

GENERAL NOTES

A. BUILDING AND DESIGN CODES:

1. 2020 FLORIDA BUILDING CODE (FBC).

2. ASCE 7-16 MINIMUM DESIGN LOADS BUILDING AND OTHER STRUCTURES.

3. AFPA NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, 2018 EDITION.

4. ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, 2014.

B. DESIGN LOADS:

ENGINEERING DATA

FLOOR LIVE LOADS: 50 PSF  
ROOF LIVE LOAD: 20 PSF  
LIVE LOAD REDUCTION ON SUPPORTING ELEMENTS IN ACCORDANCE WITH 2020 FBC.  
  
LOADING FOR MECHANICAL AND ELECTRICAL ROOMS ARE BASED ON THE WEIGHTS OF ASSUMED EQUIPMENT, AS INDICATED ON THE MECHANICAL DRAWINGS (INCLUDING THE WEIGHT OF CONCRETE PADS WHERE INDICATED ON THE MECHANICAL DRAWINGS). ANY CHANGES IN TYPE, SIZE, LOCATION, OR NUMBER OF PIECES OF EQUIPMENT SHOULD BE REPORTED TO THE ENGINEER FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO PLACEMENT OF EQUIPMENT.  
ROOF DEAD LOAD: 18 PSF  
DESIGN DEAD LOADS INCLUDE THE WEIGHT OF STRUCTURAL COMPONENTS AND PERMANENT FIXTURES.  
  
GROUND SNOW LOAD: Pg = 0 PSF  
FLAT ROOF SNOW LOAD: Pf = 0 PSF  
SNOW EXPOSURE FACTOR: Ce = 1.0  
SNOW LOAD IMPORTANCE FACTOR: I = 1.0  
THERMAL FACTOR: Ct = 1.1  
  
WIND DESIGN DATA:  
ULTIMATE DESIGN WIND SPEED (3 SECOND GUST): 120 MPH  
WIND EXPOSURE CATEGORY: C  
BUILDING CATEGORY: II - ENCLOSED BUILDING  
INTERNAL PRESSURE COEFFICIENTS: ±0.18  
ALL NEW COMPONENTS AND CLADDING NOT DESIGNED BY THE ENGINEER SHALL BE DESIGNED FOR ASCE 7-16 CBC DESIGN LOADS UNLESS OTHERWISE APPROVED BY THE ENGINEER  
  
EARTHQUAKE DESIGN DATA:  
SEISMIC IMPORTANCE FACTOR: I: 1.0  
MAPPED SPECTRAL RESPONSE ACCELERATIONS: S<sub>s</sub> = 0.086, S<sub>1</sub> = 0.081  
SITE CLASS: C  
SPECTRAL RESPONSE COEFFICIENTS - S<sub>DS</sub> = 0.092, S<sub>DI</sub> = 0.081  
SEISMIC DESIGN CATEGORY: B  
BASIC SEISMIC-FORCE-RESISTING SYSTEM: BEARING WALL SYSTEM, LIGHT FRAMED WALLS WITH SHEAR PANELS  
DESIGN BASE SHEAR: V = 1.0 kip  
SEISMIC RESPONSE COEFFICIENT: C<sub>s</sub> = 0.01  
RESPONSE MODIFICATION FACTOR: R = 6.5  
ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE METHOD  
  
FLOOD HAZARD INFORMATION: THIS BUILDING IS NOT DESIGNED FOR FLOOD LOADS.

FOUNDATION NOTES

1. FLOOR DRAIN GRATES SHALL BE FLUSH WITH FINISHED FLOOR ALL FLOOR DRAINS TO BE 1/2" BELOW FINISHED FLOOR.

2. SLOPE FLOOR 1/16" PER FOOT MIN. AT ALL FLOOR DRAINS AND FLOOR SINKS AS SHOWN.

3. ALL SIMPSON HOLD DOWNS TO BE STAINLESS STEEL OR Z-MAX G185 HOT DIPPED GALVANIZED COATING.

4. UNLESS OTHERWISE INDICATED BY SHEAR SCHEDULE, 1/2" X 10" SILL PLATE ANCHOR BOLTS WITH .229"x3"x3" PLATE WASHERS ARE TO PROVIDED AT 48" AT ALL SILL PLATES. ALL SILL PLATE ANCHOR BOLTS TO BE STAINLESS STEEL OR G185 HOT DIPPED GALVANIZED.

5. ALL STEEL IN CONTACT WITH TREATED WOOD TO BE STAINLESS STEEL OR G185 HOT DIPPED GALVANIZED.

SLAB-ON-GRADE NOTES

A. SLAB ON GRADE PER 1/52.01.

B. THE SLAB SHALL BE UNDERLAIN BY A MINIMUM 10-MIL "STEGO WRAP" CLASS A VAPOR RETARDER BY STEGO INDUSTRIES OR VAPOR BLOCK 10 BY RAVEN INDUSTRIES TO BE PLACED BELOW THE FLOOR SLAB WHERE REQUIRED TO PROTECT MOISTURE-SENSITIVE FLOOR COVERINGS (I.E. TILE, OR CARPET, ETC.). THE SHEETS OF THE VAPOR RETARDER MATERIAL SHALL BE EVALUATED FOR HOLES AND/OR PUNCTURES PRIOR TO PLACEMENT AND THE EDGES OVERLAPPED AND TAPED WITH STEGO OR RAVEN TAPE. IF MATERIALS UNDERLYING THE SYNTHETIC SHEET CONTAIN SHARP, ANGULAR PARTICLES, A LAYER OF SAND APPROXIMATELY TWO INCHES THICK OR A GEOTEXTILE SHALL BE PROVIDED TO PROTECT IT FROM PUNCTURE.

REINFORCING STEEL NOTES

A. PROVIDE DETAILING, FABRICATION, AND INSTALLATION OF REINFORCING AND ACCESSORIES IN ACCORDANCE WITH ACI 315 AND ACI 318.

B. PROVIDE NEW BILLET STEEL REINFORCING BARS IN ACCORDANCE WITH ASTM A 615, GRADE 60.

C. PROVIDE ANCHOR BOLTS CONFORMING TO THE STANDARDS OF ASTM F1554 OR 36 UNLESS OTHERWISE NOTED.

D. COORDINATE PLACEMENT OF CAST-IN-PLACE EMBEDMENTS AND ANCHOR BOLTS. SET ANCHOR BOLTS WITH A TEMPLATE. SECURELY ATTACH EMBEDDED ITEMS TO FORMWORK OR REINFORCING.

E. MINIMUM SPICE LENGTH SHALL BE 30 BAR DIAMETERS UNLESS NOTED OTHERWISE ON THE PLANS OR STANDARD DETAILS. PROVIDE STANDARD 90-DEGREE HOOKS IN ACCORDANCE WITH ACI 318, UNLESS OTHERWISE NOTED. STAGGER SPLICES UNLESS SPECIFICALLY NOTED.

F. MAINTAIN THE FOLLOWING CONCRETE COVERAGE FOR REINFORCING STEEL UNLESS OTHERWISE NOTED:

1. CONCRETE CAST AGAINST EARTH: 3"

2. CONCRETE EXPOSED TO WEATHER:

a. NO. 6 AND LARGER 2"

b. NO. 5 AND SMALLER 1 1/2"

3. ABOVE SLAB ON GRADE REINFORCEMENT: 1 1/2"

G. DO NOT WELD OR BEND REINFORCEMENT IN THE FIELD UNLESS SPECIFICALLY SHOWN OR APPROVED BY STRUCTURAL ENGINEER.

H. WHERE REQUIRED, PROVIDE DOWELS TO MATCH SIZE AND SPACING OF MAIN REINFORCING.

I. PROVIDE CONTINUOUS HORIZONTAL WALL/GRADE BEAM REINFORCEMENT WITH 90-DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL BAR PLACING DETAILS.

J. PROVIDE BAR SUPPORT ACCESSORIES IN ACCORDANCE WITH THE LATEST ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES. SUPPORT BEAM REINFORCING ON BEAM BOLSTERS SPACED NOT MORE THAN 4 FEET ON CENTER.

K. PROVIDE BAR SUPPORTS WITH PLASTIC COATED LEGS OR HOT DIP GALVANIZING AFTER FABRICATION FOR CONCRETE EXPOSED TO VIEW. PROVIDE STAINLESS STEEL BAR SUPPORTS FOR CONCRETE TO RECEIVE A SANDBLAST FINISH.

CONCRETE NOTES

A. PROVIDE DIFFERENT CLASSES OF CONCRETE AS SHOWN BELOW. PROVIDE BATCH MIXING, TRANSPORTATION, PLACING AND CURING OF CONCRETE IN ACCORDANCE WITH RECOMMENDATIONS OF ACI 301 AND ACI 318. USE TYPE I PORTLAND CEMENT (ASTM C150) UNLESS OTHERWISE NOTED. PROVIDE ADMIXTURES AND SPECIAL REQUIREMENTS AS SPECIFIED.

1. NORMAL WEIGHT(150 PCF), F'c = 3,500 PSI CONCRETE AT 28 DAYS (MINIMUM OF 5 SACKS OF PORTLAND CONCRETE PER CUBIC YARD).

a. ALL CONCRETE GRADE SUPPORTED SLABS: MID TO HIGH RANGE WATER REDUCING AGENT.

b. GRADE BEAMS AND FOOTINGS: 3%-5% AIR CONTENT USING AIR EXTRACTING AGENT AS REQUIRED.

B. UNLESS SPECIFIED BELOW, CONCRETE MUST REACH THE FOLLOWING PERCENTAGES OF ITS 28 DAY COMPRESSIVE STRENGTH (F'c) BEFORE FORMS MAY BE REMOVED:

C. REFERENCE THE APPROPRIATE DISCIPLINE DRAWINGS FOR SUBSLAB PIPING, FLOOR DRAINS AND SLAB AND WALL PENETRATIONS.

EPOXY ANCHORED DOWELS AND BOLTS

A. REFERENCE DRAWINGS FOR EPOXY ANCHORING SYSTEM.

B. INSTALL BOLTS AND DOWELS IN ACCORDANCE WITH CURRENT ICBO REPORT FOR THE BOLT, AND RECOMMENDATIONS OF THE MANUFACTURER. FOLLOW MANUFACTURER'S INSTRUCTIONS EXPLICITLY.

C. INSTALL BOLTS AND DOWELS PERPENDICULAR TO THE FACE OF CONCRETE, UNLESS OTHERWISE INDICATED IN THE DRAWINGS.

D. DIAMETER OF DRILLED HOLES SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT REDIRECT HOLES ALONG LENGTH OF BOLT.

E. DRILL HOLES IN CONTINUOUS OPERATION. BLOW DUST FROM THE HOLE USING COMPRESSED AIR.

F. FILL ABANDONED HOLES WITH EPOXY GROUT.

G. INSTALLATION OF EPOXY ANCHORED DOWELS AND BOLTS SHALL BE CONTINUOUSLY INSPECTED BY THE TESTING LABORATORY TO ENSURE THAT HOLES ARE OF THE PROPER DIAMETER AND LENGTH, ARE PROPERLY CLEANED, AND THAT BOLTS ARE INSTALLED CORRECTLY.

H. UNLESS NOTED OTHERWISE EPOXY ANCHORS, REBAR, OR THREADED RODS, SHALL BE HLTI HIT-HY 200 ANCHORS. INSTALL ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS.

STRUCTURAL WOOD NOTES

A. WOOD FRAMING:

1. ALL WOOD STRUCTURES HAVE BEEN DESIGNED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.

2. ALL DIMENSIONAL LUMBER SHALL BE EITHER DOUGLAS FIR OR SOUTHERN YELLOW PINE STAMPED BY A MEMBER OF THE WMPA AND KILN DRIED TO 19% MAXIMUM MOISTURE CONTENT. THE GRADES OF LUMBER TO BE USED ARE AS FOLLOWS:

a. STUDS AND BLOCKING STUD, NO. 2

b. JOISTS, RAFTERS AND HEADERS, NO. 1

c. LEDGERS, NO. 2

d. BEAMS AND STRINGERS, NO. 1

e. COLUMNS AND POSTS, NO. 1

3. PRESSURE TREAT ALL WOOD PRODUCTS IN DIRECT CONTACT WITH THE FOUNDATION PER SPECIFICATION.

4. JOISTS, RAFTERS AND BEAMS SHALL NOT BE NOTCHED, EXCEPT WHERE SHOWN IN DETAILS. OBTAIN STRUCTURAL ENGINEER'S APPROVAL FOR ANY HOLES THROUGH OR NOTCHES IN THE TOP OF HORIZONTAL MEMBERS.

5. SILL PLATES SHALL BE 2x (STUD WIDTH) TREATED D.F., UNO. SILL PLATES AT ALL SHEAR WALLS SHALL BE FASTENED TO THE FOUNDATION AS SCHEDULED. SILL PLATES AT EXTERIOR WALLS SHALL BE FASTENED TO THE FOUNDATION WITH 5/8" DIAMETER X 10" LONG ANCHOR BOLTS SPACED AT 48" ON CENTER. LOCATE ANCHOR BOLTS A MAXIMUM OF 8" FROM EACH END OF PLATE. SILL PLATES AT INTERIOR WALLS SHALL BE FASTENED TO THE FOUNDATION WITH ONE 5/8" DIAMETER X 7" LONG ANCHOR BOLT AT 48" ON CENTER.

6. ALL HARDWARE USED IN THE WORK SHALL BE SIMPSON STRONG-TIE, KC METALS, SILVER OR OTHER HARDWARE OF EQUAL CAPACITY AND DAVING ISO90 APPROVAL.

7. A DOUBLE PLATE MADE OF TWO MEMBERS OF THE SAME WIDTH AS THE STUDS SHALL BE PLACED AT THE TOP OF EVERY BEARING PARTITION OR EXTERIOR WALL. SUCH DOUBLE PLATES SHALL BE LAPPED AT CORNER, AND JOINTS IN UPPER AND LOWER MEMBERS SHALL BE AT LEAST 4 FEET APART, EXCEPT AT CORNERS.

8. EXTERIOR WALLS AND BEARING WALLS SHALL BE FRAMED, UNLESS NOTES OTHERWISE, AS FOLLOWS:

MEMBER SIZE AND SPACING: LOADING: ROOF:

a. 2x6 STUDS AT 16" ON CENTER W/ BLOCKING 8'-0" O.C. MIN.

9. ALL NAILING SHALL BE DONE IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF THE DESIGN BUILDING CODE, EXCEPT AS OTHERWISE SHOWN AND SHALL BE DONE WITH COMMON WIRE NAILS UNLESS ANOTHER TYPE OF NAIL IS ALLOWED BY THE DESIGN BUILDING CODE. (SEE NAILING SCHEDULE)

10. BOLTS SHALL BE UNFINISHED BOLTS CONFORMING TO ASTM A307. BOLTS HOLES THROUGH THE WOOD SHALL BE DRILLED NO MORE THAN 1/16" GREATER THAN BOLT DIAMETER.

11. ALL STRUCTURAL SHEATHING SHALL BE APA RATED SHEATHING. UNLESS NOTED OTHERWISE, EXTERIOR WALL CONSTRUCTION SHALL CONSIST OF 15/32" SHEATHING FULL HEIGHT. PROVIDE 8d COMMON NAILS AT 6" CENTER TO CENTER AT SUPPORT EDGES, AND AT 12" CENTER TO CENTER AT INTERMEDIATE STUDS, EXCEPT AT DESIGNATED SHEAR WALLS. PROVIDE SOLID BLOCKING AT ALL PANEL EDGES TO RECEIVE EDGE NAILING.

12. SOLID BLOCKING AT EDGES REQUIRED FOR ALL SHEARWALLS AND ROOF DIAPHRAGMS.

B. GLUE LAMINATED MEMBERS.

1. ALL GLUE LAMINATED MEMBERS SHALL CONFORM TO THE APPLICABLE CURRENT REQUIREMENTS OF AITC 117 SPECIFICATION AND ALL APPROVED SUPPLEMENTS THEREOF. FABRICATION SHALL BE PERFORMED BY AN "APPROVED FABRICATOR" PER REQUIREMENTS OF GOVERNING CODE, WITH WORK AITC FACTORY INSPECTED, CERTIFIED AND GRADE STAMPED.

2. ALL MEMBERS SHALL MEET OR EXCEED THE FOLLOWING STRESS VALUES:

24F-V4 DOUGLAS FIR:

a. Fbxx (TENSION ZONE IN TENSION) 2400 PSI

b. Fbxx (COMPRESSION ZONE IN TENSION) 1450 PSI

c. Fc perp. (TENSION FACE) 650 PSI

d. Fc perp. (COMPRESSION FACE) 650 PSI

e. Fvx (HORIZONTAL SHEAR) 240 PSI

f. E (MODULUS OF ELASTICITY) 1800000 PSI

3. LAMINATED MEMBERS SHALL BE SIZED TO DIMENSIONS SHOWN ON DRAWINGS WITH BEAMS CUT TO DEPTHS WITHIN AN APPROVED TOLERANCE ± 1/8", ENDS ARE TO BE SEALED AND INDIVIDUALLY WRAPPED.

4. ALL LAMINATED BEAMS SHALL BE DELIVERED WITH MOISTURE CONTENT WHICH IS IN COMPLIANCE WITH THE AITC SPECIFICATIONS CONCERNING THAT SPECIES.

5. ALL LAMINATED BEAMS SHALL BE CAPABLE OF DEVELOPING THE FULL SPECIFIED DESIGN STRESSES, I.D. BLEMSHES, IMPERFECTIONS, SPLITS, ETC., SHALL NOT EXCEED THAT WHICH HAS BEEN ACCOUNTED FOR IN ESTABLISHING THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION ALLOWABLE DESIGN STRESSES.

6. ANY GLUE LAMINATED MEMBER WHICH DOES NOT MEET THE ABOVE DESIGN REQUIREMENTS SHALL BE REJECTED. USE OF THE FEELER GAGE TEST METHOD OF MEASURING DEPTHS OF SPLITS, ETC., WILL NOT BE PERMITTED FOR THE PURPOSE OF DETERMINING MEMBERS ACCEPTABILITY, BUT MAY BE UTILIZED FOR REJECTION.

TYPICAL LUMBER NAILING SCHEDULE		
NAILING SHOWN IS TYPICAL EXCEPT AS NOTED ON PLANS. USE COMMON NAILS.		
1. JOIST TO SILL OR GIRDER, TOENAILS	3-8d	
2. BRIDGING TO JOIST, TOE NAIL EACH END	2-8d	
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d	
4. WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d	
5. 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d	
6. SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d AT 16" O.C.	
7. TOP PLATE TO STUD, END NAIL	2-16d	
8. STUD TO SOLE PLATE	2-16d END NAILS OR 4-8d TOE NAILS	
9. DOUBLE STUDS, FACE NAIL	16d AT 12" STAGGERED	
10. DOUBLED TOP PLATES, FACE NAIL	16d AT 16" O.C.	
11. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d	
12. CONTINUOUS HEADER, TWO PIECES	16d AT 16" O.C. ALONG EA. EDGE	
13. CEILING JOISTS TO PLATE, TOE NAIL	3-8d	
14. CONTINUOUS HEADER TO STUD, TOE NAIL	4-8d	
15. CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d	
16. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d	
17. RAFTER TO PLATE, TOENAIL	3-8d	
18. 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d	
19. 1"x8" SHEATHING OR LESS TO EACH JOIST, FACE NAIL	2-8d	
20. WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL	3-8d	
21. BUILT-UP CORNER STUDS	16d AT 24" O.C.	
22. BUILT-UP GIRDER AND BEAMS	20d AT 32" O.C. AT TOP & BOTTOM 2-20d AT ENDS AND AT EA. SPLICE	
23. 2" PLANKS	2-16d AT EACH BEARING	

SPECIAL INSPECTIONS NOTES

A. THE OWNER WILL EMPLOY THE SERVICES OF ONE OR MORE SPECIAL INSPECTORS TO PROVIDE SPECIAL INSPECTIONS DURING CONSTRUCTION FOR THE ITEMS IN THE SPECIAL INSPECTION TABLE BELOW IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2015 EDITION, NOT APPLICABLE TO SLABS ON GROUND OR SITE CONCRETE FULLY SUPPORTED ON EARTH.

B. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

C. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

1. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAIVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS.

2. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, THE PROFESSIONAL OF RECORD, AND THE CONTRACTOR. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION.

3. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

D. OBSERVATION BY STRUCTURAL ENGINEER OF RECORD IS NOT REQUIRED. IF STRUCTURAL OBSERVATION IS REQUIRED BY THE BUILDING AUTHORITY IT SHALL BE AS DIRECTED BY THE OWNER TO COMPLY WITH THE REQUIREMENTS OF THE GOVERNING BUILDING CODE.

E. THE FABRICATION OF TRUSSES AND OTHER ASSEMBLIES CONSTRUCTED USING WOOD AND METAL MEMBERS, OR USING LIGHT METAL PLATE CONNECTORS, SHALL BE CONTINUOUSLY INSPECTED BY A QUALIFIED INSPECTOR APPROVED BY THE ENFORCEMENT AGENCY AND PAID FOR BY THE CONTRACTOR. THE INSPECTOR SHALL FURNISH THE ARCHITECT, STRUCTURAL ENGINEER AND THE ENFORCEMENT AGENCY WITH A REPORT THAT THE LUMBER SPECIES, GRADES AND MOISTURE CONTENT, TYPE OF GLUE, TEMPERATURE AND GLUING PROCESS, TYPE OF METAL MEMBERS AND METAL CONNECTORS, AND THE WORKMANSHIP, CONFORM IN EVERY MATERIAL RESPECT WITH THE DULY APPROVED PLANS AND SPECIFICATIONS. EACH INSPECTED TRUSS SHALL BE STAMPED BY THE INSPECTOR WITH AN IDENTIFYING MARK.

F. INSPECTION OF FABRICATORS IS NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED BY THE BUILDING OFFICIAL TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS TO THE BUILDING OFFICIAL AND TO THE ENGINEER OF RECORD.

G. SPECIAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH THE TABLE BELOW:

SPECIAL INSPECTION	FREQ.	REFERENCED STANDARD
SOILS:		
1. PRIOR TO THE PLACEMENT OF PREPARED FILLS, VERIFY THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE FOUNDATION NOTES AND GEOTECHNICAL REPORT.	----	GEOTECHNICAL ENGINEERING REPORT; IBC 1704.7
2. FILL PLACEMENT 12" THICK OR GREATER - VERIFY THAT THE MATERIAL BEING USED AND MAXIMUM LIFT THICKNESS COMPLY WITH THE FOUNDATION NOTES AND GEOTECHNICAL REPORT.	CONTINUOUS	
3. VERIFY, AT THE FREQUENCY SPECIFIED IN THE FOUNDATION NOTES AND GEOTECHNICAL REPORT, THAT THE IN-PLACE DRY DENSITY OF COMPACTED FILL COMPLIES WITH THE FOUNDATION NOTES AND GEOTECHNICAL REPORT.	----	
CONCRETE:		
1. INSPECTION OF REINFORCING STEEL, SIZE AND PLACEMENT	PERIODIC	ACI 318: 3.5, 7.1-7.7
2. INSPECT BOLTS TO BE INSTALLED IN CONCRETE, PRIOR TO AND DURING PLACEMENT OF CONCRETE.	PERIODIC	IBC 1912.5
3. VERIFYING USE OF REQUIRED DESIGN MIX	PERIODIC	ACI 318: Ch. 4, 5.2-5.4
4. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP, TEMP. AND AIR CONTENT TESTS.	CONTINUOUS	ASTM C 172; ASTM C 311; ACI 318: 5.6, 5.8
5. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	CONTINUOUS	ACI 318: 5.9, 5.10
ADHESIVE ANCHORS/REINF.:		
1. DURING PLACEMENT OF ADHESIVE ANCHORS OR REINFORCEMENT EMBEDDED WITH ADHESIVE (AS SPECIFIED ON THE CONSTRUCTION DOCUMENTS) IN MASONRY AND CONCRETE: <div><div>a. SIZE AND EMBEDMENT OF ANCHORS/REINF.</div><div>b. ANCHORS/REINF. INSTALLED PER MANUFACTURERS RECOMMENDATIONS.</div></div>	CONTINUOUS	MANUFACTURERS INSTALLATION INSTRUCTIONS
	CONTINUOUS	
STRUCTURAL WOOD:		
1. INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS AND HOLD-DOWNS, WHERE THE FASTENERS SPACING OF THE SHEATHING IS MORE THAN 4" O.C.	PERIODIC	IBC 1707.3

CONSULTANT:

national  
restaurant  
designers

ARCHITECTS & ENGINEERS

7208 ACC Blvd, 2nd Floor, Raleigh, NC 27617  
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A Division of LMHT Associates

CLIENT:

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OKLAHOMA CITY, OK 73104  
OFFICES: 405-225-5000  
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PROJECT INFORMATION:

SONIC

SONIC DRIVE-IN RESTAURANT  
LOT #3 GATEWAY CROSSING CENTURION WAY  
LAKE CITY, FLORIDA  
STORE # -

SEAL:

07/13/22

PROJECT NO.: 22161  
DRAWN BY: DS  
CHECKED BY: RC  
  
ISSUE: DATE:  
DISTRIBUTION SET 2022-06-24

REVISION: DATE:

PROJECT LOCATION:  
LAKE CITY, FLORIDA

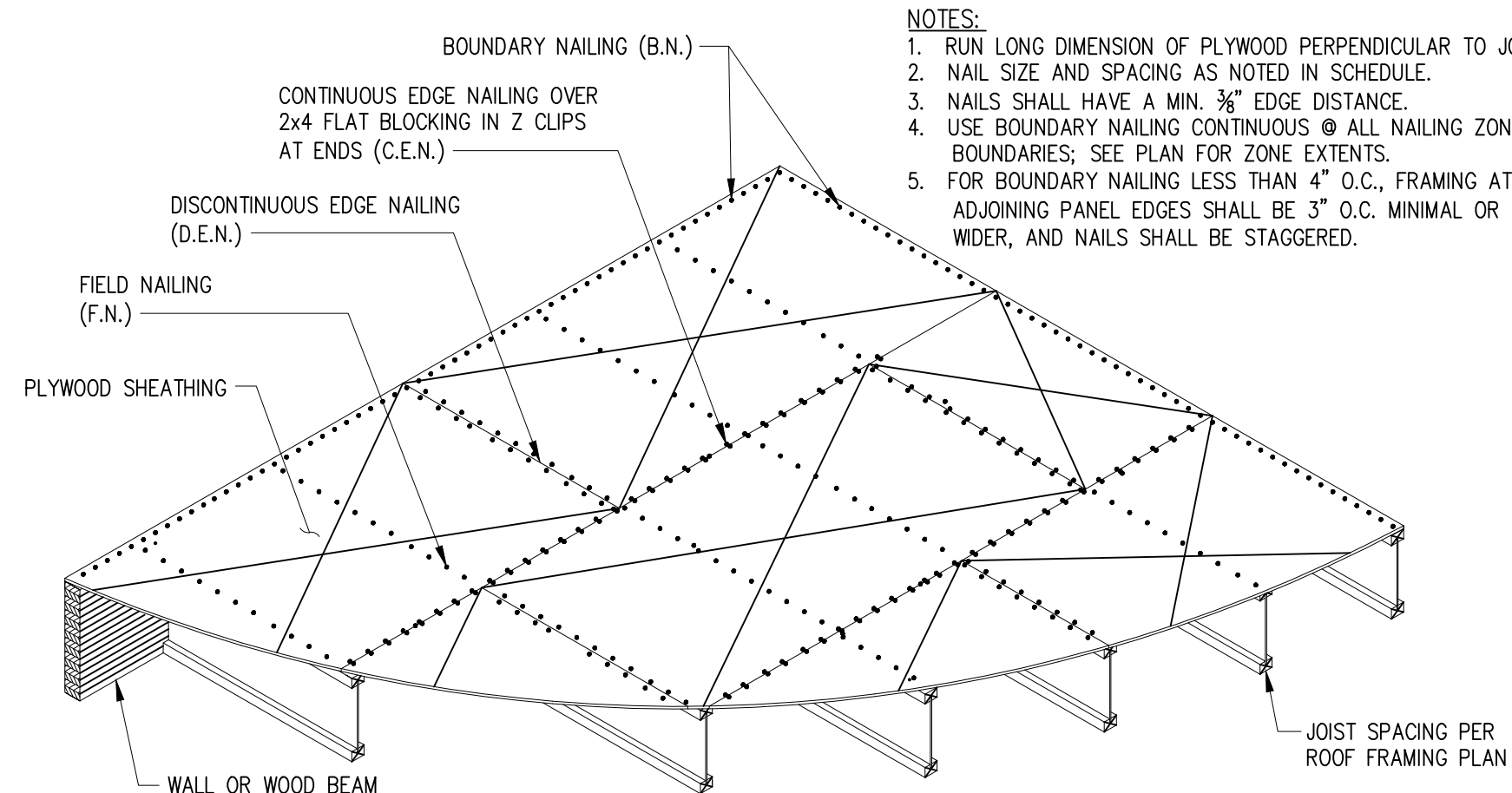
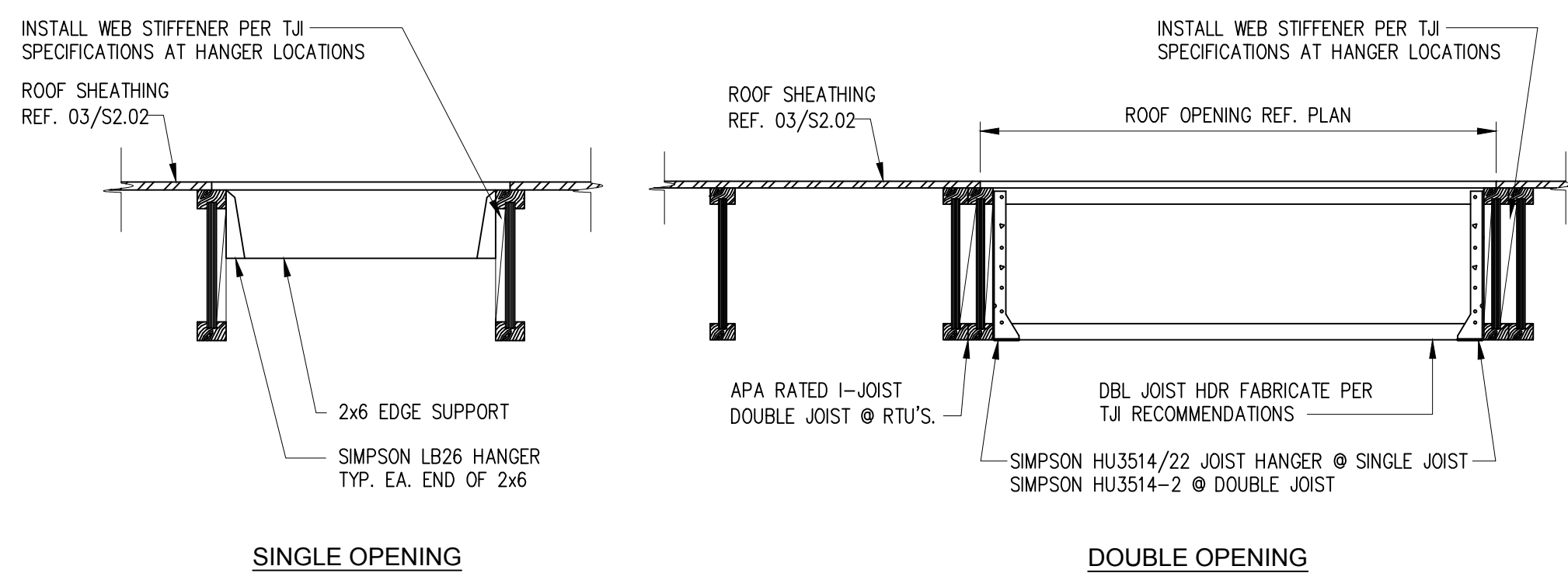
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Norfolk, VA 23510  
(757) 965-5710  
BPA Job No. 220603  
FL COA #27497

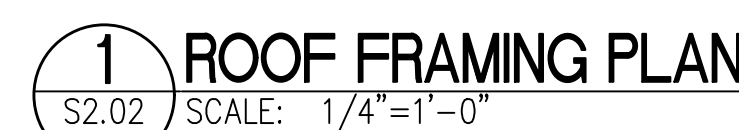
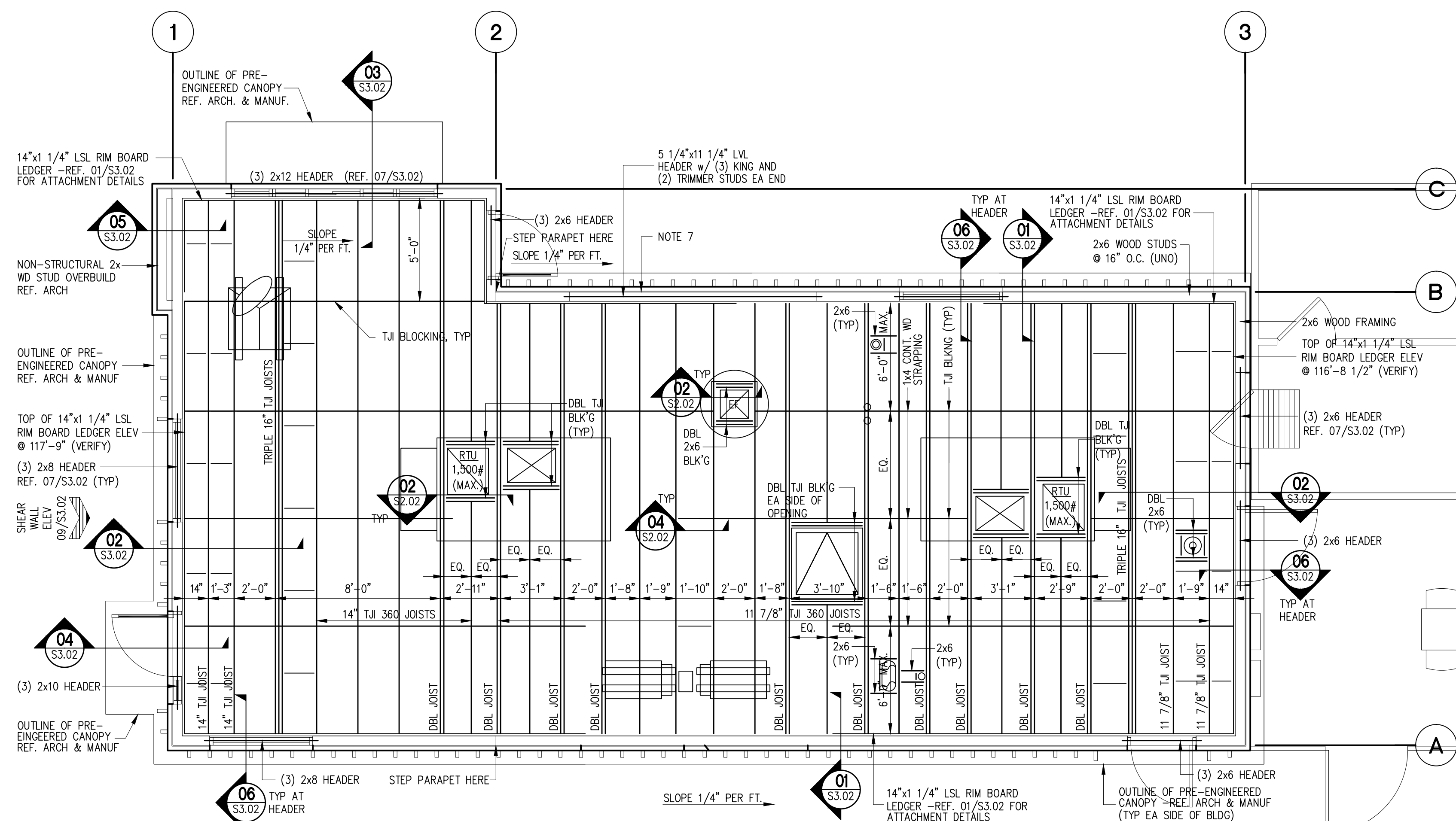
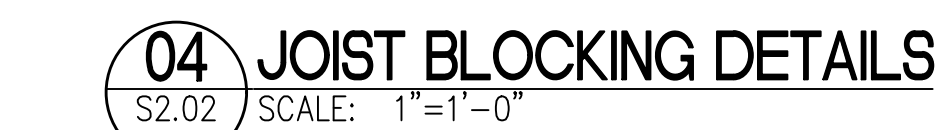
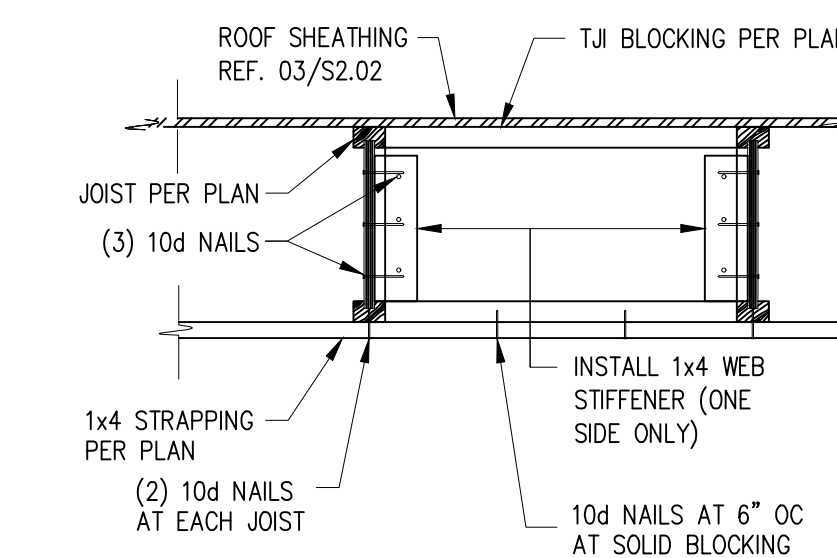
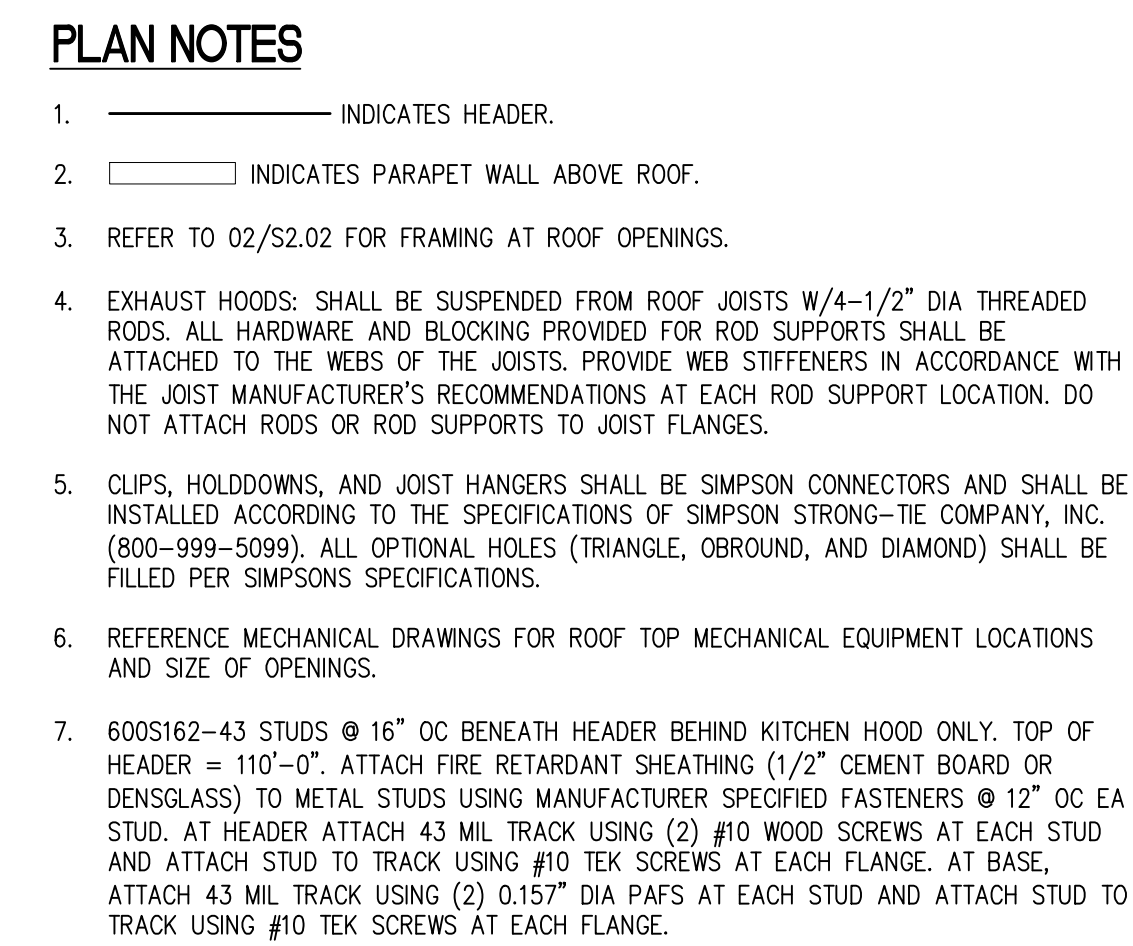




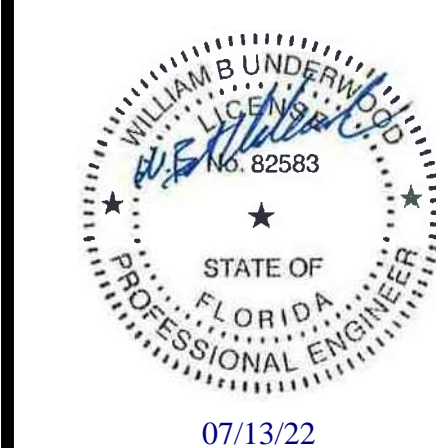
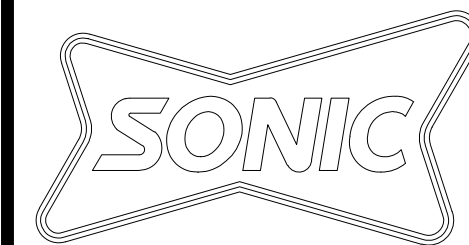




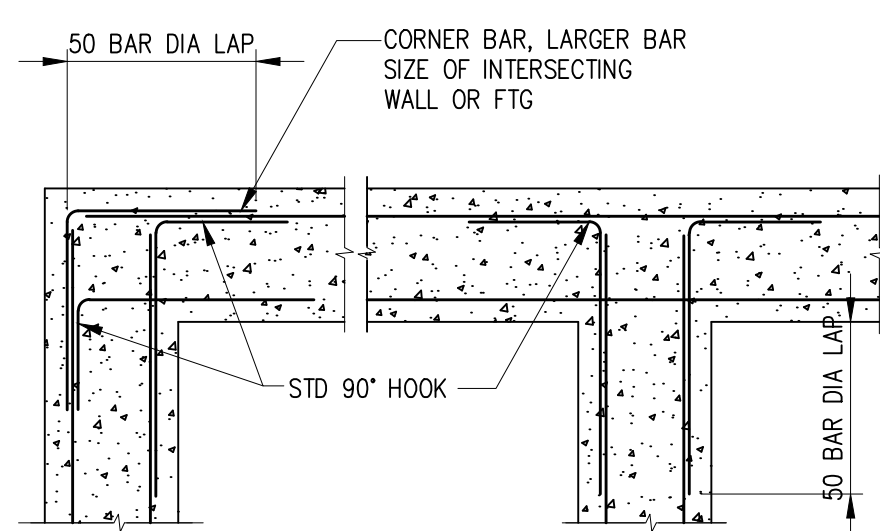
DIAPHRAGM ZONE	SHEATHING	NAILS	NAILING		
			EDGE (E.N.)	BOUNDARY (B.N.)	FIELD (F.N.)
BLOCKED	19/32" APA RATED	8d	4" O.C.	4" O.C.	12" O.C.



**NOTE:**  
VERIFY ALL DIMENSIONS WITH ARCHITECTURAL  
DRAWINGS AND EXISTING CONDITIONS.

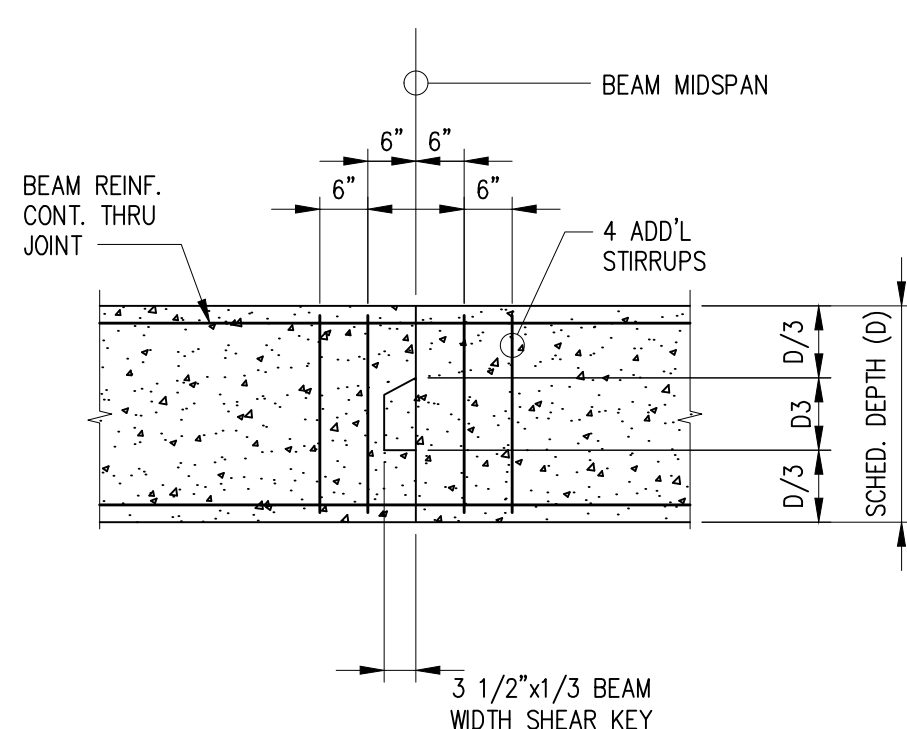




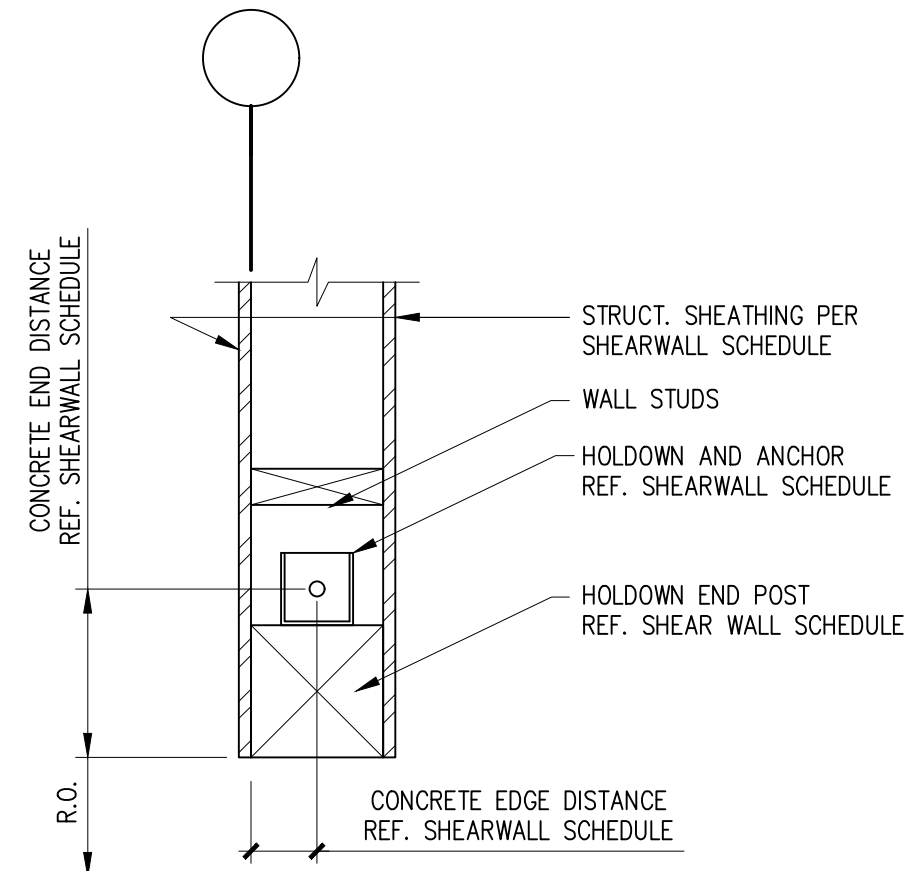


NOTE:  
1. VERT REINF NOT SHOWN FOR CLARITY.  
2. THIS DETAIL APPLIES TO BEAM AND WALL REINF.

