

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS CONNECTIONS, AND ALL OTHER REQUIREMENTS FOR THE TRUSS. FOR ALL BEARING LOCATIONS, TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER DESIGNER. THE TRUSS ENGINEER IS THE RESPONSIBILITY OF THE TRUSS DESIGNER. THIS SHALL MEET ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THIS PLAN IS NOT A TRUSS DESIGNER'S PLAN. THIS PLAN IS FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH ENG 2X8 RAFTERS 700 LB EACH END.

**SITE PREPARATION:** SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OF SOILS TEST PROVES OTHERWISE).

**CONCRETE:** MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 2500$  PSI.

**WELDED WIRE REINFORCED SLABS:** 6" X 12" X 14" FIBER WELDED WIRE REINFORCEMENT FABRIC (WWM) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

**FIBER CONCRETE SLAB:** CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FABRIC LENGTH 1/2" HIGH TO 12" INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.00 OZ. PER CUBIC YARD. FIBER FABRIC MUST BE APPROVED FOR FOUNDATIONS. FIBERS TO COMPLY WITH ASTM C 1116, SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

**CONTROL JOINTS:** WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB POUR. THE JOINTS SHALL BE CROWNED TO THE SLAB THICKNESS. JOINTS 1.5 AND TYPICAL SPACINGS OF JOINTS TO BE 12'-0" TO 15'-0" CUT WMM OR REINFORCED STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL). THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKING, BUT RATHER TO ENSURE THE SLAB TO CRACK ON A GIVEN LINE.

**ANCHOR JOINTS:** TYPE 40 DEFORMED BARS,  $F_y = 40$  KSI ALL LAP SPACERS 40" DB (25" FOR BARS), UNO ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.C.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL CONDITIONS, AND ALL OTHER REQUIREMENTS FOR THE BUILDING.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSSES TO FOUNDATION. IF YOU BELIEVE YOU CAN Omit A CONTINUOUS LOAD-PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON THE SITE, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE CONSTRUCTION AND BEARINGS LOCATIONS. THE BUILDER SHALL CONFIRM THE MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND SPECIAL REQUIREMENTS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATENT BRACINGS. THE BUILDER SHOULD USE THE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DETAILED RESTRAINTS FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**FL PE 5395**  
This item has been officially signed and sealed by Mark Disway PE on [signature date].  
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**DESIGN CRITERIA & LOADS:**

**BUILDING CODE:** 8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)

**CODE FOR DESIGN LOADS:** ASCE 7-22

**WIND LOADS:** BASIC WIND SPEED (ASCE 7-23 GUST) 130 MPH

**WIND EXPOSURE:** (BUILDER MUST FIELD VERIFY) C

**TOPOGRAPHIC FACTOR:** (BUILDER MUST FIELD VERIFY) I

**RISK CATEGORY:** II

**ENCLOSURE CLASSIFICATION:** ENCLOSED

**INTERNAL PRESSURE COEFFICIENT:** 0.18

**ROOF ANGLE:** 7.45 DEGREES

**MEAN ROOF HEIGHT:** 30 FT

**C&C DESIGN PRESSURES:** SEE TABLE

**FLOOR LOADING:** ROOMS OTHER THAN SLEEPING ROOM 40 PSF LIVE LOAD SLEEPING ROOMS 30 PSF LIVE LOAD

**ROOF LOADING:** FLAT OR 4:12 TO 12:12 16 PSF LIVE LOAD 12:12 & GREATER 12 PSF LIVE LOAD

**SOIL BEARING CAPACITY:** 1500 PSF

**FLOOD ZONE:** THIS BUILDING IS NOT IN THE FLOOD ZONE

**COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)**

**EFFECTIVE WIND AREA (FT<sup>2</sup>)**

0-20	+25.6(Vest)	-25.6(Vest)	+25.6(Vest)	-34.2(Vest)
0-20	+42.6(Vest)	-46.2(Vest)	+42.6(Vest)	-57(Vest)

**GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)**

9x7 GARAGE DOOR	+22.6(Vest)	-25.5(Vest)
16x7 GARAGE DOOR	+21.7(Vest)	-24.1(Vest)

