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COA #0 278
 Florida Certificate of Product Approval #FL1999
 05/09/2024

Alpine, an ITW Company
 155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025
 Phone: (800)755-6001
 www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 24-1030
Job Description: SALEMI/RUSSELL RESIDENCE (Suwannee Classic)	
Address: 398 SW Sanders Way, Fort White, FL 32038	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res. HVHZ	IntelliVIEW Version: 23.02.04 JRef #: 1XZO2150007
Wind Standard: ASCE 7-22 Wind Speed (mph): 130 Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 19 truss drawing(s) and 0 detail(s).

Item	Drawing Number	Truss
1	130.24.1044.19120	A01
3	130.24.1044.22340	A03
5	130.24.1044.41687	B02
7	130.24.1044.46757	C02
9	130.24.1044.54357	C04
11	130.24.1044.58393	C06
13	130.24.1045.13373	C08
15	130.24.1045.18270	C10
17	130.24.1045.22153	C12
19	130.24.1045.28173	C14

Item	Drawing Number	Truss
2	130.24.1044.20770	A02
4	130.24.1044.29110	B01
6	130.24.1044.44080	C01
8	130.24.1044.52563	C03
10	130.24.1044.56683	C05
12	130.24.1045.11360	C07
14	130.24.1045.15200	C09
16	130.24.1045.20667	C11
18	130.24.1045.23790	C13

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C-TW = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

General Notes (continued)

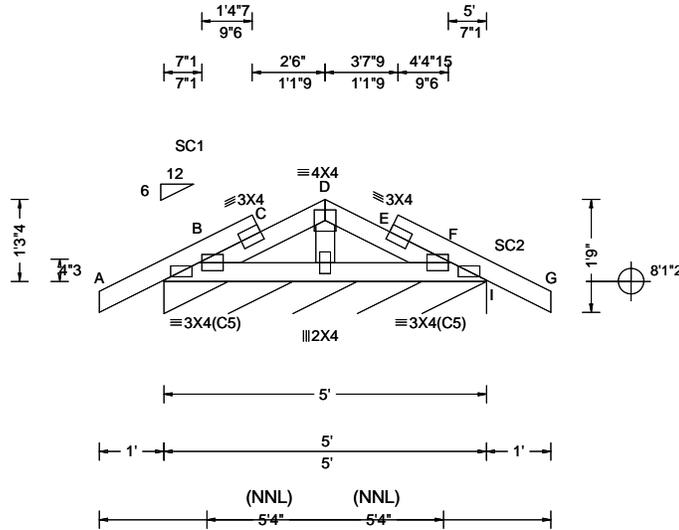
Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

SEQN: 762915 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 24-1030 SALEM/RUSSELL RESIDENCE (Suwannee Classic) Truss Label: A01	Cust: R215 JRef: 1XZO2150007 T2 DrwNo: 130.24.1044.19120 GA / DF 05/09/2024
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Private Provider Plan Review
Completed by Michael Williams
Licenses: BU2215, FX4959, BN7922
Or Associated Duty Authorized Representative
FREEDOM
CONTRACT COMPLIANCE



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): 0.000 C - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.137 Max BC CSI: 0.034 Max Web CSI: 0.027 VIEW Ver: 23.02.04.0123.14	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 109 /- /- /53 /18 /11 Wind reactions based on MWFRS I Brg Wid = 60.0 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4(C5) except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

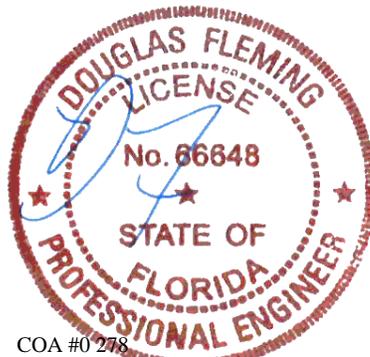
Wind loading based on both gable and hip roof types.

Additional Notes

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 1-3-4.

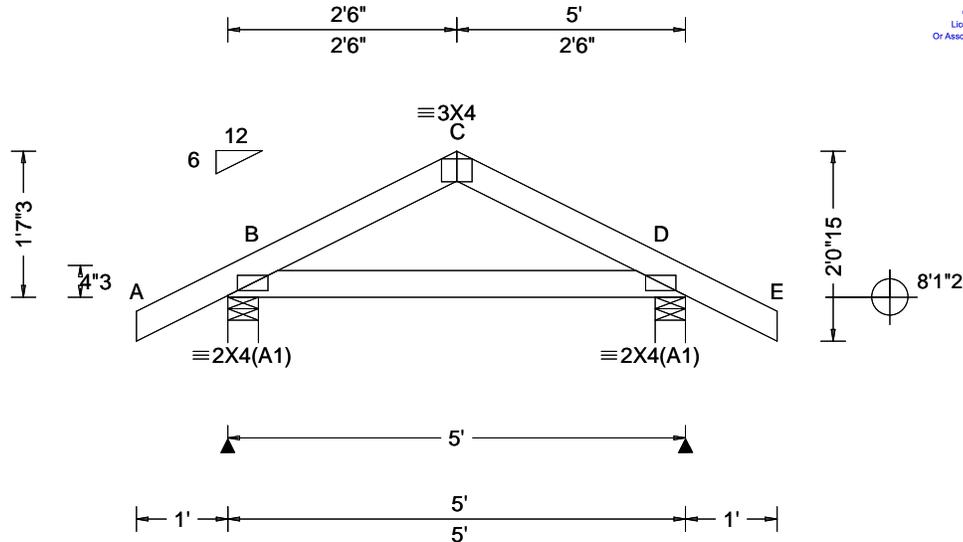
Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.004 B 999 180 HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.119 Max BC CSI: 0.143 Max Web CSI: 0.000 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 273 /- /- /180 /51 /57 D 273 /- /- /180 /51 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375#

Lumber

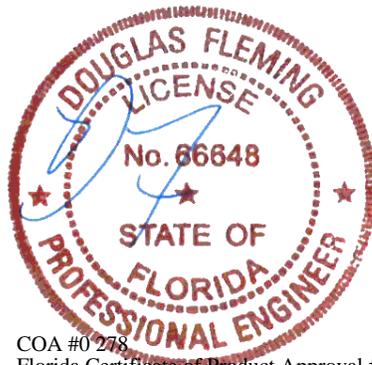
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

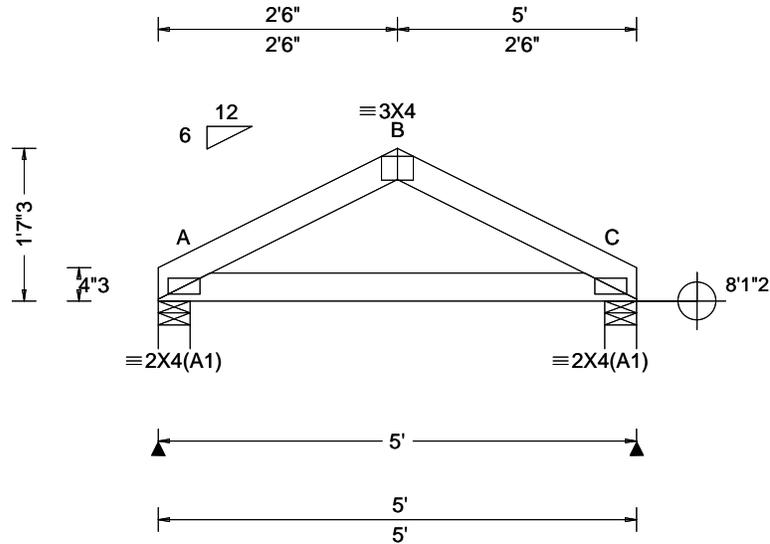
The overall height of this truss excluding overhang is 1-7-3.



