

RE: megan_holloway - Megan Holloway

Site Information:

Customer Info: Lee holloway Project Name: . Model: . Lot/Block: . Subdivision: . Address: ., . City: Alachua County State: FL

MiTek USA, Inc. 16023 Swinglev Ridge Rd Chesterfield, MO 63017

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

> Date 10/26/22 10/26/22 10/26/22 10/26/22 10/26/22 10/26/22

This package includes 28 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date No.	Seal#	Truss Name
1	T29069789	G01	10/26/2223	T29069811	<u>T</u> 13
2	129069790 T20060701	G02 H01	10/26/2224	129069812 T20060812	114 T15
4	T29069797	J01	10/26/2226	T29069813	T16
5	T29069793	J02	10/26/2227	T29069815	T17
6	T29069794	PB01	10/26/2228	T29069816	T18
/ 8	T29069795	PB1A PB02	10/26/22		
9	T29069797	T01	10/26/22		
10	T29069798	T01GE	10/26/22		
11	129069799	102	10/26/22		
13	T29069800	T02GE	10/26/22		
14	T29069802	T04	10/26/22		
15	T29069803	<u>T05</u>	10/26/22		
16	129069804	106	10/26/22		
18	T29069805	T08	10/26/22		
19	T29069807	ŤÕ9	10/26/22		
20	T29069808	T10	10/26/22		
21	T29069809	T12	10/26/22		
22	129009010	112	10/20/22		



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Lee, Julius

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



October 27,2022



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
MSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component
Safety Information
available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017

MiTek

Job	Truss	Truss Type	Qty	Ply	Megan Holloway	
					T29	9069789
MEGAN_HOLLOWAY	G01	COMMON GIRDER	1	2		
				J	Job Reference (optional)	
Mayo Truss Company, Inc.,	Mayo, FL - 32066,		8.5	530 s Aug	11 2022 MiTek Industries, Inc. Wed Oct 26 11:26:46 2022 Pa	age 2

ID:9_WKkVLZ72Nen5bY88LPMkyrWOS-7cqX7cilkt8YAJzH0jfNB06FTx3PIgRjYSGhGlyPZ0t

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-5=-60, 5-7=-60, 12-15=-20

Concentrated Loads (lb)

Vert: 8=-1305(F) 10=-2799(F) 18=-1275(F) 19=-1340(F) 20=-1174(F) 21=-1728(F) 22=-1730(F)



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Job	Truss	Truss Type	Qty	Ply	Megan Holloway
MEGAN HOLLOWAY	G02	COMMON GIRDER	1	_	T29069790
				2	Job Reference (optional)
Moura Truca Company, Inc.	Mayo EL 22066		0.0	20 0 100	11 2022 MiTak Industrian Ing. Wed Oct 26 11/26/16 2022 Dags 1

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.530 s Aug 11 2022 MiTek Industries, Inc. Wed Oct 26 11:26:46 2022 ID:9_WKkVLZ72Nen5bY88LPMkyrWOS-7cqX7cilkt8YAJzH0jfNB06OuxEdllSjYSGhGlyPZ0t 0-10-8 2-10-8

Structural wood sheathing directly applied or 2-10-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals



0-10-8 2-10-8 0-10-8 2-0-0

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 15.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2020/TPI2014	CSI. TC 0.13 BC 0.15 WB 0.05 Matrix-MP	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 5 >999 240 Vert(CT) -0.00 5 >999 180 Horz(CT) -0.00 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 109 lb FT = 20%
LUMBER-	I		BRACING-	

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.2 WFBS

REACTIONS. (size) 6=Mechanical, 4=Mechanical

Max Horz 6=-179(LC 6) Max Uplift 6=-249(LC 4), 4=-182(LC 5)

Max Grav 6=954(LC 1), 4=536(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-6=-559/177, 3-4=-391/203

WEBS 3-5=-228/333. 1-5=-190/523

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 249 lb uplift at joint 6 and 182 lb uplift at joint 4.
- 10) Use MiTek HUS26 (With 14-16d nails into Girder & 6-16d nails into Truss) or equivalent at 0-11-4 from the left end to connect truss(es) to back face of bottom chord.
- 11) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 4-6=-30

Continued on page 2

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a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
MSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component
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Julius Lee PE No. 34869 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

October 27.2022

Scale = 1:38.0



Job	Truss	Truss Type	Qty	Ply	Megan Holloway	
						T29069790
MEGAN_HOLLOWAY	G02	COMMON GIRDER	1	2		
				-	Job Reference (optional)	
Mayo Truss Company, Inc.,	Mayo, FL - 32066,		8.5	530 s Aug	11 2022 MiTek Industries, Inc. Wed Oct 26 11:26:47 2022	Page 2

ID:9_WKkVLZ72Nen5bY88LPMkyrWOS-boNwKyiwVBGPnTYTaRAckEeZeKZs1Chsm6?EpByPZ0s

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 5=-1168(B)



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-2=-844/70, 2-3=-1704/90, 3-4=-1704/90, 4-5=-2027/133, 5-6=-2018/131, 6-7=-824/148, 7-8=-824/148, 8-9=-2111/265, 1-16=-1490/33 BOT CHORD 14-15=-78/741, 13-14=-18/2110, 12-13=-18/2110, 11-12=-91/1455, 10-11=-90/1454

- WEBS 2-15=-883/44, 2-14=-27/1442, 3-14=-428/86, 4-14=-577/23, 4-13=0/280, 5-12=-437/88,
 - 6-12=-97/942, 6-11=0/280, 1-15=0/1282, 6-10=-1304/20, 8-10=-256/2208

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=33ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 9 and 17 lb uplift at joint 16.
- 9) Use MiTek THD26-2 (With 18-16d nails into Girder & 12-10d nails into Truss) or equivalent at 30-3-5 from the left end to connect truss(es) to back face of bottom chord.
- 10) Fill all nail holes where hanger is in contact with lumber.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

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is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see
MISUTPHI Quality Criteria, DSB-89 and BCSI Building Component
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Job	Truss	Truss Type	Qty	Ply	Megan Holloway		
					T29	9069791	
MEGAN_HOLLOWAY	H01	Half Hip Girder	1	1			
					Job Reference (optional)		
Mayo Truss Company, Inc.,	Mayo, FL - 32066,		8.5	530 s Aug	11 2022 MiTek Industries, Inc. Wed Oct 26 11:26:48 2022 Pa	ge 2	
		ID:9_WKkVLZ72Nen5bY88LPMkyrWOS-3?xIYIjYGUOGPd7g78irHRBcHkiMmXx0?mlnLdyPZ0r					

LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-2=-60, 2-8=-60, 9-16=-20 Concentrated Loads (lb) Vert: 10=-934(B)



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LOADING	(psf)	SPACING-	2-0-0	CSI.	0.59	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC		Vert(LL)	-0.09	4-5	>860	240	MT20	244/190
TCDL BCLL BCDL	10.0 0.0 * 10.0	Lumber DOL Rep Stress Incr Code FBC2020/TPI	1.25 YES 2014	BC WB Matri	0.52 0.00 x-AS	Vert(CT) Horz(CT)	-0.21 0.06	4-5 3	>383 n/a	180 n/a	Weight: 25 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. All bearings 7-0-0 except (jt=length) 3=Mechanical, 3=Mechanical.

