

Columbia County Building Permit Application

C# 10664

12th meglewe
1/19/07

For Office Use Only Application # 070-29 Date Received 1/9 By TW Permit # 25444
 Application Approved by - Zoning Official BLK Date 1/19/07 Plans Examiner OKTH Date 1-19-07
 Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments Section 2.3.1 Legal Non-conforming Lot of Record Lot 58
☒ NOC ☒ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Permit

Name Authorized Person Signing Permit Ronald Clark Phone 352-538-6929
 Address 15816 NW CR 1491, Alachua, FL 32615

Owners Name Ray & Darlene Findley Phone 386-454-2781
 911 Address 194 SW Otter, Ft. White, FL 32038

Contractors Name Ronald Clark Construction, Inc. Phone 352-538-6929
 Address 15816 NW CR 1491, Alachua, FL 32615

Fee Simple Owner Name & Address _____
 Bonding Co. Name & Address _____

Architect/Engineer Name & Address SCHAFER ENGINEERING LLC - (RONALD CLARK - Draftsman)
 Mortgage Lenders Name & Address BANK OF AMERICA

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 19-75-17-10024-056 Estimated Cost of Construction 234,000
 Subdivision Name Sassafras Acres Lot 58 Block _____ Unit _____ Phase _____

Driving Directions Take 47 to Ft. White, turn left on 27 toward High Springs, turn right on 138, turn right on Bobcat, turn left on Otter, 200 yds, job on left.

Type of Construction Single family dwelling Number of Existing Dwellings on Property _____
 Total Acreage 2 1/2 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 150' Side 120 Side 120 Rear 100
 Total Building Height 16' Number of Stories 1 Heated Floor Area 2831 Roof Pitch 6/12
 TOTAL 3325

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

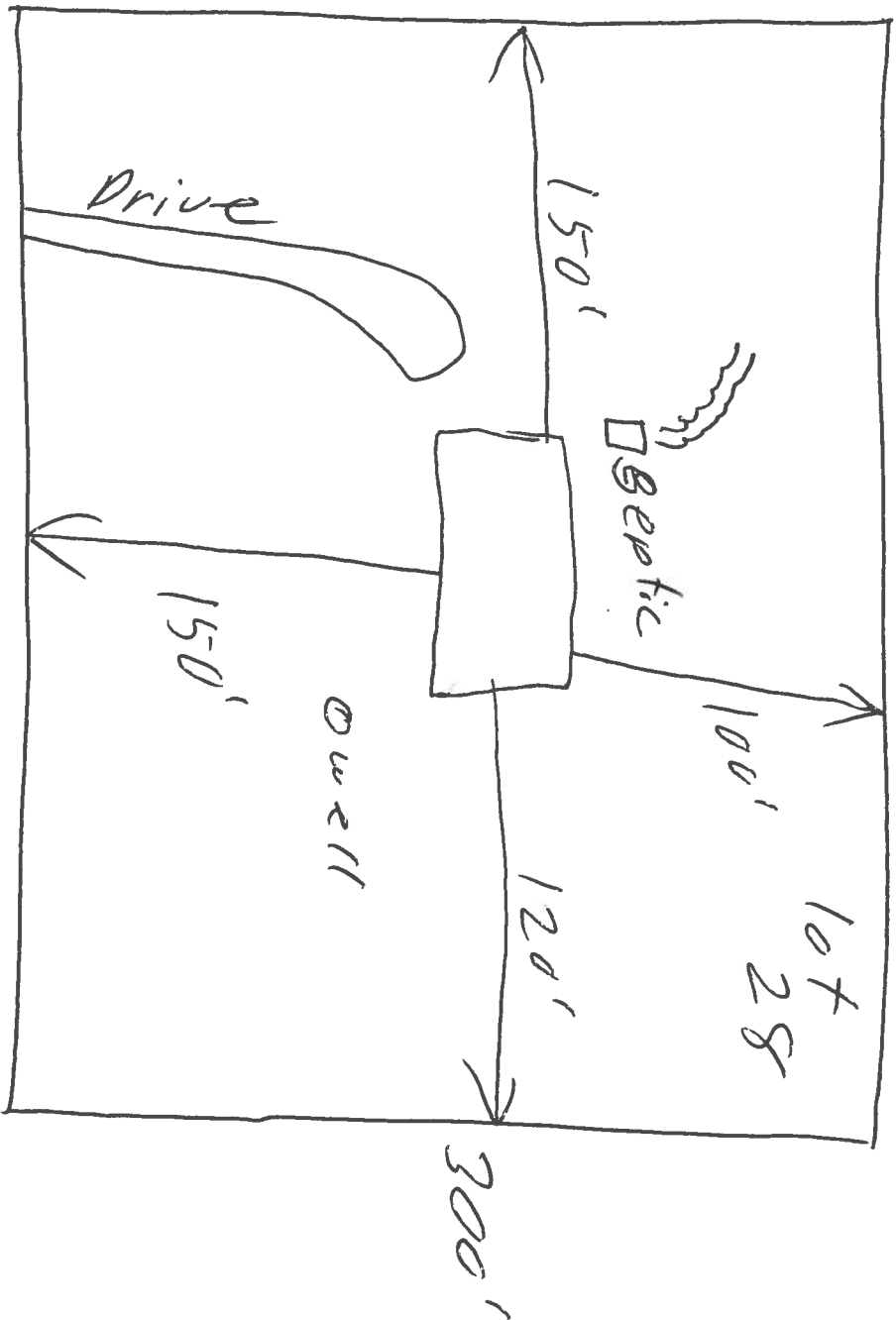
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Notarized Letter
 STATE OF FLORIDA
 COUNTY OF COLUMBIA
 Sworn to (or affirmed) and subscribed before me this 4 day of Jan 2007.
 Personally known ☒ or Produced Identification _____
 Contractor Signature Ronald Clark
 Contractors License Number CRC1326560
 Competency Card Number _____
 NOTARY STAMP/SEAL
 Notary Signature Bonnie P. Presnell
 (Revised Sept. 2006)

BONNIE P. PRESNELL
 Notary Public, State of Florida
 My comm. exp. Mar. 1, 2008
 Comm. No. DD 277528

326'

Finley
Job



Lot 58 Sassafraes Acres
Tax # 19-78-17-10024-056

Ronald Clark
Construction

This Warranty Deed Made and executed the 11th day of November A.D. 1981 by

DLC CATTLE CO., INC.

a corporation existing under the laws of State of Florida, and having its principal place of business at Columbia County in the State of Florida hereinafter called the grantor, to

RAYMOND E. FINDLEY

whose postoffice address is 25 Bennett Rd., Opa-Locka, Florida 33054

hereinafter called the grantees:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth: That the grantor, for and in consideration of the sum of \$ 10.00 and other valuable considerations, receipt whereof is hereby acknowledged, by these presents does grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees, all that certain land situate in Columbia County, Florida, viz:

Lot 58

Lot 58 of SASSAFRAS ACRES, A subdivision located in Sections 19 and 30, Township 7 South, Range 17 East, Columbia County, Florida, as recorded in Plat Book 4, Pages 8 and 8-A of the public records of Columbia County, Florida

Make sure permits are for Lot 58
not 56 or 57

Together with all the tenements, hereditaments and appurtenances thereto belonging or in any-wise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantees that it is lawfully seized of said land in fee simple; that it has good right and lawful authority to sell and convey said land; that it hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances

DOCUMENTARY STAMP

INTANGIBLE TAX

P. DEWITT CASON, CLERK OF
COURTS, COLUMBIA COUNTY

BY P. C.

(CORPORATE SEAL)

In Witness Whereof the grantor has caused these presents to be executed in its name, and its corporate seal to be hereunto affixed, by its proper officers thereunto duly authorized, the day and year first above written.

ATTEST: William D. Begg
William D. Begg Secretary

DLC CATTLE CO., INC.

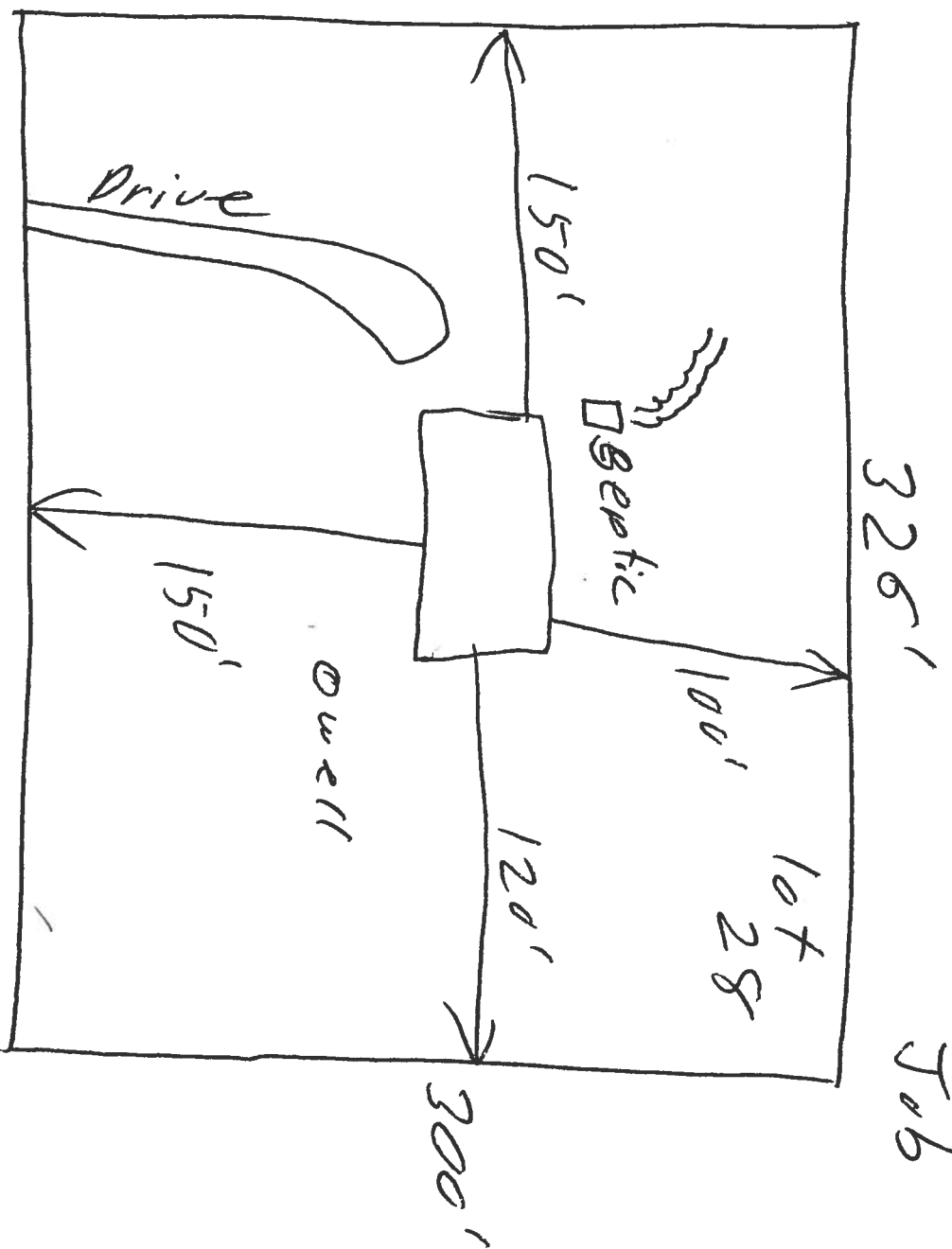
Signed, sealed and delivered in the presence of:

Bettrice D. Robinson
Name P. A. H.

By Rodney S. Dicks
Rodney S. Dicks President

Finley

Job



Lot 58 Sassafras Acres
Tax # 19-75-17-10024-056

Ronald Clark
Construction

CLYATT WELL DRILLING, INC.*Established in 1971**Post Office Box 180**Worthington Springs, Florida 32697**Phone (386)496-2488 FAX (386)496-4640***INVOICE DATE****2/26/2003****INVOICE NUMBER****4INWELL****DUE AND PAYABLE UPON RECEIPT****CUSTOMER NAME AND ADDRESS**

Ronald Clark Construction, Inc.
 Attn.: Ronald Clark
 15816 Northwest County Road 1491
 Alachua, Florida 32615

DESCRIPTION OF WORK**4" Well and Pump**

QTY	DESCRIPTION	PRICE	SUB-TOTAL
1	Feet 4" Well		
1	1 HP Submersible Pump (20 GPM)		
	1-1/4" Galvanized Pipe		
	14/3 Submersible Pump Wire With Ground		
1	WF255 (220 Gallon Equivalent) Tank (20 GPM Draw Down)		
1	4 X 1-1/4 Well Seal		
1	Pressure Relief Valve		
	Controls & Fittings		
Sales Tax @ (7.0%)			

THANK YOU FOR YOUR BUSINESS! Payment is due when work is completed. All materials remain the property of Clyatt Well Drilling, Inc., until paid for in full. Clyatt Well Drilling, Inc. does not agree to find or develop water, nor does it represent, warrant or guarantee the quality or kind of water which may be encountered. If it is necessary to install water filters, the owner agrees it is his/her responsibility to pay the cost. Please note that a surcharge equivalent to 18% per annum, will be assessed on any outstanding balance after 30 Days. Right to repossess is granted.

PLEASE PAY THIS AMOUNT

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: **FINLEY**
Address:
City, State: ,
Owner:
Climate Zone: **North**

Builder: **RON CLARK CONST.**
Permitting Office: **COLUMBIA**
Permit Number: **25444**
Jurisdiction Number: **221008**

1. New construction or existing New ☐
2. Single family or multi-family Single family ☐
3. Number of units, if multi-family 1 ☐
4. Number of Bedrooms 3 ☐
5. Is this a worst case? Yes ☐
6. Conditioned floor area (ft²) 2831 ft² ☐
7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default)
 - a. U-factor: Description Area
 - (or Single or Double DEFAULT) 7a. (Dble Default) 202.0 ft² ☐
 - b. SHGC: 7b. (Clear) 362.0 ft² ☐
 - (or Clear or Tint DEFAULT)
8. Floor types
 - a. Slab-On-Grade Edge Insulation R=0.0, 294.0(p) ft ☐
 - b. N/A ☐
 - c. N/A ☐
9. Wall types
 - a. Frame, Wood, Exterior R=13.0, 2542.0 ft² ☐
 - b. N/A ☐
 - c. N/A ☐
 - d. N/A ☐
 - e. N/A ☐
10. Ceiling types
 - a. Under Attic R=30.0, 2831.0 ft² ☐
 - b. Under Attic R=19.0, 144.0 ft² ☐
 - c. N/A ☐
11. Ducts
 - a. Sup: Unc. Ret: Unc. AH: Interior Sup. R=6.0, 288.0 ft ☐
 - b. N/A ☐

12. Cooling systems
 - a. Central Unit Cap: 56.6 kBtu/hr ☐
SEER: 13.00 ☐
 - b. N/A ☐
 - c. N/A ☐
13. Heating systems
 - a. Electric Heat Pump Cap: 55.0 kBtu/hr ☐
HSPF: 8.00 ☐
 - b. N/A ☐
 - c. N/A ☐
14. Hot water systems
 - a. Electric Resistance Cap: 50.0 gallons ☐
EF: 0.92 ☐
 - b. N/A ☐
 - c. Conservation credits ☐
(HR-Heat recovery, Solar
DHP-Dedicated heat pump)
15. HVAC credits ☐
(CF-Ceiling fan, CV-Cross ventilation,
HF-Whole house fan,
PT-Programmable Thermostat,
MZ-C-Multizone cooling,
MZ-H-Multizone heating)

Glass/Floor Area: 0.13

Total as-built points: 33122

Total base points: 34799

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

SUNCOAST INSULATORS
825 NW 253rd Terrace
Newberry, FL 32869
(352) 472-8895
Fax (352) 472-2833

PREPARED BY: _____

DATE: 12/5/04

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Ronald ClarkDATE: 12-15-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area				Type/SC Overhang Ormt Len Hgt Area X SPM X SOF = Points							
.18	2831.0	18.59	9473.0	1.Double, Clear	N	2.0	6.0	40.0	19.20	0.90	691.0
				2.Double, Clear	E	2.0	6.0	132.0	42.06	0.85	4708.0
				3.Double, Clear	S	2.0	6.0	30.0	35.87	0.78	835.0
				4.Single, Clear	W	2.0	6.0	160.0	43.84	0.85	5957.0
				As-Built Total:				362.0		12191.0	
WALL TYPES Area X BSPM = Points				Type		R-Value		Area X SPM = Points			
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior		13.0		2542.0	1.50		3813.0
Exterior	2542.0	1.70	4321.4								
Base Total:	2542.0		4321.4	As-Built Total:				2542.0	3813.0		
DOOR TYPES Area X BSPM = Points				Type				Area X SPM = Points			
Adjacent	0.0	0.00	0.0	1.Exterior Wood				36.0	6.10		219.6
Exterior	36.0	6.10	219.6								
Base Total:	36.0		219.6	As-Built Total:				36.0	219.6		
CEILING TYPES Area X BSPM = Points				Type		R-Value		Area X SPM X SCM = Points			
Under Attic	2831.0	1.73	4897.6	1. Under Attic		30.0		2831.0	1.73 X 1.00		4897.6
				2. Under Attic		19.0		144.0	2.34 X 1.00		337.0
Base Total:	2831.0		4897.6	As-Built Total:				2975.0	5234.6		
FLOOR TYPES Area X BSPM = Points				Type		R-Value		Area X SPM = Points			
Slab	294.0(p)	-37.0	-10878.0	1. Slab-On-Grade Edge Insulation		0.0		294.0(p)	-41.20		-12112.8
Raised	0.0	0.00	0.0								
Base Total:			-10878.0	As-Built Total:				294.0	-12112.8		
INFILTRATION Area X BSPM = Points								Area X SPM = Points			
	2831.0	10.21	28904.5					2831.0	10.21		28904.5

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 36938.1				Summer As-Built Points: 38249.9						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier	X Credit Multiplier	=	Cooling Points
				(sys 1: Central Unit 56600btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)						
36938.1	0.3250		12004.9	38250	1.00	(1.09 x 1.147 x 0.91)	0.260	1.000		11314.5
				38249.9	1.00	1.138	0.260	1.000		11314.5

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area				Type/SC Overhang Ormt Len Hgt Area X WPM X WOF = Points							
.18	2831.0	20.17	10278.0	1.Double, Clear	N	2.0	6.0	40.0	24.58	1.00	987.0
				2.Double, Clear	E	2.0	6.0	132.0	18.79	1.06	2630.0
				3.Double, Clear	S	2.0	6.0	30.0	13.30	1.26	502.0
				4.Single, Clear	W	2.0	6.0	160.0	28.84	1.04	4811.0
				As-Built Total:				362.0		8930.0	
WALL TYPES Area X BWPM = Points				Type		R-Value		Area X WPM = Points			
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior		13.0		2542.0		3.40 8642.8	
Exterior	2542.0	3.70	9405.4								
Base Total:		2542.0	9405.4	As-Built Total:				2542.0		8642.8	
DOOR TYPES Area X BWPM = Points				Type				Area X WPM = Points			
Adjacent	0.0	0.00	0.0	1.Exterior Wood				36.0		12.30 442.8	
Exterior	36.0	12.30	442.8								
Base Total:		36.0	442.8	As-Built Total:				36.0		442.8	
CEILING TYPESArea X BWPM = Points				Type		R-Value		Area X WPM X WCM = Points			
Under Attic	2831.0	2.05	5803.5	1. Under Attic		30.0		2831.0		2.05 X 1.00 5803.5	
				2. Under Attic		19.0		144.0		2.70 X 1.00 388.8	
Base Total:		2831.0	5803.5	As-Built Total:				2975.0		6192.3	
FLOOR TYPES Area X BWPM = Points				Type		R-Value		Area X WPM = Points			
Slab	294.0(p)	8.9	2616.6	1. Slab-On-Grade Edge Insulation		0.0		294.0(p)		18.80 5527.2	
Raised	0.0	0.00	0.0								
Base Total:			2616.6	As-Built Total:				294.0		5527.2	
INFILTRATION Area X BWPM = Points								Area X WPM = Points			
		2831.0	-0.59					2831.0		-0.59 -1670.3	

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Winter Base Points:		26876.1		Winter As-Built Points:				28064.9		
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
				(sys 1: Electric Heat Pump 55000 btuh , EFF(8.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0						
26876.1	0.5540	14889.3		28064.9	1.000	(1.069 x 1.169 x 0.93)	0.426	1.000	13902.8	
				28064.9	1.00	1.162	0.426	1.000	13902.8	

WATER HEATING & CODE COMPLIANCE STATUS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit	= Total Multiplier
3		2635.00	7905.0	50.0	0.92	3		1.00	2635.00	1.00 7905.0
				As-Built Total:						7905.0

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+ Hot Water Points = Total Points
12005		14889		7905	34799	11314		13903	7905 33122

PASS

R16R
R17E

ZONE X

19

ZONE X

ZONE AE



20

27

ZONE X

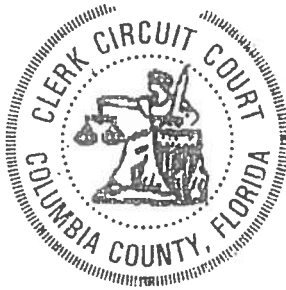
30

ZONE X

ZONE AE

44
ZONE X

PREPARED BY:
Charles Hill
Robertson & Anschutz
10333 Richmond Avenue, Suite 550
Houston, TX 77042



STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS

By *Donnie Rose*
Deputy Clerk
Date 1/18/07

AFTER RECORDED RETURN TO:

Bank of America, N.A.
9000 Southside Blvd., Ste. 700
Jacksonville, FL 32256

Inst:2007001338 Date:01/18/2007 Time:11:42
DC, P. DeWitt Cason, Columbia County B:1108 P:578

NOTICE OF COMMENCEMENT

Permit No. _____

Tax Folio No. _____

State of Florida
County of Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Legal description of property (include street address, if available)
**194 Southwest Otter Lane
Fort White, FL 32038**

See Exhibit "A" attached hereto and made a part hereof for all purposes
2. General description of improvement(s)
Construction of custom home
3. Owner information
Name: **Raymond E. Findley and Darlene Findley, husband and wife**
Address: **Post Office Box 785
High Springs, FL 32655**
4. Contractor information
Name: **Ronald Clark Construction, Inc.**
Address: **15816 Northwest Cr 1491
Alachua FL 32615**
Phone: **352-538-6929**
5. Surety
Name: _____
Address: _____
Phone #: _____ Fax #: _____ Amt. of bond: _____
6. Lender
Name: **Bank of America, N.A.**
Address: **1201 Main Street, 11th Floor, Dallas, TX 75202-0000**
Phone #: **877-719-6142**

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes

Name: _____

Address: _____

Phone #: _____

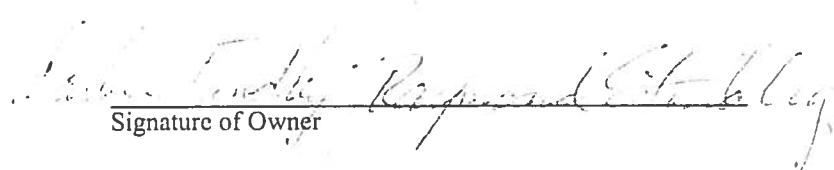
Fax #: _____

8. In addition to himself, Owner designates _____ of _____
_____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

Phone #: _____

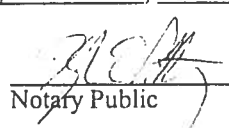
Fax #: _____

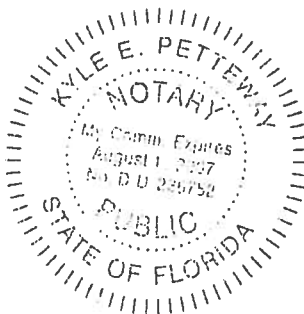
9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified).


