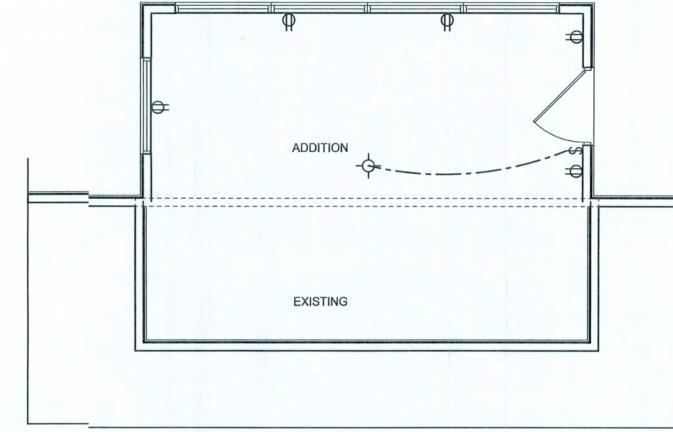
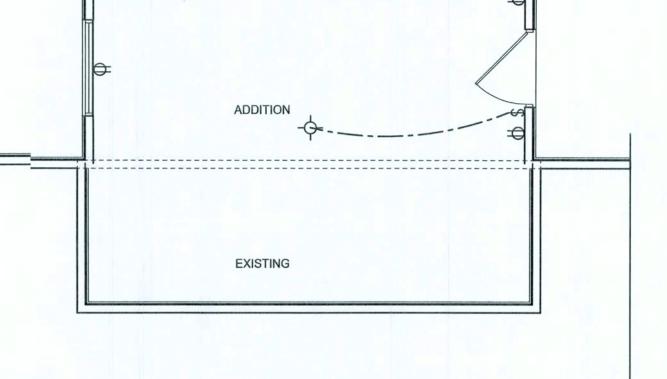


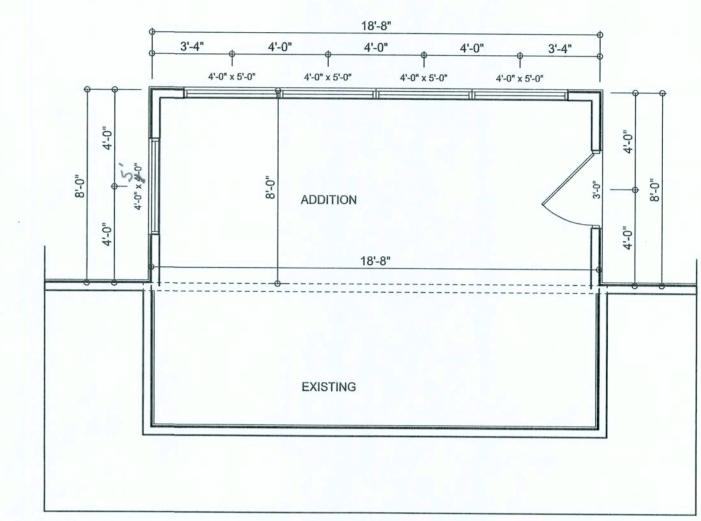
TYPICAL DESIGN WALL SECTION

ELECTRICAL LAN NOTES

- E -1 WIRE ALL APPLIANCE HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFIATIONS.
- E -2 CONSULT THE OWNEROR THE NUMBER OF SEPERATE TELEPHONE LINES TGE INSTALLED.
- E -3 ALL INSTALLATIONS SALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTCS SHALL BE 120V W/ BATTERY E -4 BACKUP OF THE PHODELECTRIC TYPE, AND SHALL BE INTERLOCKED TOETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS
- TELEPHONE, TELEVISN AND OTHER LOW VOLTAGE DEVICES OR OUTLETSHALL BE AS PER THE OWNER'S E -5 DIRECTIONS, & IN ACORDANCE W/ APPLICABLE SECTIONS OF NEC-LAEST EDITION.
- E -6 ELECTRICAL CONT'R HALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF FECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (NDERGROUND OR OVERHEAD) TO BE DETERMINED E POWER COMPANY.
- E -8 ALL BEDROOM RECEFACLES SHALL BE AFCI (ARC FAULT CIRCUIT (TERRUPT)
- E -9 ALL OUTLETS TO BE LCATED ABOVE BASE FLOOD ELEVATION
- A SERVICE DISCONNET WITH OVER CURRENT PROTECTION SHALL BE INSTALLED UTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE MEER, AT THE PLACE ELECTRIC
- E -10 CONDUCTORS ENTERHE BUILDING. SERVICE ENTRANCE ONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THBUILDING WITHOUT SPECIAL APPROVAL OF THE BLDING OFFICIAL



