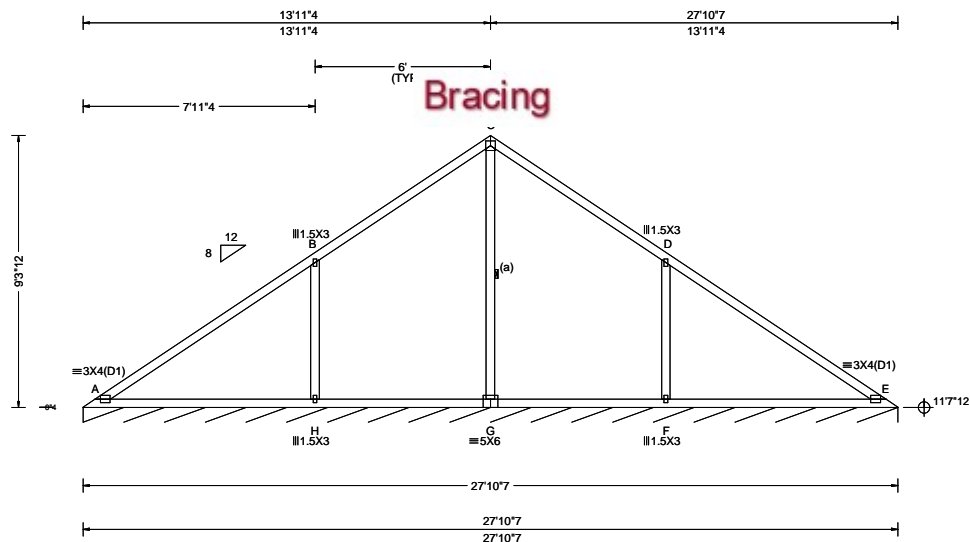
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125440 FROM: RJL	VAL	Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: V1 27'10"7 Valley	Cust: R 857 JRRef: 1XG68570009 T22 DrwNo: 157.22.1645.10797 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.45 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.036 A 999 360 VERT(CL): 0.069 A 999 240 HORZ(LL): 0.015 A - - HORZ(TL): 0.028 A - - Creep Factor: 2.0 Max TC CSI: 0.582 Max BC CSI: 0.444 Max Web CSI: 0.539 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 77 /- /- /37 /- /5 Wind reactions based on MWFRS E Brg Wid = 334 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 400 -72 D - E 400 -72 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - H 160 -458 F - D 160 -458 C - G 0 -550

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

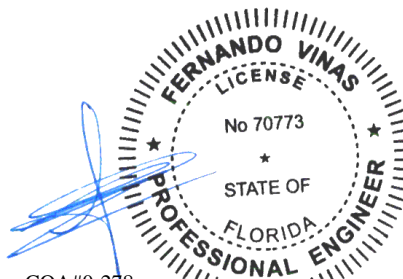
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA#0-278

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06/07/2022

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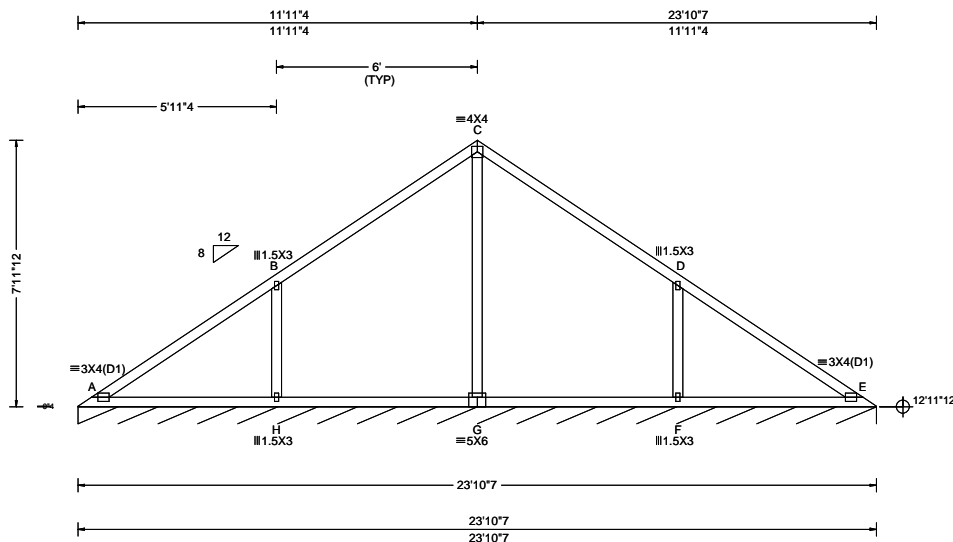
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125443 FROM: RJL	VAL	Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: V2 23'10"7 Valley	Cust: R 857 JRRef: 1XG68570009 T28 DrwNo: 157.22.1645.09300 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.12 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 A 999 360 VERT(CL): 0.025 A 999 240 HORZ(LL): 0.005 A - - HORZ(TL): 0.010 A - - Creep Factor: 2.0 Max TC CSI: 0.506 Max BC CSI: 0.270 Max Web CSI: 0.415 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 77 /- /- /37 /- /5 Wind reactions based on MWFRS E Brg Wid = 286 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - H 143 -397 F - D 143 -397

Lumber

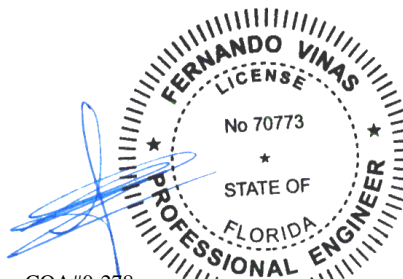
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA#0-278

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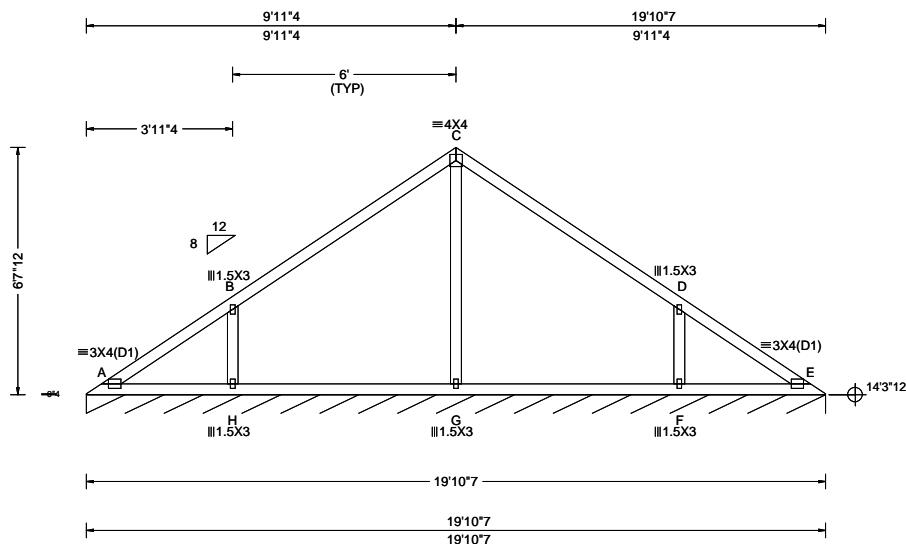
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125445 FROM: RJL	VAL Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: V3 19'10"7 Valley	Cust: R 857 JRef: 1XG68570009 T29 DrwNo: 157.22.1645.07930 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.78 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 C 999 360 VERT(CL): 0.003 A 999 240 HORZ(LL): -0.001 A - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.440 Max BC CSI: 0.214 Max Web CSI: 0.190 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 77 /- /- /37 /- /5 Wind reactions based on MWFRS E Brg Wid = 238 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

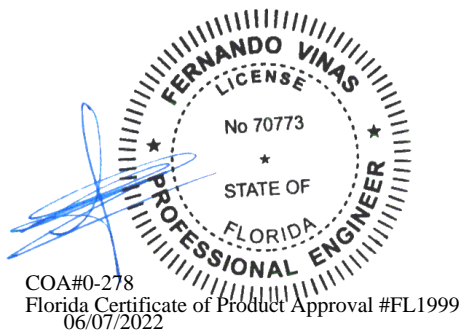
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

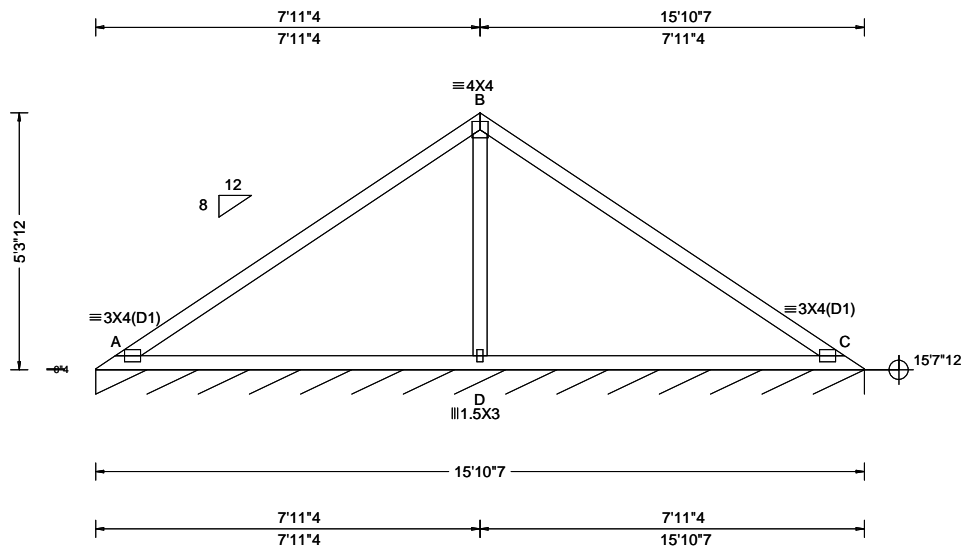
See DWG VALTN160118 for valley details.