Signature of Owner

Sworn to and subscribed before me this 16th day of January, 2007.

My commission expires:


Notary Public



Inst:2007001338 Date:01/18/2007 Time:11:42

_____DC,P.DeWitt Cason,Columbia County B:1108 P:579

Loan No.: 6170871534

EXHIBIT "A"

Lot 58 of Sassafras Acres, according to the Plat thereof as recorded in Plat Book 4, Page 8, of the Public Records of Columbia County, Florida

Inst:2007001338 Date:01/18/2007 Time:11:42

_____DC,P.Dewitt Cason,Columbia County B:1108 P:580



Log On

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4:23:05 PM 12/7/2006

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Licensee Details

Licensee Information

Name: **CLARK, RONALD WAYNE (Primary Name)**
RONALD CLARK CONSTRUCTION INC (DBA Name)
Main Address: **15816 NW CR 1491**
ALACHUA Florida 32615
County: **ALACHUA**

License Mailing:

LicenseLocation:

License Information

License Type: **Certified Residential Contractor**
Rank: **Cert Residential**
License Number: **CRC1326560**
Status: **Current,Active**
Licensure Date: **11/18/2002**
Expires: **08/31/2008**

Special Qualifications	Qualification Effective
Bldg Code Core Course Credit	
Qualified Business License Required	02/20/2004

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LATERAL TOE-NAIL DETAIL

ST-TOENAIL

MITek Industries, Chesterfield, MO Page 1 of 1

NOTES:

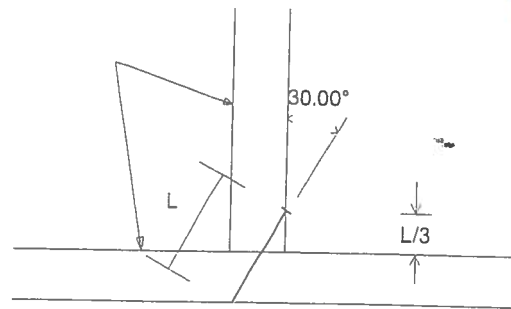
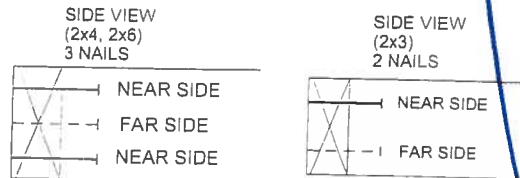
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

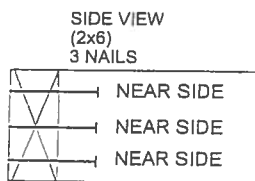
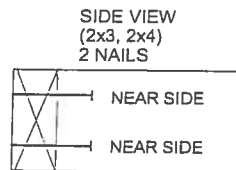
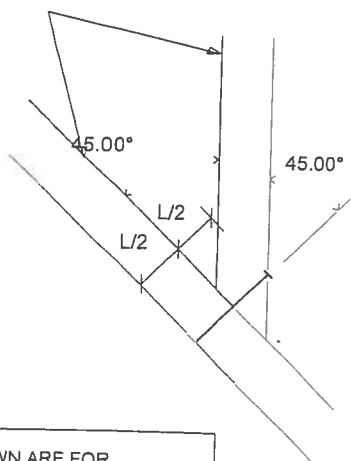
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

SQUARE CUT



45 DEGREE ANGLE BEVEL CUT

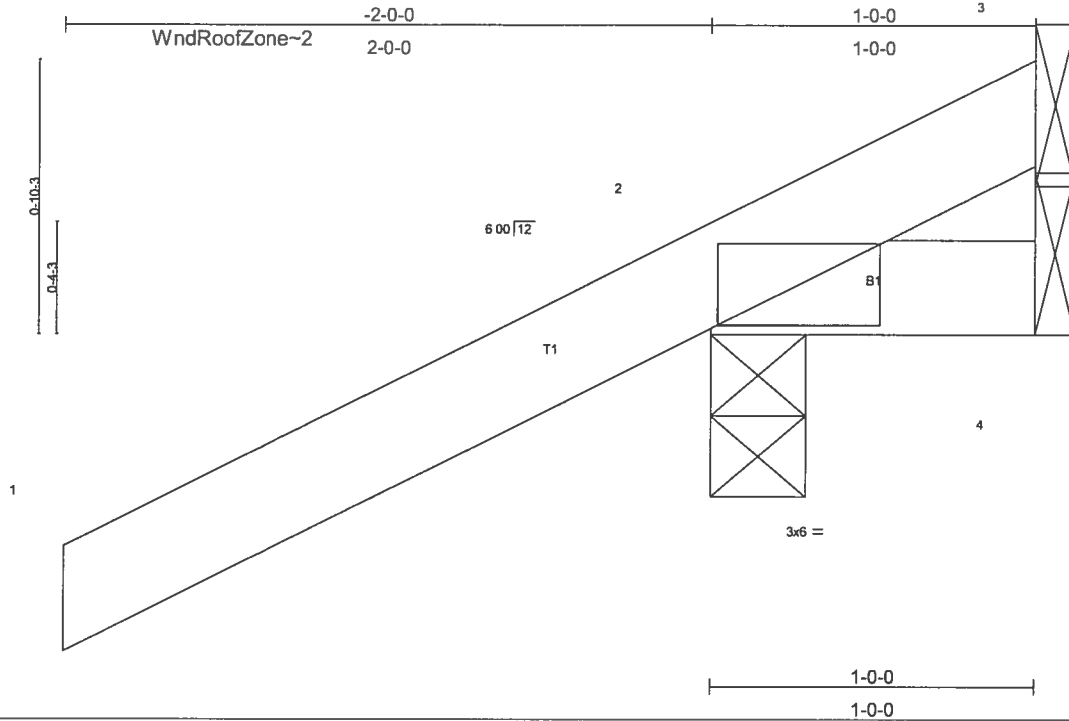
This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.



VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer.

Job L222243	Truss CJ1	Truss Type JACK	Qty 12	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:07 2007 Page 1



Scale = 1:7.2

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.28	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(LL) -0.00 2 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.00 2 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 3 n/a n/a		
	Code FBC2004/TPI2002			Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift 2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

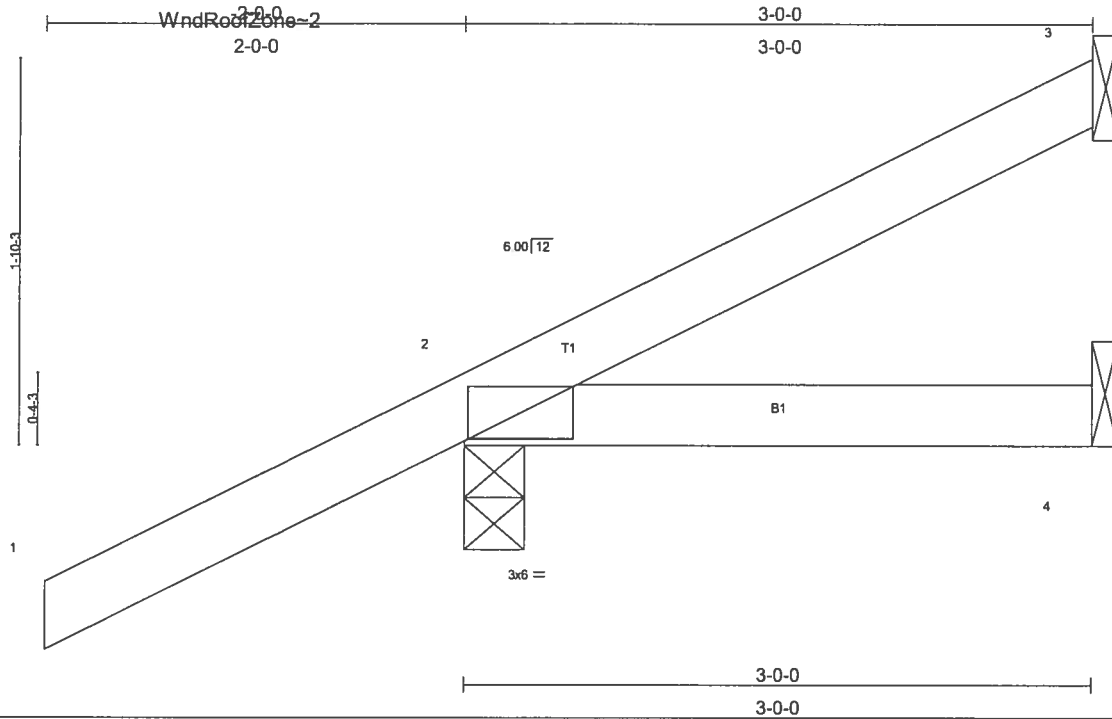
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

LOAD CASE(S) Standard

Job L222243	Truss CJ3	Truss Type JACK	Qty 12	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

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Scale = 1/11.1

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 13 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
 Max Horz 2=132(load case 5)
 Max Uplift 3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-57/7
 BOT CHORD 2-4=0/0

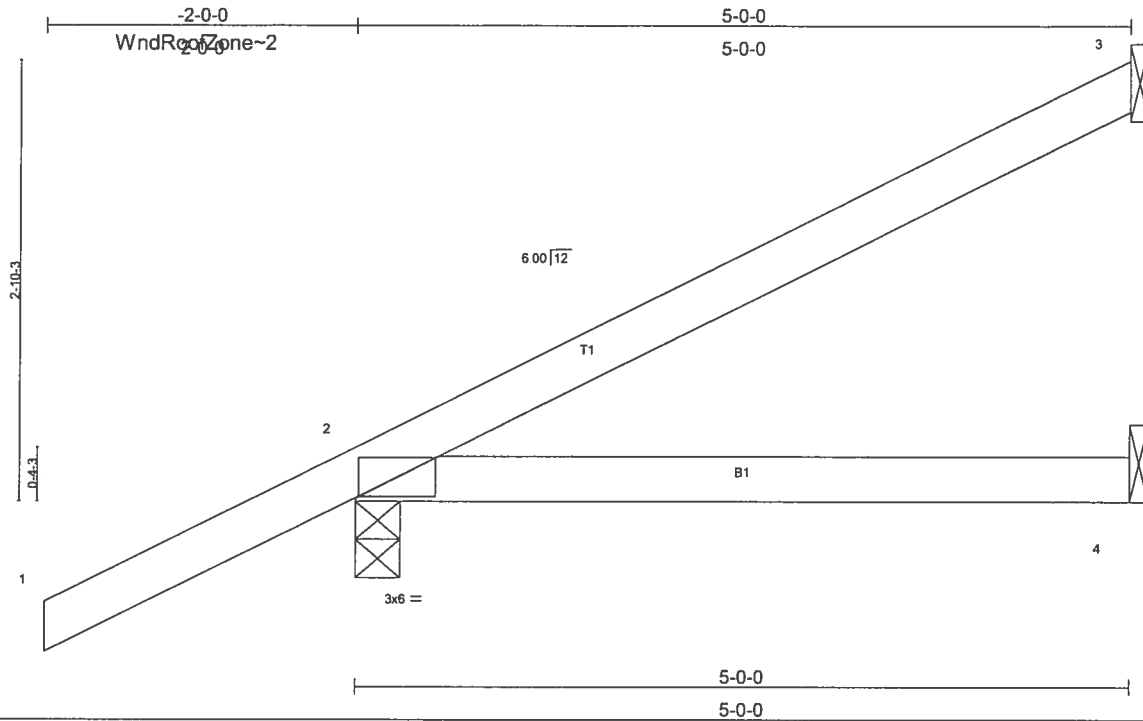
JOINT STRESS INDEX
 2 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L222243	Truss CJ5	Truss Type JACK	Qty 12	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:10 2007 Page 1



LOADING (psf)	SPACING	2'-0"-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5'-0"-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0"-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift 3=87(load case 5), 2=260(load case 5), 4=46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L222243	Truss EJ7	Truss Type MONO TRUSS	Qty 27	Ply 1	CLARK CONST. - LOT 58 SA
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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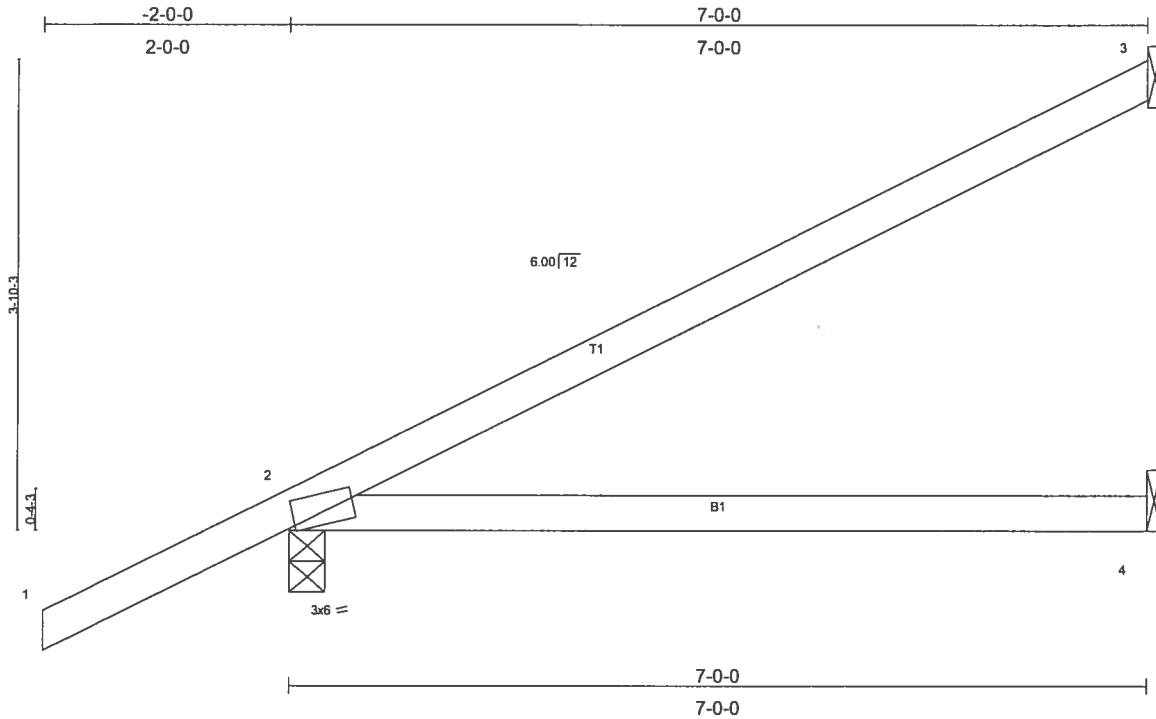


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.44	Vert(LL)	0.27	2-4	>305	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.38	Vert(TL)	0.22	2-4	>374	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 26 lb										

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
 Max Horz 2=224(load case 5)
 Max Uplift 3=-144(load case 5), 2=-295(load case 5), 4=-68(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-94/58
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX
 2 = 0.78

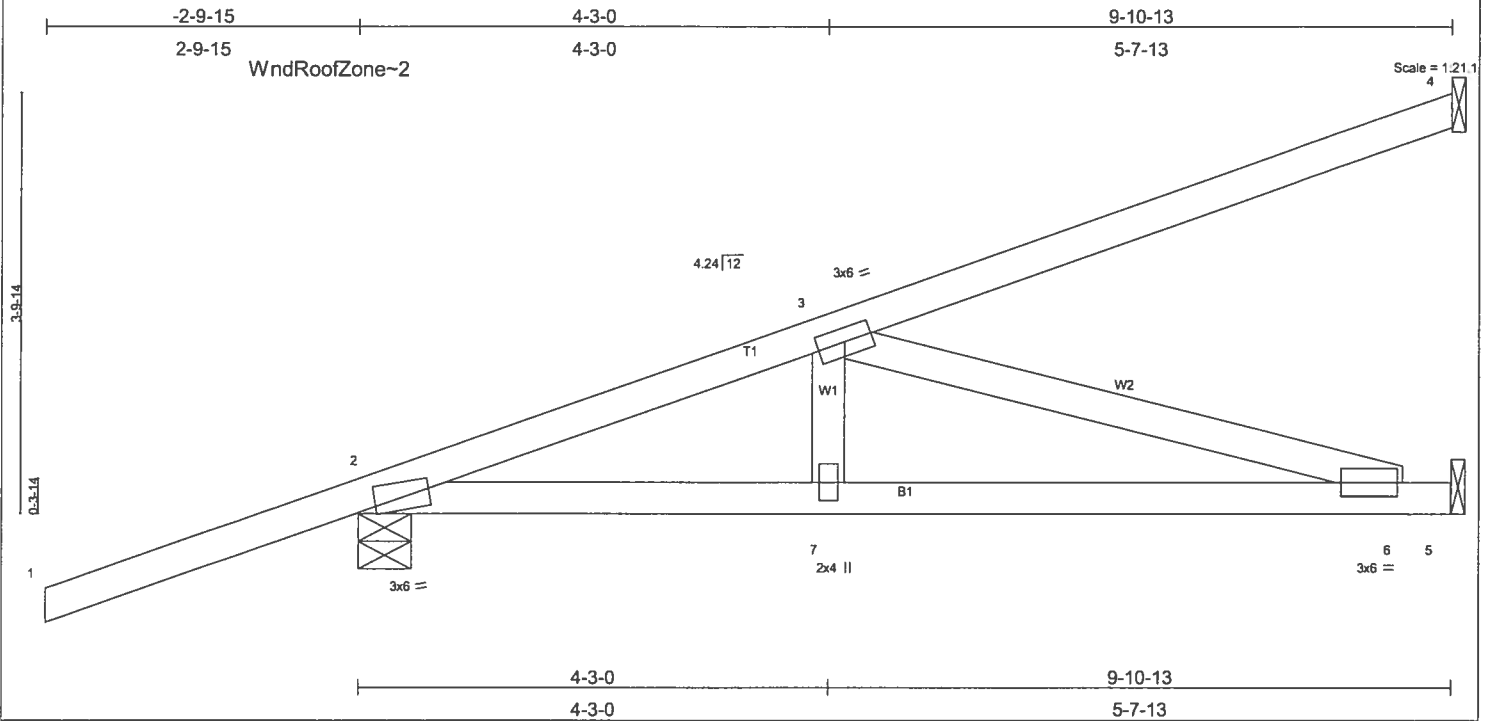
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 295 lb uplift at joint 2 and 68 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L222243	Truss HJ9	Truss Type MONO TRUSS	Qty 6	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.61	Vert(TL)	-0.18	6-7	>623	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.47	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 45 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-11-9 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical

Max Horz 2=269(load case 2)

Max Uplift 4=233(load case 2), 2=401(load case 2), 5=181(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=884/363, 3-4=-105/66

BOT CHORD 2-7=-535/818, 6-7=-535/818, 5-6=0/0

WEBS 3-7=-94/188, 3-6=-852/557

JOINT STRESS INDEX

2 = 0.77, 3 = 0.23, 6 = 0.24 and 7 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 401 lb uplift at joint 2 and 181 lb uplift at joint 5.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

Job L222243	Truss T01	Truss Type HIP	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

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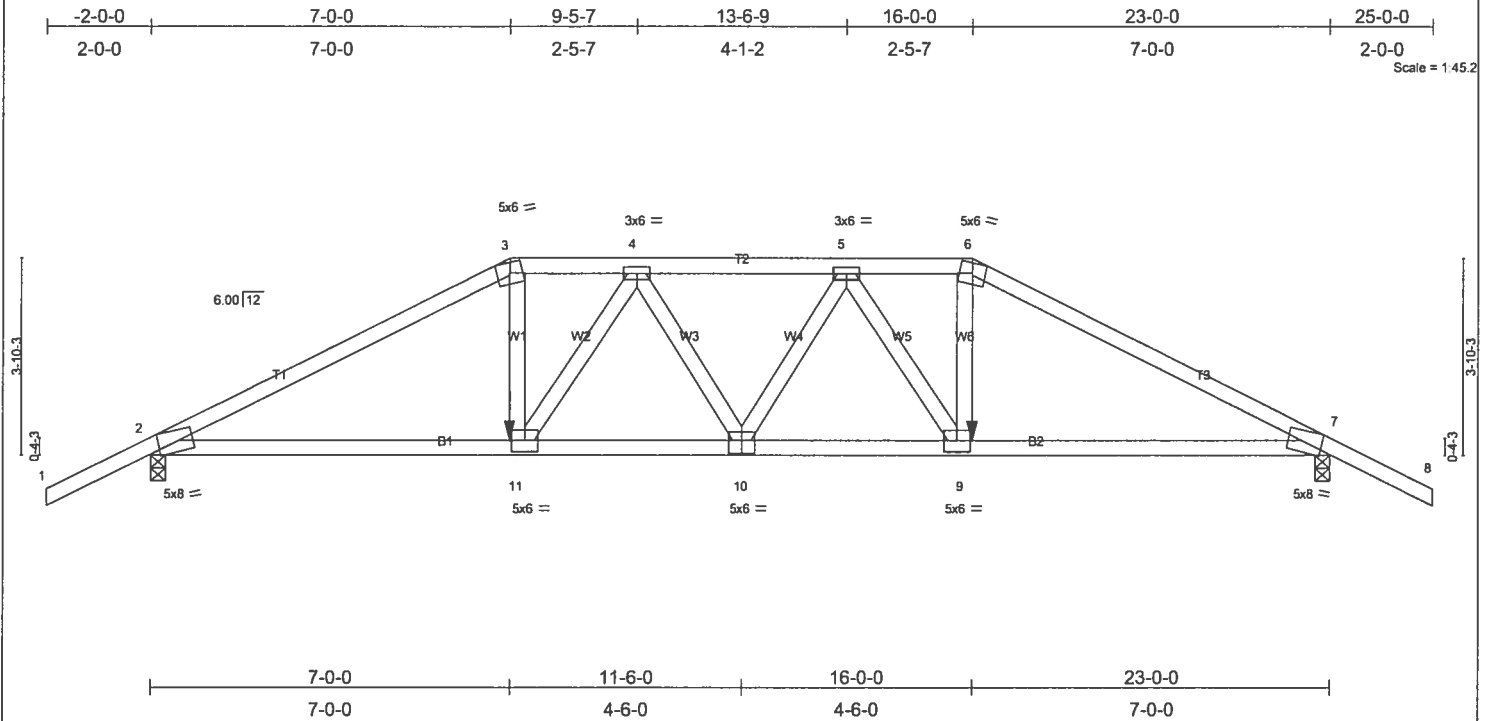


Plate Offsets (X,Y): [2-0-2-7,Edge], [7-0-2-7,Edge], [10-0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.61	Vert(LL)	-0.20	9-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.84	Vert(TL)	-0.32	10-11	>848	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.44	Horz(TL)	0.12	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 111 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-9-12 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 5-2-13 oc bracing.

