Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0605-20 Date Received 5/5/06 By TW Permit # 24535

Application Approved by - Zoning Official BLK Date 16.05.06 Plans Examiner OKJTH Date 5-8-06

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments Section 14.9 Special Family Lot Permit SOW

Pat cell 303-1981

Applicants Name Brenda Haygood Phone 752.3496

Address 12592 S. US Hwy 441 LC

Owners Name Christopher S. Fulwood Phone

911 Address 980 SW Wendy Terrace LC 32025

Contractors Name Haygood Homes Phone 752.3496

Address 12592 S. US Hwy 441 LC 303-1981

Fee Simple Owner Name & Address Peoples State Bank

Bonding Co. Name & Address

Architect/Engineer Name & Address Pat Haygood Marty Humphries

Mortgage Lenders Name & Address Peoples State Bank

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 28-4S-17-08835-092 Estimated Cost of Construction 62,000.

Subdivision Name Lot Block Unit Phase

Driving Directions Hwy 41 South, turn right on CR 242, TL on Wendy Terrace go .7 mile on right

Type of Construction new home Number of Existing Dwellings on Property 0

Total Acreage 2.32 Lot Size ~~2.32~~ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 172.48' ~~328.80'~~ ✓ Side 156.24' ✓ Side 145' ✓ Rear ~~35'~~ 501 ✓

Total Building Height 14'2" Number of Stories 1 Heated Floor Area 1080 Roof Pitch 5/12

Porches 72 TOTAL 1152

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CRC1326715

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

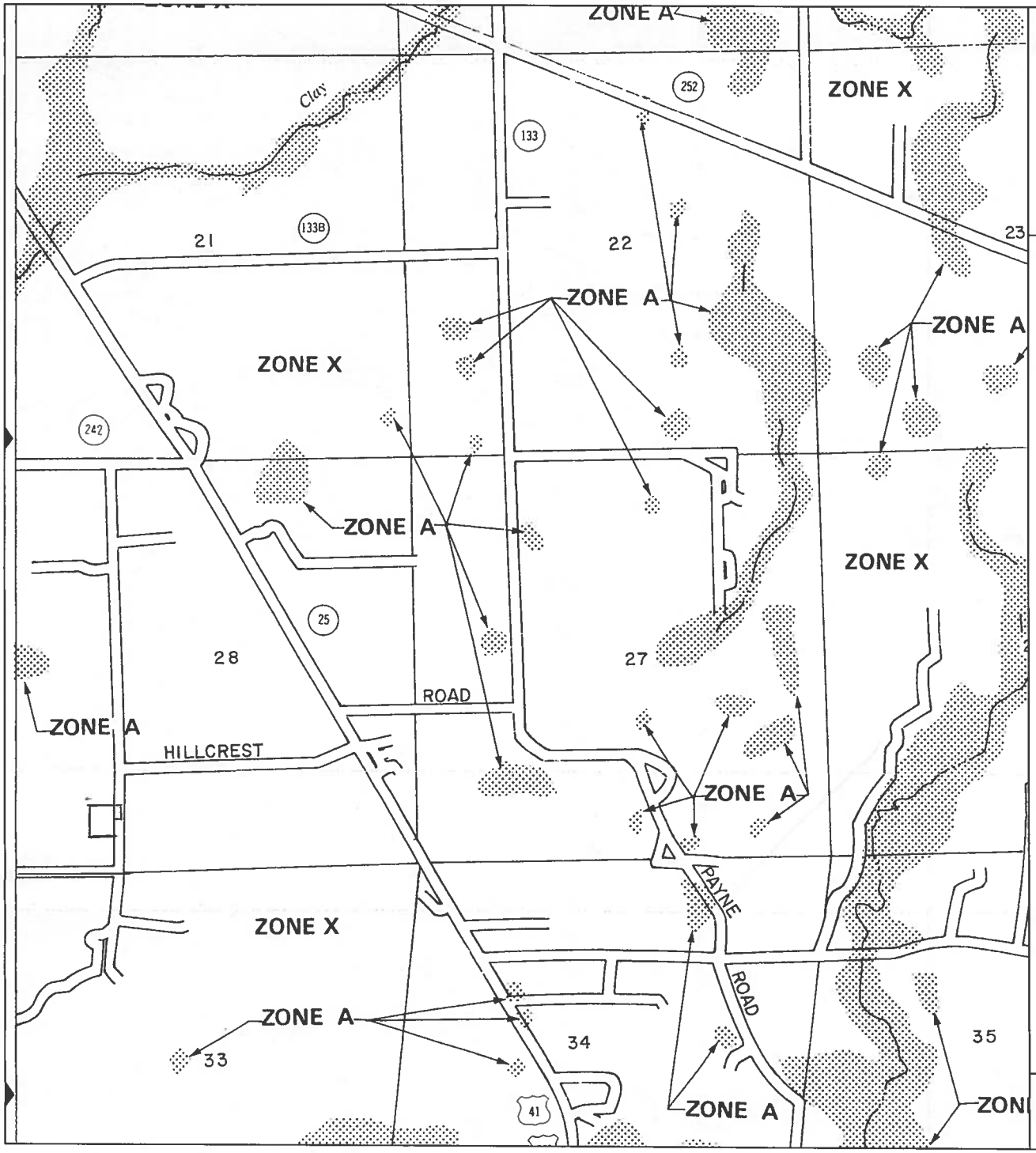
NOTARY STAMP/SEAL

Donna S. Higginbotham MY COMMISSION # DD184389 EXPIRES March 07, 2009 Bonded Thru Troy Fain-Insurance NOTARY PUBLIC STATE OF FLORIDA

Sworn to (or affirmed) and subscribed before me this 24 day of April 20 06.

Personally known ✓ or Produced Identification

Notary Signature



N

APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 200 OF 300

PANEL LOCATION

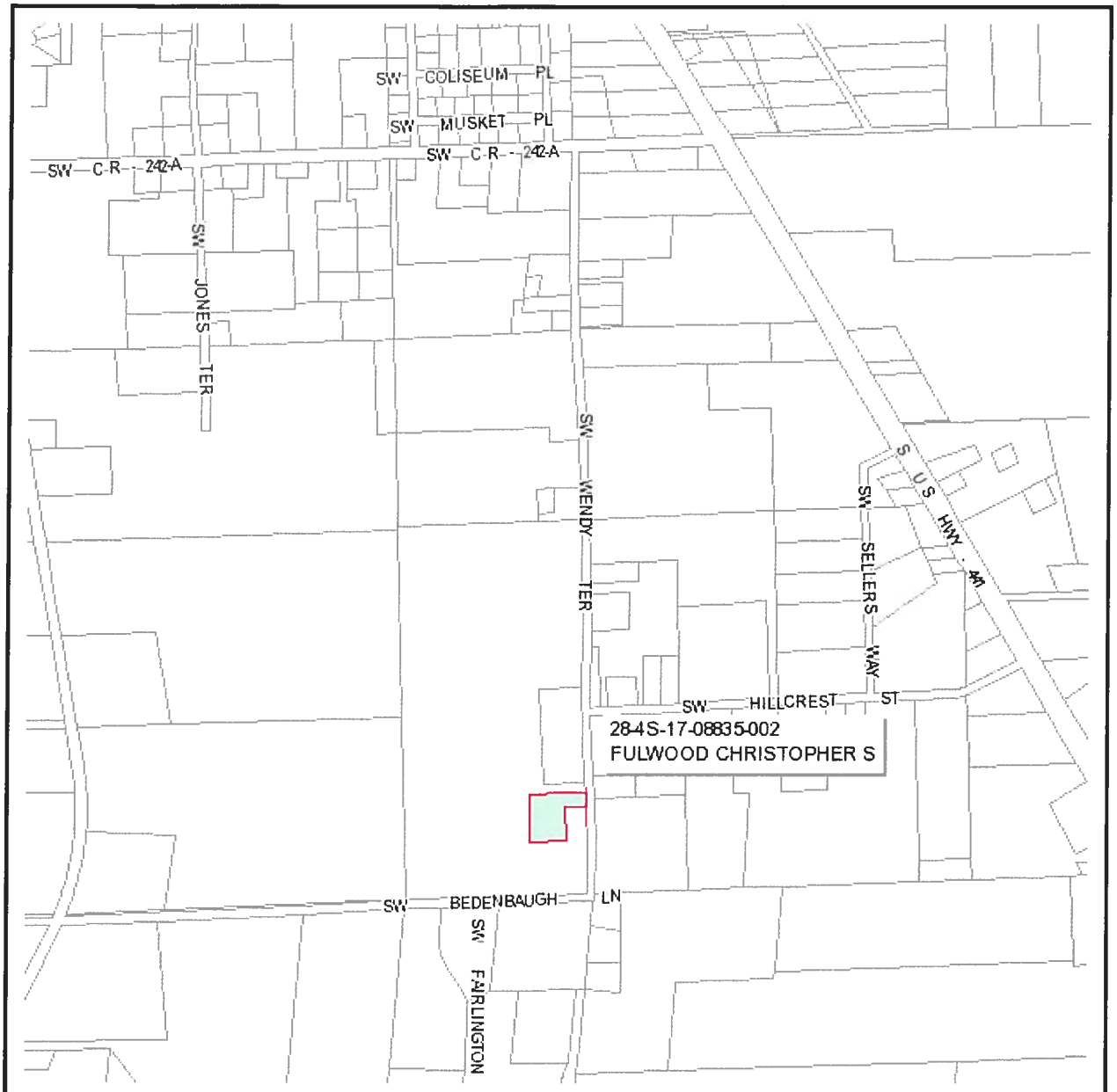
COMMUNITY-PANEL NUMBER
120070 0200 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 5/8/2006 (printed at scale and type A)



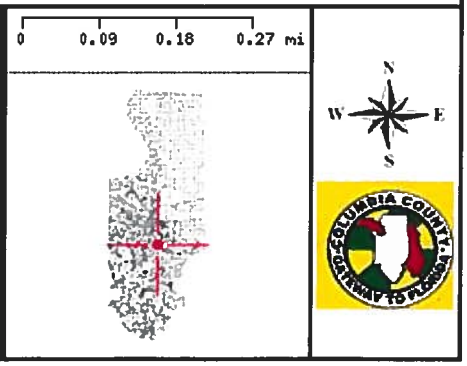
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 28-4S-17-08835-002 - PASTURELAN (006200)

COMM AT SE COR OF SW1/4 OF SW1/4, RUN W 652.63 FT, N 373.48 FT, E 226.43 FT FOR POB

Name:	FULWOOD CHRISTOPHER S	LandVal	$0.00
Site:		BldgVal	$0.00
Mail:	1004 SW WENDY TERR LAKE CITY, FL 32025	ApprVal	$492.00
		JustVal	$23,200.00
Sales	3/1/2006 $100.00 I / U	Assd	$492.00
Info	12/20/1996 $0.00 I / U	Exmpt	$0.00
		Taxable	$492.00



This information, GIS Map Updated: 4/6/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad

Columbia County Property Appraiser

DB Last Updated: 4/6/2006

2006 Proposed Values

Parcel: 28-4S-17-08835-002

Tax Record | Property Card | Interactive GIS Map | Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	FULWOOD CHRISTOPHER S
Site Address	
Mailing Address	1004 SW WENDY TERR LAKE CITY, FL 32025
Brief Legal	COMM AT SE COR OF SW1/4 OF SW1/4, RUN W 652.63 FT, N 373.48 FT, E 226.43 FT FOR POB

Use Desc. (code)	PASTURELAN (006200)
Neighborhood	28417.00
Tax District	2
UD Codes	MKTA02
Market Area	02
Total Land Area	2.310 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (0)	$0.00
Ag Land Value	cnt: (1)	$392.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (1)	$100.00
Total Appraised Value		$492.00

Just Value	$23,200.00
Class Value	$492.00
Assessed Value	$492.00
Exempt Value	$0.00
Total Taxable Value	$492.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
3/1/2006	1076/327	WD	I	U	01	$100.00
12/20/1996	832/1721	WD	I	U	02	$0.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0031	BARN,MT AE	2005	$100.00	1.000	18 x 18 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
006200	PASTURE 3 (AG)	2.310 AC	1.00/1.00/1.00/1.00	$170.00	$392.00
009910	MKT.VAL.AG (MKT)	2.310 AC	1.00/1.00/1.00/1.00	$0.00	$23,100.00