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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.002 A 999 240 VERT(CL): 0.005 A 999 180 HORZ(LL): 0.001 A - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.077 Max BC CSI: 0.152 Max Web CSI: 0.000 VIEW Ver: 23.02.04.0123.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 206 /- /- /119 /33 /32 C 206 /- /- /119 /33 /- Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) C Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings A & C are a rigid surface. Members not listed have forces less than 375#

Lumber

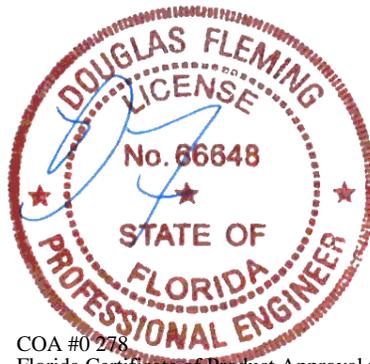
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 17-3.

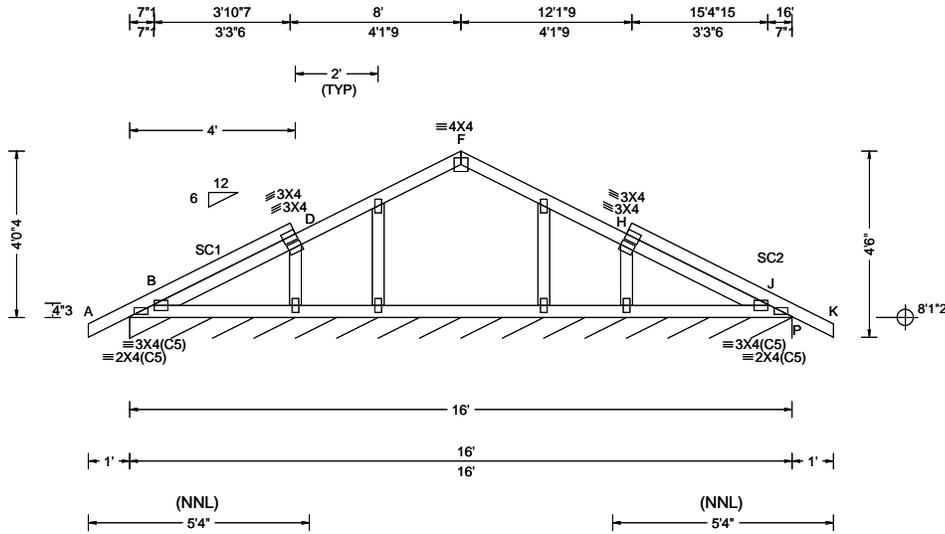


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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 762947 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 24-1030 SALEM/RUSSELL RESIDENCE (Suwannee Classic) Truss Label: B01	Cust: R 215 JRef: 1XZO2150007 T5 DrwNo: 130.24.1044.29110 GA / DF 05/09/2024
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Private Provider Plan Review
Completed by Michael Williams
Licenses: BU2216, PX4929, BN7822
Or Associated Duly Authorized Representative
FREEDOM
www.freedom.com

Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.003 F 999 240 VERT(CL): 0.006 F 999 180 HORZ(LL): -0.002 E - - HORZ(TL): 0.004 E - - Creep Factor: 2.0 Max TC CSI: 0.196 Max BC CSI: 0.096 Max Web CSI: 0.469 VIEW Ver: 23.02.04.0123.14	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL P* 91 /- /- /46 /16 /8 Wind reactions based on MWFRS P Brg Wid = 192 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

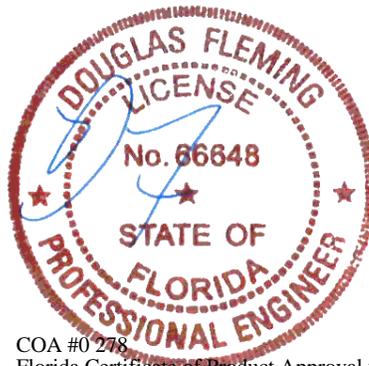
Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/865.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 4-0-4.

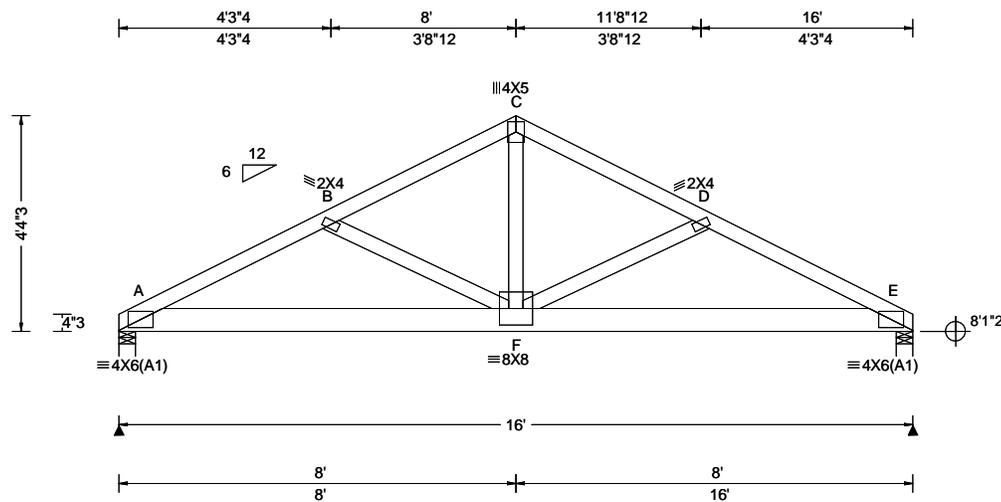


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2 Complete Trusses Required

Private Provider Plan Review
Completed by Michael Williams
Licenses: BU2215, PX4929, BN7822
Or Associated Duty Authorized Representative
FREEDOM
CORPORATION



Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.067 F 999 240 VERT(CL): 0.134 F 999 180 HORZ(LL): 0.017 A - - HORZ(TL): 0.033 A - - Creep Factor: 2.0 Max TC CSI: 0.632 Max BC CSI: 0.874 Max Web CSI: 0.925 VIEW Ver: 23.02.04.0123.14	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 3734 -/- /- /- /657 -/ E 3788 -/- /- /- /550 -/ Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) E Brg Wid = 4.0 Min Req = 1.6 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 511 -2887 C - D 476 -2762 B - C 476 -2762 D - E 504 -2889					
				Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - F 2607 -460 F - E 2610 -450 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - F 2428 -401					

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x6 SP 2400f-2.0E;
 Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 1 Row @ 3.75" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

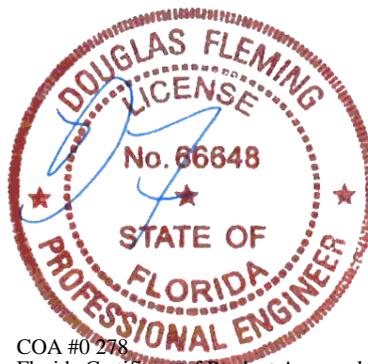
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 31 plf at 0.00 to 31 plf at 16.00
 BC: From 10 plf at 0.00 to 10 plf at 16.00
 BC: 980 lb Conc. Load at 2.06, 4.06, 6.06, 8.06, 10.06, 12.06, 14.06

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 4-4-3.