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SEQN: 125447 FROM: RJL	VAL Qty: 2	Ply: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: V4 15'10"7 Valley	Cust: R 857 JRef: 1XG68570009 T30 DrwNo: 157.22.1645.06520 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 18.45 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.043 A 999 360 VERT(CL): 0.082 A 999 240 HORZ(LL): -0.022 C - - HORZ(TL): 0.041 C - - Creep Factor: 2.0 Max TC CSI: 0.703 Max BC CSI: 0.615 Max Web CSI: 0.513 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 77 /- /- /37 /- /5 Wind reactions based on MWFRS C Brg Wid = 190 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 713 -130 B - C 713 -130 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - D 160 -509 D - C 160 -509 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 205 -1045

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

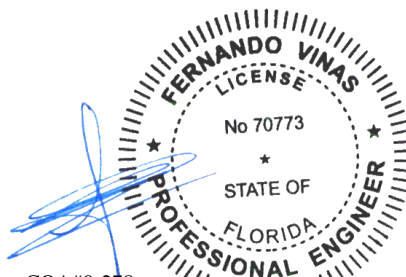
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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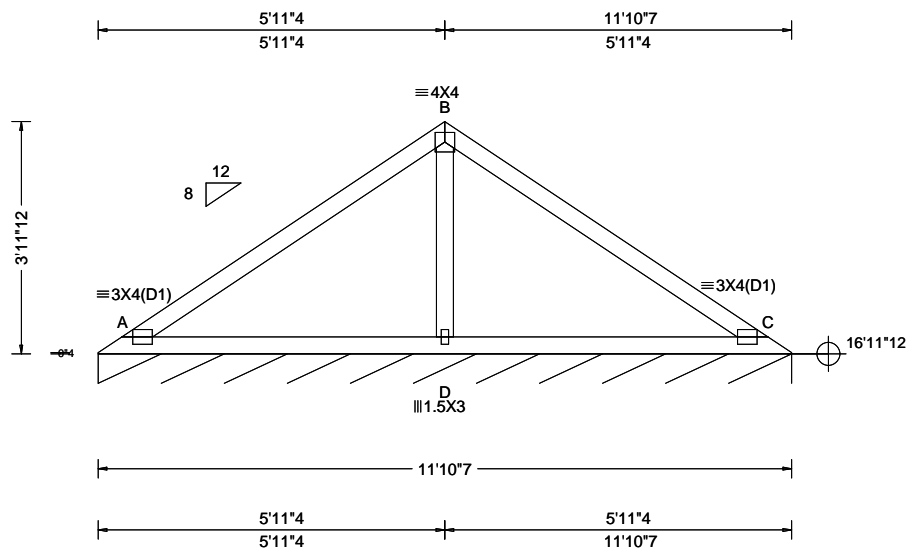
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125449 FROM: RJL	VAL Qty: 2	Ply: 1 Job Number: B55647a WENTWORTH RESIDENCE Truss Label: V5 11'10"7 Valley	Cust: R 857 JRef: 1XG68570009 T31 DrwNo: 157.22.1645.04970 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.12 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.018 A 999 360 VERT(CL): 0.034 A 999 240 HORZ(LL): -0.009 C - - HORZ(TL): 0.017 C - - Creep Factor: 2.0 Max TC CSI: 0.379 Max BC CSI: 0.324 Max Web CSI: 0.189 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 77 /- /- /36 /- /5 Wind reactions based on MWFRS C Brg Wid = 142 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 409 -99 B - C 409 -99 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 182 -640

Lumber

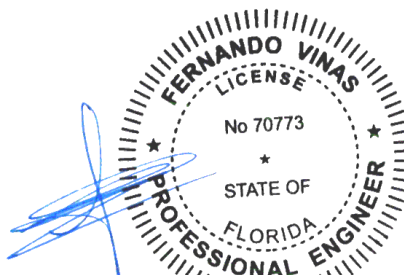
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.

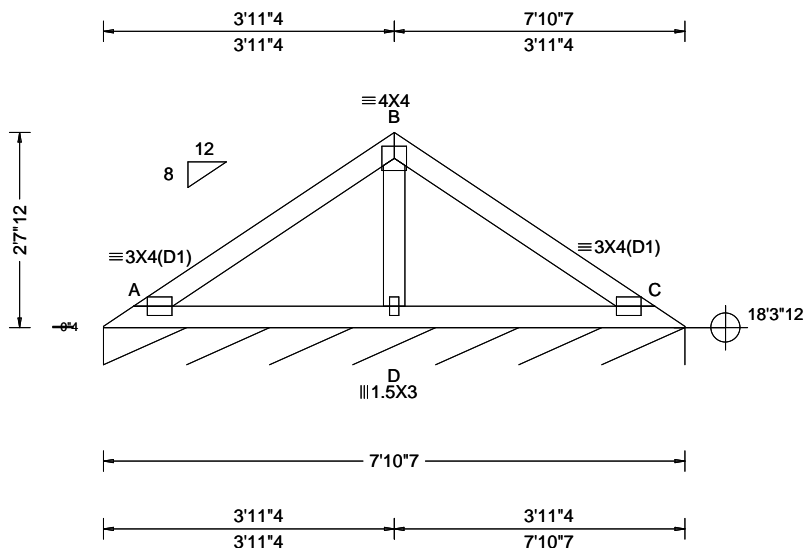


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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125451 FROM: RJL	VAL Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: V6 7'10"7 Valley	Cust: R 857 JRef: 1XG68570009 T33 DrwNo: 157.22.1645.03403 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.78 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 A 999 360 VERT(CL): 0.010 A 999 240 HORZ(LL): -0.003 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.146 Max BC CSI: 0.133 Max Web CSI: 0.067 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 77 /- /- /36 /- /5 Wind reactions based on MWFRS C Brg Wid = 94.4 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

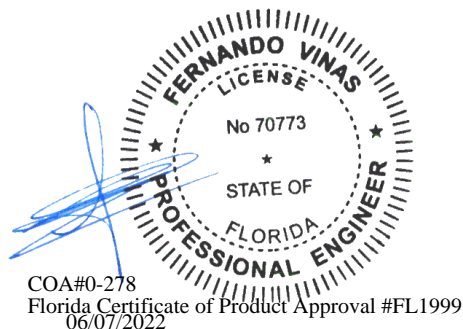
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

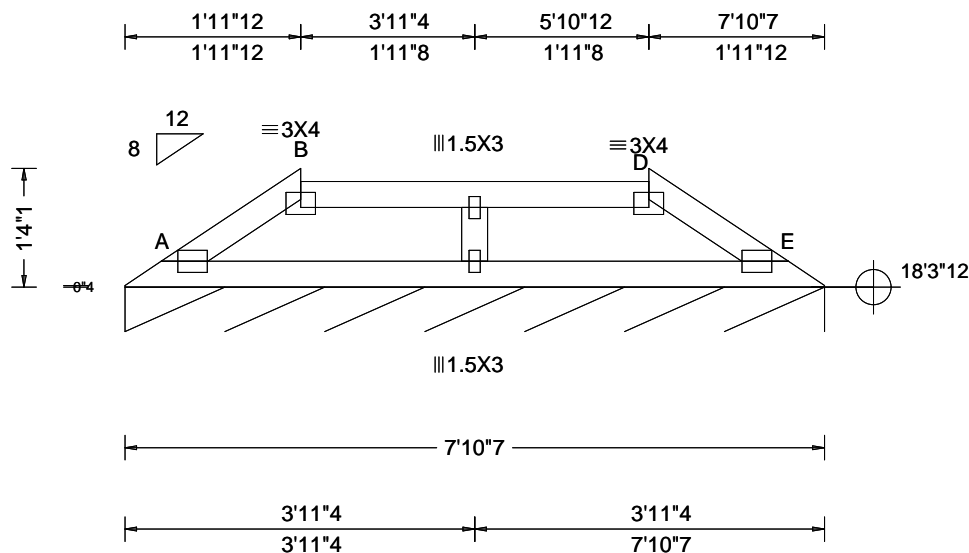
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125453 FROM: RJL	GABL Ply: 1 Qty: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: V7 7'10"7 Valley	Cust: R 857 JRef: 1XG68570009 T32 DrwNo: 157.22.1644.59550 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.13 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.003 A 999 360 VERT(CL): 0.005 A 999 240 HORZ(LL): -0.002 A - - HORZ(TL): 0.003 A - - Creep Factor: 2.0 Max TC CSI: 0.063 Max BC CSI: 0.117 Max Web CSI: 0.047 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 77 /- /- /34 /- /2 Wind reactions based on MWFRS E Brg Wid = 94.4 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 3X4(D1) except as noted.

Wind

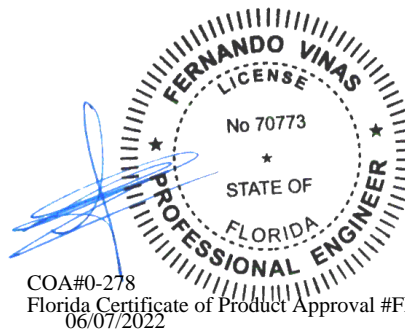
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

See DWGS VALTN160118 and VAL180160118 for valley details.