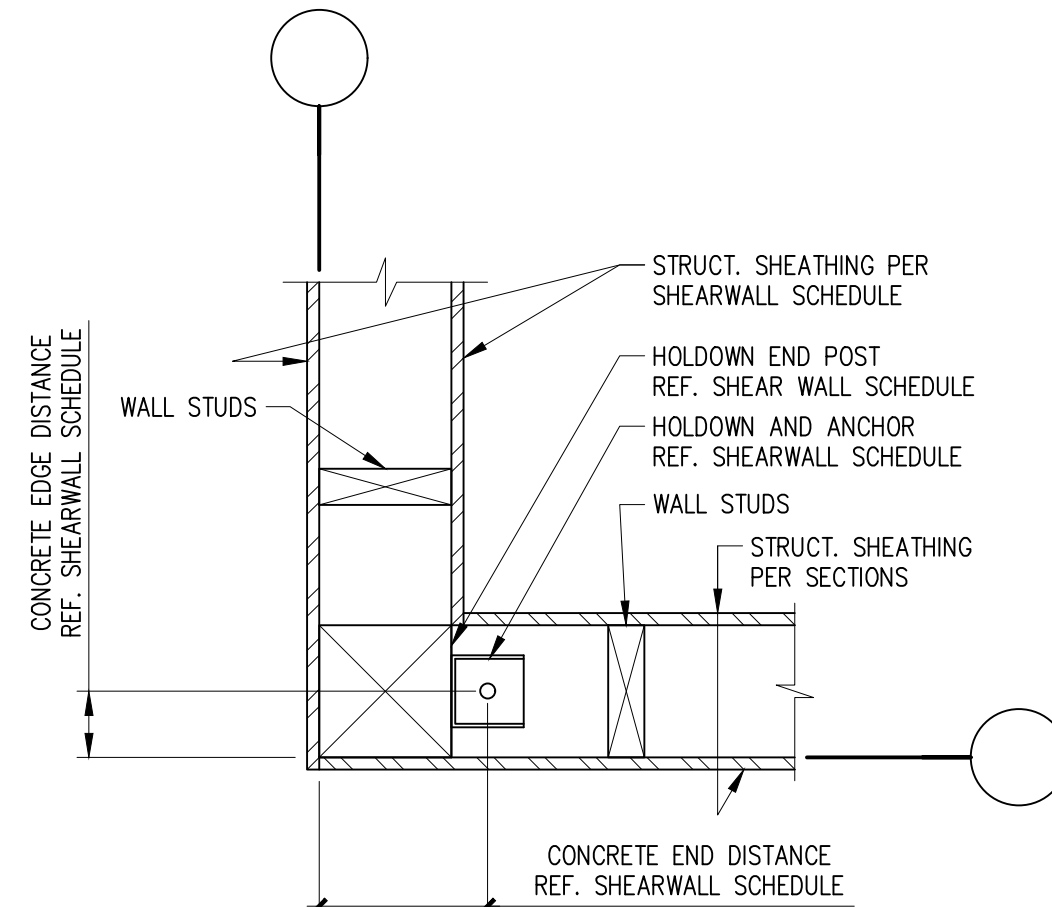
**01 TYPICAL GRADE BEAM DETAIL**  
S3.01 SCALE: 3/4"=1'-0"



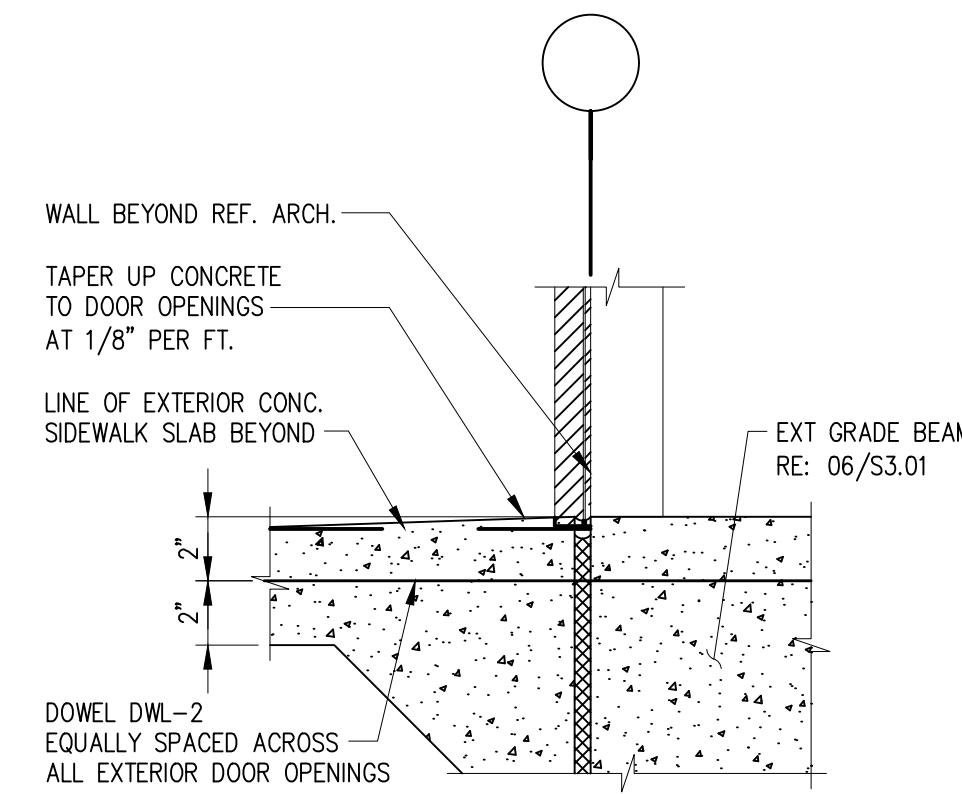
**02 TYPICAL GRADE BEAM CONSTRUCTION JOINT DETAIL**  
S3.01 SCALE: 3/4"=1'-0"



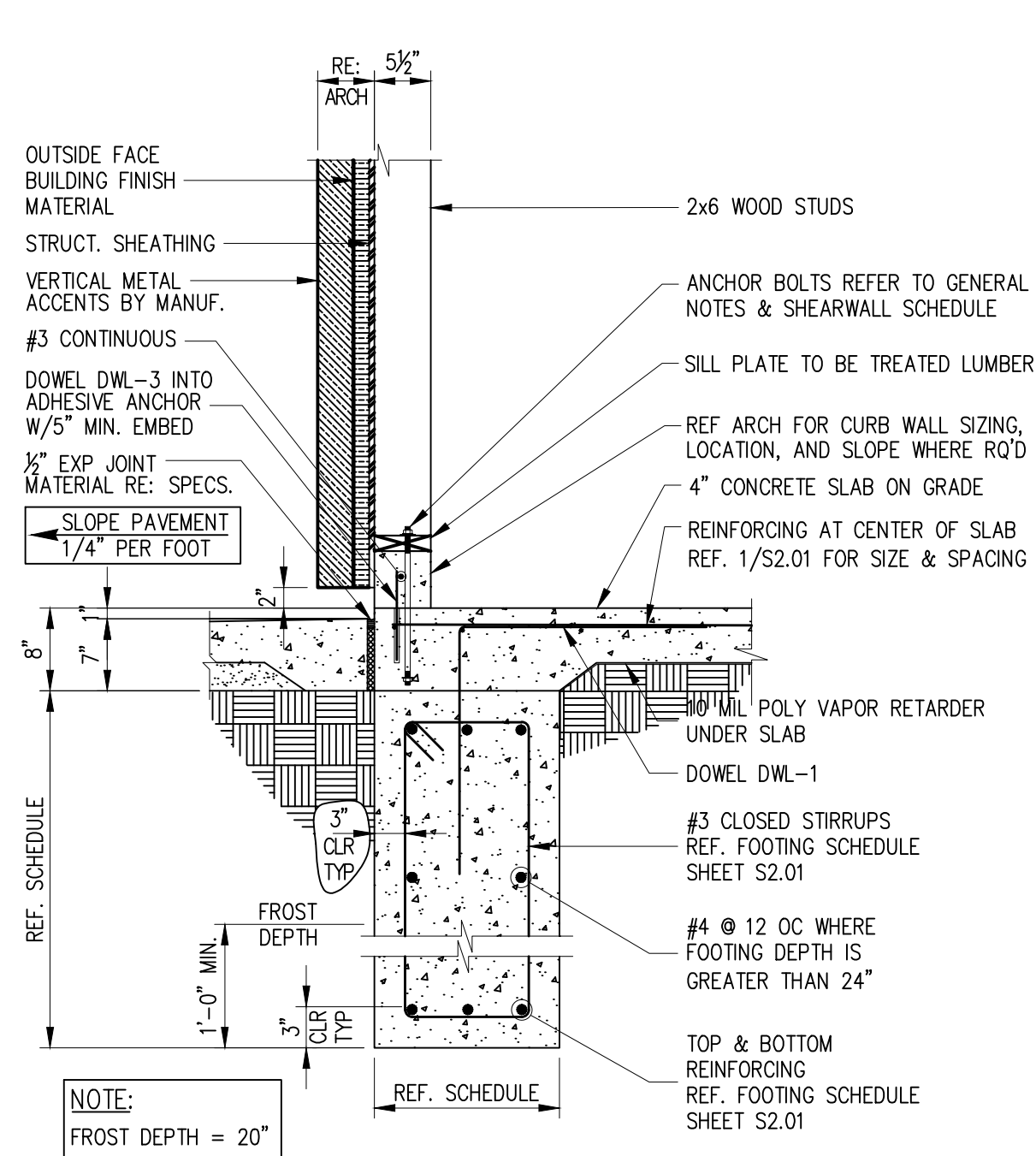
**03 TYPICAL HOLDOWN DETAILS**  
S3.01 SCALE: 1 1/2"=1'-0"



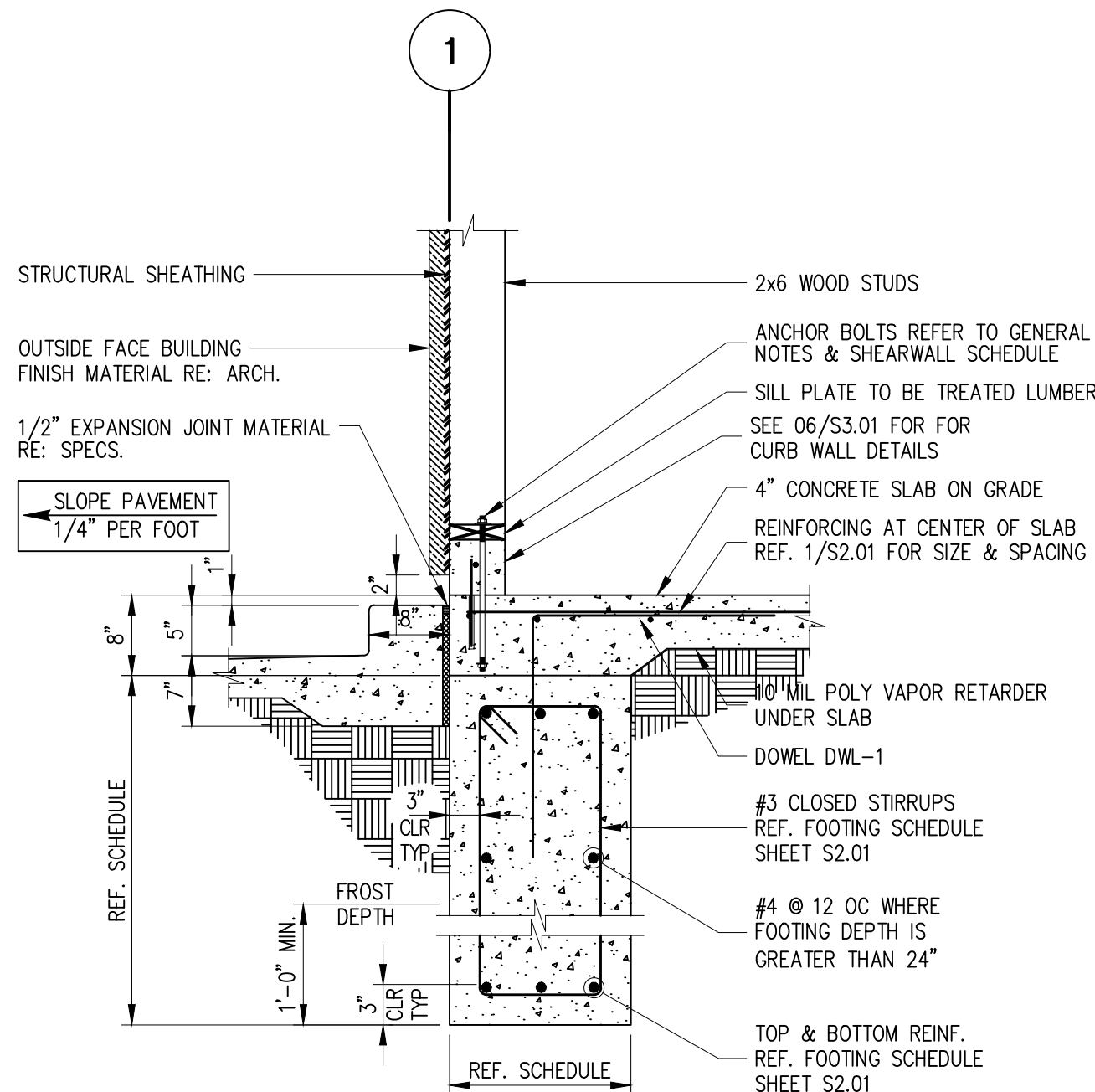
**04 TYPICAL HOLDOWN DETAILS**  
S3.01 SCALE: 1 1/2"=1'-0"



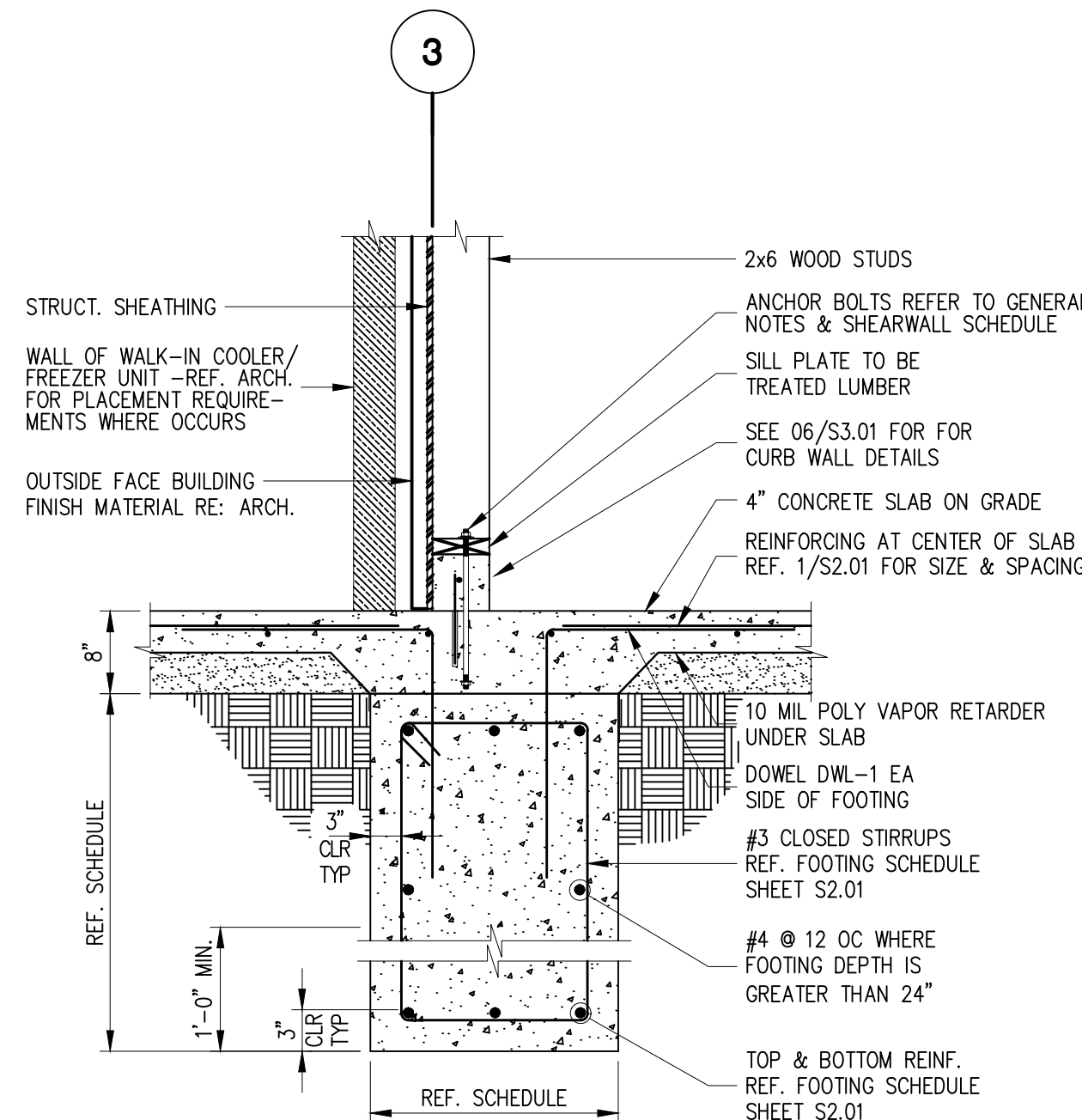
**05 TYPICAL DOOR OPENING DETAIL**  
S3.01 SCALE: 1 1/2"=1'-0"



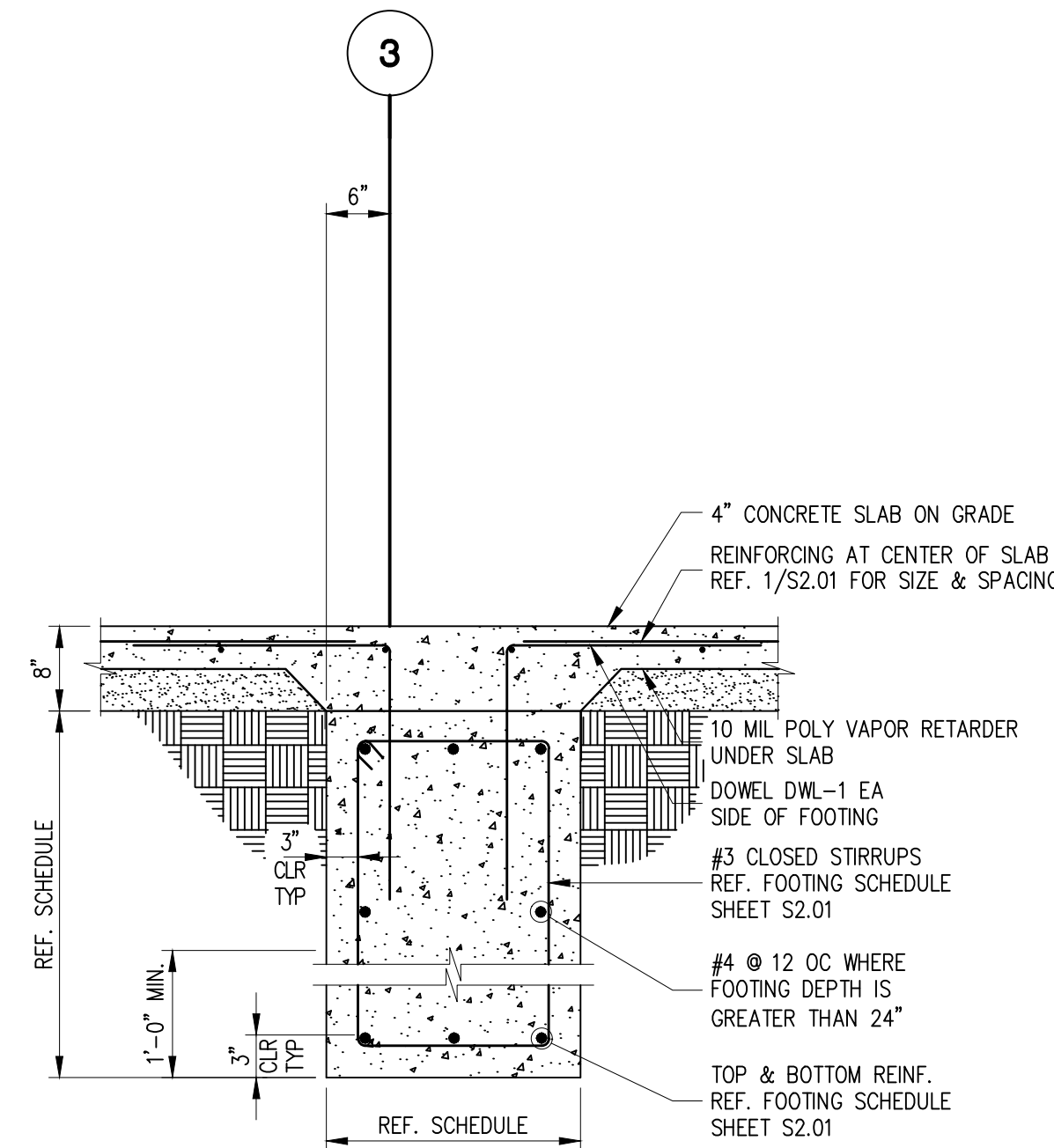
**06 TYPICAL FOUNDATION SECTION**  
S3.01 SCALE: 3/4"=1'-0"



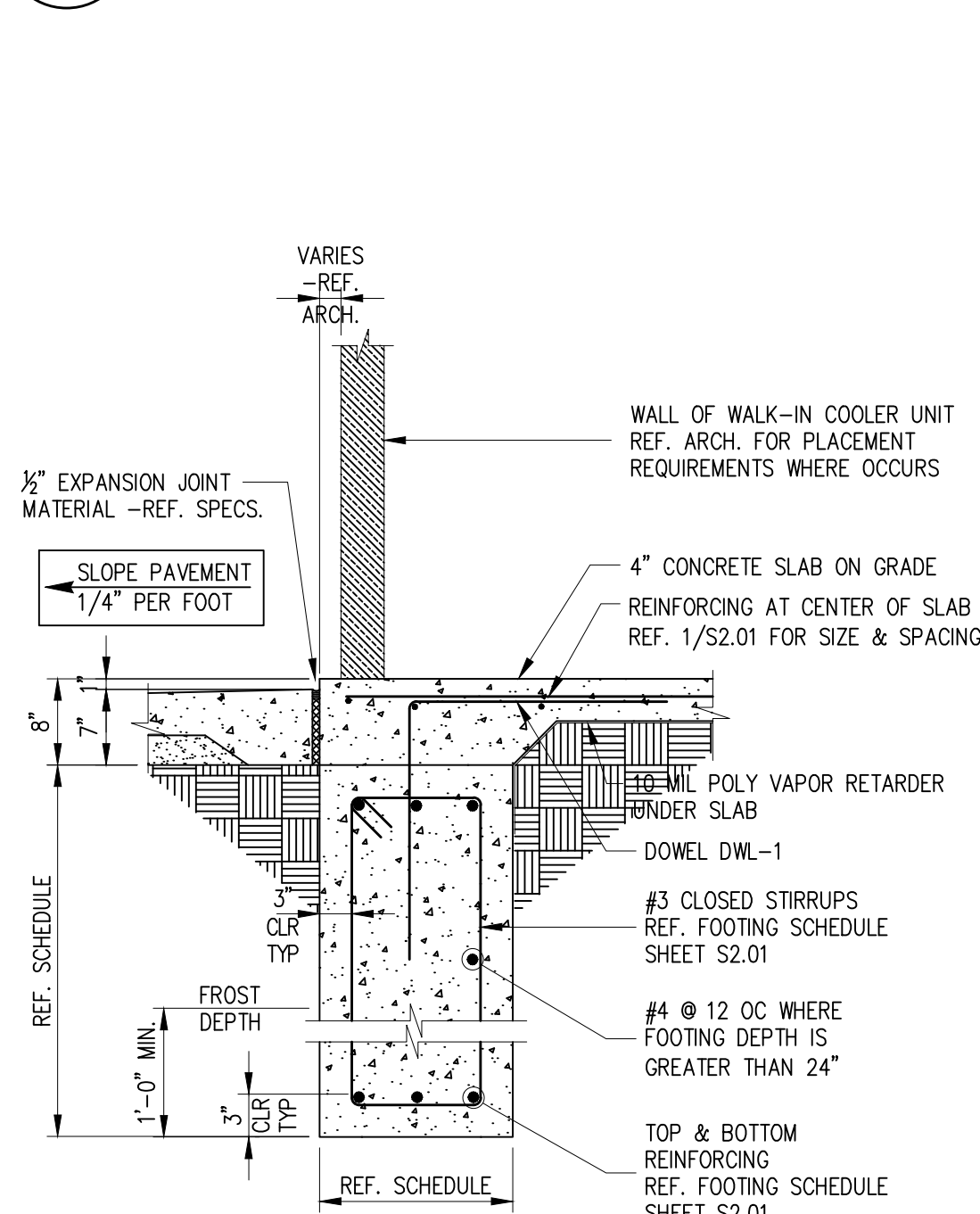
**07 TYPICAL FOUNDATION SECTION AT DRIVE THRU**  
S3.01 SCALE: 3/4"=1'-0"