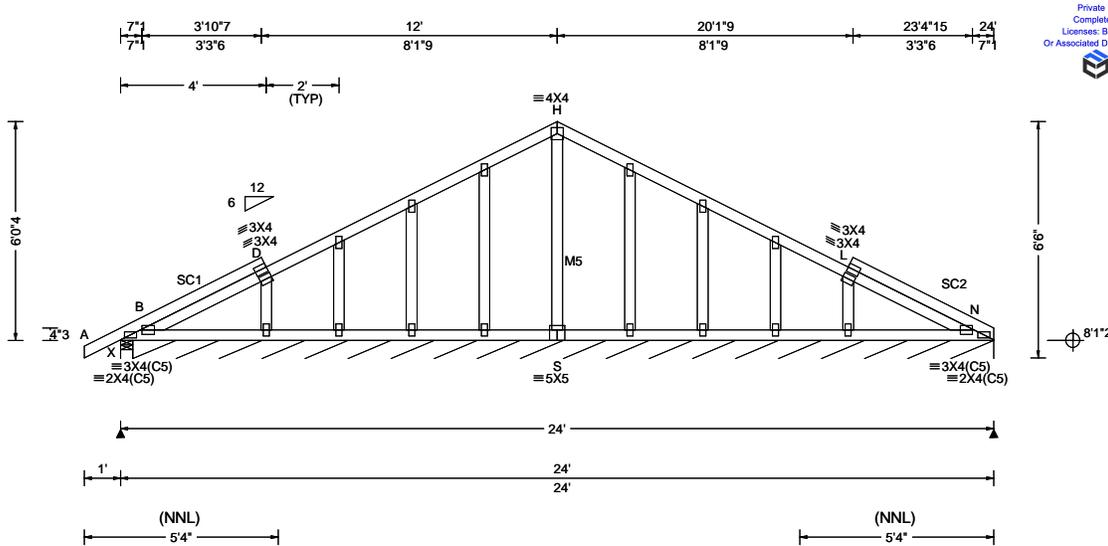


COA #0 278
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 05/09/2024

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SEQN: 762908 FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 24-1030 SALEM/RUSSELL RESIDENCE (Suwannee Classic) Truss Label: C01	Cust: R 215 JRef: 1XZO2150007 T7 DrwNo: 130.24.1044.44080 GA / DF 05/09/2024
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Private Provider Plan Review
Completed by Michael Williams
Licenses: BU2216, PX4929, BN7822
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FREEDOM
www.freedom.com

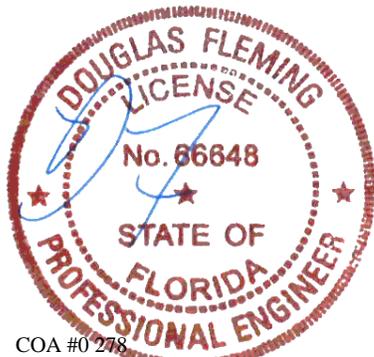
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; M5 2x4 SP #2;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/125.

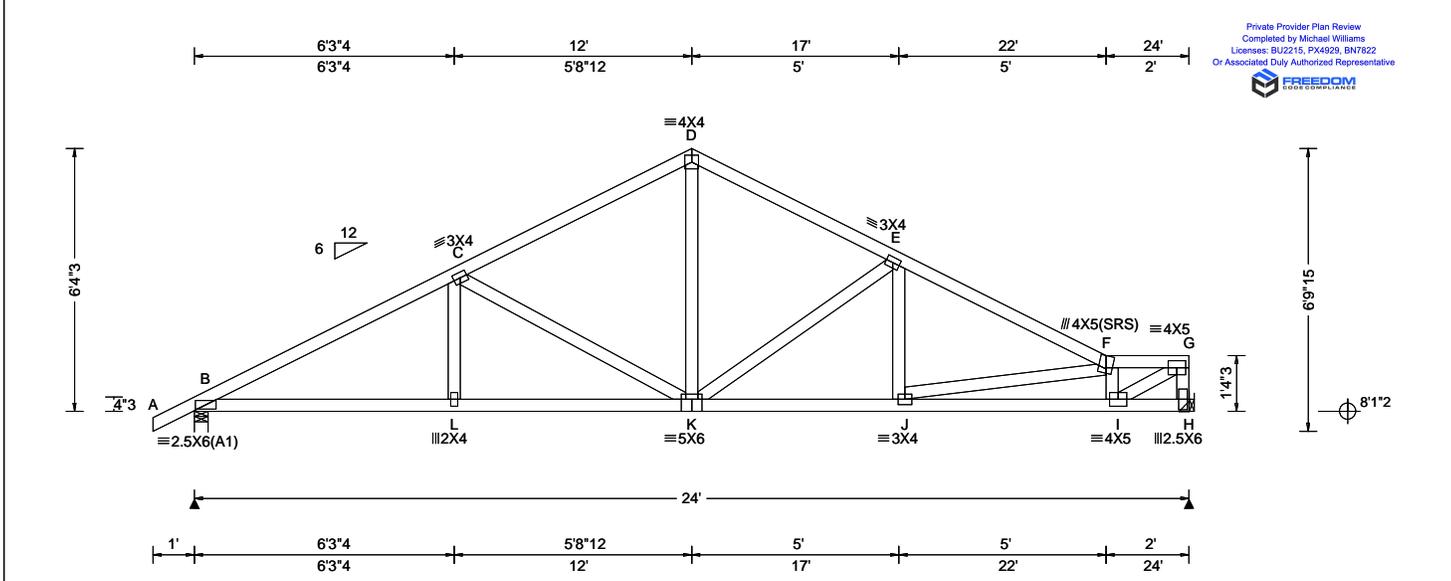
Additional Notes
 Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 6'-0-4.



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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties
 Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
 Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
 Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
 Bearing at location x=23'9" uses the following support conditions: 23'9"
 Bearing H (23'9", 8'1"2) LUS26
 Supporting Member: (2)2x6 SP 2400f-2.0E
 (4) 0.148"x3" nails into supporting member,
 (3) 0.148"x3" nails into supported member.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

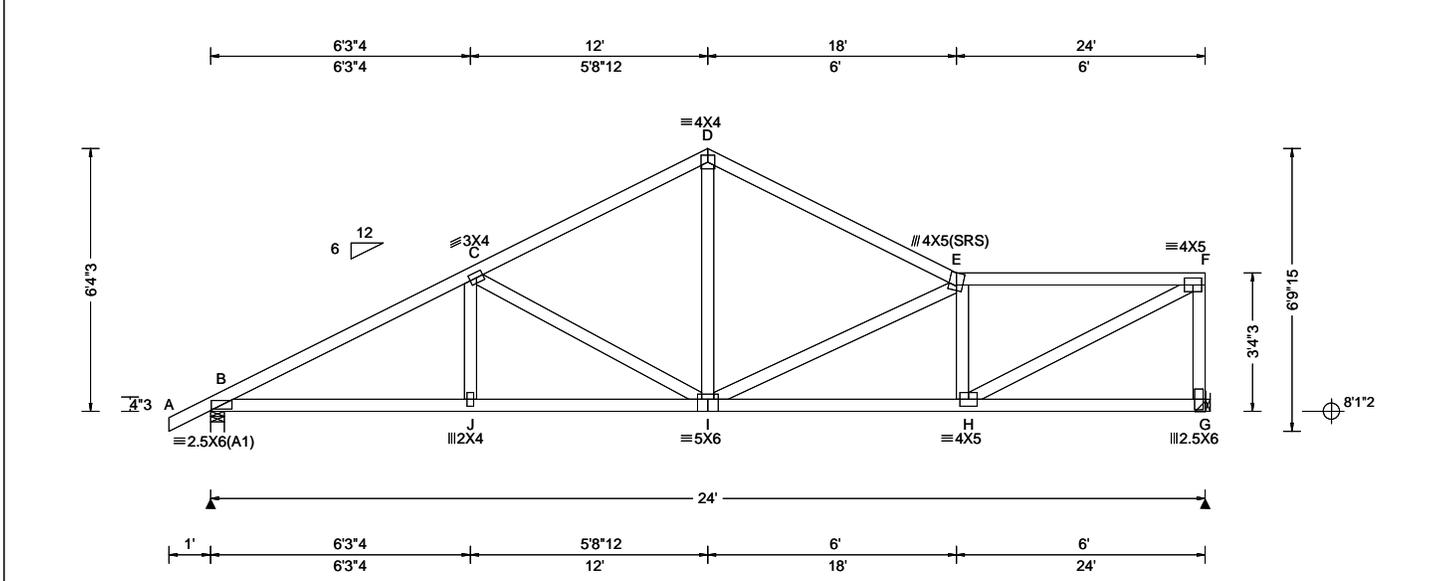
Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 6-4-3.