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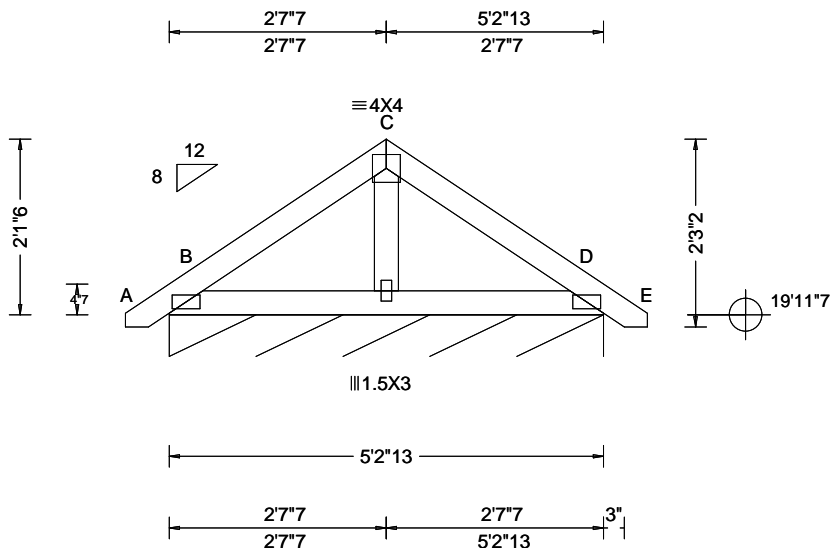
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SEQN: 125535 FROM: RJL	GABL Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: PB1 6'4" Common	Cust: R 857 JRef: 1XG68570009 T14 DrwNo: 157.22.1645.26103 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.02 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 B 999 360 VERT(CL): 0.001 D 999 240 HORZ(LL): -0.000 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.056 Max BC CSI: 0.051 Max Web CSI: 0.010 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 88 /- /- /42 /- /8 Wind reactions based on MWFRS B Brg Wid = 62.8 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

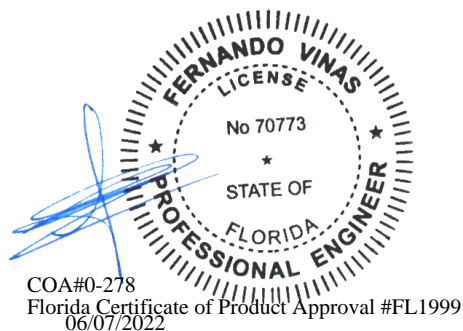
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to drawing PB160160118 or piggyback detail.



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

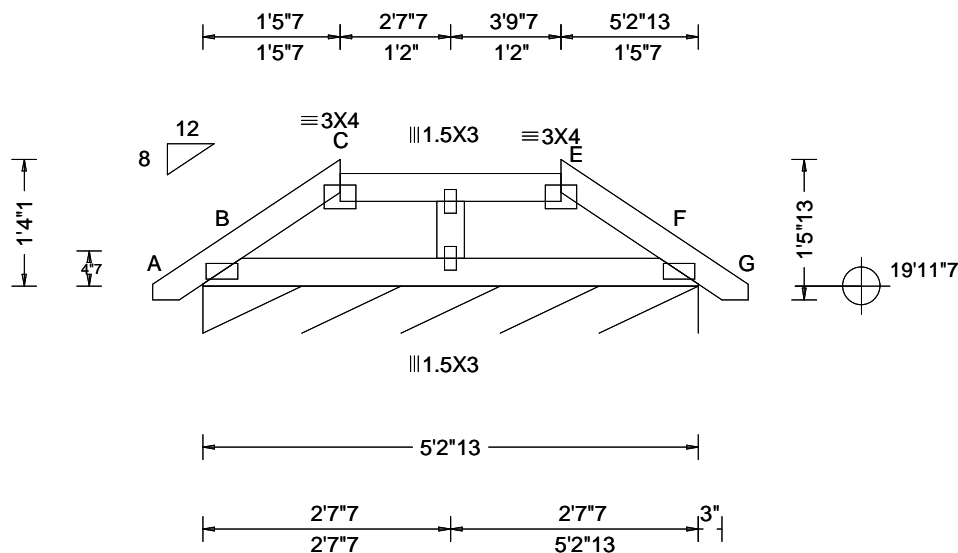
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SEQN: 125538 FROM: RJL	GABL Ply: 1 Qty: 2	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: PB2 6'4" Common	Cust: R 857 JRef: 1XG68570009 T34 DrwNo: 157.22.1645.22167 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.63 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 E 999 360 VERT(CL): 0.001 E 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.022 Max BC CSI: 0.043 Max Web CSI: 0.029 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 88 /- /- /40 /- /5 Wind reactions based on MWFRS B Brg Wid = 62.8 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

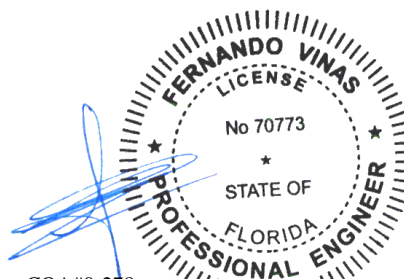
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to drawing PB160160118 or piggyback detail.



COA#0-278

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06/07/2022

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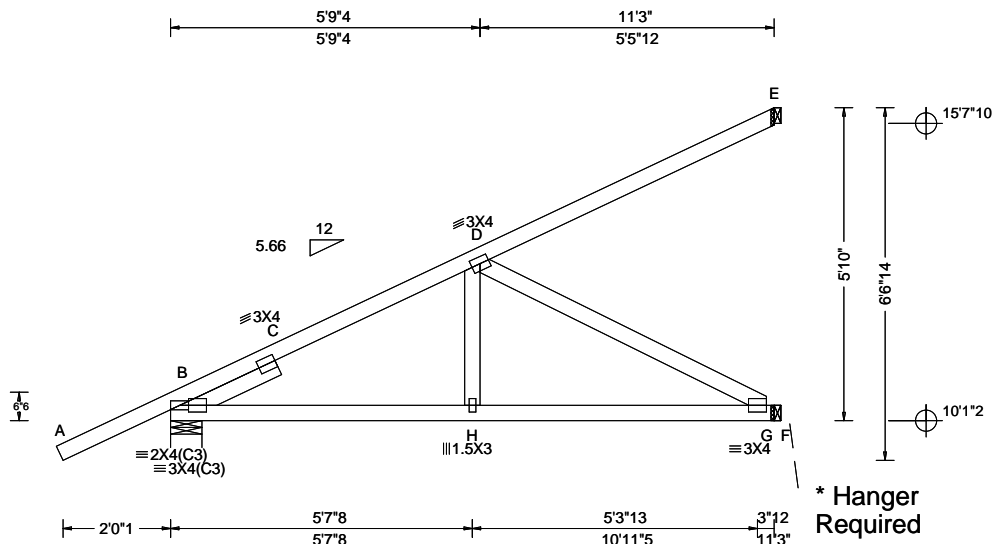
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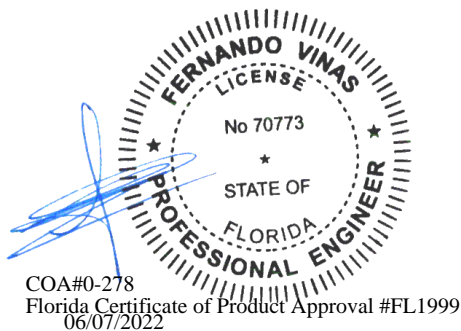
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125503 FROM: RJL	HIP_ Qty: 1	Ply: 1 Job Number: B55647a WENTWORTH RESIDENCE Truss Label: JA 11'3" Hip Jack Girder	Cust: R 857 JRRef: 1XG68570009 T15 DrwNo: 157.22.1645.40640 SSB / FV 06/06/2022
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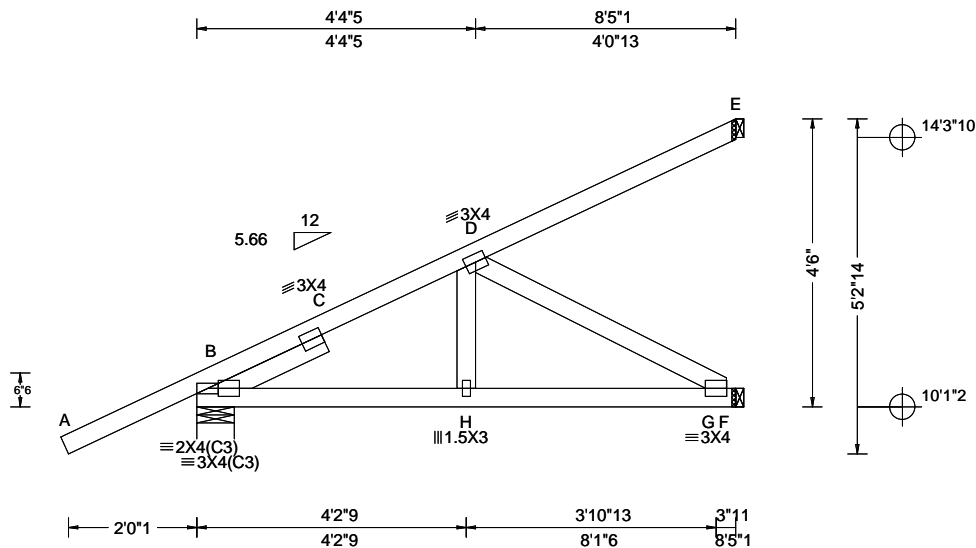
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.020 H 999 360 VERT(CL): 0.038 H 999 240 HORZ(LL): -0.012 C - - HORZ(TL): 0.023 C - - Creep Factor: 2.0 Max TC CSI: 0.604 Max BC CSI: 0.546 Max Web CSI: 0.543 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL B 495 -/- /- /8 -/ F 578 -/- /- /33 -/- E 178 -/- /- /31 -/ Wind reactions based on MWFRS B Brg Wid = 7.0 Min Req = 1.5 F Brg Wid = 1.5 E Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber Top chord: 2x4 SP #1; Bot chord: 2x4 SP #1; Webs: 2x4 SP #3; Lt Slider: 2x4 SP #3; block length = 1.958'	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - H 635 0 H - G 629 0
Loading Hipjack supports 7-11-8 setback jacks. Jacks up to 7' have no webs. Longer jacks supported to BC.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. D - G 0 -715
Wind Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.	