Columbia County Property Appraiser DB Last Updated: 4/6/2006

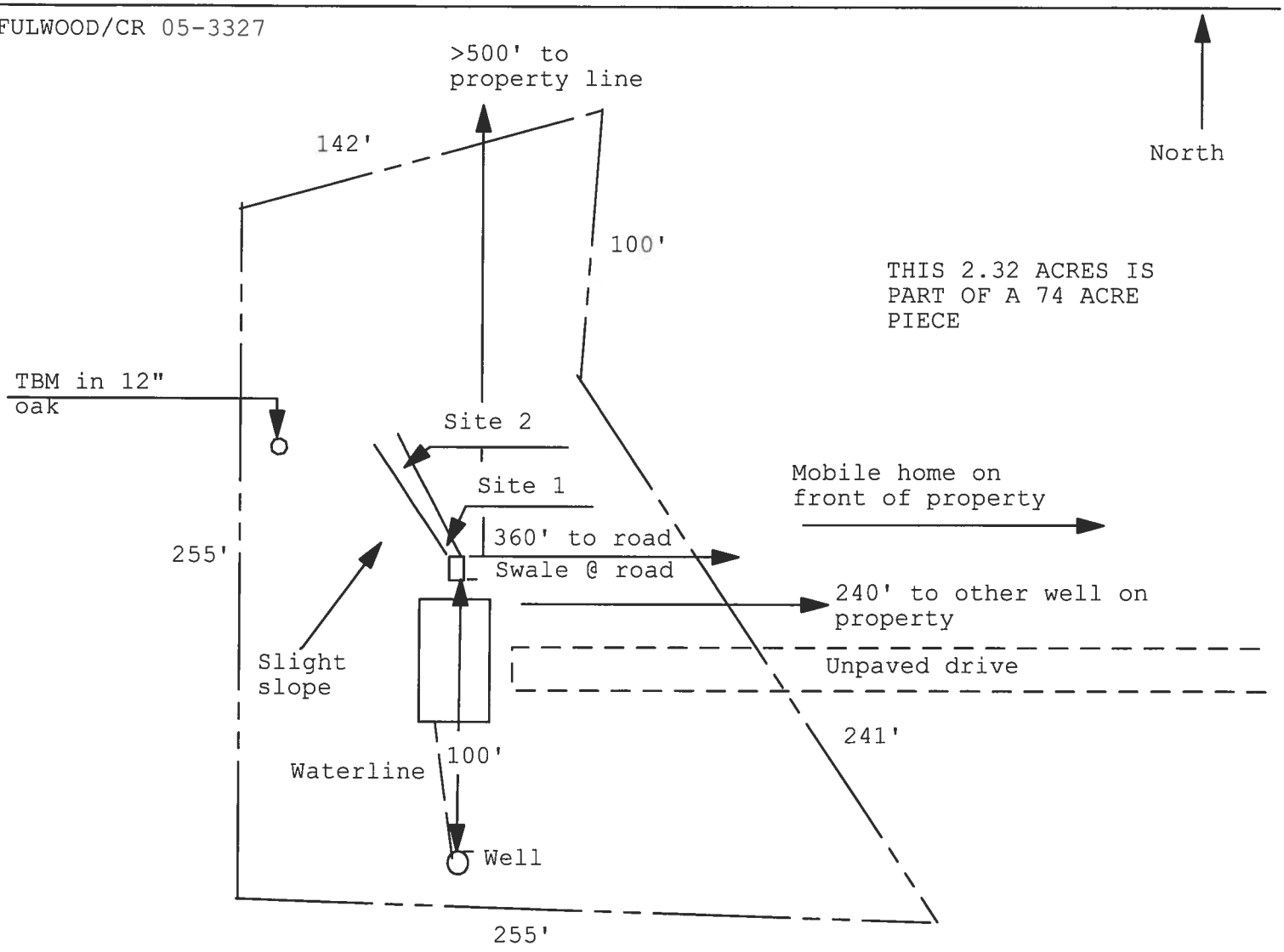
1 of 1

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0402N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

FULWOOD/CR 05-3327



1 inch = 60 feet

Site Plan Submitted By Paul Lloyd **Date** 3/6/06

Plan Approved ✓ **Not Approved** ____ **Date** 4/24/06

By [illegible] Columbia **CPHU**

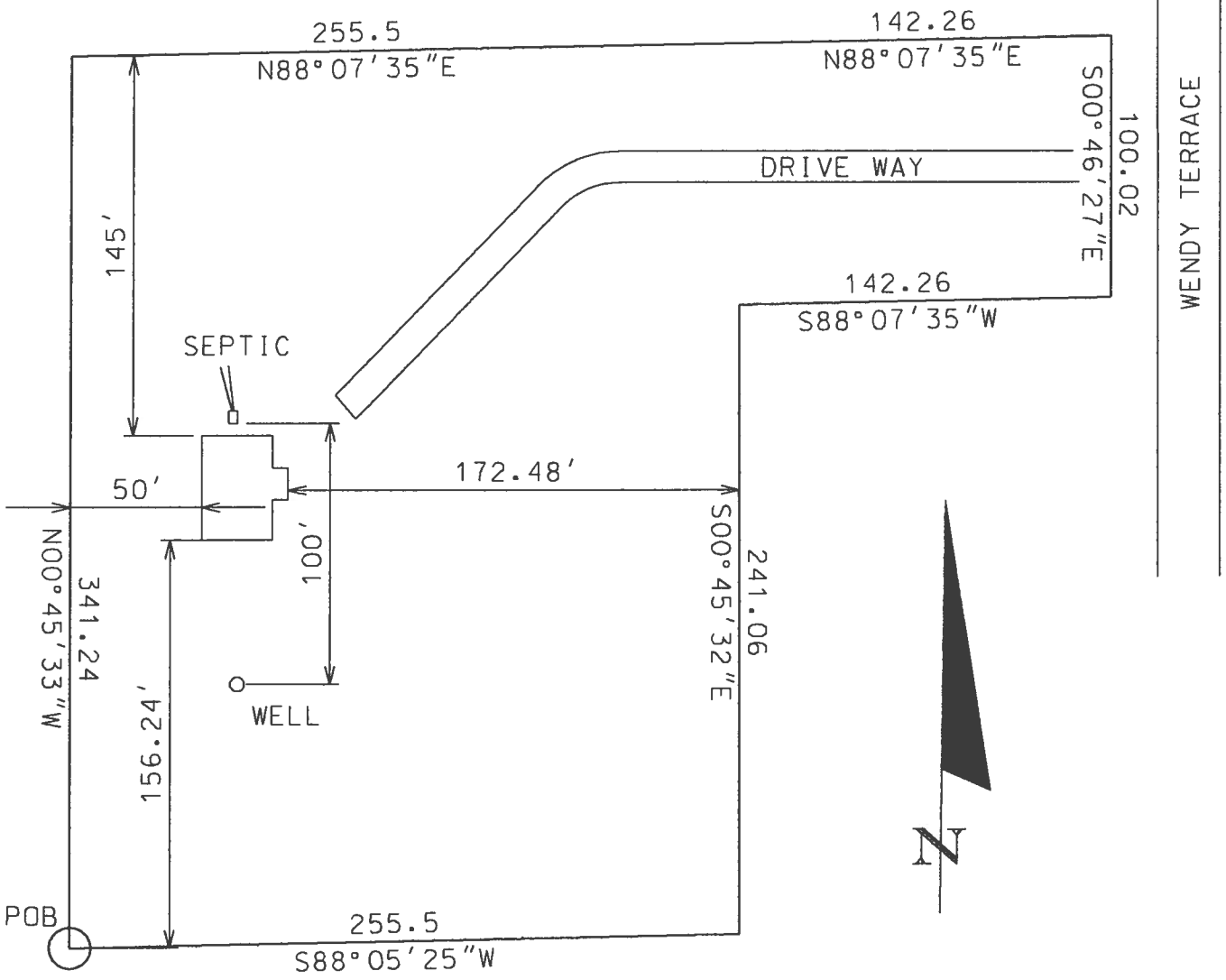
Notes: ____

SITE LOCATION

SHAWN FULWOOD

PID 28-4S-17-08835-002

HWY. 41 SOUTH TURN RIGHT ON C.R. 242,
TURN LEFT ON WENDY TERR. GO .7 MILES
SITE ON RIGHT.



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COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 4/19/2006 **DATE ISSUED:** 4/26/2006

ENHANCED 9-1-1 ADDRESS:

980 SW WENDY TER

LAKE CITY FL 32025

PROPERTY APPRAISER PARCEL NUMBER:

28-4S-17-08835-002

Remarks:

Address Issued By: _____

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

FORM 600B-04

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Residential Component Prescriptive Method B

NORTH 1 2 3

Compliance with Method B of Subchapter 6 of the Florida Energy Efficiency Code may be demonstrated by the use of Form 600B for single-and multiple-family residences of three stories or less in height, and additions to existing residential buildings. To comply, a building must meet or exceed all of the energy efficiency prescriptives in any one of the prescriptive component packages and comply with the prescriptives listed in this form. An alternative method is provided for additions of 600 square feet or less by use of Form 600C. If a building does not comply with this method, it may still comply under other sections in Chapter 6 of the code.

PROJECT NAME: AND ADDRESS:	Fulwood	BUILDER:	
		PERMITTING OFFICE: Columbia	CLIMATE ZONE: 1 ☐ 2 ☐ 3 ☑
OWNER: C.S. Fulwood		PERMIT NO.: 2453 5	JURISDICTION NO.: 221000

1. New construction including additions which incorporate any of the following features cannot comply using this method: steel stud walls, single assembly roof/ceiling construction, or skylights or other nonvertical roof glass.
2. Choose one of the component packages "A" through "E" from Table 6B-1 by which you intend to comply with the code. Circle the column of the package you have choosen.
3. Fill in all the applicable spaces of the "To Be Installed" column on "Table 6B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
4. Complete page 1 based on the "To Be Installed" column information.
5. Read "Minimum Requirements for All Packages," Table 6B-2 and check each box to indicate your intent to comply with all applicable items.
6. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

	Please Print	CK
1. Compliance package chosen (A-E)	1. B	
2. New construction or addition	2. new	
3. Single-family detached or multiple-family attached	3. Single	
4. If multiple-family--No. of units covered by this submission	4.	
5. Is this a worst case? (yes/no)	5. yes	
6. Conditioned floor area (sq. ft.)	6. 1080	
7. Predominant eave overhang (ft.)	7. 10"	
	Single Pane / Double Pane	
8. Glass type and area:		
a. Clear glass	8a. ____ sq. ft. 129.0 sq. ft.	
b. Tint, film or solar screen	8b. ____ sq. ft. 0 sq. ft.	
9. Percentage of glass to floor area	9. ____ %	
10. Floor type, area or perimeter, and insulation:		
a. Slab-on-grade (*R*-value)	10a R = 0 134 lin. ft.	
b. Wood, raised (*R*-value)	10b. R = ____ ____ sq. ft.	
c. Wood, common (*R*-value)	10c. R = ____ ____ sq. ft.	
d. Concrete, raised (*R*-value)	10d. R = ____ ____ sq. ft.	
e. Concrete, common (*R*-value)	10e. R = ____ ____ sq. Ft.	
11. Wall type, area and insulation:		
a. Exterior: 1. Masonry (Insulation R-value)	11a-1 R = ____ ____ sq. ft.	
a. Exterior: 2. Wood frame (Insulation *R*-value)	11a-2 R = 13 1072 sq. ft.	
b. Adjacent: 1. Masonry (Insulation *R*-value)	11b-1 R = ____ ____ sq. ft.	
b. Adjacent: 2. Wood frame (Insulation *R*-value)	11b-2 R = ____ ____ sq. ft.	
12. Ceiling type, area and insulation:		
a. Under attic (Insulation *R*-value)	12a. R = 30 sq. ft. 1080	
b. Single assembly (Insulation *R*-value)	12b. R = ____ ____ sq. ft.	
13. Air distribution system: Duct insulation, location	13. R = 6 ____	
Test report (attach if required)		
14. Cooling system:	14a. Type: Central	
(Types: central, room unit, package terminal A.C., gas, none)	14b. SEER/EER: 12	
	14c. Capacity: 2 ton	
15. Heating system:	15a. Type: Heat Pump	
(Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)	15b. HSPF/COP/AFUE: ____	
	15c. Capacity: 50 gal	
16. Hot water system:	16a. Type: elec	
(Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)	16b. EF: .88	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: Brenda Haywood DATE: 4/21/06

I hereby certify that this building is in compliance with the Florida Energy Code:

OWNER AGENT: [signature] DATE: 4/21/06

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

TABLE 6B-1 — MINIMUM REQUIREMENTS — Climate Zones 1 2 3

COMPONENTS		PACKAGES FOR NEW CONSTRUCTION					TO BE INSTALLED
		A	B	C	D	E	
GLASS	Max. % of Glass to Floor Area	15%	15%	20%	20%	25%	15 %
GLASS	Type	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Clear (DC)	Double Tint (DT)	DC: ☐ DT: ☐
GLASS	Overhang	1'4"	2'	2'	2'	2'	16" FEET
WALLS	Masonry	EXTERIOR AND ADJACENT MASONRY WALLS R-5 COMMON MASONRY WALLS R-3 EACH SIDE.					EXT: R = ADJ: R = COM: R =
WALLS	Wood Frame	EXTERIOR, ADJACENT, AND COMMON WOOD-FRAME WALLS R-11					EXT: R = 13 ADJ: R = COM: R =
CEILINGS		R-30	R-30	R-30	R-30	R-30	UNDER ATTIC: R= 30 COMMON: R:
		(NO SINGLE ASSEMBLY CEILINGS ALLOWED)					
FLOORS	Slab-On-Grade	R-0					R = 0
FLOORS	Raised Wood	R-19 (ONLY STEM WALL CONSTRUCTION ALLOWED EXCEPT PACKAGE C)					R =
FLOORS	Raised Concrete	R-7					R =
DUCTS		R-6	R-6	R-6, TESTED	R-6	R-6, TESTED	R = 6 COND. ☐
SPACE COOLING (SEER)		12.0	10.5	12.0	11.0	12.0	SEER = 12
HEAT	Elect. (HSPF)	7.9	7.1	7.4	7.4	7.4	HSPF =
HEAT	Gas/Oil (AFUE)	MINIMUM OF .73 (Direct heating) or .78 (Central)					AFUE =
HOT WATER SYSTEM	Electric Resistance**	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	NOT ALLOWED (SEE BELOW)	EF .92	EF = .88
HOT WATER SYSTEM	Gas & Oil**	MINIMUM EF OF .59				NATURAL GAS ONLY (SEE BELOW)	EF =
HOT WATER SYSTEM	Other	Any of the following are allowed: dedicated heat pump, heat recovery unit or solar system.					DHP: ☐ EF = HRU: ☐ SOLAR: ☐ EF =

* Single package units minimum SEER=9.7, HSPF = 6.6.
** Minimum efficiencies for gas and electric hot water systems apply to 40 gallon water heaters. Refer to Table 612.1 ABC.3.2 for minimum code efficiencies for oil water heaters and other sizes.

DESCRIPTION OF BUILDING COMPONENTS LISTED

Percent of Glass to Floor Area: This percentage is calculated by dividing the total of all glass areas by the total conditioned floor area.

Overhang: The overhang is the distance the roof or soffit projects out horizontally from the face of the glass. All glass areas shall be under an overhang of at least the prescribed length with the following exceptions: 1) glass on the gabled ends of a house and 2) the glass in the lower stories of a multistory house.