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Lumber

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 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

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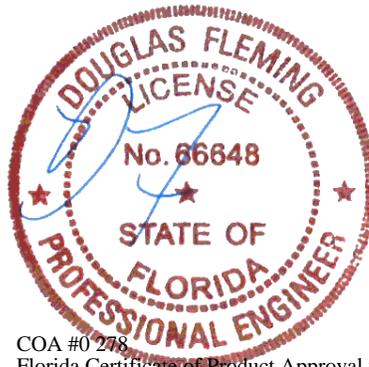
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Wind

Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Additional Notes

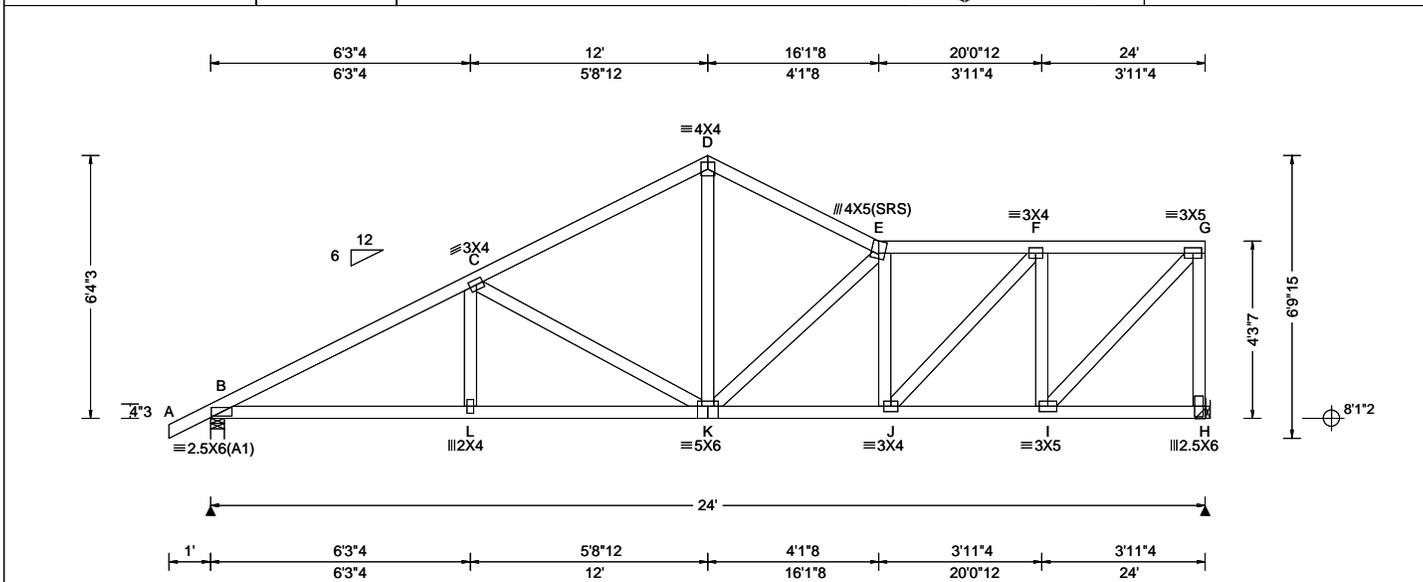
The overall height of this truss excluding overhang is 6-4-3.



COA #0278
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.057 K 999 240 VERT(CL): 0.116 K 999 180 HORZ(LL): 0.021 H - - HORZ(TL): 0.042 H - - Creep Factor: 2.0 Max TC CSI: 0.376 Max BC CSI: 0.463 Max Web CSI: 0.442 VIEW Ver: 23.02.04.0123.14	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1063</td> <td>-</td> <td>-</td> <td>/647</td> <td>/178</td> <td>/164</td> </tr> <tr> <td>H</td> <td>980</td> <td>-</td> <td>-</td> <td>/519</td> <td>/188</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p> Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>471 -1718</td> <td>E - F</td> <td>520 -1279</td> </tr> <tr> <td>C - D</td> <td>437 -1194</td> <td>F - G</td> <td>374 -803</td> </tr> <tr> <td>D - E</td> <td>444 -1159</td> <td></td> <td></td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - L</td> <td>1468 -516</td> <td>K - J</td> <td>1308 -532</td> </tr> <tr> <td>L - K</td> <td>1465 -517</td> <td>J - I</td> <td>858 -405</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - K</td> <td>186 -542</td> <td>J - F</td> <td>638 -174</td> </tr> <tr> <td>D - K</td> <td>680 -221</td> <td>F - I</td> <td>435 -760</td> </tr> <tr> <td>K - E</td> <td>271 -438</td> <td>I - G</td> <td>1160 -538</td> </tr> <tr> <td>E - J</td> <td>172 -408</td> <td>G - H</td> <td>500 -947</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1063	-	-	/647	/178	/164	H	980	-	-	/519	/188	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	471 -1718	E - F	520 -1279	C - D	437 -1194	F - G	374 -803	D - E	444 -1159			Chords	Tens.Comp.	Chords	Tens. Comp.	B - L	1468 -516	K - J	1308 -532	L - K	1465 -517	J - I	858 -405	Webs	Tens.Comp.	Webs	Tens. Comp.	C - K	186 -542	J - F	638 -174	D - K	680 -221	F - I	435 -760	K - E	271 -438	I - G	1160 -538	E - J	172 -408	G - H	500 -947
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

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Wind
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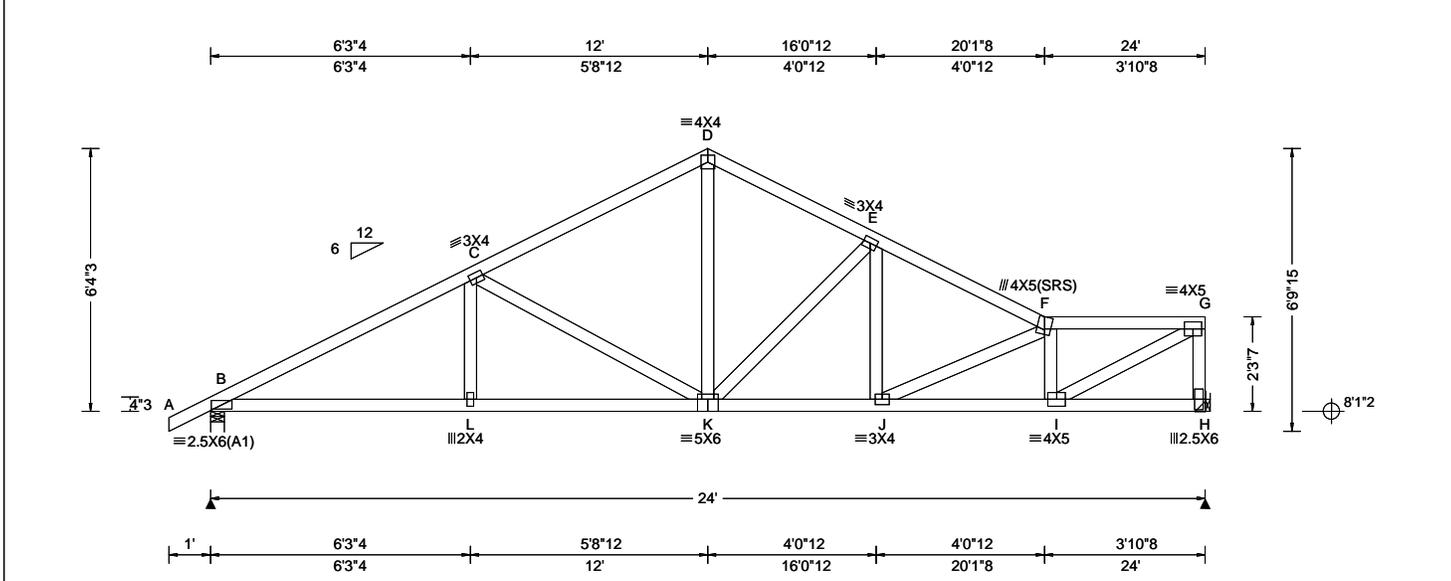
Additional Notes
 The overall height of this truss excluding overhang is 6-4-3.



