

ABBREVIATIONS

STRUCTURAL ABBREVIATIONS

ABOVE FINISHED FLOOR	- A.F.F.	KIP PER SQUARE FOOT	- KSF
ADDITIONAL	- ADDN.L.	KIP PER SQUARE INCH	- KSI
ADJACENT	- ADJ.		
AIR CONDITIONER	- A/C	LENGTH	- L
AIR HANDLING UNIT	- AHU	LIGHTWEIGHT CONCRETE	- L.W.C.
ALTERNATE	- ALT.	LIVE LOAD	- L.L.
AMERICAN CONCRETE INSTITUTE	- A.C.I.	LONGITUDINAL	- LONG.
AMERICAN INSTITUTE OF STEEL	- A.I.S.C.	LONG LEG HORIZONTAL	- LLH
ANCHOR BOLT	- A.B.	LONG LEG VERTICAL	- LLV
AND	- &	LONG SIDE HORIZONTAL	- LSH
ANGLE	- L	LONG SIDE VERTICAL	- LSV
APPROXIMATE	- APPROX.		
ARCHITECT	- ARCH.	MANUFACTURE(R)	- MFR.
ARCHITECTURAL	- ARCH'L	MAXIMUM	- MAX.
ARCHITECTURALLY EXPOSED	- A.E.S.S.	MECHANICAL	- MECH.
STRUCTURAL STEEL		METAL	- MTL.
ARCHITECTURALLY EXPOSED	- A.E.C.	MEZZANINE	- MEZZ.
CONCRETE		MINIMUM	- MIN.
		MISCELLANEOUS	- MISC.
AT	- @	MOMENT	- M
		MOMENT CONNECTION	- M.C.
BASE PLATE	- B.P.		
BEAM	- BM.		
BEARING	- BRG.	NON-SHRINK	- N.S.
BETWEEN	- BTWN.	NORMAL WEIGHT CONCRETE	- N.W.C.
BLOCKING	- BLKG.	NOT TO SCALE	- N.T.S.
BOTTOM	- BOT.	NUMBER	- NO. or #
BOTTOM OF	- B.O.		
BOTTOM OF STEEL	- B.O.S.	ON CENTER	- O.C.
BRIDGING	- BRDG.	OPENING(S)	- OPNG(S)
BUILDING	- BLDG.	OPPOSITE	- OPP.
		OPPOSITE HAND	- O.H.
CAMBER	- C	OUTSIDE FACE	- O.F.
CAST-IN-PLACE	- C.I.P.	OUTSIDE DIAMETER	- O.D.
CEILING	- CLG.		
CENTER LINE	- C.L. or C	PENETRATION	- PEN.
CLEAR OR CLEARANCE	- CLR.	PERIMETER	- PERIM.
COLD FORMED STEEL	- CFS	PERPENDICULAR	- PERP.
COLUMN	- COL.	PLATE	- PL. or P
COMPRESSION	- C or COMP.	POINT	- PT.
CONCRETE	- CONC.	POST-TENSION(ED)	- P-T
CONCRETE MASONRY UNIT	- CMU	POUNDS	- # or LBS.
CONNECTION(S)	- CONN(S)	POUNDS PER CUBIC FOOT	- PCF
CONSTRUCTION JOINT	- C.J.	POUNDS PER LINEAR FOOT	- PLF
CONTINUOUS	- CONT.	POUNDS PER SQUARE FOOT	- PSF
COORDINATE	- COORD.	POUNDS PER SQUARE INCH	- PSI
		PREFRCAST CONCRETE	- PFC
		PRE-ENGINEERED METAL	- P.E.M.B.
DETAIL	- DTL.	BUILDING	
DEAD LOAD	- D.L.	PREFABRICATED	- PREFAB.
DEFORMED BAR ANCHOR	- D.B.A.	PROJECTION	- PROJ.
DIAGONAL	- DIA.		
DIAMETER	- DIA. or Ø		
DIMENSION(S)	- DIM(S)	RADIUS	- R
DOUBLE	- DBL.	REINFORCE(ING)(ED)(MENT)	- REINF.
DOUBLE EXTRA STRONG	- DX-STR	REQUIRED	- REQ'D.
DOVEL(S)	- DWL(S)	R.O.F. DRAIN	- R.D.
DRAWING(S)	- DWG(S)	ROOF LIVE LOAD	- R.L.L.
		ROOF TOP UNIT	- R.T.U.
EACH	- EA.		
EACH FACE	- E.F.	SAWN JOINT	- S.J.
EACH WAY	- E.W.	SCHEDULE(D)	- SCHED.
ELEVATION	- EL.	SHEAR	- V
ELEVATOR	- ELEV.	SHEET	- SH.T.
EMBEDMENT	- EMBED.	SIMILAR	- SIM.
ENGINEER	- ENGR.	SLAB ON GRADE	- S.O.G.
EQUAL OR EQUALLY	- EQ.	SNOW LOAD	- S.L.
EQUIPMENT	- EQUIP.	SQUARE	- SQ.
EXISTING	- EXIST.	SQUARE FOOT	- S.F.
EXPANSION	- EXP.	STANDARD	- STD.
EXPANSION JOINT	- E.J.	STEEL	- STL.
EXTERIOR	- EXT.	STEEL JOIST INSTITUTE	- S.J.I.
EXTRA STRONG	- X-STR	STIFFENER	- S.T.
		STRUCTURAL	- STRUCT'L
		SUPPORT(S)	- SUPT(S)
FAR SIDE	- F.S.		
FIELD VERIFY	- F.V.	TEMPERATURE	- TEMP.
FINISHED FLOOR	- FIN. FL.	TENSION	- T
FIXED CONNECTION	- F.C.	THICK	- THK.
FLOOR	- FL.	TONGUE AND GROOVE	- T&G
FLOOR DRAIN	- F.D.	TOP AND BOTTOM	- T&B
FOOTING	- FTG.	TOP OF	- T.O.
FOUNDATION	- FDN.	TOP OF BEAM	- T.O.B.
		TOP OF CONCRETE	- T.O.C.
GAGE OR GAUGE	- GA.	TOP OF FOOTING	- T.O.F.
GALVANIZED	- GALV.	TOP OF JOIST	- T.O.J.
GENERAL	- GEN.	TOP OF PIER	- T.O.P.
GLULAM	- GL.	TOP OF PIER (PILE) CAP	- T.O.P.C.
GRADE	- GR.	TOP OF STEEL	- T.O.S.
GYPSUM BOARD	- GYP. BD.	TOP OF WALL	- T.O.W.
		TYPICAL	- TYP.
HEADED STUD ANCHOR	- H.S.A. or H.S.		
HEIGHT	- HT.	UNLESS NOTED OTHERWISE	- U.N.O.
INFORMATION	- INFO.		
INSIDE DIAMETER	- I.D.	VERTICAL	- VERT.
INSIDE FACE	- I.F.		
INTERIOR	- INT.	WATERSTOP	- W.S.
INTERMEDIATE	- INTERM.	WEIGHT	- WT.
		WELDED WIRE MESH (FABRIC)	- W.W.F.
JOINT	- JT.	WIND LOAD	- W.L.
JOIST(S)	- JST(S)	WITHOUT	- W/O
		WOOD	- WD.
KIPS (1000 LBS)	- K	WORK POINT	- W.P.
KIP PER LINEAR FOOT	- KLF		

GENERAL NOTES

I. GENERAL CONDITIONS & DESIGN LOADS

1. Building Code: 7th Edition of the Florida Building Code

2. Risk Category	II
3. Roof Dead Loads	
Roofing	1.5 psf
Decking	1.8 psf
Structure	6.0 psf
Insulation	4.0 psf
Mechanical and Electrical	1.0 psf
Sprinklers	2.0 psf
Miscellaneous	1.0 psf

OCCUPANCY OR USE	UNIFORM (psf)	CONCENTRATED (pounds)
Roofs		300
All roof surfaces subject to maintenance workers		
Ordinary flat, pitched, and curved roofs (that are not occupiable)	20	

5. Wind Design Data	
Ultimate design wind speed, V _{ult}	118 mph
Nominal design wind speed, V _{nom}	95 mph
Wind Importance Factor, I _w	1.0
Wind exposure	C

Windward Wall Pressures at "z"	Combined WW + LW	
	Normal to Ridge	Parallel to Ridge
0 to 15'	19.0	16
20.0 ft	19.6	16
25.0 ft	20.2	16
30.0 ft	20.7	16
h = 35.0 ft	21.1	16.2
ridge = 35.3 ft	21.1	16.2

- General
- Consult structural engineer if locations or design weights of roof top units differ from those on plans.
- All existing conditions shall be field verified.
- Contractor shall verify all dimensions prior to start of construction.
- General Contractor shall be responsible for coordination of other trades (mech, elec, etc.) prior to fabrication and installation of materials.
- General Contractor shall be responsible for coordination of architectural and structural drawings prior to fabrication, forming, or placement of materials. General Contractor shall report discrepancies immediately to Architect and shall proceed with construction only after discrepancy has been resolved.
- The details designated as "Typical Details" (TYP) apply generally to the drawings in all areas where conditions are similar.
- Contractor shall review architectural and mechanical sheets for miscellaneous steel.
- Contractor shall review mechanical sheets for miscellaneous concrete.
- Hangers supporting piping, ceilings, light fixtures, and suspended framing shall not be attached to roof deck. Hangers shall be connected to roof joist, beams, or headers provided to span between roof framing members. [Hanger spacing shall not exceed 2 times the spacing of the roof framing members. Hanger load shall not exceed a uniform load of (10/15/20) psf.]
- The design represented by these plans provides for stability of the completed structure. It is the contractor's responsibility to maintain structure stability and safety during construction per requirements of SEI/ASCE 37-14 Design Loads on Structures During Construction.

II. FOUNDATION & EARTHWORK

- Design based on Geotechnical Report No. 458560 by Universal Engineering Services, dated March 29, 2006.
- Foundation construction shall be monitored by a representative of the Geotechnical Engineer to observe excavation, identify bearing strata, and provide recommendations if seepage is encountered.
- Depth or elevations of bottom footings shown on Structural Drawings are for estimating purposes only. Final bearing elevations shall be determined in the field by a representative of the Geotechnical Engineer to satisfy assumed design bearing values.
- Footing design loadbearing values:
 - Isolated spread footing 2,500 psf
 - Continuous footing 2,500 psf
 - Bearing stratum prepared subgrade
- Slab on grade
 - Contractor shall read and be familiar with the project geotechnical report for soil pad preparation prior to beginning earthwork.
 - A 15-mil vapor barrier (0.01 perm maximum per ASTM E 1745) shall be provided above the fine insulation. Substitutions shall be approved in writing by engineer of record.
 - Contractor shall brace wall or grade beam while placing backfill or fill material.

III. STRUCTURAL CONCRETE

- Concrete shall have the following properties:

Description of Use	Unit Weight (pcf)	Min. 28 Day Strength (psi)	Max w/c Ratio	Slump	Air Content
Plasters and foundation wall	145 (NW)	4,000	-	3'-5"	3-6%
Slab-on-Grade	145 (NW)	3,000	0.45	2'-4"	-
Spread Footings	145 (NW)	-	-	3'-6"	-
- Reinforcing steel shall be ASTM A615 Grade 60 unless noted otherwise.
- Lap reinforcing splices #6 and smaller bars - 57 bar diameters with 24" minimum unless noted otherwise. Lap reinforcing splices #7 and larger - 71 bar diameters with 24" minimum unless noted otherwise.
- Provide corner bars with 30 bar diameter lap to match horizontal reinforcement at all grade beam and wall intersections. 2'-0" lap minimum.
- All reinforcement shall be detailed in accordance with the latest ACI Detailing Manual.
- Contractor shall coordinate all penetrations, conduit, chamfers and embedded items prior to concrete placement.
- Provide concrete coverage for reinforcing steel as follows unless noted otherwise:

Location	Concrete Cover (in)
Concrete cast against and permanently exposed to earth	3
Concrete exposed to earth or weather:	
No. 6 through No. 18 bars	2
No. 5 bar and smaller	1-1/2
Concrete not exposed to weather or in contact with ground:	
No. 14 and No. 18 bars	1-1/2
No. 11 bar and smaller	3/4
Beams, columns:	
Primary reinforcement, ties, stirrups, spirals	1-1/2
- Set anchor rods to have at least 3" concrete cover and to allow reinforcement to pass outside of rods.

