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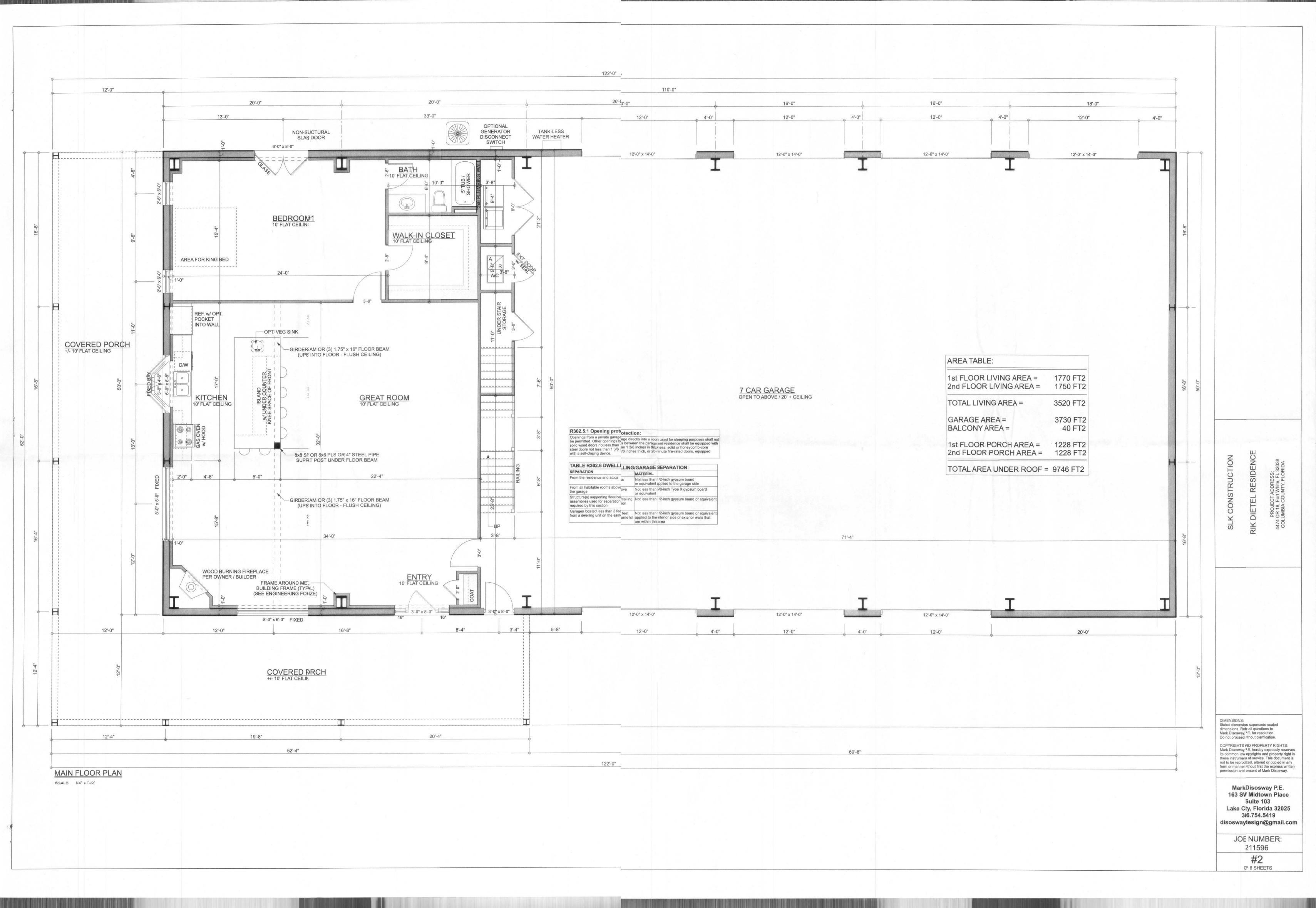
RIK DIETEL RESIDEN