COA #0 278
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.064 K 999 240 VERT(CL): 0.131 K 999 180 HORZ(LL): 0.023 H - - HORZ(TL): 0.047 H - - Creep Factor: 2.0 Max TC CSI: 0.378 Max BC CSI: 0.464 Max Web CSI: 0.655 VIEW Ver: 23.02.04.0123.14	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1063 -/ - /633 /186 /164 H 980 -/ - /516 /175 -/ Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 409 -1718 E - F 434 -1535 C - D 372 -1193 F - G 466 -1514 D - E 381 -1160 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - L 1468 -386 K - J 1311 -337 L - K 1465 -388 J - I 1627 -511 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - K 190 -545 F - I 308 -774 D - K 697 -170 I - G 1721 -526 K - E 189 -455 G - H 369 -943
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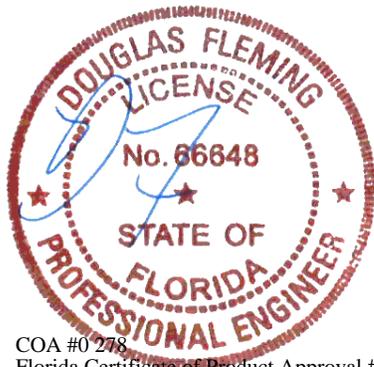
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Webs: 2x4 SP #3;

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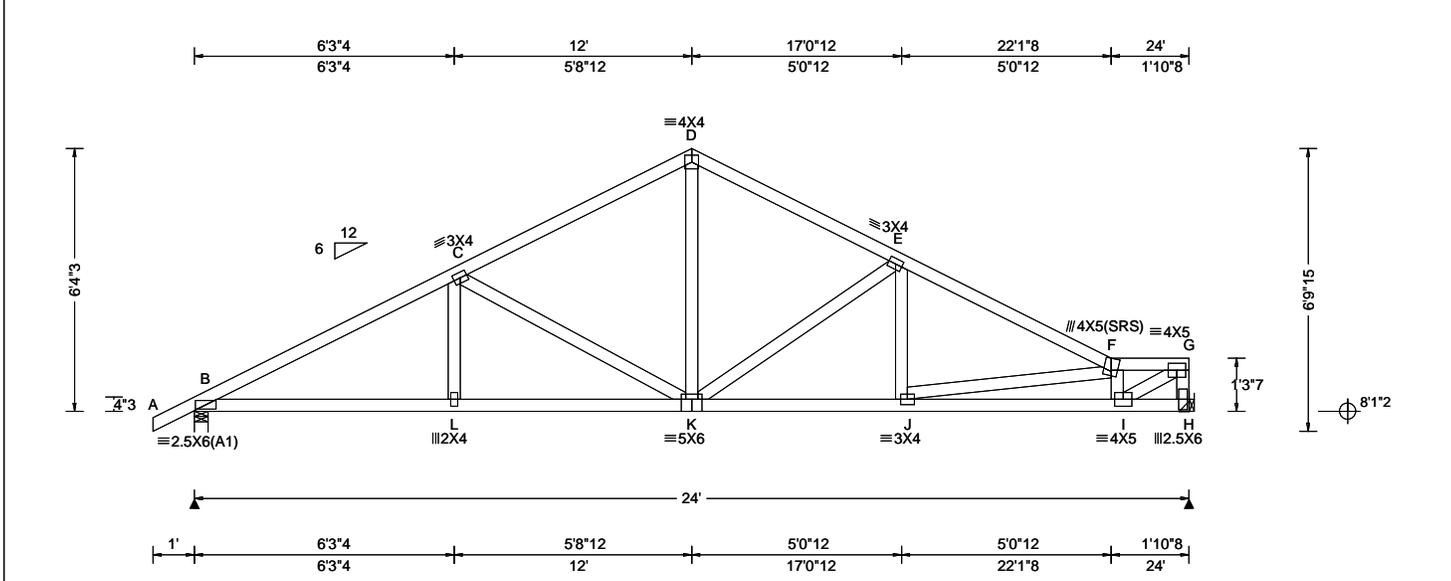
Additional Notes
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COA #0278
Florida Certificate of Product Approval #FL1999
05/09/2024

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.064 K 999 240 VERT(CL): 0.130 K 999 180 HORZ(LL): 0.026 H - - HORZ(TL): 0.054 H - - Creep Factor: 2.0 Max TC CSI: 0.375 Max BC CSI: 0.501 Max Web CSI: 0.659 VIEW Ver: 23.02.04.0123.14	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1063 - / - / - /630 /25 /164 H 980 - / - / - /541 /19 - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 394 -1717 E - F 406 -1661 C - D 357 -1195 F - G 383 -1474 D - E 362 -1183 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - L 1467 -336 K - J 1419 -309 L - K 1464 -338 J - I 1735 -470 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - K 190 -541 F - I 309 -917 D - K 679 -142 I - G 1731 -448 K - E 191 -522 G - H 273 -950
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Lumber
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Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

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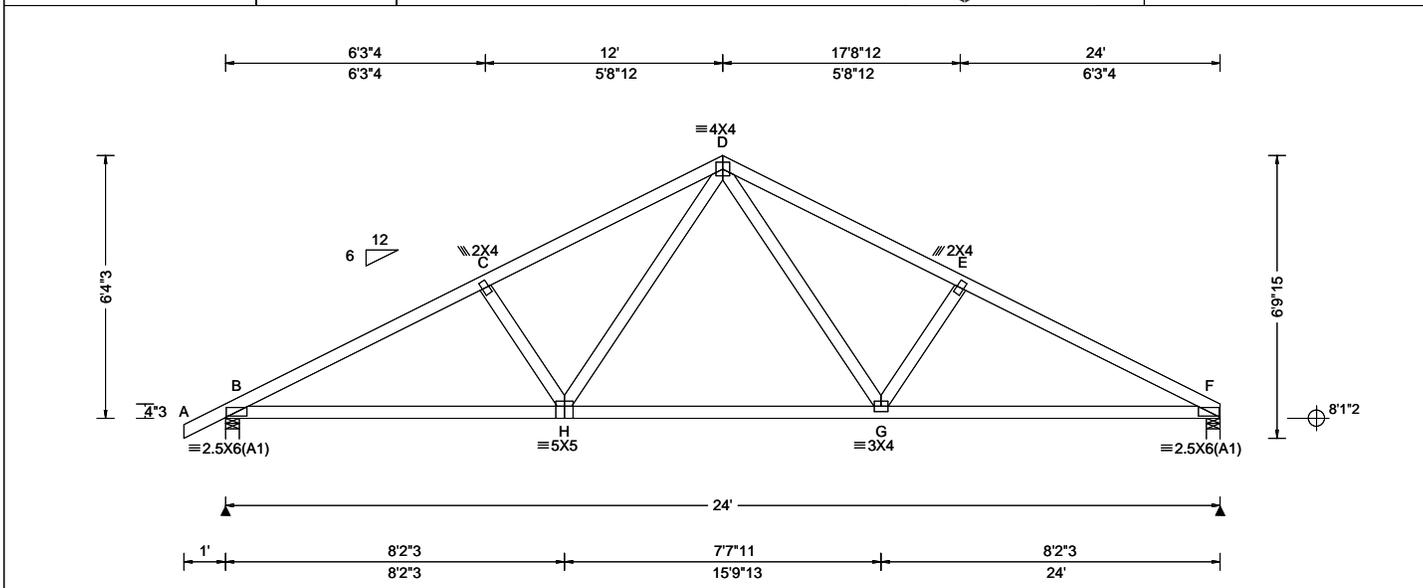
Additional Notes
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COA #0 278
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Lumber