**JOB NUMBER:** 250971

**S-1**

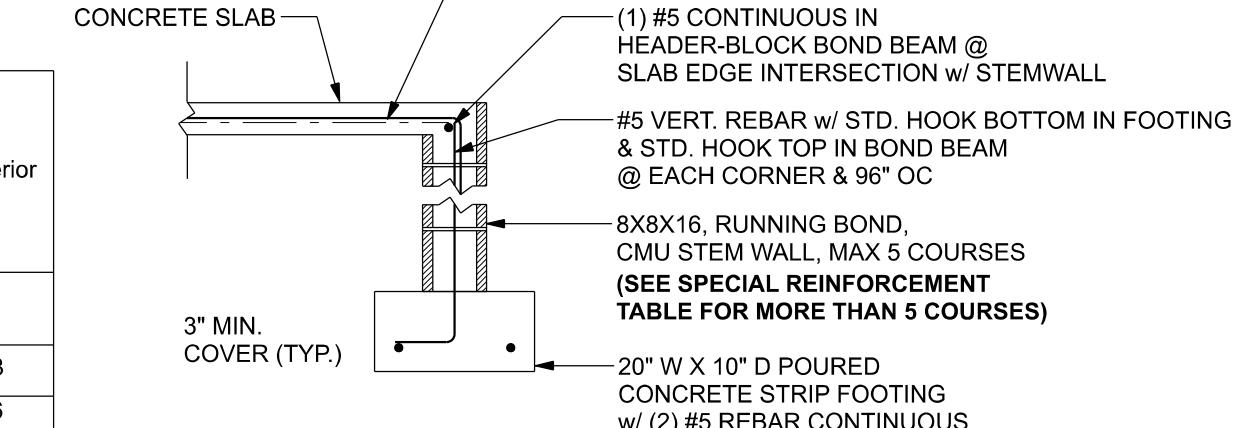
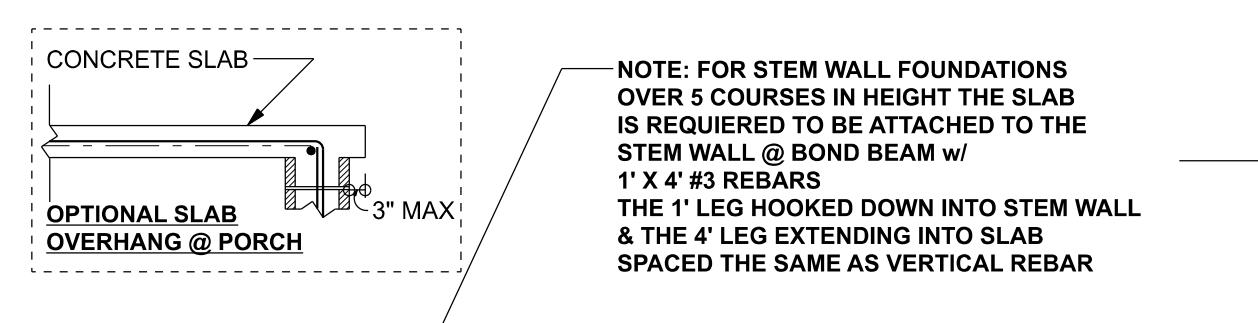
OF 3 SHEETS

**TALL STEM WALL TABLE:**  
The table assumes 40 ksf for #5 rebar and 60 ksf for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#7	#8
3.3	3.0	96	96	96	96	96
4.0	3.7	96	96	96	96	96
4.7	4.3	88	96	96	96	96
5.3	5.0	56	96	96	96	96
6.0	5.7	40	80	96	80	96
6.7	6.3	32	56	80	56	96

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT  
SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION  
FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602).  
THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE  
PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS  
BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.  
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY  
THE ENGINEER IN WRITING.

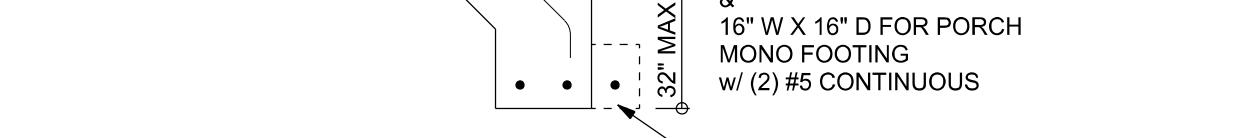
ACI530.1-02 Section	Specific Requirements
1.4A. Reinforcing bars, #3 - #11	3" block bearing walls $F_m = 1500$ psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 30-02, Normal weight, Hollow medium surface finish, 8" x 16" running bond and 12" x 12" or 16" x 16" column blocks
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5" x 2.75" x 11.5"
2.4 Reinforcing bars, #3 - #11	ASTM B 115, Grade 40, $F_y = 40$ ksf, Lap splices 10" O.D. min.
2.4F Coating for corrosion protection	Aluminum, sheet metal ties completely embedded in mortar or grout ASTM A525, Class G60, 0.60 oz/in <sup>2</sup> or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to weather, sheet metal ties completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/in <sup>2</sup> or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



**STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



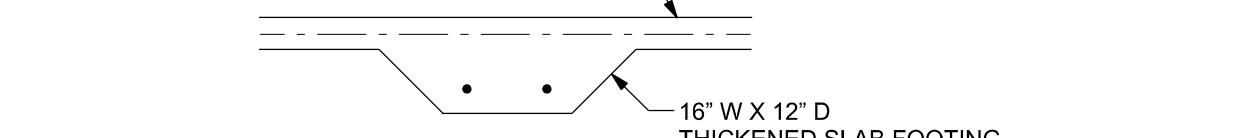
**OPTIONAL MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



**OPTIONAL MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



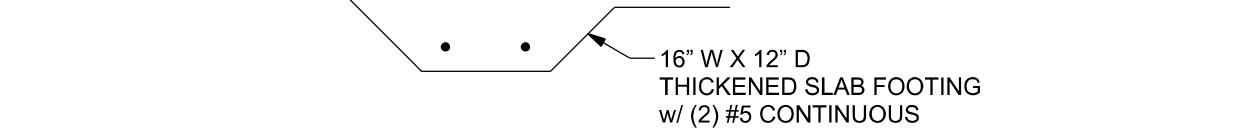
**INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



**INTERIOR BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"



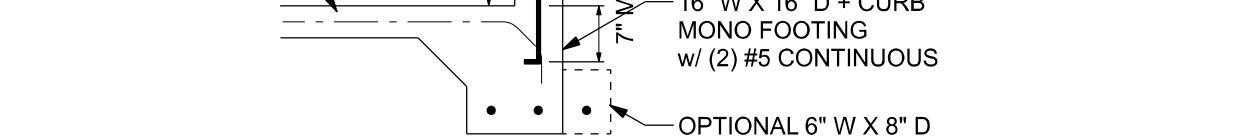
**OPTIONAL MONOLITHIC CURB FOOTING**  
SCALE: 1/2" = 1'-0"