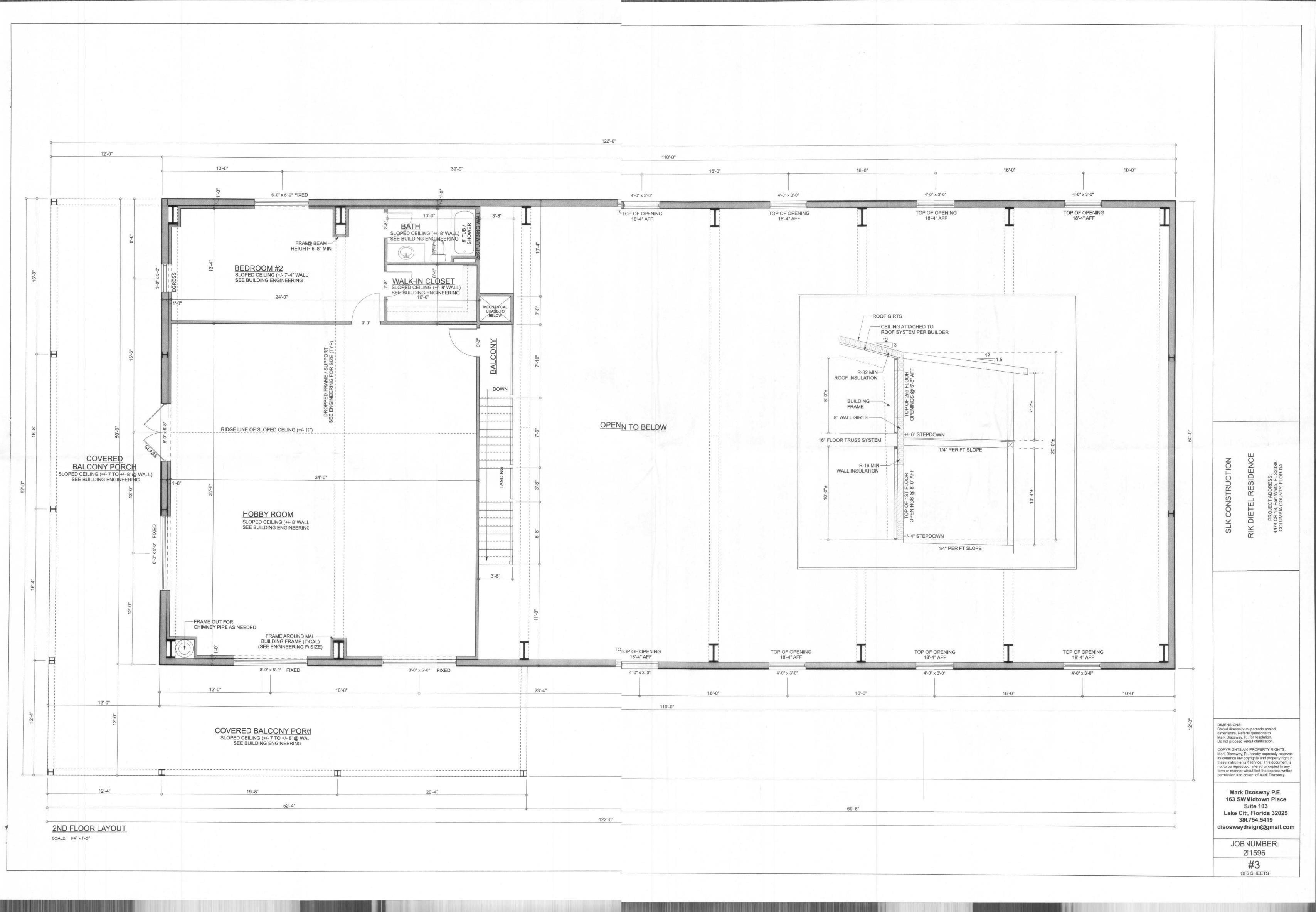
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