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Purlins

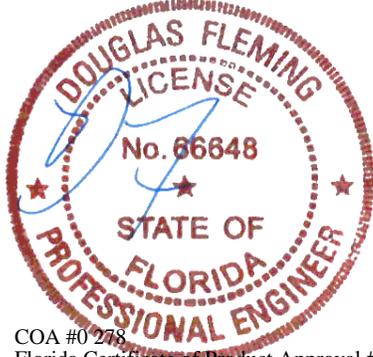
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Wind

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 Wind loading based on both gable and hip roof types.

Additional Notes

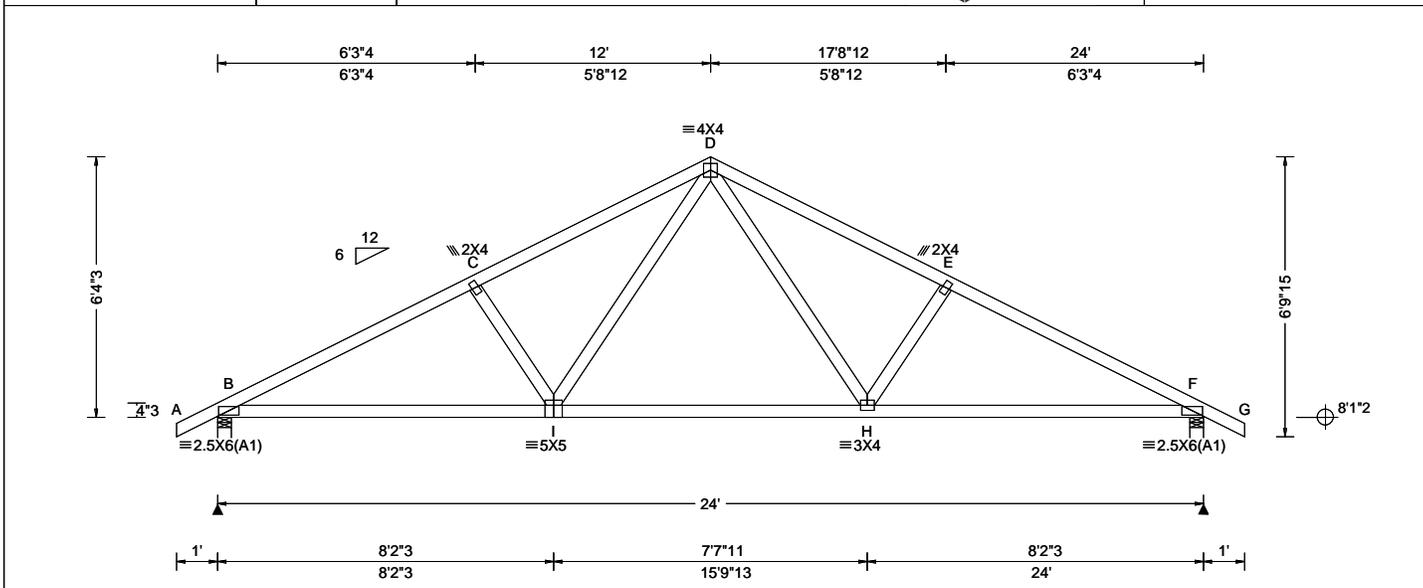
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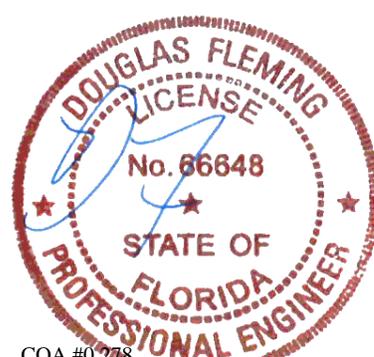
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Webs: 2x4 SP #3;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels use purlins to brace TC @ 24" oc.

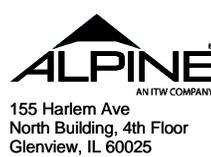
Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 6-4-3.



COA #0 278
Florida Certificate of Product Approval #FL1999
05/09/2024

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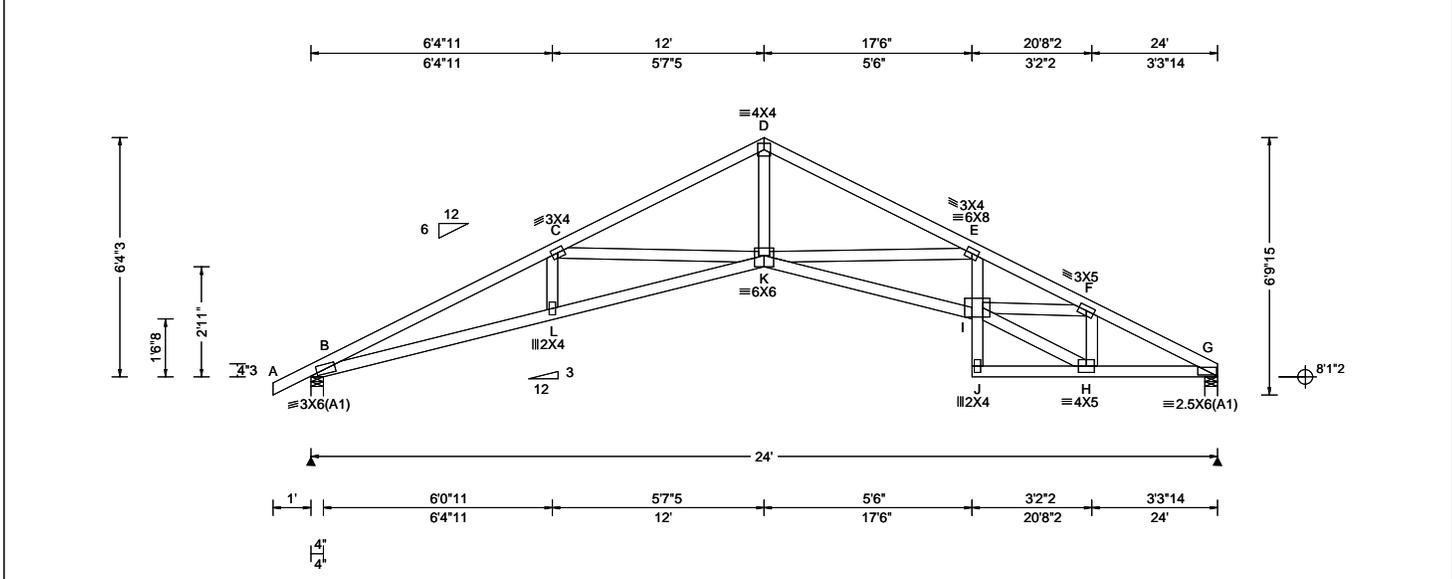
SEQN: 762955
FROM: CDM