<p>**WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!</p> <p>**IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS</p> <p>Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.</p> <p>Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.</p> <p>For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org</p>	<p>ALPINE AN ITW COMPANY</p> <p>155 Harlem Ave North Building, 4th Floor Glenview, IL 60025</p>
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SEQN: 125505 FROM: RJL	HIP_ Qty: 1	Ply: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: JB 8'5"1 Hip Jack Girder	Cust: R 857 JRRef: 1XG68570009 T11 DrwNo: 157.22.1645.39003 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): -0.009 C 999 360 VERT(CL): -0.016 C 999 240 HORZ(LL): -0.006 C - - HORZ(TL): 0.011 C - - Creep Factor: 2.0 Max TC CSI: 0.291 Max BC CSI: 0.193 Max Web CSI: 0.156 VIEW Ver: 21.01.03A.0805.15	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 324 -/- /- /9 -/ F 232 -/- /- /33 -/- E 177 -/- /- /31 -/ Wind reactions based on MWFRS B Brg Wid = 7.0 Min Req = 1.5 F Brg Wid = 1.5 E Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

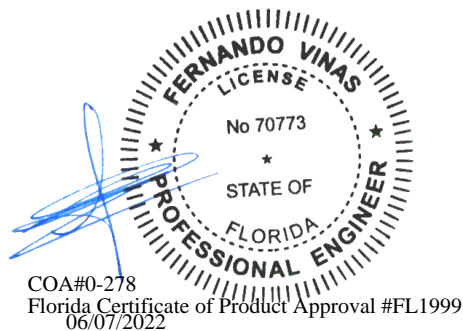
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'

Loading

Hipjack supports 5-11-8 setback jacks with no webs.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.



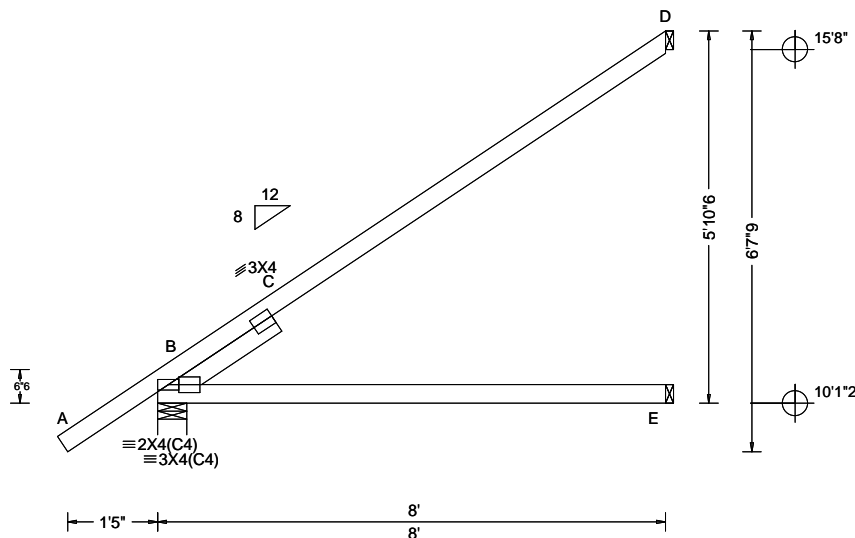
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North Building, 4th Floor
Glenview, IL 60025

SEQN: 125507 FROM: RJL	EJAC Ply: 1 Qty: 3	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: JC 8' End Jack	Cust: R 857 JRef: 1XG68570009 T23 DrwNo: 157.22.1645.36633 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.115 C - - HORZ(TL): 0.211 C - - Creep Factor: 2.0 Max TC CSI: 0.763 Max BC CSI: 0.544 Max Web CSI: 0.293 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 412 - / - /234 - /125 E 144 - / - /81 - /- D 194 - / - /106 /62 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.

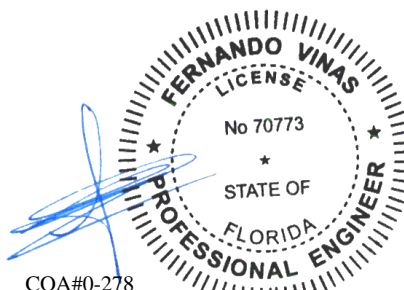
Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278

Florida Certificate of Product Approval #FL1999
06/07/2022

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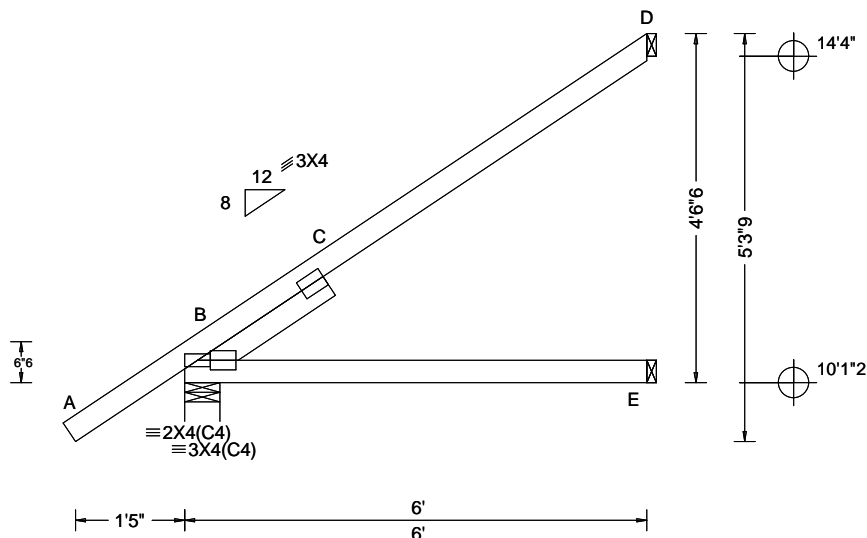
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125509 FROM: RJL	EJAC Ply: 1 Qty: 4	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: JD 6' End Jack	Cust: R 857 JRef: 1XG68570009 T51 DrwNo: 157.22.1645.34277 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.044 C - - HORZ(TL): 0.080 C - - Creep Factor: 2.0 Max TC CSI: 0.386 Max BC CSI: 0.288 Max Web CSI: 0.181 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 339 - / - /195 - /98 E 106 - / - /58 - /- D 141 - / - /76 /46 - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.

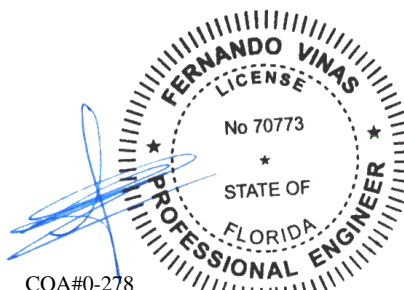
Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
06/07/2022

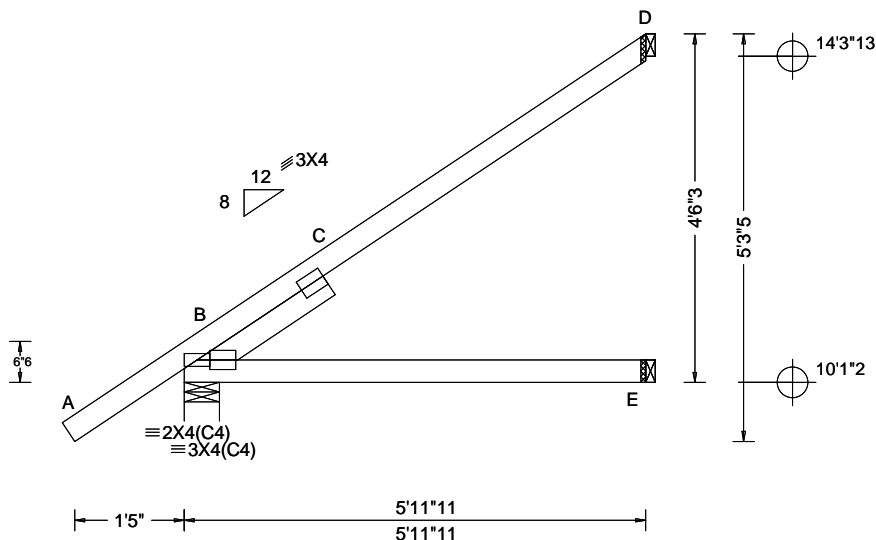
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125511 FROM: RJL	JACK Qty: 2	Ply: 1	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: JE 5'11"11 Jack	Cust: R 857 JRef: 1XG68570009 T18 DrwNo: 157.22.1645.32680 SSB / FV 06/06/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
				Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	B	338	/-	/-	/195	/-	/98
TCDL: 7.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): NA	E	106	/-	/-	/58	/-	/-
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	D	140	/-	/-	/76	/46	/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.043 C - -	Wind reactions based on MWFRS						
	EXP: B Kzt: NA		HORZ(TL): 0.079 C - -	B Brg Wid = 5.5 Min Req = 1.5						
Des Ld: 37.00	Mean Height: 15.00 ft		Creep Factor: 2.0	E Brg Wid = 1.5						
NCBCLL: 10.00	TCDL: 4.2 psf	Building Code:	Max TC CSI: 0.382	D Brg Wid = 1.5						
Soffit: 0.00	BCDL: 5.2 psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.285	Bearing B is a rigid surface.						
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max Web CSI: 0.179	Members not listed have forces less than 375#						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: No		Maximum Top Chord Forces Per Ply (lbs)						
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		Chords Tens.Comp.						
	GCpi: 0.18	Plate Type(s):	VIEW Ver: 21.01.03A.0805.15							
	Wind Duration: 1.60	WAVE								