(lb) - Max Horz 5=124(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 5, 3

Max Grav All reactions 250 lb or less at joint(s) 3, 3, 4 except 5=383(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-321/167

NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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			7-0-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2020/TPI2014	CSI. TC 0.59 BC 0.51 WB 0.00 Matrix-AS	DEFL. in Vert(LL) -0.09 Vert(CT) -0.21 Horz(CT) 0.06	(loc) l/defl L/d 4-5 >890 240 4-5 >396 180 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 25 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. (size) 5=0-4-0, 3=Mechanical, 4=Mechanical

Max Horz 5=124(LC 12)

Max Uplift 5=-16(LC 12), 3=-52(LC 12)

Max Grav 5=382(LC 1), 3=185(LC 1), 4=125(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-322/167

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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						8-5-3 8-5-3						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
FCLL :	20.0	Plate Grip DOL	1.25	TC	0.18	Vert(LL)	0.01	5	n/r	120	MT20	244/190
CDL	10.0	Lumber DOL	1.25	BC	0.12	Vert(CT)	0.01	5	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2020/TI	PI2014	Matri	x-P						Weight: 26 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.2 OTHERS

REACTIONS. (size) 2=6-7-8, 4=6-7-8, 6=6-7-8

Max Horz 2=32(LC 11) Max Uplift 2=-28(LC 12), 4=-28(LC 12)

Max Grav 2=171(LC 1), 4=171(LC 1), 6=258(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) 0-3-15 to 3-3-15, Interior(1) 3-3-15 to 4-2-9, Exterior(2R) 4-2-9 to 7-2-9, Interior(1) 7-2-9 to 8-1-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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2x4 =

1.5x4 ||

8-5-3 8-5-3 LOADING (psf) SPACING-2-0-0 CSL DEFL in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.25 тс 0.11 Vert(LL) 0.00 5 n/r 120 MT20 244/190 TCDL 10.0 Lumber DOL 1.25 вс 0.08 Vert(CT) 0.00 5 n/r 120 WB 0.02 BCLL 0.0 **Rep Stress Incr** YES Horz(CT) 0.00 4 n/a n/a BCDL 10.0 Code FBC2020/TPI2014 Matrix-P Weight: 21 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.2 2x4 SP No.2 OTHERS

REACTIONS. (size) 2=5-3-14, 4=5-3-14, 6=5-3-14 Max Horz 2=-27(LC 10)

Max Uplift 2=-26(LC 12), 4=-26(LC 12)

Max Grav 2=145(LC 1), 4=145(LC 1), 6=205(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer



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2x4 =

2x4 =

Rigid ceiling directly applied or 10-0-0 oc bracing.

8-5-3 8-5-3 LOADING (psf) SPACING-2-0-0 CSL DEFL in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.25 тс 0.13 Vert(LL) 0.00 5 n/r 120 MT20 244/190 TCDL 10.0 Lumber DOL 1.25 вс 0.08 Vert(CT) 0.00 5 n/r 120 WB 0.02 BCLL 0.0 **Rep Stress Incr** YES Horz(CT) 0.00 4 n/a n/a BCDL 10.0 Code FBC2020/TPI2014 Matrix-P Weight: 21 lb FT = 20% LUMBER-BRACING-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.2 OTHERS

REACTIONS. (size) 2=5-3-14, 4=5-3-14, 6=5-3-14 Max Horz 2=-27(LC 10)

Max Uplift 2=-26(LC 12), 4=-26(LC 12)

Max Grav 2=145(LC 1), 4=145(LC 1), 6=205(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer



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Max Horz 2=111(LC 11) Max Uplift 2=-37(LC 12), 8=-37(LC 12) Max Grav 2=1050(LC 1), 8=1050(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-1444/112, 4-5=-1070/149, 5-6=-1070/149, 6-8=-1444/112

BOT CHORD 2-12=-14/1215, 11-12=-14/1215, 10-11=-25/1215, 8-10=-25/1215

WEBS 5-11=-12/521, 6-11=-423/72, 4-11=-423/72

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 12-0-0, Exterior(2R) 12-0-0 to 15-0-0, Interior(1) 15-0-0 to 25-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 12-0-0, Corner(3R) 12-0-0 to 15-0-0, Exterior(2N) 15-0-0 to 25-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 1.5x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Solid blocking is required on both sides of the truss at joint(s), 2.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 24, 25, 26, 27, 28, 22, 21, 20, 19, 18.



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Fiale Olisels (A, I)	[2.0-0-0,Euge], [0.0-0-0,Euge]			
LOADING (psf)	SPACING- 2-0-0 Plate Grip DOI 1 25	CSI. TC 0.64	DEFL. in (loc) I/defl L/d	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.25 Rep Stress Incr YES	BC 0.61 WB 0.36	Vert(CT) -0.20 10-11 >999 180 Horz(CT) 0.06 8 n/a n/a	
BCDL 10.0	Code FBC2020/TPI2014	Matrix-AS		Weight: 124 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 2x4 SP No 2 WFBS SLIDER Left 2x6 SP No.2 1-6-0, Right 2x6 SP No.2 1-6-0

REACTIONS. (size) 8=0-4-0, 2=0-4-0 Max Horz 2=109(LC 11) Max Uplift 2=-38(LC 12) Max Grav 8=957(LC 1), 2=1053(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-1449/116, 4-5=-1075/150, 5-6=-1076/153, 6-8=-1461/124 TOP CHORD

BOT CHORD 2-11=-53/1220, 10-11=-53/1220, 9-10=-49/1233, 8-9=-49/1233

WEBS 5-10=-17/524, 6-10=-439/74, 4-10=-423/72

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 12-0-0, Exterior(2R) 12-0-0 to 15-0-0, Interior(1) 15-0-0 to 24-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code FBC2020/TPI2014	WB 0.04 Matrix-S	Horz(CT) 0.	00 14	n/a	n/a	Weight: 110 lb	FT =
LUMBER- TOP CHORD 2x4 SP 1 BOT CHORD 2x4 SP 2 14-24: 2 OTHERS 2x4 SP 1	No.2 2400F 2.0E *Except* x4 SP No.2 No.2		BRACING- TOP CHORD BOT CHORD	Structur Rigid ce	al wood s iling dire	sheathing dir ctly applied c	rectly applied or 6-0-0 oc or 10-0-0 oc bracing.	purlins.

REACTIONS. All bearings 19-0-0.

(lb) -Max Horz 2=85(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21, 22, 23, 26, 19, 18, 17, 16 Max Grav All reactions 250 lb or less at joint(s) 2, 14, 20, 21, 22, 23, 26, 19, 18, 17, 16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 9-6-0, Corner(3R) 9-6-0 to 12-6-0, Exterior(2N) 12-6-0 to 20-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 1.5x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Solid blocking is required on both sides of the truss at joint(s), 2.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21, 22, 23, 26, 19, 18, 17, 16.



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FT = 20%





- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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		4-10-12	9-6-0			14-1-4				19-0-0	1	
		4-10-12	4-7-4			4-7-4				4-10-12	1	
Plate Offsets	(X,Y)	[2:0-5-8,0-0-2], [8:0-5-8,0-	-0-2], [11:0-2-8	,0-3-0]								
LOADING (p	osf)	SPACING-	2-0-0	CSI.		DEF	L. i	n (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.25	TC	0.44	Vert	LL) -0.04	4 9-10	>999	240	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.25	BC	0.46	Vert	CT) -0.0	9 9-10	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz	(CT) 0.0	8 8	n/a	n/a		
BCDL 10	0.0	Code FBC2020/TF	Matri	x-AS						Weight: 101 lb	FT = 20%	

 LUMBER

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 SLIDER
 Left 2x6 SP No.2 1-6-0, Right 2x6 SP No.2 1-6-0

BRACING-TOP CHORD S BOT CHORD F

Structural wood sheathing directly applied. Rigid ceiling directly applied.

REACTIONS. (size) 8=0-4-0, 2=0-4-0 Max Horz 2=88(LC 11) Max Uplift 2=-38(LC 12) Max Grav 8=756(LC 1), 2=854(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1090/146, 4-5=-841/150, 5-6=-842/155, 6-8=-1095/147

BOT CHORD 2-11=-83/911, 10-11=-83/911, 9-10=-75/921, 8-9=-75/921

WEBS 5-10=-26/417, 6-10=-299/83, 4-10=-284/86

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 9-6-0, Exterior(2R) 9-6-0 to 12-6-0, Interior(1) 12-6-0 to 19-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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MiTek

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Page 2

ID:9_WKkVLZ72Nen5bY88LPMkyrWOS-7tLzhQvyk5H7iwmYWoTMNcJEGnx8nEoDSbt4NGyPZ0c

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-6=-60, 7-12=-20

Vert: 10--00, r 12- 20 Concentrated Loads (lb) Vert: 6--157(F) 11=-55(F) 2=-125(F) 13=-141(F) 14=-125(F) 15=-125(F) 16=-125(F) 17=-125(F) 18=-125(F) 19=-125(F) 21=-125(F) 23=-125(F) 24=-125(F) 25=-125(F) 26=-125(F) 26=-125(F) 26=-125(F) 30=-125(F) 31=-61(F) 32=-55(F) 33=-55(F) 34=-55(F) 35=-55(F) 35=-55