Wall, Ceiling and Floor Insulation Values: The *R*-values indicated represent the minimum acceptable insulation level added to the structural components of the wall, ceiling or floor. The *R*-value of the structural building materials shall not be included in this calculation. "Common" components are those separating conditioned tenancies in a multiple-family building. "Adjacent" components separate conditioned space from unconditioned but enclosed space. "Exterior" components separate conditioned space from unconditioned and unenclosed space.

Floor: Slab-on-grade floors without edge insulation are acceptable. Raised wood floors shall have continuous stem walls with insulation placed on the stem wall or under the floor except Package C.

Ducts: "TESTED" shall mean the ducts have less than 5% leakage based on a certified test report by a state-approved tester.

Space Cooling System: Cooling systems shall have a Seasonal Energy Efficiency Ratio (SEER) for central units or Energy Efficiency Ratio (EER) for room units or PTACs equal to or greater than the prescribed value.

Electric Space Heating Option: Heat pump systems shall be rated with a Heating Seasonal Performance Factor (HSPF) equal to or greater than the prescribed HSPF. Heat pump systems may contain electric strip backups meeting the criteria of Section 608.1.ABC.3.2.1.2. No electric resistance space heat is allowed for these packages.

Electric Resistance Hot Water Option: For packages designated "Not Allowed," an electric resistance hot water system may be installed only in conjunction with one of the "Other Hot Water System Options." See below.

Other Hot Water System Options: Any dedicated heat pump, heat recovery unit, or solar hot water system may be installed. Solar systems must have an EF of 1.5 or higher. Electric resistance systems having and EF of .92 or greater, or natural gas systems with EF .59 or greater may be used in conjunction with these systems.

TABLE 6B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	606.1	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	606.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	606.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	606.1	Type IC rated with no penetrations (two alternatives allowed).	✓
Multistory Houses	606.1	Air barrier on perimeter of floor cavity between floors.	NA
Exhaust Fans	606.1	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker electric or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Noncommercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%.	NA
Hot Water Pipes	612.1	Insulation is required for hot water circulating systems (including heat recovery units).	NA
Shower Heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 psig.	✓
HVAC Duct Construction, Insulation & Installation	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section 610.1. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
~~XXXNORTH FIRST STREET~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

19.20

This Instrument Prepared by & return to:
Name: *KIM WATSON, an employee of TITLE OFFICES, LLC*
Address: *1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025*
File No. 06Y-02081KW

Inst:2006005365 Date:03/03/2006 Time:12:49
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1076 P:327

Parcel I.D. #: *08835-000*

SPACE ABOVE THIS LINE FOR PROCESSING DATA | *SPACE ABOVE THIS LINE FOR RECORDING DATA*

THIS WARRANTY DEED *Made the 1st day of March, A.D. 2006, by* ***BEATRICE HUNTER, BY HER ATTORNEY IN FACT BRUCE C. FULWOOD AND BRUCE C. FULWOOD, INDIVIDUALLY AND WANDA G. FULWOOD, HIS WIFE,*** *hereinafter called the grantor, to* ***CHRISTOPHER S. FULWOOD,*** SINGLE, *whose post office address is* 1004 SW WENDY TERRACE, LAKE CITY, FLORIDA 32025 *hereinafter called the grantee:*

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantor, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in* ***Columbia County, State of Florida,*** *viz:*

PARCEL "A"
PART OF THE SW ¼ OF THE SW ¼ OF SECTION 28, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA, MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT AN ALUMINUM PLATE AND NAIL, LS 1079, MARKING THE SE CORNER OF THE SW ¼ OF THE SW ¼ OF SAID SECTION 28, AND THENCE S 89°40'30" W, ALONG THE MONUMENTED SOUTH LINE OF SAID SW ¼ OF THE SW ¼, A DISTANCE OF 652.63 FEET; THENCE N 00°46'27" W, 373.48 FEET TO A CONCRETE MONUMENT, LS 4708; THENCE N 88°05'28" E, 226.43 FEET TO A CONCRETE MONUMENT, LS 4708, AND THE POINT OF BEGINNING; THENCE N 00°45'33" W, 341.24 FEET TO A CONCRETE MONUMENT, LS 4708; THENCE N 88°07'35" E, 255.50 FEET TO A CONCRETE MONUMENT, LS 4708; THENCE CONTINUE N 88°07'35" E, 142.26 FEET TO A 5/8 INCH IRON ROD, LS 4708 ON THE WEST RIGHT-OF-WAY LINE OF SW WENDY TERRACE, A 60 FOOT WIDE PUBLIC RIGHT-OF-WAY; THENCE S 00°46'27" E, ALONG SAID WEST LINE, 100.02 FEET TO A 5/8 INCH IRON ROD, LS 4708; THENCE S 88°07'35" W, 142.28 FEET A 5/8 INCH IRON ROD, LS 4708; THENCE S 00°45'32" E, 241.06 FEET TO A CONCRETE MONUMENT, LS 4708; THENCE S 88°05'25" W, 255.50 FEET TO THE POINT OF BEGINNING.

Together *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.*

To Have and to Hold *the same in fee simple forever.*

And *the grantor hereby covenants with said grantee that he is lawfully seized of said land in fee simple; that he has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2006.*

In Witness Whereof, *the said grantor has signed and sealed these presents, the day and year first above written.*

Signed, sealed and delivered in the presence of:

Witness Signature
Ronnie T. Hattiwanger
Printed Name

Witness Signature
BONITA HADWIN
Printed Name

Beatrice Hunter L.S.
BEATRICE HUNTER, BY BRUCE C. FULWOOD, HER ATTORNEY IN FACT
Address:

Bruce C. Fulwood
BRUCE C. FULWOOD, INDIVIDUALLY

Wanda G. Fulwood
WANDA G. FULWOOD
1004 SW Wendy Terrace
LAKE CITY, FLORIDA 32025

Inst:2006005365 Date:03/03/2006 Time:12:49
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1076 P:328

STATE OF FLORIDA
COUNTY OF COLUMBIA

*The foregoing instrument was acknowledged before me this 1st day of March, 2006, by **BEATRICE HUNTER, BY HER ATTORNEY IN FACT BRUCE C. FULWOOD AND BRUCE C. FULWOOD, INDIVIDUALLY AND WANDA G. FULWOOD, HIS WIFE**, who is known to me or who has produced Fla Drivers Lic as identification.*

Notary Public
My commission expires _____

Bonita Hadwin
MY COMMISSION # DD233904 EXPIRES
August 10, 2007
BONDED THRU TROY FAIN INSURANCE, INC.

WARRANTY DEED
INDIVID. TO INDIVID.

RAMCO FORM 01

Return to (enclose self addressed stamped envelope)

Name Bruce C. Fulwood
Address 4261 Banks Rd
Middleburg, Fl 32068

This Instrument Prepared by
Name Bruce C. Fulwood
Address 4261 Banks Rd
Middleburg, Fl. 32068

Property Appraiser's Parcel Identification

Lot Number:

Grantee(s) S.S. #(s):

96-18240

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY FLORIDA

1996 DEC 26 PM 2:47

CLERK OF CIRCUIT COURTS
COLUMBIA COUNTY, FLORIDA
BY BRH D.C.

SPACE ABOVE THIS LINE FOR PROCESSING DATA — SPACE ABOVE THIS LINE FOR RECORDING DATA

This Warranty Deed, *Made the* 26 *day of* December, 19 96, *by* Cecil C. Hunter and his wife Beatrice Hunter, *hereinafter called the Grantor, to* Cecil C. Hunter and his wife Beatrice Hunter and Bruce C. Fulwood and his wife Linda G. Fulwood, *whose post office address is* LAKE City R.F.D. 2 Box 201A, *hereinafter called the Grantee.*

(Wherever used herein the terms "Grantor" and "Grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth, *That the Grantor, for and in consideration of the sum of* $ 10.00 *and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the Grantee all that certain land, situate in* Columbia *County, State of* Florida, *viz:*

See attached Schedule 'A'

DOC. STAMPS .70
INTANGIBLE TAX C
P. DEWITT CASON, CLERK OF
CIR. CT. COLUMBIA COUNTY
BY D.C.

OR 0832 PG 1721

OFFICIAL RECORDS

Together, *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.* **To Have and to Hold,** *the same in fee simple forever.*

And *the Grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land, and hereby warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 19 .*

In Witness Whereof, *the said Grantor has signed and sealed these presents the day and year first above written.*

Signed, sealed and delivered in the presence of:

Witness Signature (as to first Grantor)
Gail Hinson
Printed Name
Margeline Hines
Witness Signature (as to first Grantor)
MARGELINE HINES
Printed Name

Cecil C. Hunter (L.S.)
Grantor Signature
Cecil C Hunter
Printed Name
Rt 6 Box 510 Lake City FL 32024
Post Office Address

Witness Signature (as to Co-Grantor, if any)
Gail Hinson
Printed Name
Margeline Hines
Witness Signature (as to Co-Grantor, if any)
MARGELINE HINES
Printed Name

Beatrice Hunter (L.S.)
Co-Grantor Signature (if any)
Beatrice Hunter
Printed Name
Rt 6 Box 510 Lake City FL 32024
Post Office Address

STATE OF Florida)
COUNTY OF Columbia)

I hereby Certify that on this day, before me, an officer duly authorized to administer oaths and take acknowledgments, personally appeared Cecil C Hunter # VA Driver's License # 10672895 E, Beatrice Hunter # FL DL # 536-063-16-216 9, known to me to be the person(s) described in and who executed the foregoing instrument, who acknowledged before me that executed the same, and an oath was not taken. (Check one.) ☐ Said person(s) is/are personally known to me ☒ Said person(s) provided the following type of identification:

NOTARY RUBBER STAMP SEAL

BELINDA G. SCIPPIO
MY COMMISSION # CC 407894
EXPIRES September 16, 1998
Bonded Thru Notary Public Underwriters

Witness my hand and official seal in the County and State last aforesaid this 26 day of December, A.D. 19 96

Belinda G. Scippio
Signature
Belinda G. Scippio
Printed Name

SCHEDULE 'A'

BK 0832 PG 1722

OFFICIAL RECORDS

The West half of the Southwest Quarter (W1/2 of SW1/4) Section Twenty eight (28) Township Four (4) South of Range Seventeen (17) East, containing Eighty (80) acres more or less.

One acre of land lying in Northeast Quarter of Southwest Quarter (NE1/4 of SW1/4) of Section 28, Township four South of Range Seventeen (17) East in Columbia County, Florida, beginning at Southwest corner of said Northeast Quarter of Southwest Quarter (NE1/4 of SW1/4) and run North 150 yards , thence East 64 2/3 yards, thence South 150 yards, thence West 64 2/3 yards to place of beginning.

Note: In addition to the above consideration, grantees agree to live on the above descirbed property and look after the grantors as long as either of them live and grantors reserve a life estate in the above described property.

Fulwood Residence Columbia County FL

Wind Load Analysis Requirements

(In Compliance with the 2004 Florida Building Code and Amendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071
(386)935-2406

Description of New Residence:

Footprint: 27' x 40' rectangular with 12'x 6'covered front porch
Walls: 2x4-16" O.C. with 7/16" OSB sheathing minimum with vinyl siding and ½"gypsum wall board interior.
Roof Structure: Pre-engineered roof trusses and 7/16" OSB (min.) sheathing
Roof Type: Gable construction (analyzed for 1'4" eave overhang and front porch area)
Foundation: monolithic footer with slab construction

Windload Data and Exposure:

Basic Wind Speed = 110 mph
Importance Factor = 1.0
Exposure category = B
Height and Exposure Adjustment Coefficient = 1.0
Residential Occupancy = Group R3
Analysis Method = FBC 1609.6 - Simplified Provisions for Low Rise Buildings
(see tables 1609.6A, 1609.6B, 1609.6C and 1609.6E for wind pressure values)
Mean roof height = 13'
Roof Cross Slope = 4:12
Eave Overhang= (Analyzed for 1'4" overhang and front porch)
Wall Height = 8'
Shear Wall locations = exterior walls only(all walls 3' in length or greater)
Bracing method for gable locations = framing from wall to roof diaphragm(see attached detail)

Nailing Pattern Requirements:

Wall sheathing:	Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center around edges(including around doors and windows) and 6" on center interior. Full depth blocking shall be installed At horizontal joints in sheathing.
Roof sheathing:	Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center at panel ends and eave overhang areas and 6" on center elsewhere.
Top wall plate:	Nail with 1-16d common nail 12" O.C.(average)

Marty J. Humphries

3-6-06

Strapping and Anchor Requirements:

truss to exterior wall plate and porch beam locations: install one Simpson model H10 hurricane anchor at each truss. (see attached detail)

wall strap tie requirements: at top and bottom of wall install one Simpson model SP4 at each side of each door and window. All other wall locations install SP4's top and bottom of wall 4' on center.

Porch Columns: Install Simpson model ABU44, ABU46 or ABU66 and Simpson model ACE4Max or ACE6Max

Lookouts: Install one Simpson model H5 where lookouts connect to end gable truss.

Gable end: Install one LSTA18 - 4' on center connecting gable end truss to wall framing.