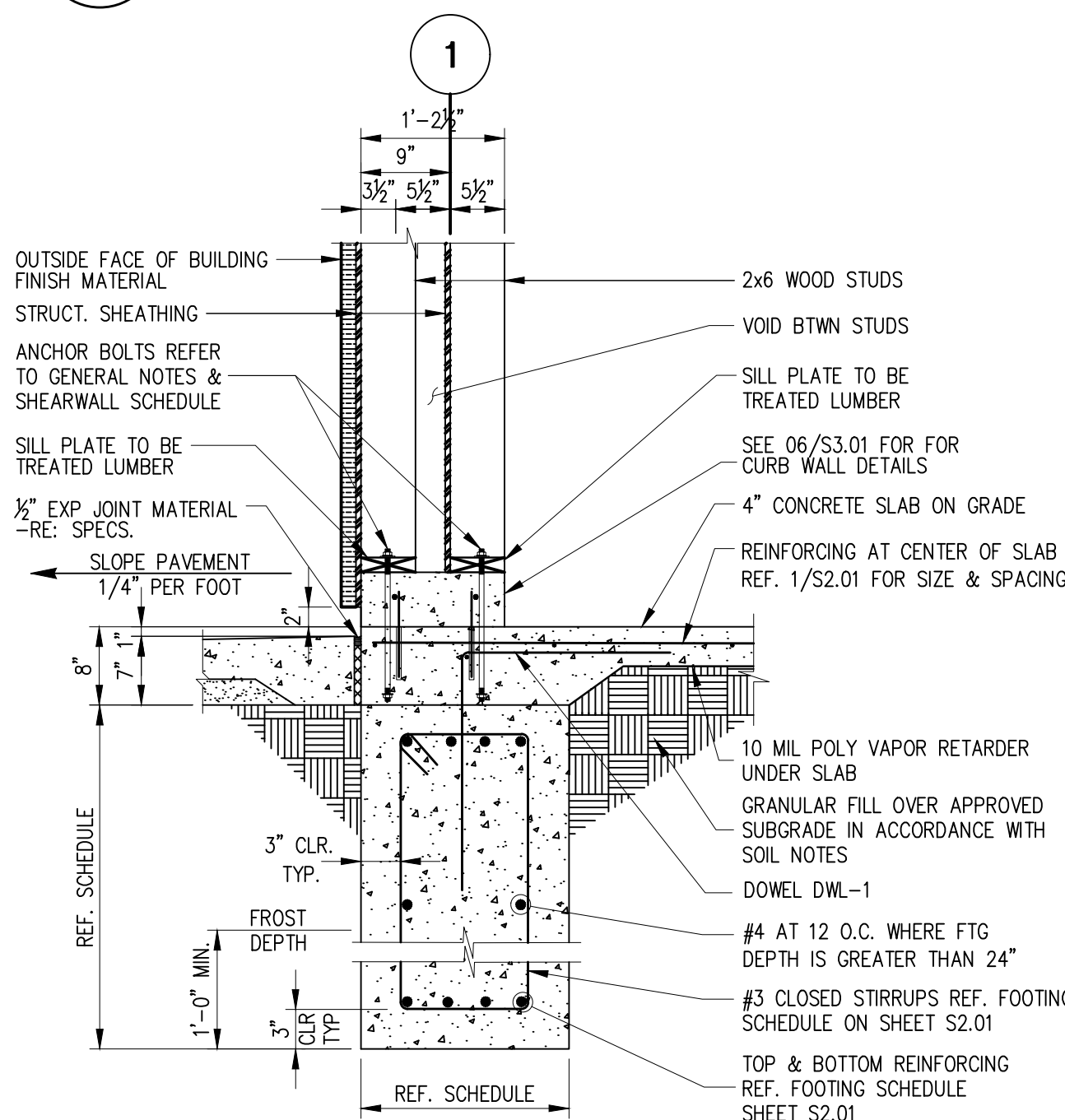
**08 FOUNDATION SECTION AT WALK-IN FREEZER/COLLER**  
S3.01 SCALE: 3/4"=1'-0"



**09 FOUNDATION SECTION AT OPENING TO WALK-IN FREEZER/COLLER**  
S3.01 SCALE: 3/4"=1'-0"

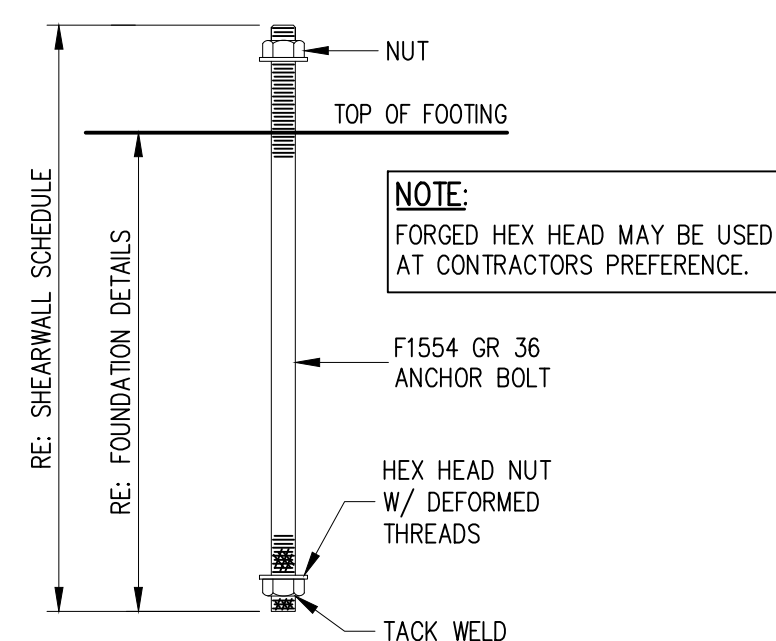


**10 TYPICAL FOUNDATION SECTION AT WALK-IN COOLER EXTERIOR WALL**  
S3.01 SCALE: 3/4"=1'-0"



**11 FOUNDATION SECTION AT FRONT BUMP-OUT**  
S3.01 SCALE: 3/4"=1'-0"

DOWEL SCHEDULE						A	
MARK	SIZE	SPACING	TYPE	A	B	TYPE 1	B
DWL-1	#4	16"	2	2'-0"	2'-0"		
DWL-2	#4	12"	1	5'-0"		TYPE 2	
DWL-3	#4	16"	1	8"			

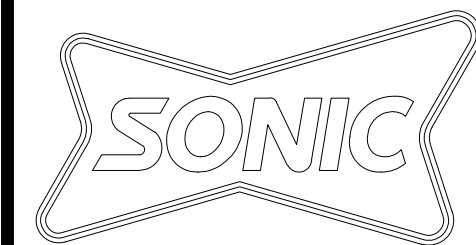


**12 TYP. HOLDOWN AND ANCHOR BOLT DETAIL**  
S3.01 SCALE: N.T.S.

CONSULTANT:

**national restaurant designers**  
**ARCHITECTS & ENGINEERS**  
7208 ACC Blvd. 2nd Floor, Raleigh, NC 27617  
ph: 919 544 0087 fax: 919 544 9399  
A Division of LMHT Associates

CLIENT:



SONIC CORP.  
300 JOHNNY BENCH DR  
OKLAHOMA CITY, OK 73104  
OFFICES: 405-225-5000  
FAX: 405-225-5991

PROJECT INFORMATION:

**SONIC**  
SONIC DRIVE-IN RESTAURANT  
LOT #3 GATEWAY CROSSING CENTURION WAY  
LAKE CITY, FLORIDA  
STORE # -

SEAL:



PROJECT NO.: 22161

DRAWN BY: DS

CHECKED BY: RC

ISSUE: DATE:

DISTRIBUTION SET 2022-06-24

REVISION: DATE:

PROJECT LOCATION:

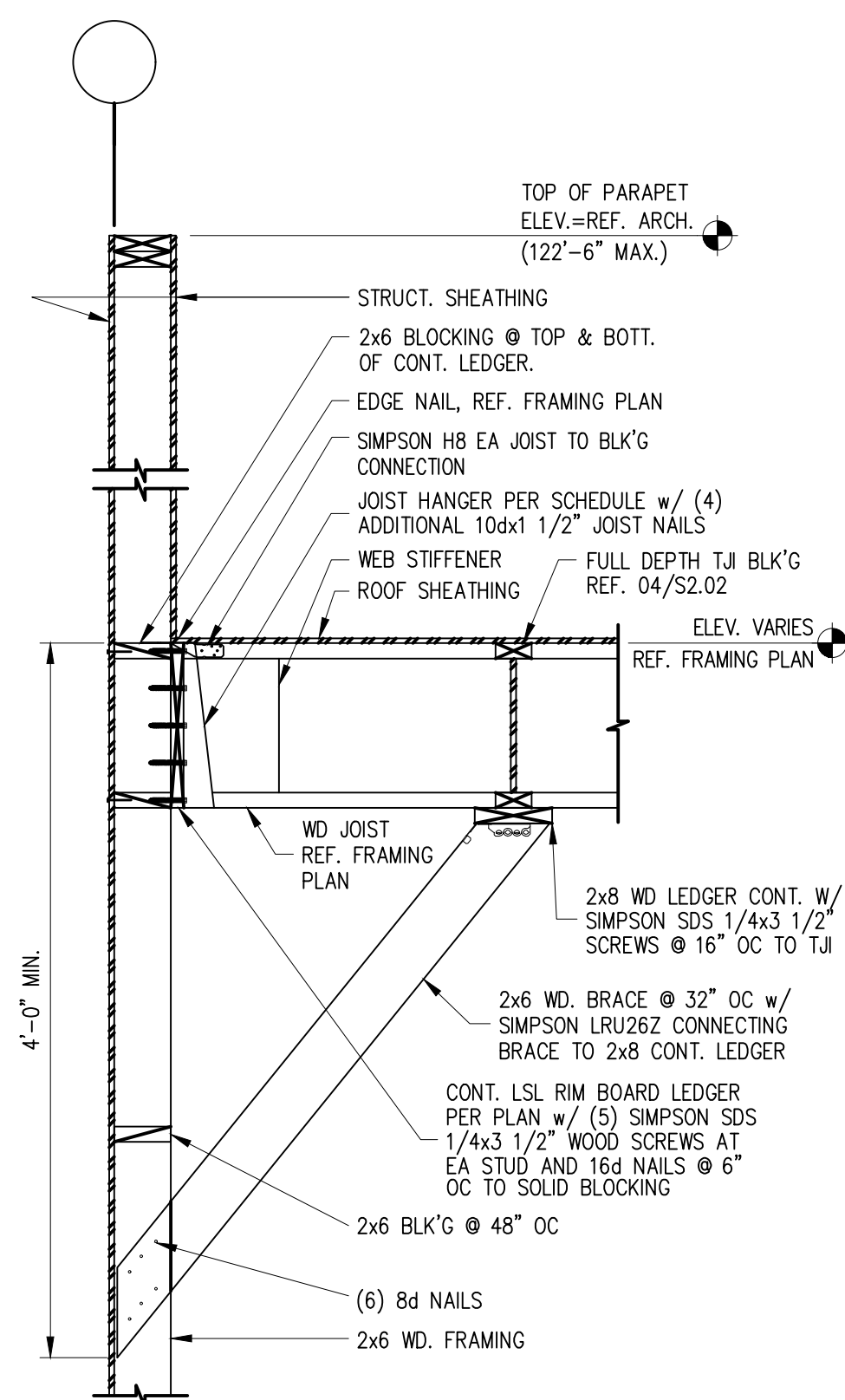
LAKE CITY, FLORIDA

SHEET NUMBER / TITLE:

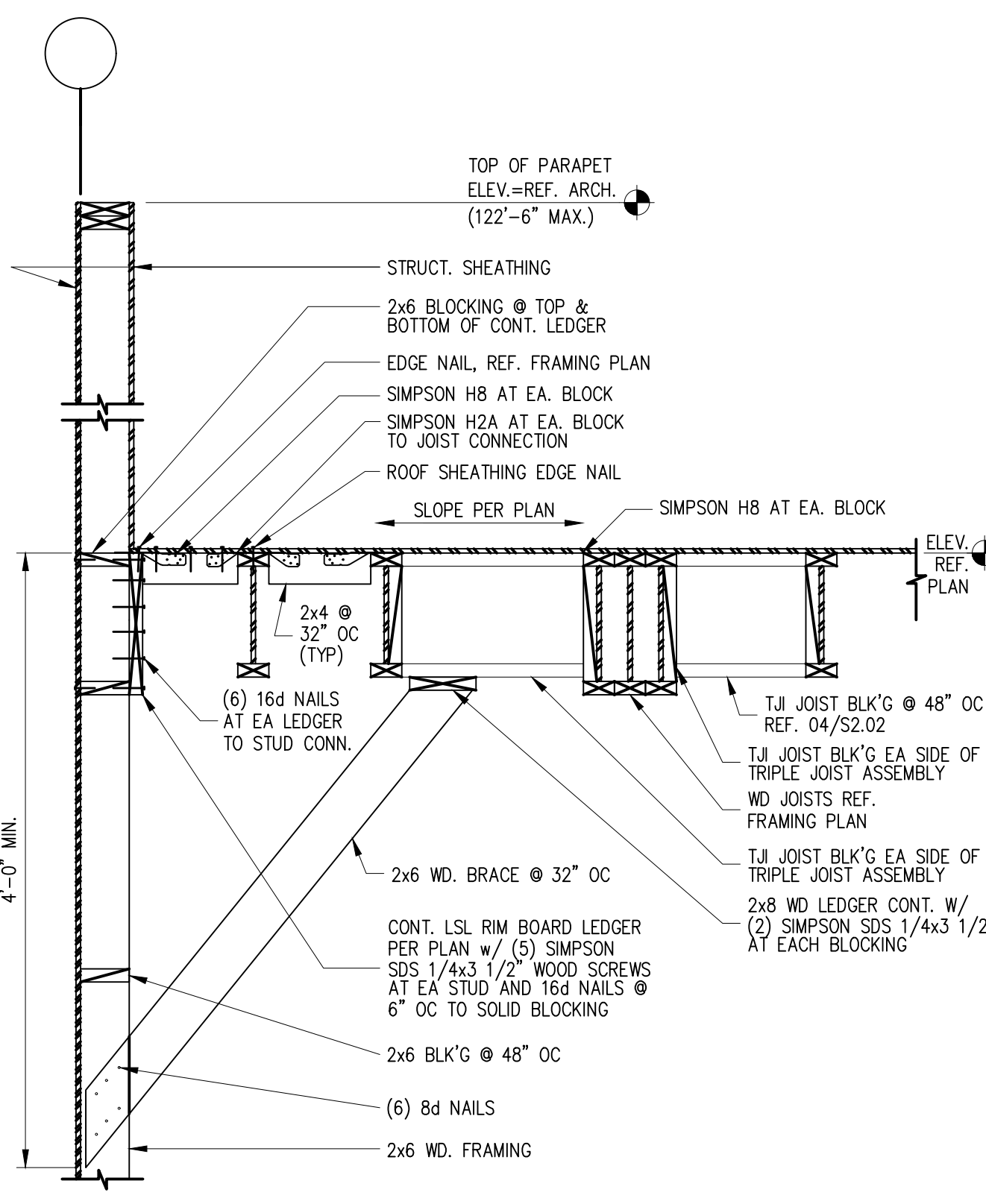
**S3.01**  
FOUNDATION & FRAMING DETAILS

**BRITT PETERS**  
AND  
ASSOCIATES  
INC.  
consulting engineers  
999 Waterside Drive  
Suite 2202  
Norfolk, VA 23510  
(757) 965-5710  
BPA Job No. 220603  
FL COA #27497

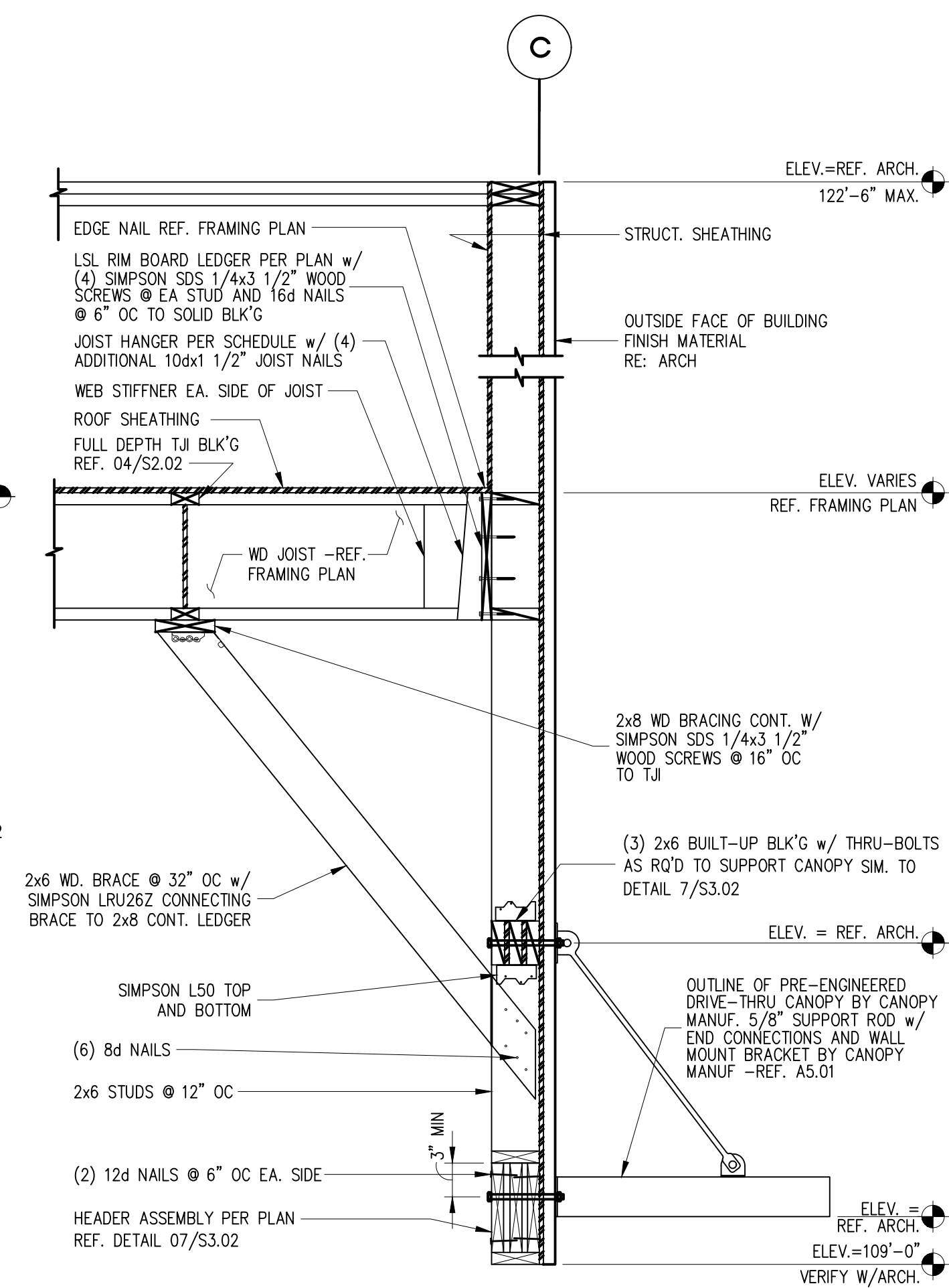




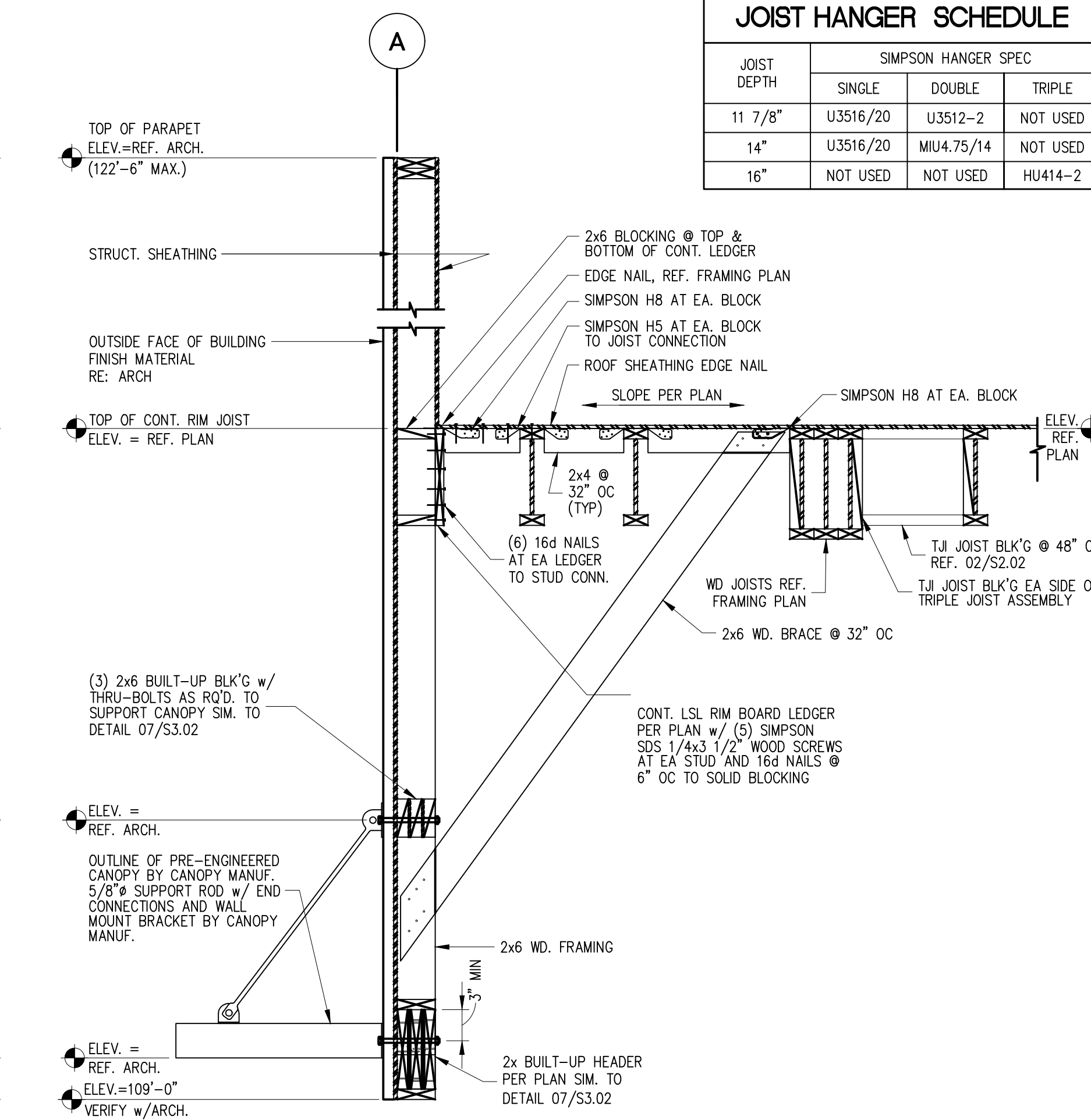
**01 TYPICAL JOIST/WALL SECTION**  
S3.02 SCALE: 3/4"=1'-0"