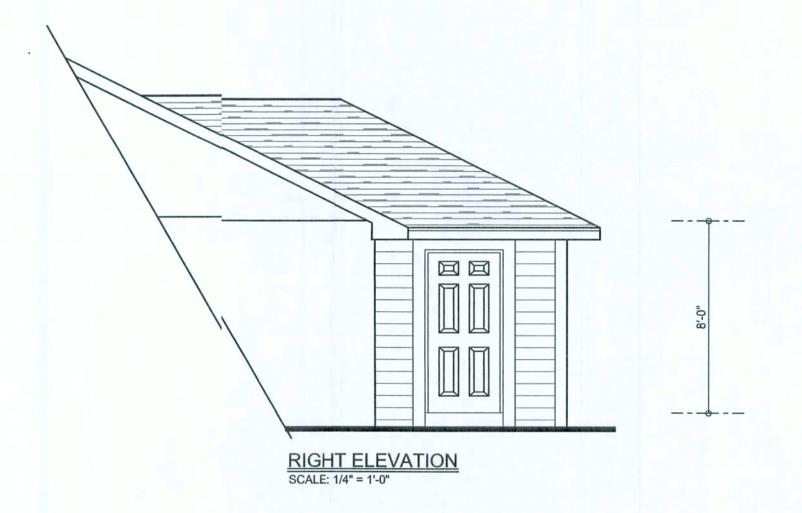




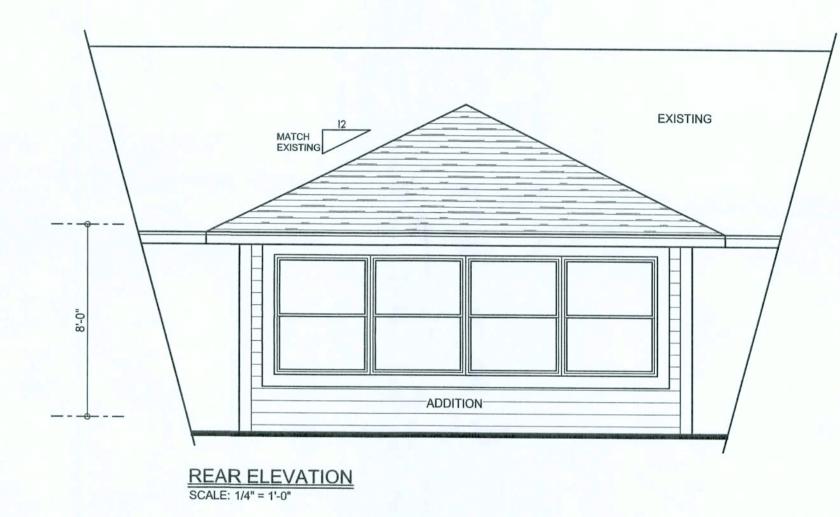
FLOOR PLAN SCALE: 1/4" = 1'-0"

AREA SUMMARY

ADDITION AREA 149 S.F. TOTAL AREA 149 S.F.



FLOOR PLAN
SCALE: 1/4"4" = 1'-0"



ELECTRICAL LEGEND (PRE-WIRE FOR LIGHT KIT) DOUBLE SECURITY 2X4 FLUORESCENT LIGHT FIXTURE RECESSED CAN LIGHT BATH EXAUST FAN WITH LIGHT BATH EXAUST FAN LIGHT FIXTURE DUPLEX OUTLET 220v OUTLET GFI DUPLEX OUTLET SMOKE DETECTOR WALL SWITCH 3 WAY WALL SWITCH 4 WAY WALL SWITCH WATER PROOF GFI OUTLET PHONE JACK ∇ TELEVISION JACK GARAGE DOOR OPENER 岡 WALL HEATER