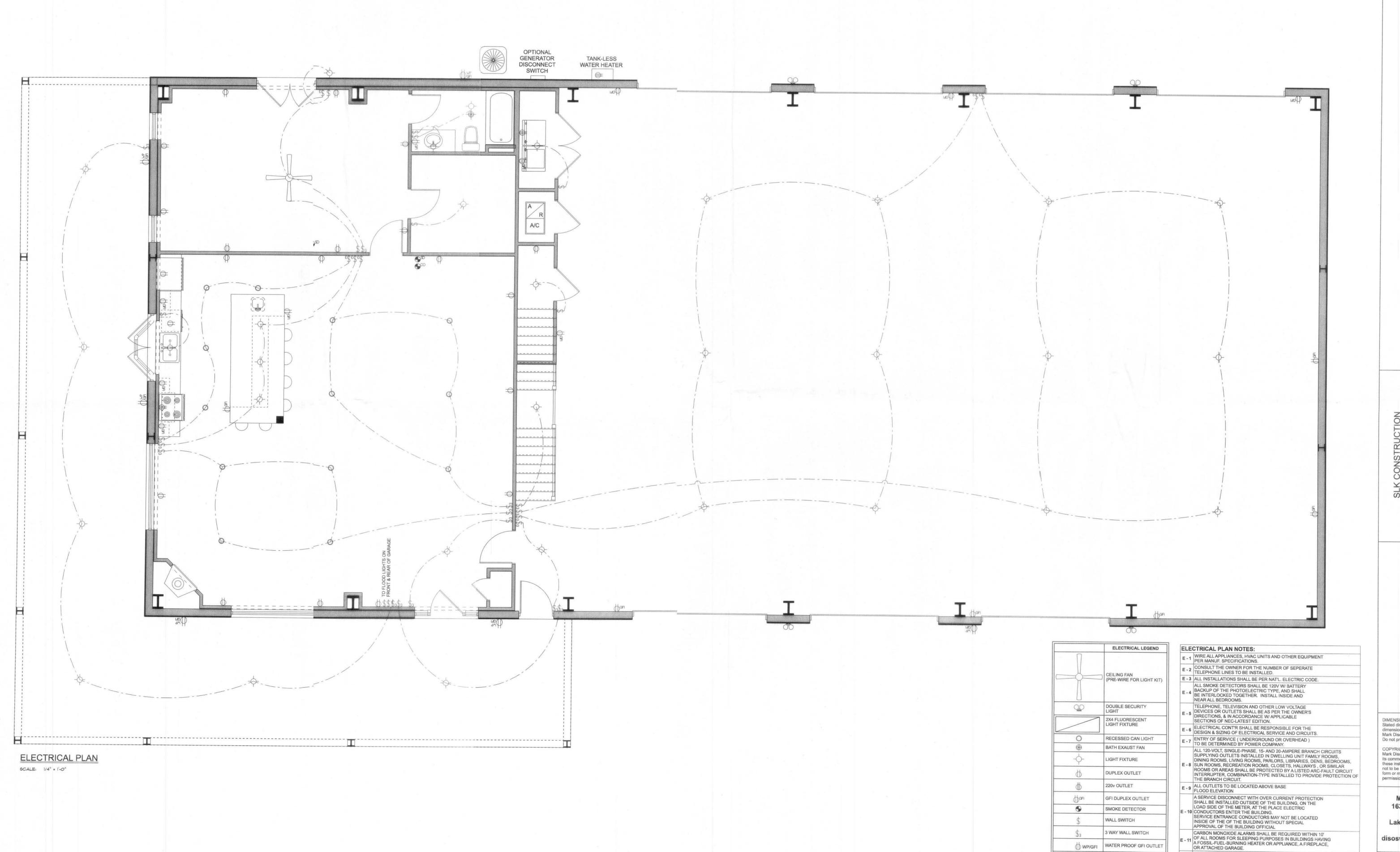
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JOB NUMBER: 211596