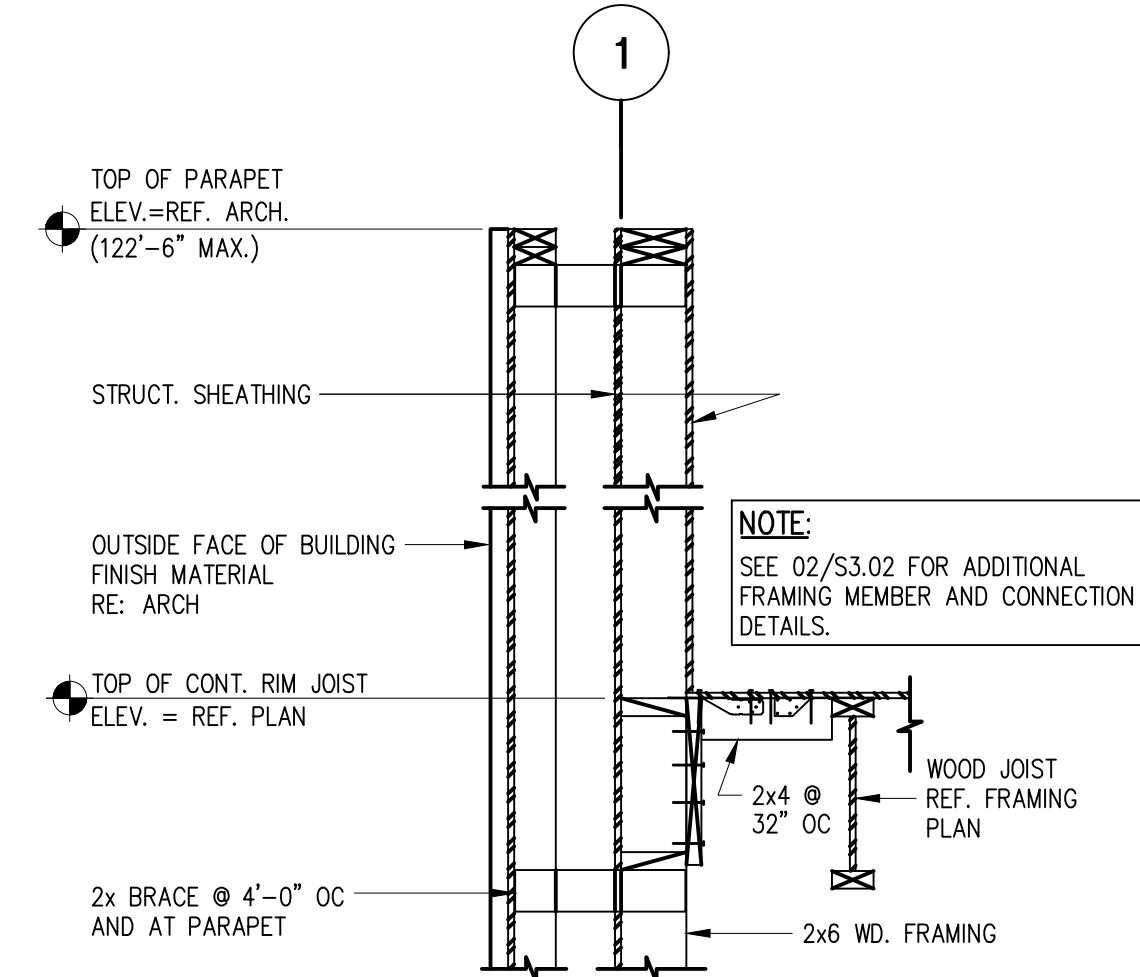
**02 TYPICAL END WALL SECTION**  
S3.02 SCALE: 3/4"=1'-0"



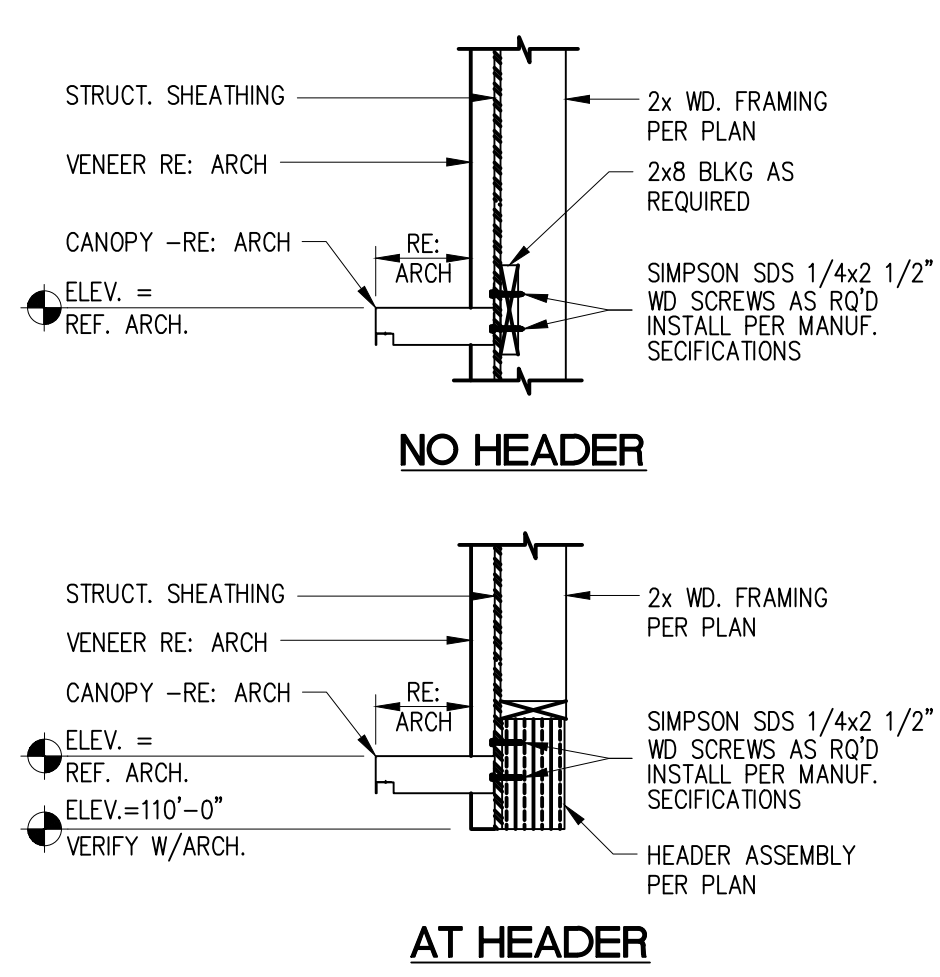
**03 SECTION AT DRIVE-THRU**  
S3.02 SCALE: 3/4"=1'-0"



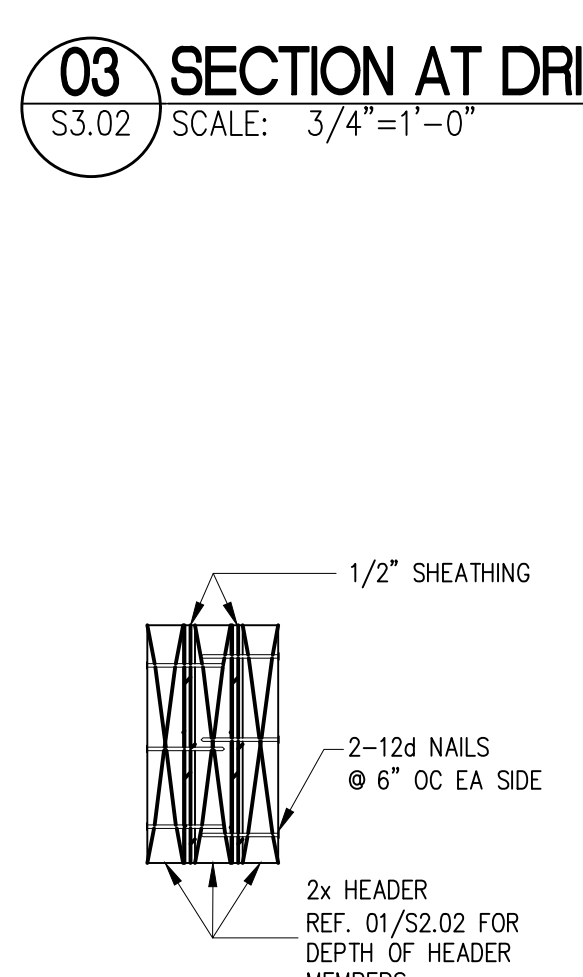
**04 SECTION AT FRONT OF BLDG**  
S3.02 SCALE: 3/4"=1'-0"



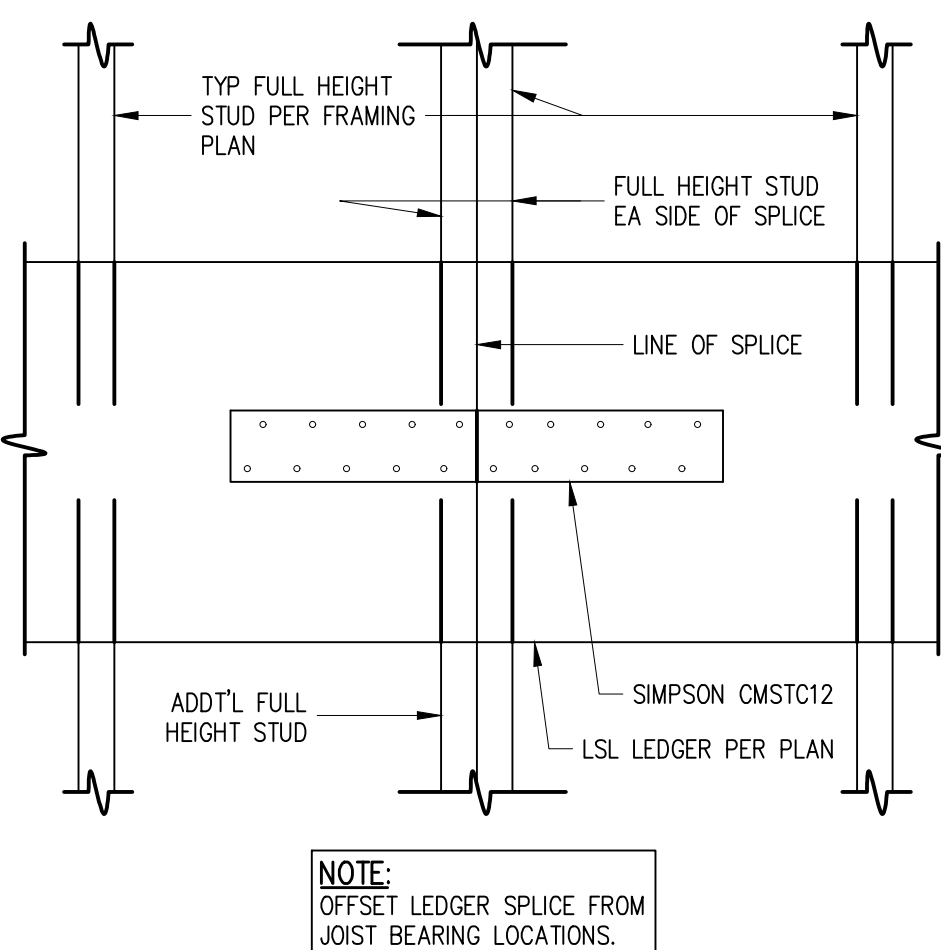
**05 SECTION AT BUMP-OUT, FRONT OF BUILDING**  
S3.02 SCALE: 3/4"=1'-0"



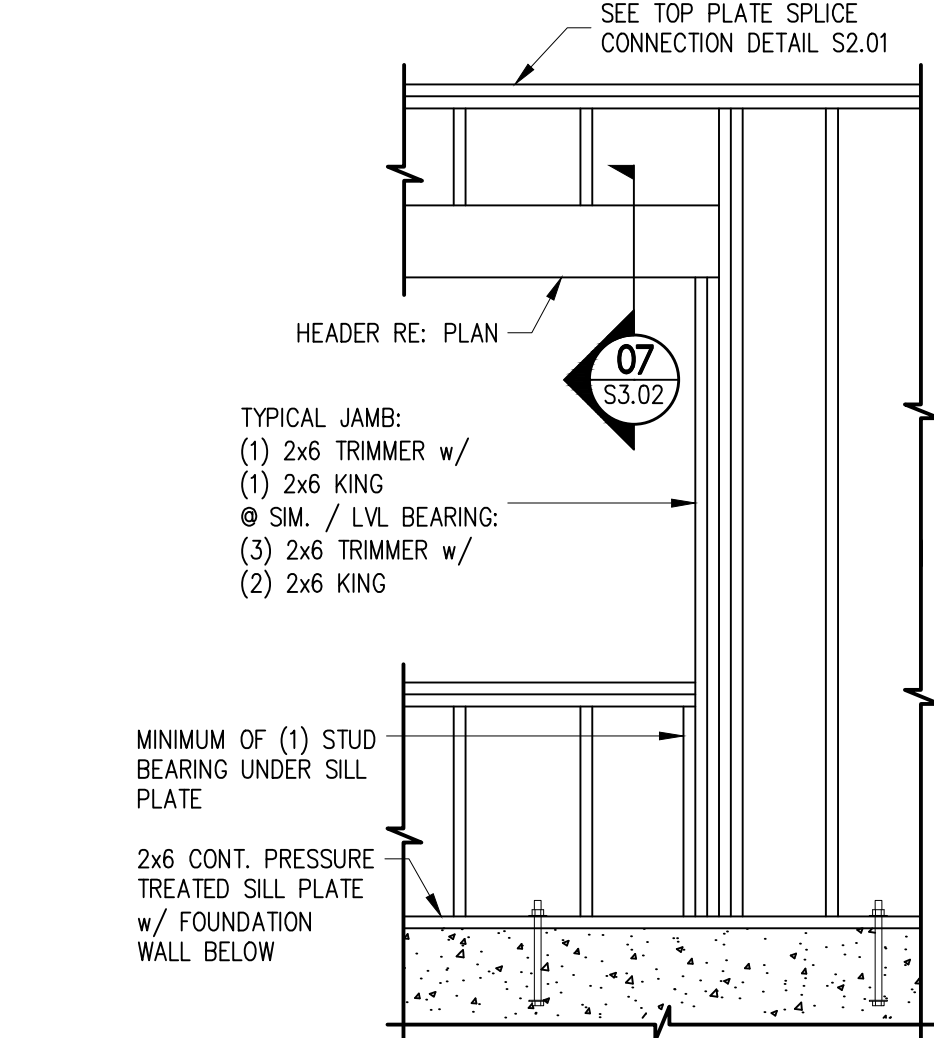
**06 TYPICAL SECTION AT CANOPY ACCENT**  
S3.02 SCALE: 3/4"=1'-0"



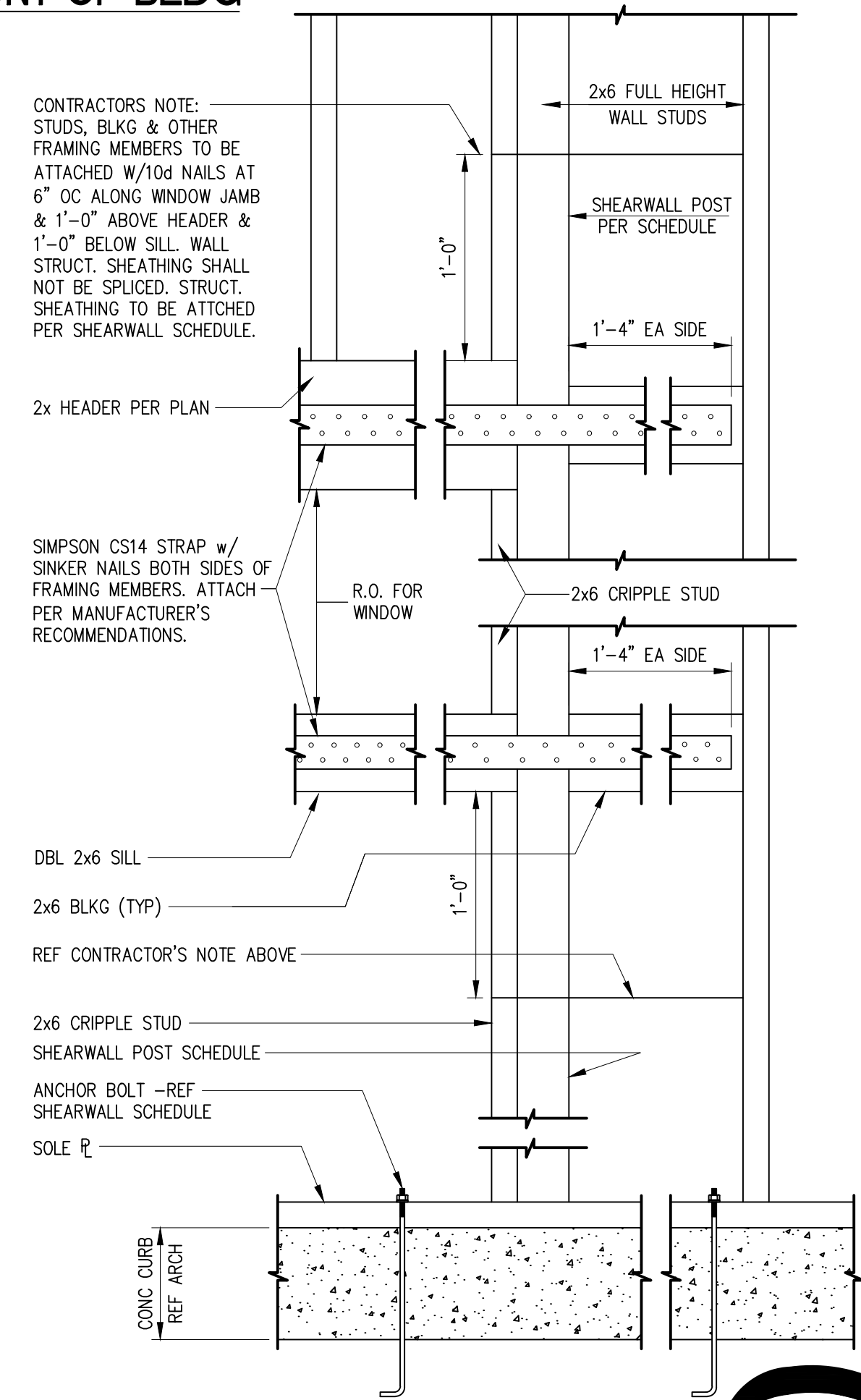
**07 TYP. 2x BUILT-UP HEADER DETAIL**  
S3.02 SCALE: 1 1/2"=1'-0"



**08 TYP. LEDGER SPLICE DETAIL**  
S3.02 SCALE: 1 1/2"=1'-0"



**09 TYPICAL JAMB CONSTRUCTION DETAIL**  
S3.02 SCALE: N.T.S.



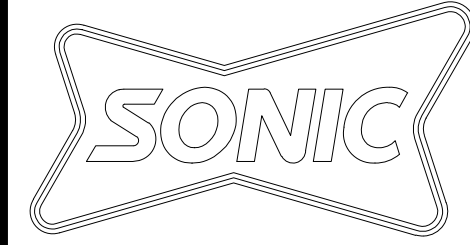
**09 ELEVATION**  
S3.02 SCALE: 1 1/2"=1'-0"

JOIST DEPTH	SIMPSON HANGER SPEC		
	SINGLE	DOUBLE	TRIPLE
11 7/8"	U3516/20	U3512-2	NOT USED
14"	U3516/20	MU4.75/14	NOT USED
16"	NOT USED	NOT USED	HU414-2

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PROJECT INFORMATION:

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LOT #3 GATEWAY CROSSING CENTURION WAY  
LAKE CITY, FLORIDA  
STORE # -

SEAL:



PROJECT NO.: 22161  
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CHECKED BY: RC

ISSUE: DATE:  
DISTRIBUTION SET 2022-06-24

REVISION: DATE:

PROJECT LOCATION:  
LAKE CITY, FLORIDA

SHEET NUMBER / TITLE:  
**S3.02**  
ROOF FRAMING DETAILS

**BRITT PETERS**  
AND  
ASSOCIATES  
INC.  
consulting engineers  
999 Waterside Drive  
Suite 2202  
Norfolk, VA 23510  
(757) 965-5710  
BPA Job No. 220603  
FL COA #27497



GENERAL NOTES:

- ALL DIMENSIONS TO, OF, AND IN EXISTING STRUCTURES SHALL BE VERIFIED IN FIELD BY CONTRACTOR WITH ALL DISCREPANCIES REPORTED TO THE ENGINEER.
- DO NOT CHANGE THE SIZE NOR SPACING OF STRUCTURAL ELEMENTS WITHOUT THE APPROVAL OF THE ENGINEER.
- THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.
- THE DESIGN IS BASED ON THE FLORIDA BUILDING CODE, 2020 EDITION.
- CONTRACTOR SHALL DETERMINE EXACT LOCATION OF EXISTING UTILITIES BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE UNDERGROUND UTILITIES.
- INCORRECTLY FABRICATED, DAMAGED, OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE OWNER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER'S APPROVAL.

GENERAL CANOPY NOTES:

- CANOPIES ARE TO REMAIN LEVEL WITH BUILDING.
- ALL WELDING ON ARCHED CANOPY IS TO BE PERFORMED BY THE CANOPY MANUFACTURER WITHIN THEIR FABRICATION FACILITY.
- DO NOT SLOPE CANOPIES WITH GRADE.
- SEE CIVIL OR SITE PLANS FOR LOCATION DIMENSIONS.

FOUNDATION AND SOIL PREPARATION NOTES:

- THE FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING PRESSURE OF 2,000 PSF AS PROVIDED IN THE SOIL PREPARATION NOTES S1.01.
- ALL PIERS SHALL BE EXCAVATED AND CONCRETE SHALL BE PLACED THE SAME DAY OR THE DRILLED SHAFT FOR THE PIER IS TO BE RE-EXCAVATED PER THE INSTRUCTIONS OF THIS OFFICE.

DRILLED SHAFT (PIER) NOTES:

- THE BOTTOM OF THE SHAFT SHALL BE CLEAN AND FREE OF LOOSE SOIL PRIOR TO PLACING CONCRETE.
- CONCRETE FOR DRILLED SHAFTS SHALL HAVE A 28 DAY DESIGN COMPRESSIVE STRENGTH OF 3,000 PSI, A MINIMUM OF 470 POUNDS OF PORTLAND CEMENT PER CUBIC YARD, MINIMUM 4-6% TOTAL AIR CONTENT (ENTRAINED), MID OR HIGH RANGE WATER REDUCING AGENT AND A 6-7" INCH SLUMP. AIR ENTRAINING AGENT OR FLASH SHALL NOT BE USED WITHOUT THE APPROVAL OF THE ENGINEER PRIOR TO BIDDING.
- NO SPUCING OF THE DRILLED SHAFT STEEL SHALL BE ALLOWED.

CONCRETE NOTES:

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE STANDARDS "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318) AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301).
- REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO A.S.T.M. A-615 GRADE 60.

- REINFORCING BAR, BAR SUPPORTS, AND SPACERS SHALL BE DETAILED AND PROVIDED IN ACCORDANCE WITH A.C.I. DETAILING MANUAL. CHAIRS SHALL NOT BE PLACED FURTHER THAN 4 FEET APART.

