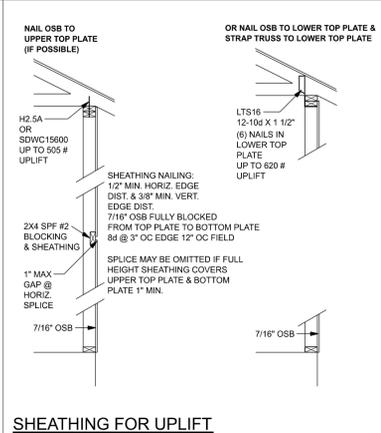
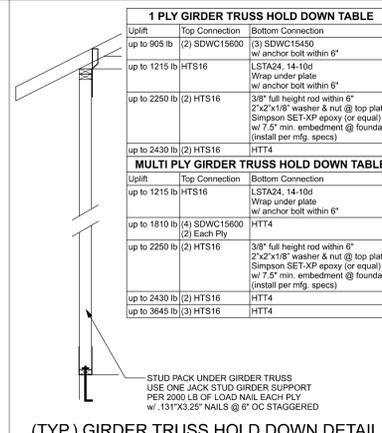


Wind Speed	Sheathing Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. D	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.38" x 0.113")	4" oc	4" oc



Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pfl	SP1 @ 32" OC	SP1 @ 32" OC	48" OC
454 pfl	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
223 pfl	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
636 pfl	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
672 pfl	(2) SDWC15600 @ 16" OC	(3) SDWC15450 @ 16" OC	24" OC
257 pfl	SPH46, (12) 148" x 1 1/2" @ 48" OC	SPH46, (12) 148" x 1 1/2" @ 48" OC	48" OC
387 pfl	SPH46, (12) 148" x 1 1/2" @ 32" OC	SPH46, (12) 148" x 1 1/2" @ 32" OC	32" OC
309 pfl	LSTA24, (14) 148" x 1 1/2" @ 48" OC Wrap Under Plate	LSTA24, (14) 148" x 1 1/2" @ 48" OC Wrap Under Plate	32" OC
465 pfl	LSTA24, (14) 148" x 1 1/2" @ 32" OC Wrap Under Plate	LSTA24, (14) 148" x 1 1/2" @ 32" OC Wrap Under Plate	32" OC



Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
605	505	SDWC15600	4-131'x1 1/2"	4-131'x1 1/2"
605	290	H2.5	5-131'x1 1/2"	5-131'x1 1/2"
625	540	HS3A	5-131'x1 1/2"	5-131'x1 1/2"
1040	1015	H10A	9-148'x1 1/2"	9-148'x1 1/2"
645	515	LTS12-30	6-148'x1 1/2"	6-148'x1 1/2"
990	850	MST512-30	7-148'x1 1/2"	7-148'x1 1/2"
1415	1215	HTS16-30	8-148'x1 1/2"	8-148'x1 1/2"
1235	1235	LSTA21	8-148'x1 1/2"	8-148'x1 1/2"
771	771	MST424	8-148'x1 1/2"	8-148'x1 1/2"
1030	1030	CS20	7-148'x1 1/2"	7-148'x1 1/2"
555	535	SP1	4-148'x3"	4-148'x3"
1010	605	SP2	6-148'x3"	6-148'x3"
1280	1100	SPH46	12-148'x1 1/2"	wrap under or over plate
1640	1460	MST424	10-148'x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14-148'x1 1/2"	wrap under or over plate
Uplift SP Uplift SPF	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-162'x3 1/2"	1/2"x12" Titen HD
Uplift SP Uplift SPF	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
2145	1835	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-162'x3 1/2"	1/2"x12" Titen HD
Uplift SP Uplift SPF	Uplift SPF	Post Bases @ Stewall	To Post	Anchor
1900	1800	ABU622	12-162'x3 1/2"	5/8"x12" Drill & Epoxy
1900	1800	ABU622	12-162'x3 1/2"	5/8"x12" Drill & Epoxy
1900	1800	ABU622	12-162'x3 1/2"	5/8"x12" Drill & Epoxy
2475	1800	ABU662	12-162'x3 1/2"	5/8"x12" Drill & Epoxy

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS MANUFACTURER CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION LOADS ON BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415L EACH END. 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 150 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x 4 x W1.4, Fy = 80KSI. WELDED WIRE REINFORCEMENT FABRIC (W1.4) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.175 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWF OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, Fy = 40 KSI, ALL LAP SPACES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 318-98, U.N.O.