REACTIONS

(lb/size) 2=2048/0-3-8, 7=2048/0-3-8
 Max Horz 2=-87(load case 5)
 Max Uplift 2=-911(load case 4), 7=-912(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3711/1493, 3-4=-3277/1406, 4-5=-3654/1531, 5-6=-3277/1406, 6-7=-3711/1493, 7-8=0/47
 BOT CHORD 2-11=-1285/3224, 10-11=-1475/3579, 9-10=-1455/3579, 7-9=-1243/3224
 WEBS 3-11=-561/1374, 4-11=-680/411, 4-10=0/188, 5-10=0/188, 5-9=-680/411, 6-9=-561/1374

JOINT STRESS INDEX

2 = 0.85, 3 = 0.74, 4 = 0.44, 5 = 0.44, 6 = 0.74, 7 = 0.85, 9 = 0.48, 10 = 0.86 and 11 = 0.48

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 911 lb uplift at joint 2 and 912 lb uplift at joint 7.
- Girder carries hip end with 7-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 16-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-11=-30, 9-11=-65(F=-35), 7-9=-30
 Concentrated Loads (lb)
 Vert: 11=-539(F) 9=-539(F)

Job L222243	Truss T01A	Truss Type HIP	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

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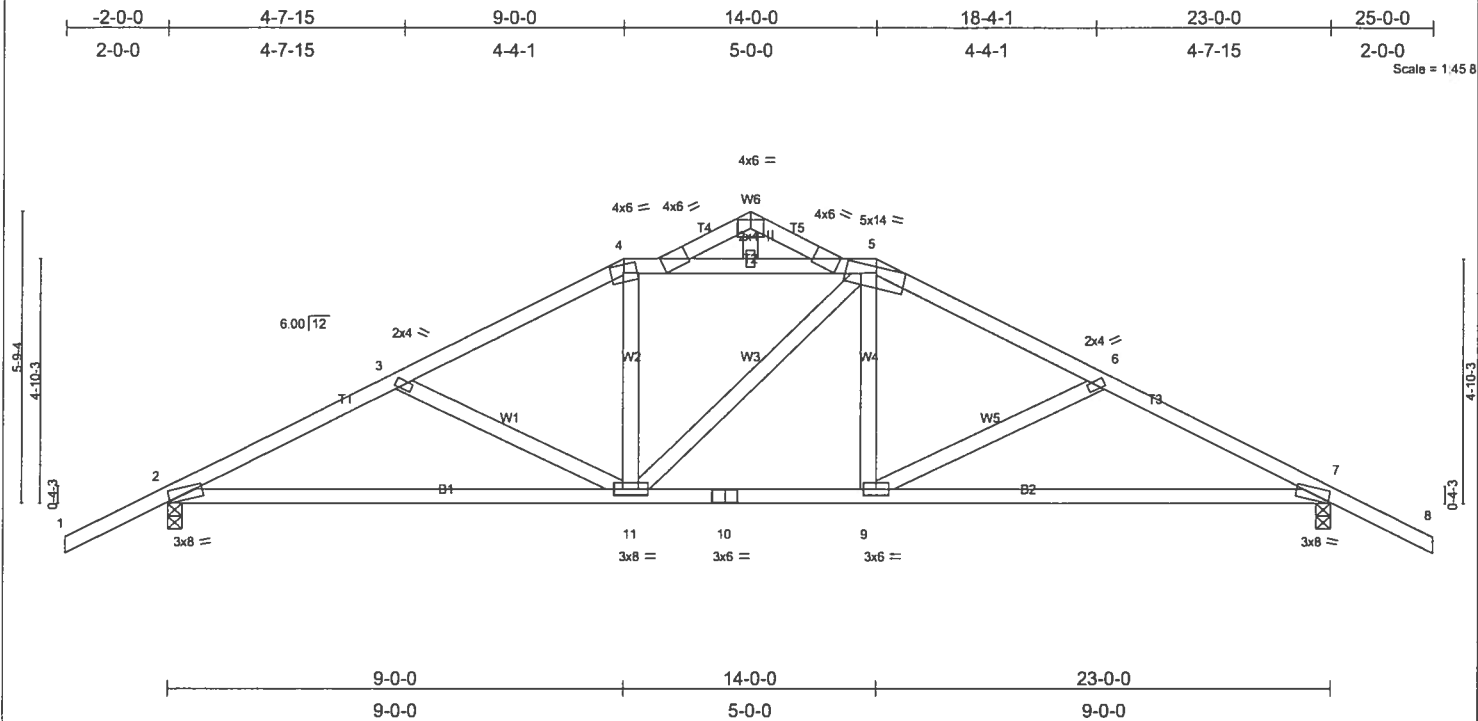


Plate Offsets (X,Y): [2:0-0-10,Edge], [5:0-6-13,Edge], [7:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.41	Vert(LL)	-0.19	7-9	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.56	Vert(TL)	-0.32	7-9	>842	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.11	Horz(TL)	0.06	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 122 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-5 oc purlins. Except:
 1 Row at midpt 4-5
 BOT CHORD Rigid ceiling directly applied or 8-10-8 oc bracing.

REACTIONS

(lb/size) 2=1220/0-3-8, 7=1220/0-3-8
 Max Horz 2=-101(load case 6)
 Max Uplift 2=476(load case 5), 7=476(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1909/601, 3-4=-1678/505, 4-5=-1471/501, 5-6=-1678/505, 6-7=-1909/601, 7-8=0/47
 BOT CHORD 2-11=-500/1660, 10-11=-301/1470, 9-10=-301/1470, 7-9=-408/1660
 WEBS 3-11=-223/189, 4-11=-43/302, 5-11=-105/107, 5-9=-36/302, 6-9=-224/189

JOINT STRESS INDEX

2 = 0.84, 3 = 0.34, 4 = 0.80, 5 = 0.86, 6 = 0.34, 7 = 0.84, 9 = 0.35, 10 = 0.61, 11 = 0.57, 12 = 0.13, 13 = 0.25, 14 = 0.13 and 15 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 476 lb uplift at joint 7.
- 6) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-54, 4-5=-114(F=-60), 5-8=-54, 2-7=-30

Job L222243	Truss T03	Truss Type COMMON	Qty 12	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:18 2007 Page 1

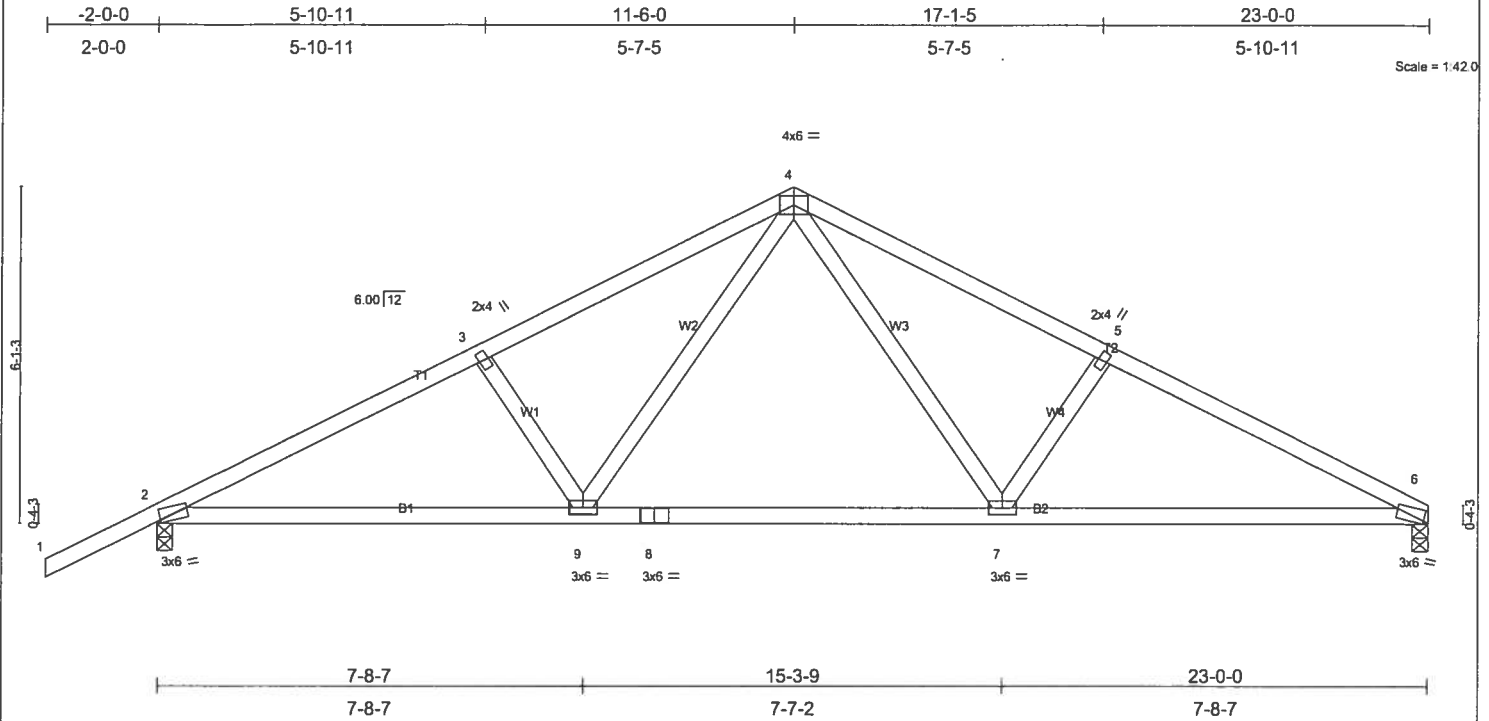


Plate Offsets (X,Y): [2:0-1-0,0-0-7], [6:0-1-0,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.13	6-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	-0.21	6-7	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.19	Horz(TL)	0.05	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 106 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-11 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-6-6 oc bracing.

REACTIONS

(lb/size) 6=948/0-3-8, 2=1075/0-3-8
 Max Horz 2=143(load case 5)
 Max Uplift 6=-308(load case 6), 2=-437(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1623/494, 3-4=-1455/491, 4-5=-1477/528, 5-6=-1632/533
 BOT CHORD 2-9=-439/1384, 8-9=-210/938, 7-8=-210/938, 6-7=-401/1411
 WEBS 3-9=-285/241, 4-9=-179/569, 4-7=-220/601, 5-7=-301/262

JOINT STRESS INDEX

2 = 0.78, 3 = 0.34, 4 = 0.48, 5 = 0.34, 6 = 0.78, 7 = 0.45, 8 = 0.39 and 9 = 0.45

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 6 and 437 lb uplift at joint 2.

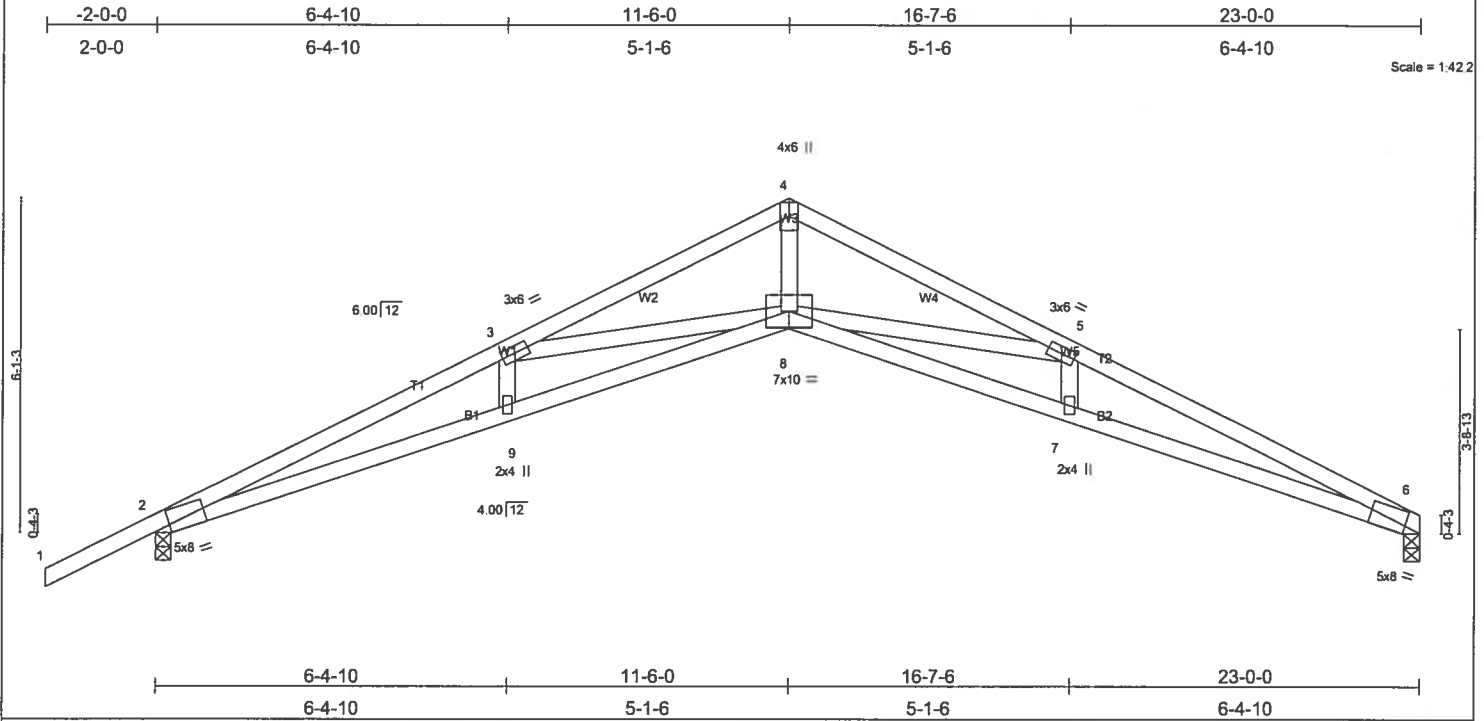
LOAD CASE(S) Standard

Job L222243	Truss T04	Truss Type SCISSOR	Qty 8	Ply 1	CLARK CONST. - LOT 58 SA
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Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.56	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.97	Vert(LL) -0.56 8-9 >486 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.77	Vert(TL) -0.90 8-9 >303 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.75 6 n/a n/a		
	Code FBC2004/TPI2002			Weight: 98 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-6-14 oc purtins.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1075/0-3-8, 6=948/0-3-8
 Max Horz 2=141(load case 5)
 Max Uplift 2=437(load case 5), 6=308(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/46, 2-3=4110/1211, 3-4=3090/795, 4-5=3091/812, 5-6=4169/1182
 BOT CHORD 2-9=-1124/3781, 8-9=-1129/3790, 7-8=-1021/3864, 6-7=-1022/3860
 WEBS 3-9=0/156, 3-8=-953/492, 4-8=-552/2410, 5-8=-1025/603, 5-7=0/167

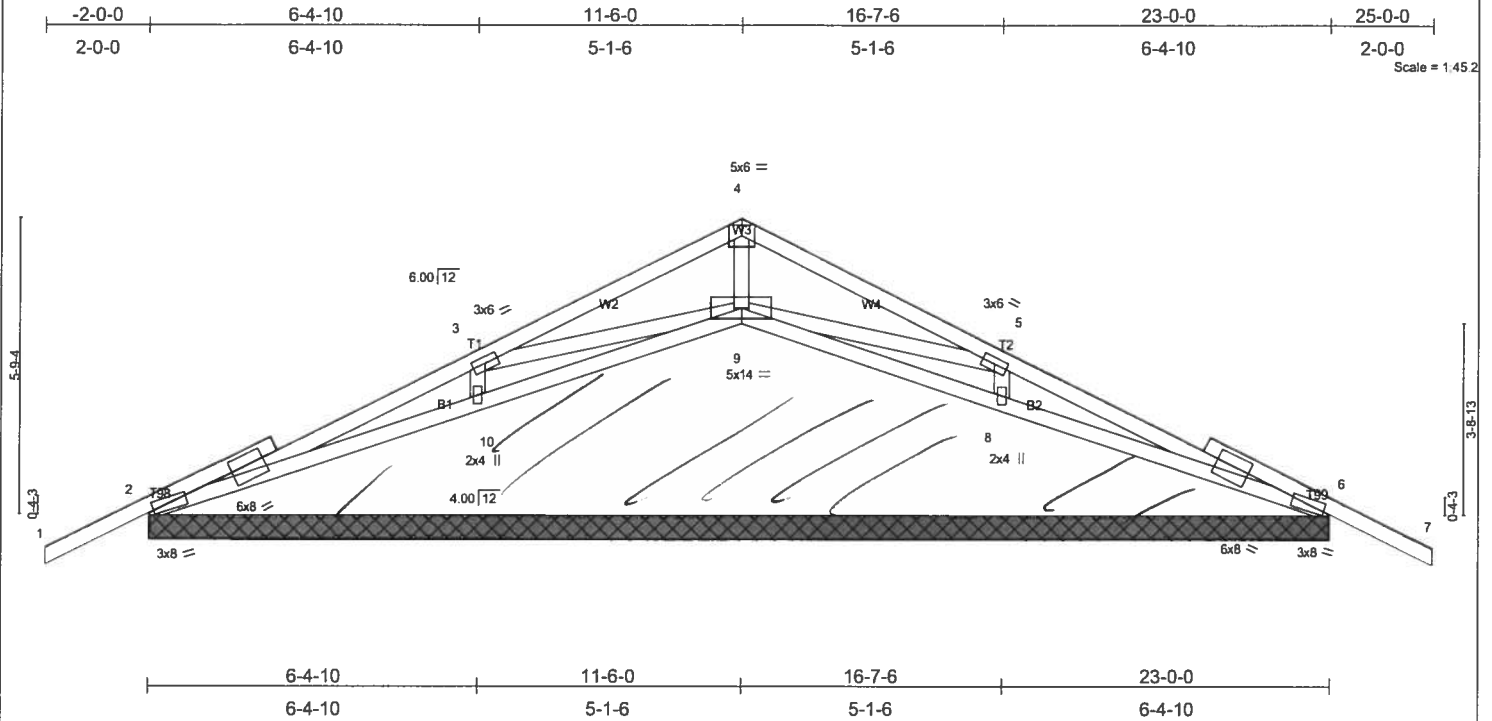
JOINT STRESS INDEX
 2 = 0.84, 3 = 0.41, 4 = 0.64, 5 = 0.41, 6 = 0.84, 7 = 0.34, 8 = 0.72 and 9 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 437 lb uplift at joint 2 and 308 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L222243	Truss T04G	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:21 2007 Page 1		



Job L222243	Truss T06	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

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Plate Offsets (X,Y): [2:0-1-0,0-8-0], [2:0-2-1,0-0-9], [3:0-6-0,0-6-8], [3:0-2-1,0-1-8], [4:0-2-0,0-2-0], [8:0-2-0,0-2-0], [9:0-2-1,0-1-8], [9:0-6-0,0-6-8], [10:0-2-1,0-0-9], [10:0-1-0,0-8-0], [13:0-3-0,0-3-4], [15:0-3-0,0-3-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.40	Vert(LL)	-0.39 14-16	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.85	Vert(TL)	-0.65 14-16	>736	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.67	Horz(TL)	0.23 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 351 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 "Except"
T1 2 X 6 SYP No.1D, T2 2 X 6 SYP No.1D
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-3 oc bracing.
WEBS 1 Row at midpt 5-14, 6-14, 7-14

REACTIONS

(lb/size) 2=2430/0-3-8, 10=2430/0-3-8
Max Horz 2=-172(load case 6)
Max Uplift 2=911(load case 5), 10=-911(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-4799/1619, 3-4=-4504/1521, 4-5=-4438/1521, 5-6=-3229/1157, 6-7=-3229/1157, 7-8=-4438/1522, 8-9=-4504/1522,
9-10=-4799/1620, 10-11=0/47
BOT CHORD 2-16=-1487/4299, 15-16=-1135/3552, 14-15=-1135/3552, 13-14=-1020/3552, 12-13=-1020/3552, 10-12=-1316/4299
WEBS 3-16=-493/345, 5-16=-228/783, 5-14=-1209/603, 6-14=-709/2102, 7-14=-1209/603, 7-12=-229/783, 9-12=-493/346

JOINT STRESS INDEX

2 = 0.85, 2 = 0.86, 3 = 0.92, 3 = 0.38, 3 = 0.38, 3 = 0.38, 3 = 0.38, 4 = 0.00, 4 = 0.56, 4 = 0.97, 5 = 0.28, 6 = 0.77, 7 = 0.28, 8 = 0.00, 8 = 0.97, 8 = 0.56, 9 = 0.92, 9 = 0.38, 9 = 0.38, 9 = 0.38, 9 = 0.38, 10 = 0.85, 10 = 0.86, 12 = 0.60, 13 = 0.91, 14 = 0.63, 15 = 0.91, 16 = 0.60, 17 = 0.44, 18 = 0.34, 19 = 0.34, 20 = 0.44, 21 = 0.34, 22 = 0.34, 23 = 0.66, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.44, 31 = 0.34, 32 = 0.34, 33 = 0.44, 34 = 0.34, 35 = 0.34, 36 = 0.66, 37 = 0.34 and 38 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 911 lb uplift at joint 2 and 911 lb uplift at joint 10.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-114(F=-60), 6-8=-114(F=-60), 8-11=-54, 2-10=-30

Job L222243	Truss T07	Truss Type COMMON	Qty 2	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)

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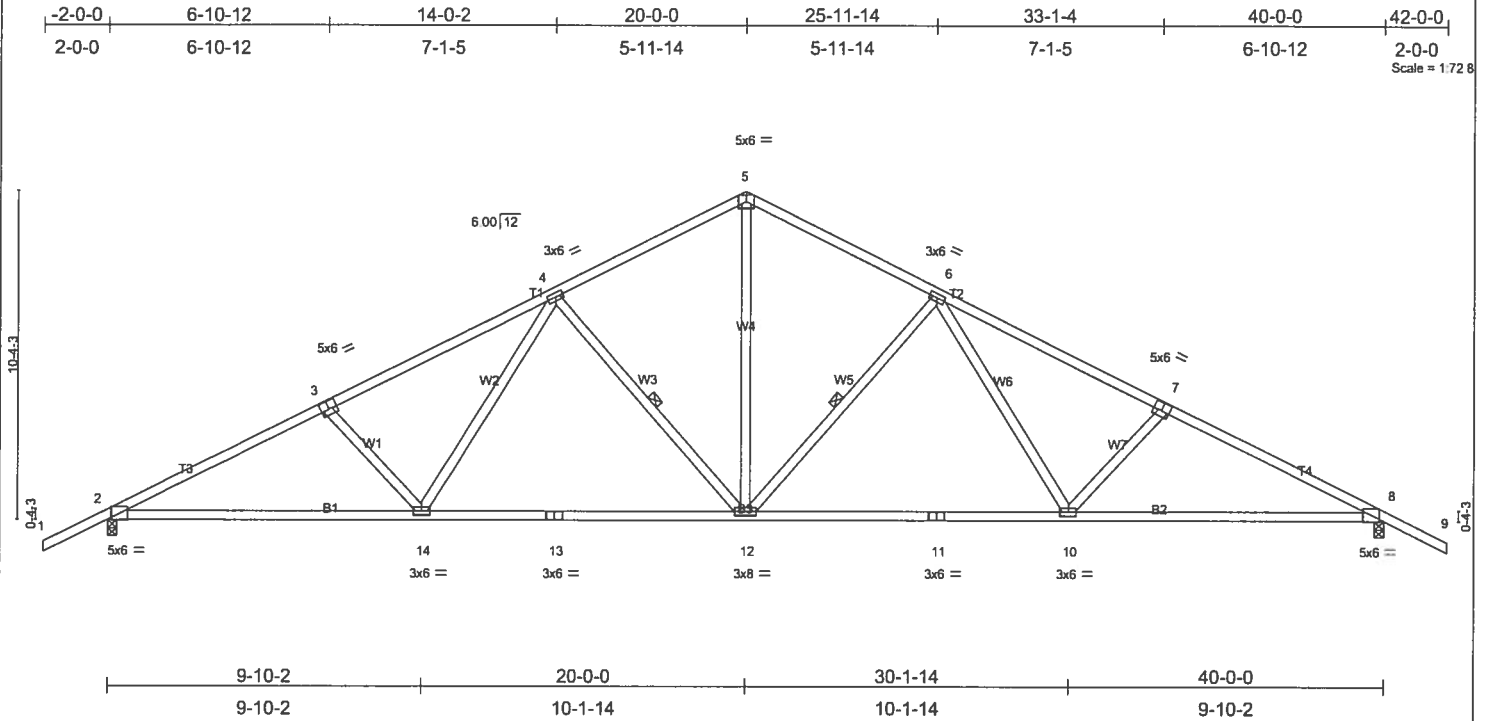