COMN
Ply: 1
Qty: 1

Job Number: 24-1030
SALEM/RUSSELL RESIDENCE (Suwannee Classic)
Truss Label: C11

Private Provider Plan Review
Completed by Michael Williams
License: BU2215, PA4829, BN7822
Or Associated Duty Authorized Representative
FREEDOM
CORPORATION

Cust: R 215 JRef: 1XZO2150007 T19
DrwNo: 130.24.1045.20667
GA / DF 05/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria

Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res. HVHZ
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
VERT(LL): 0.207 K 999 240
VERT(CL): 0.425 K 668 180
HORZ(LL): 0.150 G - -
HORZ(TL): 0.308 G - -
Creep Factor: 2.0
Max TC CSI: 0.445
Max BC CSI: 0.790
Max Web CSI: 0.652

VIEW Ver: 23.02.04.0123.14

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
B	1064	-	-	/626	/24	/171
G	990	-	-	/574	/15	-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.5 (Truss)
G Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings B & G are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	648 - 3053	E - F	715 - 3271
C - D	470 - 2232	F - G	400 - 1804
D - E	483 - 2230		

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 6-4-3.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - L	2744 - 547	K - I	3026 - 575
L - K	2751 - 550	H - G	1568 - 315

Maximum Web Forces Per Ply (lbs)

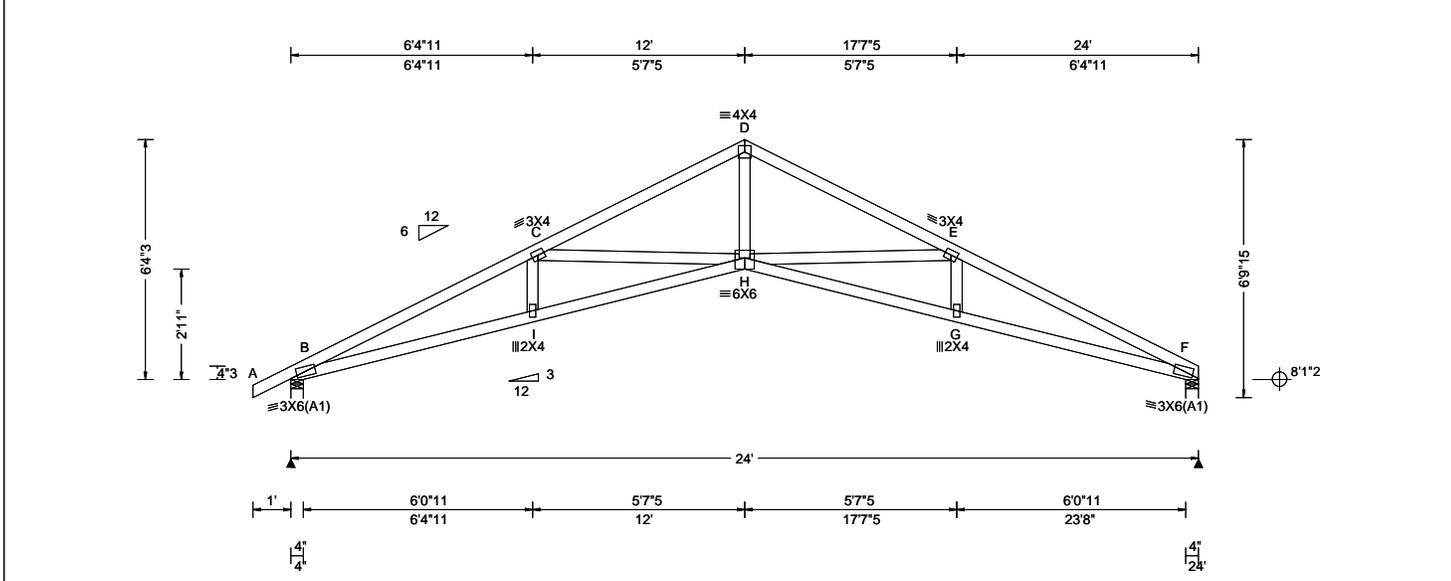
Webs	Tens.Comp.	Webs	Tens. Comp.
C - K	274 - 721	I - H	1710 - 340
K - E	343 - 1011	I - F	1360 - 247
D - K	1559 - 232	H - F	210 - 830



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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. HVHZ TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.224 H 999 240 VERT(CL): 0.460 H 618 180 HORZ(LL): 0.163 F - - HORZ(TL): 0.336 F - - Creep Factor: 2.0 Max TC CSI: 0.473 Max BC CSI: 0.793 Max Web CSI: 0.598 VIEW Ver: 23.02.04.0123.14	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1064</td> <td>-</td> <td>-</td> <td>/626</td> <td>/187</td> <td>/171</td> </tr> <tr> <td>F</td> <td>994</td> <td>-</td> <td>-</td> <td>/570</td> <td>/169</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>647 -3056</td> <td>D - E</td> <td>485 -2233</td> </tr> <tr> <td>C - D</td> <td>469 -2232</td> <td>E - F</td> <td>644 -3075</td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - I</td> <td>2747 -546</td> <td>H - G</td> <td>2772 -524</td> </tr> <tr> <td>I - H</td> <td>2754 -550</td> <td>G - F</td> <td>2767 -521</td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - H</td> <td>275 -724</td> <td>D - H</td> <td>1570 -241</td> </tr> <tr> <td>H - E</td> <td>285 -742</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1064	-	-	/626	/187	/171	F	994	-	-	/570	/169	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	647 -3056	D - E	485 -2233	C - D	469 -2232	E - F	644 -3075	Chords	Tens.Comp.	Chords	Tens. Comp.	B - I	2747 -546	H - G	2772 -524	I - H	2754 -550	G - F	2767 -521	Webs	Tens.Comp.	Webs	Tens. Comp.	C - H	275 -724	D - H	1570 -241	H - E	285 -742		
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Lumber

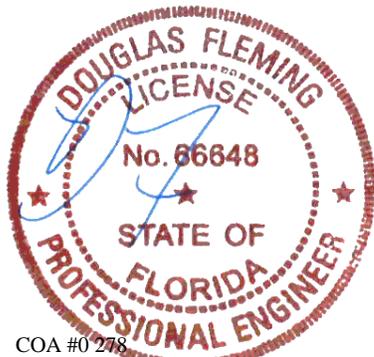
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 6-4-3.