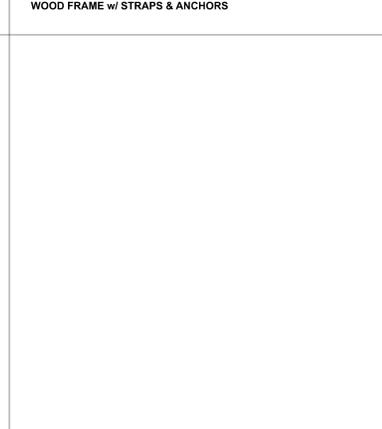
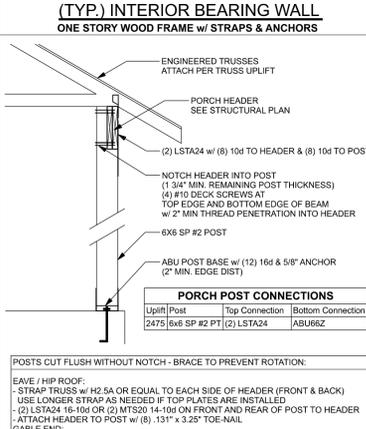
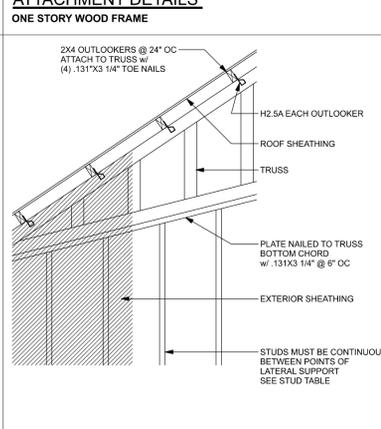
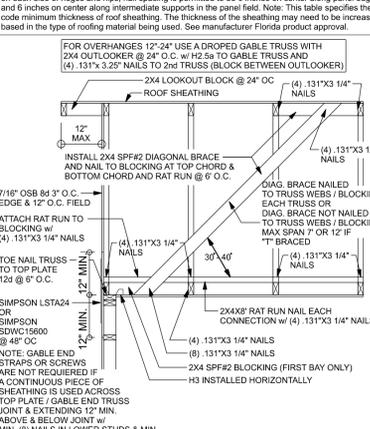
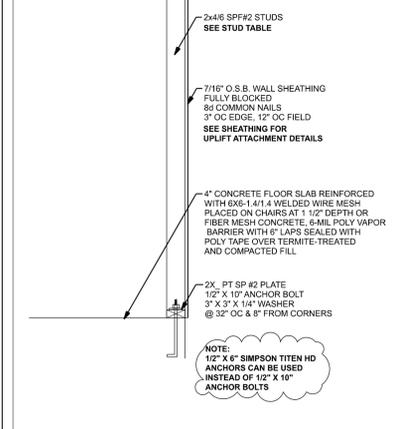
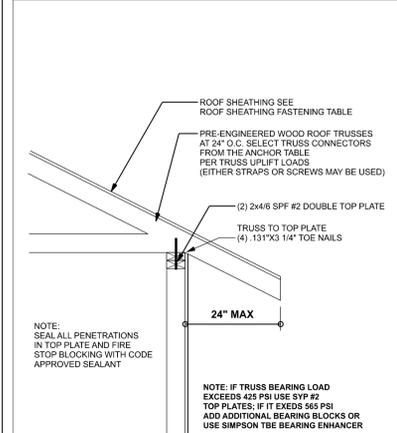
ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3' FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