IV. STRUCTURAL STEEL

- Structural steel properties:

Shape	ASTM Designation	F _y (ksi)	F _u (ksi)
W shapes	A992	50	65
Channels	A36	36	58
Angles	A36	36	58
Plates & Bars	A36	36	58
HSS Rectangular	A500 Gr. C	50	62
- All bolted connections shall use ASTM F3125 Grade A325 bolts unless noted otherwise.
- All welds shall be made using E70 electrodes.
- Temporary construction bracing of the structural steel frame shall remain in place until the roof deck attachments have been completed and all permanent bracing has been installed.
- Welding shall be in accordance with "Structural Welding Code" by American Welding Society (AWS D1.1 - Latest Edition). Welding shall be performed by prequalified operators only.
- The CDSE shall submit sealed calculations for structural steel connections which are delegated for connection design. These include, but are not limited to, beam shear connections (including those with axial loads and transfer forces), column splices, moment connections, vertical and horizontal bracing connections, girder and truss splices, truss-to-column and truss-to-truss connections, and truss web-to-chord and web-to-gusset connections, column stiffeners, web doubler plates, and beam bearing stiffeners.
- All approval drawings which include connections that have been delegated to the Fabricator for design shall be reviewed by the CDSE for conformance with the CDSE's connection design. The CDSE shall submit a sealed letter to attest and confirm in writing that the approval drawings have been reviewed and properly incorporate the CDSE's sealed connection calculations. The sealed letter shall accompany all approval drawings submitted for approval and shall include an index of applicable approval drawings, including erection and shop drawing numbers, dates, and revision numbers.
- The approval drawings shall clearly identify the CDSE's connection calculation identification and connection capacity on each end of each member for each connection designed by the CDSE.
- If alternate connections to the specified conceptual connection details are desired by the Fabricator, the Fabricator shall request written authorization from the Structural Engineer of Record. If accepted, the CDSE shall provide sealed calculations indicating that the alternate connections satisfy the requirements of the Contract Documents.

VI. ROOF DECK

- Metal deck schedule:

Deck Gauge	Deck Type	Deck Depth	Sheet Width	S _y (in ² /ft)	S _x (in ² /ft)	I _y (in ⁴ /ft)	I _x (in ⁴ /ft)	Finish
22	WR	1.5"	36"	0.186	0.192	0.155	0.183	Galvanized
- Roof deck shall be continuous over three (3) spans minimum.
- Suspended ceilings, light fixtures, ducts or other utilities shall not be supported by the steel deck.
- Metal deck connection schedule:

Location	Connection at Supports	Connection at Parallel Edges	Sidelap Connection No./Span	Required Shear Capacity (plf)
All Roof	36/4	12" o.c.	5	
- Support and parallel edge connections shall be:
 - Hilti X-ENP 19 for steel beam of flange thickness greater than 1/4"
- Sidelap connections shall be Hilti SLC-01.
- Provide 18" wide 20 gage filler sheets centered on non-nesting side laps as required. Connect as per note 4 above.



VII. SHOP DRAWINGS AND SUBMITTALS

- Shop drawings shall be prepared and submitted for review to the Architect/Engineer for each structural building material as specified in the structural General Notes and the Project Specifications. See the contract specifications for submittal procedures and additional information.
- Shop drawings shall use drafting line work and lettering that is clearly legible. Shop drawings shall not contain reproductions of the contract drawing plans or details. Shop drawings shall not show materials for more than one level on the same plan.
- Shop drawings shall show clear and complete information for the fabrication (detail sheets and/or material list) and installation (erection and/or placing drawings) of materials. Erection and placing drawing shall provide every note and detail required to correctly install materials; they shall not refer to the contract documents for installation information.
- Erection and placing drawing plans shall be oriented and referenced in the same manner as plans on the contract documents.
- Shop drawings shall list the grade, class and/or strength of materials.
- Submittals received by the Architect/Engineer which are insufficient or incomplete may be rejected and returned to the Contractor without review. The Contractor shall be responsible for any delays in the construction schedule due to rejected, insufficient, or incomplete shop drawing submittals.
- Shop drawings shall be checked and initialed by the supplier prior to submittal to the Contractor.
- The Contractor shall thoroughly check, coordinate, stamp, date and initial each and every shop drawing print prior to submittal to the Architect/Engineer for review. Shop drawings, which do not meet these requirements, shall be considered incomplete and returned to the Contractor without review.
- Contractor's request to deviate from the contract documents shall be clearly identified by "clouding" on the shop drawings.
- Resubmitted shop drawings shall clearly identify items that have been corrected. Shop Drawings that are resubmitted with revisions that are not clouded shall be rejected and returned to the Contractor without review.

VIII. DEFERRED SUBMITTALS

- The following items are portions of the design that are not included in the Contract Documents. Design of these elements shall be the responsibility of Specialty Contractors and shall be designed and sealed by a registered Professional Engineer licensed in the State of Florida.
 - Structural Steel Connections

IX. SPECIAL INSPECTIONS

- The Owner shall employ one or more Special Inspectors to perform this work. The Special Inspector(s) shall keep records of all inspections and furnish interim inspection reports to the Building Official and to the Registered Design Professional in Responsible Charge. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge prior to completion of that phase of work. A Final Report of Special Inspections documenting required Special Inspection and correction of any discrepancies noted in the inspections shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge at the conclusion of the project.
- The Special Inspection program does not relieve the Contractor of responsibility to comply with the Contract Documents.

Inspection of Fabricators
Where fabrication of structural load-bearing members and assemblies is being performed on the premises of a fabricator's shop, special inspection of the fabricated items shall be required by Section 1704.2.5.

Inspection of Welding
Welding inspection shall be in compliance with AWS D1.1. The basis for welding inspector qualification shall be AWS D1.1.

Inspection of Structural Steel
Special inspection of structural steel shall be in accordance with the quality assurance inspection requirements of AISC 360.

Concrete Construction (IBC 1705.3)		
TYPE	Continuous Special Inspection	Periodic Special Inspection
1. Inspect anchors cast in concrete.	—	X
2. Verify use of required design mix.	—	X
3. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	—
4. Inspect formwork for shape, location and dimensions of the concrete member being formed.	—	X

Soils (IBC 1705.6)		
TYPE	Continuous Special Inspection	Periodic Special Inspection
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	—	X
2. Verify excavations are extended to proper depth and have reached proper material.	—	X
3. Perform classification and testing of compacted fill materials.	—	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X	—
5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.	—	X

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Project Number: 221207

Issue Date: 05/28/21

Revisions:

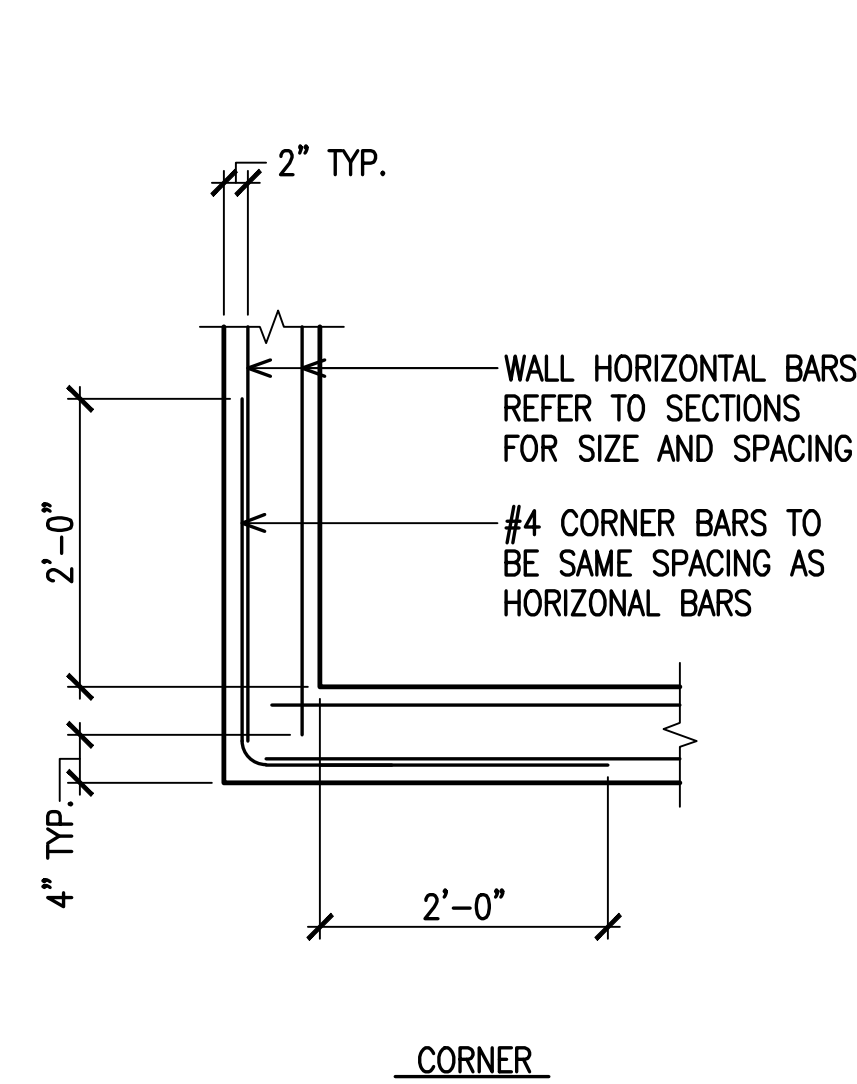
United States Cold Storage
211 MCCLOSKEY AVE
LAKE CITY, FLORIDA 32055

Project Engineer: F.N.

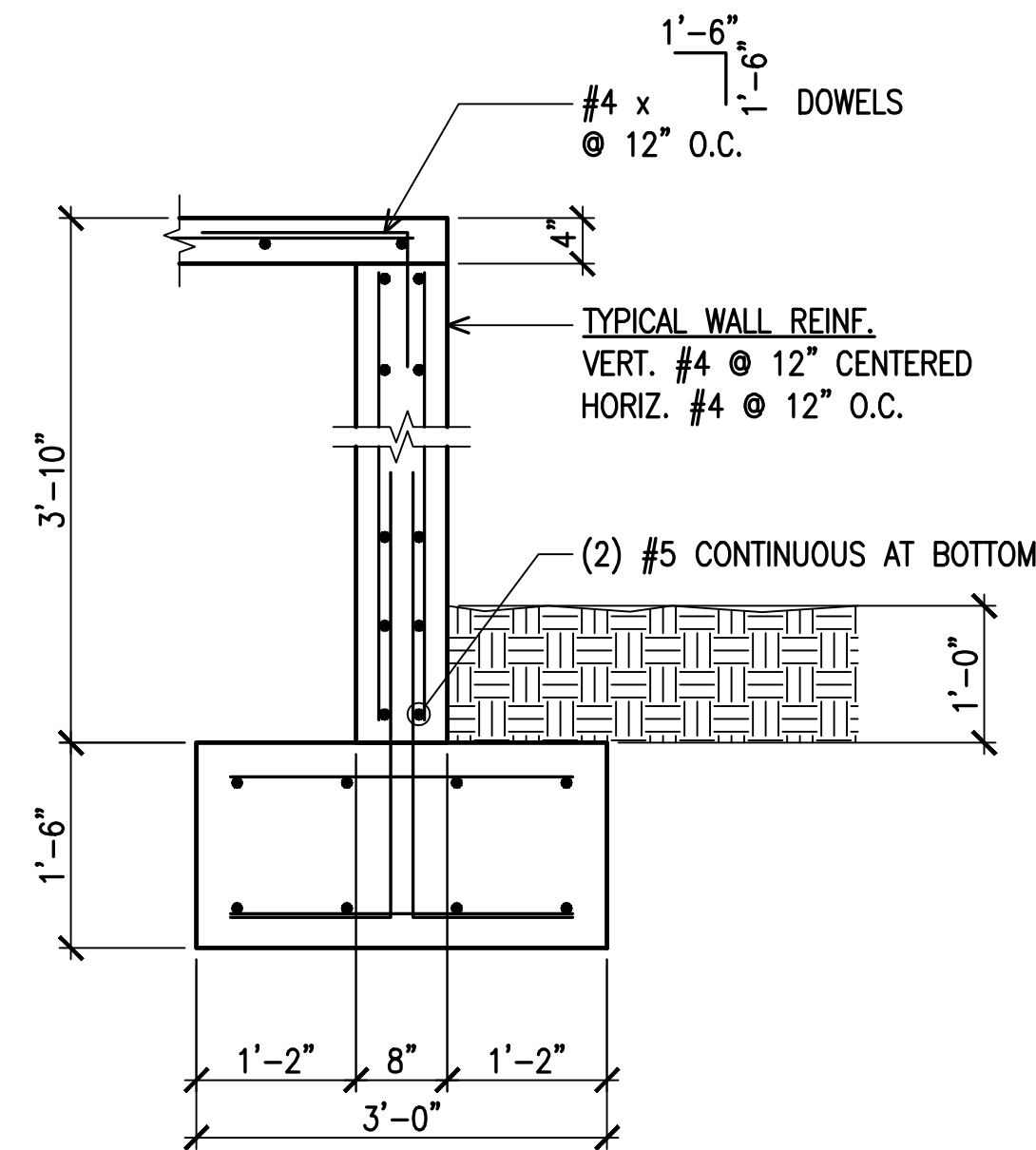
Project Draftsman: R.H.