Plate Offsets (X,Y): [2:0-1-11,Edge], [3:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-1-11,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.46	TC (LL)	-0.34	12-14	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.81	Vert(TL)	-0.56	12-14	>846	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.80	Horz(TL)	0.16	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 211 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-1-14 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-8-1 oc bracing.
 WEBS 1 Row at midpt 4-12, 6-12

REACTIONS

(lb/size) 2=1784/0-3-8, 8=1784/0-3-8
 Max Horz 2=178(load case 5)
 Max Uplift 2=-666(load case 5), 8=-666(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3080/982, 3-4=-2851/938, 4-5=-1950/714, 5-6=-1950/714, 6-7=-2851/938, 7-8=-3080/982, 8-9=0/47
 BOT CHORD 2-14=-897/2691, 13-14=-601/2129, 12-13=-601/2129, 11-12=-491/2129, 10-11=-491/2129, 8-10=-719/2691
 WEBS 3-14=-345/303, 4-14=-195/673, 4-12=-711/418, 5-12=-448/1362, 6-12=-711/418, 6-10=-195/673, 7-10=-345/303

JOINT STRESS INDEX

2 = 0.78, 3 = 0.65, 4 = 0.46, 5 = 0.48, 6 = 0.46, 7 = 0.65, 8 = 0.78, 10 = 0.53, 11 = 0.95, 12 = 0.68, 13 = 0.95 and 14 = 0.53

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 666 lb uplift at joint 2 and 666 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L222243	Truss T08	Truss Type SPECIAL	Qty 2	Ply 1	CLARK CONST. - LOT 58 SA
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Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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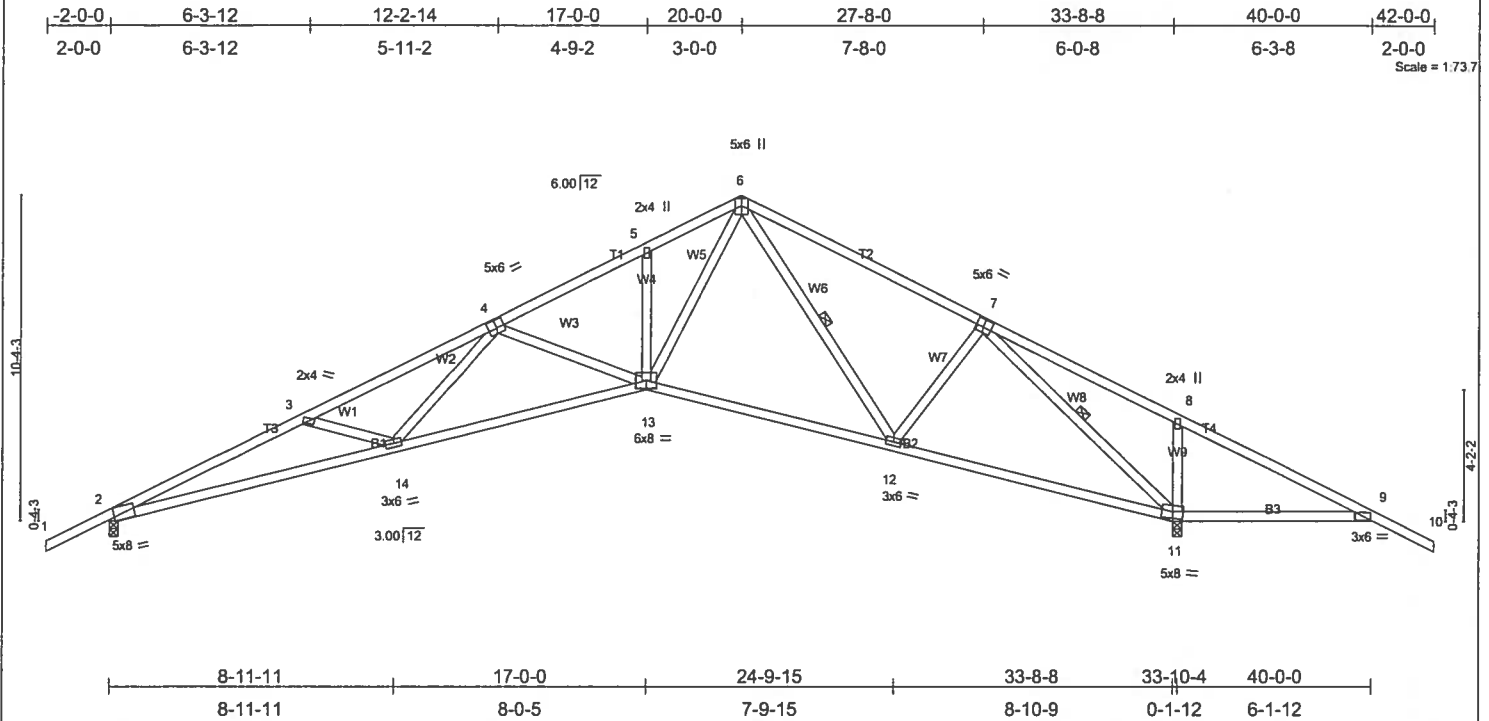


Plate Offsets (X,Y): [2:0-2-6,Edge], [4:0-3-0,0-3-0], [7:0-2-12,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.52	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.91	Vert(LL) -0.47 13-14 >863 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.94	Vert(TL) -0.75 13-14 >535 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.45 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 206 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-1 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
 WEBS 1 Row at midpt 6-12, 7-11

REACTIONS

(lb/size) 2=1456/0-3-8, 11=2115/0-3-8
 Max Horz 2=-178(load case 6)
 Max Uplift 2=-579(load case 5), 11=-956(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-4449/1535, 3-4=-4097/1329, 4-5=-2808/879, 5-6=-2735/948, 6-7=-1621/565, 7-8=-654/806, 8-9=-772/794, 9-10=0/47
 BOT CHORD 2-14=-1439/4046, 13-14=-1001/3284, 12-13=-269/1602, 11-12=-214/1149, 9-11=-636/808
 WEBS 3-14=-319/323, 4-14=-188/710, 4-13=-771/453, 5-13=-167/130, 6-13=-638/2016, 6-12=-303/146, 7-12=-40/490, 7-11=-2485/825, 8-11=-342/303

JOINT STRESS INDEX

2 = 0.86, 3 = 0.34, 4 = 0.55, 5 = 0.34, 6 = 0.66, 7 = 0.74, 8 = 0.34, 9 = 0.62, 11 = 0.86, 12 = 0.43, 13 = 0.84 and 14 = 0.45

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 579 lb uplift at joint 2 and 956 lb uplift at joint 11.

LOAD CASE(S) Standard

Job L222243	Truss T09	Truss Type SPECIAL	Qty 4	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:30 2007 Page 1

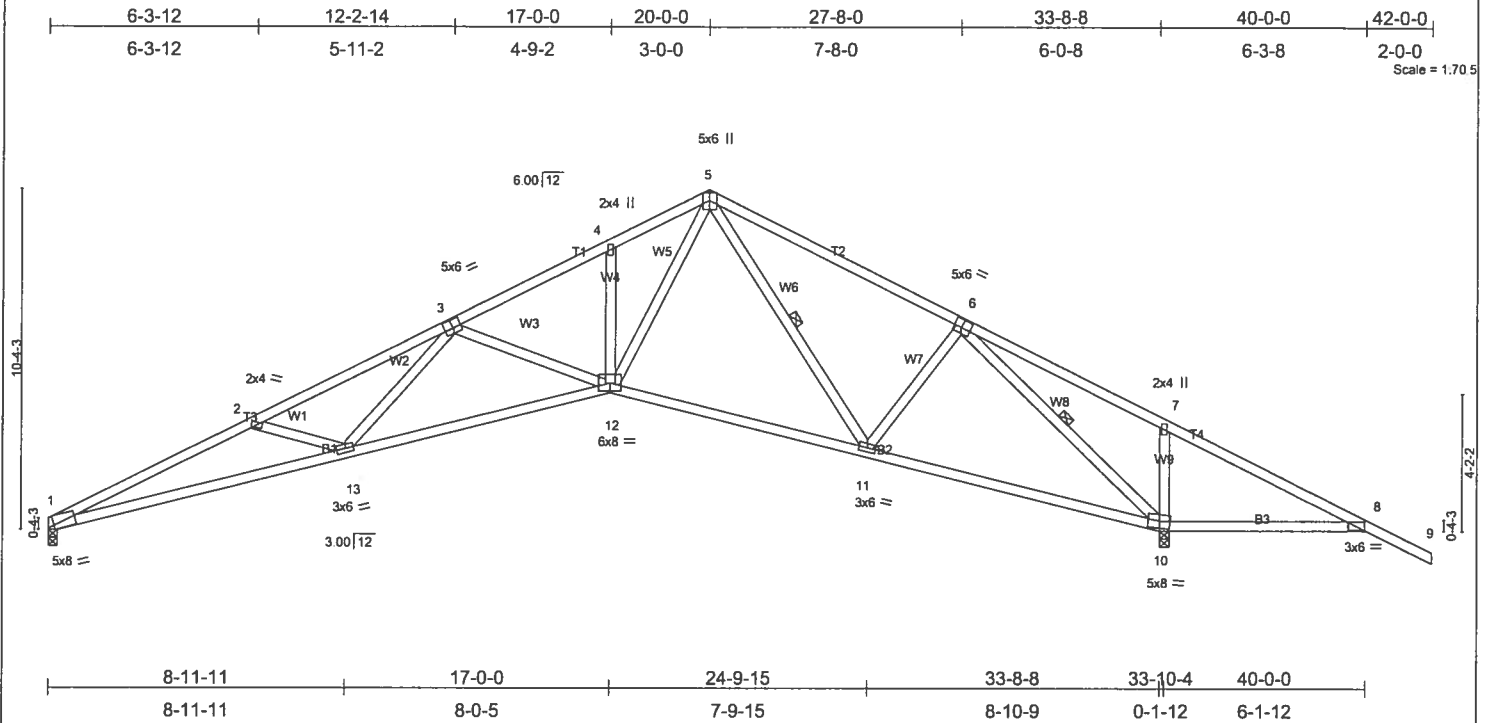


Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [6:0-2-12,0-3-0]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.73	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.95	Vert(LL) -0.47 12-13 >863 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.94	Vert(TL) -0.75 12-13 >535 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.46 10 n/a n/a		
	Code FBC2004/TPI2002			Weight: 203 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 5-11, 6-10

REACTIONS (lb/size) 1=1337/0-3-8, 10=2119/0-3-8
 Max Horz 1=-202(load case 6)
 Max Uplift 1=-454(load case 5), 10=-957(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-4524/1626, 2-3=-4155/1400, 3-4=-2824/898, 4-5=-2752/968, 5-6=-1629/574, 6-7=-654/806, 7-8=-772/795, 8-9=0/47
 BOT CHORD 1-13=-1531/4124, 12-13=-1034/3312, 11-12=-278/1610, 10-11=-220/1154, 8-10=-637/809
 WEBS 2-13=-343/351, 3-13=-233/746, 3-12=-784/470, 4-12=-169/133, 5-12=-656/2031, 5-11=-305/150, 6-11=-44/492, 6-10=-2493/837, 7-10=-342/303

JOINT STRESS INDEX
 1 = 0.87, 2 = 0.34, 3 = 0.54, 4 = 0.34, 5 = 0.67, 6 = 0.74, 7 = 0.34, 8 = 0.62, 10 = 0.87, 11 = 0.43, 12 = 0.83 and 13 = 0.47

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 1 and 957 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L222243	Truss T10	Truss Type SPECIAL	Qty 6	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:32 2007 Page 1

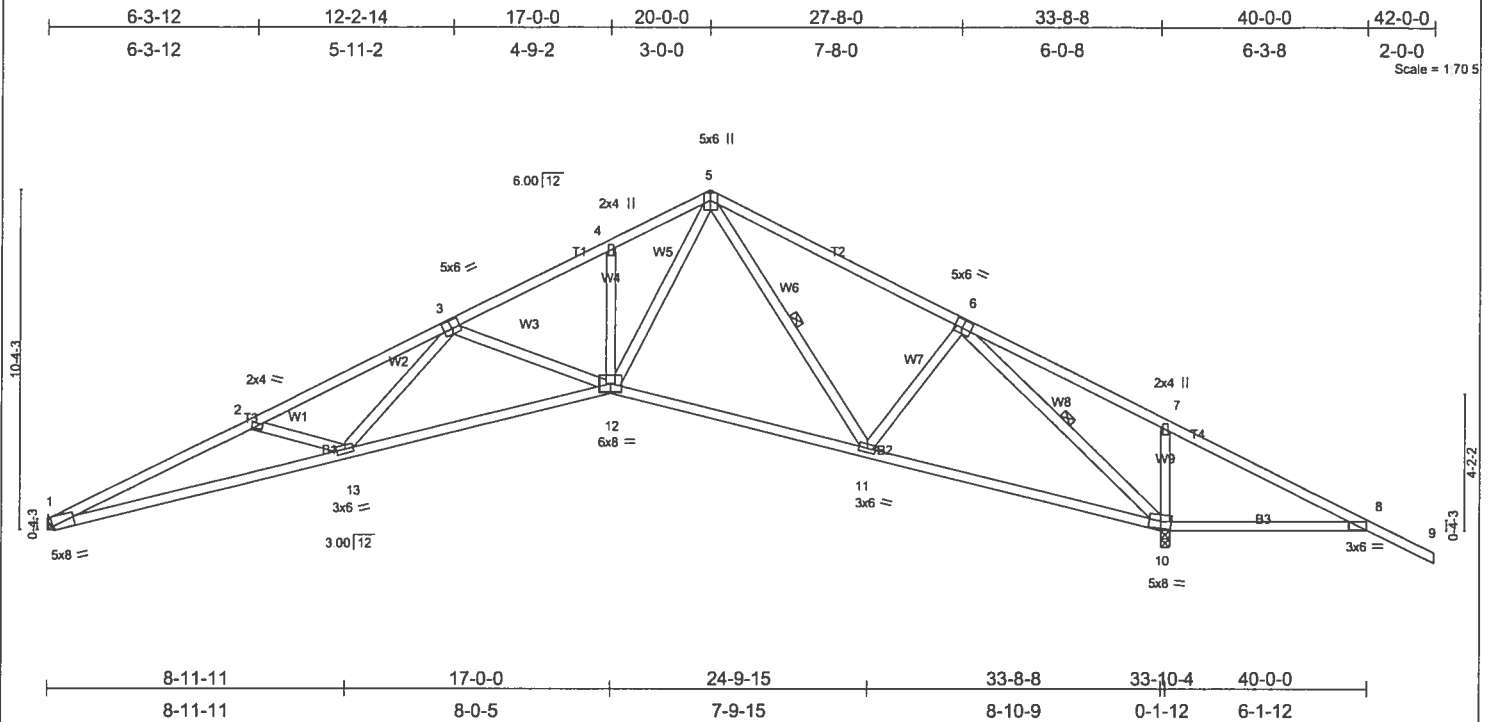


Plate Offsets (X,Y): [1:0-2-6,Edge], [3:0-3-0,0-3-0], [6:0-2-12,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.73	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.95	Vert(LL) -0.47 12-13 >865 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.94	Vert(TL) -0.75 12-13 >536 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.46 10 n/a n/a		
	Code FBC2004/TPI2002				
					Weight: 203 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-11, 6-10

REACTIONS (lb/size) 1=1337/Mechanical, 10=2119/0-3-8

Max Horz 1=-202(load case 6)

Max Uplift 1=-454(load case 5), 10=-957(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4519/1624, 2-3=-4152/1399, 3-4=-2822/897, 4-5=-2751/968, 5-6=-1628/574, 6-7=-654/806, 7-8=-772/794, 8-9=0/47

BOT CHORD 1-13=-1530/4119, 12-13=-1033/3310, 11-12=-278/1609, 10-11=-220/1154, 8-10=-636/808

WEBS 2-13=-341/350, 3-13=-233/745, 3-12=-783/469, 4-12=-169/133, 5-12=-655/2030, 5-11=-306/150, 6-11=-44/492, 6-10=-2493/836, 7-10=-342/303

JOINT STRESS INDEX

1 = 0.87, 2 = 0.34, 3 = 0.54, 4 = 0.34, 5 = 0.67, 6 = 0.74, 7 = 0.34, 8 = 0.62, 10 = 0.87, 11 = 0.43, 12 = 0.83 and 13 = 0.47

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 1 and 957 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L222243	Truss T11	Truss Type SPECIAL	Qty 3	Ply 1	CLARK CONST. - LOT 58 SA
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Job Reference (optional)

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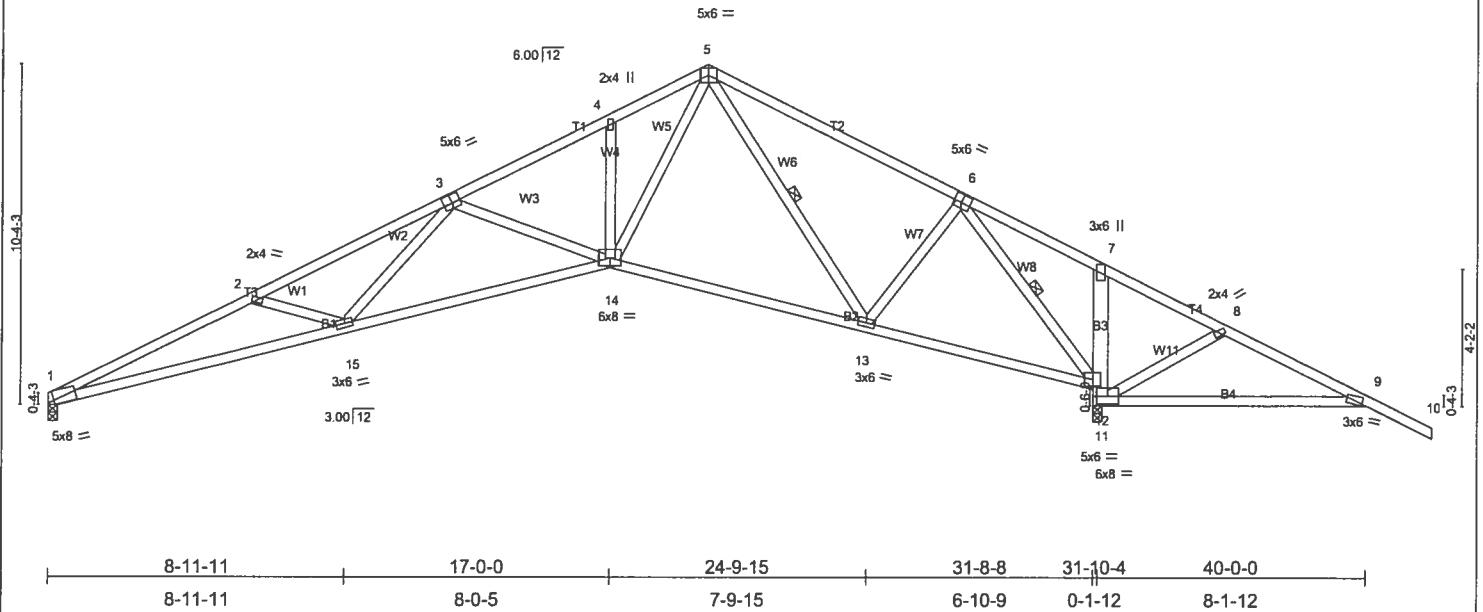
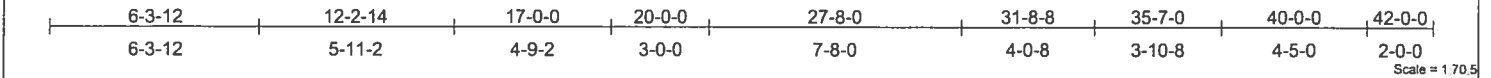


Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [6:0-2-12,0-3-0], [9:0-1-5,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.65	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.88	Vert(LL) -0.40 1-15 >957 240		
BCDL 10.0	Lumber Increase 1.25	WB 0.62	Vert(TL) -0.65 1-15 >586 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.37 11 n/a n/a		
	Code FBC2004/TPI2002				
				Weight: 211 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 5-2-4 oc bracing.
 WEBS 1 Row at midpt 5-13, 6-12

REACTIONS

(lb/size) 1=1219/0-3-8, 11=2237/0-3-8
 Max Horz 1=-202(load case 6)
 Max Uplift 1=-422(load case 5), 11=-1056(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4045/1499, 2-3=-3664/1269, 3-4=-2317/761, 4-5=-2240/832, 5-6=-1014/414, 6-7=-947/1146, 7-8=-833/998, 8-9=-749/755, 9-10=0/47
 BOT CHORD 1-15=-1416/3687, 14-15=-910/2850, 13-14=-181/1245, 12-13=-26/591, 11-12=-1977/877, 7-12=-123/88, 9-11=-615/773
 WEBS 2-15=-354/355, 3-15=-238/753, 3-14=-789/472, 4-14=-172/133, 5-14=-594/1837, 5-13=-672/314, 6-13=-172/725, 6-12=-2290/883,
 8-11=-275/227

JOINT STRESS INDEX

1 = 0.78, 2 = 0.34, 3 = 0.52, 4 = 0.34, 5 = 0.84, 6 = 0.75, 7 = 0.29, 8 = 0.34, 9 = 0.91, 11 = 0.40, 12 = 0.82, 13 = 0.58, 14 = 0.80 and 15 = 0.48

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 422 lb uplift at joint 1 and 1056 lb uplift at joint 11.