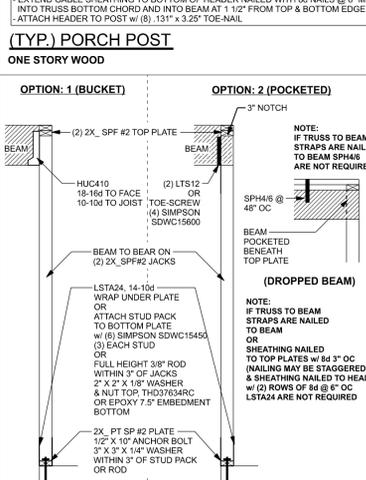
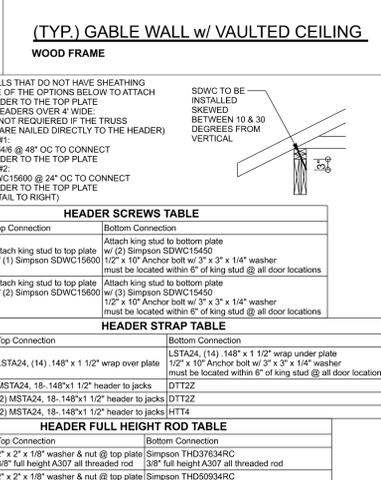
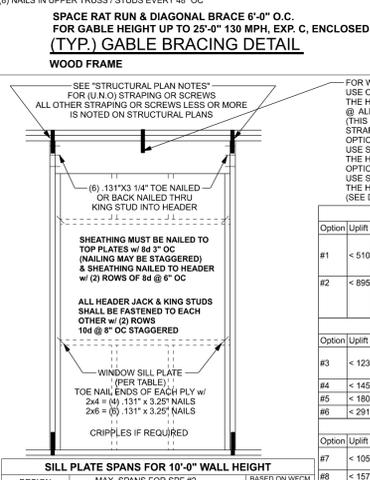
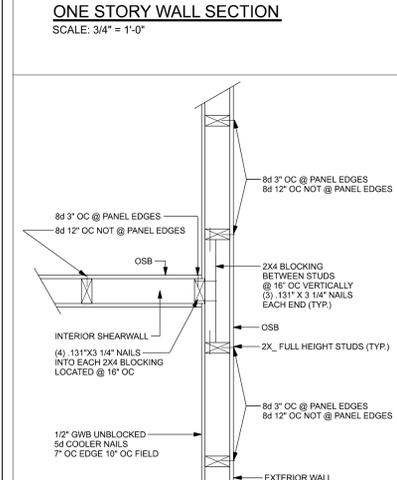
BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH ALL FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WITH RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SHEETS.

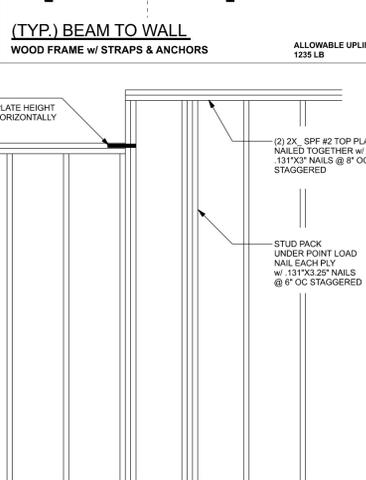
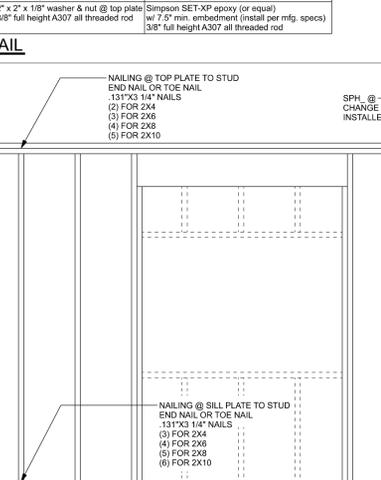
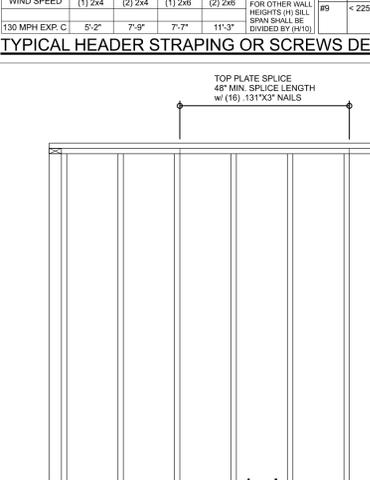
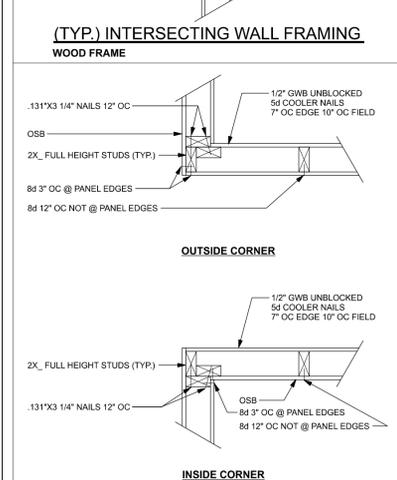
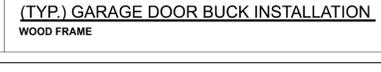


Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
(1) 2x4 @ 16" OC	(1) 2x4 @ 16" OC	(1) 2x4 @ 16" OC	10" x 1" STUD HEIGHT	
(1) 2x4 @ 12" OC	(1) 2x4 @ 12" OC	(1) 2x4 @ 12" OC	11" x 2" STUD HEIGHT	
(1) 2x6 @ 16" OC	(1) 2x6 @ 16" OC	(1) 2x6 @ 16" OC	15" x 7" STUD HEIGHT	
(1) 2x6 @ 12" OC	(1) 2x6 @ 12" OC	(1) 2x6 @ 12" OC	17" x 3" STUD HEIGHT	

Grade	Species	Fd	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND 1700	2.0	2.0
LVL	MICROLAM	1950	1.7
PSL	PARALAM	2900	2.0



DOOR WIDTH	3/8"x4" LAG	16d STAGGER	(2) ROWS OF 131'x3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	BASIC WIND SPEED (ASCE 7-22, 3S GUST) 130 MPH WIND EXPOSURE (BUILDER MUST FIELD VERIFY) C TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY) I ENCLOSURE CLASSIFICATION ENCLOSED RISK CATEGORY II INTERNAL PRESSURE COEFFICIENT 0.18 ROOF ANGLE 7-45 DEGREES MEAN ROOF HEIGHT 30 FT
C&D DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	ROOMS OTHER THAN SLEEPING ROOMS 40 PSF LIVE LOAD FLAT OR < 4:12 20 PSF LIVE LOAD 4:12 TO < 12:12 16 PSF LIVE LOAD 12:12 & GREATER 12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Valt) -46.2(Valt)	+42.6(Valt) -57(Valt)

Amira Builders
Miliken Res.
PROJECT ADDRESS: Gainesville, FL

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS: Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

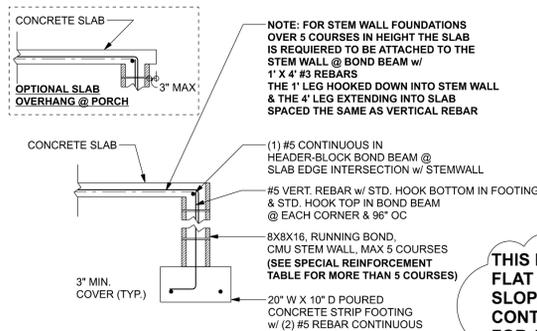
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
260007

S-1
OF 3 SHEETS



TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

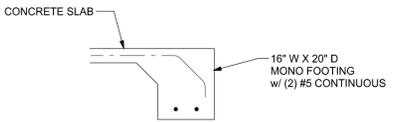
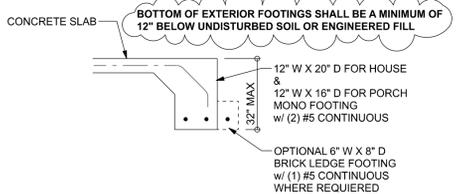
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

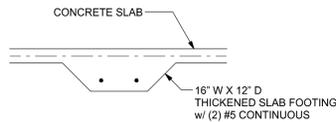
ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength: 8" block bearing walls F'm = 1500 psi
2.1	Mortar: ASTM C 270, Type N, UNO
2.2	Grout: ASTM C 476, admixtures require approval
2.3	CMU standard: ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard: ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11: ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection: Anchors, sheet metal ties completely embedded in mortar or grout. ASTM A525, Class G60, 0.60 oz/lb or 304SS. Joint reinforcement in walls exposed to moisture or wet ties, anchors, sheet metal ties not completely embedded in mortar or grout. ASTM A153, Class B2, 1.50 oz/lb or 304SS
2.4F	Coating for corrosion protection: Same as above
3.3.E.2	Pipes, conduits, and accessories: Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints: Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12', CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING

F1 S-2 OPTIONAL STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

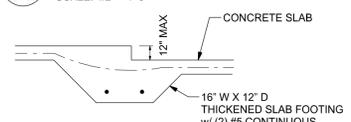


F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"

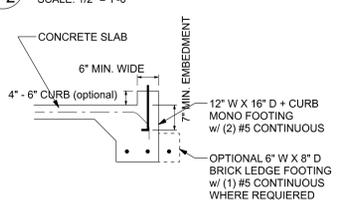


F1A S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"

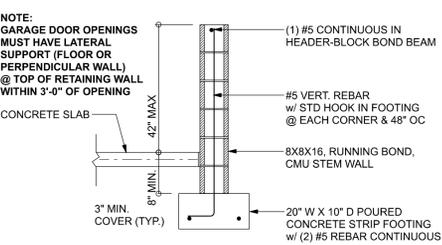
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