Gable End Bracing Requirements:

At each gable end install one 2x4 SPF 8' stud spaced 6' on center horizontal along top of bottom chord of trusses, nail with 2-12d nails at each truss including end truss. In addition, install a 2x4 brace extending from this stud at the gable end truss approx. 45 degrees to truss at roof sheathing, nail with 2 -12d nails where it crosses truss members and at ends. Gable end trusses shall be built to receive sheathing with vertical members 2' on center. Vertical members of gable end truss greater than 5' in height shall be stiffened with one 2x4 SPF nailed with 12d nails 8" on center to back of vertical member. (See attached detail)

Foundation Requirements:

Monolithic Footer: Minimum size of footer shall be 20" x 12" wide with 2-#5 rebar continuous(beveled to slab). ½" anchor bolts with 2" washers shall be installed 3' on center and 9" from corners each way and at each side of door openings. (3000 psi concrete min.)(Note: foundation designed using an allowable bearing pressure of 1000 psf)

Header Requirements:

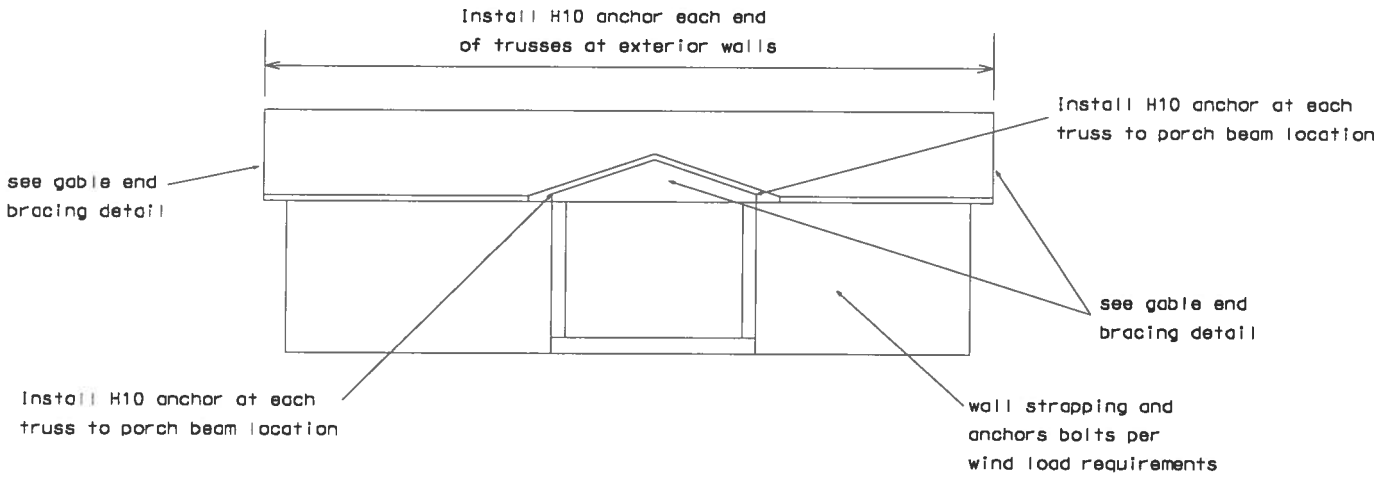
Windows & Doors: Header shall be 2 - #2 SYP 2x12's with ½" plywood/OSB between. .

Porch Beams: Header shall be 2-#2 SYP 2x10's with ½" plywood/OSB between

Note: Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

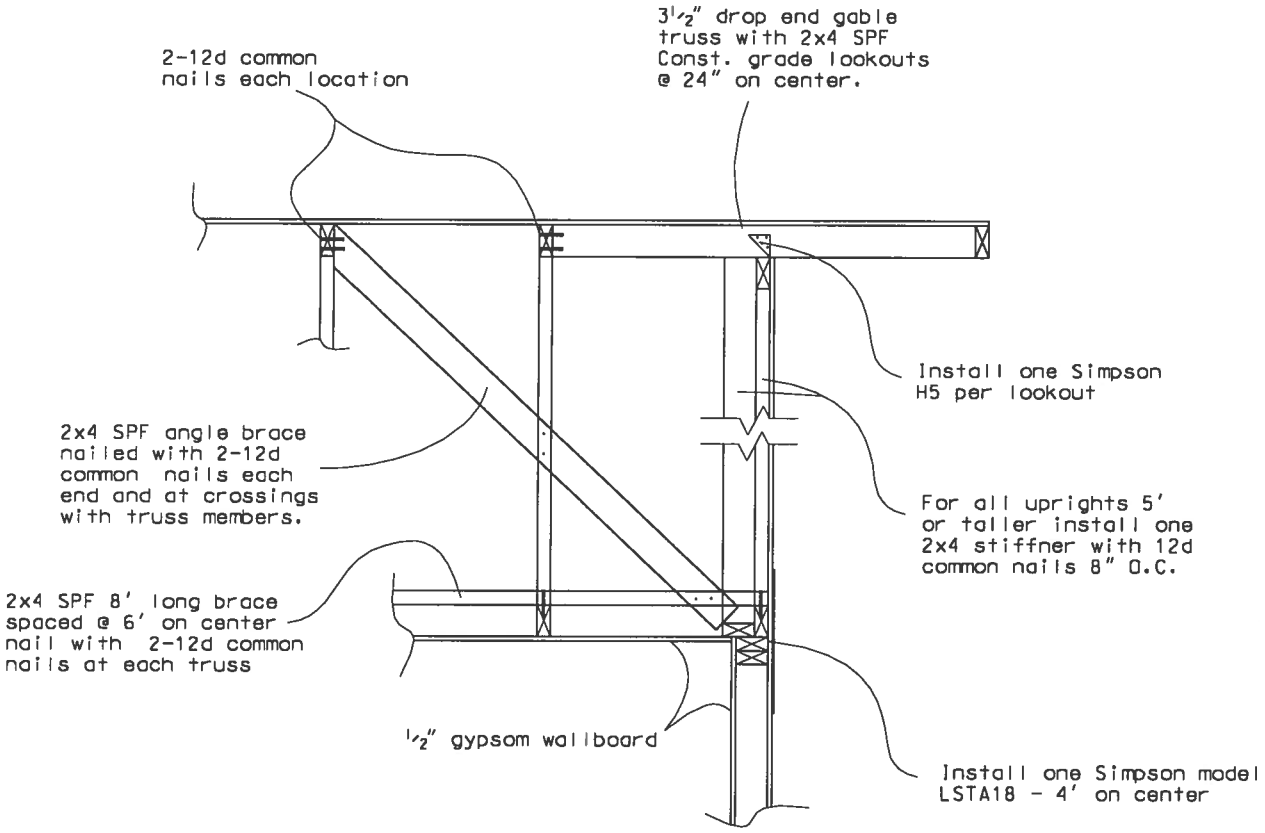
Marty D. Wright

3-6-06



Location of Truss Anchors N.T.S.

3-6-06



GABLE END BRACING DETAIL (N.T.S.)

Marty J. Humphries

3-6-06

Fulwood Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071

NEW! The H2.5A is symetrically designed for easy installation, with higher uplift loads to meet new code requirements. A placement mark allows easy installation on double top plates.

NEW! The H5A has an installed cost benefit, as it only requires 6 nails, to meet lower uplift requirements.

The H connector series provides wind and seismic ties for trusses and rafters.

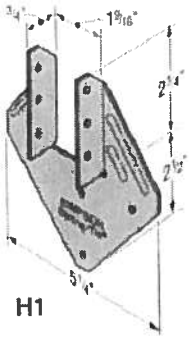
Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
Design Shear/Allowable Shear + Design Tension/Allowable Tension ≤ 1.0

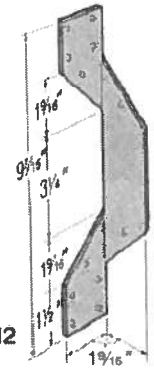
MATERIAL: See table

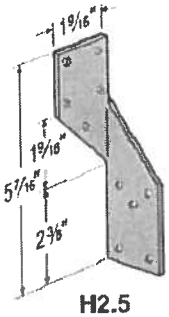
FINISH: Galvanized; H10-2, H11Z–Z-MAX. Other models available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- H1 can be installed with flanges facing outwards (reverse of drawing number 1). When installed inside a wall, a birdsmouth cut is required.
- H2.5, H3, H4, H5 and H6 ties are shipped in equal quantities of rights and lefts.
- Bend the H7 over the top of the truss. Install a minimum of four 8d nails into the truss, including two into the truss side.
- Hurricane Ties do not replace solid blocking.

CODES: BOCA, ICBO, SBCCI NER-422, NER-393, NER-432; NER-499; City of L.A. RR 24818; Dade Co. FL 00-0512.11 (H10), 00-0926.01 (H1, H2.5, H3, H4, H5)


H1


H2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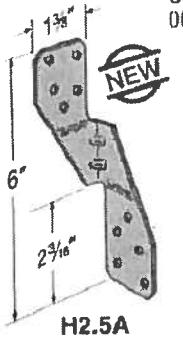
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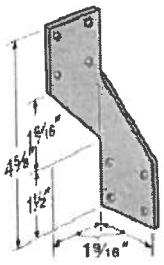
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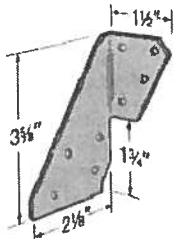
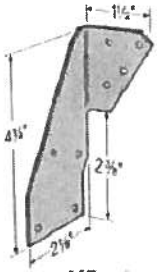
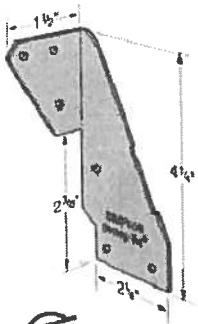
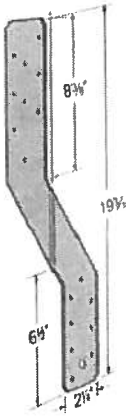

H4
U.S. Patent 4,714,372


H5
U.S. Patent 4,714,372


H5A
U.S. Patent 4,714,372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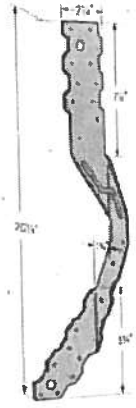
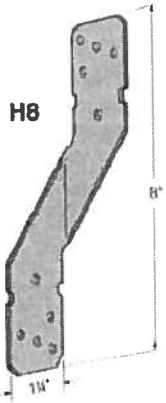
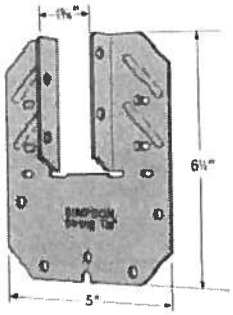

H7


H8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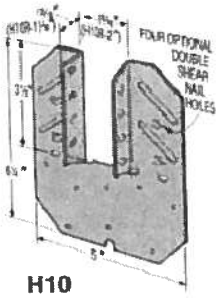
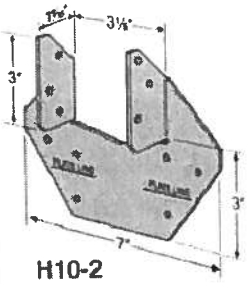
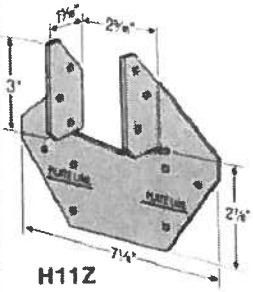

H10
(H10R similar)
U.S. Patents 4,460,941; Canada Patent 1,193,418

Model No.	Ga	Fasteners: To Rafters/ Truss	Fasteners: To Plates	Fasteners: To Studs	Uplift Avg Ult	Doug-Fir Larch/So. Pine Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½ Nails (133 & 160)	Spruce-Pine-Fir Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½ Nails (133 & 160)
H1	18	6-8dx1½	4-8d	—	1958	490	585	485	165	455	400	400	415	140	370
H2	18	5-8d	—	5-8d	1040	335	335	—	—	335	230	230	—	—	230
H2.5	18	5-8d	5-8d	—	1300	415	415	150	150	415	365	365	130	130	365
NEW H2.5A	18	5-8d	5-8d	—	1793	600	600	110	110	480	520	535	110	110	480
H3	18	4-8d	4-8d	—	1433	455	455	125	160	415	320	320	105	140	290
H4	20	4-8d	4-8d	—	1144	360	360	165	160	360	235	235	140	135	235
H5	18	4-8d	4-8d	—	1485	455	465	115	200	455	265	265	100	170	265
NEW H5A	18	3-8d	3-8d	—	1500	350	420	115	180	290	245	245	100	120	170
H6	16	—	8-8d	8-8d	3983	915	950	650	—	—	785	820	560	—	—
H7	16	4-8d	2-8d	8-8d	2991	930	985	400	—	—	800	845	345	—	—
H8	18	5-10dx1½	5-10dx1½	—	2422	620	745	—	—	—	530	565	—	—	—
H9KT	18	4-SDS¼x1½	5-SDS¼x1½	—	2812	875	875	680	125	—	755	755	660	125	—
H10	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10R	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10-2	18	6-10d	6-10d	—	2447	760	760	455	395	—	655	655	390	340	—
H11Z	18	6-16dx2½	6-16dx2½	—	5097	830	830	525	760	—	715	715	450	655	—


H10-2


H11Z

1. Loads have been increased 33⅓% and 60% for earthquake or wind loading with no further increase allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5), 390 lbs (H2.5A), 360 lbs (H4) and 310 lbs (H6).
4. The H9KT is sold in 20 piece packs with screws.
5. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
6. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

Z2 clips secure 2x4 flat blocking between joists or trusses to support sheathing.

MATERIAL: Z clips–see table. A21 and A23–18 ga.; all other A angles–12 ga.

FINISH: Galvanized

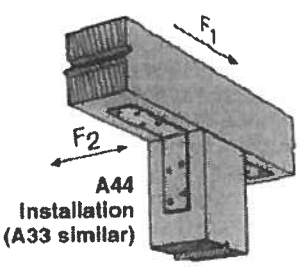
INSTALLATION: • Use all specified fasteners. See General Notes.
• Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.

CODES: BOCA, ICBO, SBCCI NER-421 (except A33, A44); City of L.A. RR 25076 (except A33, A44); Dade Co. FL 99-0623.04 (A21 and A23).