LOAD CASE(S) Standard

Job L222243	Truss T12	Truss Type HIP	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
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Job Reference (optional)

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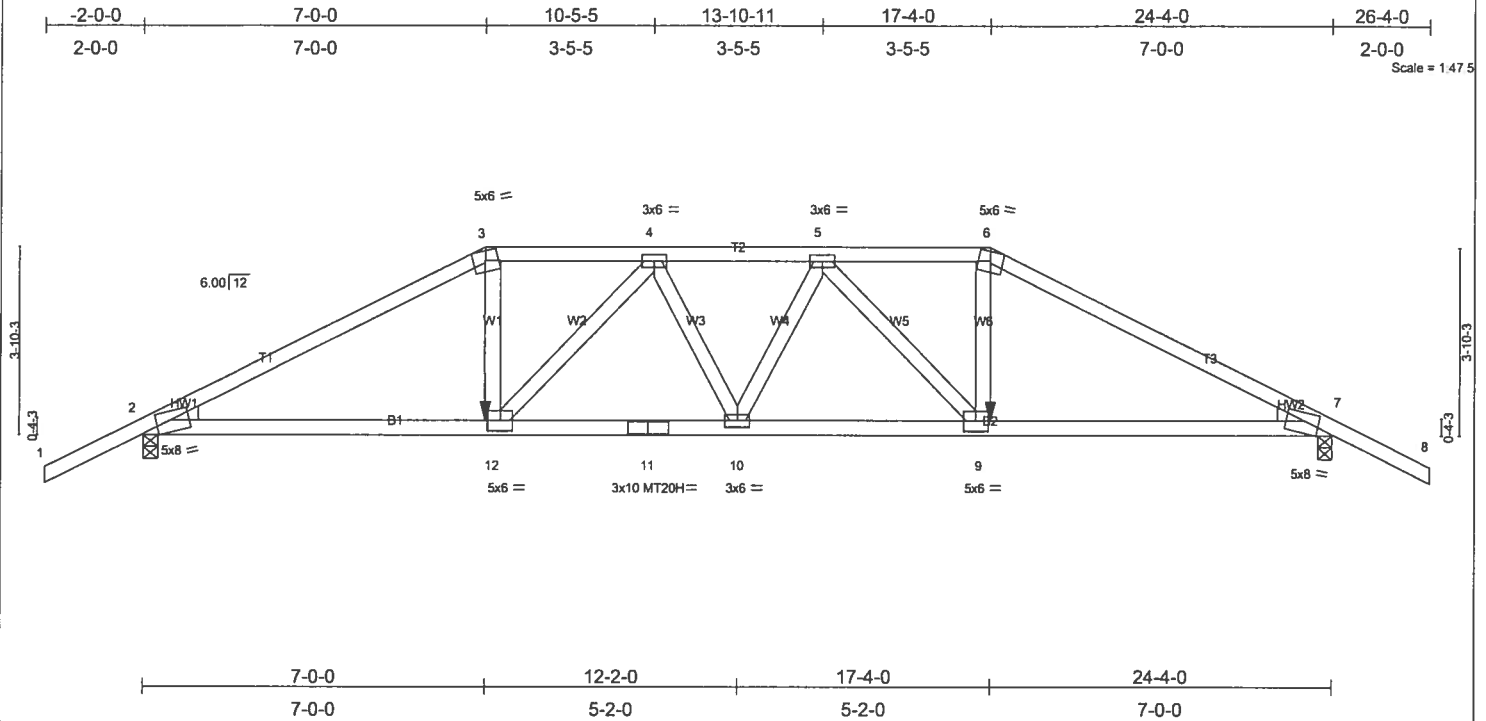


Plate Offsets (X,Y): [2:0-3-13,Edge], [7:0-3-13,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.65	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.93	Vert(LL) -0.24 9-10 >999 240	MT20H	187/143
BCLL 10.0	Lumber Increase 1.25	WB 0.46	Vert(TL) -0.39 9-10 >743 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.14 7 n/a n/a		
	Code FBC2004/TPI2002			Weight: 118 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 WEDGE
 Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 4-3-5 oc bracing.

REACTIONS (lb/size) 2=2170/0-3-8, 7=2170/0-3-8
 Max Horz 2=-87(load case 5)
 Max Uplift 2=-1260(load case 4), 7=-1260(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3982/2114, 3-4=-3522/1951, 4-5=-4082/2239, 5-6=-3522/1951, 6-7=-3982/2114, 7-8=0/47
 BOT CHORD 2-12=-1842/3466, 11-12=-2156/4005, 10-11=-2156/4005, 9-10=-2142/4005, 7-9=-1800/3466
 WEBS 3-12=-758/1421, 4-12=-823/513, 4-10=-37/208, 5-10=-37/208, 5-9=-823/513, 6-9=-758/1421

JOINT STRESS INDEX

2 = 0.87, 3 = 0.79, 4 = 0.46, 5 = 0.46, 6 = 0.79, 7 = 0.87, 9 = 0.50, 10 = 0.46, 11 = 0.94 and 12 = 0.50

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1260 lb uplift at joint 2 and 1260 lb uplift at joint 7.
- Girder carries hip end with 7-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 17-4-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-6=-118(F=-64), 6-8=-54, 2-12=-30, 9-12=-65(F=-35), 7-9=-30
 Concentrated Loads (lb)
 Vert: 12=-539(F) 9=-539(F)

Job L222243	Truss T13	Truss Type HIP	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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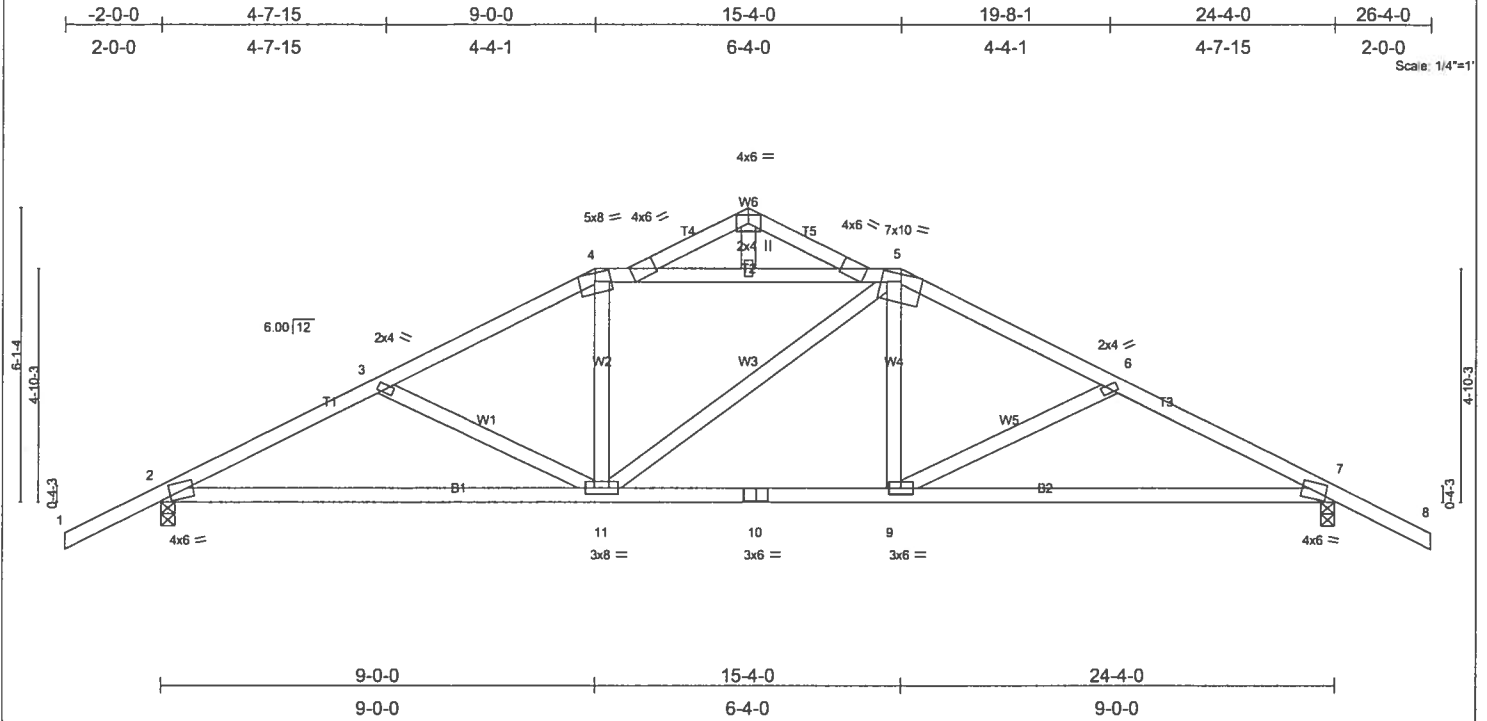


Plate Offsets (X,Y): [2:0-2-9,0-0-11], [7:0-2-9,0-0-11]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.64	Vert(LL)	0.40	7-9	>720	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.62	Vert(TL)	-0.34	7-9	>861	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.14	Horz(TL)	0.07	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
Weight: 130 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins. Except:
 1 Row at midpt 4-5
 BOT CHORD Rigid ceiling directly applied or 4-10-2 oc bracing.

REACTIONS

(lb/size) 2=1316/0-3-8, 7=1316/0-3-8
 Max Horz 2=101(load case 5)
 Max Uplift 2=863(load case 5), 7=863(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-2104/1750, 3-4=-1903/1696, 4-5=-1683/1578, 5-6=-1902/1695, 6-7=-2104/1750, 7-8=0/47
 BOT CHORD 2-11=-1431/1829, 10-11=-1335/1683, 9-10=-1335/1683, 7-9=-1431/1829
 WEBS 3-11=-174/187, 4-11=-389/302, 5-11=-112/113, 5-9=-389/302, 6-9=-175/188

JOINT STRESS INDEX

2 = 0.86, 3 = 0.34, 4 = 0.77, 5 = 0.92, 6 = 0.34, 7 = 0.86, 9 = 0.35, 10 = 0.57, 11 = 0.57, 12 = 0.13, 13 = 0.25, 14 = 0.13 and 15 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 863 lb uplift at joint 2 and 863 lb uplift at joint 7.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-54, 4-5=-114(F=60), 5-8=-54, 2-7=-30

Job L222243	Truss T14	Truss Type COMMON	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:38 2007 Page 1		

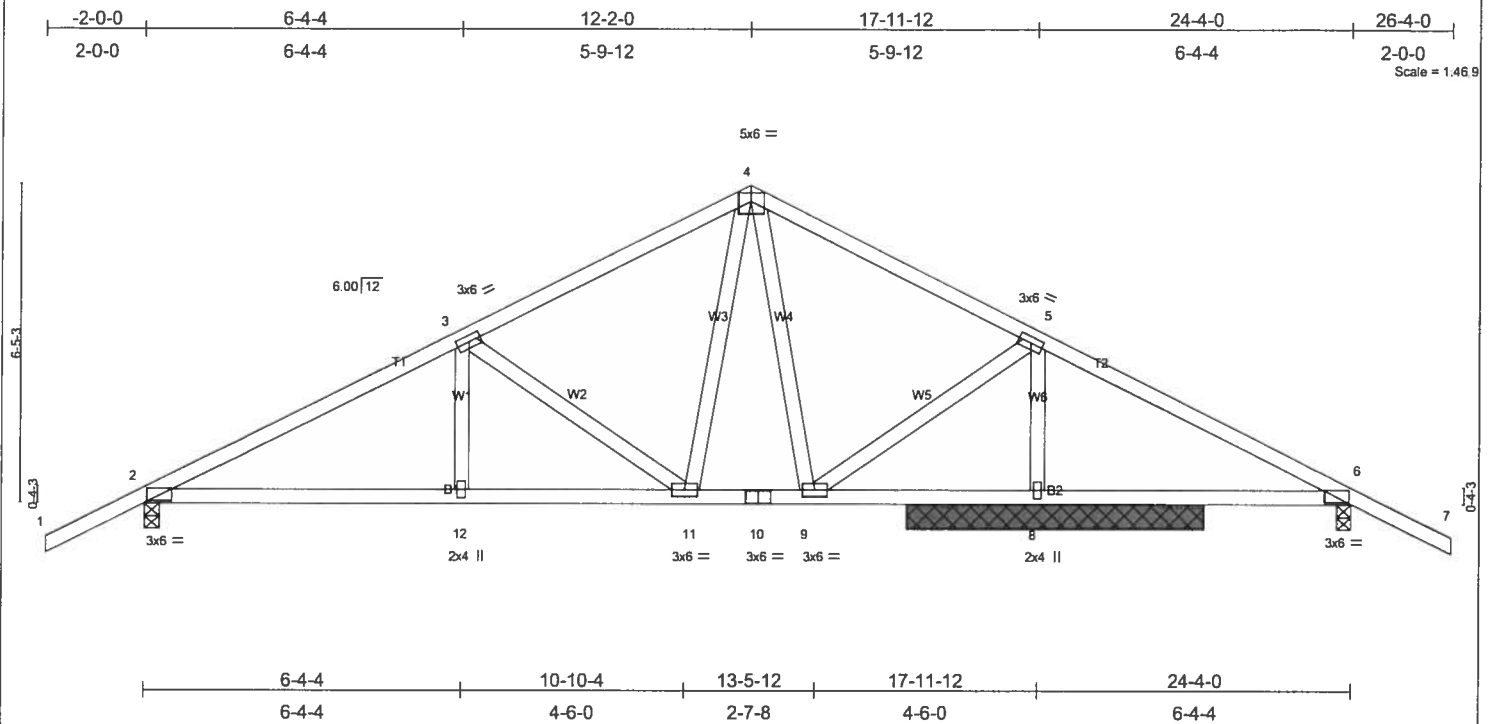


Plate Offsets (X,Y): [2-0-6-8,0-0-6], [4-0-3-0,0-2-2], [6-0-6-8,0-0-6]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.29	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.33	Vert(LL) 0.10 2-12 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.30	Vert(TL) -0.10 2-12 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.02 8 n/a n/a		
	Code FBC2004/TPI2002			Weight: 126 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-8-14 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=826/0-3-8, 8=1159/6-0-0, 6=266/0-3-8
 Max Horz 2=-123(load case 6)
 Max Uplift 2=-572(load case 5), 8=-470(load case 5), 6=-201(load case 6)
 Max Grav 2=826(load case 1), 8=1159(load case 1), 6=306(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-1099/855, 3-4=-611/527, 4-5=-468/389, 5-6=-115/261, 6-7=0/47
 BOT CHORD 2-12=-614/912, 11-12=-614/912, 10-11=-130/405, 9-10=-130/405, 8-9=-173/233, 6-8=-173/233
 WEBS 3-12=-263/190, 3-11=-540/486, 4-11=-444/404, 4-9=-276/199, 5-9=-395/622, 5-8=-959/600

JOINT STRESS INDEX
 2 = 0.69, 3 = 0.42, 4 = 0.39, 5 = 0.42, 6 = 0.69, 8 = 0.35, 9 = 0.37, 10 = 0.15, 11 = 0.37 and 12 = 0.35

NOTES

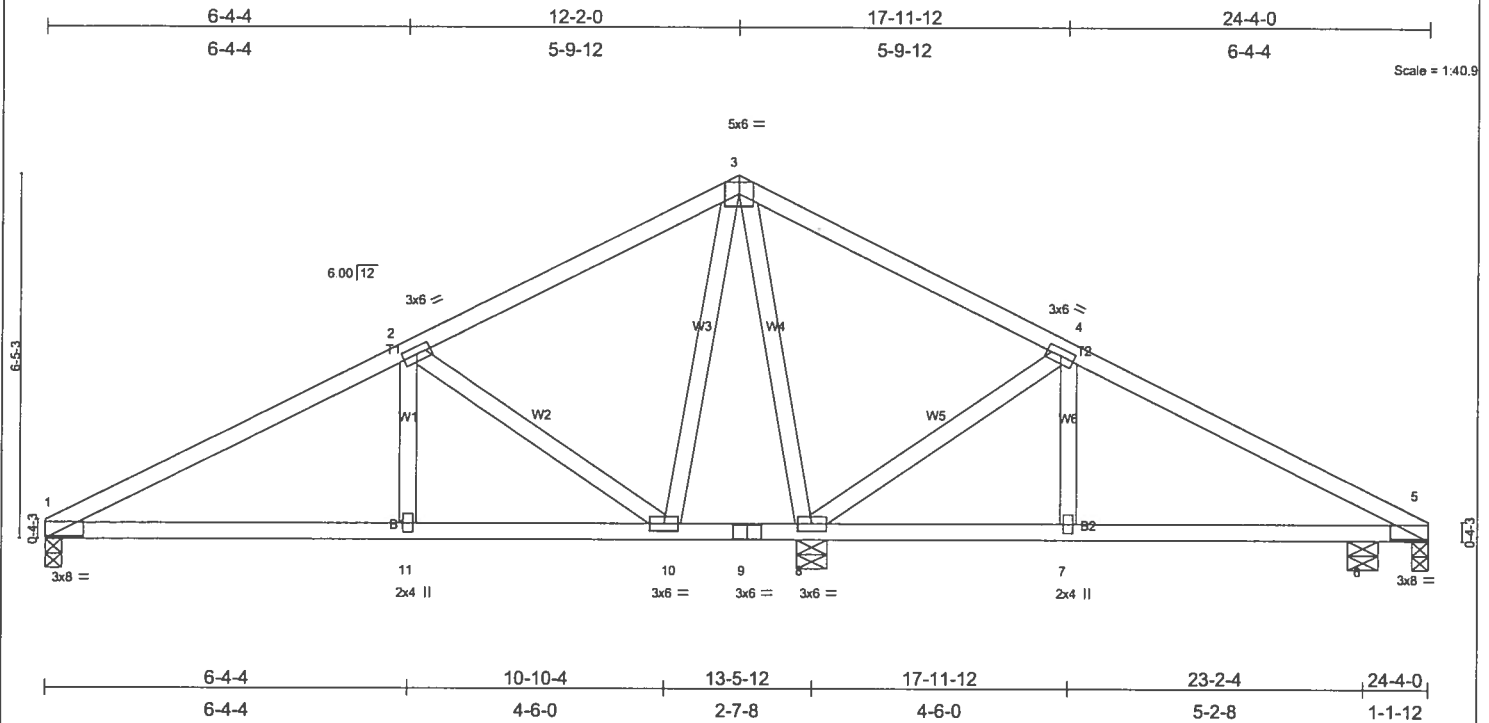
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 572 lb uplift at joint 2, 470 lb uplift at joint 8 and 201 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L222243	Truss T15	Truss Type COMMON	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.27	Vert(LL) 0.12	1-11	>999	240		MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.31	Vert(TL) -0.11	1-11	>999	180			
BCLL 10.0	Lumber Increase 1.25	WB 0.58	Horz(TL) 0.01	5	n/a	n/a			
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TP12002								
								Weight: 119 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=430/0-3-8, 5=6/0-3-8, 8=1255/0-6-7, 6=340/0-6-7

Max Horz 1=-89(load case 3)
 Max Uplift 1=-298(load case 5), 5=-72(load case 9), 8=-594(load case 5), 6=-145(load case 6)
 Max Grav 1=454(load case 9), 5=38(load case 10), 8=1255(load case 1), 6=340(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-599/501, 2-3=-70/124, 3-4=-128/389, 4-5=-285/58
 BOT CHORD 1-11=-386/474, 10-11=-386/474, 9-10=-122/192, 8-9=-122/192, 7-8=-35/186, 6-7=-35/186, 5-6=-35/186
 WEBS 2-11=-299/216, 2-10=-588/556, 3-10=-460/402, 3-8=-865/615, 4-8=-492/274, 4-7=0/148

JOINT STRESS INDEX

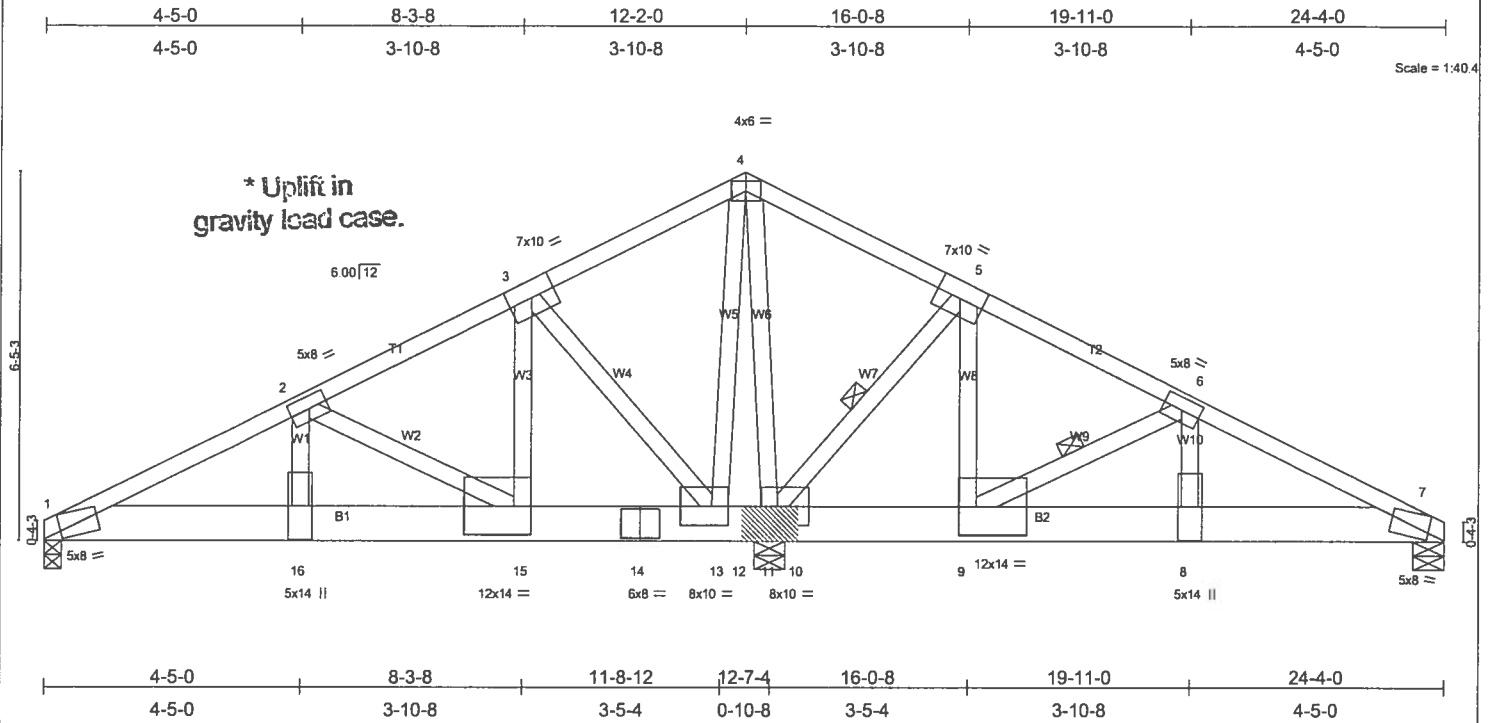
1 = 0.69, 2 = 0.41, 3 = 0.52, 4 = 0.41, 5 = 0.69, 7 = 0.34, 8 = 0.37, 9 = 0.15, 10 = 0.37 and 11 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 298 lb uplift at joint 1, 72 lb uplift at joint 5, 594 lb uplift at joint 8 and 145 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L222243	Truss T16	Truss Type COMMON	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:41 2007 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.46	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.73	Vert(LL) -0.10 8-9 >999 240	Weight: 177 lb	
BCLL 10.0	Lumber Increase 1.25	WB 0.95	Vert(TL) -0.16 8-9 >869 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 7 n/a n/a		
Code FBC2004/TPI2002					

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 8 SYP No.1D
 WEBS 2 X 4 SYP No.3 "Except"
 W3 2 X 4 SYP No.2, W8 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 5-11-9 oc bracing.
 WEBS 1 Row at midpt 5-11, 6-9

REACTIONS (lb/size) 1=-338/0-3-8, 7=3381/0-6-7, 11=6332/0-7-8 (0-6-7 + bearing block)
 Max Horz 1=-87(load case 2)
 Max Uplift1=-547(load case 9), 7=-1247(load case 5), 11=-2516(load case 4)
 Max Grav 1=139(load case 2), 7=3424(load case 9), 11=6332(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-399/1365, 2-3=-521/1636, 3-4=-656/1890, 4-5=-658/1892, 5-6=-1062/349, 6-7=-4440/1584
 BOT CHORD 1-16=-1213/452, 15-16=-1213/452, 14-15=-1454/618, 13-14=-1454/618, 12-13=-1579/781, 11-12=-1579/781, 10-11=-221/914,
 9-10=-221/914, 8-9=-1361/3934, 7-8=-1361/3934
 WEBS 2-16=-111/199, 2-15=-451/339, 3-15=-239/303, 3-13=-473/397, 4-13=-938/298, 4-11=-952/439, 5-11=-3857/1499, 5-9=-1429/3956,
 6-9=-3444/1363, 6-8=-1055/2948