**OPTIONAL MONOLITHIC CURB FOOTING**  
SCALE: 1/2" = 1'-0"



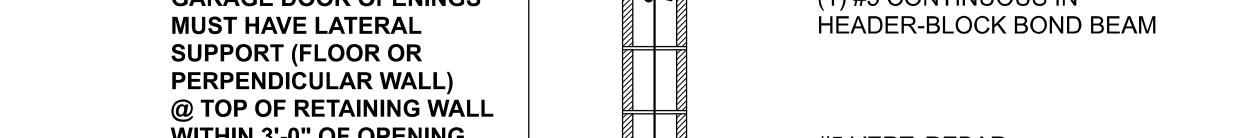
**STEM WALL CURB FOOTING**  
SCALE: 1/2" = 1'-0"



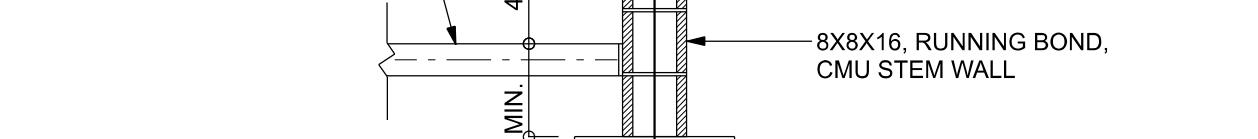
**STEM WALL CURB FOOTING**  
SCALE: 1/2" = 1'-0"



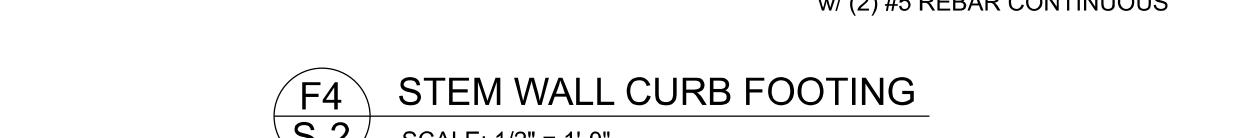
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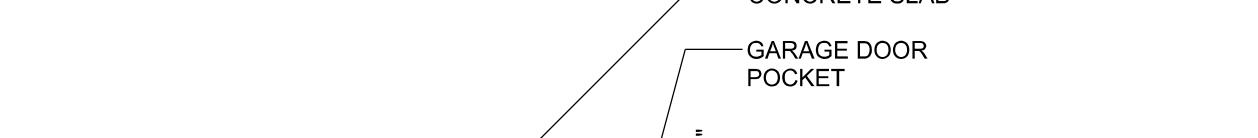