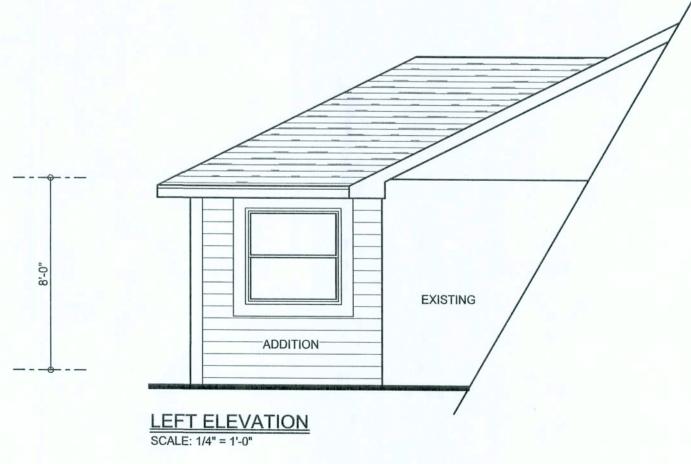
REQUIRED ROOF VENTILATION: AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT MIN. 50% TOTAL VENT AREA LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE) 149 S.F. / 300 x 50% = 0.3 S.F. RIDGE VENT AREA REQUIRED 3 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT 149 S.F. / 300 x 50% = 0.3 S.F. SOFFIT VENT AREA REQUIRED 4 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT) 2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



VINDLOAD EN;INEER: Mark Disosway, PE No.53915, P)B 868, Lake City, FL 32056, 386-7545419 Stated dimensions supercede scaled dimensions. Retr all questions to Mark Disosway,P.E. for resolution. Do not proceed vithout clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, J.E. hereby expressly reserves its common law opyrights and property right in these instrumens of service. This document is not to be reprodiced, altered or copied in any form or manner vithout first the express written permission and onsent of Mark Disosway.

REVISIONS

SCFTPIXN

CERTIFICATIOI: I hereby certify that I have examined this pan, and that the applicable portions of the pan, relating to wind engineering comply with secon R301.2.1, florida building code residential?004, to the best of my

LIMITATION: The design is valid for one building, at spedied location.

MARK DISOSWAY P.E. 53915

CURRIS & BEVERLY WIXON

ADDITION

ADDRESS: Lot 4 Block I, East Side Village, Unit Lak City FL 32025

Mark Disosway P.E. P.J. Box 868 Lake Ciy, Florida 32056 Phone: (386) 754 - 5419 Fax: (\$86) 269 - 4871

PRINTED DATE: Mach 07, 2007 DRAWN BY STRUCTURAL BY: Ben Sparks Ben Sparks

FINALS DATE:

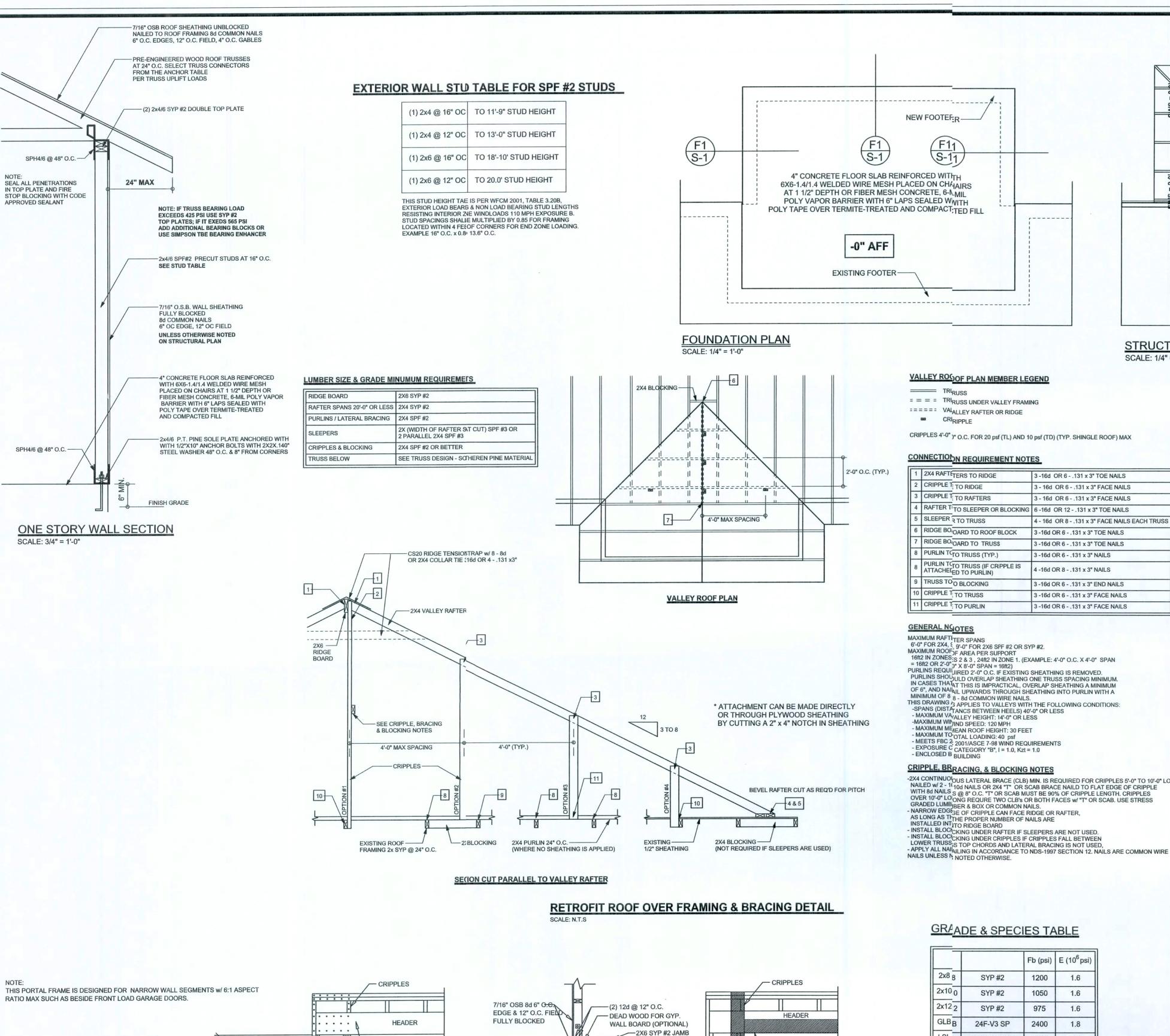
07 / Mar / (7 JOE NUMBER: 702264

> DRAWING NUMBER A-1 CF 2 SHEETS

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MINIMUM (2) 2X12 WITH 1/2" OSBPACER

BUILT UP WITH 12dS COMMON NLS @

FASTEN SHEATHING TO HEADEWITH 8d

-FOR A PANEL SPLICE (IF NEED!), PANEL

EDGES SHALL BE BLOCKED w/) 2X4, AND

OCCUR WITHIN 24" OF MID-HEIGT. 2X_'S

MUST BE STITCH NAILED TOGETER WITH

COMMON NAILS IN 3" GRID PATRN IN

HEADER AND 3" O.C. IN ALL FRANG

16" O.C. ALONG EACH EDGE

(STUD, AND SILLS) (TYP.)

(2) 12dS @ 3" O.C.

-1/2"x8" ANCHOR BOLTS w

FOUNDATION

OUTSIDE ELEVATION

2"x2"x.140" STEEL WASHE

-2X4/6 SPF #2

PORTAL FRAME WALL

SEGMENT (TYP.)

NOTE: DO NOT

SPLIT WOOD

CORNER NEXT TO-

FULLY SHEATHED

FRAMING DETAIL

MINIMUM WIDTH

MSTA18, 10-10d X 1 1/2" WRAP UNDER

PLATE FOR UP TO 530 LB PER TRUSS

UPLIFT (OR SEE HEADER DETAIL FOR

UPLIFT CONNECTION)

16" FOR 8'-0" WALL

18" FOR 9'-0" WALL

20" FOR 10'-0" WALL

(6:1 ASPECT RATIO MAX)

WALL SEE CORNER

-1/2" SHEATHING FILLER

(STUD, AND SILLS) (TYP.)