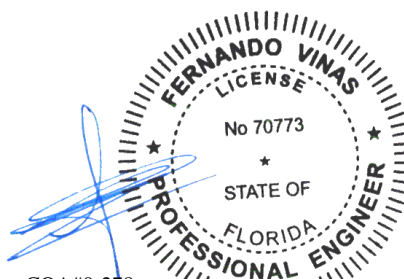
Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA#0-278
Florida Certificate of Product Approval #FL1999
06/07/2022

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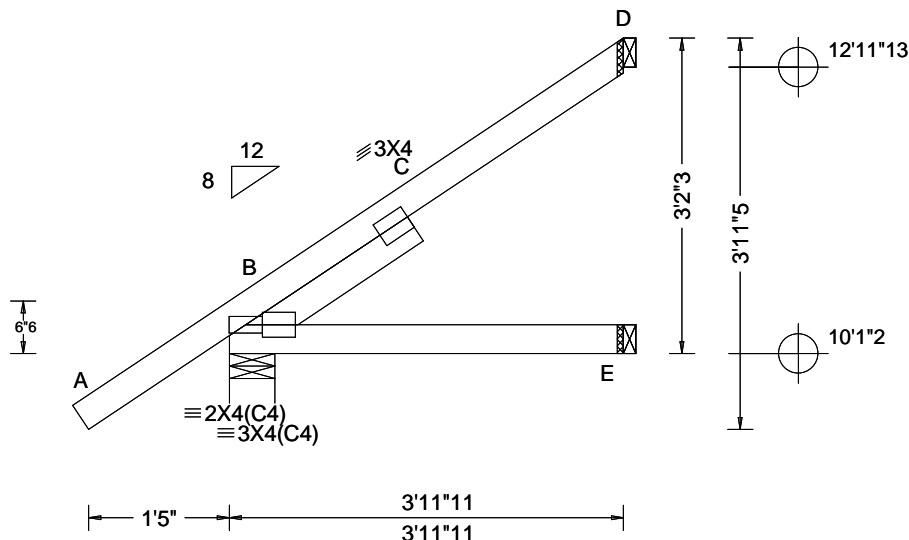
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125513 FROM: RJL	JACK Ply: 1 Qty: 4	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: JF 3'11"11 Jack	Cust: R 857 JRef: 1XG68570009 T44 DrwNo: 157.22.1645.30523 SSB / FV 06/06/2022
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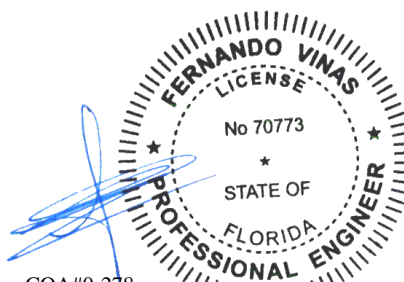
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.009 C - - HORZ(TL): 0.016 C - - Creep Factor: 2.0 Max TC CSI: 0.145 Max BC CSI: 0.107 Max Web CSI: 0.074 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 269 -/- /159 -/- /70 E 67 -/- /37 -/- /- D 84 -/- /44 /30 -/- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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06/07/2022

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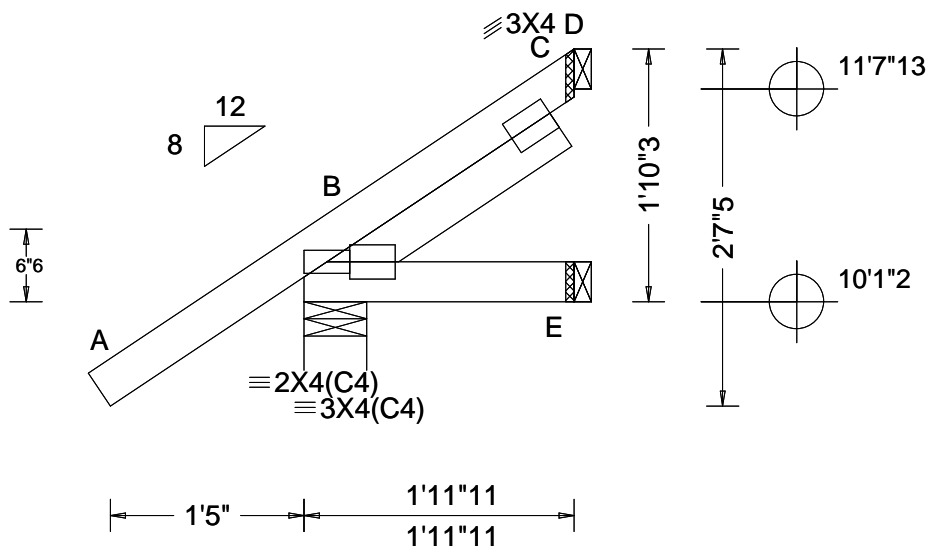
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 125515 FROM: RJL	JACK Ply: 1 Qty: 4	Job Number: B55647a WENTWORTH RESIDENCE Truss Label: JG 1'11"11 Jack	Cust: R 857 JRef: 1XG68570009 T45 DrwNo: 157.22.1645.28207 SSB / FV 06/06/2022
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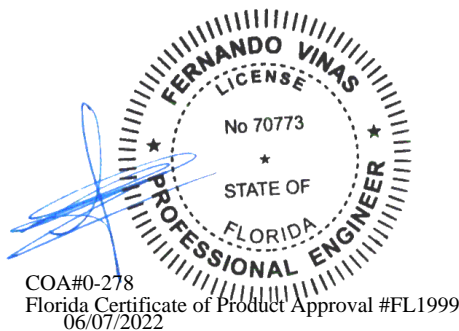
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 D - - HORZ(TL): 0.001 D - - Creep Factor: 2.0 Max TC CSI: 0.130 Max BC CSI: 0.039 Max Web CSI: 0.037 VIEW Ver: 21.01.03A.0805.15	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 219 /- /- /136 /5 /43 E 28 /- /- /16 /- /- D 11 /- /- /20 /12 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

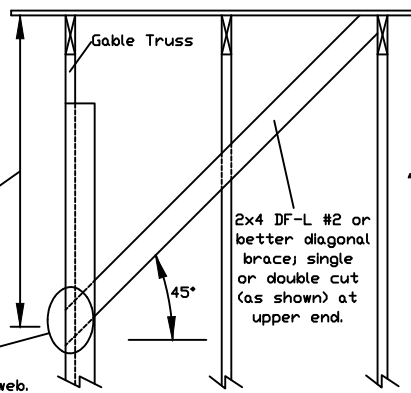
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 4"	7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
	SP	DFL	Stud	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
			Standard	4' 4"	5' 9"	6' 1"	7' 7"	8' 2"	10' 4"	11' 1"	11' 11"	12' 10"	14' 0"	14' 0"
		Standard	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	8' 9"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 6"	9' 1"	9' 5"	10' 8"	11' 1"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#2	#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 6"	8' 8"	9' 3"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	6' 0"	10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	Stud	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

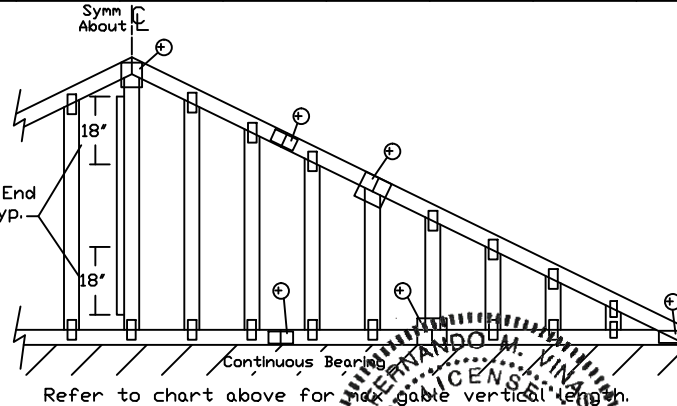
Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 385# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



'L' Brace End Zones, typ.



Refer to chart above for max gable vertical length.

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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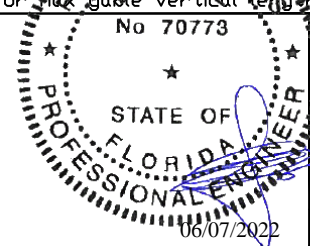
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



COA#0-278
 Florida Certificate of Product Approval #FL1999

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-16-GAB12030

DATE 01/26/2018

DRWG A12030ENC160118

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail
for lumber, plates, and other information not shown
on this detail.