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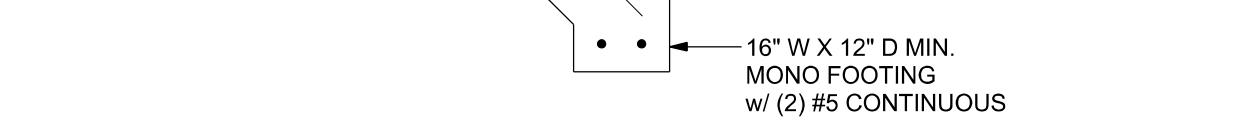
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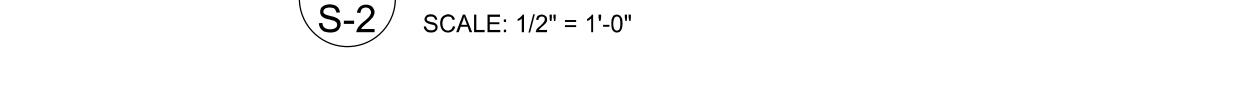
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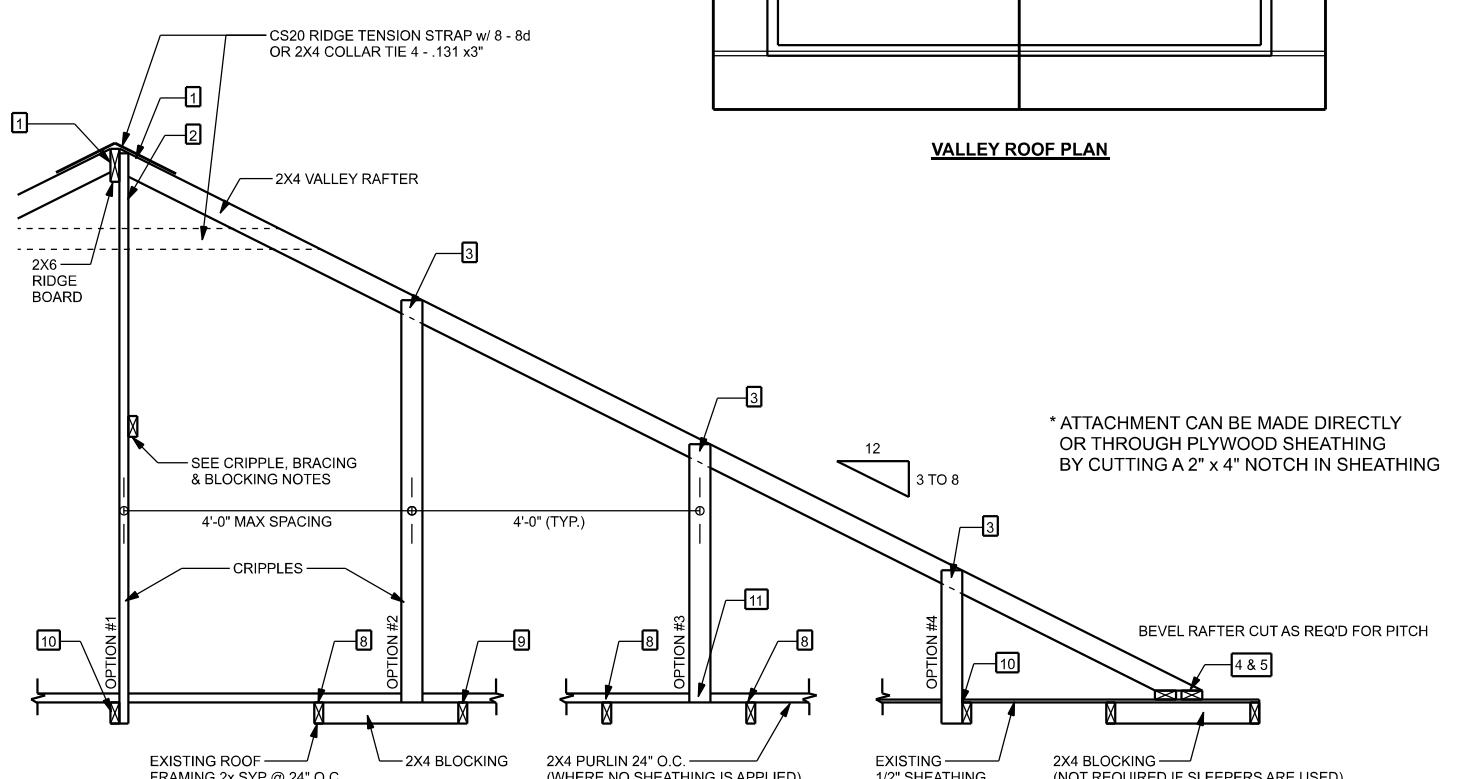
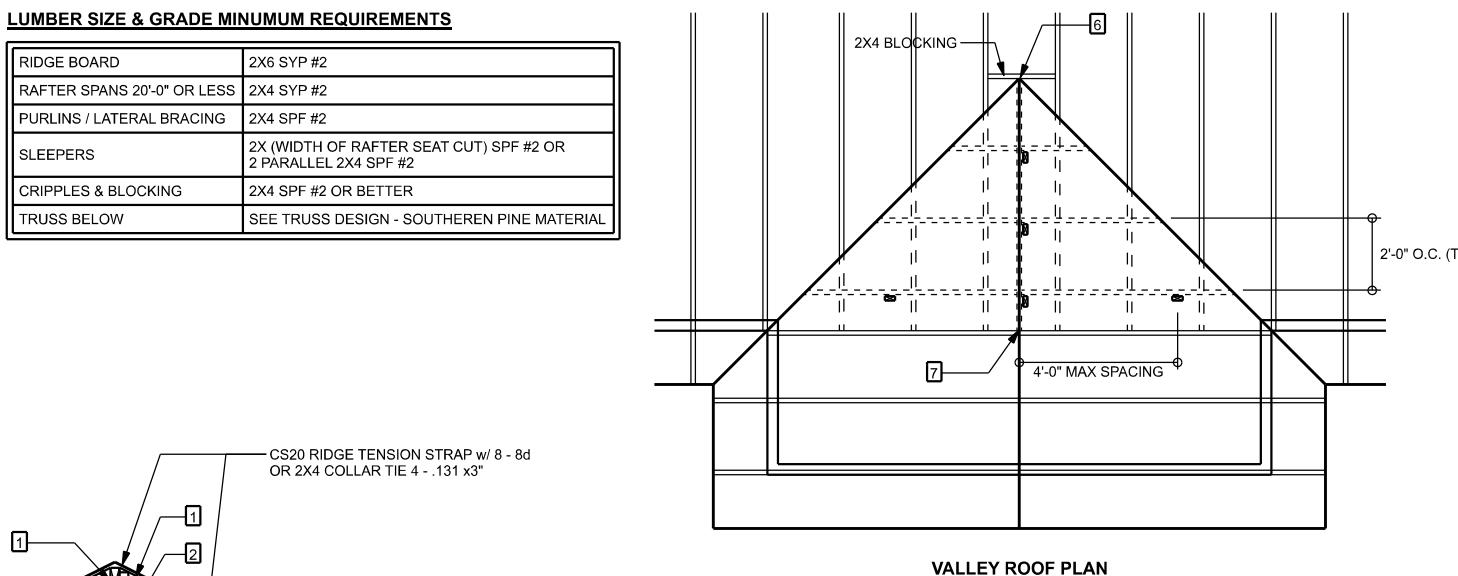