1 1/2" GRID IN HEADER

UPLIFT CONNECTION)

SECTION DETAIL

-1/2"x8" ANCHOR BOLTS w

2"x2"x.140" STEEL WASHER

-2X6 TOP JAMB (NON STRUCTURAL)

-FASTEN SHEATHING TO HEADER WITH 8d

-2X6 SYP #2 JAMB CONTIUOUS FLOOR TO TOP

OF HEADER (2) 12d @ 6" O.C. IN JACK STUDS &

W58 - PORTAL FRAME SHEARWALL DETAIL
SCALE: 1/2"=1'-0"

COMMON NAILS IN 3" GRID PATTERN IN

HEADER AND 3" O.C. IN ALL FRAMING

MSTA18, 10-10d X 1 1/2" WRAP UNDER

PLATE FOR UP TO 530 LB PER TRUSS

UPLIFT (OR SEE HEADER DETAIL FOR

(OPTIONAL)

GRAADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8 ₈	SYP #2	1200	1.6
2x10 ₀	SYP #2	1050	1.6
2x12 2	SYP #2	975	1.6
GLBB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

ROOF SYSTEM DESIGN

-2X6 TOP JAMB (NON STRUCTURAL)

-2X4/6 SPF #2 3J & 1K

LOCATED AS SHOWN

-1/2"x8" ANCHOR BOLTS v

FOUNDATION

INSIDE ELEVATION

2"x2"x.140" STEEL WASHE

—(3) 2X4 SPF #2 JACK STUD\$

(1) 2X4 SPF #2 KING STUD

2X6 SYP #2 JAMB CONTIUOUS FLOOR T

JACK STUDS & 1 1/2" GRID IN HEADER

MSTA18, 10-10d X 1 1/2" WRAP UNDER

PLATE FOR UP TO 530 LB PER TRUSS

UPLIFT (OR SEE HEADER DETAIL FOR

UPLIFT CONNECTION)

TOP OF HEADER (2) 12d @ 6" O.C. IN

CORNER FRAMING DETAIL

THE SEAL OF IN THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS B BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN COMPLETE F ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTU-URER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONNAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND 5 ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EAC, CH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF BRACING. TITHE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECICAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT TRUSS MANUUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEELETS

USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

2.5'

8.5

-6"X6" W1.4XW1.4 W.W.M. PLACED AT 2" DEPTH ON CHAIRS OR FIBERMESH CONCRETE

3000 - PSI AT 28 DAYS

-6 MIL VAPOR BARRIER

WITH POLY TAPE

COMPACTED FILL

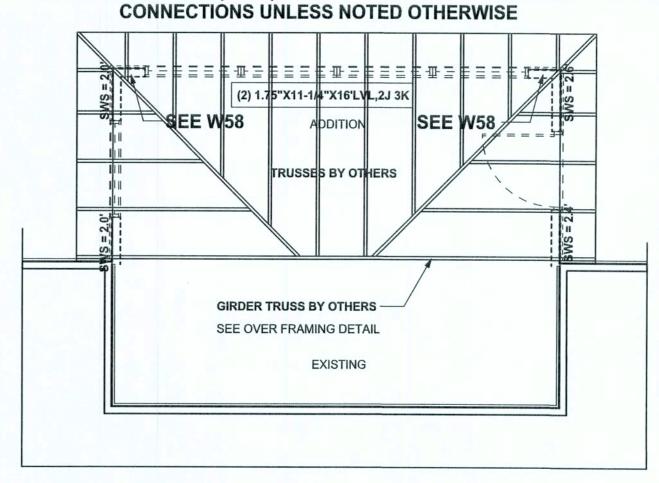
(2) #5 CONTINUOUS

MONOLITHIC FOOTING

WITH 6" LAPS SEALED

TRANSVERSE 2.0'

LONGITUDINAL 8.0'



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

VALLEY ROGOF PLAN MEMBER LEGEND

==== TRI_{RUSS} = = = TRI_{RUSS} UNDER VALLEY FRAMING ===== VALALLEY RAFTER OR RIDGE CRI_{RIPPLE}

CRIPPLES 4'-0")" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTETERS TO RIDGE	3 -16d OR 6131 x 3" TOE NAILS
2	CRIPPLE T: TO RIDGE	3 - 16d OR 6131 x 3" FACE NAILS
3	CRIPPLE T: TO RAFTERS	3 - 16d OR 6131 x 3" FACE NAILS
4	RAFTER TITO SLEEPER OR BLOCKING	6 -16d OR 12131 x 3" TOE NAILS
5	SLEEPER R TO TRUSS	4 - 16d OR 8131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOOARD TO ROOF BLOCK	3 -16d OR 6131 x 3" TOE NAILS
7	RIDGE BOIOARD TO TRUSS	3 -16d OR 6131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3 -16d OR 6131 x 3" NAILS
8	PURLIN TOTO TRUSS (IF CRIPPLE IS ATTACHELED TO PURLIN)	4 -16d OR 8131 x 3" NAILS
9	TRUSS TO O BLOCKING	3 -16d OR 6131 x 3" END NAILS
10	CRIPPLE T: TO TRUSS	3 -16d OR 6131 x 3" FACE NAILS
11	CRIPPLE T: TO PURLIN	3 -16d OR 6131 x 3" FACE NAILS