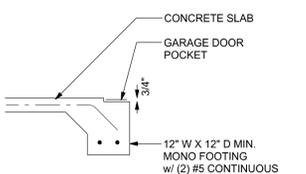
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



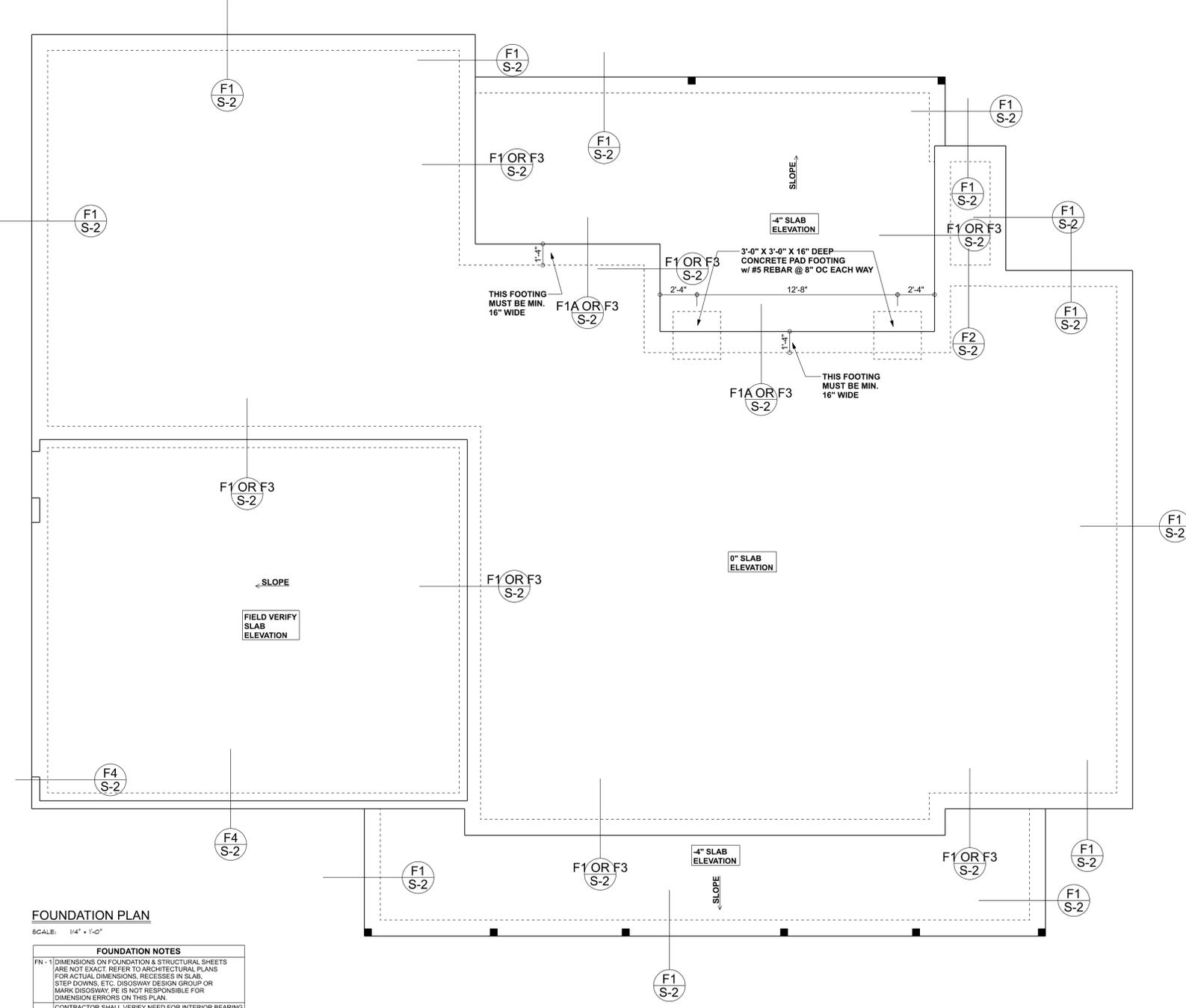
F4 S-2 MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 OPTIONAL STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOWAY DESIGN GROUP OR MARK DISOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ #6X8-1/4" WELDED WIRE MESH PLACED ON CHAIRS @ 11" DEPTH OR FIBER MESH CONCRETE, 5-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMI-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMI-TREATMENT METHOD CAN BE USED INSTEAD)

Amira Builders
Miliken Res.
PROJECT ADDRESS:
Columbia County, FL

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
260007

S-2
OF 3 SHEETS