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	6	-7-7	13-1-3			19-6-14			26-0-9		32-8-0	
	6	6-7-7	6-5-11	1		6-5-11	1		6-5-11		6-7-7	1
Plate C	Offsets (X,Y)	[3:0-4-0,0-4-8], [5:	0-4-0,0-4-8], [9:0-4-0,0)-4-8], [11:0-	4-0,0-4-8]							
LOAD TCLL TCDL BCLL BCDL	NG (psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip I Lumber DO Rep Stress Code FBC2	2-0-0 DOL 1.25 L 1.25 Incr YES 2020/TPI2014	CSI. TC BC WB Matrix	0.41 0.37 0.89 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.18 0.05	(loc) 9-10 9-10 7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 249 lb	GRIP 244/190 FT = 20%
LUMB TOP C BOT C WEBS	E R- HORD 2x6 SF HORD 2x6 SF 2x4 SF	9 No.2 9 No.2 9 No.2				BRACING- TOP CHORI BOT CHORI WEBS	0	Structur Rigid ce 1 Row a	ral wood eiling dire at midpt	sheathing dir ctly applied. 5 [.]	ectly applied, except e	end verticals.
REAC	REACTIONS. (size) 12=Mechanical, 7=0-4-0 Max Horz 12=-138(LC 8) Max Grav 12=1295(LC 1), 7=1295(LC 1)											
FORC TOP C BOT C WEBS	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-12=-1223/350, 1-2=-1420/381, 2-3=-1420/381, 3-4=-2104/518, 4-5=-2104/518 BOT CHORD 10-11=-542/2064, 9-10=-542/2064, 8-9=-349/1380, 7-8=-349/1380 WEBS 1-11=-448/1756, 2-11=-461/242, 3-11=-808/163, 3-10=0/315, 4-9=-471/239, 5-9=-273/909, 5-8=0/307, 5-7=-1702/386											
NOTE: 1) Win II; E	5- d: ASCE 7-16; V xp B; Encl., GCp	/ult=130mph (3-sec pi=0.18; MWFRS (c	cond gust) Vasd=101n lirectional) and C-C C	nph; TCDL=6 orner(3) 0-1-	6.0psf; BCD 12 to 3-1-1	DL=6.0psf; h=15ft; B= 2, Exterior(2) 3-1-12	=45ft; L to 29-	.=33ft; e 6-4, Coi	ave=4ft; ner(3) 29	Cat. 9-6-4		

to 32-6-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type		Qty	Ply	Megan Holloway		T29069805
MEGAN_HOLLOWAY	Т07	PIGGYBACK BASE		1	1	Job Reference (ontion	al)	
Mayo Truss Company, Inc.,	Mayo, FL - 32066,		וריס אאגעי	8.	530 s Aug	11 2022 MiTek Industrie	es, Inc. Wed Oct 26 11:2	27:07 2022 Page 1
	4-0-1 6-	3-5 <u>12-3-3 12-</u> 3-5 <u>5-11-13</u> 07	11-0 16-5-12	20-0)-8 20-8	5 24-10-11	<u>29-1-0</u> <u>30-1-13</u> <u>4-2-5</u> <u>1-0-13</u>	CpNDIW IV 201
	401 2			5-0-	12 0-7-		+2-5 1-0-13	Scolo - 1:67 8
		42	$x^4 = Bra$	cing	4x4	TOP CHORD MUS	ST BE BRACED BY END M, OR PROPERLY CON	JACKS, NECTED
I		11		× ×		²³ PURLINS AS SPE	CIFIED.	Ţ
	Bi	acing, 469				²⁵ 47 ¹⁶ 27		
	6.00 12 6x6 =	5					5x5 ≈ ^{2x4}	
Ţ	2		13 2x4 16	- 18	2022		Brac	ing
0-(3x4 = 42 3	$2x4 \parallel 10x10 = 2x4 \parallel$	2×4		1091	$A \equiv 2x4 \parallel 10x10$	=2x4 30 10	9
11-0			Bra	cing				11-0
2-01								9-11
								6
			t E		Ø			
	41 40	39 3	38	37	3	6 35	34 33 4×4 —	
	3x4 =	3x8 = 7x	(8 = 7	x8 =	2x4	4 3x8 =	2x4	
	4-0-1 6·	3-5 12-3-3	16-5-12	, 20	-8-5	29-1-0	30-1-13	
Plate Offsets (X,Y) [2:0	<u>4-0-1</u> 2· 0-3-0.0-2-7], [10:0-5-0.0-2-8],	<u>3-5</u> 5-11-13 [20:0-3-4,0-2-8], [31:0-2-12,0-3-	4-2-9 ·0], [37:0-4-0,0-4·	4- -8], [38:0	-2-9 -4-0,0-4-8	8-4-11	1-0-13	
LOADING (psf)	SPACING- 2-0-	0 CSI .	DEFL.	i	n (loc)	l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.2	5 TC 0.26	Vert(LL)	-0.03	3 38	>999 240	MT20	244/190
BCLL 0.0 *	Rep Stress Incr YE	S WB 0.72	Horz(CT) 0.02	2 33	n/a n/a	Mainht 200 lb	FT 200/
		Matrix-AS					weight: 388 lb	F1 = 20%
TOP CHORD 2x4 SP No	.2 *Except*		TOP CHO	g- Drd	Structura	al wood sheathing dire	ctly applied, except er	nd verticals, and
31-32,2-20 BOT CHORD 2x6 SP No),20-31: 2x6 SP No.2 .2		BOT CHO	ORD	2-0-0 oc Rigid ce	purlins (6-0-0 max.): iling directly applied.	12-21, 2-32.	
WEBS 2x4 SP No	.2		JOINTS		1 Brace	at Jt(s): 32, 13, 20, 24	, 26, 8, 6, 16	
REACTIONS. (size)	33=Mechanical, 41=Mechar 41-256(I C 11)	nical			OF THE 1	ESIGNED FOR WIND LA RUSS ONLY. FOR STU	DADS IN THE PLANE DS EXPOSED TO WIND	
Max Uplift	33=-60(LC 12), 41=-47(LC	2)			GABLE E	ND DETAILS AS APPLIC	CABLE, OR CONSULT	
	33=1194(LC 1), 41=1194(LC							
TOP CHORD 1-2=-740	np./Max. Ten All forces 25 //109, 12-14=-528/137, 14-17	0 (lb) or less except when showr ′=-528/137, 17-19=-528/137, 19∙	n. -21=-528/137,					
2-3=-945 10-13=-5	/136, 3-4=-484/145, 4-6=-48 11/153, 13-15=-506/152, 15-	4/145, 6-8=-484/145, 8-10=-484 16=-506/152, 16-18=-506/152,	/145, 18-20=-506/152,					
20-22=-5 30-31=-7	09/152, 22-24=-257/153, 24 21/151, 1-41=-1158/128, 3-5	26=-257/153, 26-28=-257/153, 2 =-612/35, 5-7=-596/65, 7-9=-61	28-30=-257/153, 9/102.					
9-11=-61 27-295	4/133, 11-12=-566/146, 21-2	3=-546/133, 23-25=-617/125, 2	5-27=-623/94,					-
BOT CHORD 39-40=-2	14/711, 38-39=-216/1043, 3	7-38=-216/1043, 36-37=-149/882	2, 35-36=-149/88	2			IN ILUS	
22-37=-6	5/135, 1-40=-94/966, 26-35= 57/373, 22-35=-349/30, 31-35	i=-124/934, 31-33=-1072/115	=-89/604,			12	IT CEN	E THE
NOTES-								
 Unbalanced roof live loa Wind: ASCE 7-16; Vult= 	ads have been considered for 130mph (3-second gust) Vas	[·] this design. sd=101mph; TCDL=6.0psf; BCD	L=6.0psf; h=15ft	; B=45ft;	L=30ft; ea	ave=4ft; Cat.	*: 0 348	
II; Exp B; Encl., GCpi=0 12-11-0 to 15-11-3 Inte	.18; MWFRS (directional) an rior(1) 15-11-3 to 30-0-1 Ext	d C-C Exterior(2E) 0-1-12 to 3-1 erior(2R) 20-0-8 to 23-0-11 Inte	-15, Interior(1) 3	-1-15 to	12-11-0, E	xterior(2R)		
and right exposed ; end	vertical left and right expose	d;C-C for members and forces 8	MWFRS for rea	ctions sh	nown; Lum	nber	D. Acture	OF WE
 Building Designer / Project 	ect engineer responsible for	verifying applied roof live load sh	nown covers rain	loading	requireme	nts specific	The second	DAIR
4) Provide adequate draina	age to prevent water ponding							ENGIN
6) This truss has been des	igned for a 10.0 psf bottom o	hord live load nonconcurrent wit	th any other live I	oads.			THILIN .	min.
7) * This truss has been de will fit between the botto	esigned for a live load of 20.0 om chord and any other mem	pst on the bottom chord in all an bers.	eas where a rect	angle 3-6	5-0 tall by	2-0-0 wide Juli Mi1	us Lee PE No. 34869 Fek Inc. DBA MiTek USA	FL Cert 6634
 8) Refer to girder(s) for true 9) Provide mechanical con 	ss to truss connections. nection (by others) of truss to	bearing plate capable of withst	anding 100 lb up	lift at joir	nt(s) 33, 4 [.]	160 1. Dat	23 Swingley Ridge Rd. Ch e:	esterfield, MO 63017
10) This truss design requi sheetrock be applied d	ires that a minimum of 7/16" lirectly to the bottom chord.	structural wood sheathing be ap	plied directly to the	ne top ch	ord and 1	/2" gypsum		October 27,2022
11) Graphical purlin repres	sentation does not depict the	size or the orientation of the pur	lin along the top	and/or b	ottom cho	rd.		
WARNING - Verify design Design valid for use only with	parameters and READ NOTES ON TH MITek® connectors. This design is	HIS AND INCLUDED MITEK REFERENCE based only upon parameters shown, and	E PAGE MII-7473 rev. I is for an individual be	5/19/2020 I uilding com	BEFORE USI ponent, not	Ε.		
building design. Bracing indi is always required for stability	cated is to prevent buckling of individ y and to prevent collapse with possib	lual truss web and/or chord members onl le personal injury and property damage.	ly. Additional tempor For general guidance	ary and per	manent brac	ing	MiTek	
fabrication, storage, delivery, Safety Information available	erection and bracing of trusses and le from Truss Plate Institute, 2670 Cr	truss systems, see ANSI/TPI1 ain Highway, Suite 203 Waldorf, MD 206	Quality Criteria, DSI	3-89 and B	CSI Buildin	g Component	16023 Swingley R Chesterfield, MO	idge Rd 63017