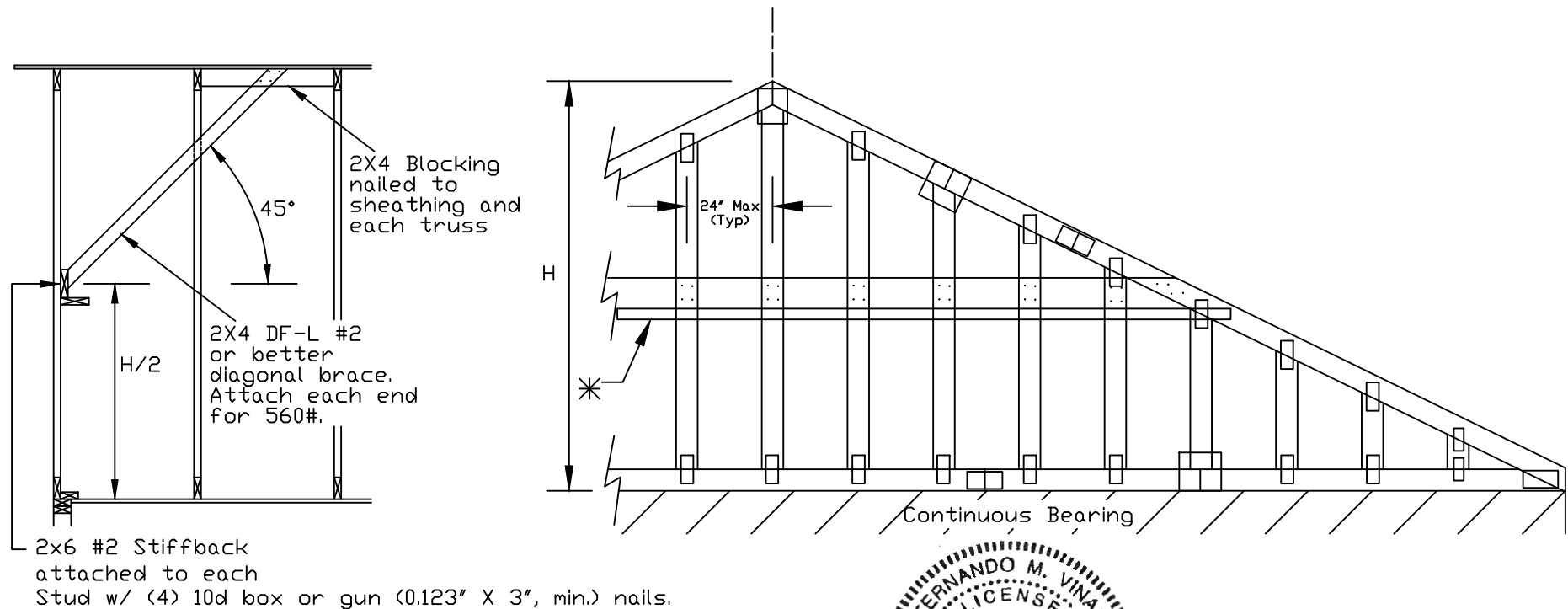
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC160118).

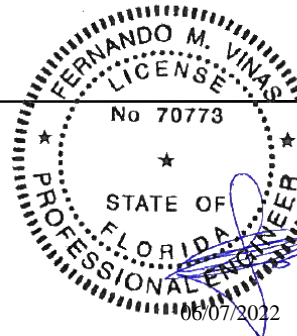
H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 4'0" (see detail below or
refer to DRWG A12030ENC160118).

* Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128" x 3", min.) nails @ 6" o.c.



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ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



COA#0-278

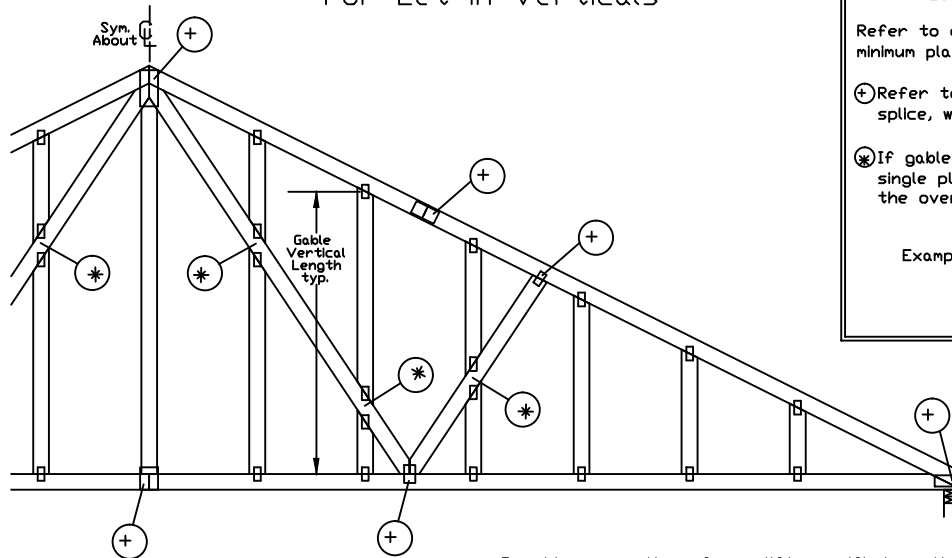
Florida Certificate of Product Approval #FL1999

MAX. TOT. LD. 60 PSF

MAX. SPACING

REF GE WHALER
DATE 01/02/2018
DRWG GABRST160118

Gable Detail For Let-in Verticals

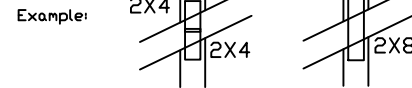


Gable Truss Plate Sizes

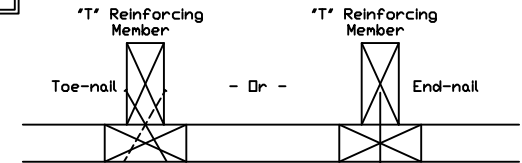
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

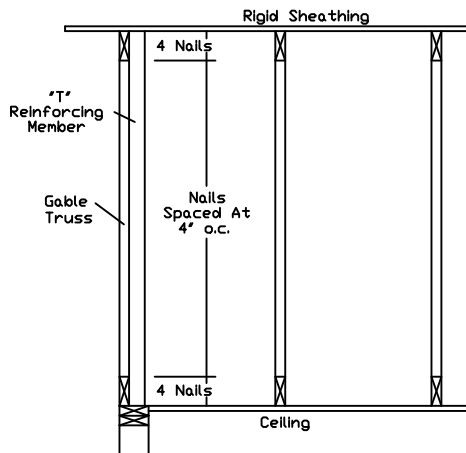
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



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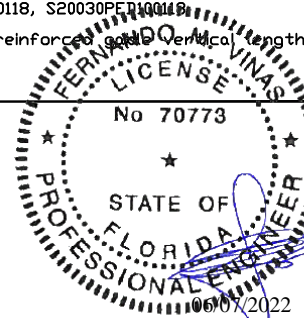
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For more information see this Job's general notes page and these web sites:

ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

ALPINE
AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



COA#0-278

Florida Certificate of Product Approval #FL 1009

REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

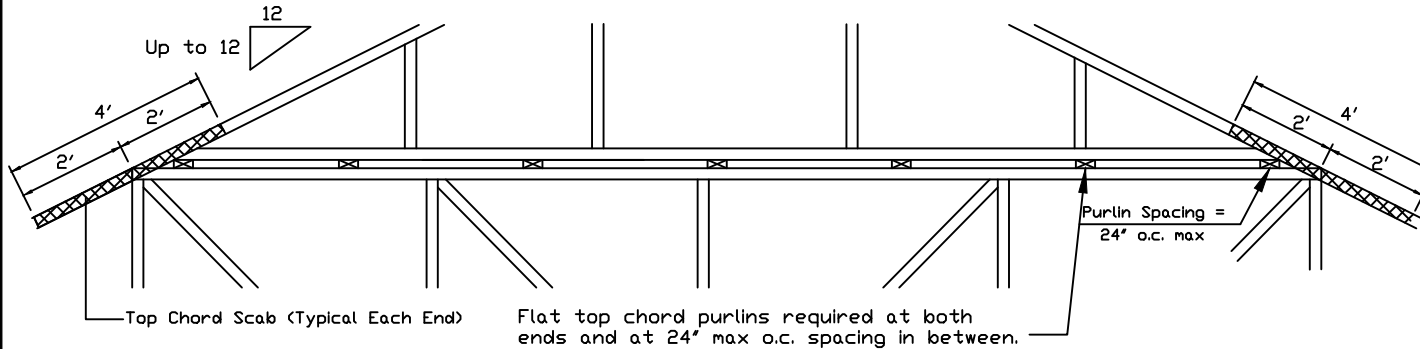
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