MAXIMUM RAFTITER SPANS 6'-0" FOR 2X4, §, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
MAXIMUM ROOF)F AREA PER SUPPORT 16ft2 IN ZONES: S 2 & 3 , 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN PURLINS REQUILIBRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED. PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINING. IN CASES THATAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIIAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 8 - 8d COMMON WIRE NAILS. THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
-SPANS (DISTATANCS BETWEEN HEELS) 40'-0" OR LESS MAXIMUM VA/ALLEY HEIGHT: 14'-0" OR LESS -MAXIMUM WIDIND SPEED: 120 MPH
- MAXIMUM MIDIND SPEED: 120 MPH
- MAXIMUM MEALAN ROOF HEIGHT: 30 FEET
- MAXIMUM TO-OTAL LOADING: 40 psf
- MEETS FBC 2: 2001/ASCE 7-98 WIND REQUIREMENTS
- EXPOSURE C CATEGORY "B", I = 1.0, Kzt = 1.0
- ENCLOSED B BUILDING:

CRIPPLE, BRRACING, & BLOCKING NOTES

2X4 CONTINUO OUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LON NAILED W/ 2 - 1(10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS S @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LO ONG REQUIRE TWO CLB'S OR BOTH FACES W/ "T" OR SCAB. USE STRESS GRADED LUMBIBER & BOX OR COMMON NAILS.

- NARROW EDGEGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS TITHE PROPER NUMBER OF NAILS ARE INSTALLED INTITO RIDGE BOARD - INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED. - INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS STOP CHORDS AND LATERAL BRACING IS NOT USED,

		Fb (psi)	E (10 ⁶ psi)
2x8 8	SYP #2	1200	1.6
2x10 ₀	SYP #2	1050	1.6
2x12 2	SYP #2	975	1.6
GLB _B	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSLL	PARALAM	2900	2.0

TRUSS ENGISINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONNSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE SYSTEM AS 5 A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL RESPONSIBL₃LE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE

STRUCTURAL PLAN NOTES

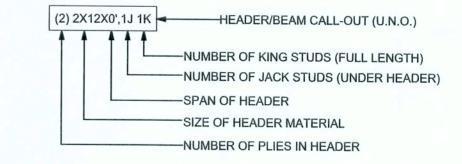
ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW \$5555551 = = = = 12555553	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

SCALE: 1/2" = 1'-0"

GRADE

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND | BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTMIC 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

DESIGN DATA WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B 3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING) 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2)) 1 | 19.9 | -21.8 | 18.1 | -18.1 2 | 19.9 | -25.5 | 18.1 | -21.8 2 O'hg -40.6 3 19.9 -25.5 18.1 -21.8 3 O'hg -68.3 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows | 21.8 | -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22. 16x7 Garage Door 18.5 -21.0

	55 22			
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

PE No.53915, POB 868, ake City, FL 32056, 386-754-5419

REVISIONS

SOFTPIAN

Stated dimensions superede scaled dimensions. Refer all quistions to Mark Disosway, P.E. for esolution Do not proceed without carification.

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CERTIFICATION: I herely certify that I have amined this plan, and nat the applicable ortions of the plan, relang to wind engine comply with section R30.2.1, florida buildin ode residential 2004, tothe best of my

IMITATION: This design is valid for one



CURRIS & BEVERLY WIXON

ADDITION

ADDRESS: Lot 4 Block F, East side Village, Uni Lake City L 32025

Mark Disosway P.E. P.O. Bex 868 Lake City, Fbrida 32056 Phone: (386 754 - 5419 Fax: (386) 269 - 4871

PRINTEL DATE: March 08,2007 DRAWN BY: STRUCTURAL BY Ben Sparks Ben Sparks

FINALS DATE: 07 / Mar / 07

JOB NUMBER: 702264

DRAWING NUMBER

OF 2 SHEETS