Sheet Title
GENERAL NOTES

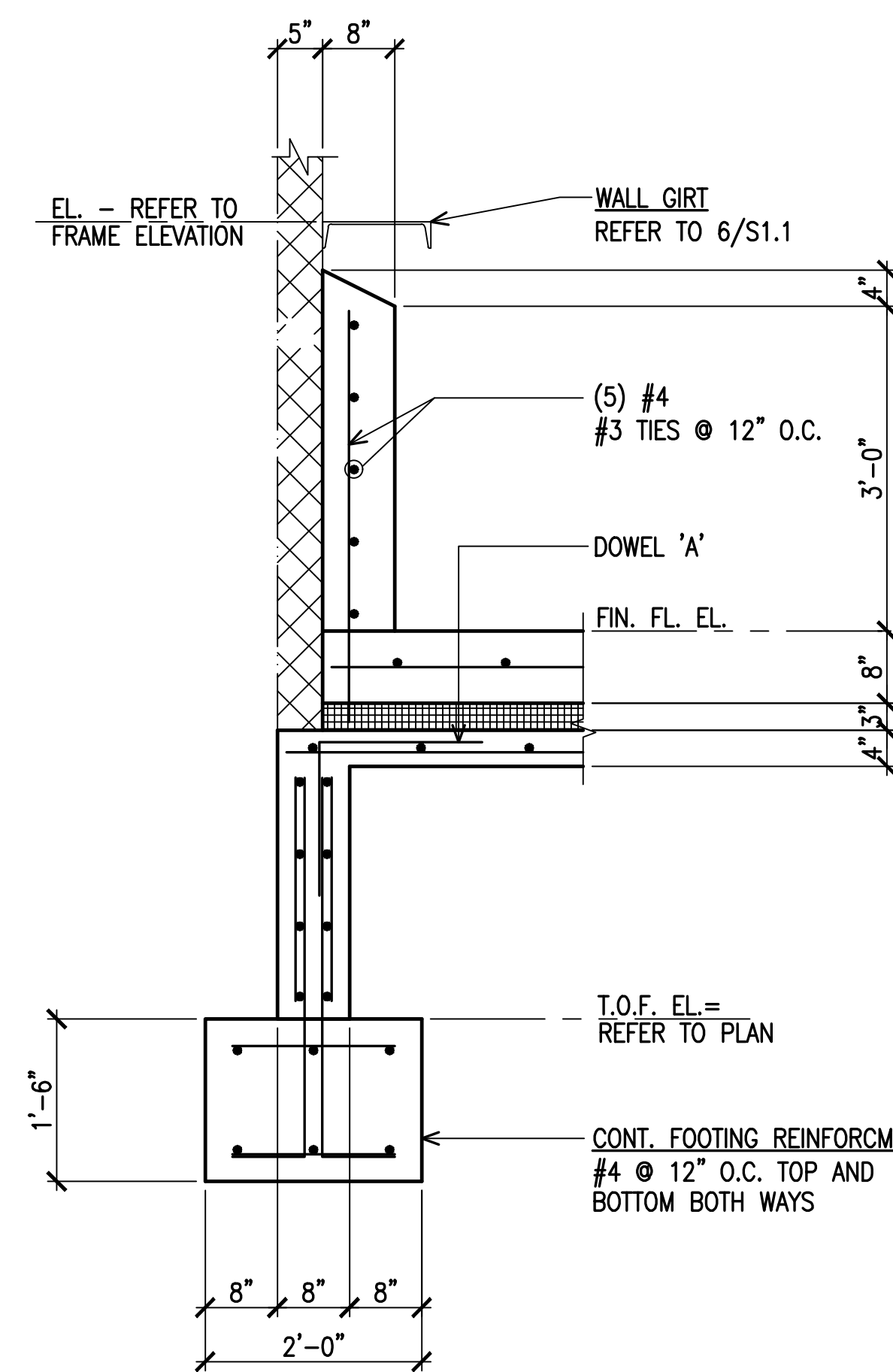
SO.1



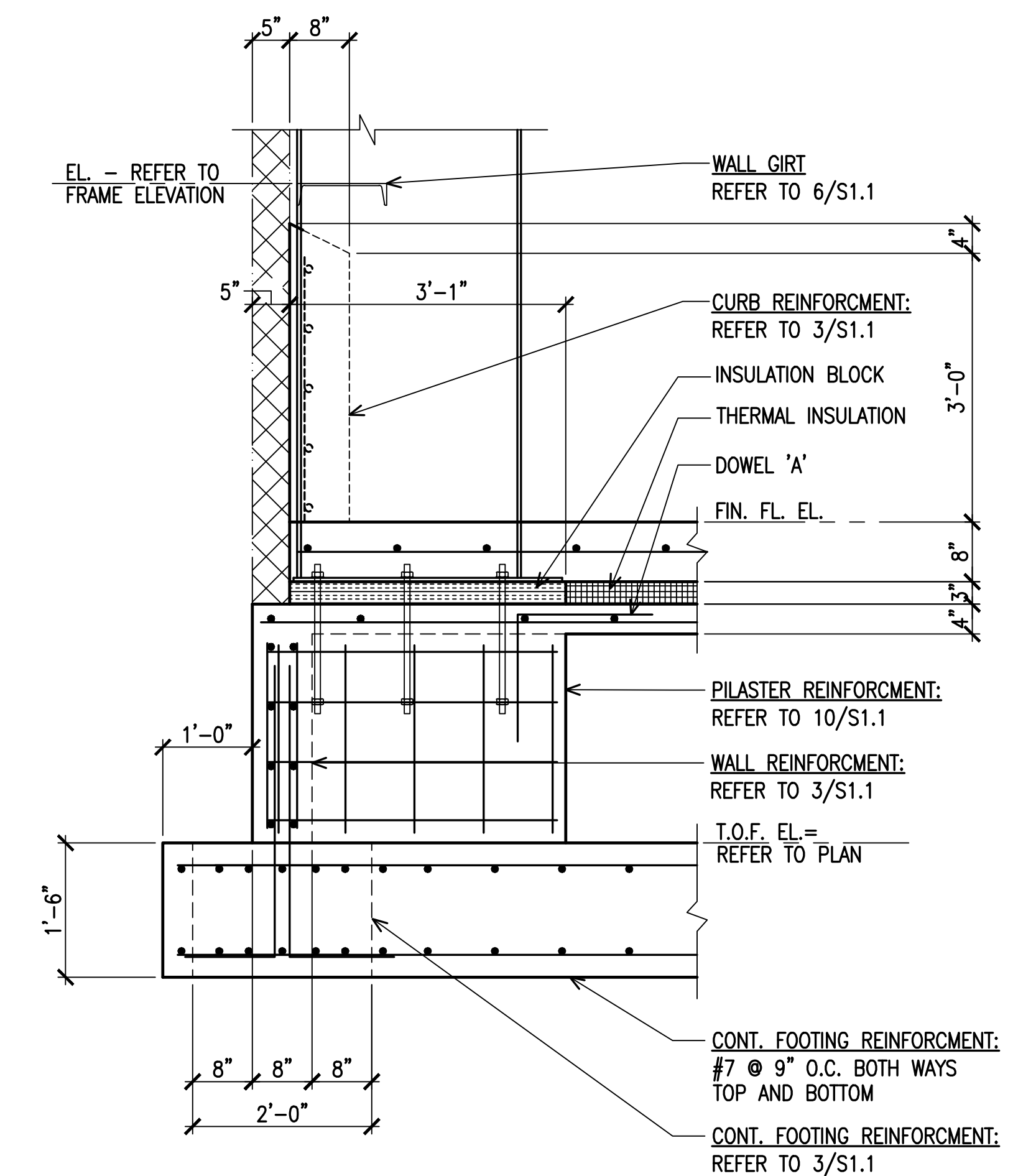
1 | **DETAIL**
3/4" = 1'-0" CONCRETE WALL REINFORCING BARS



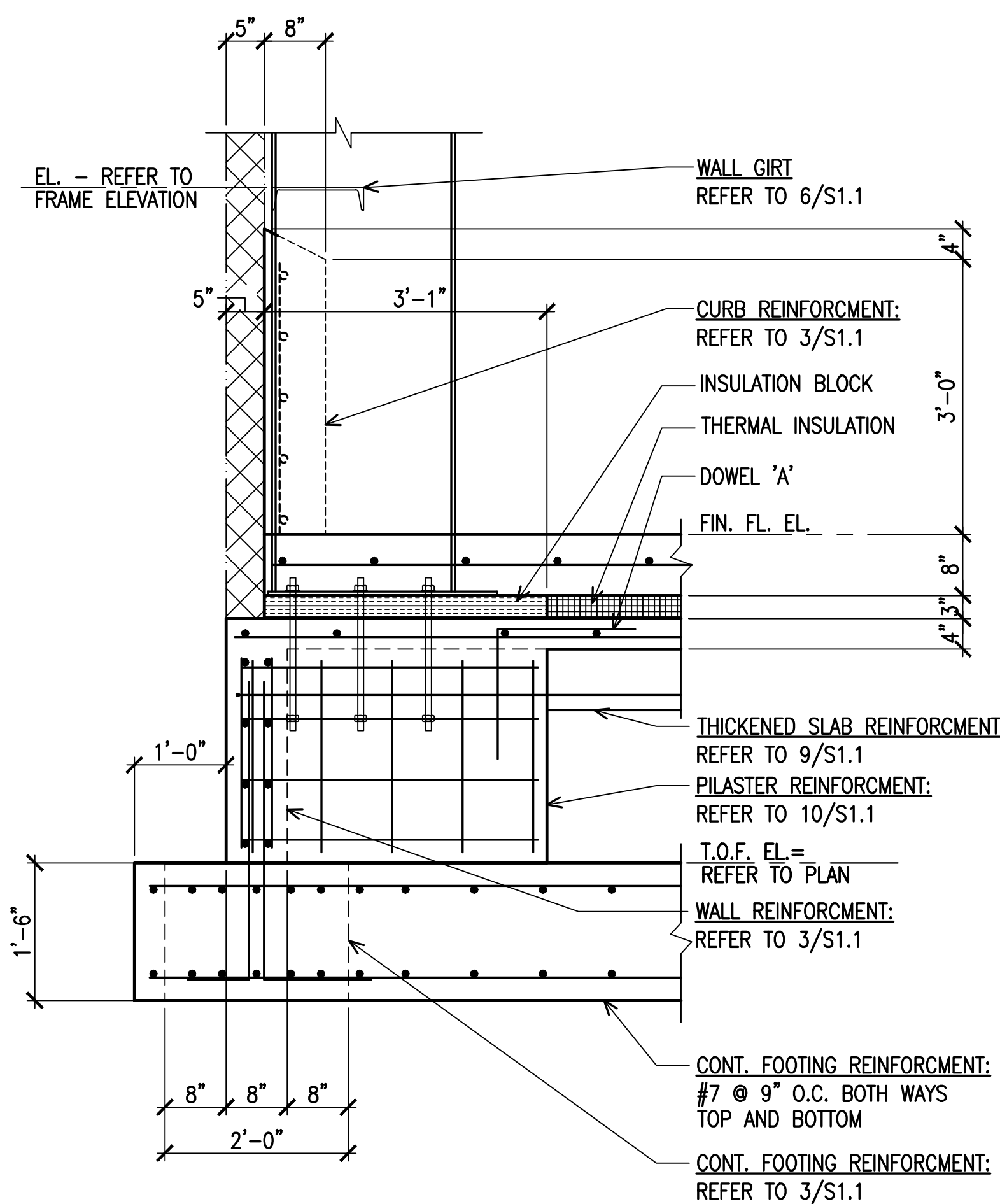
2 | **DETAIL**
3/4" = 1'-0" CONCRETE LANDING