**STEM WALL CURB FOOTING**  
SCALE: 1/2" = 1'-0"



**STEM WALL CURB FOOTING**  
SCALE: 1/2" = 1'-0"

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER-SLEEPER CUT) SPF #2 OR 2 PARALLEL 2X4 SPF #2
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- VALLEY RAFTER OR RIDGE
- CRIPPLE

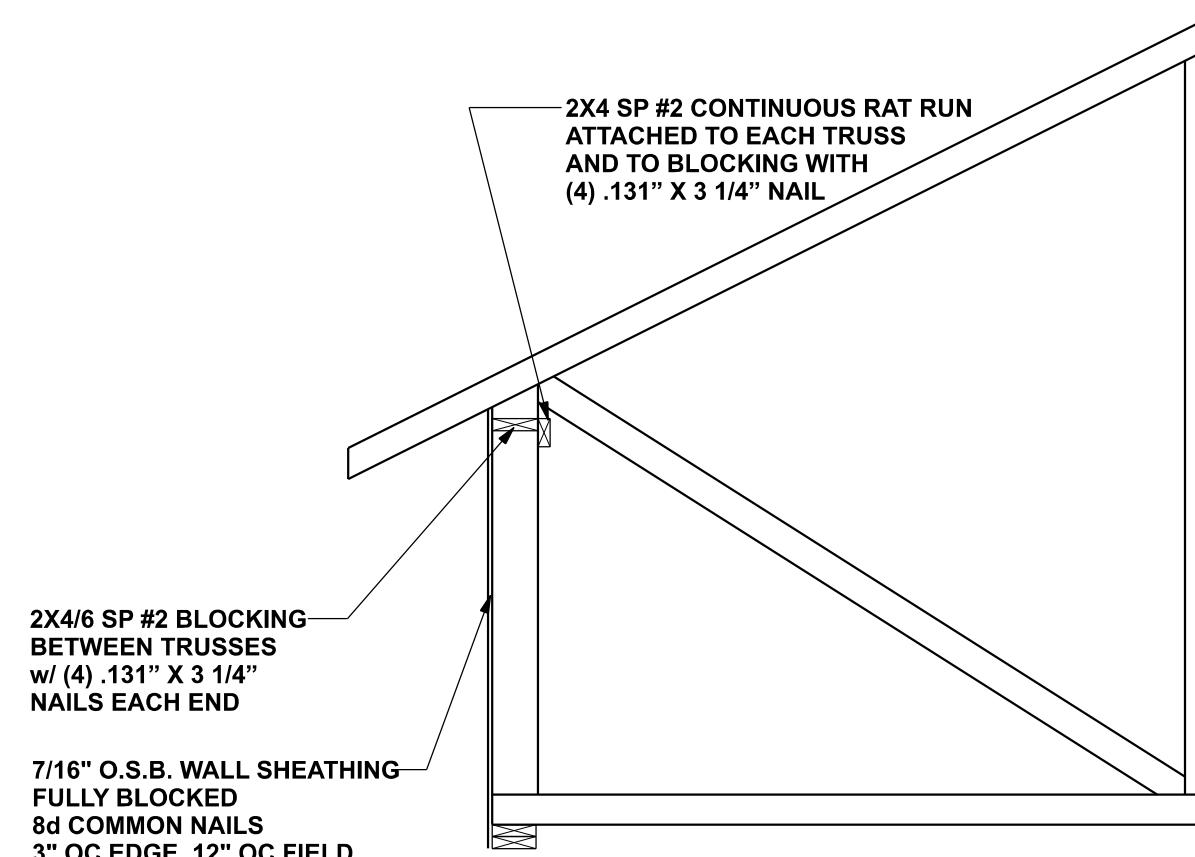
CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1. 2X4 RAFTERS TO RIDGE	4 - 131 x 3" TOE NAILS
2. CRIPPLE TO RIDGE	4 - 131 x 3" FACE NAILS
3. CRIPPLE TO RAFTERS	4 - 131 x 3" FACE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	4 - 131 x 3" TOE NAILS
5. SLEEPER TO TRUSS	4 - 131 x 3" FACE NAILS
6. RIDGE BOARD TO ROOF BLOCK	4 - 131 x 3" TOE NAILS
7. RIDGE BOARD TO TRUSS	4 - 131 x 3" TOE NAILS
8. PURLIN TO TRUSS (TYP.)	4 - 131 x 3" NAILS
9. PURLIN TO CRIPPLE (IF CRIPPLE IS ATTACHED TO PURLIN)	6 - 131 x 3" NAILS
10. TRUSS TO BLOCKING	4 - 131 x 3" END NAILS
11. CRIPPLE TO TRUSS	4 - 131 x 3" FACE NAILS
12. CRIPPLE TO PURLIN	4 - 131 x 3" FACE NAILS