#1 OF6 SHEETS







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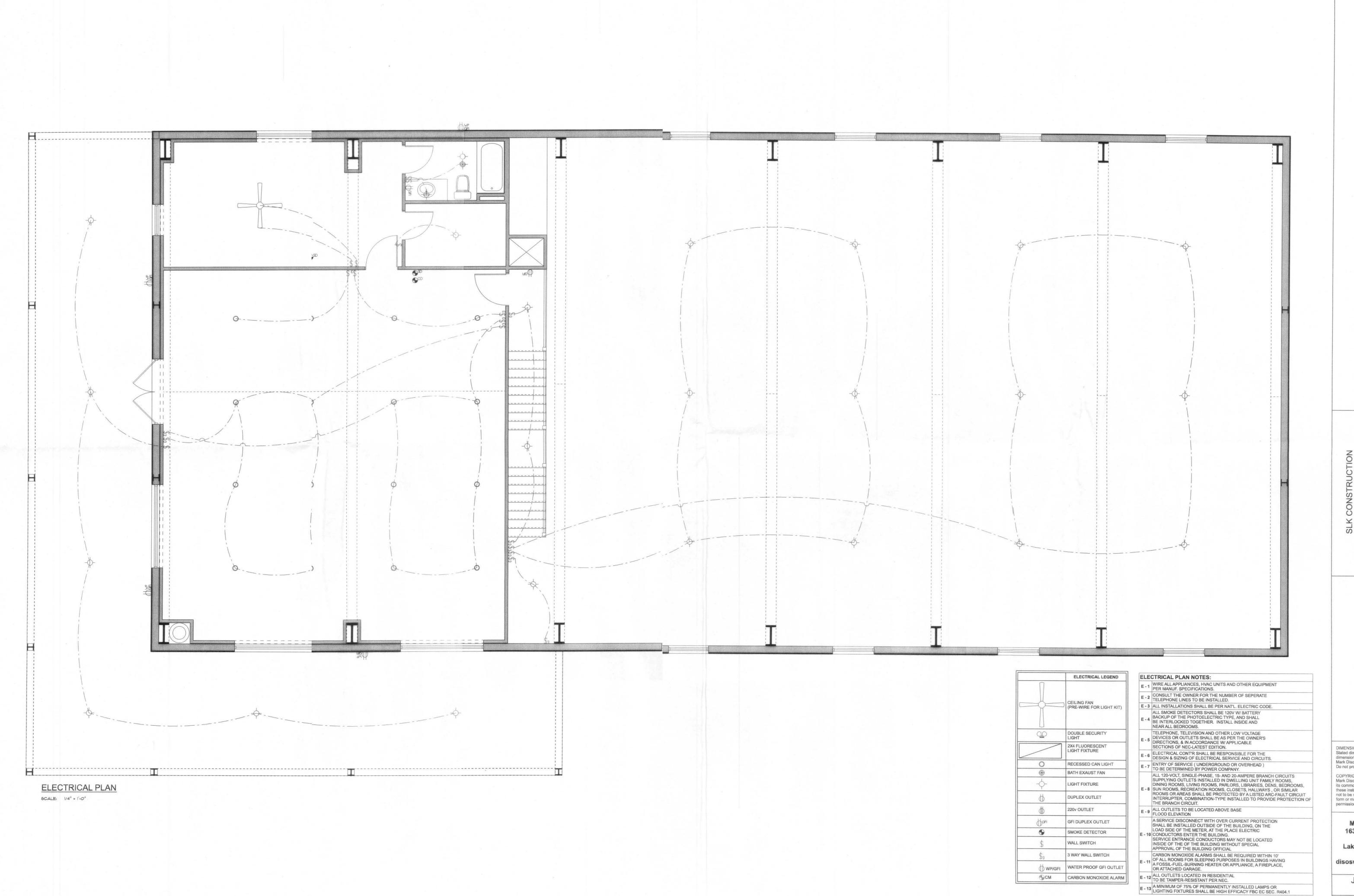
WP/GFI WATER PROOF GFI OUTLET

CARBON MONOXIDE ALARM

E - 12 ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.