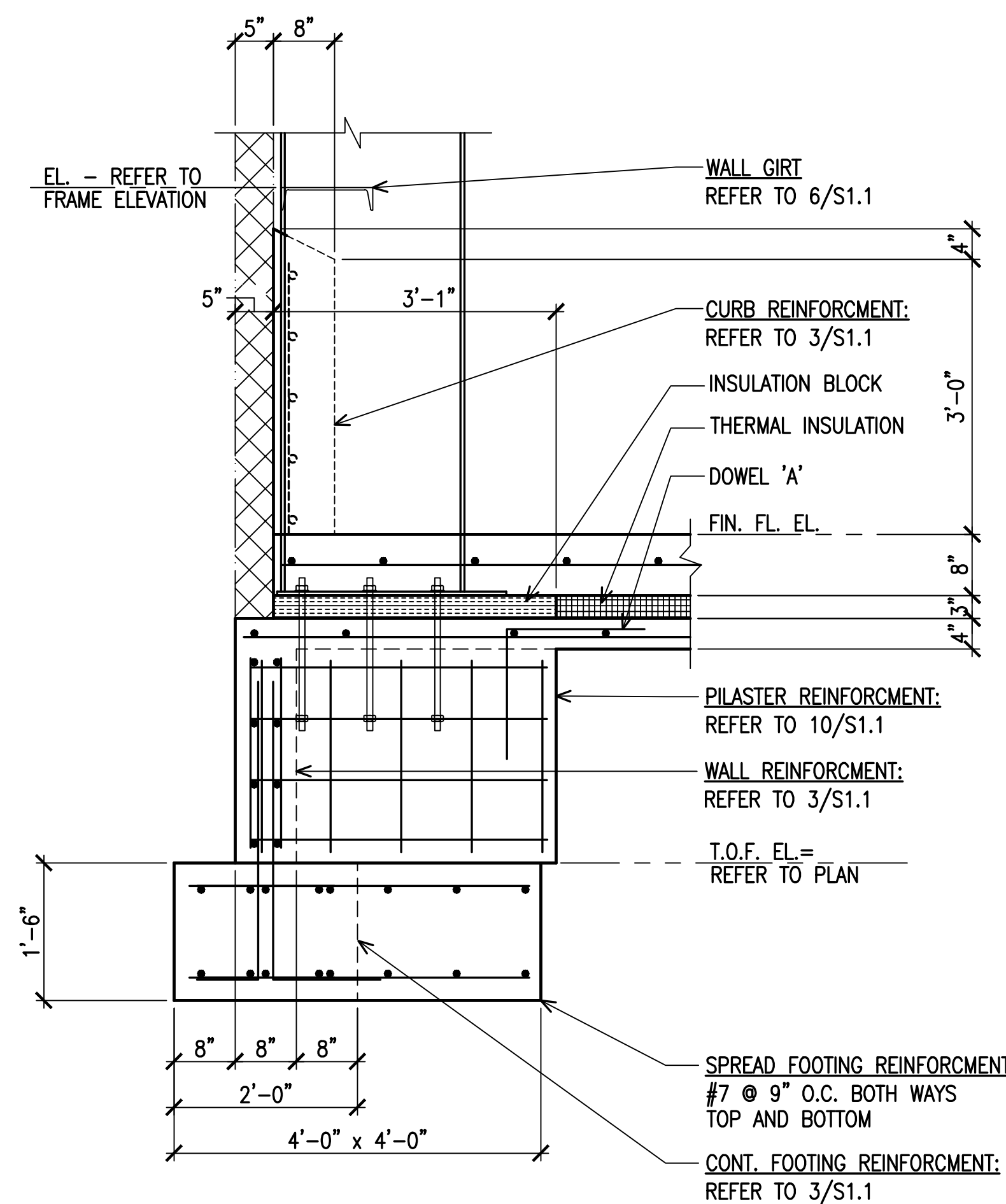
3 | **DETAIL**
3/4" = 1'-0" PERIMETER GRADE BEAM WITH LEDGE



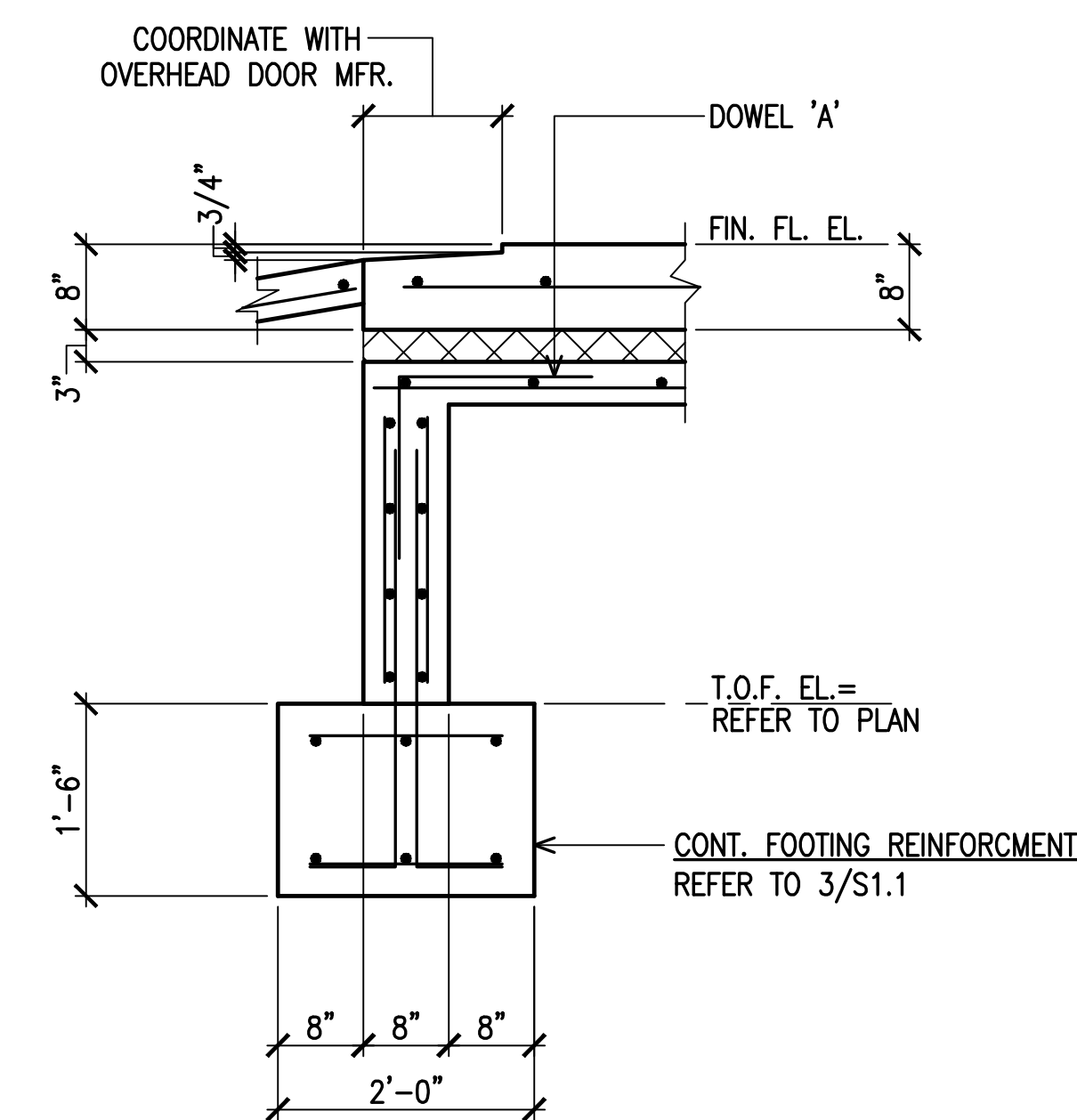
4 | **DETAIL**
3/4" = 1'-0" PERIMETER GRADE BEAM WITH LEDGE