JOINT STRESS INDEX
 1 = 0.92, 2 = 0.97, 3 = 0.73, 4 = 0.57, 5 = 0.73, 6 = 0.97, 7 = 0.92, 8 = 0.58, 9 = 0.79, 10 = 0.00, 10 = 0.00, 11 = 0.59, 11 = 0.00, 12 = 0.00, 12 = 0.00, 13 = 0.59, 14 = 0.19, 15 = 0.79 and 16 = 0.58

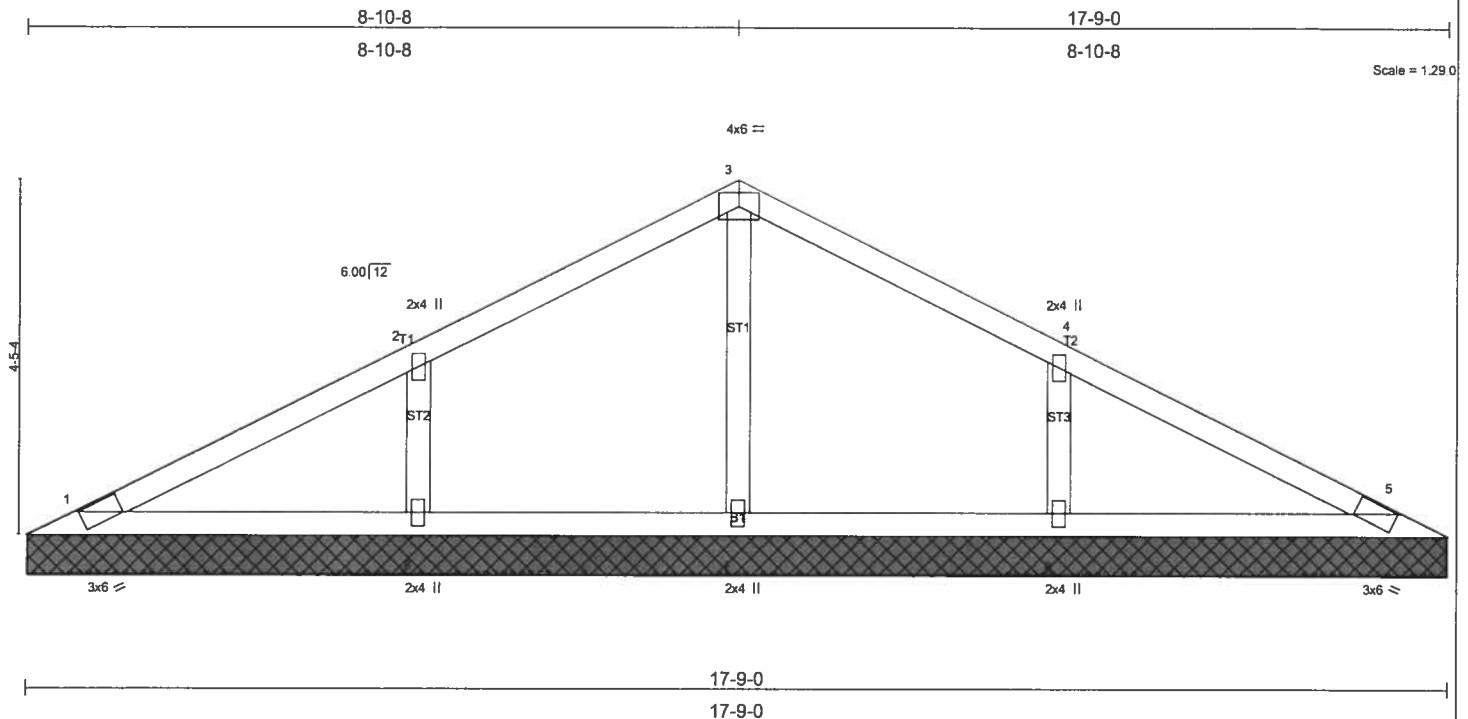
NOTES

- 1) 2 X 8 SYP No.1D bearing block 12" long at jt. 11 attached to front face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners. Bearing is assumed to be SYP.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 547 lb uplift at joint 1, 1247 lb uplift at joint 7 and 2516 lb uplift at joint 11.
- 6) Girder carries tie-in span(s): 34-0-0 from 13-0-0 to 24-4-0
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-4=-54, 4-7=-54, 1-10=-30, 7-10=-702(F=-672)

Job L222243	Truss V17	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:42 2007 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.13	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.12	Vert(LL) n/a - n/a 999		
BCLL 10.0	Lumber Increase 1.25	WB 0.06	Vert(TL) n/a - n/a 999		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 5 n/a n/a		
	Code FBC2004/TPI2002			Weight: 65 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=153/17-9-0, 5=153/17-9-0, 7=279/17-9-0, 8=401/17-9-0, 6=401/17-9-0
 Max Horz 1=-60(load case 3)
 Max Uplift 1=-43(load case 6), 5=-42(load case 6), 7=-16(load case 5), 8=-209(load case 5), 6=-209(load case 6)
 Max Grav 1=153(load case 1), 5=153(load case 1), 7=279(load case 1), 8=409(load case 9), 6=409(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-69/44, 2-3=-77/116, 3-4=-77/105, 4-5=-69/44
 BOT CHORD 1-8=0/61, 7-8=0/61, 6-7=0/61, 5-6=0/61
 WEBS 3-7=-177/46, 2-8=-258/220, 4-6=-258/220

JOINT STRESS INDEX
 1 = 0.23, 2 = 0.34, 3 = 0.31, 4 = 0.34, 5 = 0.23, 6 = 0.34, 7 = 0.34 and 8 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Gable requires continuous bottom chord bearing.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 1, 42 lb uplift at joint 5, 16 lb uplift at joint 7, 209 lb uplift at joint 8 and 209 lb uplift at joint 6.

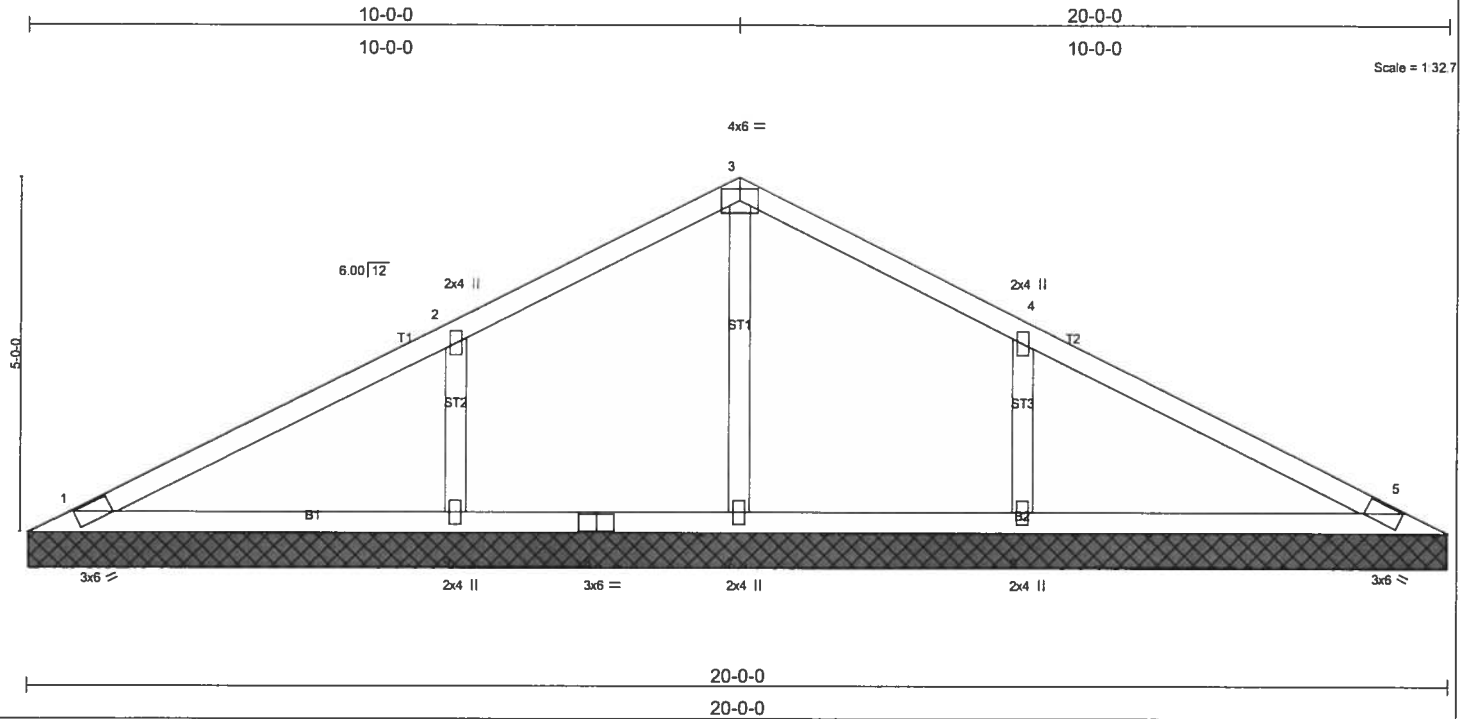
LOAD CASE(S) Standard

Job L222243	Truss V19	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
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Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.26	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.19	Vert(LL) n/a - n/a 999		
BCLL 10.0	Lumber Increase 1.25	WB 0.06	Vert(TL) n/a - n/a 999		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 5 n/a n/a		
	Code FBC2004/TPI2002			Weight: 75 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS

(lb/size) 1=191/20'-0", 5=191/20'-0", 7=226/20'-0", 9=484/20'-0", 6=484/20'-0"
 Max Horz 1=-68(load case 3)
 Max Uplift 1=-52(load case 6), 5=-57(load case 6), 9=-250(load case 5), 6=-250(load case 6)
 Max Grav 1=191(load case 1), 5=191(load case 1), 7=226(load case 1), 9=488(load case 9), 6=488(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-76/61, 2-3=-79/134, 3-4=-79/123, 4-5=-76/61
 BOT CHORD 1-9=0/74, 8-9=0/74, 7-8=0/74, 6-7=0/74, 5-6=0/74
 WEBS 3-7=-149/16, 2-9=-303/257, 4-6=-303/257

JOINT STRESS INDEX

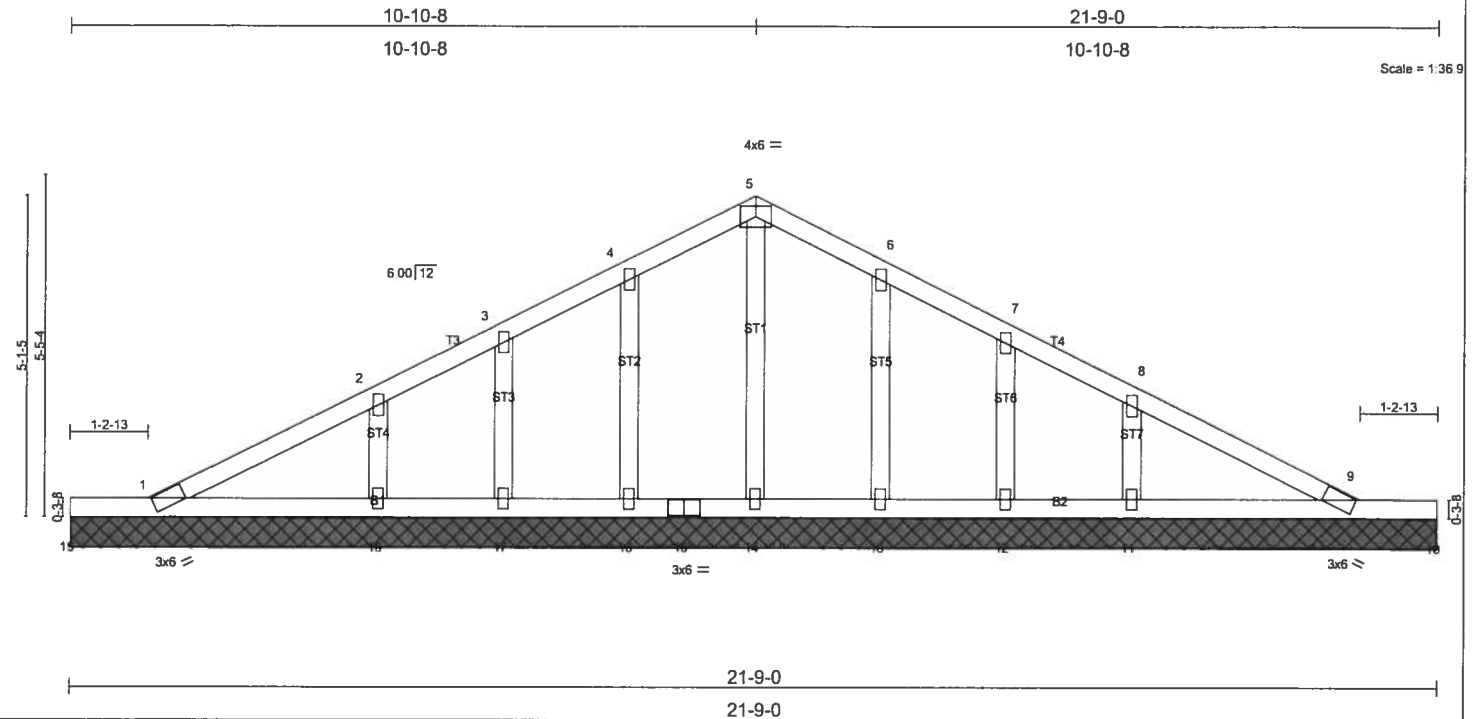
1 = 0.37, 2 = 0.34, 3 = 0.25, 4 = 0.34, 5 = 0.37, 6 = 0.34, 7 = 0.34, 8 = 0.15 and 9 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Gable requires continuous bottom chord bearing.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1, 57 lb uplift at joint 5, 250 lb uplift at joint 9 and 250 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L222243	Truss V21	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:46 2007 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.21	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.19	Vert(LL) n/a - n/a 999		
BCLL 10.0	Lumber Increase 1.25	WB 0.08	Vert(TL) n/a - n/a 999		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.00 10 n/a n/a		
	Code FBC2004/TPI2002			Weight: 94 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 19=73/21-9-0, 14=258/21-9-0, 16=306/21-9-0, 17=218/21-9-0, 18=471/21-9-0, 13=306/21-9-0, 10=73/21-9-0, 12=218/21-9-0, 11=471/21-9-0, 1=345/21-9-0, 9=345/21-9-0
 Max Horz 19=70(load case 4)
 Max Uplift 19=73(load case 1), 16=137(load case 5), 17=102(load case 5), 18=215(load case 5), 13=136(load case 6), 10=73(load case 1), 12=102(load case 6), 11=215(load case 6), 1=101(load case 6), 9=107(load case 6)
 Max Grav 19=39(load case 5), 14=258(load case 1), 16=310(load case 9), 17=218(load case 1), 18=471(load case 9), 13=310(load case 10), 10=39(load case 6), 12=218(load case 1), 11=471(load case 10), 1=345(load case 1), 9=345(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=117/71, 2-3=93/86, 3-4=72/113, 4-5=77/148, 5-6=77/142, 6-7=72/89, 7-8=93/58, 8-9=117/71
 BOT CHORD 1-19=70/70, 1-18=0/77, 17-18=0/77, 16-17=0/77, 15-16=0/77, 14-15=0/77, 13-14=0/77, 12-13=0/77, 11-12=0/77, 9-11=0/77, 9-10=0/0
 WEBS 5-14=201/11, 4-16=244/148, 3-17=180/117, 2-18=357/221, 6-13=244/147, 7-12=180/117, 8-11=357/221

JOINT STRESS INDEX
 1 = 0.38, 2 = 0.34, 3 = 0.34, 4 = 0.34, 5 = 0.25, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.38, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.15, 16 = 0.34, 17 = 0.34 and 18 = 0.34

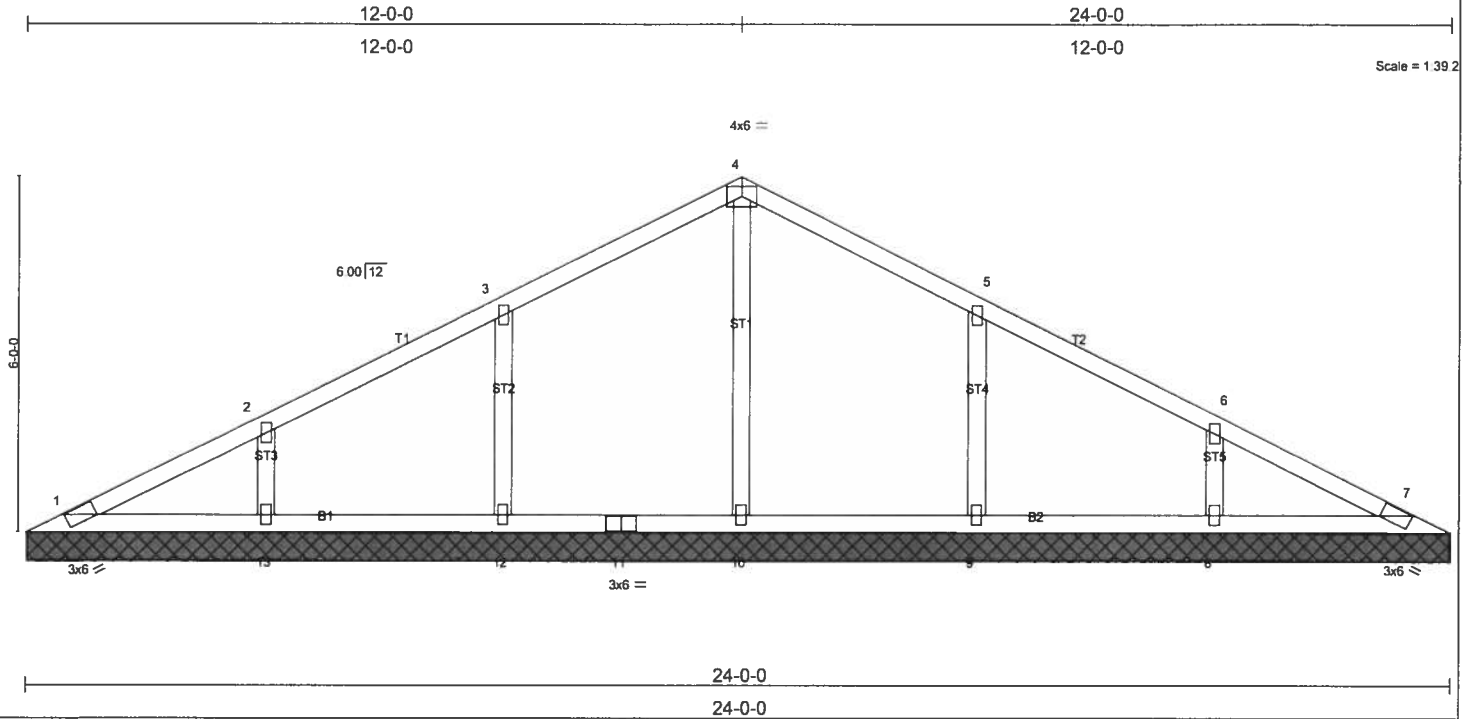
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 73 lb uplift at joint 19, 137 lb uplift at joint 16, 102 lb uplift at joint 17, 215 lb uplift at joint 18, 136 lb uplift at joint 13, 73 lb uplift at joint 10, 102 lb uplift at joint 12, 215 lb uplift at joint 11, 101 lb uplift at joint 1 and 107 lb uplift at joint 9.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-19=-10, 1-9=-30, 9-10=-10, 1-5=-114(F=-60), 5-9=-114(F=-60)

Job L222243	Truss V23	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:47 2007 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.10	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(LL) n/a - n/a 999		
BCLL 10.0	Lumber Increase 1.25	WB 0.11	Vert(TL) n/a - n/a 999		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 7 n/a n/a		
	Code FBC2004/TPI2002			Weight: 96 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 1=120/24'-0", 7=120/24'-0", 10=313/24'-0", 12=334/24'-0", 13=346/24'-0", 9=334/24'-0", 8=346/24'-0"
 Max Horz 1=-83(load case 3)
 Max Uplift 1=-28(load case 6), 7=-13(load case 5), 12=-176(load case 5), 13=-177(load case 5), 9=-175(load case 6), 8=-177(load case 6)
 Max Grav 1=120(load case 1), 7=120(load case 1), 10=313(load case 1), 12=345(load case 9), 13=346(load case 1), 9=345(load case 10), 8=346(load case 1)

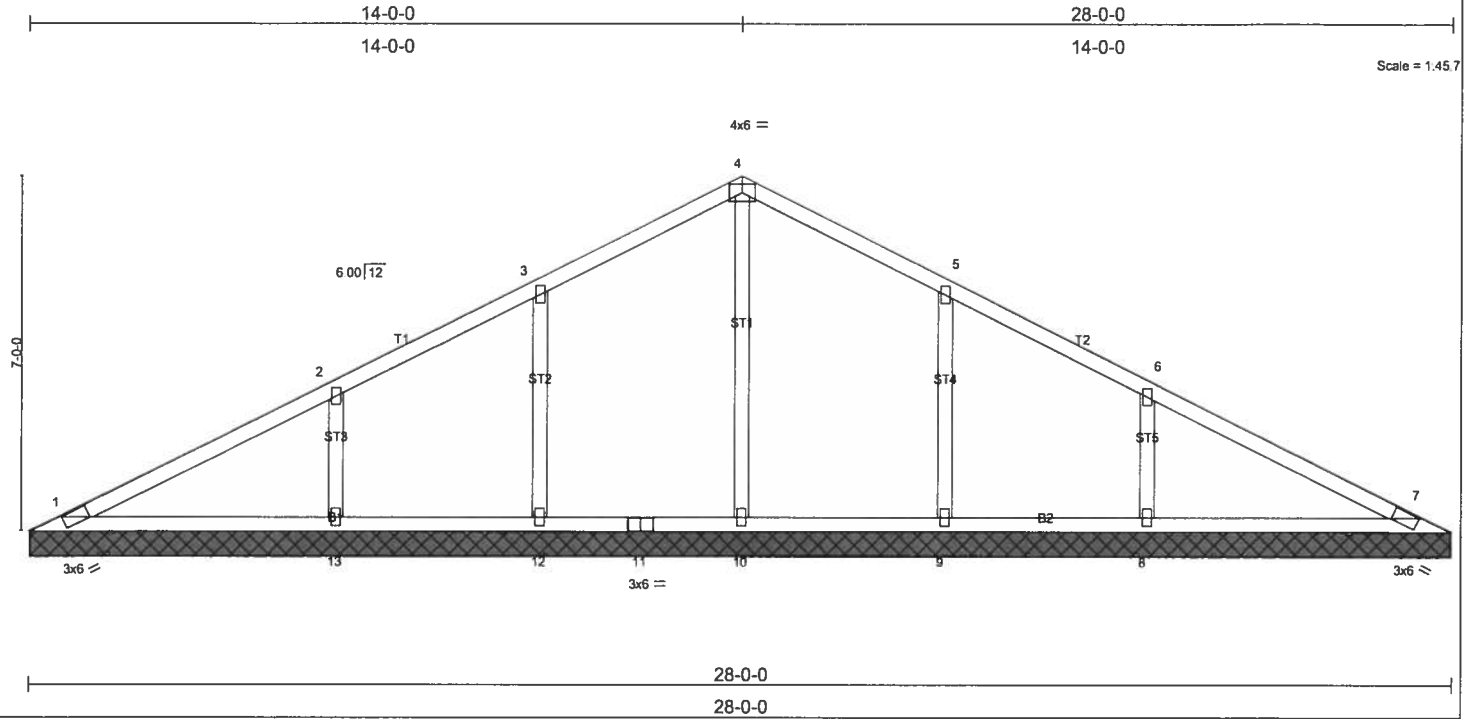
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-100/42, 2-3=-75/94, 3-4=-75/164, 4-5=-75/152, 5-6=-75/61, 6-7=-67/31
 BOT CHORD 1-13=0/89, 12-13=0/89, 11-12=0/89, 10-11=0/89, 9-10=0/89, 8-9=0/89, 7-8=0/89
 WEBS 4-10=-192/19, 3-12=-227/202, 2-13=-217/191, 5-9=-227/202, 6-8=-217/191