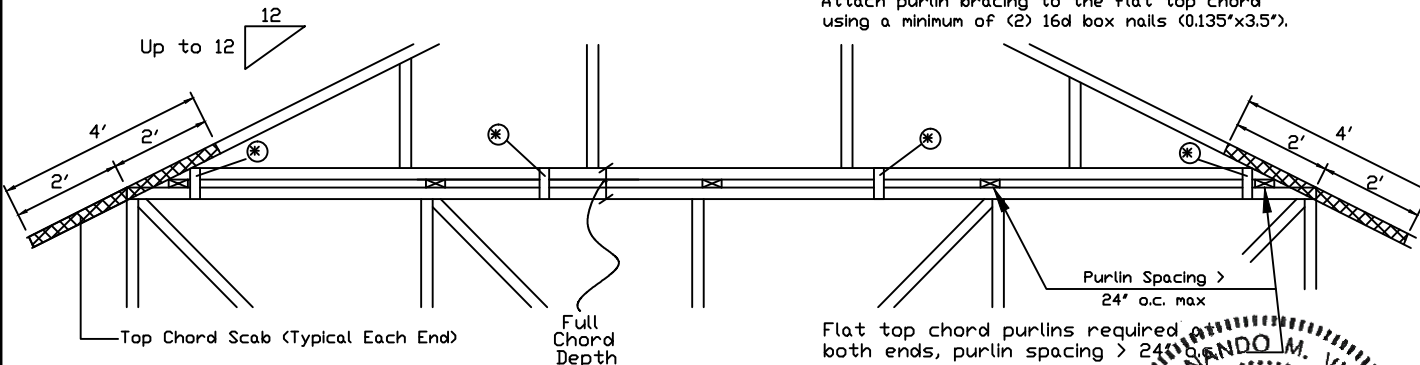


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

Trulox

Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset

8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs

2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate

One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

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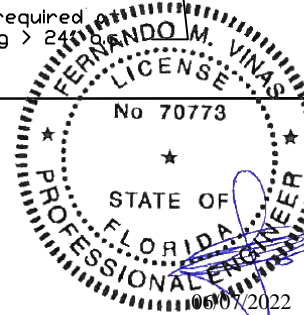
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



COA#0-278

Florida Certificate of Product Approval #FL 1999

SPACING 24.0"

REF PIGGYBACK

DATE 01/02/2018

DRWG PB160160118

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 160 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

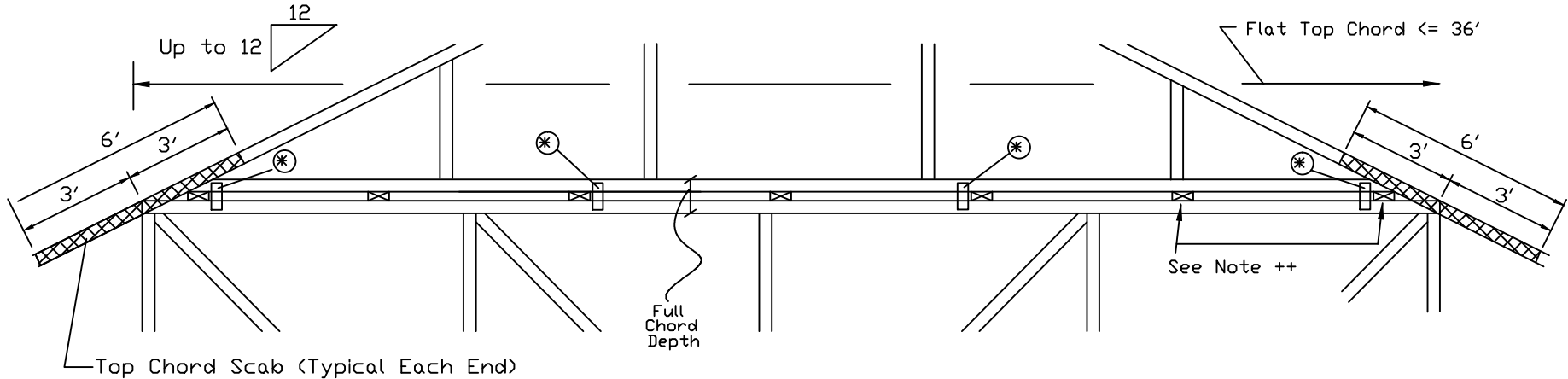
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24' intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



* In addition, provide connection with one of the following methods:

Trulox

Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate

One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset

8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs

2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

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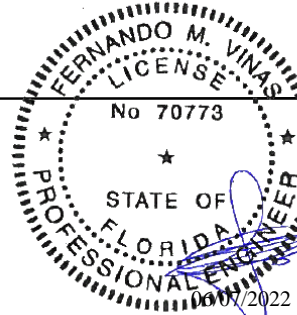
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



COA#0-278

Florida Certificate of Product Approval #PI 1999

SPACING 24.0"

REF PIGGYBACK

DATE 01/02/2018

DRWG PB180160118

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face.
Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

This repair detail may be used for broken connector plate at mid-panel splices.

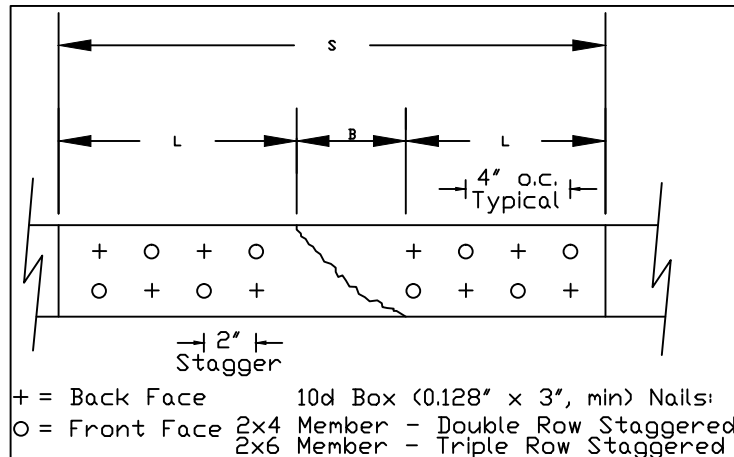
This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

Broken chord may not support any tie-in loads.

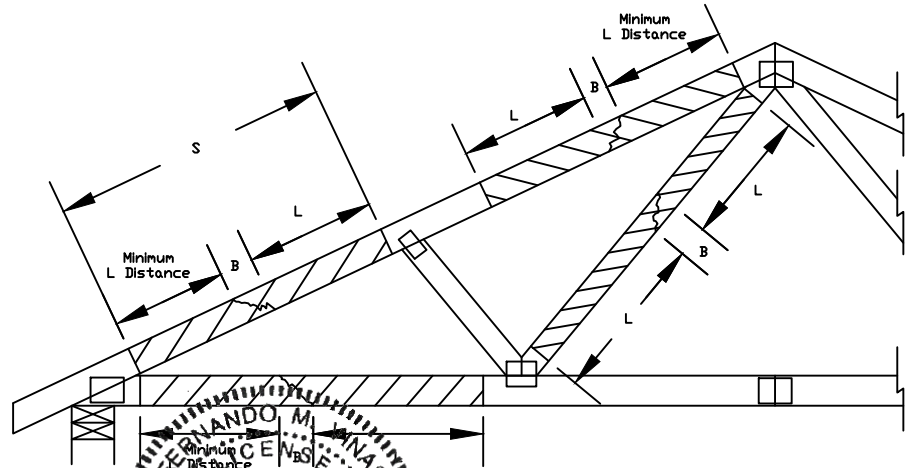
Load Duration = 0%

Member forces may be increased for Duration of Load

Member	Size	L	Maximum Member Axial Force			
			SPF-C	HF	DF-L	SYP
Web Only	2x4	12"	620#	635#	730#	800#
Web Only	2x4	18"	975#	1055#	1295#	1415#
Web or Chord	2x4	24"	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30"	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36"	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42"	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48"	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



Nail Spacing Detail



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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No 70773

STATE OF FLORIDA

PROFESSIONAL ENGINEER

06/07/2022

COA#0-278

SPACING 24.0" MAX

REF MEMBER REPAIR

DATE 10/01/14

DRWG REPCHRD1014

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for
ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
supporting truss material at connection location:
170 mph for SP (G = 0.55, min.),
155 mph for DF-L (G = 0.50, min.), or
120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
below valley trusses.

Bottom chord of valley trusses may be square or
pitched cut as shown.

Valleys short enough to be cut as solid triangular
members from a single 2x6, or larger as required,
shall be permitted in lieu of fabricating from
separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
web, same species and grade or better, attached with 10d box
(0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
Lateral Restraint applied at mid-length of web is permitted with diagonal
bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss
installation.