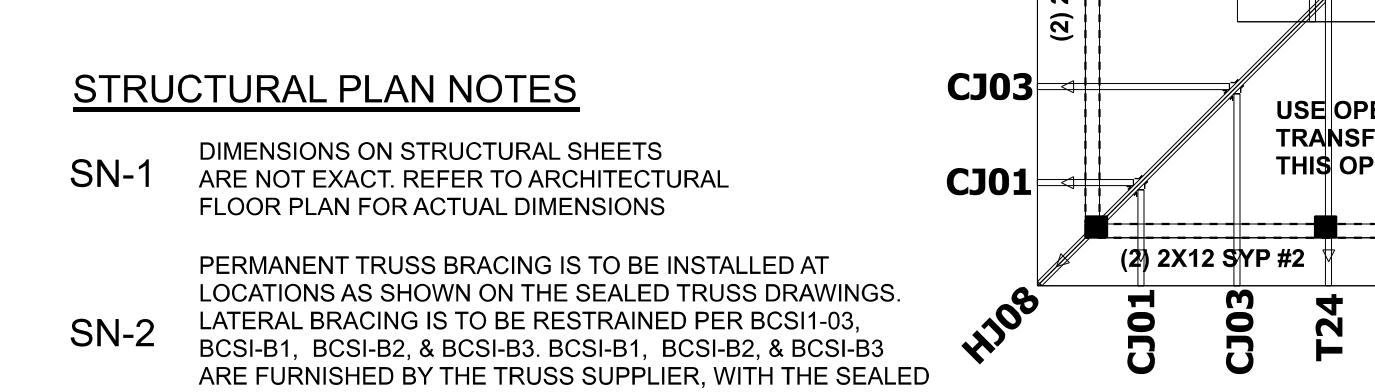
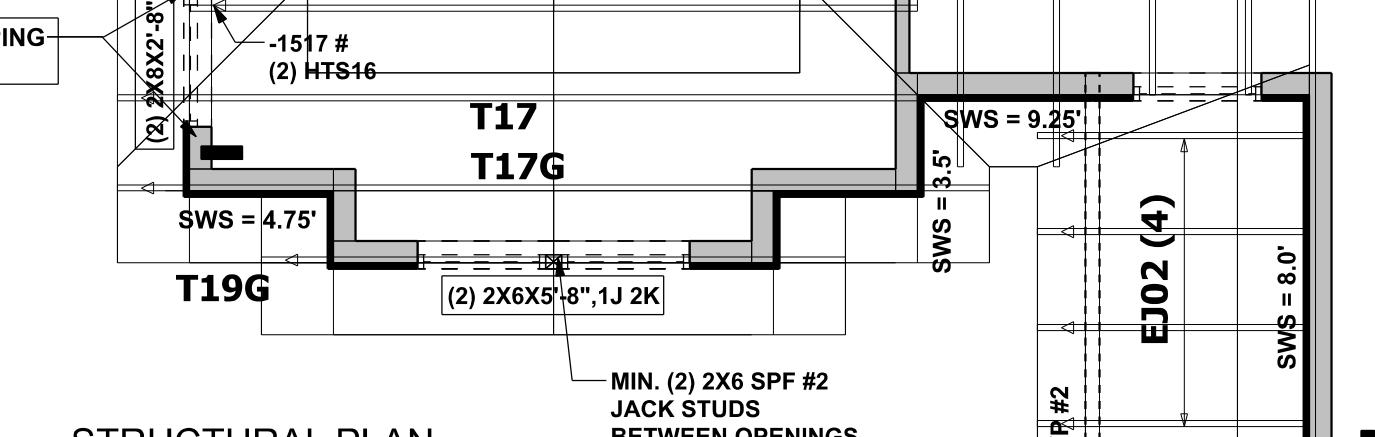
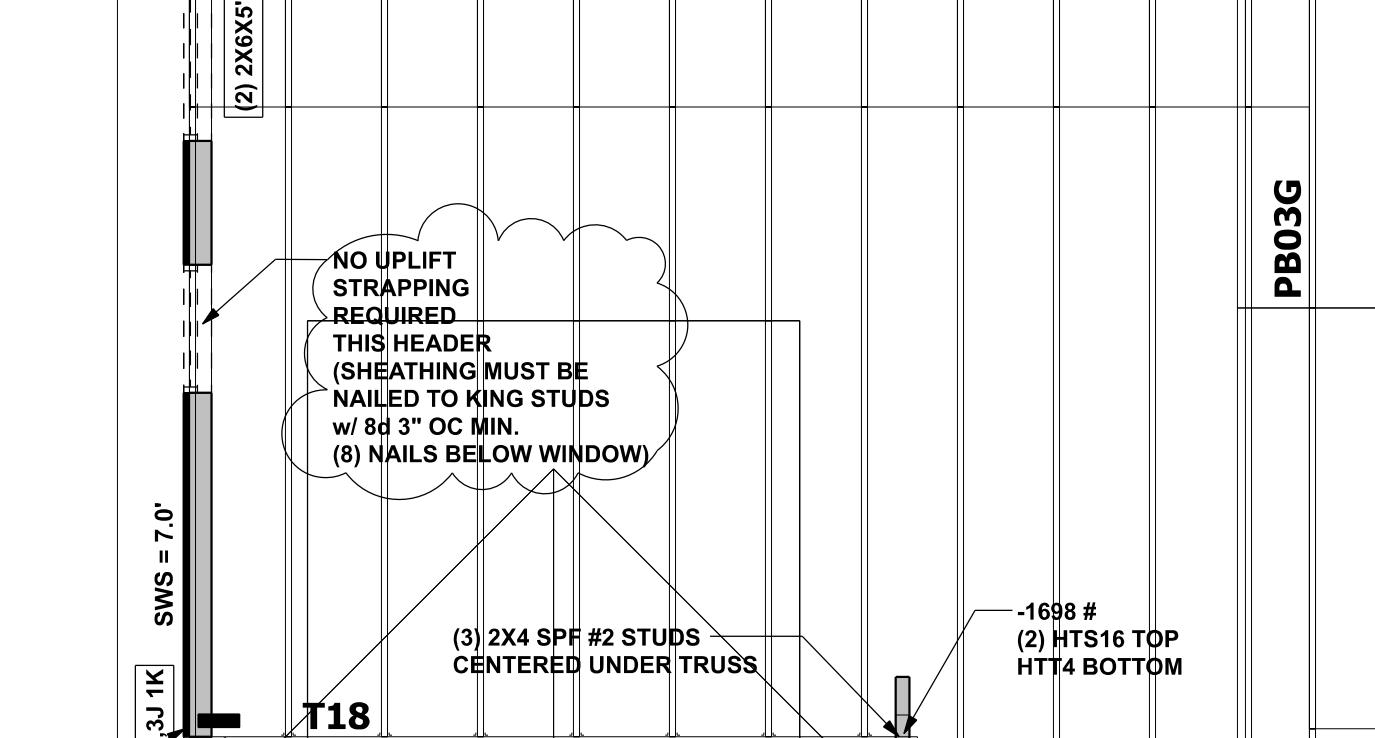