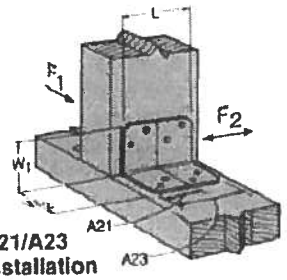
Model No.	Dimensions W1	W2	L	Fasteners Base Bolts	Base Nails	Post Bolts	Post Nails	Avg Ult F2	Allowable Loads[2] DF/SP (133) F1	(133) F2	(160) F1	(160) F2
A21	2	1½	1⅜	—	2-10dx1½	—	2-10dx1½	540	245	175	290	175
A23	2	1½	2¼	—	4-10dx1½	—	4-10dx1½	1767	485	485	585	565
A33	3	3	1½	—	4-10d	—	4-10d	2635	625	330	750	330
A44	4⅜	4⅜	1½	—	4-10d	—	4-10d	2490	625	295	750	295
A66	5⅛	5⅛	1½	2-⅜	—	2-⅜	—	N/A	N/A	N/A	N/A	N/A
A88	8	8	2	3-⅜	—	3-⅜	—	N/A	N/A	N/A	N/A	N/A
A24	3½	2	2½	1-½	—	1-½	2-10d	N/A	N/A	N/A	N/A	N/A
A311	11	3⅜	2	1-½	—	1-½	4-10d	N/A	N/A	N/A	N/A	N/A

Model No.	Ga	Dimensions W	H	B	TF	Fasteners[1] (Total)	Avg Ult	Allowable[2] Download (125)
Z2	20	2⅜₁₆	1½	1⅜	1⅜	4-10dx1½	1507	465
Z4	12	1½	3½	2⅛	1¼	2-16d	1450	465
Z6	12	1½	5⅛	2	1⅜	2-16d	1517	485
Z28	28	2⅜₁₆	1½	1⅜	1⅜	10dx1½[3]	—	—
Z38	28	2⅜₁₆	2½	1⅜	1⅜	10dx1½[3]	—	—
Z44	12	2½	3½	2	1⅜	4-16d	2800	865

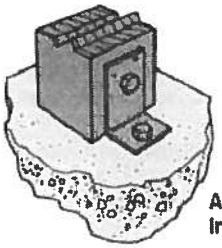
1. Z28 and Z38 do not have nail holes. Fastener quantities are as required.
2. Allowable loads have been increased 25% for roof loading (Z clips), 33% and 60% for earthquake or wind loading (A angles); no further increase allowed; reduce for other load durations according to the code.
3. Z4 and Z6 loads apply with a nail into the top and a nail into the seat.

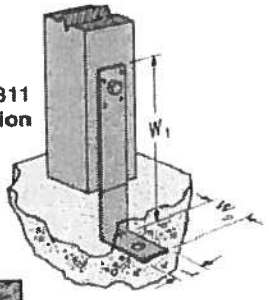


A44 Installation (A33 similar)

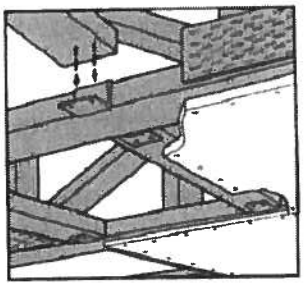


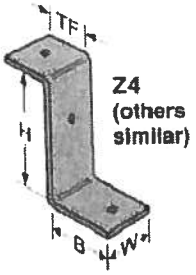
A21/A23 Installation


A24 Installation



A311 Installation


Typical Z2 Installation



Z4 (others similar)

SP/SPH/RSP4 STUD PLATE TIES

The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.

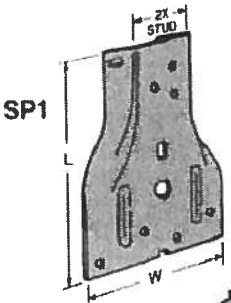
MATERIAL: SPH–18 gauge, all others–20 gauge **FINISH:** Galvanized

INSTALLATION: • Use all specified fasteners; see General Notes.
• SP–one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.

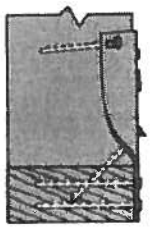
CODES: BOCA, ICBO, SBCCI NER-432, NER-443, NER-499; SBCCI 9603A; City of LA RR 25318 (RSP4); Dade Co. FL 99-0623.04 (SP1, SP2, SP4, SP6, SP8).

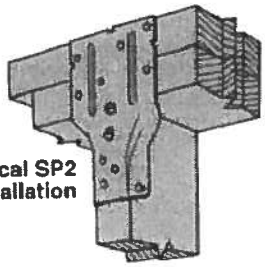
Model No.	Dimensions W	L	Fasteners Stud[1]	Plate	Avg Ult	Allowable Uplift Loads DF/SP (133)[2]	(160)[2]
SP1	3½	5⅛	6-10d	4-10d	1950	585	585
SP2	3½	6⅝	6-10d	6-10d	3300	890	1065
SP3	4½	6⅝	6-10d	6-10d	3467	890	1065
SP4	3⅛	7¼	6-10dx1½	—	2917	735	885
SP5	4½	5⅛	6-10d	4-10d	1950	585	585
SP6	5⅛	7¼	6-10dx1½	—	2917	735	885
SP8	7⅛	8⅝	6-10dx1½	—	2917	735	885
SPH4	3⅝	8¾	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH6	5⅛	9¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH8	7⅛	8¾	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
RSP4 (1)	2⅛	4½	4-8dx1½	4-8dx1½	1032	315	315
RSP4 (2)	2⅛	4½	4-8dx1½	4-8dx1½	1445	450	450

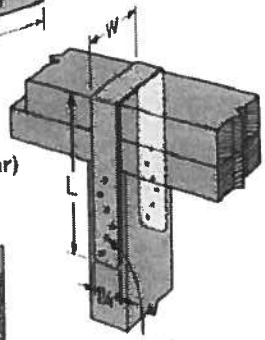
1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).
2. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
3. RSP4–see Installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement



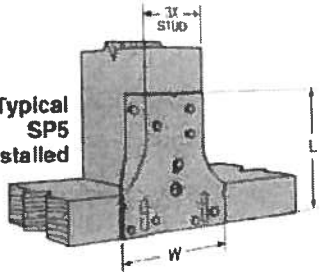
SP1


SP1 Nailing Profile

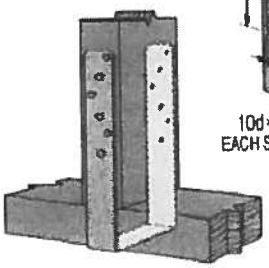

Typical SP2 Installation

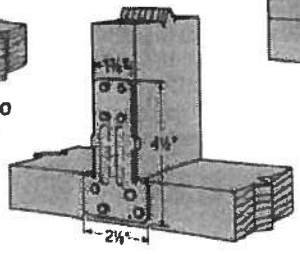


Typical SPH Installation (SP4, 6, 8 similar)



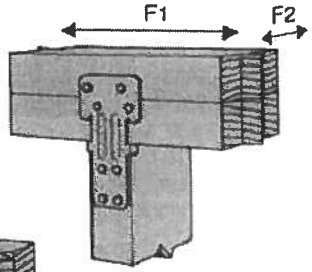
Typical SP5 Installed


Typical SPH4 Stud to Single Bottom Plate



(1) Typical RSP4 Stud to Single Bottom Plate

U.S. Patent 5,697,725



(2) Typical RSP4 Stud to Double Top Plate (see footnote 4)

RPS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI

STRAP TIES

SIMPSON StrongTie CONNECTORS

The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling. The RPS meets UBC and City of Los Angeles code requirements for notching plates where plumbing, heating or other pipes are placed in partitions.

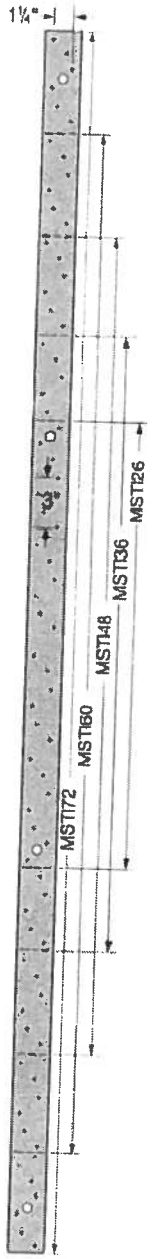
Install Strap Ties where plates or soles are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" center-to-center nail spacing reduces the possibility of splitting. For the MST, this may be a problem on lumber narrower than 3½"; either fill every nail hole with 10dx1½" nails or fill every other nail hole with 16d commons. Reduce the allowable load based on the size and quantity of fasteners used. The LSTI light strap ties are suitable where gun-nailing is necessary through diaphragm decking and wood chord open web trusses.

FINISH: HST–Simpson gray paint; PS–HDG; all others–galvanized. Some products are available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5

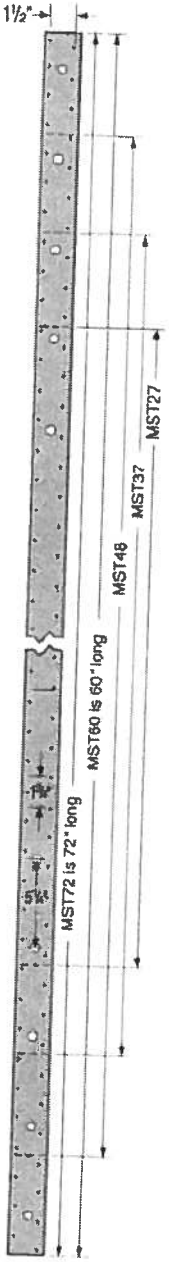
INSTALLATION: Use all specified fasteners. See General Notes.

OPTIONS: Special sizes can be made to order. See also HCST.

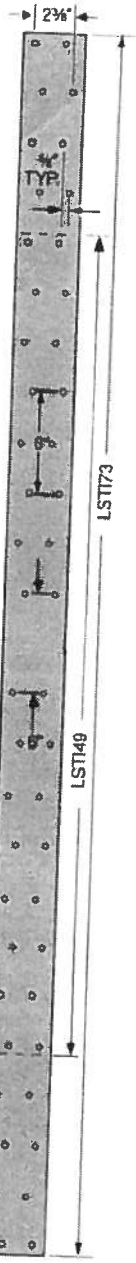
CODES: BOCA, ICBO, SBCCI NER-413, NER-443; ICBO 4935, 5357, Dade County FL. 00-1023.05 (MSTA30, MSTA36, ST12, ST18, ST22) City of L.A. RR 25119, RR 25149, RR 25281.



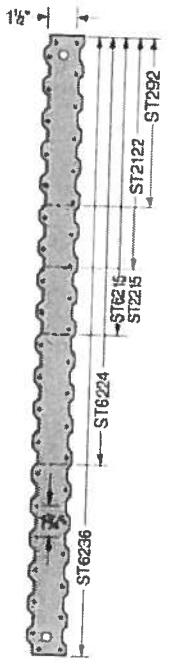
MSTI



MST

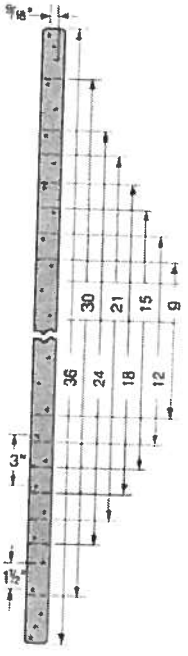


LSTI

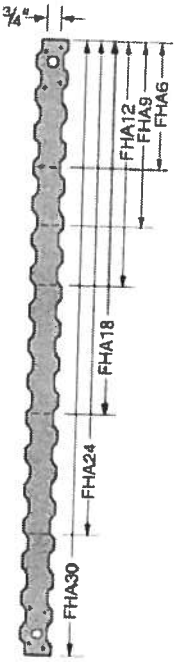


ST
U.S. Patent 4,367,973

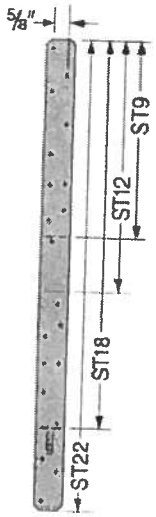
MSTAM36



LSTA and MSTA
(pilot holes not shown)

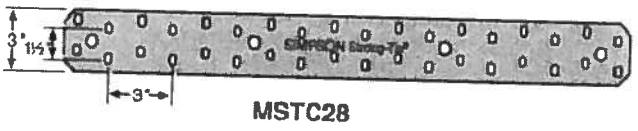


FHA

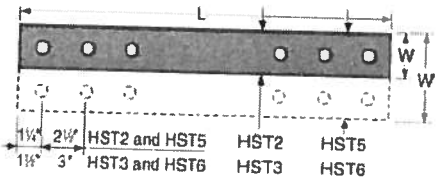


ST9, ST12, ST18, ST22

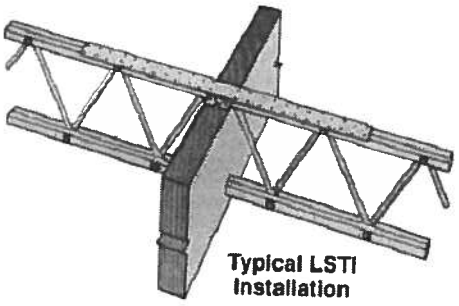
ST2115



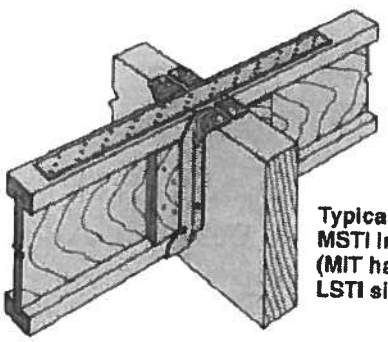
MSTC28



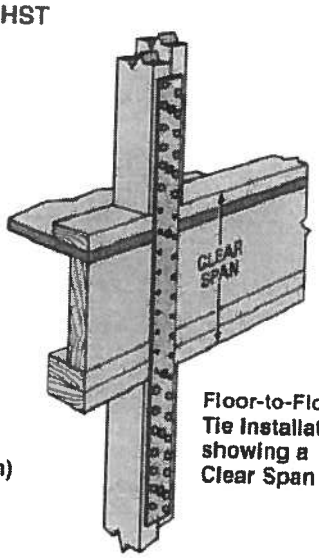
HST



Typical LSTI Installation



Typical MSTI Installation (MIT hanger shown) LSTI similar

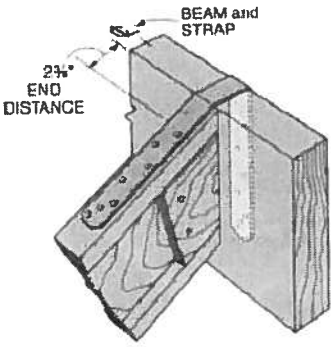


Floor-to-Floor Tie Installation showing a Clear Span

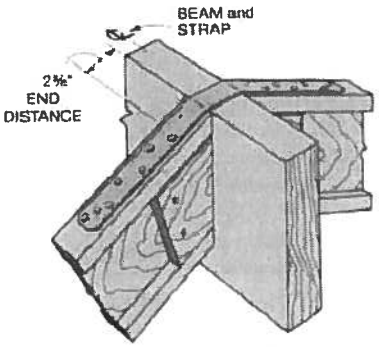
RPS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI

STRAP TIES

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Allowable Tension Loads Floor (100)	(133)	(160)
RPS18	16	1½	18⁵⁄₁₆	12-16d	810	1080	1295
RPS22		1½	22⁵⁄₁₆	16-10d	905	1205	1445
RPS28		1½	28⅛	12-16d	810	1080	1295
LSTA9	20	1¼	9	8-10d	450	605	725
LSTA12		1¼	12	10-10d	565	755	905
LSTA15		1¼	15	12-10d	680	905	1085
LSTA18		1¼	18	14-10d	790	1055	1265
LSTA21		1¼	21	16-10d	905	1205	1295
LSTA24		1¼	24	18-10d	1015	1295	1295
ST292		2⁹⁄₁₆	9⅝	12-16d	790	1055	1130
ST2122		2⁹⁄₁₆	12⁵⁄₁₆	16-16d	1070	1425	1505
ST2115		¾	16⁵⁄₁₆	10-16d	450	600	600
ST2215		2⁹⁄₁₆	16⁵⁄₁₆	20-16d	1270	1695	1695
LSTA30	18	1¼	30	22-10d	1255	1670	1715
LSTA36		1¼	36	26-10d	1480	1715	1715
LSTI49		3¾	49	32-10dx1½	1455	1940	2330
LSTI73		3¾	73	48-10dx1½	2185	2910	3495
MSTA9		1¼	9	8-10d	455	610	730
MSTA12		1¼	12	10-10d	570	760	910
MSTA15		1¼	15	12-10d	685	910	1095
MSTA18		1¼	18	14-10d	800	1065	1275
MSTA21		1¼	21	16-10d	910	1215	1460
MSTA24		1¼	24	18-10d	1025	1370	1640
MSTA30	16	1¼	30	22-10d	1265	1685	2025
MSTA36		1¼	36	26-10d	1495	1995	2135
ST6215		2⁹⁄₁₆	16⁵⁄₁₆	20-16d	1330	1775	2130
ST6224		2⁹⁄₁₆	23⅛	28-16d	1890	2520	2630
ST9		1¼	9	8-16d	530	705	850
ST12		1¼	11⅝	10-16d	665	885	1065
ST18		1¼	17¾	14-16d	900	1200	1200
ST22		1¼	21⅝	18-16d	1025	1370	1370
MSTC28		3	28¼	36-16d sinkers	2070	2760	3310
MSTC40		3	40¼	52-16d sinkers	2990	3985	4740
MSTC52		3	52¼	62-16d sinkers	3555	4740	4740
MSTC66		3	65¼	76-16d sinkers	4390	5855	5855
MSTC78	14	3	77¼	76-16d sinkers	4390	5855	5855
ST6236		2⁹⁄₁₆	33⁵⁄₁₆	40-16d	2575	3430	3430
FHA6	12	1⁷⁄₁₆	6⅜	8-16d	550	735	885
FHA9		1⁷⁄₁₆	9	8-16d	550	735	885
FHA12		1⁷⁄₁₆	11⅜	8-16d	550	735	885
FHA18		1⁷⁄₁₆	17¾	8-16d	550	735	885
FHA24		1⁷⁄₁₆	23⅜	8-16d	550	735	885
FHA30		1⁷⁄₁₆	30	8-16d	550	735	885
MSTI26		2⁹⁄₁₆	26	26-10dx1½	1130	1510	1810
MSTI36		2⁹⁄₁₆	36	36-10dx1½	1565	2090	2505
MSTI48		2⁹⁄₁₆	48	48-10dx1½	2135	2850	3420
MSTI60		2⁹⁄₁₆	60	60-10dx1½	2760	3680	4415
MSTI72		2⁹⁄₁₆	72	72-10dx1½	3310	4415	4725

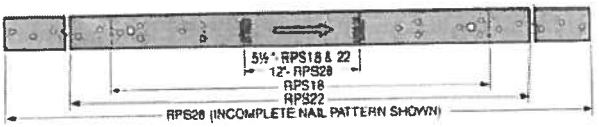


Typical LSTA installation (hanger not shown)

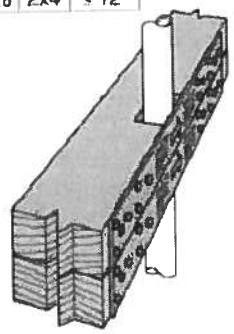


Typical LSTA installation (hanger not shown)

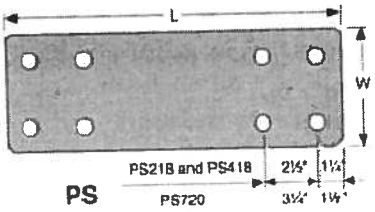
Model No.	Plate	Notch Width
RPS18	2x4	≤ 5½"
RPS22	2x6	≤ 5½"
RPS28	2x4	≤ 12"



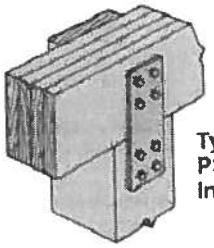
RPS



Typical RPS installation



PS



Typical PS720 installation

Model No.	Ga	Dimensions W	Dimensions L	Bolts Qty	Bolts Dia
PS218[4]	7	2	18	4	⅝
PS418[4]		4	18	4	⅝
PS720[6]		6¾	20	8	½

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Load (133)	(160)
MSTC28	18	12-16d sinker	920	1105
	16	16-16d sinker	1225	1470
MSTC40	18	28-16d sinker	2145	2575
	16	36-16d sinker	2455	2945
MSTC52	18	44-16d sinker	3375	4050
	16	48-16d sinker	3680	4415
MSTC66	18	64-16d sinker	5035	5855
	16	68-16d sinker	5350	5855
MSTC78	18	80-16d sinker	5855	5855
	16	80-16d sinker	5855	5855
MST37	18	20-16d	1905	2285
	16	22-16d	2100	2515
MST48	18	32-16d	3135	3765
	16	34-16d	3330	4000
MST60	18	46-16d	4785	5740
	16	48-16d	4990	5800
MST72	18	56-16d	5800	5800
	16	56-16d	5800	5800
MSTI36	18	14-10dx1½	810	975
	16	16-10dx1½	930	1115
MSTI48	18	26-10dx1½	1545	1855
	16	28-10dx1½	1660	1990
MSTI60	18	38-10dx1½	2330	2800
	16	40-10dx1½	2455	2945
MSTI72	18	50-10dx1½	3065	3680
	16	52-10dx1½	3190	3830

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Bolts Qty	Bolts Dia	Allowable Tension Loads: Nails Floor (100)	Nails (133)	Nails (160)	Bolts[5] Floor (100)	Bolts[5] (133)	Bolts[5] (160)
MST27	12	2⁹⁄₁₆	27	30-16d	4	⅝	2070	2760	2790	1295	1725	2070
MST37		2⁹⁄₁₆	37½	42-16d	6	½	2660	3815	3815	1625	2435	2920
MST48		2⁹⁄₁₆	48	48-16d	8	½	3345	4460	4460	2225	2970	3560
MST60	10	2⁹⁄₁₆	60	58-16d	10	½	4350	5800	5800	2670	3565	4275
MST72		2⁹⁄₁₆	72	56-16d	10	½	4350	5800	5800	2670	3565	4275
HST2	7	2½	21¼	—	6	⅝	—	—	—	3130	4175	5005
HST5		5	21¼	—	12	⅝	—	—	—	6385	8510	10210
HST3	3	3	25½	—	6	¾	—	—	—	4645	6195	7435
HST6		6	25½	—	12	¾	—	—	—	9350	12465	14955

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed. Floor loads may not be increased for other load durations.
2. 10dx1½" nails may be substituted where 16d sinkers are specified at 0.80 of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers (9 gauge x 3¼") or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
5. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: MST–2½"; HST2 and HST5–4". HST3 and HST6–4½".
6. PS strap design loads must be determined by the building designer for each installation. Bolts are installed both perpendicular and parallel-to-grain.
7. Use half of the nails at each member being connected to achieve the listed loads.

Locking prongs inserts into concrete. The one-piece design assures maximum strength.

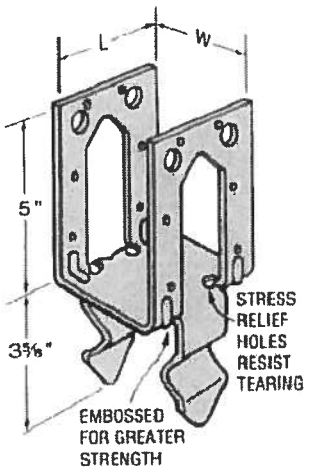
MATERIAL: 12 gauge. **FINISH**: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.
- Holes are provided for installation with either 16d commons or ½" bolts for PB66 and PB66R; all other models use 16d commons only.
- A 2" minimum sidecover is required to obtain the full load.
- Not recommended for non-top-supported installations such as fences.

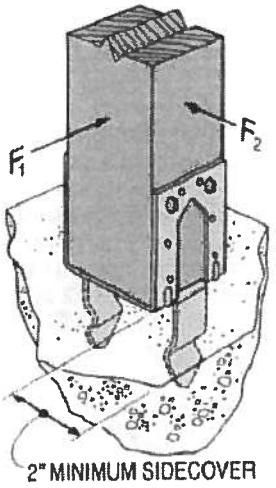
CODES: BOCA, ICBO, SBCCI NER-443; City of LA RR 25149; **Dade Co. 00-0512.11 (PB44)**.

Model No.	Dimensions W	Dimensions L	Uplift Avg Ult	Allowable Loads 12-16d Nails (133 & 160) Uplift	F_1	F_2	2-½MB Uplift (133 & 160)
PB44	3⅜	3¼	4267	1365	765	1325	—
PB44R	4	3¼	4267	1365	765	1325	—
PB46	5½	3¼	4267	1365	765	1325	—
PB46R	6	3¼	4267	1365	765	1325	—
PB66	5½	5¼	5143	1640	765	1325	1640
PB66R	6	5¼	5143	1640	765	1325	1640

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading, with no further increase allowed.



PB



Typical PB Installation

AC/LPC/LCE POST CAPS

The LCE4's universal design provides high capacity while eliminating the need for rights and lefts.

The AC MAX design allows for higher load capacity to match comparable post bases.

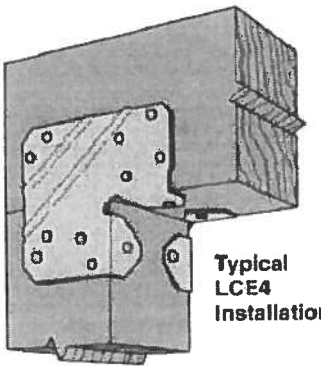
LPC—Adjustable design allows greater connection versatility.

MATERIAL: LCE4—20 ga; AC, ACE, LPC4—18 ga; LPC6—16 ga

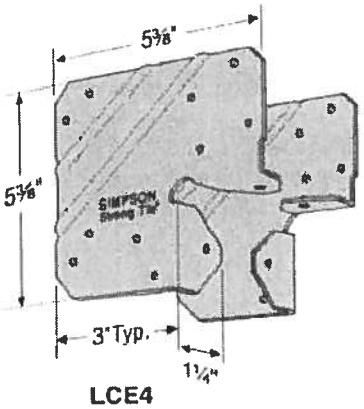
FINISH: Galvanized. Some products available with Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION: • Use all specified fasteners. See General Notes.
- Install all models in pairs. LPC—2½" beams may be used if 10dx1½" nails are substituted for 10d commons.

CODES: BOCA, ICBO, SBCCI NER-421, NER-443, NER-469; City of L.A. RR 25076; Dade County, FL 99-0623.04 (LPC) and Dade County, FL 99-0713.05 (AC, ACE).