Or

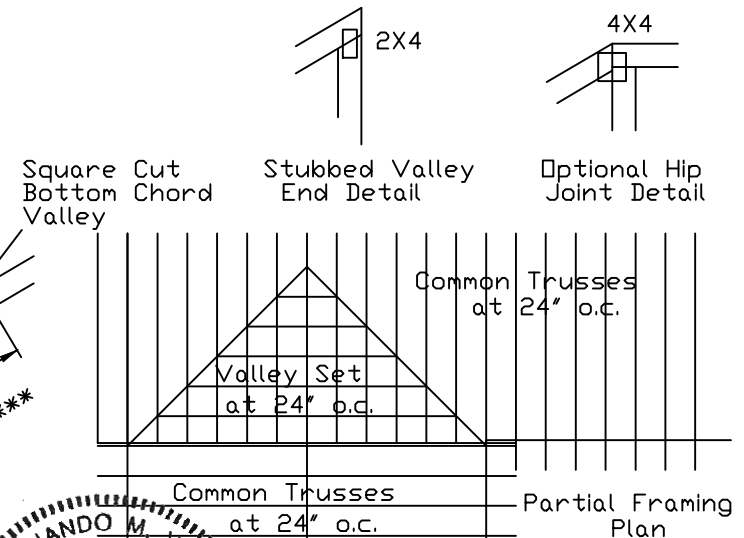
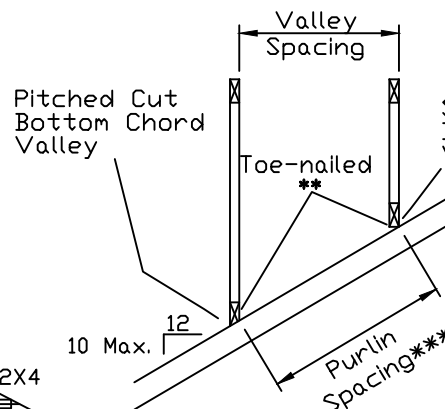
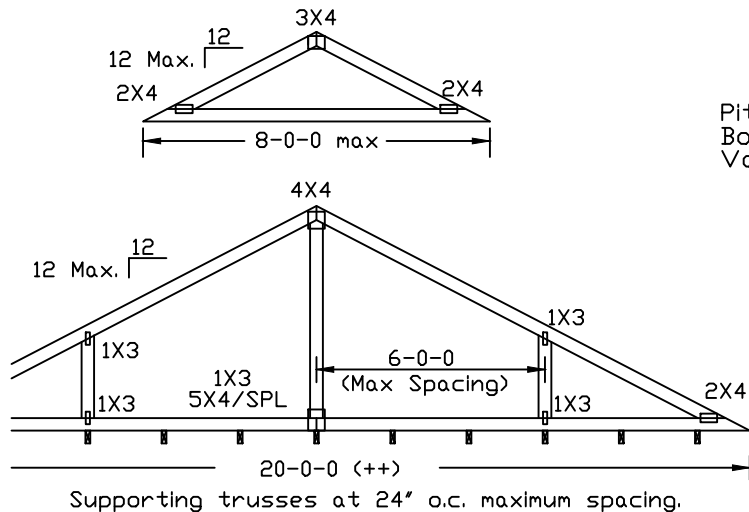
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or

By valley trusses used in lieu of purlin spacing as specified on
Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does
not exceed 14'-0".



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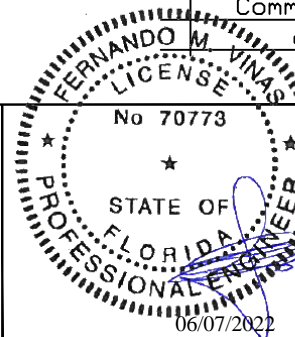
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COA#0-278

Florida Certificate of Product Approval #FL1999

TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10PSF	DRWG	VALTN160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.1.25/1.33	1.15	1.15			
SPACING	24.0"				

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

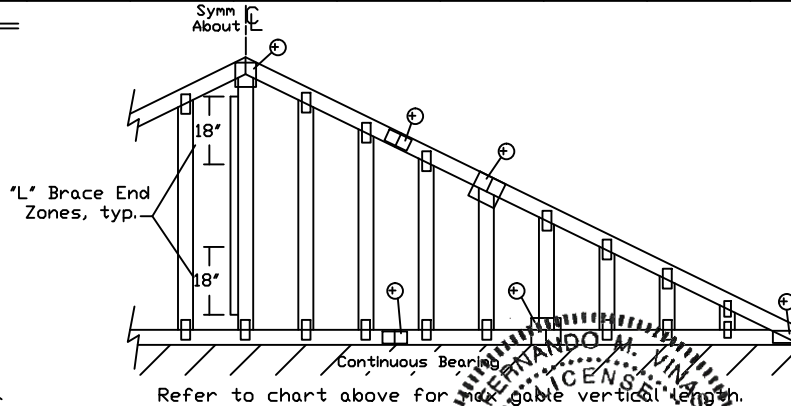
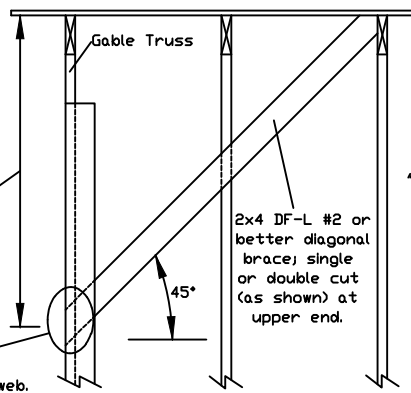
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	7' 8"	8' 2"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 7"	6' 7"	7' 0"	8' 10"	9' 5"	11' 5"	11' 10"	13' 10"	14' 0"	14' 0"	14' 0"
			#2	5' 0"	8' 4"	8' 7"	9' 10"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"
			Standard	4' 7"	6' 0"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"
		Standard	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	#1	5' 9"	9' 6"	9' 10"	11' 3"	11' 8"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1 / #2	#1	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	6' 4"	10' 6"	10' 10"	12' 4"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	#1	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1 / #2	#1	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 335# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' brace: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4
+ Refer to common truss design for peak, splice, and heel plates.	

Refer to the Building Designer for conditions not addressed by this detail.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

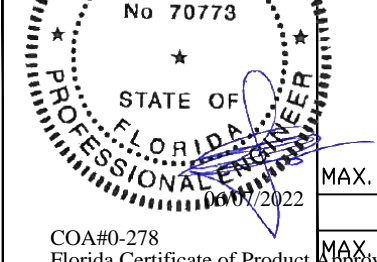
WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING. FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

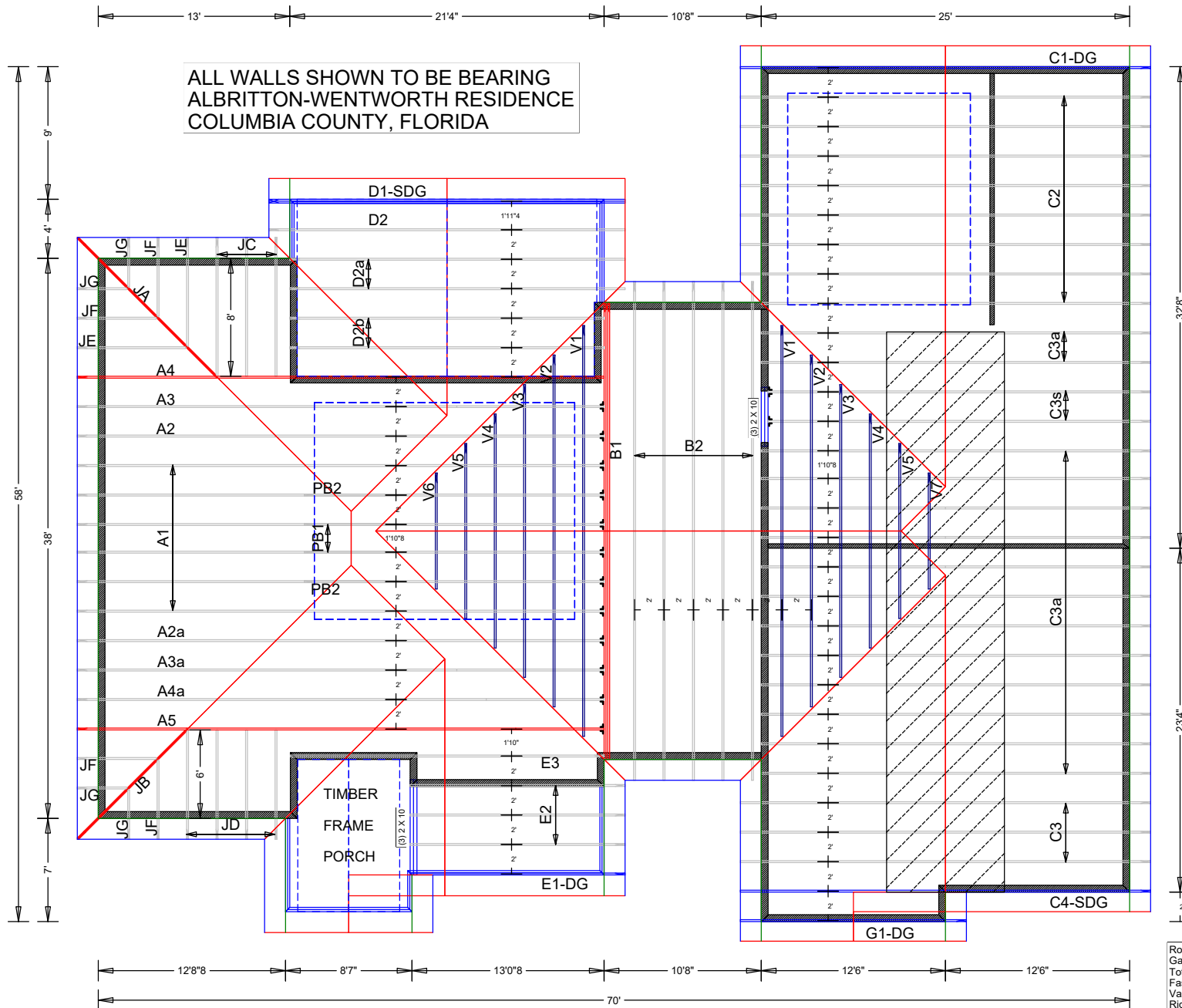
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



REF	ASCE7-16-GAB12015
DATE	01/26/2018
DRWG	A12015ENC160118
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"



Job Name: WENTWORTH RESIDENCE
Customer: CASH-CHASE
Designer: ROBERT J. LITTLE
PlanName: CALEB WENTWORTH 850-295-3
Created : 06-06-2022
SemRef# : B55647a

JOB NO:
B55647a

PAGE NO:
1 OF 1