E - 13 A MINIMUM OF 75% OF PERMANENTLY INSTALLED LAMPS OR LIGHTING FIXTURES SHALL BE HIGH EFFICACY FBC EC SEC. R404.1

OF 6 SHEETS



RIK DIETEL RESIDENCE

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#5 (F 6 SHEETS

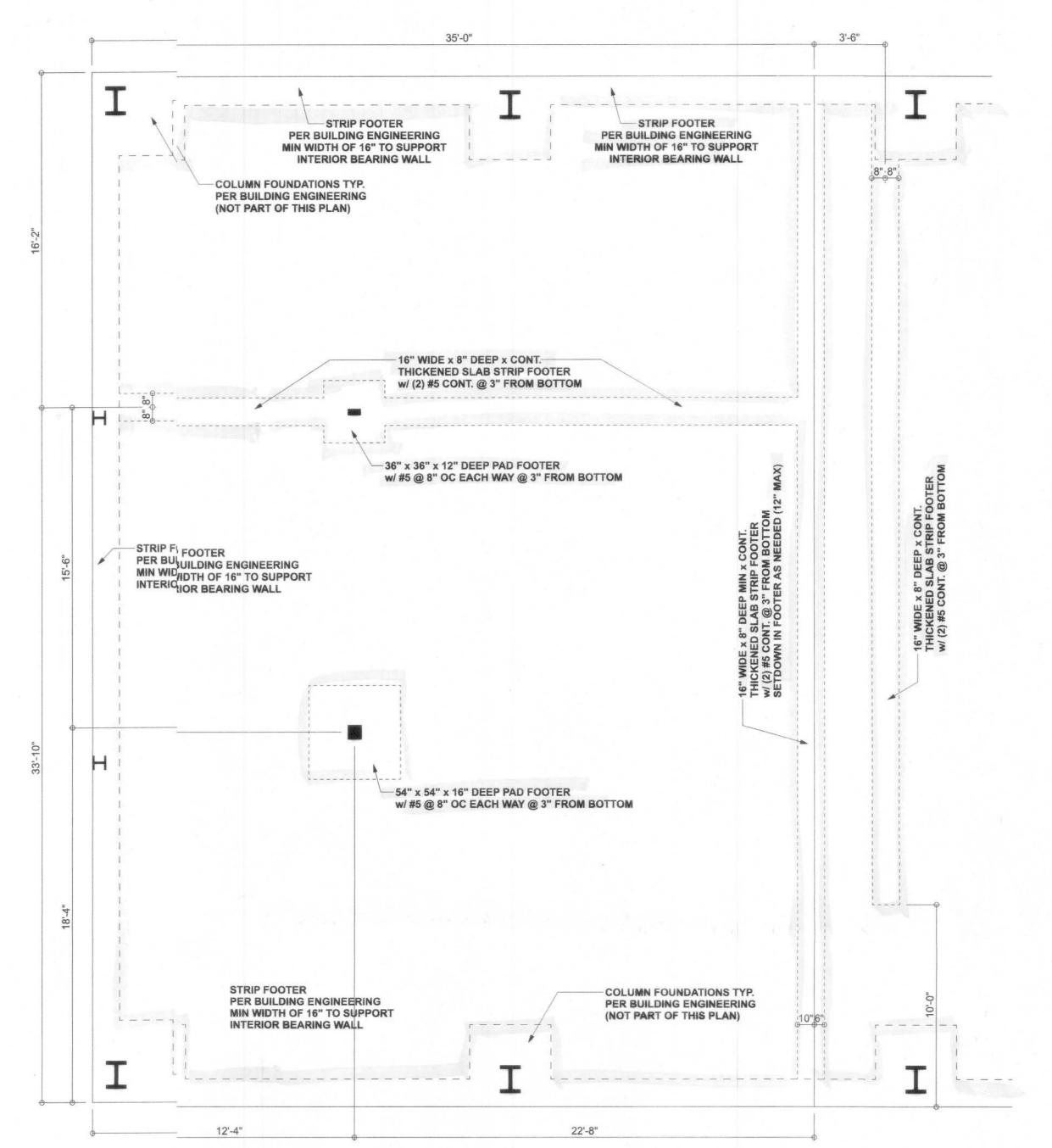
STRUCTURAL LAYOUT

SCALE: 1/4" = 1'-0"