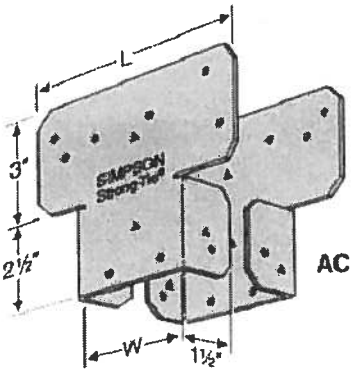
Typical LCE4 Installation



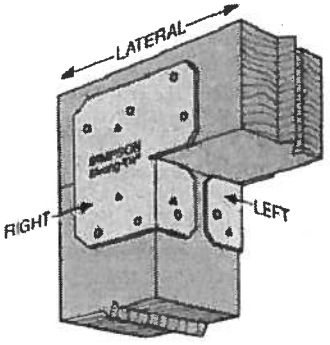
LCE4

Model No.	Dimensions W	Dimensions L	Total No. Fasteners Beam	Total No. Fasteners Post	Uplift Avg Ult	Allowable Loads (133 & 160)[1] Uplift	Allowable Loads (133 & 160)[1] Lateral
AC4 MIN	3⅜	6½	12-16d	6-16d	4467	1430	715
AC4 MAX	3⅜	6½	14-16d	14-16d	10000	2500	1070
AC4R MIN	4	7	12-16d	8-16d	4467	1430	715
AC4R MAX	4	7	14-16d	14-16d	10000	2500	1070
ACE4 MIN	—	4½	8-16d	6-16d	4215	1070	715
ACE4 MAX	—	4½	10-16d	10-16d	6238	1785	1070
AC6 MIN	5½	8½	12-16d	8-16d	4467	1430	715
AC6 MAX	5½	8½	14-16d	14-16d	10000	2500	1070
AC6R MIN	6	9	12-16d	8-16d	4467	1430	715
AC6R MAX	6	9	14-16d	14-16d	10000	2500	1070
ACE6 MIN	—	6½	8-16d	6-16d	4537	1070	715
ACE6 MAX	—	6½	10-16d	10-16d	6432	1785	1070
LPC4	3⅜	3⅜	8-10d	8-10d	2333	760	325
LPC6	5⅜	5½	8-10d	8-10d	2817	915	490
LCE4	—	5⅜	14-16d	10-16d	5518	1800	1425

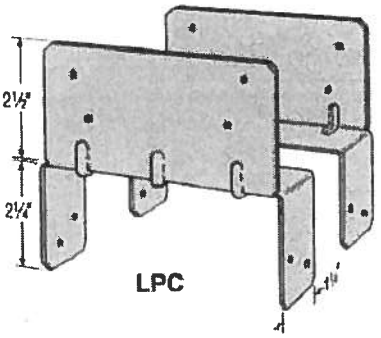
1. Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce for other load durations according to the code.
2. Loads apply only when used in pairs.
3. LPC lateral load is in the direction of the beam's axis.
4. MIN nailing quantity and load values – fill all round holes; MAX nailing quantities and load values – fill round and triangle holes.



AC



Typical ACE Installation



LPC

AB/ABA/ABE/ABU/PBS ADJUSTABLE AND STANDOFF POST BASES

SIMPSON Strong-Tie

The AB is a fully-adjustable post base which offers moisture protection and finished hardware appearance.

Post Bases provide tested capacity. They feature 1" standoff height above concrete floors, code-required when supporting permanent structures that are exposed to the weather or water splash, or in basements. They reduce the potential for decay at post and column ends.

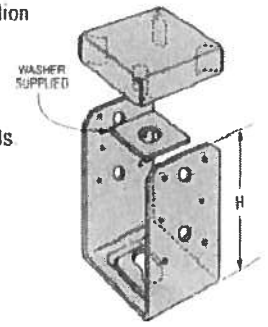
MATERIAL: AB—12 ga plates; 16 ga base cover; all others—see table.

FINISH: Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 5.

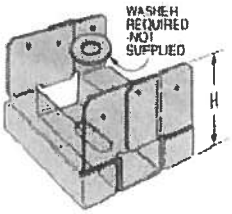
INSTALLATION: • Use all specified fasteners.See General Notes.

- Not recommended for non-top-supported installations such as fences.
- PBS embed into wet concrete up to the bottom of the 1" standoff base plate. A 2" minimum side cover is required to obtain the full load for PBS. Holes in the bottom of the PBS straps allow for free concrete flow.
- AB—Post nail holes are sized for 10d commons. Rectangular adjustment plate assumes ½" dia anchorage. Supplied as shown; position the post, secure the easy-access nut, then bend up the fourth side.
- AB, ABA, ABE and ABU—for pre-pour installed anchors. For epoxy or wedge anchors, select and install according to anchor manufacturer's recommendations; anchor diameter shown in table. Install required washer, which is not included for ABAs.
- See Simpson Anchor Systems for tested, load-rated anchors.

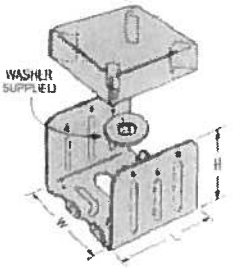
CODES: BOCA, ICBO, SBCCI NER-393, NER-422, NER-432, NER-469, NER-499; **ICBO 5670**; City of L.A. RR 24818, RR 25064, 25074, 25158; Dade Co FL. 99-0713.05 (ABA, ABE), **00-0512.11 (ABU)**.



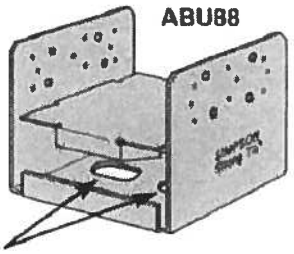
ABU44
(other sizes similar)



ABA44
(other sizes similar)
U.S. Patent 5,333,435

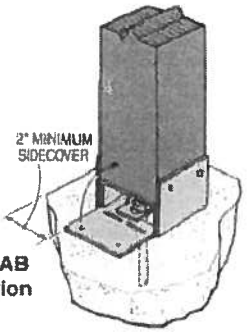


ABE44
ABE46,46R,66 and 66R supplied with rectangular washer

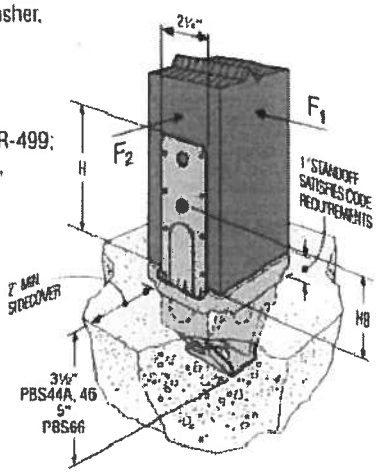


ABU88

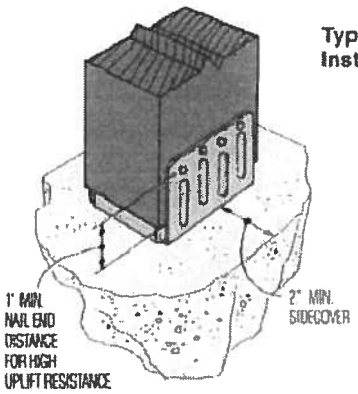
2 load transfer plates supplied



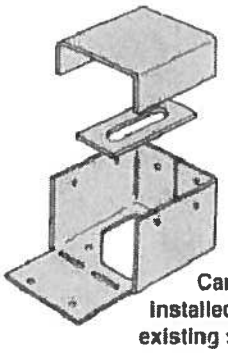
Typical AB Installation



Typical PBS44A Installation



Typical ABE46R Installation for rough lumber (ABE similar)



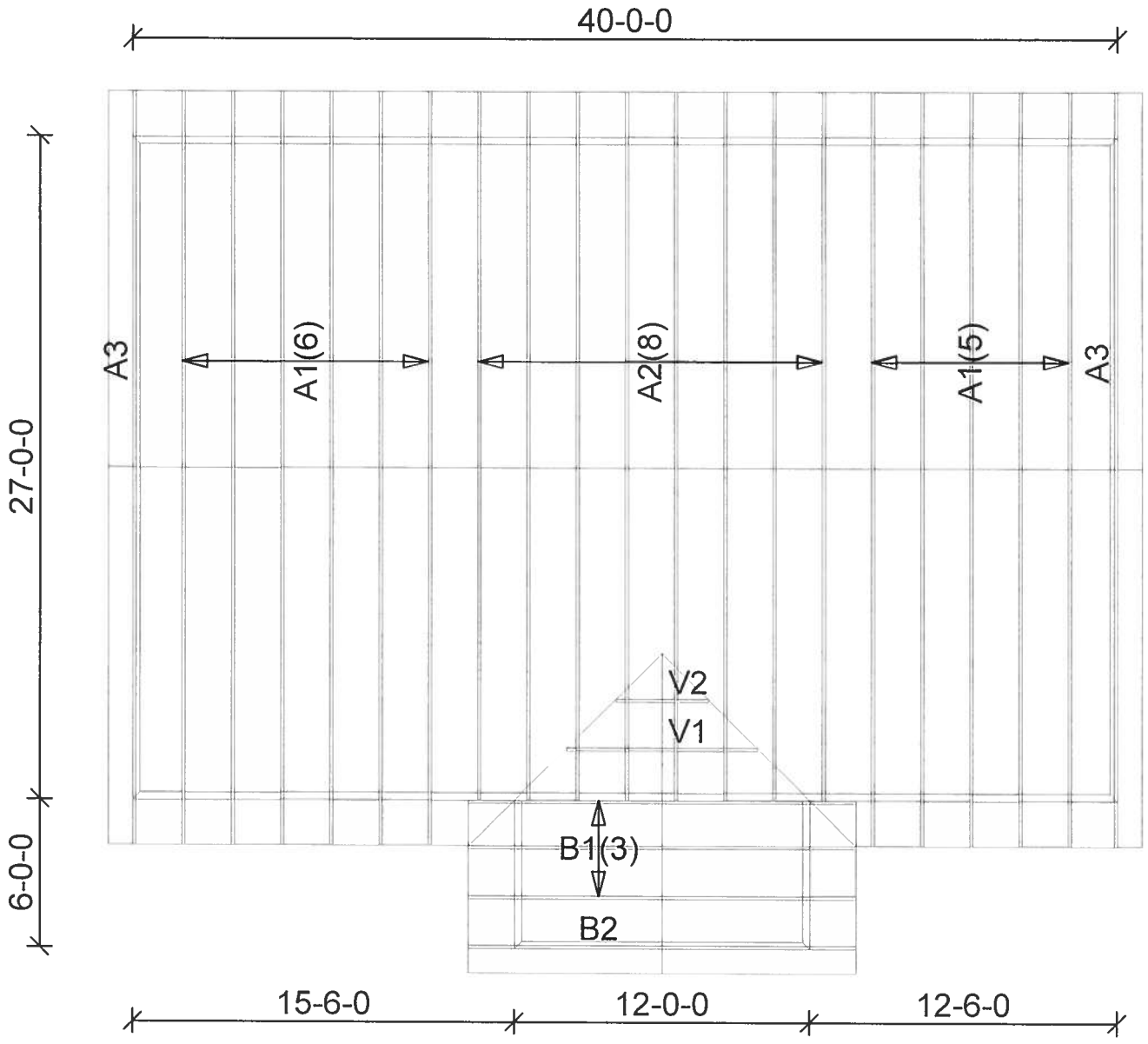
AB Can be installed on existing slab

Model No.	Dimensions W	Dimensions L	Allowable Downloads (100)
AB44	3⁵⁄₁₆	3⁵⁄₁₆	4065
AB44R	4	4⅛	4065
AB46	3⁵⁄₁₆	5⅜	4165
AB46R	4	6	4165
AB66	5½	5⁵⁄₁₆	5335
AB66R	6	6	5335

1. Loads may not be increased for short-term loading.

Model No.	Nominal Post Size	Material Base (Ga)	Material Strap (Ga)	Dimensions W	L	H	HB	Fasteners Anch. Dia	Post Nails	Post Bolts Qty	Post Bolts Dia	Uplift Avg Ult	Allowable Loads Uplift (133) Nails	Uplift (133) Bolts	Uplift (160) Nails	Uplift (160) Bolts	F_1 (133 & 160) Nails	F_1 (133 & 160) Bolts	F_2 (133 & 160) Nails	F_2 (133 & 160) Bolts	Down (100)
ABA44	4x4	16	16	3⁵⁄₁₆	3⅛	3¹¹⁄₁₆	—	½	6-10d	—	—	2120	555	—	555	—	—	—	—	—	6000
ABE44	4x4	16	16	3⁵⁄₁₆	3½	2¾	—	½	6-10d	—	—	1893	520	—	520	—	—	—	—	—	6665
ABU44	4x4	16	12	3⁵⁄₁₆	3	5⅛	1¼	⅝	12-16d	2	½	7833	2200	1800	2200	2160	—	—	—	—	6665
PBS44A	4x4	12	14	3⁵⁄₁₆	2¼	6¼	3¹⁄₁₆	—	14-16d	2	⅝	7733	2400	2400	2400	2400	1165	230	885	885	6665
ABA44R	RGH 4x4	16	16	4⁵⁄₁₆	3⅛	2¹³⁄₁₆	—	⅝	6-10d	—	—	2120	555	—	555	—	—	—	—	—	8000
ABE44R	RGH 4x4	16	16	4	3½	2⅜	—	½	6-10d	—	—	1893	400	—	400	—	—	—	—	—	6665
ABE46	4x6	12	16	3⁵⁄₁₆	5⁵⁄₁₆	4¹⁄₁₆	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
PBS46	4x6	12	14	3⁵⁄₁₆	2¼	6⁵⁄₁₆	3⅜	—	14-16d	2	½	7733	2400	2400	2400	2400	1165	360	885	885	9335
ABA46	4x6	14	14	3⁵⁄₁₆	5⁵⁄₁₆	3⅛	—	⅝	8-16d	—	—	2967	700	—	700	—	—	—	—	—	9435
ABU46	4x6	12	12	3⁵⁄₁₆	5	7	2⅛	⅝	12-16d	2	½	8633	2255	2300	2300	2300	—	—	—	—	10335
ABE46R	RGH 4x6	12	16	4⁵⁄₁₆	5⁵⁄₁₆	3¹³⁄₁₆	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
ABA46R	RGH 4x6	14	14	4⁵⁄₁₆	5⁵⁄₁₆	2⅞	—	⅝	8-16d	—	—	2967	935	—	935	—	—	—	—	—	12000
PBS66	6x6	12	12	5½	2¼	6½	3¹⁄₁₆	—	14-16d	2	⅞	13100	2630	3560	3160	4000	1865	570	1700	1700	9335
ABA66	6x6	14	14	5½	5¼	3⅛	—	⅝	8-16d	—	—	3050	720	—	720	—	—	—	—	—	10665
ABE66	6x6	12	14	5½	5⁵⁄₁₆	3⅛	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU66	6x6	12	10	5½	5	6⅞	1¼	⅞	12-16d	2	½	8900	2300	2300	2300	2300	—	—	—	—	12000
ABA66R	RGH 6x6	14	14	6	5⁵⁄₁₆	2⅞	—	⅝	8-16d	—	—	3050	985	—	985	—	—	—	—	—	12665
ABE66R	RGH 6x6	12	14	6¹⁄₁₆	5⁵⁄₁₆	2⅞	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU88[4]	8x8	12	14	7½	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335
ABU88R[4]	RGH 8x8	12	14	8	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335

1. Uplift and lateral loads have been increased 33% and 60% for earthquake or wind loading, no further increase allowed. Reduce by 33% and 60% for normal loading.
2. Downloads may not be increased for short-term loading.
3. Specifier to design concrete for shear capacity.
4. ABU88 and ABU88R may be installed with 8-SDS¼X3 wood screws for the same table load.