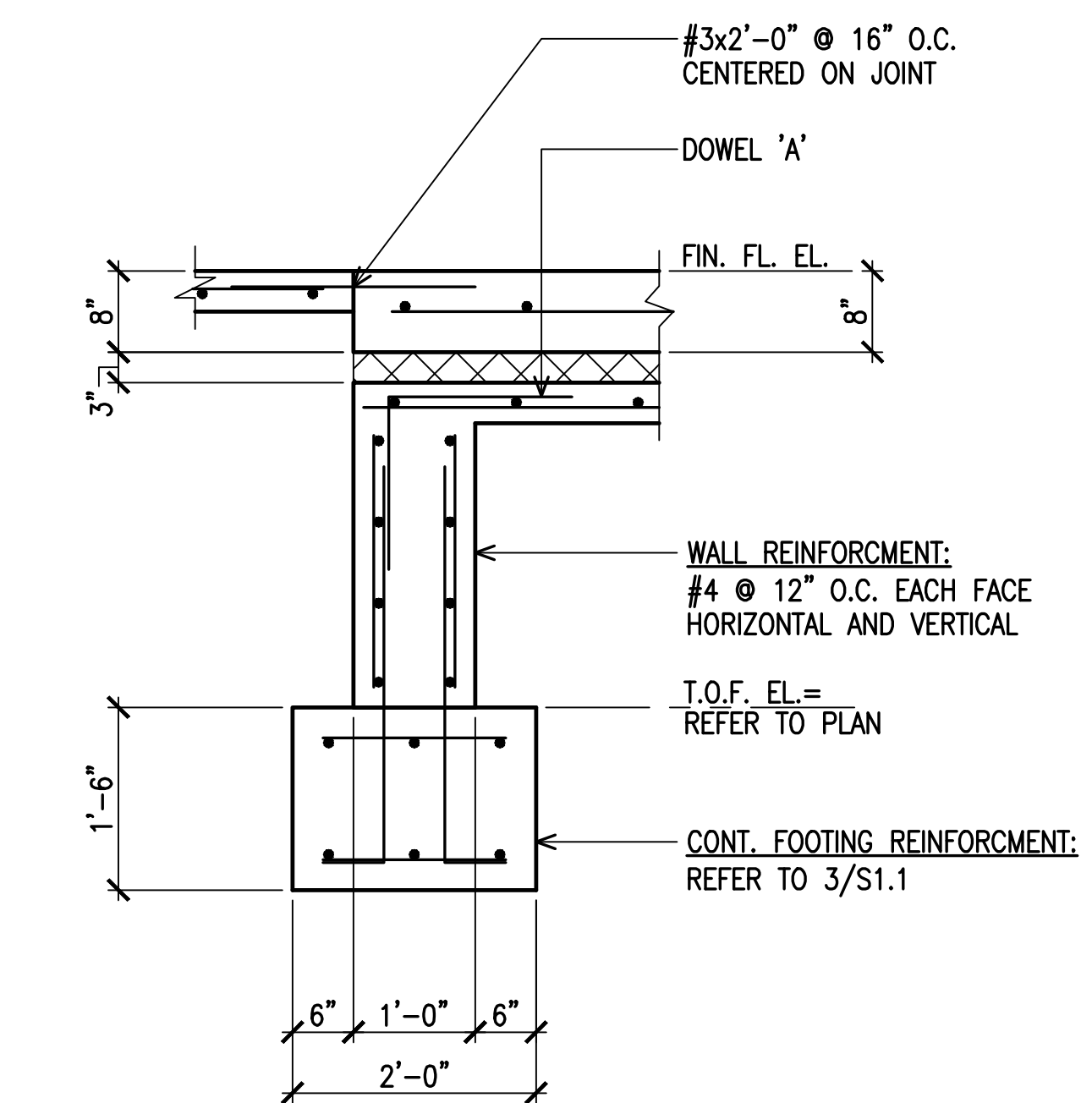
5 | **DETAIL**
3/4" = 1'-0" PERIMETER GRADE BEAM WITH
ADJACENT THICKENED SLAB



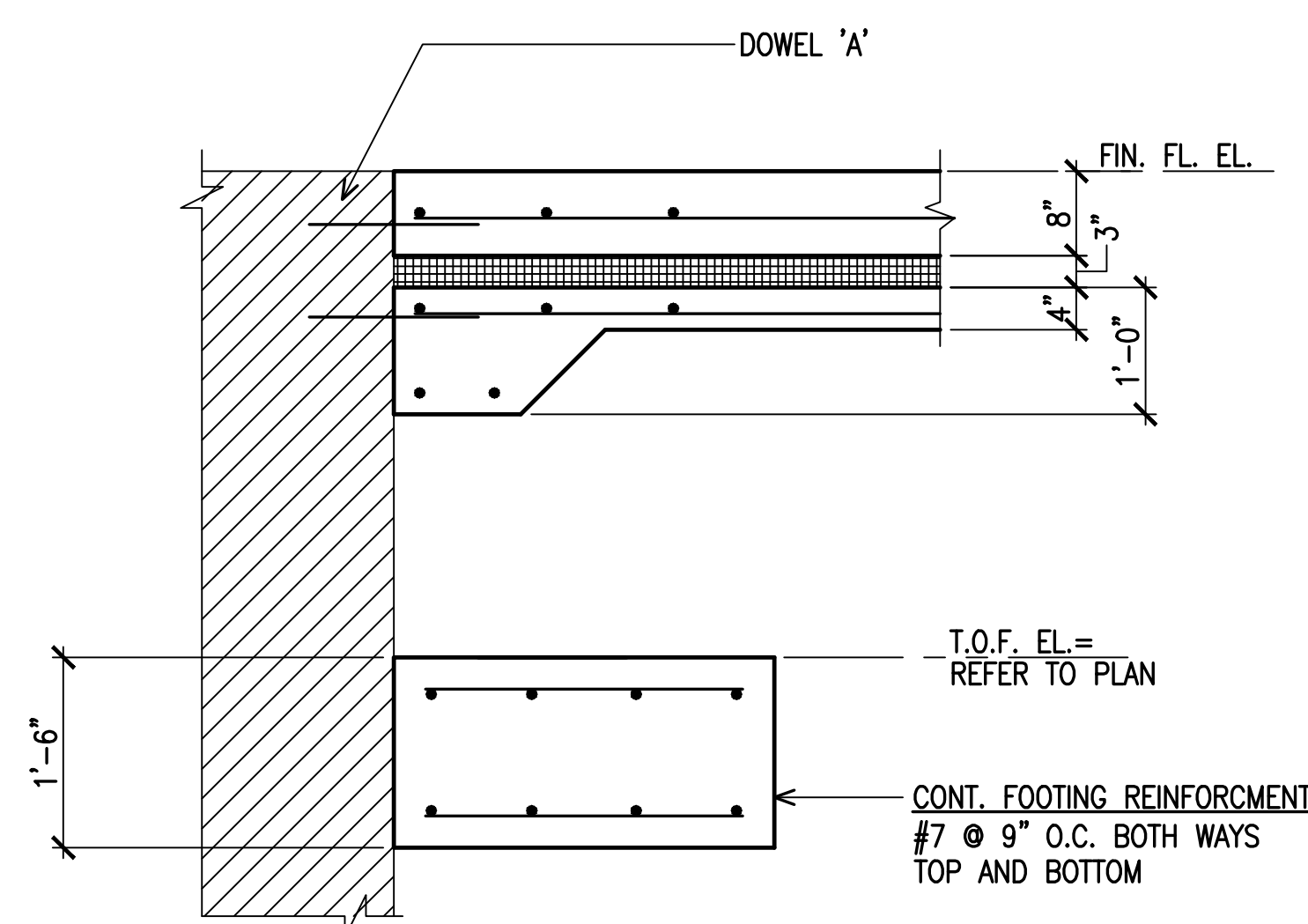
6 | **DETAIL**
3/4" = 1'-0" PERIMETER GRADE BEAM WITH LEDGE



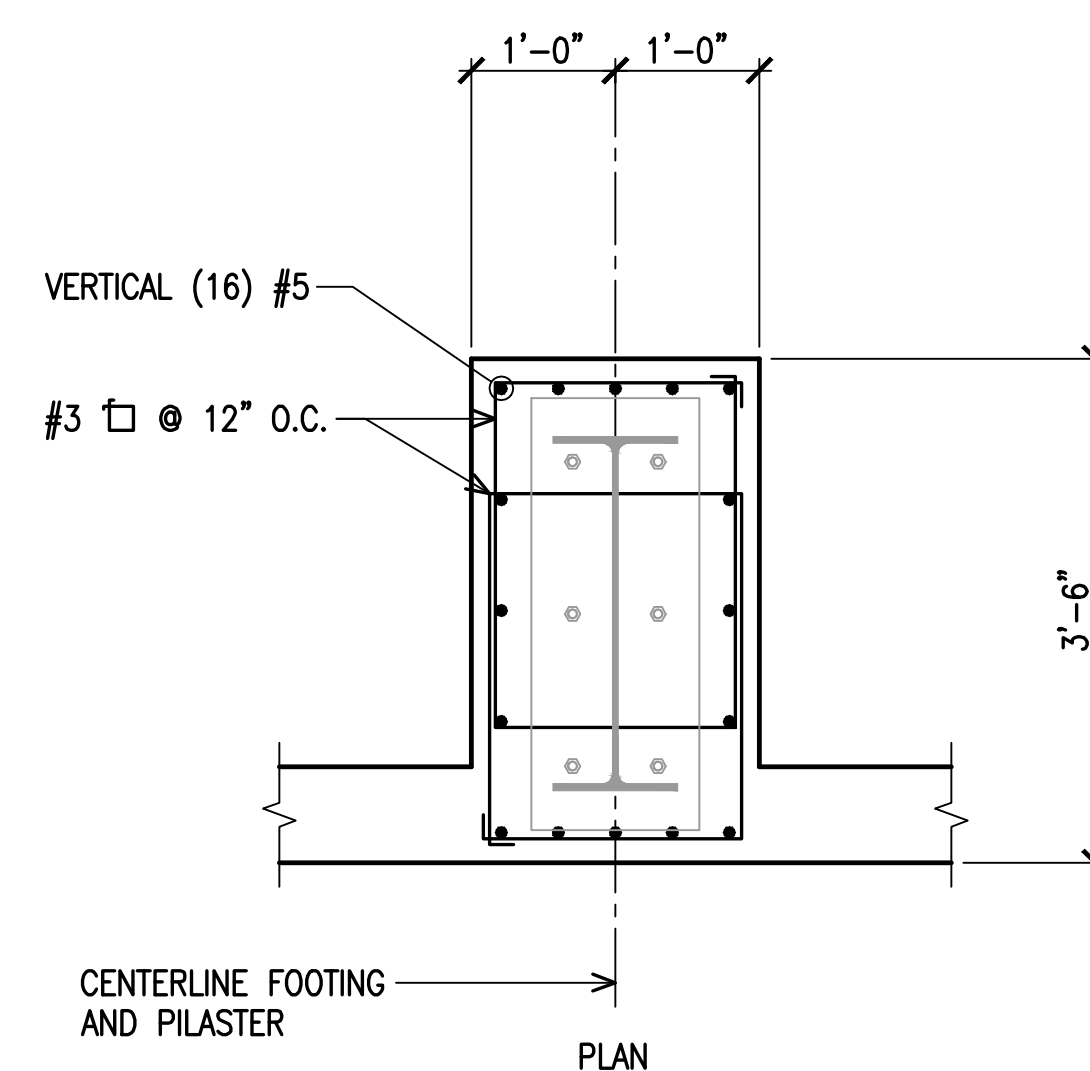
7 | **DETAIL**
3/4" = 1'-0" GRADE BEAM WITH DOOR POCKET



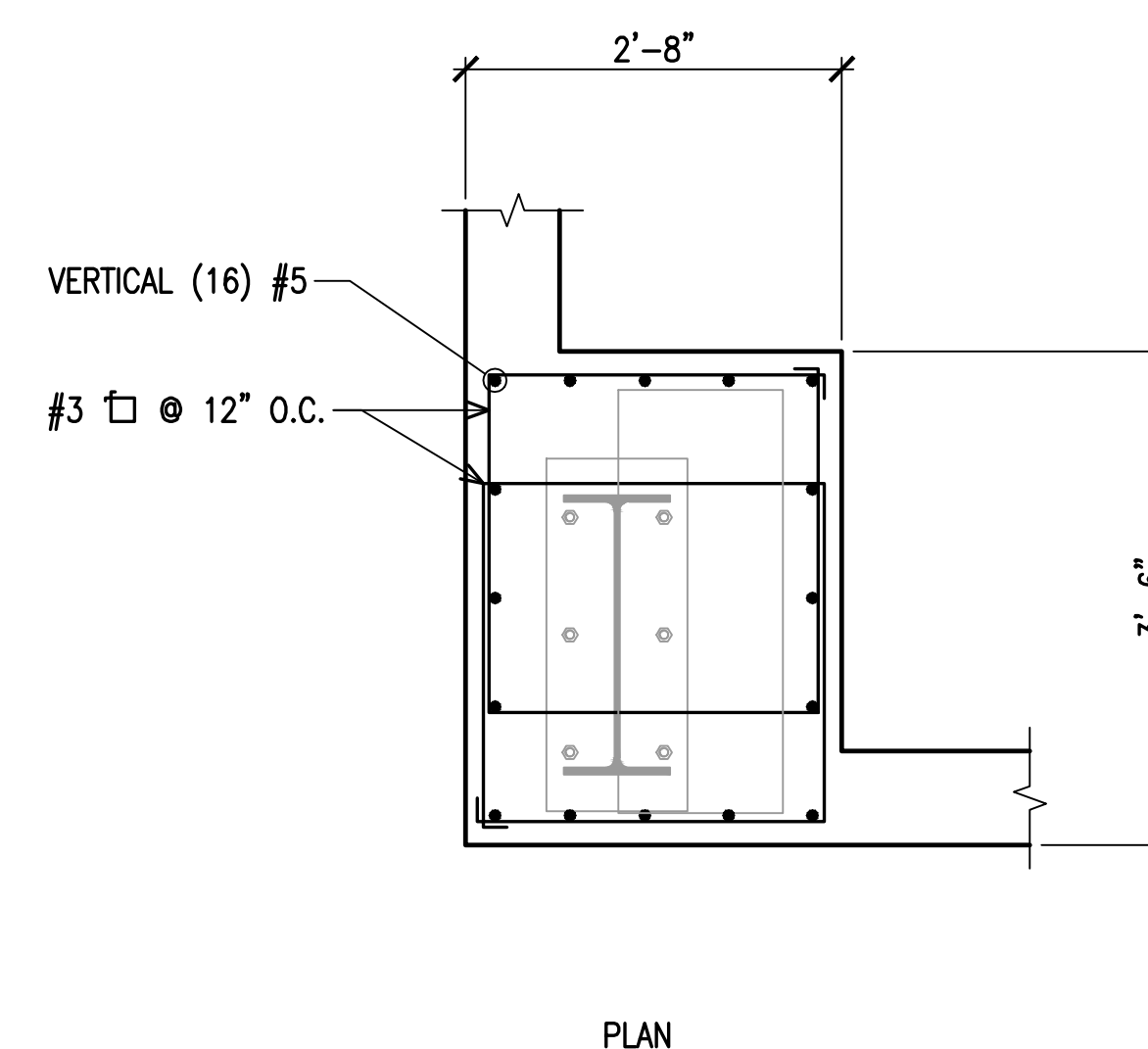
8 | **DETAIL**
3/4" = 1'-0" GRADE BEAM WITH DOOR POCKET



9 | **DETAIL**
3/4" = 1'-0" PERIMETER GRADE BEAM AT STEPS

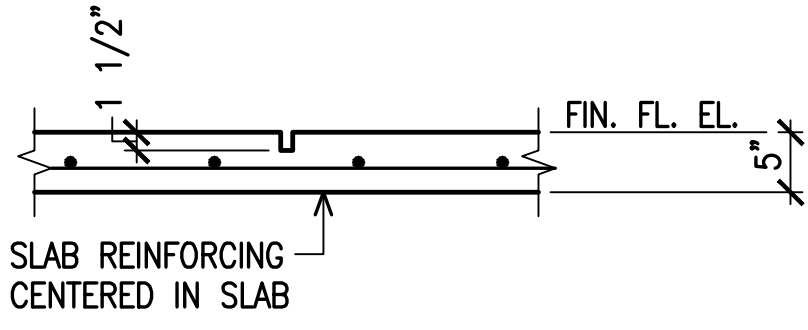


10 | **DETAIL**
3/4" = 1'-0" ADDITIONAL REINFORCING AT PILASTER

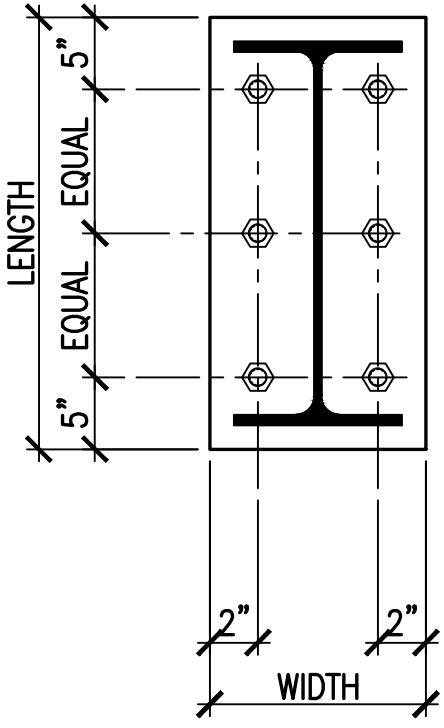


11 | **DETAIL**
3/4" = 1'-0" ADDITIONAL REINFORCING AT CORNER PILASTER

DOWEL SCHEDULE				
SCHEDULED DOWELS ARE MARKED 'DWL' ON THE SECTIONS AND DETAILS.				
		B		
		A		
MARK	SIZE	A	B	SPACING
'A'	#4	2'-0"	2'-0"	18" O.C.