- EPOXY ANCHORS, REBAR, OR THREADED RODS SHALL BE EITHER HILTI HIT HY-200 MAX ANCHORS OR SIMPSON AT-XP ANCHORS. INSTALL ACCORDING TO THE MANUFACTURER'S RECOMMENDATIONS. THIS INCLUDES CLEANING THE HOLE WITH AIR.

STEEL NOTES:

- STRUCTURAL STEEL FABRICATION AND ERECTION SHALL CONFORM TO THE A.I.S.C. MANUAL OF STEEL CONSTRUCTION.
- WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY.
- ANY CONNECTIONS WITHOUT WELD SYMBOLS SHALL BE AT A MINIMUM WELDED ALL AROUND WITH THE MINIMUM FILLET OR BUTT WELD SIZE.
- STRUCTURAL STEEL ANGLES, PLATES, ETC. SHALL CONFORM TO A.S.T.M. A36 REQUIREMENTS (36 KSI). STRUCTURAL TUBING AND PIPES SHALL CONFORM TO THE A.S.T.M. A500 GRADE B REQUIREMENTS (46 KSI); STRUCTURAL STEEL CHANNELS SHALL CONFORM A.S.T.M. A992 REQUIREMENTS (50 KSI).
- DO NOT PLACE HOLES THROUGH STRUCTURAL STEEL MEMBERS EXCEPT AS SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.
- ALL BOLTED CONNECTIONS ARE TO BE A325 TYPE N BOLTS IN STANDARD HOLES UNLESS NOTED OTHERWISE AND SHALL BE MADE USING THE TURN OF THE NUT METHOD.

BEAM SCHEDULE

NO.	FRAMING MEMBER SIZE	
	STANDARD	ALTERNATE
B1	HSS6x4x5/16"	NOT USED
B2	HSS6x4x3/16"	NOT USED
B3	L2x2x3/16"	
D1	SEE CONNECTION DETAIL 3/S5.01	

COLUMN SCHEDULE

COLUMN NUMBER	COLUMN SIZE
C1	HSS10x4x1/4"
C2	HSS6x4x1/4"

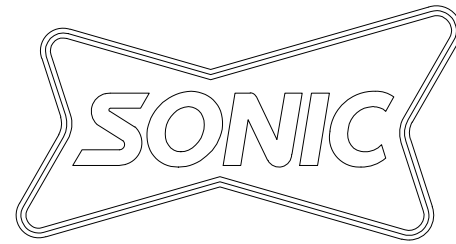
ENGINEERING DATA

ROOF LIVE LOAD: 20 PSF  
LIVE LOAD REDUCTION ON SUPPORTING ELEMENTS IN ACCORDANCE WITH 2020 IBC.  
ROOF DEAD LOAD: 5 PSF  
DESIGN DEAD LOADS INCLUDE THE WEIGHT OF STRUCTURAL COMPONENTS AND PERMANENT FIXTURES.  
GROUND SNOW LOAD:  $P_g = 0$  PSF  
FLAT ROOF SNOW LOAD:  $P_f = 0$  PSF  
SNOW EXPOSURE FACTOR:  $C_e = 1.0$   
SNOW LOAD IMPORTANCE FACTOR:  $I = 1.0$   
THERMAL FACTOR:  $C_t = 1.1$   
WIND DESIGN DATA:  
ULTIMATE DESIGN WIND SPEED (3 SECOND GUST): 120 MPH  
WIND EXPOSURE CATEGORY: C  
BUILDING CATEGORY: II - OPEN BUILDING  
INTERNAL PRESSURE COEFFICIENTS: 0  
ALL NEW COMPONENTS AND CLADDING NOT DESIGNED BY THE ENGINEER SHALL BE DESIGNED BASED ON CORRESPONDING ASCE 7-16 DESIGN LOADS UNLESS OTHERWISE APPROVED BY THE ENGINEER  
DESIGN UPLIFT - 25.0 PSF  
EARTHQUAKE DESIGN DATA:  
SEISMIC IMPORTANCE FACTOR,  $I_s = 1.0$   
MAPPED SPECTRAL RESPONSE ACCELERATIONS:  $S_s = 0.086$   $S_1 = 0.081$   
SITE CLASS: C  
SPECTRAL RESPONSE COEFFICIENTS -  $S_{DS} = 0.092$   $S_{D1} = 0.081$   
SEISMIC DESIGN CATEGORY: B  
BASIC SEISMIC FORCE-RESISTING SYSTEM: STEEL ORDINARY CANTILEVER COLUMN  
DESIGN BASE SHEAR:  $V = 2.204$  kips  
SEISMIC RESPONSE COEFFICIENT:  $C_s = 0.143$   
RESPONSE MODIFICATION FACTOR:  $R = 1.25$   
ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE METHOD  
FLOOD HAZARD INFORMATION: THIS BUILDING IS NOT DESIGNED FOR FLOOD LOADS.

CONSULTANT:

**national restaurant designers**  
**ARCHITECTS & ENGINEERS**  
7208 ACC Blvd. 2nd Floor, Raleigh, NC 27617  
ph: 919 544 0087 fax: 919 544 9399  
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CLIENT:



SONIC CORP.  
300 JOHNNY BENCH DR  
OKLAHOMA CITY, OK 73104  
OFFICES: 405-225-5000  
FAX: 405-225-5991

PROJECT INFORMATION:

**SONIC**  
SONIC DRIVE-IN RESTAURANT  
LOT #3 GATEWAY CROSSING CENTURION WAY  
LAKE CITY, FLORIDA  
STORE # -

SEAL:



07/13/22

PROJECT NO.: 22161

DRAWN BY: DS

CHECKED BY: RC

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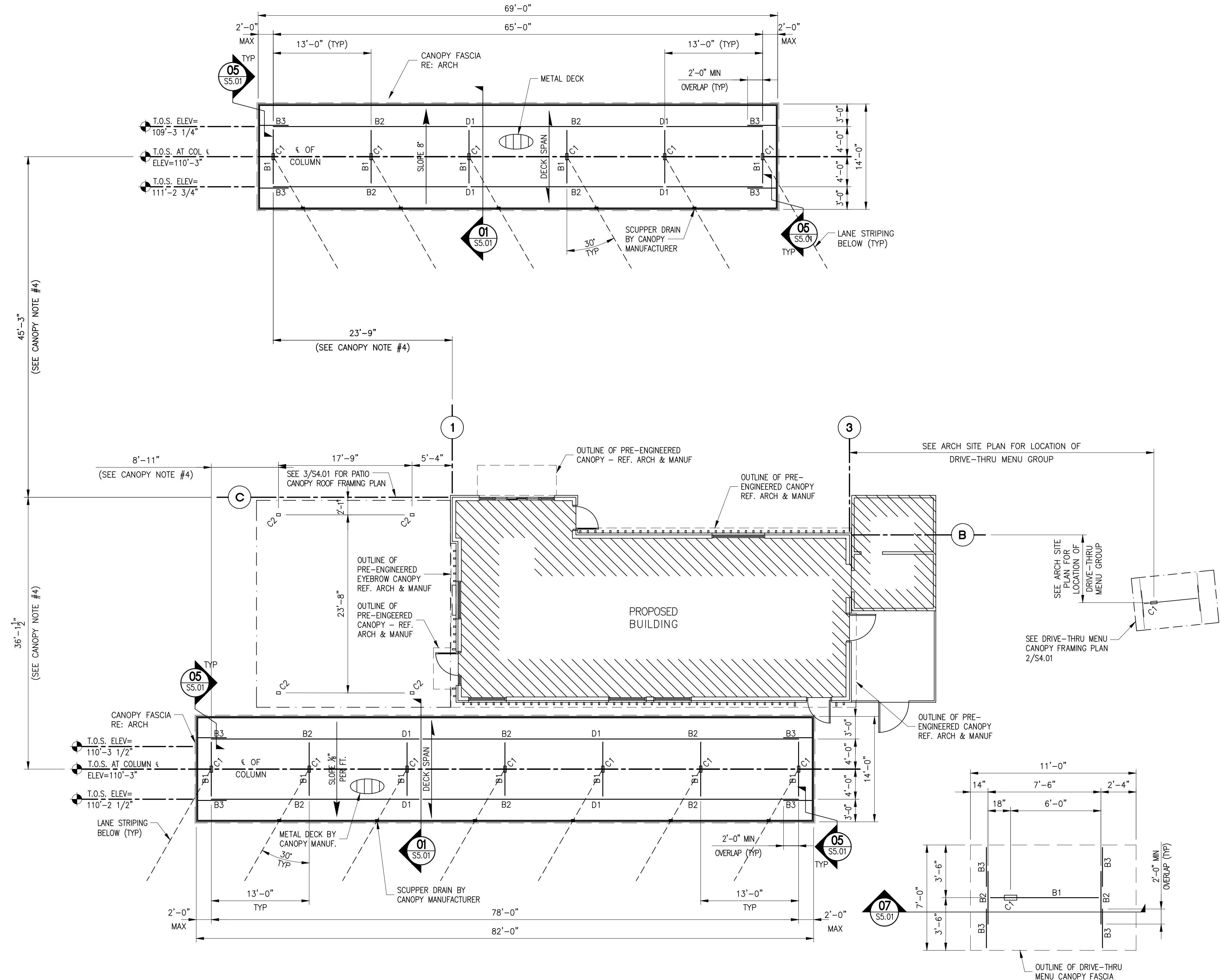
LAKE CITY, FLORIDA

COVERED PATIO

SHEET NUMBER / TITLE:

**S4.01**

CANOPY FRAMING PLAN



**1 CANOPY FRAMING PLAN**  
SCALE: 1/8"=1'-0"

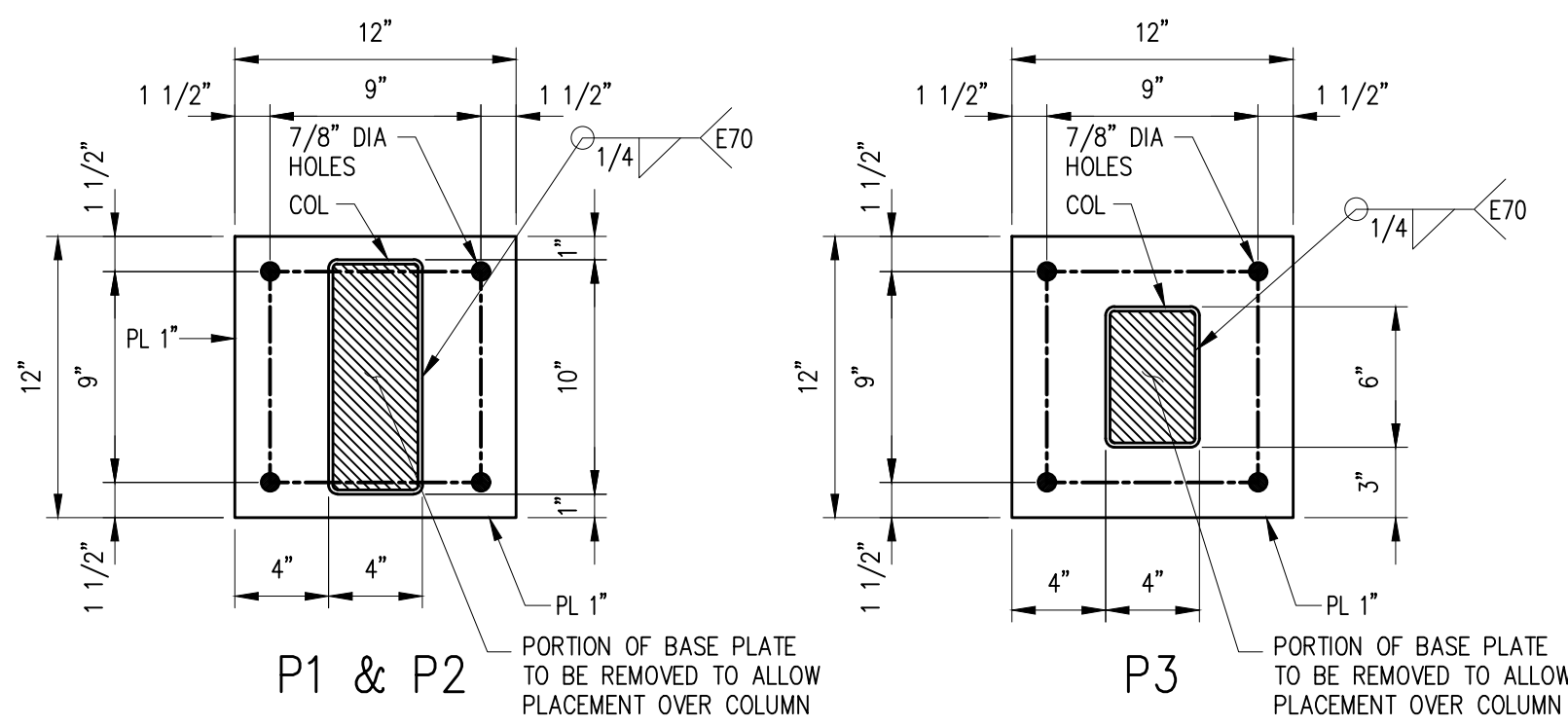
NOTE:  
VERIFY ALL DIMENSIONS WITH ARCHITECTURAL  
DRAWINGS AND EXISTING CONDITIONS.

**2 DRIVE-THRU MENU CANOPY FRAMING PLAN**  
SCALE: 1/4"=1'-0"

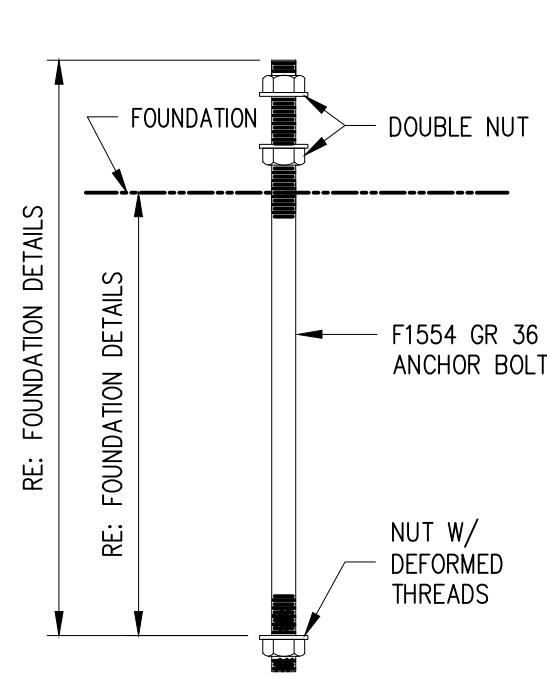
**3 PATIO CANOPY FRAMING PLAN**  
SCALE: 1/4"=1'-0"

**BRITT PETERS**  
AND  
ASSOCIATES  
INC.  
consulting engineers  
999 Waterside Drive  
Suite 2202  
Norfolk, VA 23510  
(757) 965-5710  
BPA Job No. 220603  
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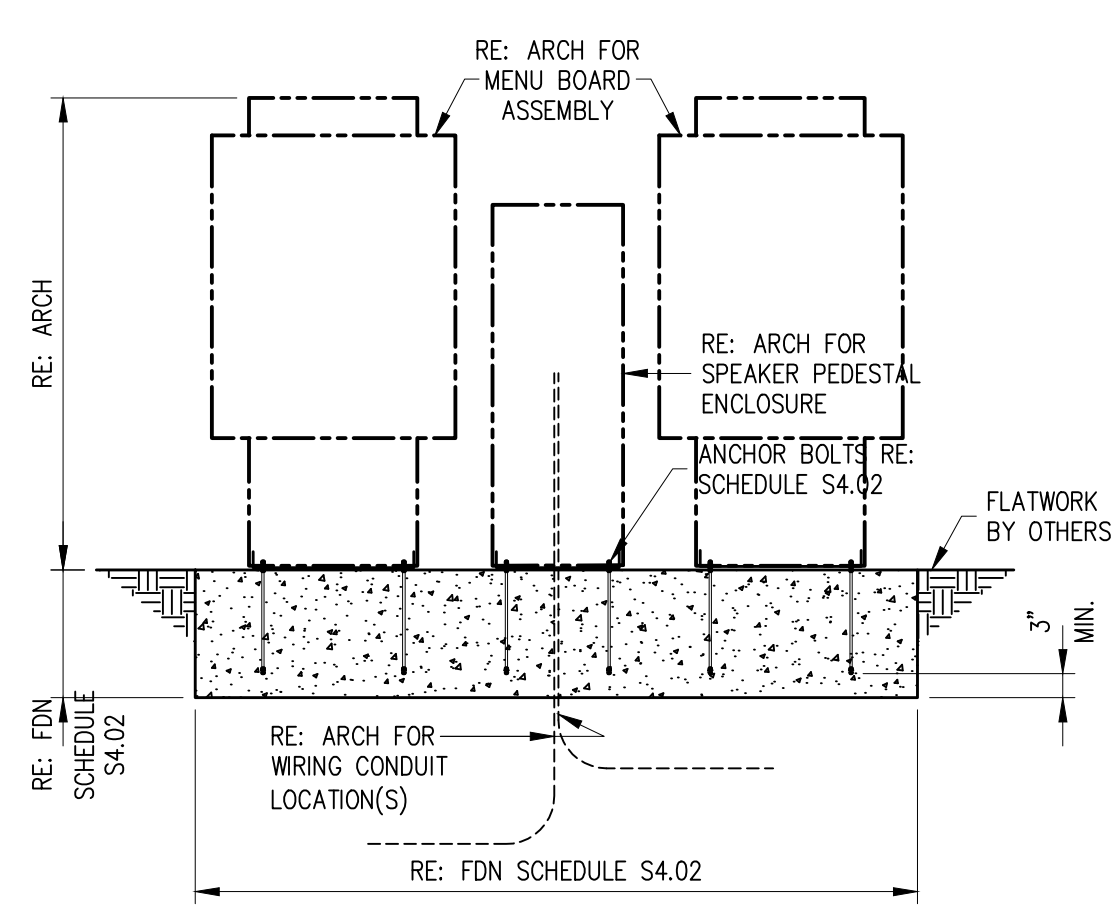




02 COLUMN BASE PLATE DETAILS  
S4.02 SCALE: 1 1/2"=1'-0"



03 TYP. ANCHOR BOLT DETAIL  
S4.02 SCALE: N.T.S.



04 DRIVE-THRU MENU BOARD AND  
POPS ASSEMBLY FDN DETAIL  
S4.02 SCALE: 1/2"=1'-0"

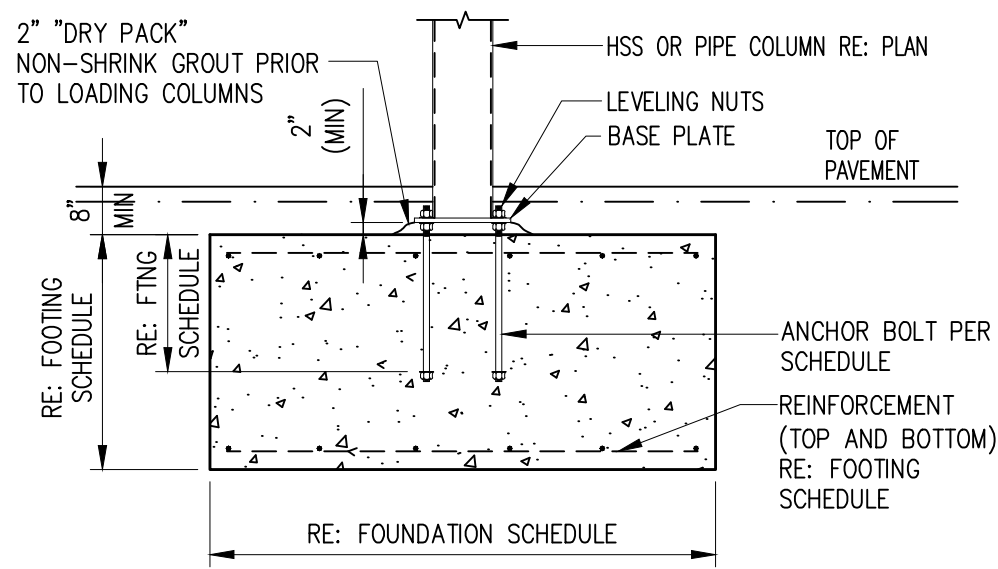
SPREAD FOOTINGS		
FOOTING NUMBER	VERTICAL REBAR	HORIZONTAL REBAR
P1	NOT USED	#6 @ 6" C/C EW TAB
P2	NOT USED	#6 @ 6" C/C EW TAB

CONCRETE PAD FOOTING		
F1	NOT USED	#6 @ 9" C/C EW TAB

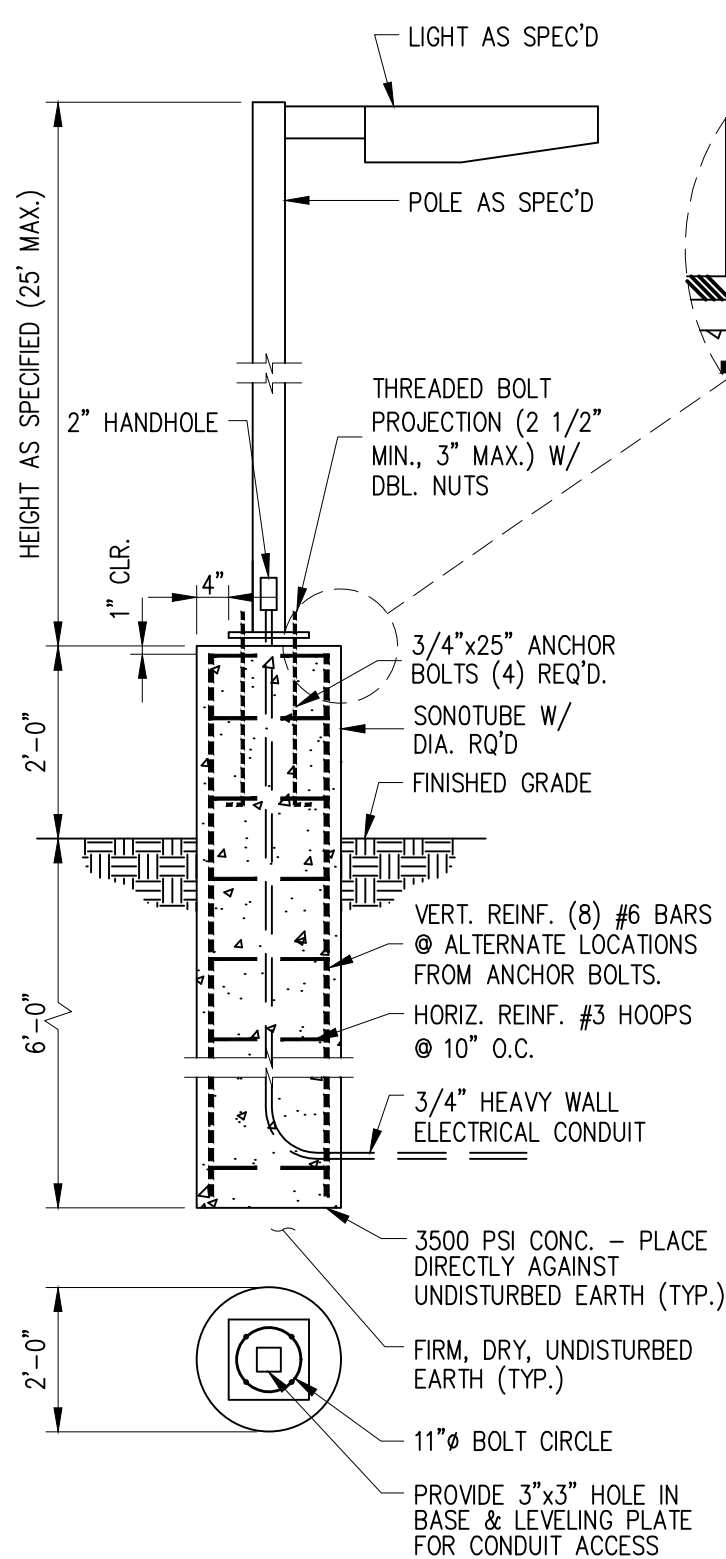
FOUNDATION SCHEDULE			
SPREAD FOOTINGS			
FOOTING NUMBER	WIDTH	LENGTH	DEPTH
P1	5'-6"	5'-6"	3'-0"
P2	2'-6"	2'-6"	3'-0"