LOAD CASE(S) Standard

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses safe and truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information**available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Job	Truss	Truss Type	Qty	Ply	Megan Holloway			
					T2	9069806		
MEGAN_HOLLOWAY	T08	PIGGYBACK BASE GIRDE	1	1				
					Job Reference (optional)			
Mayo Truss Company, Inc.,	Mayo, FL - 32066,		8.5	530 s Aug	11 2022 MiTek Industries, Inc. Wed Oct 26 11:27:09 2022 Pa	age 2		
		ID:9_WKkVLZ72Nen5bY88LPMkyrWOS-y1iEyTzjKx2HQrEis2amdtYDjCzqBvq6qXKOavyPZ0W						

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (pf) Vert: 1-3=-60, 3-5=-60, 5-8=-60, 8-9=-60, 10-18=-20

Concentrated Loads (lb) Vert: 12=-426(F)



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1	6-3-5	12-3-3	16-5-12	20-8-5	26-11-7	33-2-8	38-5-8	44-0-0	1
	6-3-5	5-11-13	4-2-9	4-2-9	6-3-1	6-3-1	5-3-0	5-6-8	
Plate Offsets (X,Y)	[3:0-5-4,0-2-8	8], [5:0-5-4,0-2-8], [7:0)-5-4,0-3-0], [10:E	Edge,0-1-8], [12	2:0-2-8,0-3-0], [14:0-2-8,0	0-3-0], [16:0-2-8,0-3-0]			

LOADING (ps TCLL 20. TCDL 10. BCLL 0. BCDL 10.	sf) .0 .0 .0 * .0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/TF	2-0-0 1.25 1.25 YES Pl2014	CSI. TC 0.71 BC 0.85 WB 0.80 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.26 13-14 -0.46 13-14 0.13 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 331 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP 2x4 SP 12-14: 2 2x4 SP	No.2 No.2 *Except* 2x4 SP No.1 No.2			BRACING- TOP CHOR BOT CHOR WEBS	D Structur 2-0-0 oc D Rigid ce 1 Row a	ral wood s c purlins (eiling dire at midpt	sheathing dir (3-9-14 max. ctly applied. 3	rectly applied, except (): 3-5, 7-9. 3-16, 4-15, 5-15, 6-14, 7	end verticals, and 7-13, 7-11
REACTIONS.	(size Max Ho Max Gi	e) 10=0-4-0, 18=Mechar Drz 18=-243(LC 10) rav 10=1958(LC 18), 18=	nical 1998(LC 17)							

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-1674/139, 2-3=-1927/204, 3-4=-1938/215, 4-5=-1938/215, 5-6=-2391/214,

- 6-7=-3213/171, 7-8=-2178/105, 8-9=-2178/105, 9-10=-1872/105, 1-18=-1910/130
- BOT CHORD 16-17=-152/1539, 15-16=-120/1725, 14-15=-116/2062, 13-14=-150/2780,
- 12-13=-179/3573, 11-12=-177/3578

 WEBS
 2-17=-775/138, 2-16=0/436, 3-16=-251/40, 3-15=-40/829, 4-15=-257/66, 5-15=-381/38, 5-14=-25/1116, 6-14=-1228/93, 6-13=0/907, 7-13=-1024/65, 7-11=-1931/88, 8-11=-347/87, 9-11=-101/2739, 1-17=-78/1744

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=44ft; eave=5ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 12-3-3, Exterior(2R) 12-3-3 to 16-5-12, Interior(1) 16-5-12 to 20-8-5, Exterior(2R) 20-8-5 to 25-1-2, Interior(1) 25-1-2 to 43-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Refer to girder(s) for truss to truss connections.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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1	6-3-5 I	12-3-3	16-5-12 20-8-5		27-0-10	33-5-0	38-6-12	44-0-0	1
Γ	6-3-5	5-11-13	4-2-9	4-2-9	6-4-5	6-4-5	5-1-12	5-5-4	
Plate Offsets (X,Y)	[3:0-5-4,0-2-8], [5:0-5-4,0-2-8], [10:	Edge,0-1-8], [11:	:0-3-8,0-2-0], [12	2:0-2-8,0-3-0], [14:0-2-8,	,0-3-0], [16:0-2-8,0-3-0]			

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2020/TPI2014	CSI. TC 0.71 BC 0.74 WB 0.80 Matrix-AS	DEFL. Vert(LL) -0.3 Vert(CT) -0.4 Horz(CT) 0.7	in (loc) l/defl L/d 27 13-14 >999 240 48 13-14 >999 180 12 10 n/a n/a	PLATES C MT20 2 Weight: 323 lb	FT = 20%
LUMBER- TOP CHORD 2x4 Sf BOT CHORD 2x4 Sf 12-14: WEBS 2x4 Sf	P No.2 P No.2 *Except* 2x4 SP No.1 P No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing di 2-0-0 oc purlins (4-1-15 max Rigid ceiling directly applied. 1 Row at midpt	rectly applied, except end .): 3-5. 3-16, 4-15, 5-15, 6-14, 7-13	verticals, and
REACTIONS. (siz	e) 18=Mechanical, 10=0-4-0					

Max Horz 18=-242(LC 10) Max Grav 18=1992(LC 17), 10=1975(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-2=-1669/243. 2-3=-1920/339. 3-4=-1937/358. 4-5=-1937/358. 5-6=-2392/375.
- 6-7=-3231/367, 7-8=-3812/379, 8-9=-3207/288, 1-18=-1904/235, 9-10=-1878/199
- BOT CHORD 16-17=-117/1533, 15-16=-93/1718, 14-15=-116/2050, 13-14=-212/2792, 12-13=-327/3650,
- 11-12=-273/3049

 WEBS
 2-17=-774/197, 2-16=-11/436, 3-16=-251/54, 3-15=-92/828, 4-15=-256/91, 5-15=-383/63, 5-14=-91/1112, 6-14=-1233/181, 6-13=0/908, 7-13=-1039/139, 8-12=-58/692, 8-11=-782/157, 1-17=-174/1743, 9-11=-261/3105