COA #0 278
 Florida Certificate of Product Approval #FL1999
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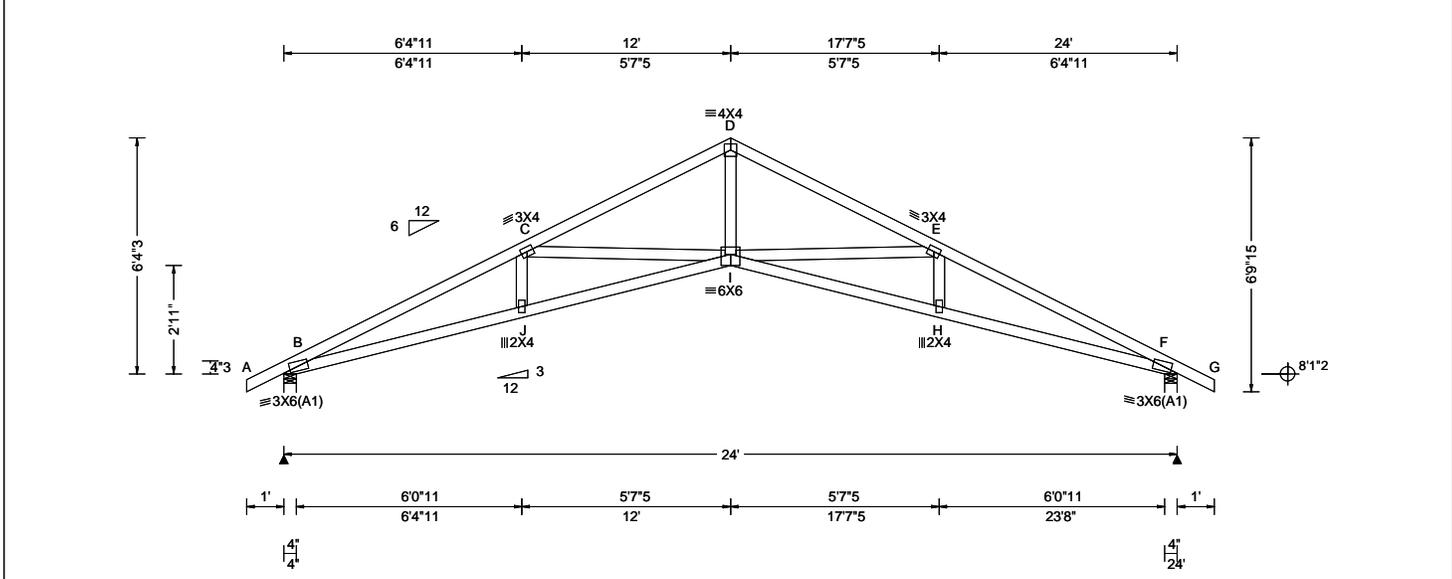
SEQN: 762959
FROM: CDM

COMN
Ply: 1
Qty: 5

Job Number: 24-1030
SALEMI/RUSSELL RESIDENCE (Suwannee Classic)
Truss Label: C13

Private Provider Plan Review
Completed by Michael Williams
Licenses: BU2215, PA4829, BN7822
Or Associated Duty Authorized Representative
FREEDOM
CORPORATION

Cust: R215 JRef: 1XZO2150007 T18
DrwNo: 130.24.1045.23790
GA / DF 05/09/2024



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria

Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res. HVHZ
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in loc L/def L/#
VERT(LL): 0.225 I 999 240
VERT(CL): 0.460 I 618 180
HORZ(LL): 0.164 F - -
HORZ(TL): 0.336 F - -
Creep Factor: 2.0
Max TC CSI: 0.442
Max BC CSI: 0.791
Max Web CSI: 0.595

VIEW Ver: 23.02.04.0123.14

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
B	1063	-	-	/626	/187	/180
F	1063	-	-	/626	/187	-

Wind reactions based on MWFRS
B Brg Wid = 4.0 Min Req = 1.5 (Truss)
F Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	1071 -3049	D - E	753 -2225
C - D	753 -2225	E - F	1071 -3049

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 6-4-3.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	2740 -861	I - H	2747 -866
J - I	2747 -866	H - F	2740 -862

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - I	447 -724	D - I	1562 -421
I - E	447 -724		



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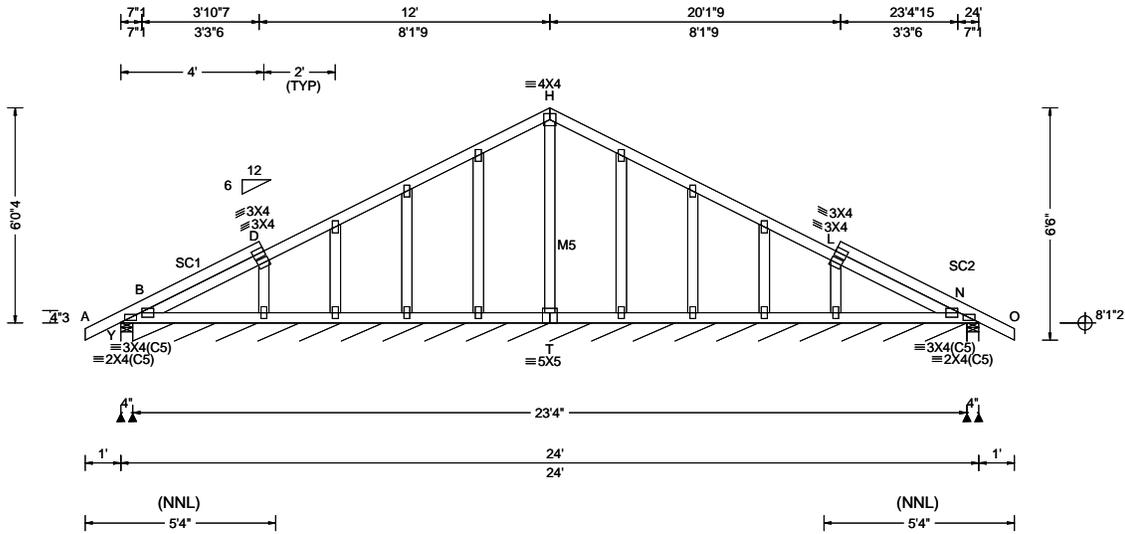
SEQN: 762911
FROM: CDM

GABL Ply: 1
Qty: 1

Job Number: 24-1030
SALEMI/RUSSELL RESIDENCE (Suwannee Classic)
Truss Label: C14

Private Provider Plan Review
Completed by Michael Williams
Licenses: BU2215, P14529, BN7922
Or Associated Duty Authorized Representative
FREEDOM
CORPORATION

Cust: R215 JRef: 1XZO2150007 T10
DrwNo: 130.24.1045.28173
GA / DF 05/09/2024



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
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Load Duration: 1.25
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Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg, Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA
Building Code: FBC 8th Ed. 2023 Res. HVHZ
TPI Std: 2014
Rep Fac: Yes
FT/RT: 20(0)/10(0)
Plate Type(s): WAVE

Defl/CSI Criteria
PP Deflection in loc L/defl L/#
VERT(LL): 0.001 B 999 240
VERT(CL): 0.003 B 999 180
HORZ(LL): 0.003 L - -
HORZ(TL): 0.003 L - -
Creep Factor: 2.0
Max TC CSI: 0.195
Max BC CSI: 0.067
Max Web CSI: 0.989
VIEW Ver: 23.02.04.0123.14

▲ Maximum Reactions (lbs), or *=PLF						
Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
Y	254	-	-	/137	/36	/178
B*	69	-	-	/37	/13	-
N	254	-	-	/165	/36	-

Wind reactions based on MWFRS
 Y Brg Wid = 4.0 Min Req = 1.5 (Truss)
 B Brg Wid = 280 Min Req = -
 N Brg Wid = 4.0 Min Req = 1.5 (Truss)
 Bearings Y, B, & N are a rigid surface.
 Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; M5 2x4 SP #2;
 Stack Chord: SC1 2x4 SP #2;
 Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Loading

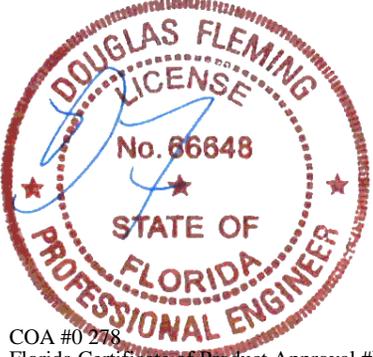
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/125.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.
 Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
 The overall height of this truss excluding overhang is 6-0-4.



COA #0 278
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 05/09/2024

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