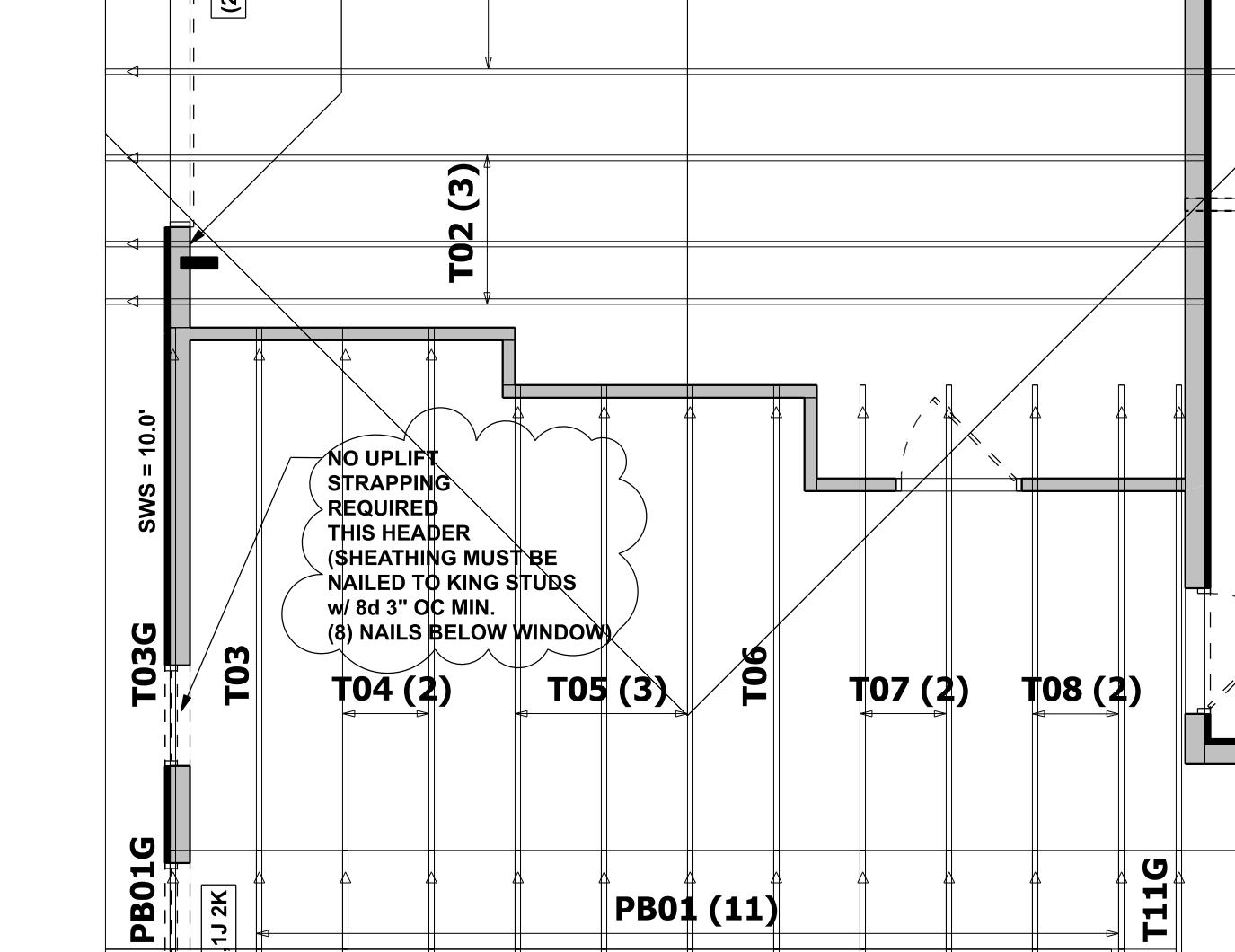
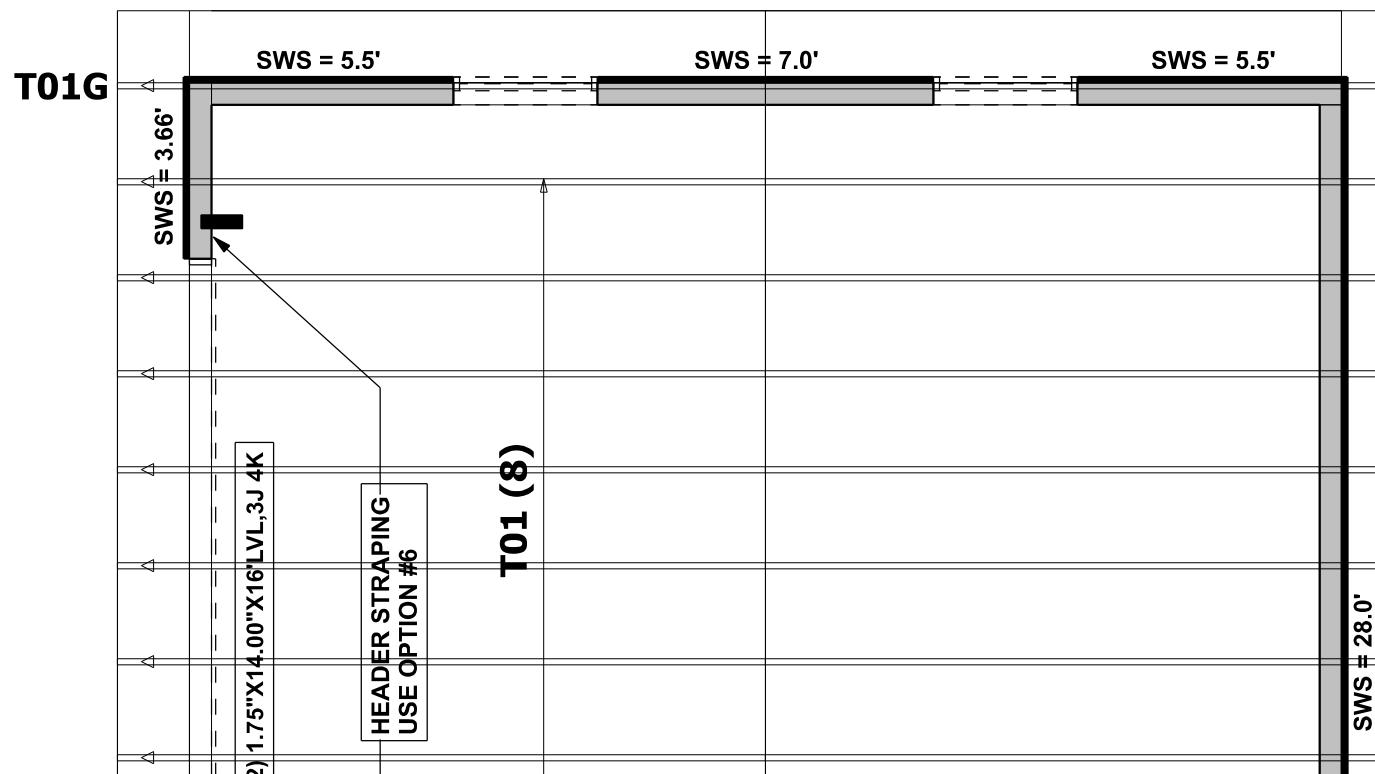
ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.