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=44ft; eave=5ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 12-3-3, Exterior(2R) 12-3-3 to 18-5-13, Interior(1) 18-5-13 to 20-8-5, Exterior(2R) 20-8-5 to 27-0-10, Interior(1) 27-0-10 to 43-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Refer to girder(s) for truss to truss connections.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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4	4-2-12	8-2-0	14-2-9	20-3-3	24-5-12	28-8-5	35-0-10	41-5-0	46-6-12	52-0-0	
4	4-2-12 ′	3-11-4	6-0-9	6-0-9	4-2-9	4-2-9	6-4-5	6-4-5	5-1-12	5-5-4	1
Plate Offsets (X,Y)) [1:0-3	3-8.0-0-2]. [4	4:0-4-0.0-4-8], [6:0-	5-4.0-3-0]. [8:0-5-4	.0-3-0]. [13:E	dae.0-1-8].	[14:0-3-8.0-2-0]. [1	5:0-2-8.0-3-0]. [17:0-2-	8.0-3-0]. [20:0-2	-4.0-3-01	

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2020/TPI2014	CSI. TC 0.29 BC 0.72 WB 0.84 Matrix-AS	DEFL. i Vert(LL) -0.2 Vert(CT) -0.4 Horz(CT) 0.1	in (loc) l/defl L/d 3 16-17 >999 240 1 15-16 >999 180 0 13 n/a n/a	PLATES GRIP MT20 244/190 Weight: 404 lb FT = 20%					
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x4 SF 15-17: WEBS 2x4 SF SLIDER Left 2x	9 No.2 9 No.2 *Except* 2x4 SP No.1 9 No.2 4 SP No.2 1-6-0		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing di 2-0-0 oc purlins (5-7-5 max.) Rigid ceiling directly applied. 1 Row at midpt	rectly applied, except end verticals, and : 6-8. 5-20, 6-19, 7-18, 8-18, 9-17, 10-16					
REACTIONS. (siz Max H Max L Max C	e) 1=0-3-8, 21=0-4-0, 13=0-4-0 lorz 1=198(LC 11) plift 1=-232(LC 24) rav 1=26(LC 21), 21=2999(LC 17), 13='	1895(LC 18)								
FORCES. (lb) - Max. TOP CHORD 1-3= 7-8= 12-1: BOT CHORD 1-22	Comp./Max. Ten All forces 250 (lb) or -71/614, 3-4=-108/1021, 4-5=-1090/223, -1727/364, 8-9=-2203/388, 9-10=-3053/3 3=-1798/196 -540/64, 21-22=-540/64, 20-21=-897/15	less except when shown. 5-6=-1667/332, 6-7=-1727 92, 10-11=-3666/407, 11-1 0. 19-20=-82/1020, 18-19=	/364, 2=-3128/313, :-66/1445.							
17-1 WEBS 3-21 6-19 9-16	BOT CHORD 1-22=540/64, 21-22=-540/64, 20-21=-897/150, 19-20=-82/1020, 18-19=-66/1445, 17-18=-100/1851, 16-17=-198/2634, 15-16=-316/3514, 14-15=-269/2970 WEBS 3-21=-559/107, 4-21=-2476/324, 4-20=-227/2238, 5-20=-1173/200, 5-19=0/767, 6-19=-397/67, 6-18=-110/861, 8-18=-499/65, 8-17=-95/1122, 9-17=-1241/191, 9-16=0/911, 10-16=-1059/143, 11-15=-59/625, 11-14=-701/153, 12-14=-263/2983									
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-16; \ II; Exp B; Encl., GC to 27-7-7, Interior(1) exposed; end vertic grip DOL=1.60 3) Building Designer / to the use of this tru 4) Provide adequate d 5) This truss has been 6) * This truss has been will fit between the fit	e loads have been considered for this der /ult=130mph (3-second gust) Vasd=101n pi=0.18; MWFRS (directional) and C-C E 27-7-7 to 28-8-5, Exterior(2R) 28-8-5 to al left and right exposed;C-C for membe Project engineer responsible for verifying ss component. rainage to prevent water ponding. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on the pottom chord and any other members, with	sign. hph; TCDL=6.0psf; BCDL= xterior(2E) 0-0-0 to 5-2-6, 1 36-0-9, Interior(1) 36-0-9 t rs and forces & MWFRS for applied roof live load show a load nonconcurrent with a he bottom chord in all area th BCDL = 10.0psf.	6.0psf; h=15ft; B=45ft; nterior(1) 5-2-6 to 20-3 o 51-10-4 zone; cantile r reactions shown; Lui wn covers rain loading any other live loads. s where a rectangle 3-	; L=52ft; eave=6ft; Cat. 3-3, Exterior(2R) 20-3-3 ever left and right mber DOL=1.60 plate requirements specific -6-0 tall by 2-0-0 wide	PROVIDENCIAL ORIDACIAL					

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=232.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

October 27,2022



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Julius Lee PE No. 34869

Date:



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MiTek



Scale = 1:103.2



L	4-2-1	2 8-2-0	14-2-9	20-3-3	24-5-12	28-8-5	L 35-0-10	41-5-0	45-7-8	49-10-0	53-8-10	58-0-0	
Г	4-2-1	2 3-11-4	6-0-9	6-0-9	4-2-9	4-2-9	6-4-5	6-4-5	4-2-8	4-2-8	3-10-10	4-3-6	
Plate Offsets (X	(,Y)	[2:0-3-4,0-0-2	2], [5:0-4-0,0-4-8],	[7:0-5-4,0-3-0], [9	9:0-5-4,0-3-0), [12:0-4-0,	0-4-8], [15:0-3-4,E	Edge], [18:0-1-12,0)-3-0], [20:0	-2-8,0-3-0],	[21:0-2-8,0-	3-0],	
		[24:0-2-8.0-3	-01										

	[= =	0,000										
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	*	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/TP	2-0-0 1.25 1.25 YES I2014	CSI. TC BC WB Matrix	0.35 0.97 0.95 (-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.24 -0.44 0.07	(loc) 20-21 20-21 17	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 437 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2 SLIDER L	2x6 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 _eft 2x4 SP N	o.2 1-6-0				BRACING- TOP CHORI BOT CHORI WEBS	D D	Structur 2-0-0 oo Rigid ce 1 Row a	ral wood s c purlins (eiling direo at midpt	sheathing dire 5-11-4 max.): ctly applied. 7-2	ctly applied, except 7-9. 23, 8-22, 9-22, 10-21	
REACTIONS.	(size) 2=	0-3-8, 25=0-4-0, 17=	=0-4-0, 15=0-3-8									

Max Horz 2=-192(LC 10) Max Uplift 2=-52(LC 22), 15=-73(LC 23)

Max Grav 2=210(LC 21), 25=2657(LC 17), 17=2693(LC 18)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD	3-4=-1/344, 4-5=-29/723, 5-6=-1108/215, 6-7=-1548/308, 7-34=-1545/331,
	8-34=-1545/331, 8-35=-1545/331, 35-36=-1545/331, 9-36=-1545/331, 9-37=-1899/341,
	10-37=-1912/306, 10-11=-2423/324, 11-12=-2393/263, 12-13=-1249/150, 13-38=-94/1488,
	14-38=-104/1446, 14-15=-65/775
BOT CHORD	2-26=-293/55, 25-26=-293/55, 24-25=-598/124, 24-39=-24/1037, 23-39=-24/1037,
	23-40=0/1350, 22-40=0/1350, 22-41=-10/1601, 21-41=-10/1601, 21-42=-109/2084,
	20-42=-109/2084, 19-20=-147/2317, 18-19=-72/1262, 17-18=-1421/142, 16-17=-714/77,
	15-16=-714/77
WEBS	4-25=-491/89, 5-25=-2168/254, 5-24=-157/1922, 6-24=-978/157, 6-23=0/587,
	7 22 272/50 7 22 70/040 0 22 205/40 0 24 57/704 40 24 705/455

 XEBS
 4-25-49 (1/8), 5-25=2160/234, 5-24=-157/1922, 6-24=-976/157, 6-25=0/567, 7-23=-273/50, 7-22=-78/649, 9-22=-285/40, 9-21=-57/794, 10-21=-765/155, 10-20=0/397, 11-20=-273/52, 11-19=-608/140, 12-19=-91/1333, 12-18=-1283/141, 13-18=-229/2930, 13-17=-2303/254, 14-17=-739/84

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=58ft; eave=7ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 4-2-12, Interior(1) 4-2-12 to 20-3-3, Exterior(2R) 20-3-3 to 26-0-12, Interior(1) 26-0-12 to 28-8-5, Exterior(2R) 28-8-5 to 34-5-15, Interior(1) 34-5-15 to 58-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 2 and 73 lb uplift at joint 15.