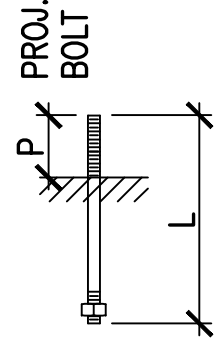
- NOTES:
1. PROVIDE JOINTS IN SLAB AT MAXIMUM SPACING OF 25 FEET O.C. EACH WAY.
 2. TYPICAL JOINT IS A SAWED JOINT. CONTRACTOR TO LOCATE CONSTRUCTION JOINTS AT HIS DISCRETION.
 3. SAWN JOINT SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT.



PLAN

BASE PLATE SCHEDULE						
MARK	DIMENSIONS		THICK.	ANCHOR RODS	COLUMN SIZE	DETAIL
	L	W				
BP-1	36"	14"	1 1/2"	(4) AR1	W30	REFER TO 2/S1.2
BP-2	30"	12"	1 1/2"	(6) AR1	W24	REFER TO 2/S1.2

ANCHOR ROD SCHEDULE					
MARK	DIA.	DIMENSIONS			MATERIAL
		L	H	P	
AB1	1"Ø	20"	-	4"	F1554, GRADE 36



1 | DETAIL
3/4" = 1'-0" | SAW JOINT (S.J.)

2 | DETAIL
NOT TO SCALE | BASE PLATE - WIDE FLANGE COLUMN



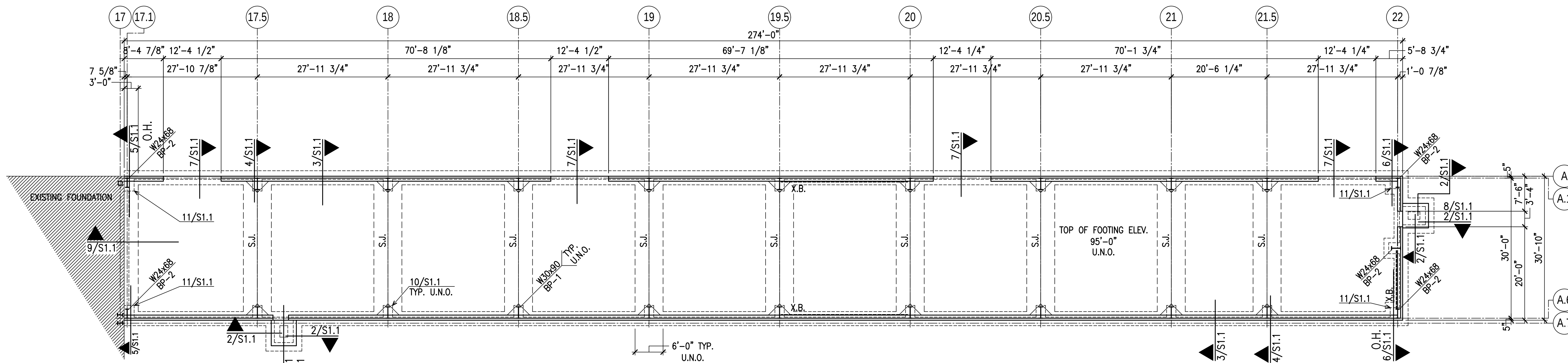
Project Number: 221207
Issue Date: 05/28/21
Revisions:

United States
Cold Storage
211 MCCLOSKEY AVE
LAKE CITY, FLORIDA 32055

Project Engineer: F.N.
Project Draftsman: R.H.

Sheet Title
Foundation Details

S1.2

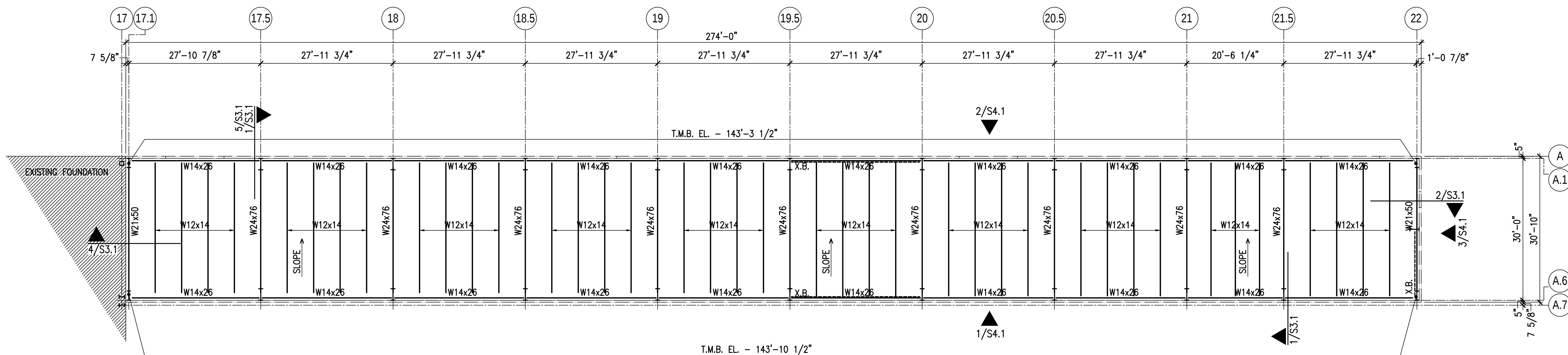


1
3/32" = 1'-0"

FOUNDATION PLAN

PLAN NOTES:

- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO BEGINNING CONSTRUCTION.
- REFER TO SHEET S0.1 FOR GENERAL NOTES.
- REFER TO SHEET S1.1 FOR DETAILS NOT NOTED ON PLAN.
- FINISH FLOOR = DATUM ELEVATION 100'-0", ACTUAL =
- COLUMNS ARE CENTERED ON GRIDLINE INTERSECTIONS UNLESS DIMENSIONED OTHERWISE ON PLAN.
- COLUMNS ARE NOTED ON PLAN THUS:
 W_x ← COLUMN SIZE (STD. PIPE U.N.O.)
 BP-1 ← BASE PLATE MARK. REFER TO BASE PLATE SCHEDULE FOR INFORMATION.
- FLOOR SLAB SHALL BE 8" THICK CONCRETE REINFORCED WITH #4 @ 12" O.C.B.W. CENTERED IN SLAB.
- PROVIDE (2) #4x3'-0" DOWELS CENTERED IN SLAB AT ALL INTERIOR CORNERS WITHOUT A CONSTRUCTION OR SAWN JOINT.
- REFER TO ARCHITECTURAL FOR LOCATION AND EXTENT OF ALL LEDGES, POCKETS OR DEPRESSIONS.
- S.J. - INDICATES SAWN JOINT ON PLAN. REFER TO 1/S1.2.



2
3/32" = 1'-0"

ROOF FRAMING PLAN

PLAN NOTES:

- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND ELEVATIONS PRIOR TO BEGINNING CONSTRUCTION.
- REFER TO SHEET S0.1 FOR GENERAL NOTES.
- REFER TO SHEET S3.1 FOR TYPICAL DETAILS NOT NOTED ON PLAN.
- REFER TO PLAN FOR TOP OF METAL DECK (T.M.D.) ELEVATIONS.
- INDICATES ALLOWABLE AREAS TO RECEIVE FUTURE ROOF TOP UNITS. ASSUMED DESIGN WEIGHT = UNITS MUST BE SUPPORTED BY A MINIMUM OF 2 JOISTS. REFER TO 7/S3.1 FOR TYPICAL MECHANICAL UNIT CURB SUPPORT DETAIL.
- STEEL JOIST BRIDGING SHALL BE PER THE STEEL JOIST INSTITUTE (SJI) SPECIFICATIONS.



MAY 28 2021

Project Number: 221207

Issue Date: 05/28/21

Revisions:

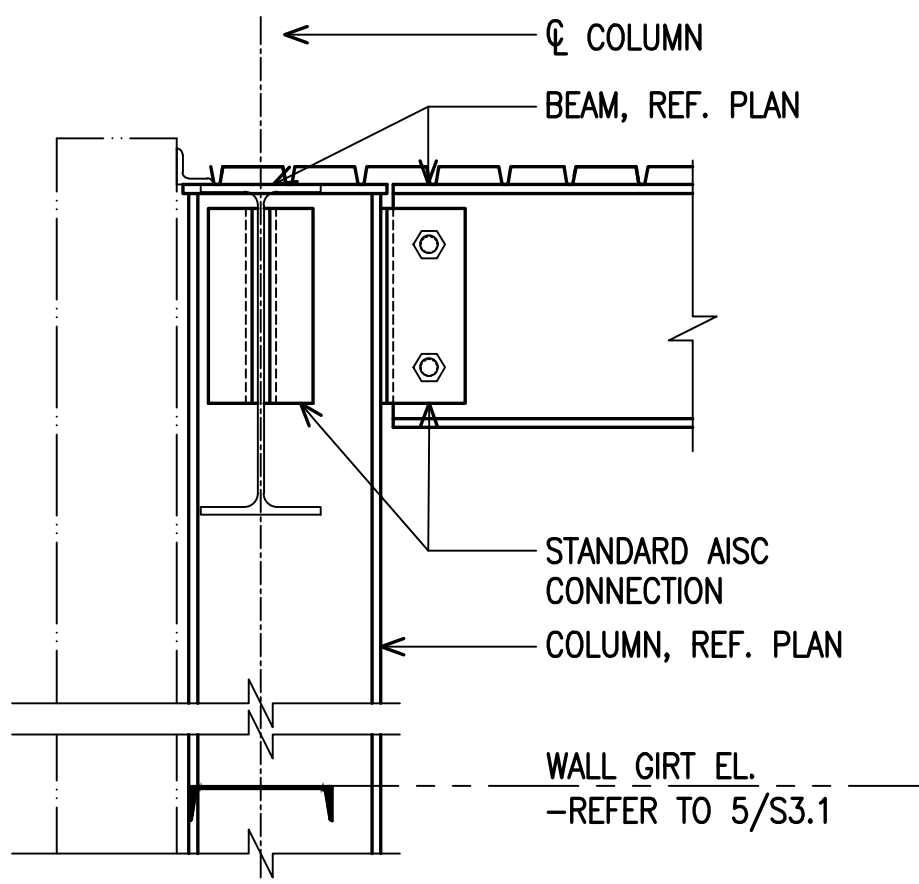
**United States
Cold Storage**
211 MCCLOSKEY AVE
LAKE CITY, FLORIDA 32055

Project Engineer: F.N.

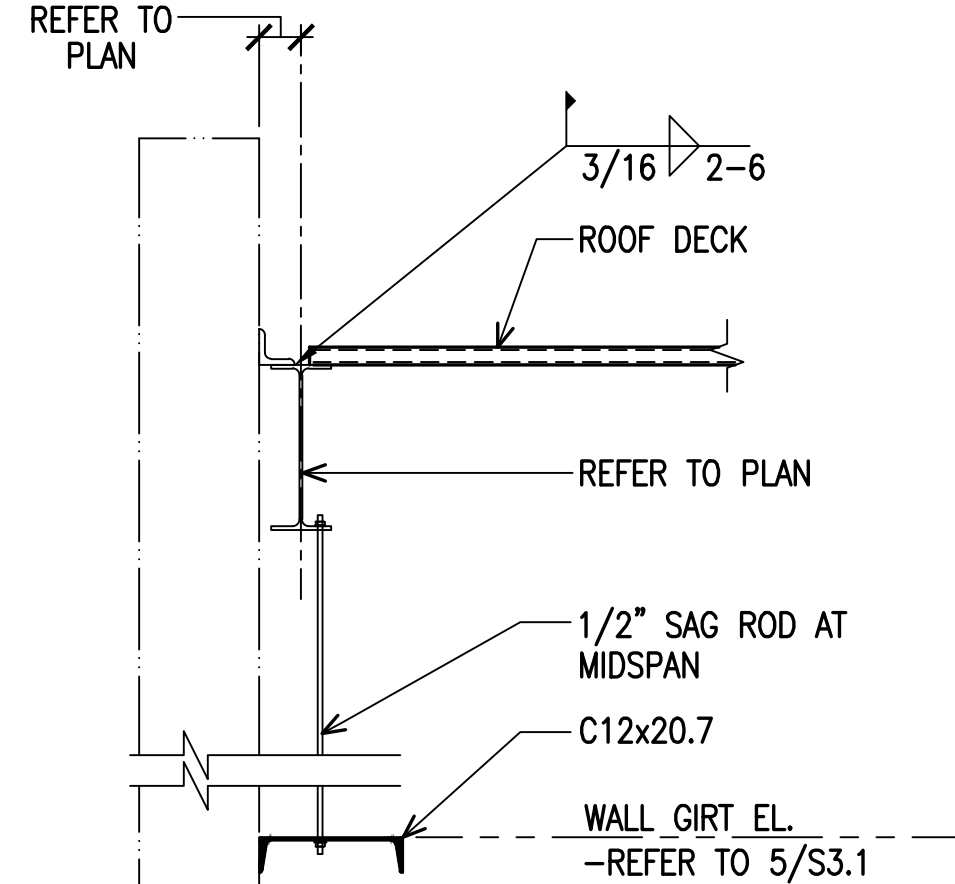
Project Draftsman: R.H.