JOINT STRESS INDEX
 1 = 0.23, 2 = 0.34, 3 = 0.34, 4 = 0.36, 5 = 0.34, 6 = 0.34, 7 = 0.23, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.15, 12 = 0.34 and 13 = 0.34

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) All plates are 2x4 MT20 unless otherwise indicated.
 4) Gable requires continuous bottom chord bearing.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 13 lb uplift at joint 7, 176 lb uplift at joint 12, 177 lb uplift at joint 13, 175 lb uplift at joint 9 and 177 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L222243	Truss V27	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Jan 08 14:35:49 2007 Page 1		



LOADING (psf)	SPACING 2'-0"	CSI	DEFL in (loc) l/defl L/d	PLATES MT20	GRIP 244/190
TCLL 20.0	Plates Increase 1.25	BC 0.25	Vert(LL) n/a - n/a 999		
TCDL 7.0	Lumber Increase 1.25	BC 0.18	Vert(TL) n/a - n/a 999		
BCLL 10.0	Rep Stress Incr YES	WB 0.16	Horz(TL) 0.00 7 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			Weight: 116 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 1=196/28'-0", 7=196/28'-0", 10=335/28'-0", 12=286/28'-0", 13=474/28'-0", 9=286/28'-0", 8=474/28'-0"
 Max Horz 1=97(load case 4)
 Max Uplift 1=49(load case 6), 7=41(load case 6), 12=151(load case 5), 13=243(load case 5), 9=151(load case 6), 8=243(load case 6)
 Max Grav 1=196(load case 1), 7=196(load case 1), 10=335(load case 1), 12=298(load case 9), 13=474(load case 1), 9=298(load case 10), 8=474(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-102/57, 2-3=-85/130, 3-4=-73/193, 4-5=-73/180, 5-6=-85/90, 6-7=-85/55
 BOT CHORD 1-13=0/107, 12-13=0/107, 11-12=0/107, 10-11=0/107, 9-10=0/107, 8-9=0/107, 7-8=0/107
 WEBS 4-10=-204/5, 3-12=-201/182, 2-13=-291/251, 5-9=-201/181, 6-8=-291/251

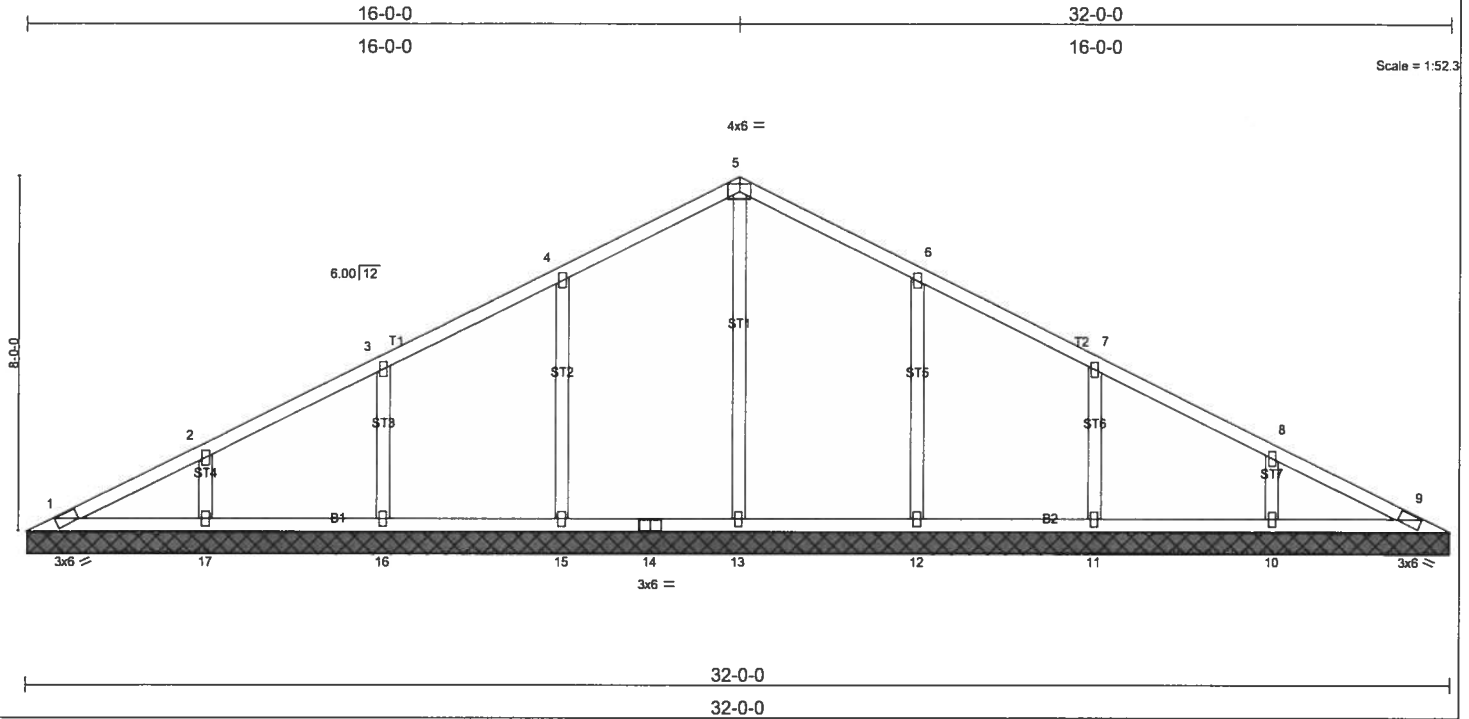
JOINT STRESS INDEX
 1 = 0.38, 2 = 0.34, 3 = 0.34, 4 = 0.41, 5 = 0.34, 6 = 0.34, 7 = 0.38, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.15, 12 = 0.34 and 13 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 41 lb uplift at joint 7, 151 lb uplift at joint 12, 243 lb uplift at joint 13, 151 lb uplift at joint 9 and 243 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L222243	Truss V31	Truss Type GABLE	Qty 1	Ply 1	CLARK CONST. - LOT 58 SA
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jan 08 14:35:50 2007 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.10	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(LL) n/a - n/a 999		
BCLL 10.0	Lumber Increase 1.25	WB 0.20	Vert(TL) n/a - n/a 999		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 9 n/a n/a		
	Code FBC2004/TP12002			Weight: 141 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 1=120/32'-0", 9=120/32'-0", 13=312/32'-0", 15=336/32'-0", 16=334/32'-0", 17=346/32'-0", 12=336/32'-0", 11=334/32'-0", 10=346/32'-0"
 Max Horz 1=112(load case 4)
 Max Uplift 1=-24(load case 6), 9=-4(load case 5), 15=-176(load case 5), 16=-172(load case 5), 17=-178(load case 5), 12=-176(load case 6), 11=-172(load case 6), 10=-178(load case 6)
 Max Grav 1=120(load case 1), 9=120(load case 1), 13=312(load case 1), 15=347(load case 9), 16=334(load case 1), 17=346(load case 9), 12=347(load case 10), 11=334(load case 1), 10=346(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-154/47, 2-3=-75/83, 3-4=-75/156, 4-5=-75/225, 5-6=-75/212, 6-7=-75/111, 7-8=-75/38, 8-9=-109/32
 BOT CHORD 1-17=0/126, 16-17=0/126, 15-16=0/126, 14-15=0/126, 13-14=0/126, 12-13=0/126, 11-12=0/126, 10-11=0/126, 9-10=0/126
 WEBS 5-13=-192/0, 4-15=-226/199, 3-16=-216/198, 2-17=-217/192, 6-12=-226/199, 7-11=-216/198, 8-10=-217/192

JOINT STRESS INDEX
 1 = 0.23, 2 = 0.34, 3 = 0.34, 4 = 0.34, 5 = 0.36, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.23, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.15, 15 = 0.34, 16 = 0.34 and 17 = 0.34

NOTES

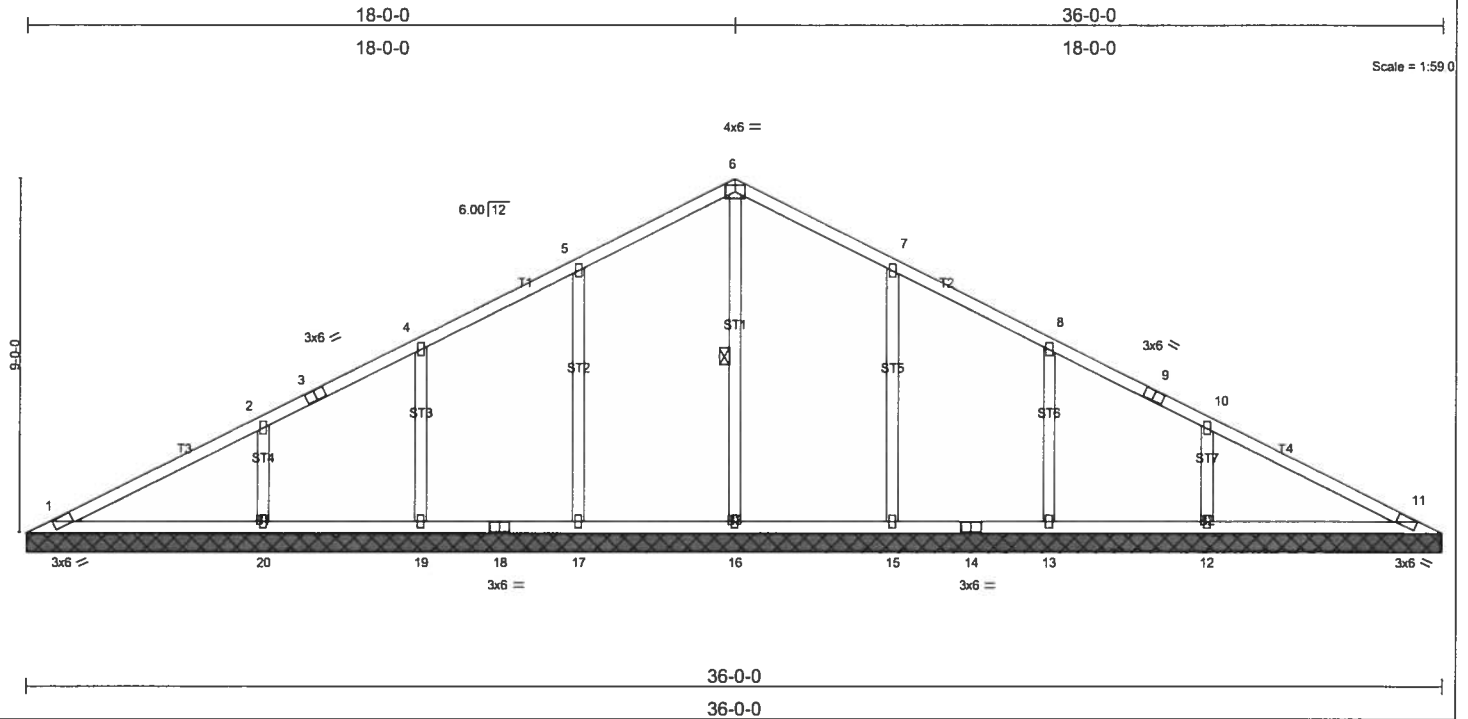
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 1, 4 lb uplift at joint 9, 176 lb uplift at joint 15, 172 lb uplift at joint 16, 178 lb uplift at joint 17, 176 lb uplift at joint 12, 172 lb uplift at joint 11 and 178 lb uplift at joint 10.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	CLARK CONST. - LOT 58 SA
L222243	V35	GABLE	1	1	Job Reference (optional)

Job Reference (optional)

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL) n/a - n/a 999	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.18	Vert(TL) n/a - n/a 999		
BCLL 10.0	Rep Stress Incr YES	WB 0.18	Horz(TL) 0.01 11 n/a n/a		
BCDL 5.0	Code FBC2004/TP1/2002	(Matrix)			Weight: 164 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-16

REACTIONS (lb/size) 1=195/36-0-0, 16=307/36-0-0, 17=349/36-0-0, 19=289/36-0-0, 20=473/36-0-0, 15=349/36-0-0, 13=289/36-0-0, 12=473/36-0-0, 11=195/36-0-0
 Max Horz 1=126(load case 4)
 Max Uplift 1=-44(load case 6), 17=-182(load case 5), 19=-149(load case 5), 20=-244(load case 5), 15=-181(load case 6), 13=-149(load case 6), 12=-244(load case 6), 11=-22(load case 6)
 Max Grav 1=195(load case 1), 16=307(load case 1), 17=359(load case 9), 19=289(load case 1), 20=474(load case 9), 15=359(load case 10), 13=289(load case 1), 12=474(load case 10),
 11=195(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-157/62, 2-3=-83/104, 3-4=-37/117, 4-5=-72/185, 5-6=-75/255, 6-7=-75/243, 7-8=-72/137, 8-9=-24/66, 9-10=-83/53, 10-11=-106/57
BOT CHORD 1-2=0/146, 19-20=0/146, 18-19=0/146, 17-18=0/146, 16-17=0/146, 15-16=0/146, 14-15=0/146, 13-14=0/146, 12-13=0/146, 11-12=0/146
WEBS 6-16=-190/0, 5-17=-233/204, 4-19=-191/179, 2-20=-292/251, 7-15=-233/204, 8-13=-191/179, 10-12=-292/251

JOINT STRESS INDEX
1 = 0.38, 2 = 0.34, 3 = 0.15, 4 = 0.34, 5 = 0.34, 6 = 0.35, 7 = 0.34, 8 = 0.34, 9 = 0.15, 10 = 0.34, 11 = 0.38, 12 = 0.34, 13 = 0.34, 14 = 0.15, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.15, 19 = 0.34 and 20 = 0.34

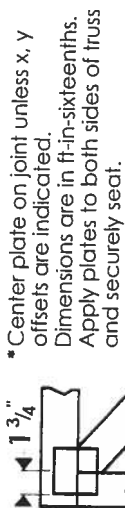
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); $h=14$ ft; TCDFL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 182 lb uplift at joint 17, 149 lb uplift at joint 19, 244 lb uplift at joint 20, 181 lb uplift at joint 15, 149 lb uplift at joint 13, 244 lb uplift at joint 12 and 22 lb uplift at joint 11.

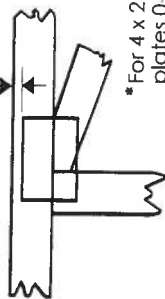
LOAD CASE(S) Standard

Symbols

PLATE LOCATION AND ORIENTATION



0-1/16"



* This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

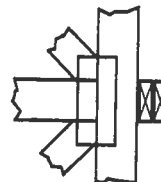
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

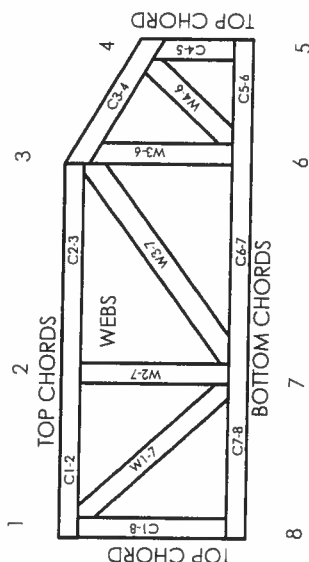


General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP11.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP11.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A



Mitek Engineering Reference Sheet: MIL-7473

RESIDENTIAL WIND DESIGN & ANALYSIS

NO COPIES ARE TO BE PERMITTED

PREPARED FOR:

RONALD CLARK CONSTRUCTION \ THE FINLEY RESIDENCE

**SCHAFER ENGINEERING, LLC
7104 N.W. 42ND LANE
GAINESVILLE FL 32606
386-462-1340 / 352-375-6329**

SUMMARY

OF WIND DESIGN & ANALYSIS

Trusses: Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

Roof sheathing: Type OSB Size 7/16 Fastener type Nails Size 8d/131 EA.
Interior zone spacing: Interior 8 in. Periphery 4 in.
Edge and end zone spacing: Interior 8 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

Studs: Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4
Interior stud spacing 16 in. Composite (yes or no) Y
End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB Thickness 7/16 in.
63' — Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.
44' — Long: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.

Allowable unit shear on shearwalls: 323 pounds per linear foot

Wall tension transferred by: Siding nails 8d/131@ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi
Size 1/2 in. Shape L Washer 2" Embedment 7 in.
Location of first anchor bolt from corner 8 in.

Anchor Bolts @ 48" O.C. Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. # 5 at 72 in.
Monolithic footing: Depth 20 in. Bottom width 12 in.

Footing: Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars
Interior Footings: 16" W X 10" D

Porch Columns: 6X6X10' SYP #2 ST @ 16" O.C.

Porch Column Fasteners: Simpson CB66 / CC66 OR EQUAL

NOTE:

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

Finley Rd
B-Shul
12-20-08
48984
704 NW 42nd LN
GVILLE FL

WOOD FASCIA - REFER TO
CONST DWG'S (PREPARED BY
OTHERS)

OH VARIES

GABLE END TRUSS

10d GUN NAILS @ 6" OC

SIMPSON ST22 @ 48" OC

SEE CONST DWG'S
(PREPARED BY OTHERS)
FOR TYP WALL

(3) 10d GUN NAILS
(TYP)

COMMON WD TRUSS (TYP)

2x4 GABLE X-BRACING @
48" OC (TYP)

1/2" GYP CLG BOARD (MIN)

THIS WIND ANALYSIS ASSUMES IDEAL SOIL
CONDITIONS W/MIN 2500 PSI SOIL BRG
CAPACITY & 95% DRY PROCTOR DENSITIES

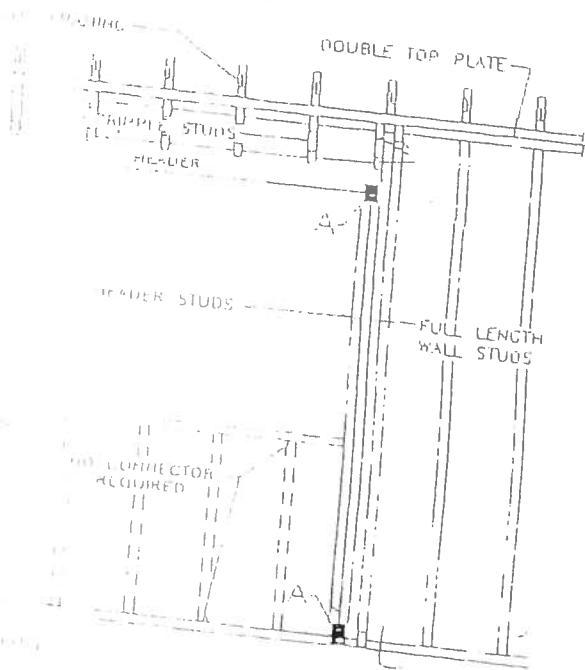
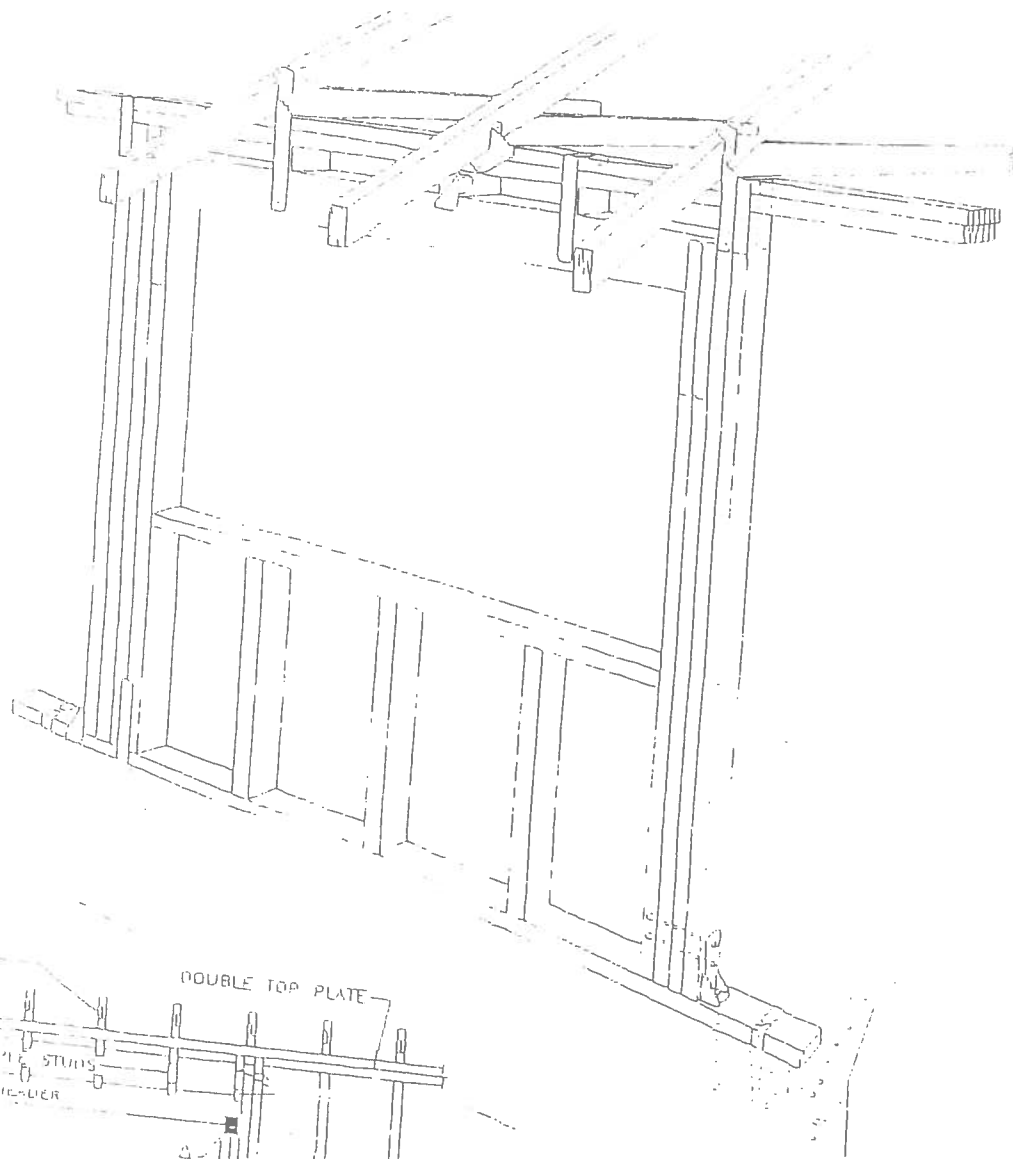
GABLE END DETAIL

SCALE: 1/2" = 1'-0"

NOTE:

INSTALL AND ERECT ALL TRUSS MEMBERS IN STRICT CONFORMANCE WITH
THE PRE-ENGINEERED ROOF TRUSS MANUFACTURERS ERECTION SHEET
ACCOMPANYING THE TRUSS PACKAGE. IF NOT AVAILABLE, IT IS THE
CONTRACTOR'S RESPONSIBILITY TO CONTACT THE SUPPLIER TO OBTAIN
ERECTION & BRACING SHEET.