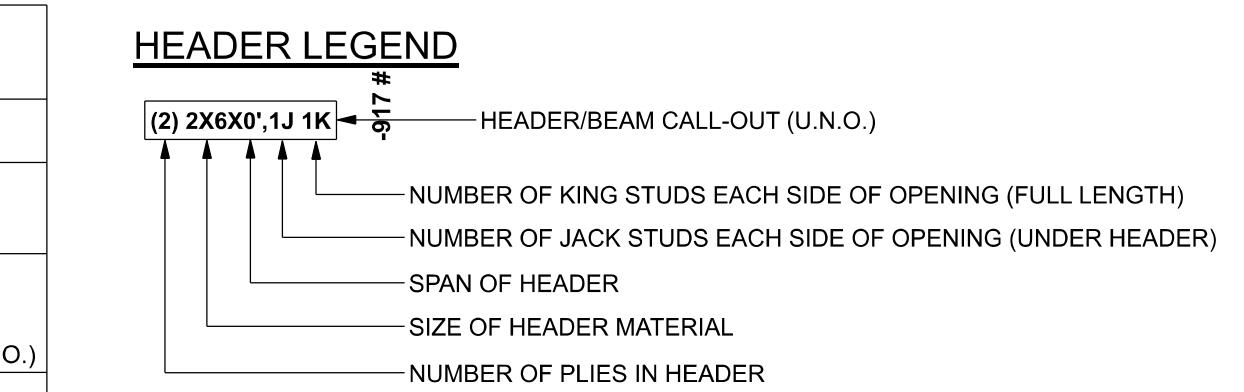
DETAIL @ TRUSSES WITH RAISED HEELS

SCALE: 1/2" = 1'-0"



UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS)	
***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL BE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



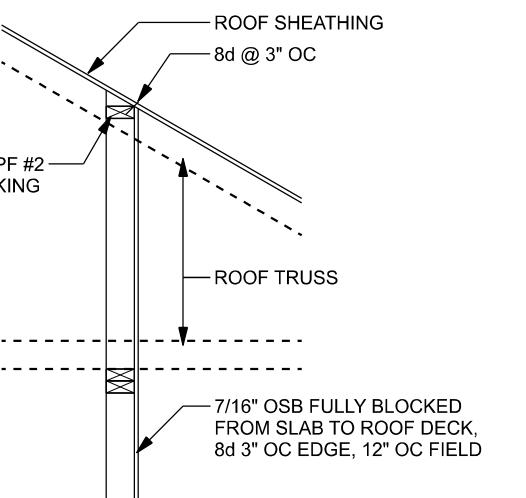
ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	31116 LBF	19557 LBF
REQUIRED	24970 LBF	18365 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER. BUILDERS FIRST SOURCE  
JOB #4894417

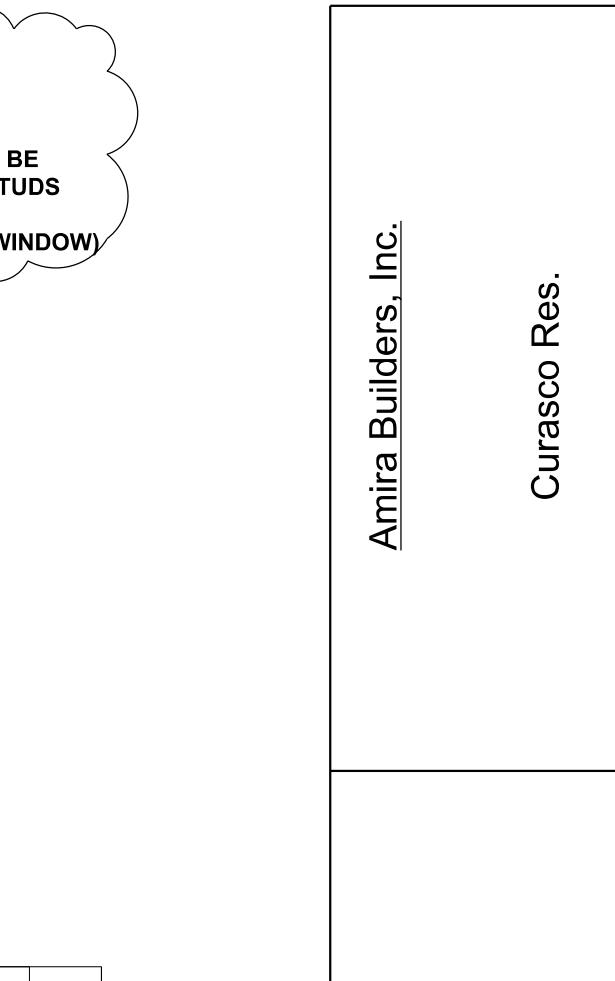
JOB NUMBER:  
250971

S-3  
OF 3 SHEETS



ALTERNATE IF TRUSSES  
ARE PERPENDICULAR TO  
SHEARWALL

NOTE:  
IF THE ABOVE DETAIL IS USED  
ON THE REAR PORCH WALL  
THE REAR PORCH CEILING DOES NOT  
NEED TO BE SHEATHED



Amra Builders, Inc.

Curasco Res.

PROJECT ADDRESS:  
316 SW Cypresswood Glen, Lake City, FL

FL PE E3016

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CERTIFICATION: I hereby certify that I have  
checked the plans and specifications for any applicable  
portions of the plan, relating to wind engineering  
comply with the 8th Edition Florida  
Building Code (2023) to the best of my knowledge.

LIMITATION: This design is valid for one  
building, at specified location.

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