Sheet Title
FOUNDATION AND
ROOF FRAMING
PLAN

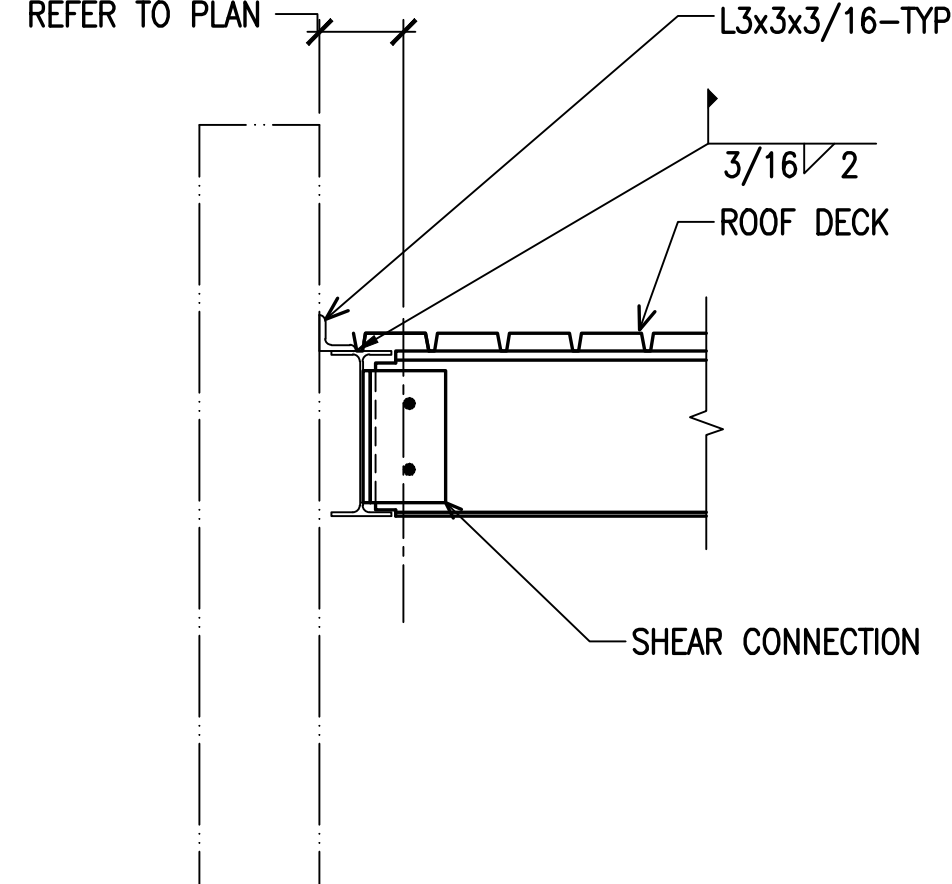
S2.1



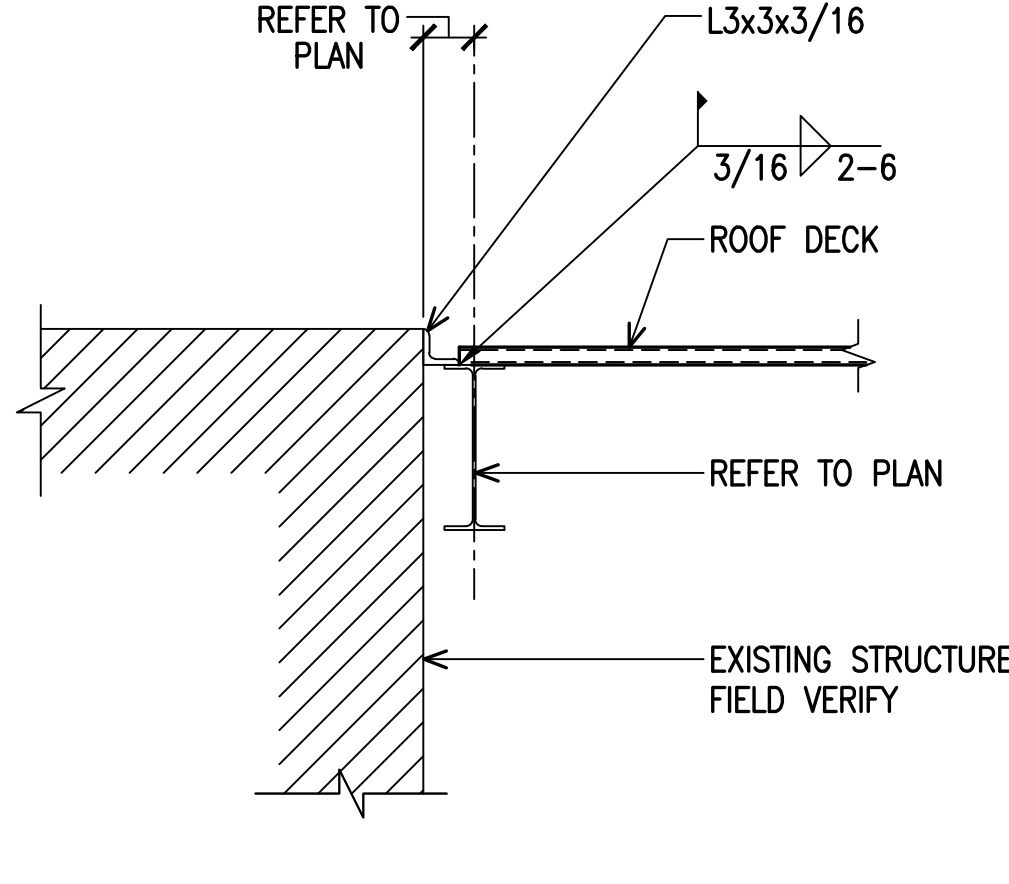
1 | DETAIL
SCALE: N.T.S. | BEAM TO COLUMN CONNECTION



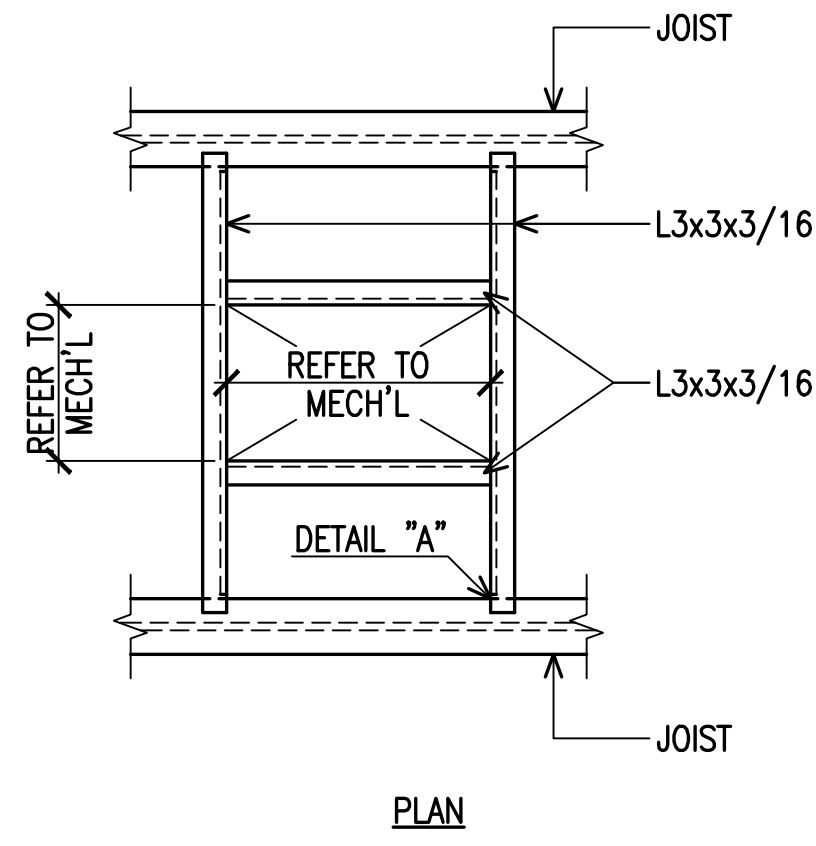
2 | DETAIL
NOT TO SCALE | DECK TO BEAM CONNECTION



3 | DETAIL
NOT TO SCALE | STEEL BEAM CONNECTION

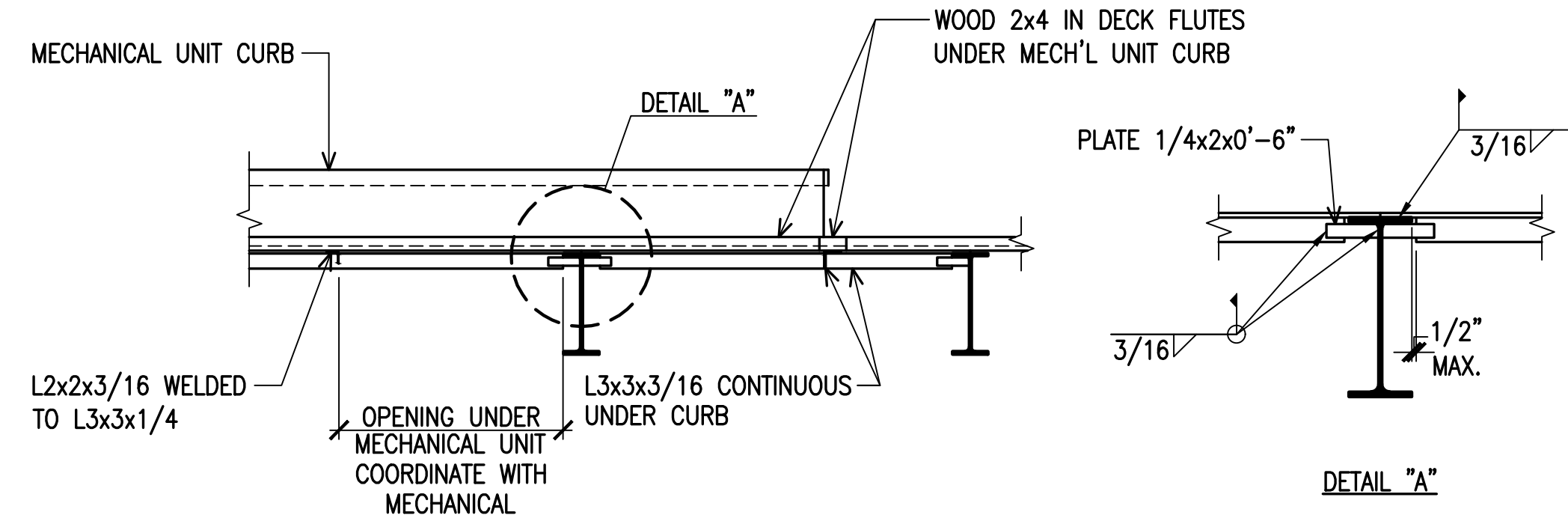
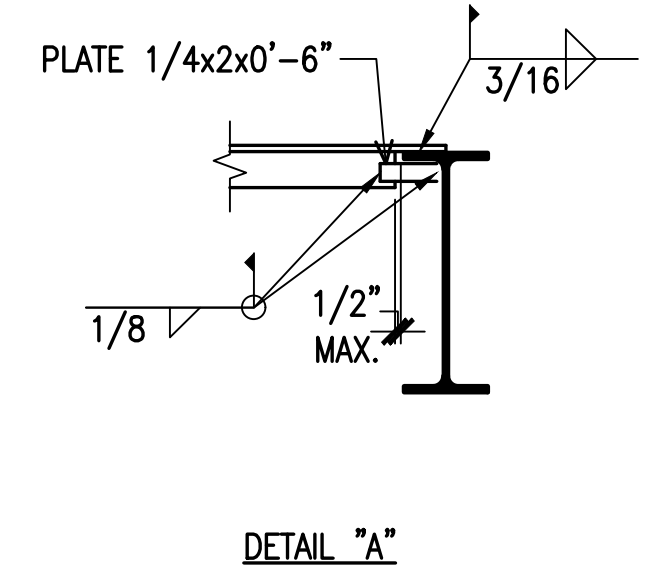