B. Shl
12-20-06
48984
7104 NW 42nd Ln
Gville FL



		Maximum Header Span (ft)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting Ends of Header					
		1	1	2	2	3	3
		Number of Full Length Studs at Each End of Header					
Unsupported Wall Height	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
greater than 10'	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

Total each truss uplift on the header divide by 2 for header anchorage

TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700
Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.				

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A
Two 12d common toenails are required per truss/rafter per bearing point into plate. Use proper bolt anchors. Strap rafters to truss or at each end with minimum uplift resistance of 450# each end. Strap ridge beam at each end with minimum uplift resistance of 1000#. It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.		

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300
*or per truss engineering Use proper bolt anchors All beams to be sheathed or strapped to Double Top Plate when applicable.				

CRIPPLES	Sheathing nailing alone adequate w/8d nails @ 3" O.C.
-----------------	---

STUDS
Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
Use SP1 & SP2 @ 32" O.C. on all bearing walls.
Interior anchor bolts to be 1/2" x 8" A307 or 1/2" x 7" wedge anchor or equivalent.

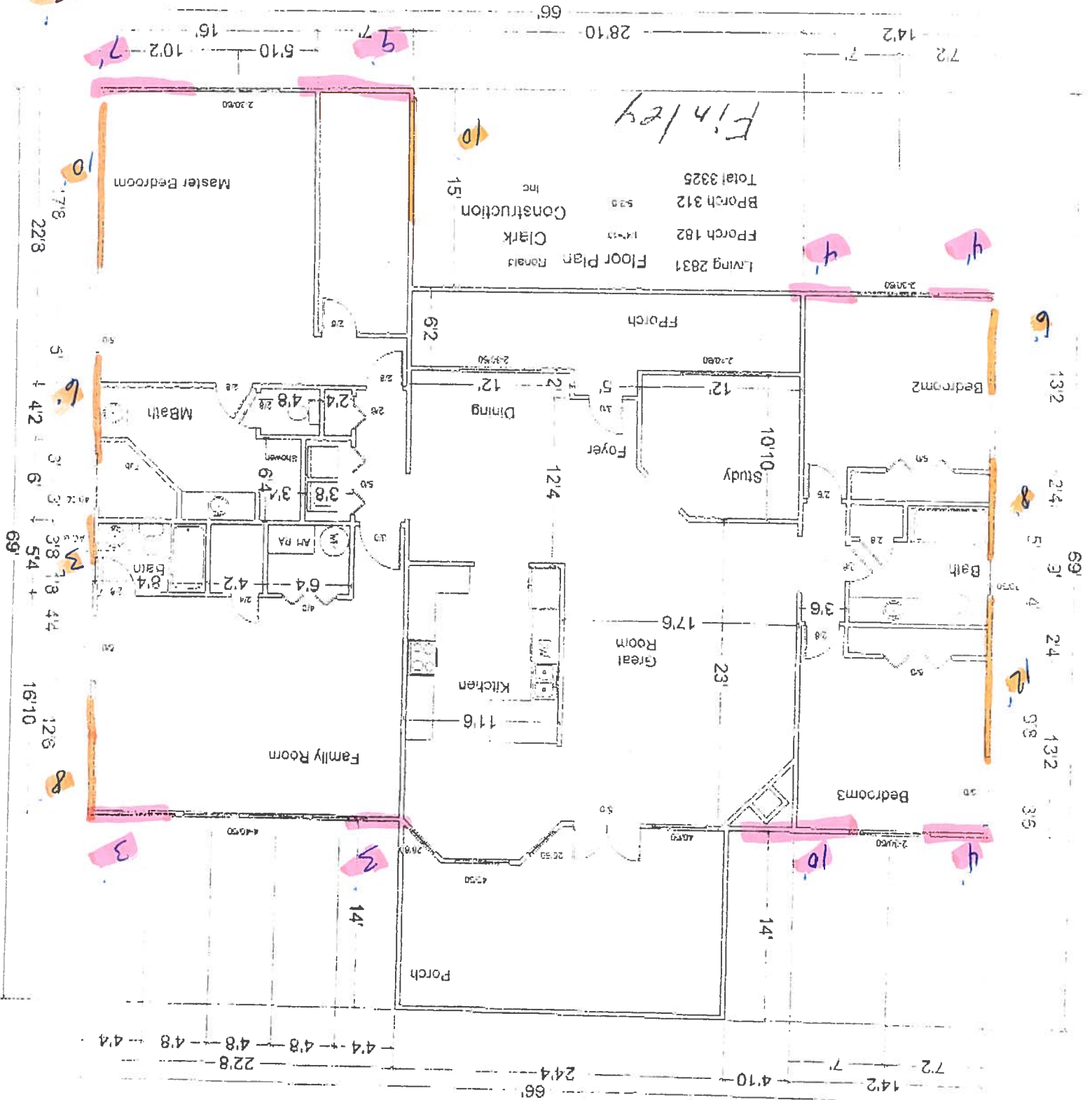
** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE:

1. For nailing into SPF members, multiply table values by .86
2. See truss engineering for anchor tie-down values.

hta - ring

TS-63



ASCE 7-02

12/8/06

Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	10.00	ft
Ridge Height (RHt)	21.38	ft
Mean Roof Height (Ht)	16.19	ft
Width Perp. to Wind (B)	69.00	ft
Width Parallel to Wind (L)	66.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.25
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
lzm	$Cc * (33/z)^{0.167}$	0.3048	
Lzm	$l * (zm/33)^{Epsilon}$	309.99	ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.8842	
Gust2	$0.925 * ((1+1.7 * lzm * 3.4 * Q)/(1+1.7 * 3.4 * lzm))$	0.8566	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	$V * (5280/3600)$	161.33	ft/s
Vzm	$bm * (zm/33)^{Am} * Vhref$	70.89	ft/s
NF1	$NatFreq * Lzm / Vzm$	4.37	Hz
Rn	$(7.47 * NF1) / (1 + 10.302 * NF1)^{1.667}$	0.0552	
Nh	$4.6 * NatFreq * Ht / Vzm$	1.05	
Nb	$4.6 * NatFreq * B / Vzm$	4.48	
Nd	$15.4 * NatFreq * Depth / Vzm$	14.34	
Rh	$1/Nh - (1/(2 * Nh^2) * (1 - Exp(-2 * Nh)))$	0.5543	
Rb	$1/Nb - (1/(2 * Nb^2) * (1 - Exp(-2 * Nb)))$	0.1984	
Rd	$1/Nd - (1/(2 * Nd^2) * (1 - Exp(-2 * Nd)))$	0.0673	
RR	$((1/Beta) * Rn * Rh * Rb * (0.53 + 0.47 * Rd))^{0.5}$	0.5838	
gg	$+(2 * LN(3600 * n1))^{0.5} + 0.577 / (2 * LN(3600 * n1))^{0.5}$	4.19	
Gust3	$0.925 * ((1 + 1.7 * lzm * (3.4^2 * Q^2 + GG^2 * RR^2)^{0.5}) / (1 + 1.7 * 3.4 * lzm))$	1.01	

Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

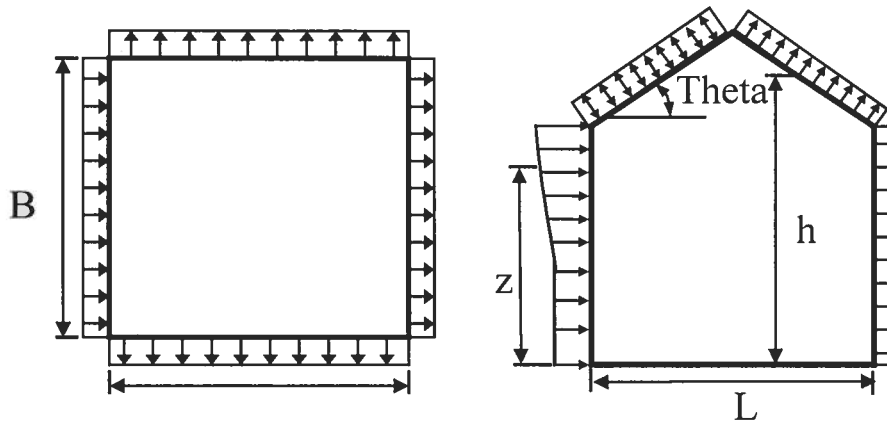
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft^2	Pressure (lb/ft^2)	
					Windward Wall*	
			1.00		+GCpi	-GCpi
21.38	0.70	1.00	1.00	21.70	11.60	18.15
20	0.70	1.00	1.00	21.70	11.60	18.15
16.19	0.70	1.00	1.00	21.70	11.60	18.15
15	0.70	1.00	1.00	21.70	11.60	18.15

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (Ht/zg)^{2/\alpha}$	0.59	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	18.20	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 69 ft wall)	-0.50	-11.07	-4.52
Leeward Walls (Wind Dir Parallel to 66 ft wall)	-0.49	-10.93	-4.38
Side Walls	-0.70	-14.19	-7.64
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	-0.20	-6.39	0.16
Windward - Max Positive	0.30	1.40	7.95
Leeward Normal to Ridge	-0.60	-12.63	-6.08
Overhang Top	-0.20	-3.12	-3.12
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 8.095 ft	-0.90	-17.30	-10.75

ASCE 7-02

12/8/06

Wind Load Design per ASCE 7-02

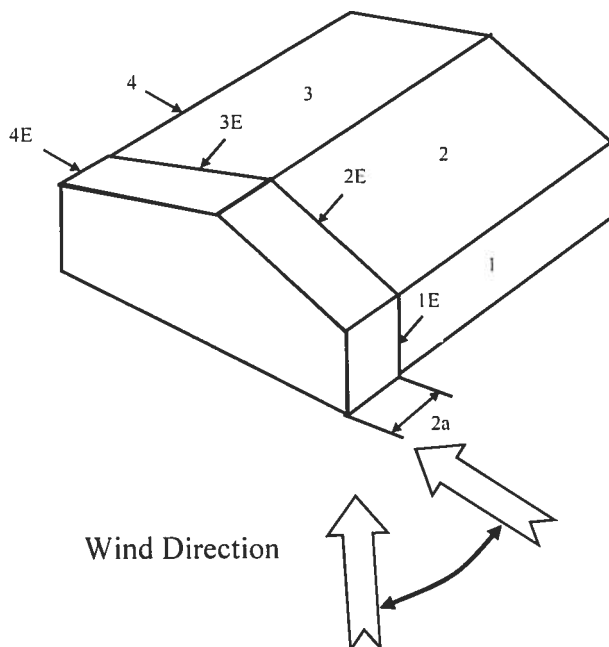
Dist from Windward Edge: 8.095 ft to 16.19 ft	-0.90	-17.30	-10.75
Dist from Windward Edge: 16.19 ft to 32.38 ft	-0.50	-11.07	-4.52
Dist from Windward Edge: > 32.38 ft	-0.30	-7.95	-1.40

* Horizontal distance from windward edge

Figure 6-4 - External Pressure Coefficients, GCpfLoads on Main Wind-Force Resisting Systems w/ $H_t \leq 60$ ft

$K_h =$	$2.01 \cdot (H_t/z_g)^{(2/\alpha)}$	$=$	0.59
$K_{ht} =$	Topographic factor (Fig 6-2)	$=$	1.00
$Q_h =$	$0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d$	$=$	18.20

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* $p = q_h \cdot (GC_{pf} - GC_{pi})$ 

ASCE 7-02

12/8/06

Wind Load Design per ASCE 7-02

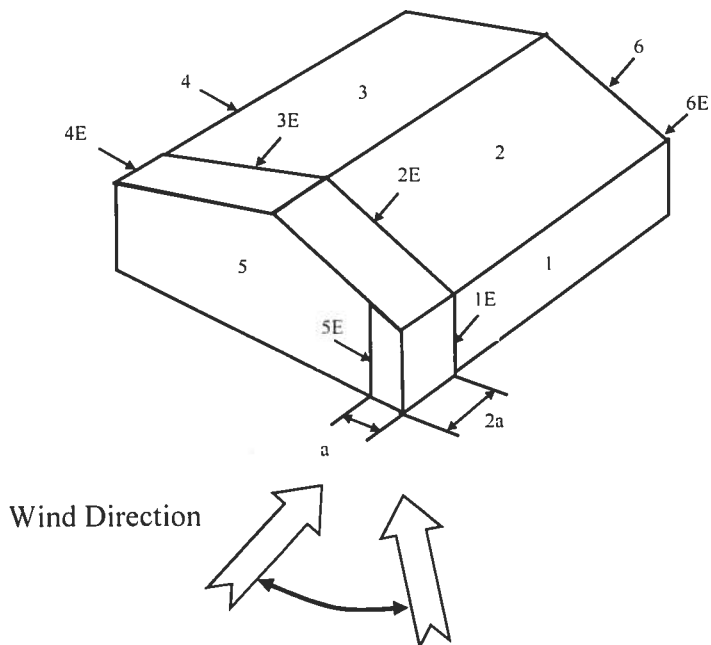
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (H_t/z_g)^{2/\alpha} &= & 0.59 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 18.20 \end{aligned}$$

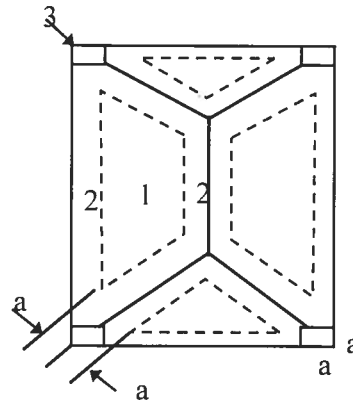
Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

$$* p = q_h \cdot (GC_{pf} - GC_{pi})$$



12/8/06

Figure 6-5 - External Pressure Coefficients, G_{Cp}



10 < Theta <= 30

$$a = 6.476 \quad \Rightarrow \quad \boxed{6.48 \text{ ft}}$$
[illegible]

Table 6-7 Internal Pressure Coefficients for Buildings, C_{gpi}

ASCE 7-02

12/8/06

Wind Load Design per ASCE 7-02

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.33	-18.86	-11.07
	P (-GCpi) -psf	5.22	-12.31	-4.52
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.27	-18.86	-11.07
	P (-GCpi) -psf	3.27	-18.86	-11.07

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	66.00	ft
B	Roof dimension parallel to wind direction	69.00	ft
L/B	Ratio of L to B	0.957	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.19	
X	Distance to center of pressure from windward edge	0.42	ft

PRE-ENGINEERED WOOD
TRUSSES @ 2'-0" O.C.
AND HURRICANE TIED
AT EACH END

FIBERGLASS/ASPHALT SHINGLE ROOFING
ON # 30 ROOFING FELT ON 7/16"
O.S.B. SHEATHING NAILED AS REQ.

R-30 FIBERGLASS CEILING INSULATION

2" X 4" WALLS WITH
DBL. TOP PLATE AND
P.T. BOTTOM PLATE,
WITH DBL. 2" X 12"
WINDOW AND DOOR
HEADERS

2' - 0" OVERHANG WITH
2" X 6" SUB-FASCIA AND
ALUMINUM FASCIA AND
SOFFIT

R-13 FIBERGLASS WALL
INSULATION AND 1/2"
SHEETROCK WITH
HARDCOTE PLASTER
FINISH

7/16" O.S.B. WALL SHEATHING
WITH HOUSEWRAP VAPOR
BARRIER AND HARDI-PLANK
LAP SIDING

4" THICK CONC. SLAB
ON 6 MIL VAPOR
BARRIER ON CLEAN,
COMPACTED, AND
TREATED FILL DIRT

1/2" X 8" ANCHOR BOLTS
(AS REQUIRED)

GRADE

10" X 20" CONC. FOOTING
WITH 2 # 5 REBAR CONT.
AND 8" C.M.U. FOUNDATION
WALL POURED SOLID

WALL SECTION

3/4" = 1'-0"



Duct System Summary Entire House Bounds Heating & Air

Job: Finley Job
Date: 12-21-06
By: Michael C.

P.O. BOX 1617, Newberry, FL 32669 Phone: (352)472-2761 Fax: (352) 472-1809

Project Information

For: Ronald Clark Construction

External static pressure	Heating	Cooling
Pressure losses	1.00 in H2O	1.00 in H2O
Available static pressure	0.30 in H2O	0.30 in H2O
Supply / return available pressure	0.70 in H2O	0.70 in H2O
Lowest friction rate	0.53 / 0.17 in H2O	0.53 / 0.17 in H2O
Actual air flow	0.100 in/100ft	0.100 in/100ft
Total effective length (TEL)	1850 cfm	1850 cfm
	503 ft	

Supply Branch Detail Table

Name	Design (Btu/h)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
bed room 3	h 4200	168	131	0.100	8	16x4	VIFx	50.0	305.0	st2
Bath 2	c 1238	54	66	0.100	6	16x2	VIFx	42.0	315.0	st2
Bed room 2	h 2540	102	64	0.100	7	16x3	VIFx	52.0	305.0	st2
Bed room 2-A	h 2540	102	64	0.100	7	16x3	VIFx	55.0	325.0	st2
Great room-A	c 2050	99	107	0.100	7	16x3	VIFx	32.0	255.0	st2
Great room	c 2050	99	107	0.100	7	16x3	VIFx	26.0	265.0	st2
study	h 2664	107	85	0.100	7	16x3	VIFx	36.0	255.0	st2
nook	c 2319	118	121	0.100	7	16x3	VIFx	28.0	180.0	st2
kitchen	c 310	8	16	0.100	4	16x1	VIFx	25.0	180.0	st2
dining room	h 2708	108	88	0.100	7	16x3	VIFx	15.0	180.0	st2
foyer	h 1237	50	43	0.100	5	16x1	VIFx	27.0	265.0	st2
hall	c 55	1	3	0.100	4	16x1	VIFx	39.0	320.0	st2
hall 2	c 102	3	5	0.100	4	16x1	VIFx	15.0	180.0	st2
bath	c 962	33	50	0.100	5	16x1	VIFx	13.0	160.0	st1
master bath	c 2018	61	105	0.100	7	16x3	VIFx	18.0	120.0	st1
family room-A	c 2650	123	138	0.100	8	16x4	VIFx	17.0	160.0	st1
family room	c 2650	123	138	0.100	8	16x4	VIFx	6.0	95.0	
family room-B	c 2650	123	138	0.100	8	16x4	VIFx	12.0	120.0	st1
master suite-A	c 2440	123	127	0.100	7	16x3	VIFx	20.0	125.0	st1
master suite	c 2440	123	127	0.100	7	16x3	VIFx	27.0	160.0	st1
master suite-B	c 2440	123	127	0.100	7	16x3	VIFx	23.0	170.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Cig (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	708	813	0.100	813	13	16 x 9	RectFbg	
st2	Peak AVF	1019	899	0.100	764	15	16 x 12	RectFbg	

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Cig (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1850	1850	123.0	0.050	387	28	16x 43		VIFx	rt1

Return Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Cig (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
rt1	Peak AVF	1850	1850	0.050	423	27	18 x 35	RectFbg	

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Entergy	wood-edge steel door in wood frame	00-0720.05
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER	Entergy	double-wood edge steel door/wd.frame	01-0314.29
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	Capitol	650 FIN Aluminum	42963.03-122-4
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	Elk	Prestique 30 (H)	01-0919.11
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS	Simpson	H16, H2.5, H10	FL1423+FL4
B. WOOD ANCHORS	"	"	"
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.


APPLICANT SIGNATURE

1-4-07
DATE



FOUNDED 1949

CORPORATE HEADQUARTERS:

P.O. BOX 5369
116 N.W. 16TH AVENUE
GAINESVILLE, FL 32602-5369

(352) 376-2661
FAX (352) 376-2791

SCIENTIFIC PEST CONTROL DIRECTED BY GRADUATE ENTOMOLOGISTS

Complete Pest Control Service
Member Florida & National Pest Control Associations

RONALD CLARK
15816 NW CR 1491
ALACHUA FL 32615-3153

Reply to: 116 NW 16th Ave
Gainesville, FL 32601
Phone (352) 376-2661 Fax (352) 376-2791

TERMITE TREATMENT CERTIFICATION

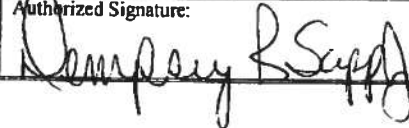
Owner: M/M FINLEY	Permit Number: 25444
Lot:	Block:
Subdivision:	Street Address: 194 SW OTTER LN
City: HIGH SPRINGS	County: COLUMBIA
General Contractor: RONALD CLARK	Area Treated: EXTERIOR PERIMETER
Date: 6-28-07	Time: AM
Name of applicator DAVID REED	Applicator ID Number: JE149226
Product Used: Active Ingredient: % Concentration Premise: Imidacloprid 0.1%	Number of gallons used: 62
Method of termite prevention treatment: Soil Treatment	

The building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws established by the Florida Department of Agriculture and Consumer Services.

This form is proof of complete treatment for Certificate of Occupancy or Closing.

THIS IS PROOF OF WARRANTY

Warranty and Treatment Certifications Have Been Issued.

Authorized Signature: 	Date: 6-28-07
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BRANCHES:

• Crystal River • Daytona Beach • Ft. Walton Beach • Jacksonville South • Jacksonville West • Lake City • Milton • Ocala • Orlando • Palatka • Panama City • Pensacola • Starke • St. Augustine • Tallahassee • Winter Haven • Leesburg • Kissimmee •



FOUNDED 1949

CORPORATE HEADQUARTERS:

P.O. BOX 5369
116 N.W. 16TH AVENUE
GAINESVILLE, FL 32602-5369

(352) 376-2661
FAX (352) 376-2791

SCIENTIFIC PEST CONTROL DIRECTED BY GRADUATE ENTOMOLOGISTS

Complete Pest Control Service
Member Florida & National Pest Control Associations

RONALD CLARK
15816 NW CR 1491
ALACHUA FL 32615-3153

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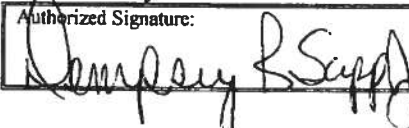
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Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: _____

City _____ Phone _____

Site Location: Subdivision _____

Lot # _____ Block# _____ Permit # 25444

Address _____

<u>Product used</u>	<u>Active Ingredient</u>	<u>% Concentration</u>
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☐ Dursban TC	Chlorpyrifos	0.5%
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☐ Termidor	Fipronil	0.06%
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☐ Bora Care	Disodium Octaborate Tetrahydrate	23.0%
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Type treatment:

☐ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

_____ Date

_____ Time

_____ Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

6/04

©

COLUMBIA COUNTY OFFICE OF CIVIL ENGINEERING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 19-7S-17-10024-056

Building permit No. 000025444

Use Classification SFD, UTILITY

Fire: 0.00

Permit Holder RONALD CLARK

Waste:

Owner of Building RAY & DARLENE FINDLEY

Total: 0.00

Location: 194 SW OTTER LANE(SASSAFRAS ACRES, LOT 58)

Date: 06/28/2007

Shary Dicks

Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)