CONCRETE PAD FOOTING			
F1	SEE PLAN	SEE PLAN	16"

ANCHOR BOLT SCHEDULE	
FOOTING NUMBER	ANCHOR BOLT
P1	3/4" DIA.x24" W/MIN. 18" EMBED
P2	3/4" DIA.x24" W/MIN. 18" EMBED
F1	3/8" DIA. x 6" STAINLESS STEEL BOLT W/MIN. 5" EMBED



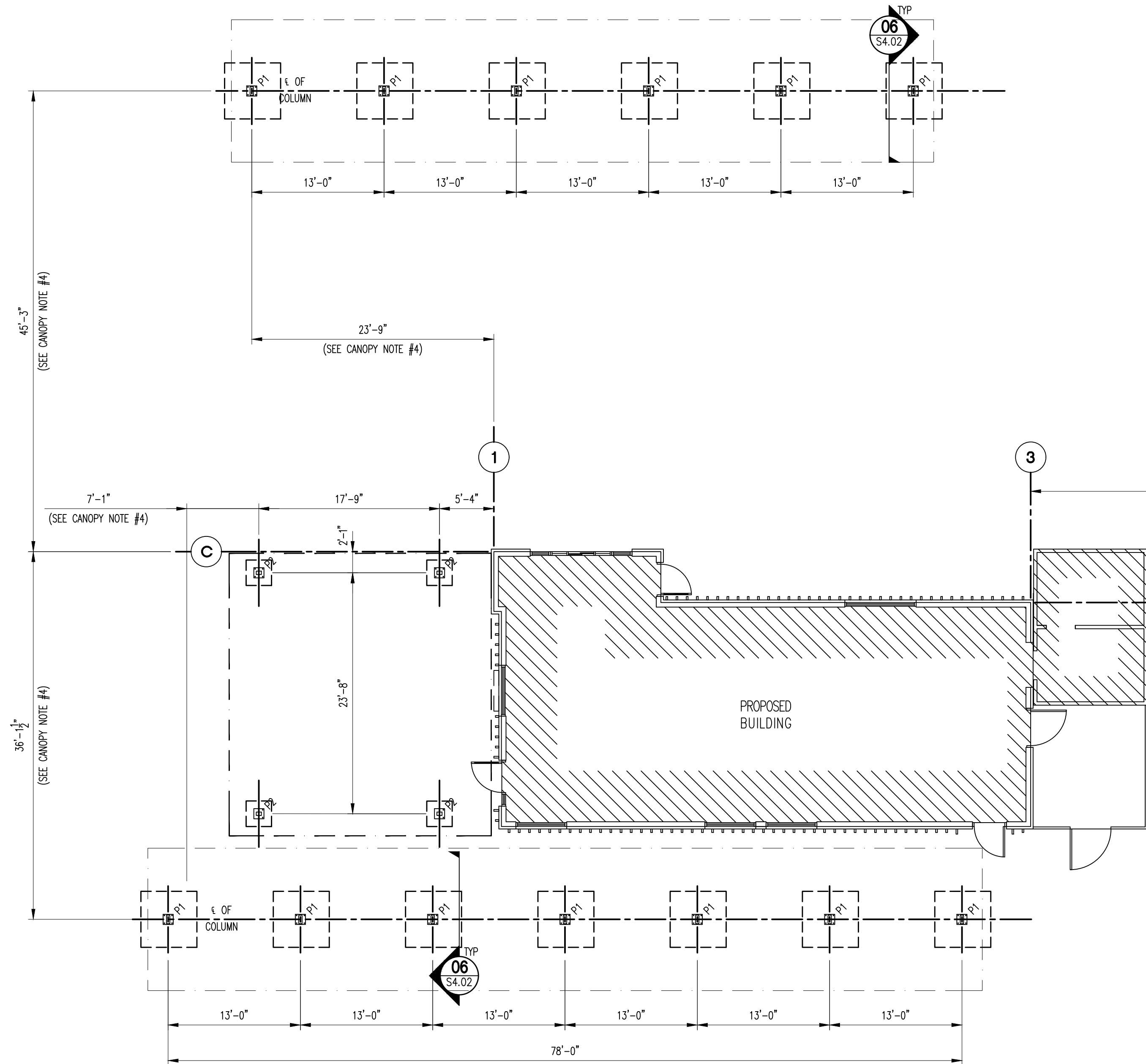
06 P1 / P2 FOUNDATION DETAIL  
S4.02 SCALE: 3/8"=1'-0"



07 STANDARD LIGHT BASE DETAIL  
S4.02 SCALE: 1/2"=1'-0"

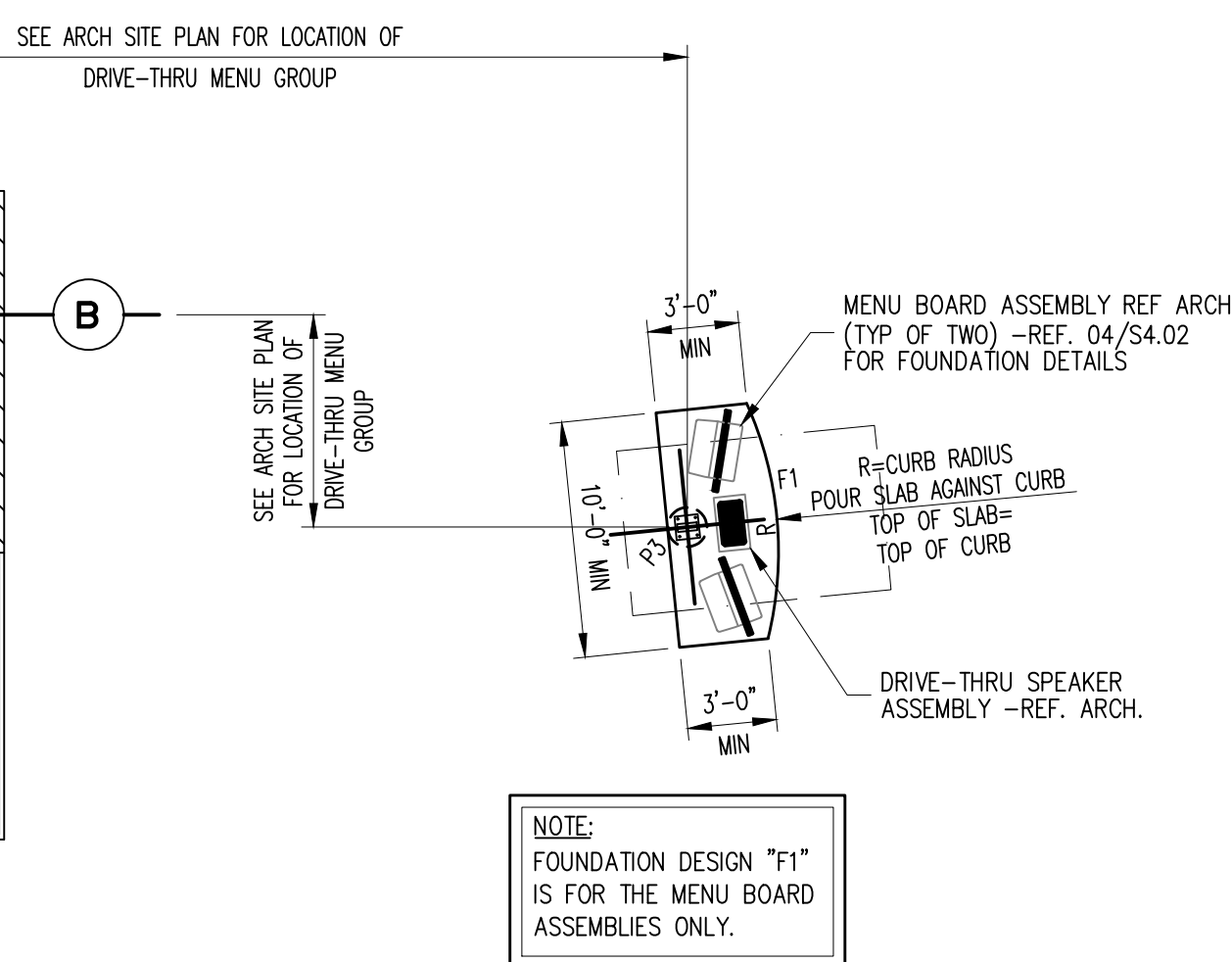
- NOTES:
- THIS FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING CAPACITY SPECIFIED WITHIN THE CORRESPONDING NOTES, S4.01.
  - THIS FOUNDATION IS BASED ON WIND LOADS SPECIFIED WITHIN THE ENGINEERING DESIGN LOADS, S4.01.
  - ALL REINFORCING STEEL SHALL CONFORM TO ASTM GRADE 60.
  - THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE BASE PLATE BOLT PATTERN.
  - WHEN ORDERING BLOCK FINISHES, ADDITIONAL LEAD TIME IS REQUIRED.

NOTE:  
REFER TO E1.01 FOR LOCATIONS OF BOTH STANDARD AND DECORATIVE LIGHT POLES



1 CANOPY FOUNDATION PLAN  
SCALE: 1/8"=1'-0"

NOTE:  
VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS.

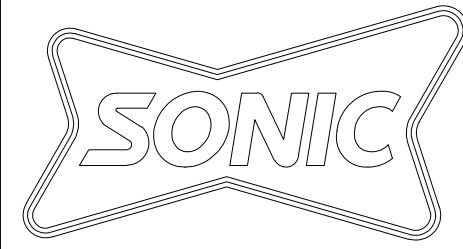


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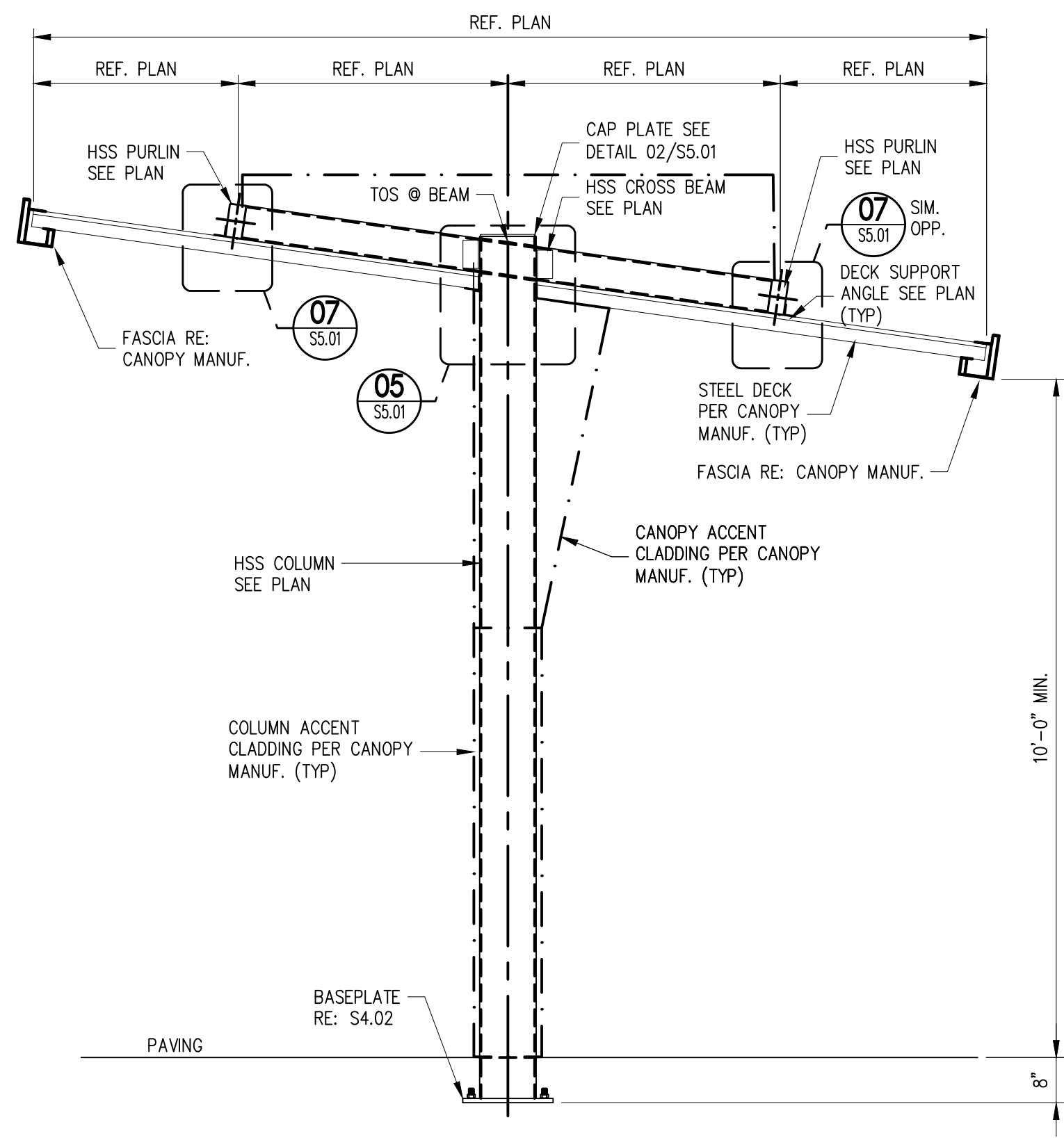
REVISION: DATE:

PROJECT LOCATION:  
LAKE CITY, FLORIDA  
COVERED PATIO

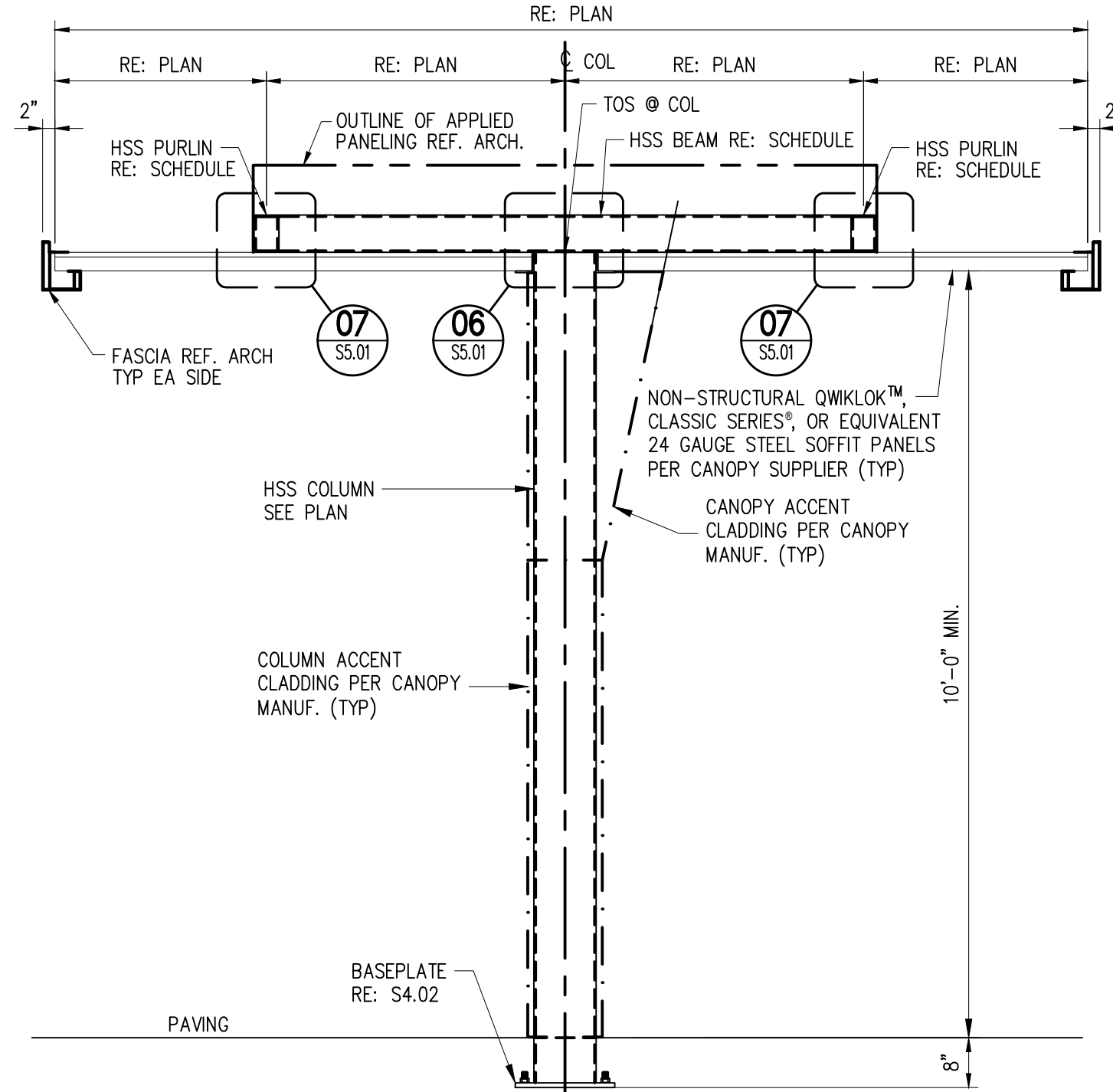
SHEET NUMBER / TITLE:

**S4.02**  
CANOPY FOUNDATION PLAN & DETAILS

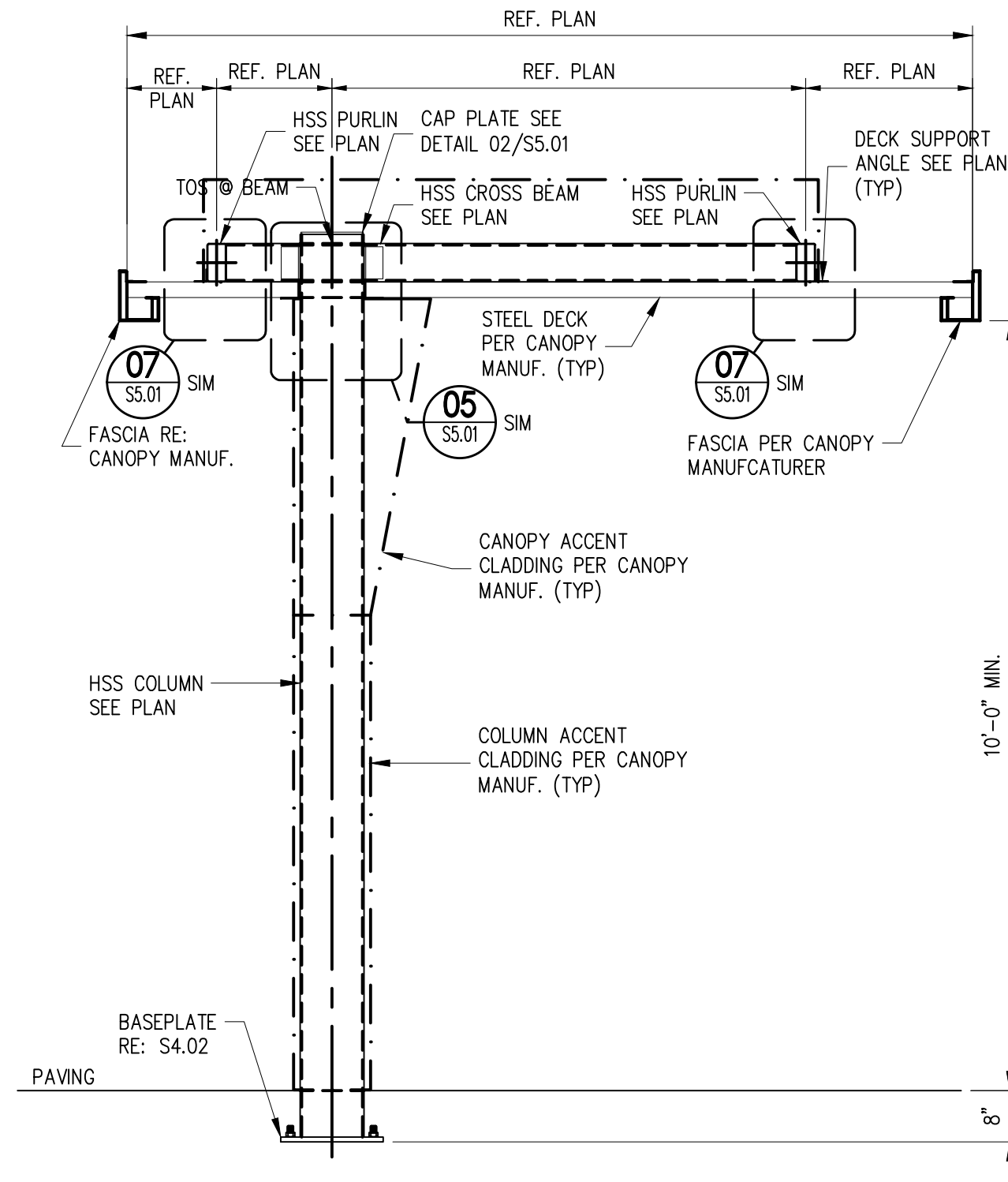




**01 SECTION AT CAR CANOPY**  
S5.01 SCALE: 1/2"=1'-0"

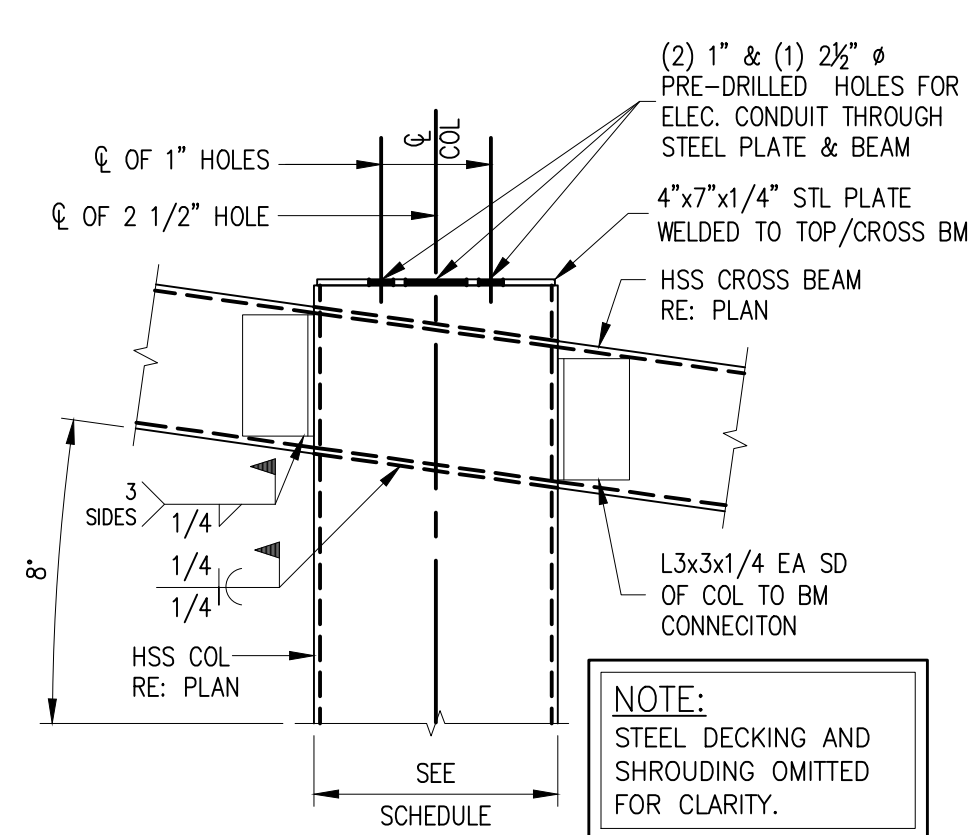


**02 SECTION AT CAR CANOPY**  
S5.01 SCALE: 1/2"=1'-0"

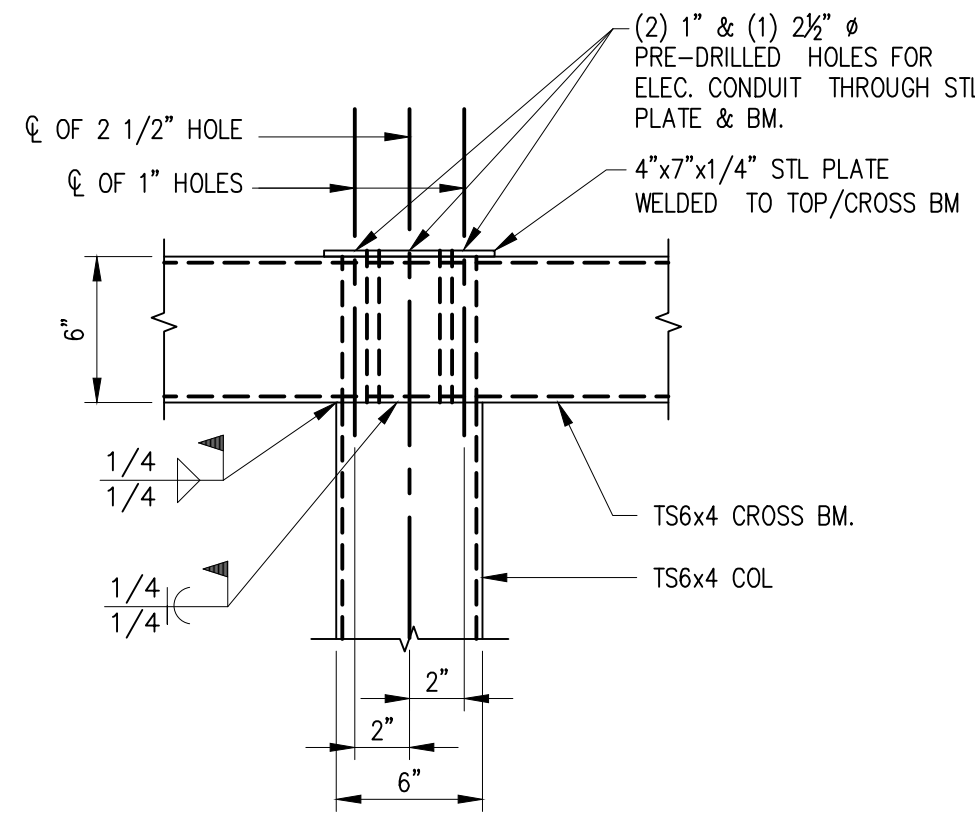


**03 SECTION AT DRIVE THRU MENU CANOPY**  
S5.01 SCALE: 1/2"=1'-0"

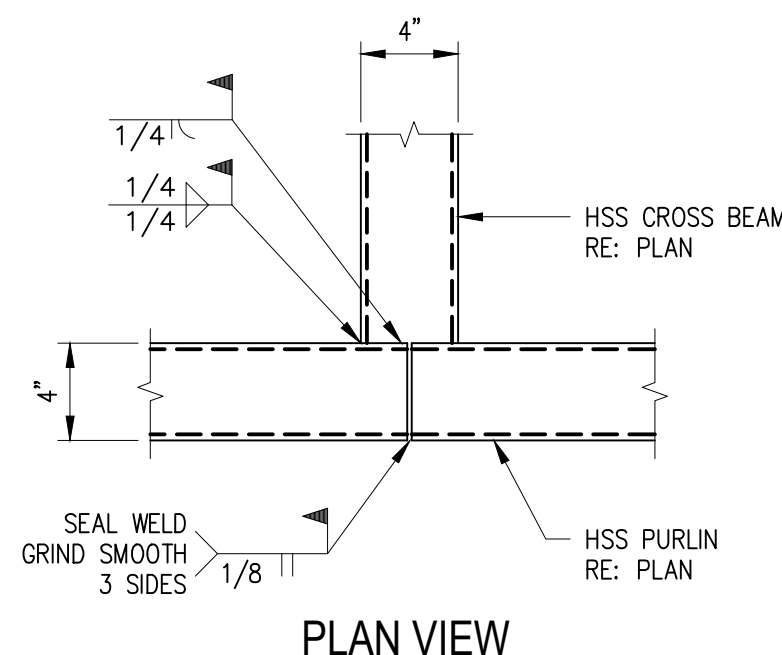
**04 NOT USED**  
S5.01 SCALE: NONE



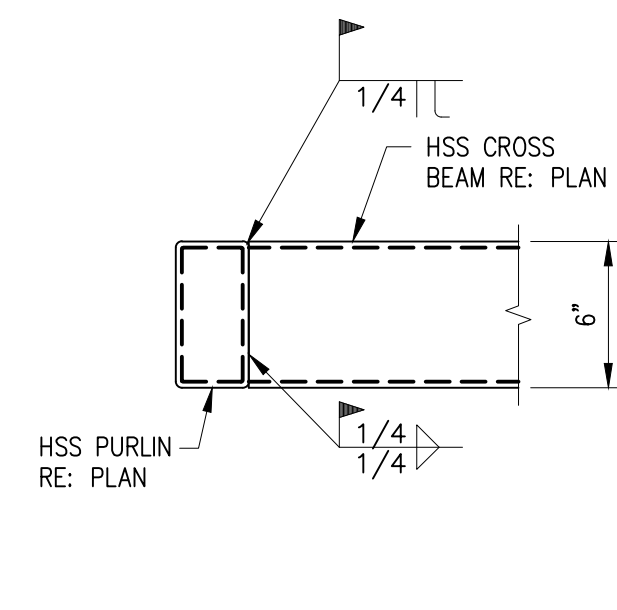
**05 CONNECTION DETAIL**  
S5.01 SCALE: 1 1/2"=1'-0"



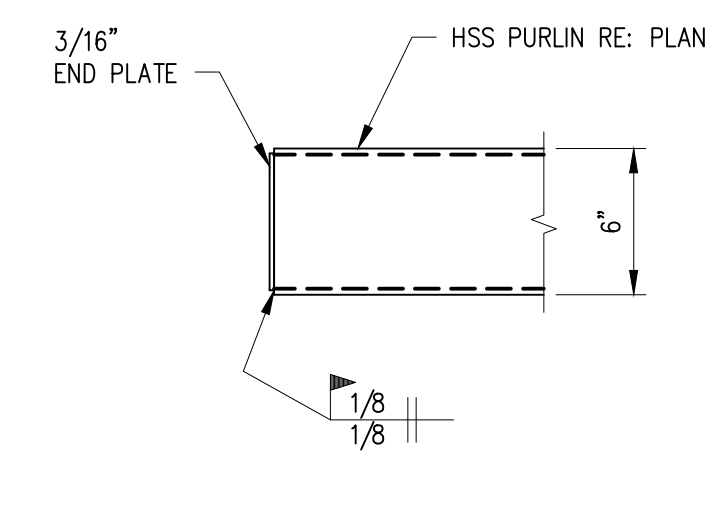
**06 CONNECTION DETAIL**  
S5.01 SCALE: 1 1/2"=1'-0"



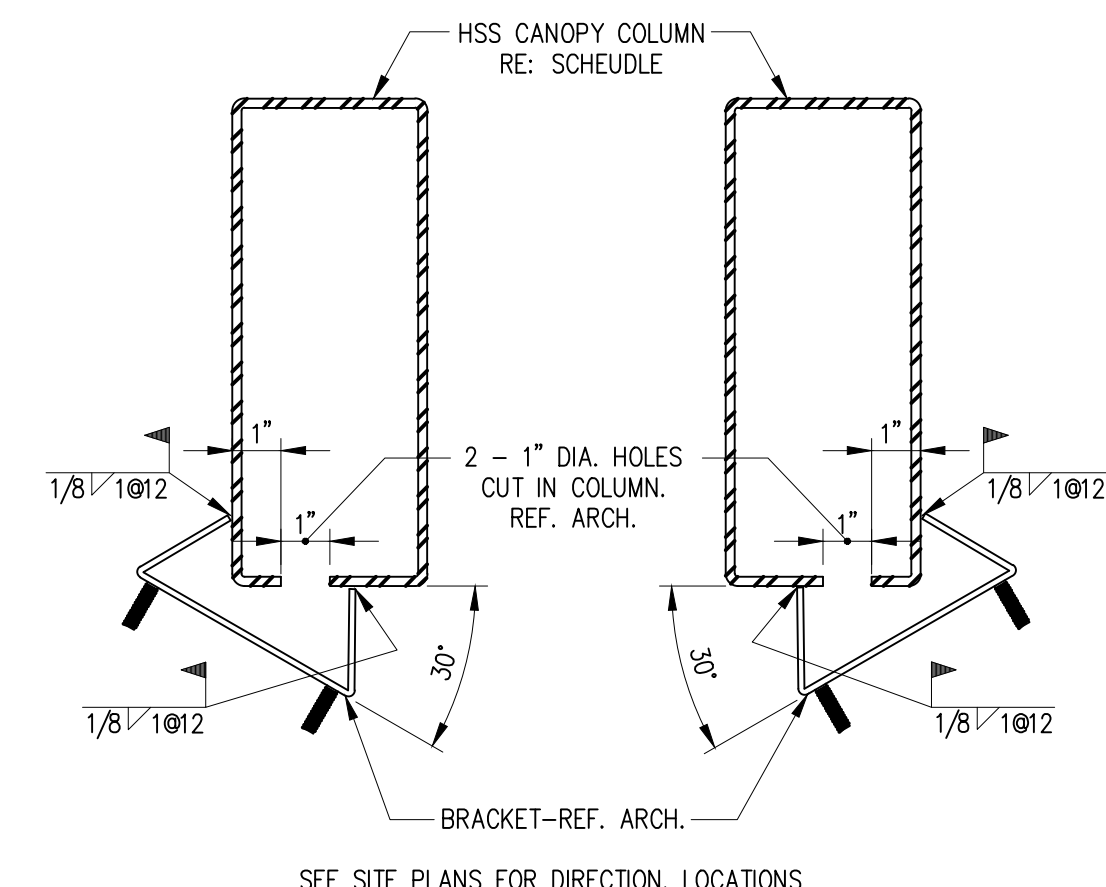
**07 CONNECTION DETAIL**  
S5.01 SCALE: 1 1/2"=1'-0"



**08 CONNECTION DETAIL**  
S5.01 SCALE: 1 1/2"=1'-0"



**09 CONNECTION DETAIL**  
S5.01 SCALE: 1 1/2"=1'-0"



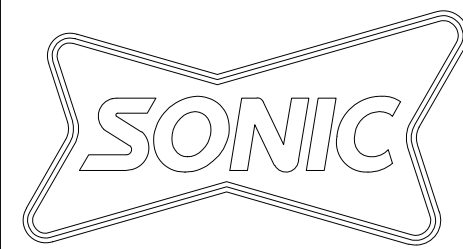
**10 CONNECTION DETAIL**  
S5.01 SCALE: 3"=1'-0"

**BRITT PETERS**  
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ASSOCIATES  
INC.  
consulting engineers  
999 Waterside Drive  
Suite 2202  
Norfolk, VA 23510  
(757) 965-5710  
BPA Job No. 220603  
FL COA #27497

CONSULTANT:

**national restaurant designers**  
**ARCHITECTS & ENGINEERS**  
7208 ACC Blvd. 2nd Floor, Raleigh, NC 27617  
ph: 919 944 0087 fax: 919 544 9399  
A Division of LMHT Associates

CLIENT:

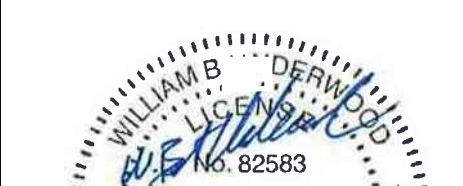


**SONIC CORP.**  
300 JOHNNY BENCH DR  
OKLAHOMA CITY, OK 73104  
OFFICES: 405-225-5000  
FAX: 405-225-5991

PROJECT INFORMATION:

**SONIC**  
**SONIC DRIVE-IN RESTAURANT**  
**LOT #3 GATEWAY CROSSING CENTURION WAY**  
**LAKE CITY, FLORIDA**  
**STORE # -**

SEAL:



STATE OF  
FLORIDA  
PROFESSIONAL ENGINEER  
07/13/22

PROJECT NO.: 22161  
DRAWN BY: DS  
CHECKED BY: RC

ISSUE: DATE:  
DISTRIBUTION SET 2022-06-24

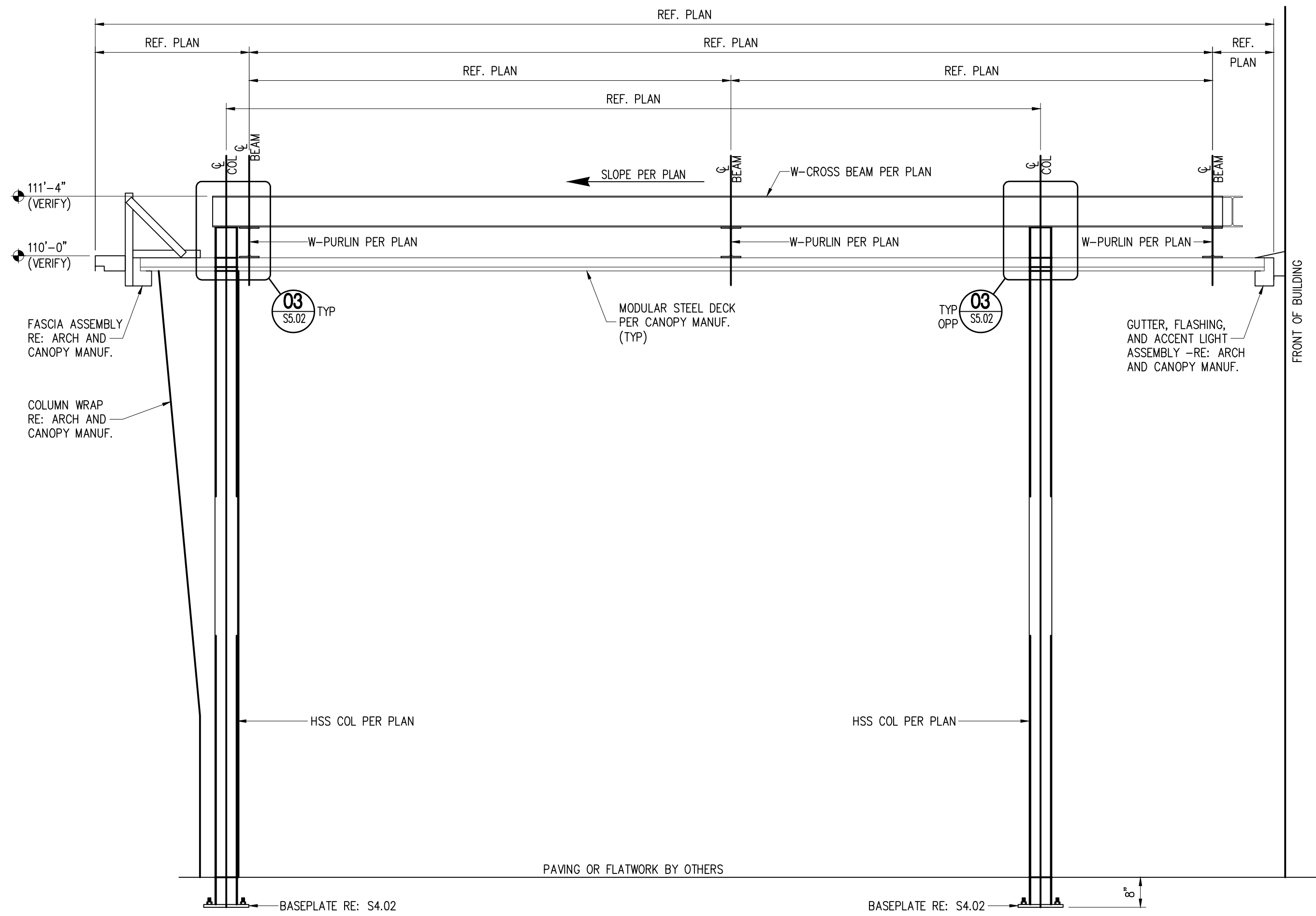
REVISION: DATE:

PROJECT LOCATION:  
**LAKE CITY, FLORIDA**  
**COVERED PATIO**

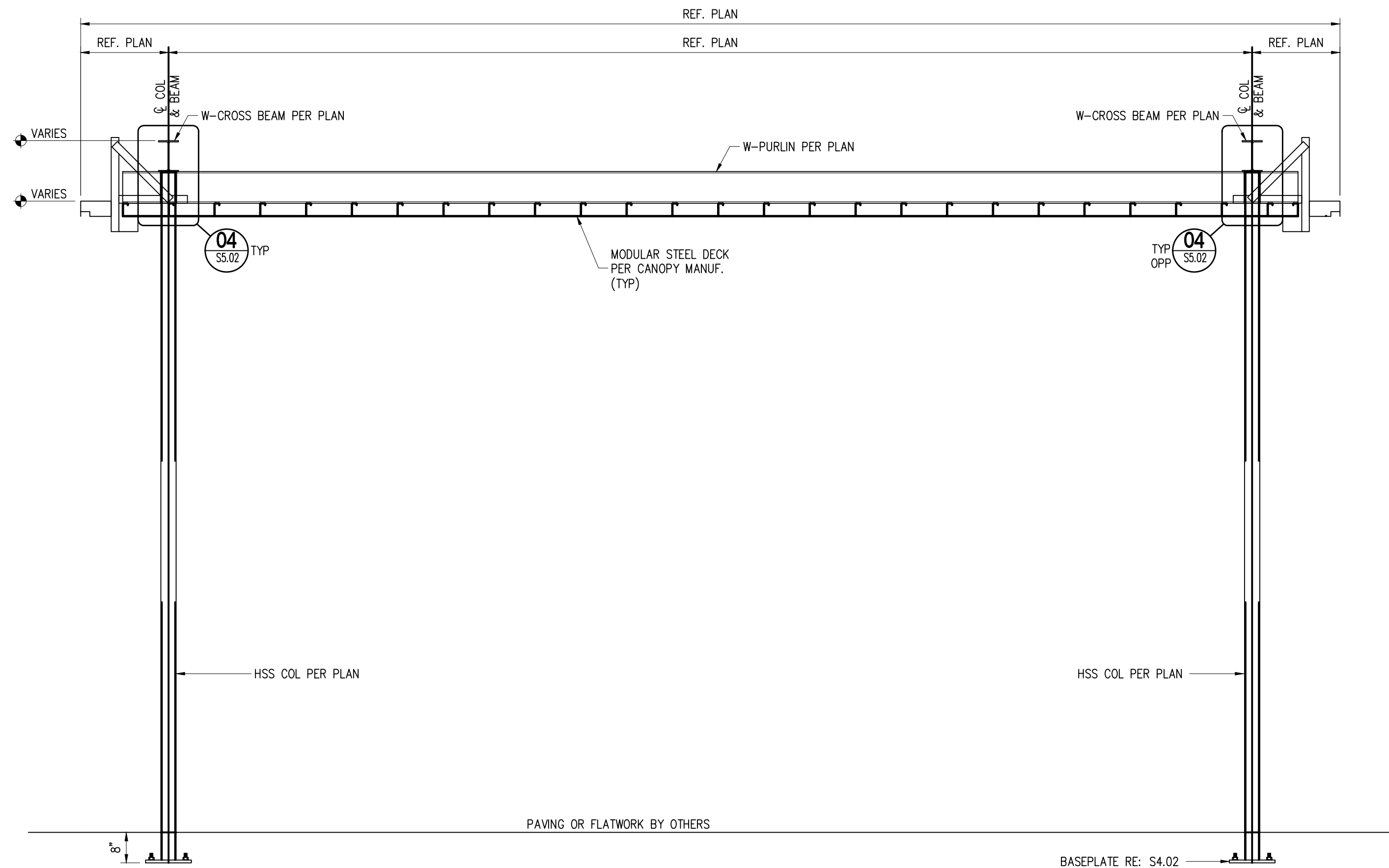
SHEET NUMBER / TITLE:

**S5.01**  
CAR STALL / MENU CANOPY DETAILS

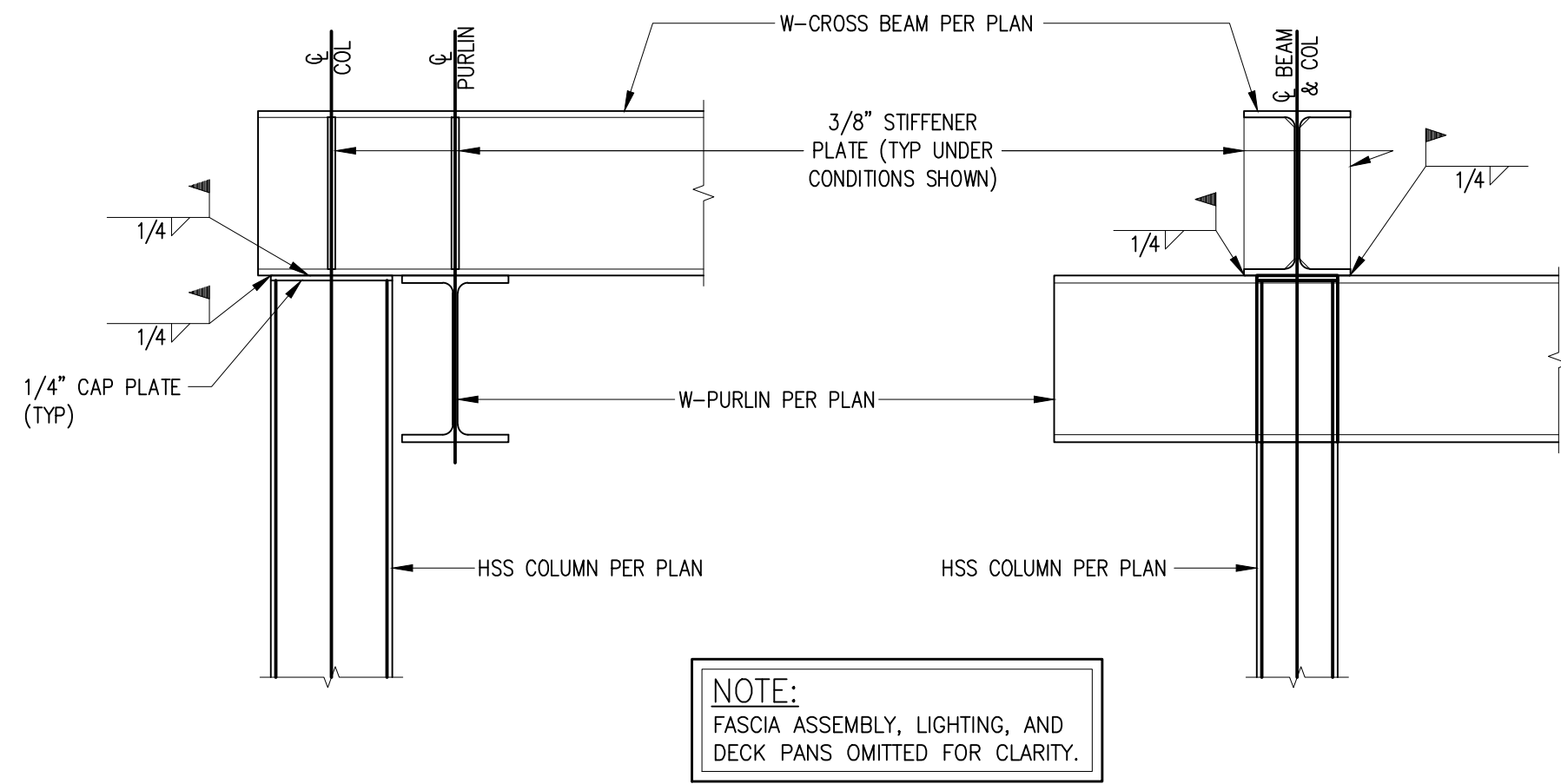




**01 SECTION AT PATIO CANOPY**  
S5.02 SCALE: 3/4"=1'-0"



**02 SECTION AT PATIO CANOPY**  
S5.02 SCALE: 3/4"=1'-0"



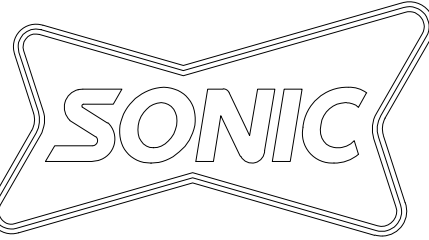
**03 CONNECTION DETAIL**  
S5.02 SCALE: 1 1/2"=1'-0"

**04 CONNECTION DETAIL**  
S5.02 SCALE: 1 1/2"=1'-0"

CONSULTANT:

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SHEET NUMBER / TITLE:  
**S5.02**  
PATIO CANOPY DETAILS

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