4 | DETAIL
NOT TO SCALE | DECK TO BEAM CONNECTION

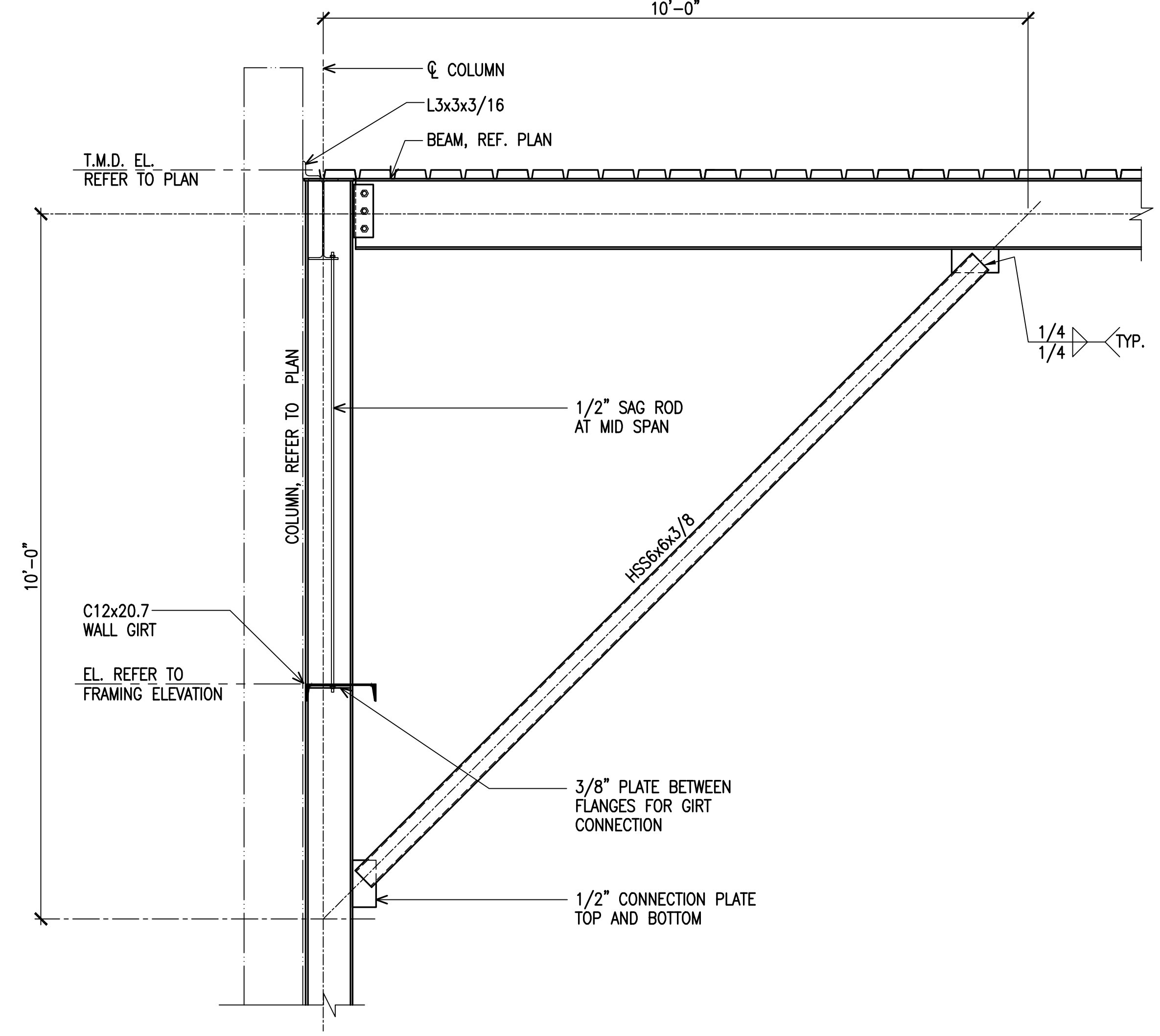


6 | DETAIL
NOT TO SCALE | FRAMING FOR ROOF OPENING

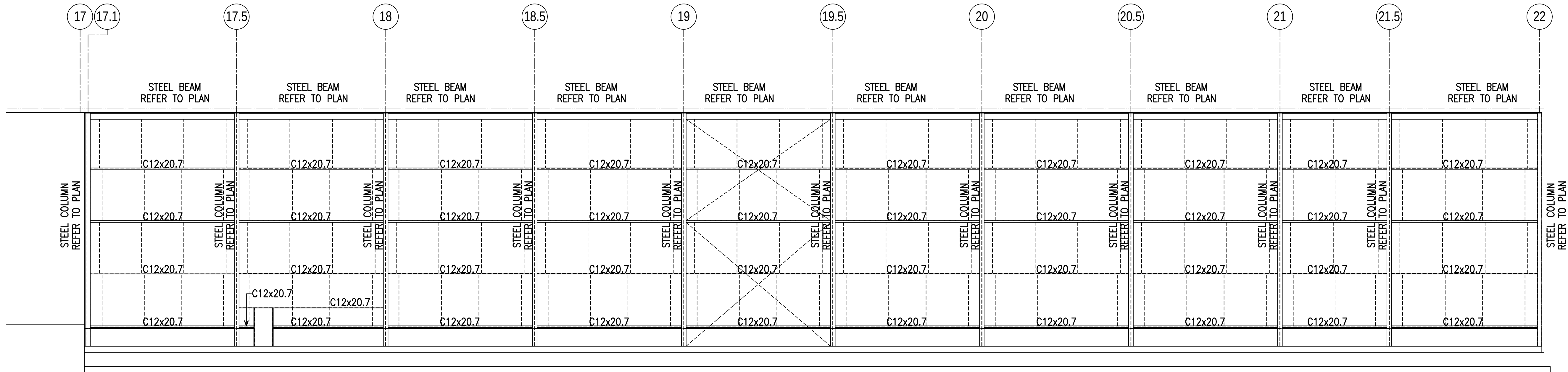
NOTE:
WHERE MECHANICAL FRAME BEARS
ON ROOF DECK, PLACE
HSS 1 1/2 x 2 1/2 x 14 GA. x 0'-6"
BLOCKING IN DECK FLUTES FOR
DIRECT BEARING ON STRUCTURE.



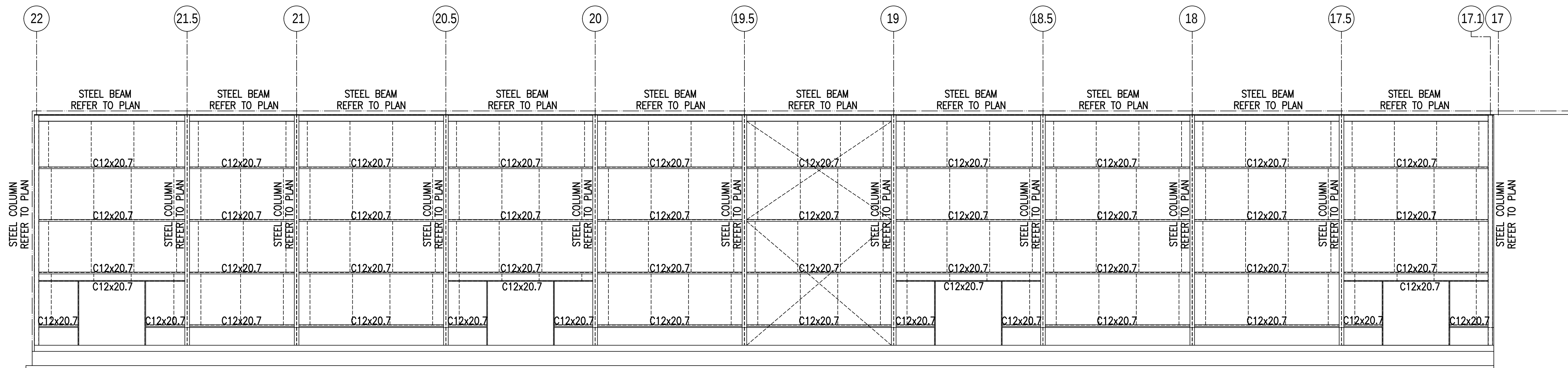
7 | DETAIL
NOT TO SCALE | MECHANICAL UNIT CURB SUPPORT



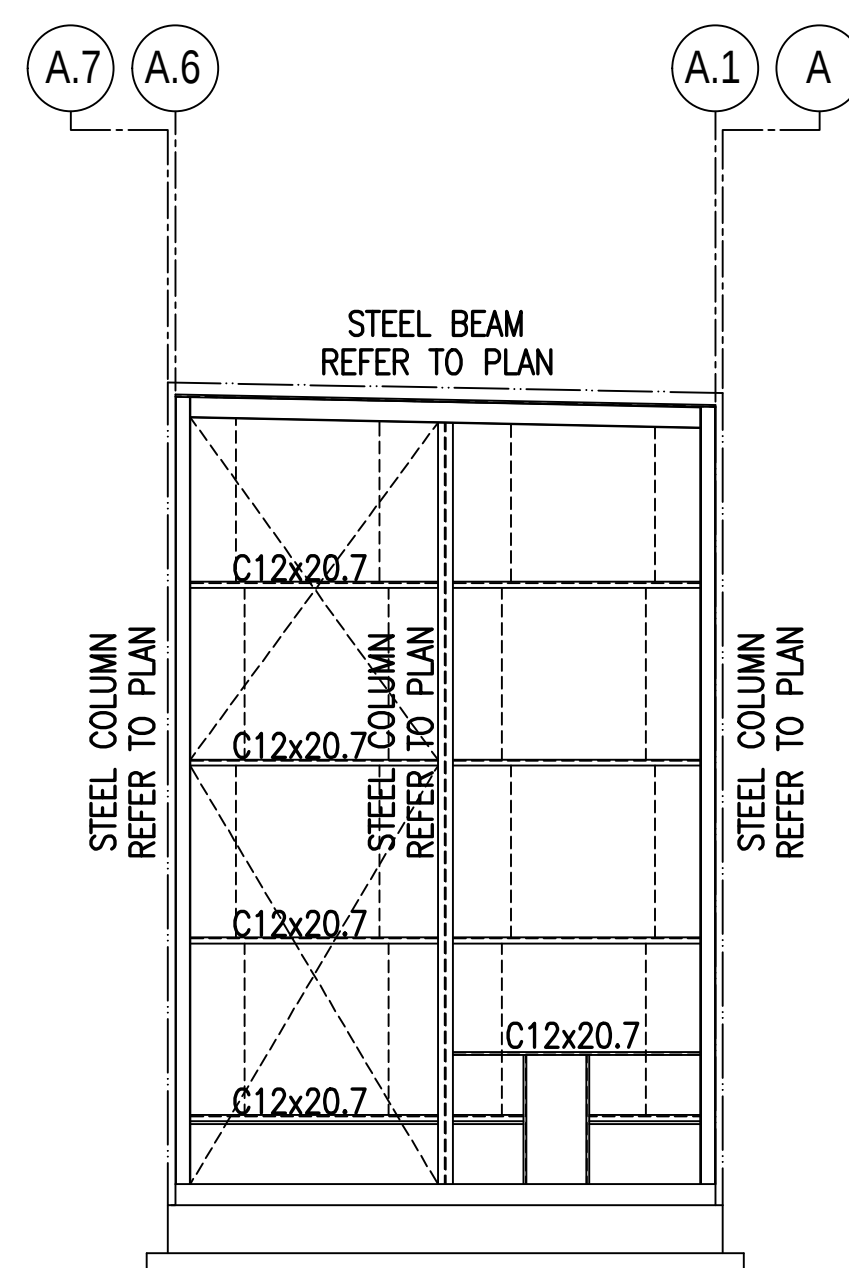
5 | DETAIL
SCALE: N.T.S. | BEAM TO COLUMN CONNECTION



1 | FRAMING ELEVATION
3/32" = 1'-0"



2 | FRAMING ELEVATION
3/32" = 1'-0"



3 | FRAMING ELEVATION
3/32" = 1'-0"