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
MSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component
Safety Information
available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Julius Lee PE No. 34869 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

October 27,2022



Job	Truss	Truss Type	Qty	Ply	Megan Holloway
MEGAN HOLLOWAY	T13	PIGGYBACK BASE	1	1	129069811
_					Job Reference (optional)
Mayo Truss, Mayo, Fl				8	.530 s May 26 2022 MiTek Industries, Inc. Thu Oct 27 09:12:53 2022 Page 2
ID:9_WKkVLZ72Nen5bY88LPMkyrWOS-vOwyeXzctsFqoeMQ9GbKYX67TzTCUSM					

NOTES-

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

October 27,2022





Scale = 1:103.4

16023 Swingley Ridge Rd Chesterfield, MO 63017



5-0)-5 9-9-1 15-0-2 20-)-5 4-8-13 5-3-1 5-	$\frac{3-3}{3-1}$ + $\frac{24-5-12}{4-2-9}$ + $\frac{28-3}{4-2}$	8-5 <u>35-0-10</u> 2-9 6-4-5	41-5-0	<u>45-7-8</u> <u>49-10-0</u> <u>4-2-8</u> <u>4-2-8</u>	<u> 53-8-10 58-0-0</u> 3-10-10 4-3-6		
Plate Offsets (X,Y)	[2:0-3-4,0-0-2], [5:0-4-0,0-4-8], [7:0-5-4,0 [25:0-2-8,0-3-0]	-3-0], [9:0-5-4,0-3-0], [12:	0-4-0,0-4-8], [15:0-3-4	,Edge], [19:0-1-12,0-3	3-0], [21:0-2-8,0-3-0], [[22:0-2-8,0-3-0],		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2020/TPI2014	CSI. TC 0.34 BC 0.95 WB 0.99 Matrix-AS	DEFL. Vert(LL) -0.2 Vert(CT) -0.4 Horz(CT) 0.0	in (loc) l/defl 23 21-22 >999 2 13 21-22 >999 13 21-22 >999 16 18 n/a	L/d PLA 240 MT2 180 n/a Wei	ATES GRIP 20 244/190 ight: 444 lb FT = 20%		
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x4 SF WEBS 2x4 SF SLIDER Left 2x	P No.2 P No.2 P No.2 P No.2 K4 SP No.2 1-6-0		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood she 2-0-0 oc purlins (6- Rigid ceiling directly 1 Row at midpt	eathing directly applie 0-0 max.): 7-9. y applied. 6-25, 7-24, 8	¹ d, except 3-23, 9-23, 10-22		
REACTIONS. All b (lb) - Max H Max U Max G	earings 0-3-8 except (jt=length) 26=0-5-1 forz 2=-196(LC 10) Jplift All uplift 100 lb or less at joint(s) 2, Srav All reactions 250 lb or less at joint(s	∣, 18=0-4-0. 15) 15 except 2=311(LC 21), 26=2600(LC 17), 18	=2591(LC 18)				
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 4-5=0/633, 5-6=-845/206, 6-7=-1321/295, 7-8=-1384/318, 8-9=-1384/318, 9-10=-1764/330, 10-11=-2284/258, 12-13=-1205/159, 13-14=-88/1411, 14-15=-58/733 BOT CHORD 25-26=-523/131, 24-25=0/793, 23-24=0/1148, 22-23=0/1465, 21-22=-92/1959, 20-21=-134/2210, 19-20=-65/1215, 18-19=-1345/134, 17-18=-677/83, 15-17=-677/83 WEBS 4-26=-575/96, 5-26=-2090/236, 5-25=-130/1734, 6-25=-1085/147, 6-24=0/709, 7-24=-388/52, 7-23=-84/744, 9-23=-362/47, 9-22=-60/800, 10-22=-775/157, 10-21=0/409, 11-21=-295/61, 11-20=-560/135, 12-20=-83/1258, 12-19=-1220/134, 13-19=-214/2707, 13-18=-2013/2014, 14.18=-609/56								
 NOTES- 1) Unbalanced roof liv. 2) Wind: ASCE 7-16; \ II; Exp B; Encl., GC, 20-3-3 to 26-0-12, Ir right exposed ; end plate grip DOL=1.60 3) Building Designer / to the use of this tru. 4) Provide adequate d 5) This truss has been 6) * This truss has been the trust russ has been the trust russ has been the trust russ design re sheetrock be applie 9) Graphical purlin rep 	e loads have been considered for this des /ult=130mph (3-second gust) Vasd=101m pi=0.18; MWFRS (directional) and C-C Ex nterior(1) 26-0-12 to 28-8-5, Exterior(2R) 2 vertical left and right exposed;C-C for me o Project engineer responsible for verifying iss component. Irainage to prevent water ponding. I designed for a 10.0 psf bottom chord live en designed for a 10.0 psf bottom chord live optom chord and any other members, with connection (by others) of truss to bearing equires that a minimum of 7/16" structural d directly to the bottom chord. resentation does not depict the size or the	ign. ph; TCDL=6.0psf; BCDL: terior(2E) -1-6-0 to 4-3-10 28-8-5 to 34-5-15, Interior mbers and forces & MWF applied roof live load sho load nonconcurrent with e bottom chord in all area n BCDL = 10.0psf. plate capable of withstar wood sheathing be applie	=6.0psf; h=15ft; B=45ft 0, Interior(1) 4-3-10 to (1) 34-5-15 to 59-6-0 z RS for reactions show wn covers rain loading any other live loads. as where a rectangle 3 hding 100 lb uplift at jo rd directly to the top ch along the top and/or bo	i; L=58ft; eave=7ft; Ca 20-3-3, Exterior(2R) zone; cantilever left an 'n; Lumber DOL=1.60 requirements specific -6-0 tall by 2-0-0 wide int(s) 2, 15. lord and 1/2" gypsum ottom chord.	at. Ind Julius Lee PE N MiTek Inc. DB/ 16023 Swingley Date:	No. 34869 ORIDAGO ONALENGINA No. 34869 A MITek USA FL Cert 6634 Ridge Rd. Chesterfield, MO 63017 October 27,2022		
WARNING - Verify de Design valid for use onl a truss system. Before building design. Bracin	esign parameters and READ NOTES ON THIS AND IN by with MiTek® connectors. This design is based only use, the building designer must verify the applicability g indicated is to prevent buckling of individual truss w builting and to prevent buckling of individual truss w	CLUDED MITEK REFERENCE P upon parameters shown, and is of design parameters and prop- eb and/or chord members only.	AGE MII-7473 rev. 5/19/2020 for an individual building co erly incorporate this design in Additional temporary and p	BEFORE USE. mponent, not nto the overall ermanent bracing we the				