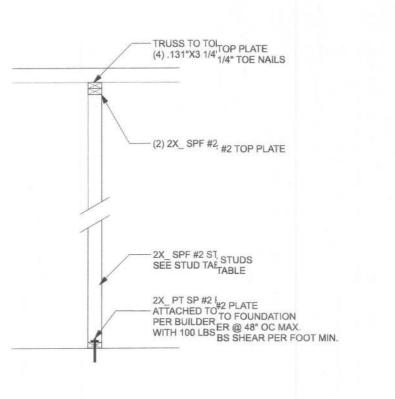
METAL BUILDING SYSTEM

STAIR DESIGN LOAD REQUIRMES: TYPE I: HANDRAILS WITH A CIRCULAR CROSS SECTION SHALL HAVE AN OUTSIDE DIA. OF 1 1/4" - 2". IF HANDRAIL IS NOT CIRCULAR IT SHALL HAVE - 200 LB SINGLE CONCENTRATED LIVE LD APPLIED IN ANY A PERIMETER DIMENSION OF 4" - 6 1/4" AND MAX. CROSS SECTION OF 2 1/4". **DIRECTION AT ANY POINT ALONG THE 1** TYPE II: HANDRAILS WITH A PERIMETER GREATER THAN 6 1/4" SHAL PROVIDE A GRASPABLE FINGER RECESS AREA ON BOTH SIDES OF THE PROFILE. THE FINGER RECESS SHALL BEGIN WITHIN A DISTANCE OF 3/4" **GUARDRAILS IN-FILL COMPONENTS:** - 50 LB LIVE LOAD APPLIED HORIZONTAION AN AREA EQUAL TO 1 FT2 VERTICALLY FROM THE TALLEST PORTION OF THE PROFILE AND A DEPTH (THIS LOAD NEED NOT BE ASSUMED TOT CONCURRENTLY WITH ANY OF AT LEAST 5/8" WITHIN 7/8" BELOW THE WIDEST PORTION OF THE PROFILE. THIS REQUIRED DEPTH SHALL CONTINUE FOR AT LEAST 3/8" TO A LEVEL THAT IS NOT LESS THAN 1 3/4" BELOW THE TALLEST PORTION OF THE - GLAZING USED IN HANDHAIL ASSEMBL AND GUARDS SHALL BE PROFILE. THE WIDTH OF THE HANDRAIL ABOVE THE RECESS SHALL BE DESIGNED WITH A SAFETY FACTOR OF HE SAFETY FACTOR SHALL BE 1 3/4" - 2 3/4". EDGES SHALL HAVE A MIN. RADIUS OF 0.01". APPLIED TO EACH OF THE CONCENTRO LOADS APPLIED TO THE TOP OF THE RAIL, AND TO THE LOAD ON THI-FILL COMPONENTS. THIES LOADS SHALL BE DETERMINED INDEPENT OF ONE ANOTHER, AND LOADS ARE ASSUMED NOT TO OCCUR 'H ANY OTHER LIVE LOAD. A 4 3/8" SPHERE CANNOT PASS THROUGH OPENING IN **GUARDS ON SIDES OF STAIR** - 40 PSF LIVE LOAD, OR 300 LB CONCENTED LOAD OVER AN AREA OF 4 IN2 (WHICHEVER PRODUCES THE GREER STRESSES) 36" MIN. CLEAR WIDTH ABOVE HANDRAIL SPINDLES / RAILS SPACED 31.5" MIN. CLEAR WIDTH HANDRAIL ON ONE SIDES SO THAT A 4" SPHERE -4 1/2" MAX OPENING IN GUARD U.N.O. PROJECTION 4 1/2" MAX. HANDRAIL PROJECTION EVERY TREAD LESS THAT 11" SHALL HAVE A NOSING OR EFFECTIVE PROJECTION OF APPROX. 1" OVER THE LEVEELOW. BETWEEN A NOSING IS NOT REQUIRED WHERE THE TREAD IS A MIN.'. THROUGH THE TRIANGULAR THE RADIUS OF CURVATURE AT THE LEADING EDGE OF TLD AND WALL AREA FORMED BY RISER. SHALL BE NO GRATER THAT 9/16" TREAD AND GUARD RAIL **BEVELING OF NOSING SHALL NOT EXCEED 1/2"** HANDRAILS SHALL BE CONT. ON AT LEAST ONE SIDE OF ALL STAIRS RISERS SHALL BE VERTICAL OR SLOPED AT AN WITH 4 OR MORE RISERS A LANDING SHALL BE PROVIDED AT ANGLE NOT MORE THAN 30 DEGREES FROM VBCAL. OPEN RISERS ARE PERMITTED, PROVIDED THATE PENING BETWEEN TREADS DOES NOT PERMITE SHALL HAVE A MIN WIDTH OF NOT PASSAGE OF A 4" DIA, SPHERE, OPENING BETVN LESS THAT STAIR WIDTH AND HAVE TREADS IS NOT LIMITED IF TOTAL RISE IS 30" OESS OF TRAVEL. A DOOR AT THE TOP OF -7 3/4" MAX. RISER HEIGHT GYP. BOARD STAIR IS PERMITTED, PROVIDED THE GREATEST RISER IN AN UNDER ALL IT DOES NOT SWING OVER STAIR. FLIGHT OF STAIRS SHALL NO EXCEED THE SMALLEST BY MORE THAN 3/8" WINDER TREADS SHALL HAVE A MIN. TREAD DEPTH OF 10" MEASURED AT A 10" MIN. TREAD DEPTH -POINT 12" FROM NARROWEST END. THE GREATEST DEPTH IN ANY WINDER TREADS SHALL HAVE A MIN. DEPTH FLIGHT OF STAIRS SHALL NOT OF NOT LESS THAN 6" AT ANY POINT WITHIN **EXCEED THE SMALLEST** THE CLEAR WIDTH OF THE STAIR BY MORE THAN 3/8" WITHIN A FLIGHT OF STAIRS THE MAX. TREAD SLOPE OF 2 PERCENT **GREATEST DEPTH AT 12" SHALL NOT** SECTION VIEW EXCEED THE SMALLEST BY MORE THAN 3/8" FLOOR OR GRADE (1" VERT UNIT IN 48" HORZ. UNITS)