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





$\begin{array}{c c c c c c c c c c c c c c c c c c c $	<u>-9-1</u> <u>20-3-3</u> <u>24-5-12</u> <u>28-8-5</u> <u>5-14</u> <u>3-6-1</u> <u>4-2-9</u> <u>4-2-9</u> , [7:0-5-4,0-3-0], [9:0-5-4,0-3-0], [12:0-4	-0,0-4-8], [15:0-3-4,Edge], [19:0-2-4	<u>45-7-8</u> 4-2-8 3,0-3-0], [21:0-2-8,	49-10-0 53-8-10 58-0-0 4-2-8 3-10-10 4-3-6 (0-3-0], [26:0-2-8,0-3-0]					
LOADING (psf) SPACING- 2- TCLL 20.0 Plate Grip DOL 1 TCDL 10.0 Lumber DOL 1 BCLL 0.0 * Rep Stress Incr N BCDL 10.0 Code FBC2020/TPI20 10	0-0 CSI. 25 TC 0.29 25 BC 0.67 'ES WB 0.77 14 Matrix-AS	DEFL. in (loc) l/de Vert(LL) -0.16 21-22 >99 Vert(CT) -0.29 21-22 >99 Horz(CT) 0.04 18 n/	fl L/d 9 240 9 180 a n/a	PLATES GRIP MT20 244/190 Weight: 449 lb FT = 20%					
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2 SLIDER Left 2x4 SP No.2 1-6-0		BRACING- TOP CHORD Structural wo 2-0-0 oc purli BOT CHORD Rigid ceiling o WEBS 1 Row at mid	od sheathing direct ns (6-0-0 max.): 7- lirectly applied. ot 6-25	tly applied, except 9. 5, 7-24, 8-23, 9-23, 10-22					
(Ib) - Max Horz 2=-196(LC 10) Max Uplift All uplift 100 lb or less a Max Grav All reactions 250 lb or le	25=0-5-11, 18=0-4-0. joint(s) 2, 15 ss at joint(s) 15 except 2=619(LC 21), 2	5=2541(LC 17), 18=2166(LC 18)							
FORCES. (lb) - Max. Comp./Max. Ten All forces 3 TOP CHORD 2-3=-303/0, 3-35=-675/74, 4-35=-5 7-37=-582/249, 8-37=-582/249, 8-3 10-39=-1034/223, 10-11=-1576/25	250 (lb) or less except when shown. 73/90, 5-36=0/470, 6-36=0/546, 6-7=-26 8=-582/249, 9-38=-582/249, 9-39=-102 2, 11-12=-1703/207, 12-13=-914/133, 1	52/215, 2/258, 3-14=-62/1155,							
BOT CHORD 2-27=-2/643, 26-27=-2/643, 24-25= 22-42=-35/1325, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-42=-35/1425, 21-425, 20	14-15=-35/497 HORD 2-27=-2/643, 26-27=-2/643, 24-25=-408/163, 23-41=0/829, 22-41=0/829, 22-42=-35/1325, 21-42=-35/1325, 20-21=-83/1639, 19-20=-39/913, 18-19=-1096/109,								
WEBS 4-26=-606/95, 5-26=0/474, 5-25=-6 7-24=-1102/97, 7-23=-125/1150, 8 10-22=-793/168, 10-21=0/429, 11-2 12-19=-938/108, 13-19=-159/2197,	73/102, 6-25=-1832/217, 6-24=-56/135 23=-275/97, 9-23=-800/61, 9-22=-42/88 21=-371/69, 11-20=-357/115, 12-20=-52 13-18=-1796/206, 14-18=-676/54	6, 88, /920,	ŝ	ULIUS LEG MAR					
 NOTES- 1) Unbalanced roof live loads have been considered 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) N II; Exp B; Encl., GCpi=0.18; MWFRS (directional) 20-3-3 to 26-0-12, Interior(1) 26-0-12 to 28-8-5, Exright exposed; end vertical left and right exposed; plate grip DOL=1.60 3) Building Designer / Project engineer responsible for to the use of this truss component. 4) Provide adequate drainage to prevent water pondi 5) This truss has been designed for a 10.0 ps bottor will fit between the bottom chord and any other me 7) Provide mechanical connection (by others) of trus: 8) This truss design requires that a minimum of 7/16' sheetrock be applied directly to the bottom chord. 9) Graphical purlin representation does not depict the 	ior this design. 'asd=101mph; TCDL=6.0psf; BCDL=6.0 and C-C Exterior(2E) -1-6-0 to 4-3-10, In terior(2R) 28-8-5 to 34-5-15, Interior(1) C-C for members and forces & MWFRS r verifying applied roof live load shown ng. 1 chord live load nonconcurrent with any 0.0psf on the bottom chord in all areas v mbers, with BCDL = 10.0psf. Is to bearing plate capable of withstandir structural wood sheathing be applied d e size or the orientation of the purlin alor	Dpsf; h=15ft; B=45ft; L=58ft; eave=7 nterior(1) 4-3-10 to 20-3-3, Exterior 34-5-15 to 59-6-0 zone; cantilever for reactions shown; Lumber DOL: covers rain loading requirements s / other live loads. / other live loads. / other a rectangle 3-6-0 tall by 2-0-0 ng 100 lb uplift at joint(s) 2, 15. irectly to the top chord and 1/2" gyp ng the top and/or bottom chord.	rft; Cat. 2R) eft and =1.60 becific wide Julius MiTel 16023 Date:	No 34869 ORI ONAL S Lee PE No. 34869 k Inc. DBA MiTek USA FL Cert 6634 3 Swingley Ridge Rd. Chesterfield, MO 63017 October 27,2022					
WARNING - Verify design parameters and READ NOTES ON Design valid for use only with MiTek® connectors. This design a truss system. Before use, the building designer must verify t building design. Bracing indicated is to prevent buckling of inc is always required for stability and to prevent collapse with pos fabrication, storage, delivery, erection and bracing of trusses a Safety Information available from Truss Plate Institute, 2670	THIS AND INCLUDED MITEK REFERENCE PAGE is based only upon parameters shown, and is for a eapplicability of design parameters and properly ividual truss web and/or chord members only. Add sible personal injury and properly damage. For ge druss systems, see ANSUTPH Qualit Crain Highway, Suite 203 Waldorf, MD 20601	MII-7473 rev. 5/19/2020 BEFORE USE. an individual building component, not incorporate this design into the overall ditional temporary and permanent bracing eneral guidance regarding the y Criteria, DSB-89 and BCSI Building Com	ponent	16023 Swingley Ridge Rd Chesterfield, MO 63017					

- -



Scale = 1:103.4

1



35-0-10

41-5-0

1

45-7-8 49-10-0 53-8-10 58-0-0

6-5-6

Т

12-7-4

18-9-1

1

20-3-3 24-5-12 28-8-5

Plate Offsets (X,Y)	-5-6 6-1-14 6-1-14 [2:0-3-8,0-0-6], [5:0-4-0,0-4-8], [7:0-5-4,0	<u> </u>	<u>2-9 </u>	<u>6-4-5</u> Edge], [19:0-2-8,0-3-0],	<u>-2-8 4-2-8 3-10-10 4-3-6</u> [21:0-2-8.0-3-0], [26:0-2-8.0-3-0]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCodeFBC2020/TPI2014	CSI. TC 0.34 BC 0.64 WB 0.74 Matrix-AS	DEFL. i Vert(LL) -0.1 Vert(CT) -0.2 Horz(CT) 0.0	n (loc) l/defl L/ 5 21-22 >999 24 8 21-22 >999 18 4 18 n/a n/	d PLATES GRIP 0 MT20 244/190 0 a Weight: 456 lb FT = 20%			
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x4 SF WEBS 2x4 SF SLIDER Left 2x	P No.2 P No.2 P No.2 4 SP No.2 1-6-0		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheat 2-0-0 oc purlins (6-0-0 Rigid ceiling directly a 1 Row at midpt	hing directly applied, except) max.): 7-9. pplied. 5-25, 6-25, 7-24, 8-23, 9-23, 10-22			
REACTIONS. All be (Ib) - Max H Max U Max G	earings 0-3-8 except (jt=length) 25=0-5-1 lorz 2=-196(LC 10) plift All uplift 100 lb or less at joint(s) 2, irav All reactions 250 lb or less at joint(1, 18=0-4-0. 15 s) 15 except 2=725(LC 17	7), 25=2579(LC 17), 18=	-2049(LC 18)				
FORCES. (lb) - Max. TOP CHORD 2-3=- 7-37= 10-39	Comp./Max. Ten All forces 250 (lb) or -377/0, 3-35=-837/85, 4-35=-729/103, 4- 366/226, 8-37=-366/226, 8-38=-366/22 =-820/201, 10-11=-1369/231, 11-12=-1 -28/45	less except when shown. 5=-314/113, 5-36=0/425, .6, 9-38=-366/226, 9-39=- 527/190, 12-13=-819/123.	6-36=0/522, 807/236, 13-14=-55/1099,					
BOT CHORD 2-27= 22-43	=-5/0443 =-5/803, 26-27=-5/803, 24-25=-352/176, 3=-16/1138, 21-43=-16/1138, 20-21=-66/	23-42=0/665, 22-42=0/66 /1467, 19-20=-30/815, 18	5, 19=-1043/102,					
WEBS 4-27= 6-24= 9-22= 12-20	17-18=-400/51, 15-17=-400/51 WEBS 4-27=0/273, 4-26=-683/108, 5-25=-759/122, 6-25=-1696/215, 6-24=-30/1088, 7-24=-1084/53, 7-23=-137/1307, 8-23=-301/99, 9-23=-902/72, 9-22=-42/895, 10-22=-801/169, 10-21=0/439, 11-21=-388/72, 11-20=-317/109, 12-20=-42/827, 12-19=-860/100, 13-19=-141/2031, 13-18=-1681/194, 14-18=-671/53							
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-16; V II; Exp B; Encl., GCp 20-3-3 to 26-0-12, Ir right exposed ; end plate grip DOL=1.60 3) Building Designer / I	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=101r pi=0.18; MWFRS (directional) and C-C E tterior(1) 26-0-12 to 28-8-5, Exterior(2R) vertical left and right exposed;C-C for me) Project engineer responsible for verifying	sign. nph; TCDL=6.0psf; BCDL xterior(2E) -1-6-0 to 4-3-1 28-8-5 to 34-5-15, Interio embers and forces & MWI	=6.0psf; h=15ft; B=45ft; 0, Interior(1) 4-3-10 to 2 r(1) 34-5-15 to 59-6-0 z RS for reactions shown	L=58ft; eave=7ft; Cat. 20-3-3, Exterior(2R) one; cantilever left and n; Lumber DOL=1.60	No 34869			
 a) Building Designer / i to the use of this true 4) Provide adequate di 	roject engineer responsible for verifying ss component. rainage to prevent water ponding.			requirements specific	SONAL ENGLINE			
 5) This truss has been 6) * This truss has bee will fit between the b 	designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on the pottom chord and any other members, wi	e load nonconcurrent with he bottom chord in all are th BCDL = 10.0psf.	any other live loads. as where a rectangle 3-	6-0 tall by 2-0-0 wide	Julius Lee PE No. 34869			
7) Provide mechanical8) This truss design reasonable sheetrock be applied	 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 15. 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date: 							
9) Graphical purlin rep	resentation does not depict the size or th	e orientation of the purlin	along the top and/or bo	ttom chord.	October 27,2022			
WARNING - Verify de Design valid for use only a truss system. Before u building design. Bracing is always required for st fabrication, storage, deli Safety Information av	sign parameters and READ NOTES ON THIS AND II y with MiTek® connectors. This design is based only use, the building designer must verify the applicability j indicated is to prevent buckling of individual truss ability and to prevent collapse with possible persona very, erection and bracing of trusses and truss syst aliable from Truss Plate Institute, 2670 Crain Highw	NCLUDED MITEK REFERENCE I y upon parameters shown, and is y of design parameters and proy web and/or chord members only al injury and property damage. F ems, see <u>ANS/TPI1 Q</u> ay, Suite 203 Waldorf, MD 2060	PAGE MII-7473 rev. 5/19/2020 for an individual building con erly incorporate this design in Additional temporary and pe or general guidance regarding uality Criteria, DSB-89 and b	BEFORE USE. aponent, not to the overall rmanent bracing the BCSI Building Component	16023 Swingley Ridge Rd Chesterfield, MQ, 63017			

16023 Swingley Ridge Rd Chesterfield, MO 63017





6	-11-6 13-7-4 -11-6 6-7-14	19-2-0	<u>20-3₁3 24-5-12</u>	28-8-5	35-0-10	41	-5-0 4-5	45-7-8 4-2-8	49-10-0	53-8-10	<u>58-0-0</u>
Plate Offsets (X,Y)	[2:0-3-8,0-0-6], [4:0-4-0,0-4-8 [26:0-2-8,0-3-0]	3], [7:0-5-4,0-3	3-0], [9:0-5-4,0-3-0]	, [12:0-4-0,0)-4-8], [15:0-	-3-4,Edge], [1	9:0-2-8,0-3-0)], [21:0-2-0	8,0-3-0], [2	22:0-2-8,0-3	-0],
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/TPI2	2-0-0 1.25 1.25 YES 014	CSI. TC 0.26 BC 0.82 WB 0.70 Matrix-AS		DEFL. √ert(LL) √ert(CT) Horz(CT)	in (loc) -0.19 21-22 -0.35 21-22 0.03 18	l/defl l >999 2 >999 1 n/a r	L/d 240 80 n/a	PL/ MT: We	ATES 20 ight: 459 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x4 SF WEBS 2x4 SF SLIDER Left 2x REACTIONS. All be (lb) - Max H Max U Max U	 No.2 No.2 No.2 No.2 4 SP No.2 1-6-0 earings 0-3-8 except (jt=lengt lorz 2=-196(LC 10) plift All uplift 100 lb or less a 	n) 18=0-4-0, 2	25=0-4-0.	B T B V	RACING- OP CHORD OT CHORD /EBS	Structu 2-0-0 o Rigid c 1 Row	ral wood shea c purlins (6-0 eiling directly at midpt	athing dire I-0 max.): applied. 7-:	ectly applie 7-9. 24, 8-23, 9	ed, except 9-23, 10-22,	6-25, 5-25, 7-25
Max Grav All reactions 250 lb or less at joint(s) 15 except 2=657(LC 21), 18=1918(LC 18), 25=2743(LC 17) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-661/92, 5-6=0/707, 6-7=0/538, 9-10=-621/248, 10-11=-1188/237, 11-12=-1391/197, 12-13=-772/129, 13-14=-50/979, 14-15=-24/337 BOT CHORD 2-27=0/584, 26-27=0/581, 24-25=-440/162, 23-24=-441/162, 22-23=0/479, 21-22=-22/976, 20-21=-73/1333, 19-20=-36/763, 18-19=-926/98, 17-18=-296/48, 15-17=-296/48 WEBS 4-27=0/304, 4-26=-656/63, 5-26=0/529, 7-23=-122/1424, 8-23=-281/94, 9-23=-942/91, 9-22=-636/88, 10-22=-636/163, 10-21=-0/480, 11-21=-421/73, 11-20=-262/109, 12-20=-43/724, 12-19=-774/101, 13-19=-143/1848, 13-18=-1554/195, 14-18=-659/53, 6-25=-425/121, 5-25=-843/163, 7-25=-1408/87											
 NOTES- Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=58ft; eave=7ft; Cat. I; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 4-3-10, Interior(1) 4-3-10 to 20-3-3, Exterior(2R) 20-3-3 to 26-0-12, Interior(1) 26-0-12 to 28-8-5, Exterior(2R) 28-8-5 to 34-5-15, Interior(1) 34-5-15 to 59-6-0 zone; cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component. Provide adequate drainage to prevent water ponding. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 15. This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. Cotober 27,2022 											
									1		

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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MiTek



Scale = 1:99.8



	L	24-5-12		41-5-0	1	58-0-0		
	1	24-5-12		16-11-4	I	16-7-0		
Plate Offsets ((X,Y) [[8:0-4-0,0-4-8]						
LOADING (ps TCLL 20 TCDL 10 BCLL 0 BCDI 10	sf) 1.0 1.0 1.0 1.0 *	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2020/TPI2014	CSI. TC 0.06 BC 0.05 WB 0.15 Matrix-S	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 32 n/a n/a	PLATES GRIP MT20 244/190 Weight: 500 lb ET = 2	20%	
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP 2x6 SP 2x4 SP	No.2 No.2 No.2		BRACING- TOP CHORD S 2 BOT CHORD F	Structural wood sheathing 2-0-0 oc purlins (6-0-0 ma Rigid ceiling directly appli	g directly applied or 6-0-0 oc purlins, (ix.): 12-16. ed or 10-0-0 oc bracing.	except	
				WEBS 1	Row at midpt	13-51, 11-52, 10-53, 15-48, 17-47, 14-50	18-46,	

REACTIONS. All bearings 58-0-0.

(lb) - Max Horz 1=-193(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 56, 53, 54, 58, 57, 59, 61, 62, 46, 45, 44, 43, 42, 41, 40, 37, 35, 36, 34, 33, 50

Max Grav All reactions 250 lb or less at joint(s) 1, 51, 52, 56, 53, 54, 58, 57, 59, 60, 61, 62, 48, 47, 46, 45, 44, 43, 42, 41, 40, 38, 37, 35, 36, 34, 50, 32 except 33=304(LC 22)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 10-11=-97/252

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=58ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3E) 0-3-2 to 6-0-11, Exterior(2N) 6-0-11 to 21-3-7, Corner(3R) 21-3-7 to 27-1-1, Exterior(2N) 27-8-0, Corner(3R) 27-8-0 to 33-5-10, Exterior(2N) 33-5-10 to 56-2-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 56, 53, 54, 58, 57, 59, 61, 62, 46, 45, 44, 43, 42, 41, 40, 37, 35, 36, 34, 33, 50.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Julius Lee PE No. 34869 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

October 27,2022