TYPICAL STAIR AND GUARDRAIL REQUIRMENTS



FOUNDALTION LAYOUT



(TYP.) INTERIOR BEARING V WALL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE
REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE
OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600		*	
415	290	H3	4-8dx1 1/2"	4-8dx1 1/2"	
575	495	H2.5A	5-8dx1 1/2"	5-8dx1 1/2"	
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"	
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"	
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	8-10d	8-10d	
1640	1455	MSTA24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
585	535	SP1	6-10d	4-10d	
1065	605	SP2	6-10d	6-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor	
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD	
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor	
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy	

DE & SPECIES TA	& SPECIES TABLE			
	Fb			
SP #2	925	1		
SP #2	800	1		
SP #2	750	1		
24F-V3 SP	2600	1		
TIMBERSTRAND	1700	1		
MICROLAM	2950	2		
PARALAM	2900	2		
	SP #2 SP #2 SP #2 24F-V3 SP TIMBERSTRAND MICROLAM	SP #2 925 SP #2 800 SP #2 750 24F-V3 SP 2600 TIMBERSTRAND 1700 MICROLAM 2950		

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020) ASCE 7-16	
CODE FOR DESIGN LOADS		
WINDLOADS		
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I .	
RISK CATEGORY	H	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
ROOF ANGLE	7-45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
C&C DESIGN PRESSURES	SEE BUILDING ENGINEERING	
FLOOR LOADING		
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
ROOF LOADING		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY	1500 PSF	
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZO	

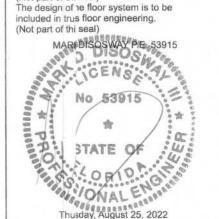
portions of the lans comply with the 7th Edition Floda Building Code lesidential (2020) to the best of ry knowledge.

LIMITATION: his design is valid for one building, at specified loation.
This seal is linted to the desing of the the interior bearingwalls only.
The design of xterior walls and roof is to be included in meal building engineering.

(Not part of thi seal)

CERTIFICATION: I hereby certify that I have

examined this lan, and that the applicable



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