Mayo Truss Co. Inc.
362 NE CLYDE AVE.
MAYO, FL 32066
(386)294-3988
(877)-558-6262

HAYGOOD HOMES

SHAWN FULWOOD

110 MPH ASCE WIND LOAD

Roof Loading
TC Live: 20.00 psf
TC Dead: 10.00 psf
BC Live: 0.00 psf
BC Dead: 10.00 psf
TC Stress Inc: 25.00
BC Stress Inc: 25.00
Spacing: 2- 0- 0 o.c.

Account: CONTRACTORS
Job: HAYGOOD-FULWOO
Designer: M.MURRAY
Checker: M.MURRAY
Date: 04-10-06

Permit Number: _____ Lot Number: _____

Miscellaneous: _____ Address: _____

The information in this box is for administrative purposes only and is not part of the engineering review.

Truss Fabricator:Mayo Truss Company, Inc

Job Reference:HAYGOOD-FULWOOD - FULWOOD RESIDENCE

ROBBINS ENGINEERING, INC.	P.O. Box 280055 Tampa, FL 33682-0055 Phone: (813) 972-1135	Engineering Index Sheet Index Page 1 of 1

Job Number	Date	FBC - 2004 Chapter 16 and 23	Specification Quantity
T06040791	04/07/2006		7

A Professional Engineer's seal affixed to this Index Sheet indicates the acceptance of Professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibilty of the Building Designer, as defined in ANSI/TPI 1-1995, Section 2.2. Permanent files of the original Truss Specificstion Sheet are maintained by Robbins Engineering, Inc. Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Robbins Engineering, Inc. (Sofware - Online Plus)

Standard Loading:

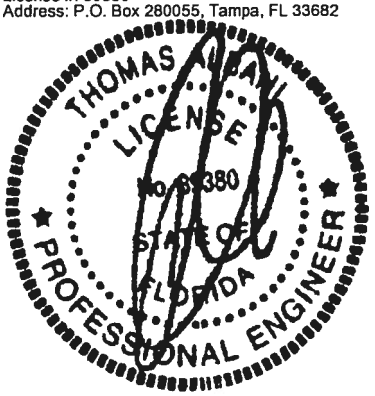
T.C. Live	20 psf
T.C Dead	10 psf
B.C Live	0 psf
B.C. Dead	10 psf
Total	40 psf

ANSI/ASCE 7-02
Wind Speed - 110 mph
Mean Roof Ht. - 15 ft.
Exposure Catergory - B
Occupancy Factor - 1.00
MWFRS
Enclosed

Notes: Refer to individual truss design drawings for special loading conditions.

	Date	Mark		Date	Mark		Date	Mark		Date	Mark
1	04/07/06	A1	2	04/07/06	A2	3	04/07/06	A3	4	04/07/06	B1
5	04/07/06	B2	6	04/07/06	V1	7	04/07/06	V2			

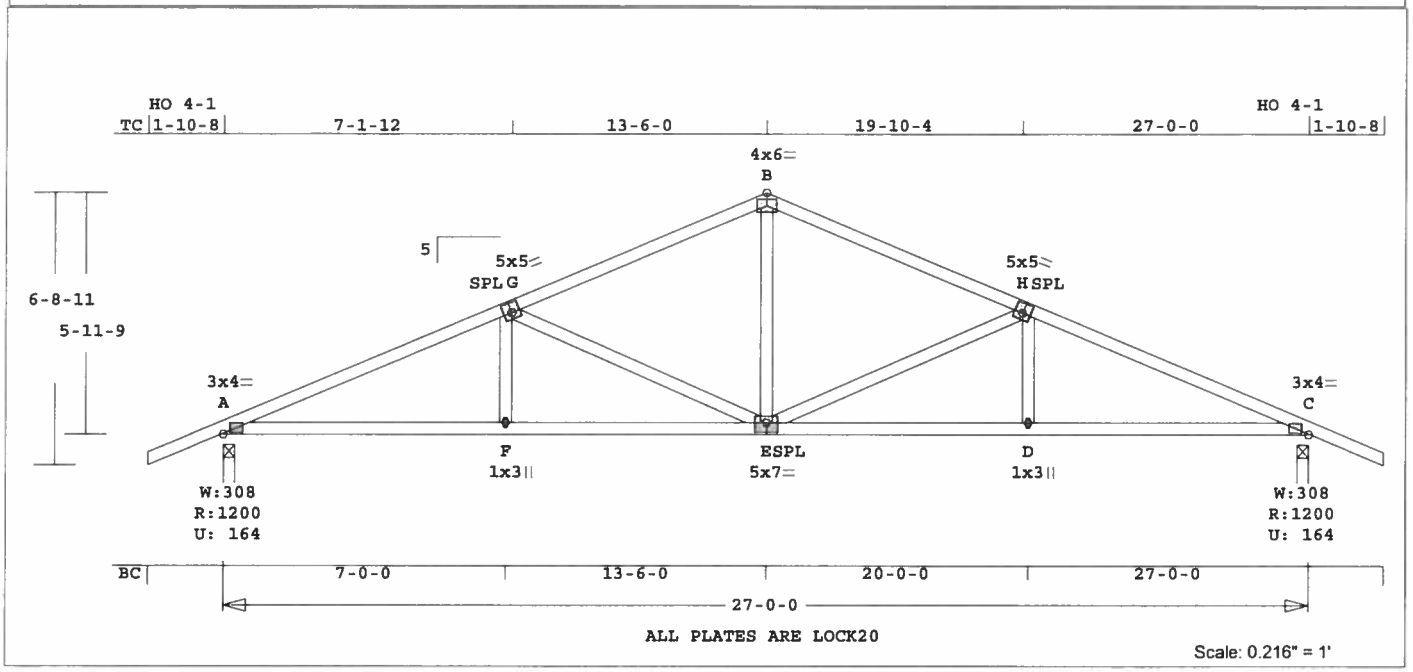
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 4/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-FULWOOD	A1	11	TR	270000	5	1-10- 8	1-10- 8	T06040791

U# J#HAYGOOD-FULWOOD FULWOOD RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 160.3 LBS

Online Plus -- Version 19.0.011
RUN DATE: 07-APR-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.47	2x 4	SP-#2
WB	0.44	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	27- 0- 0
BC	Cont.	0- 0- 0	27- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1200	165	3- 8	1- 8
			Hz =	-98
C	1200	165	3- 8	1- 8
			Hz =	99

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -G	0.43	2099 C	0.02	0.41
G -B	0.42	1412 C	0.01	0.41
B -H	0.42	1412 C	0.01	0.41
H -C	0.43	2099 C	0.02	0.41
--------Bottom Chords---------				
A -F	0.47	1944 T	0.32	0.15
F -E	0.45	1944 T	0.32	0.13
E -D	0.45	1944 T	0.32	0.13
D -C	0.47	1944 T	0.32	0.15
------------Webs------------				
F -G	0.04	271 T		
G -E	0.44	709 C		
E -B	0.13	715 T		
E -H	0.44	709 C		
D -H	0.04	271 T		

TL Defl -0.19" in E -D L/999
LL Defl -0.08" in E -D L/999
Shear // Grain in A -G 0.25

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.91
G	LOCK	5.0x 5.0	-0.2	0.5	0.65
B	LOCK	4.0x 6.0	Ctr	Ctr	0.65
H	LOCK	5.0x 5.0	0.2	0.5	0.65
C	LOCK	3.0x 4.0	Ctr	Ctr	0.91
F	LOCK	1.0x 3.0	Ctr	Ctr	0.81
E	LOCK	5.0x 7.0	Ctr	-0.5	0.65
D	LOCK	1.0x 3.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

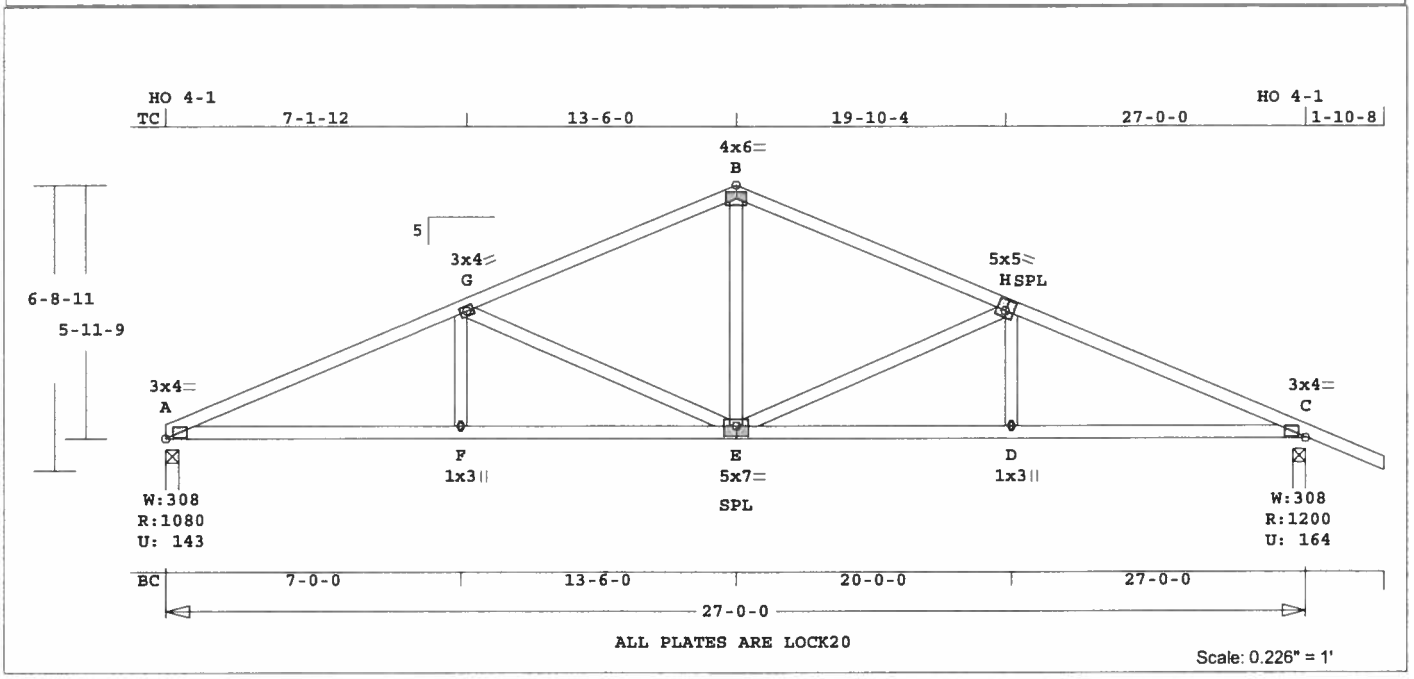
NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 2099 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 4/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-FULWOOD	A2	8	TR	270000	5	0	1-10- 8	T06040791

U# J#HAYGOOD-FULWOOD FULWOOD RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 156.5 LBS

Online Plus -- Version 19.0.011
RUN DATE: 07-APR-06

	CSI	-Size-	----Lumber----
TC	0.43	2x 4	SP-#2
BC	0.47	2x 4	SP-#2
WB	0.44	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	27- 0- 0
BC	Cont.	0- 0- 0	27- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	1080	144	3- 8	1- 8
			Hz =	-98
C	1200	165	3- 8	1- 8
			Hz =	99

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -G	0.43	2099	C 0.02	0.41
G -B	0.42	1412	C 0.01	0.41
B -H	0.42	1412	C 0.01	0.41
H -C	0.43	2099	C 0.02	0.41
--------Bottom Chords---------				
A -F	0.47	1944	T 0.32	0.15
F -E	0.45	1944	T 0.32	0.13
E -D	0.45	1944	T 0.32	0.13
D -C	0.47	1944	T 0.32	0.15
----------Webs----------				
F -G	0.04	271	T	
G -E	0.44	709	C	
E -B	0.13	715	T	
E -H	0.44	709	C	
D -H	0.04	271	T	

TL Defl -0.19" in F -E L/999
LL Defl -0.08" in F -E L/999
Shear // Grain in A -G 0.25

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

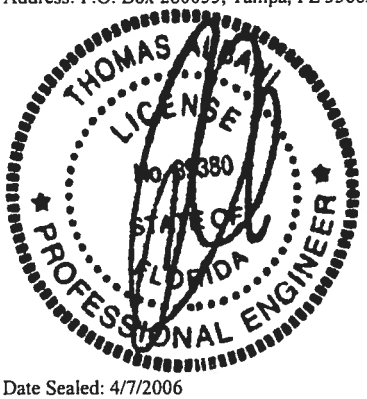
Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.91
G	LOCK	3.0x 4.0	Ctr	Ctr	0.64
B	LOCK	4.0x 6.0	Ctr	Ctr	0.65
H	LOCK	5.0x 5.0	0.2	0.5	0.65
C	LOCK	3.0x 4.0	Ctr	Ctr	0.91
F	LOCK	1.0x 3.0	Ctr	Ctr	0.81
E	LOCK	5.0x 7.0	Ctr	-0.5	0.65
D	LOCK	1.0x 3.0	Ctr	Ctr	0.81

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 2099 Lbs
Quality Control Factor 1.25

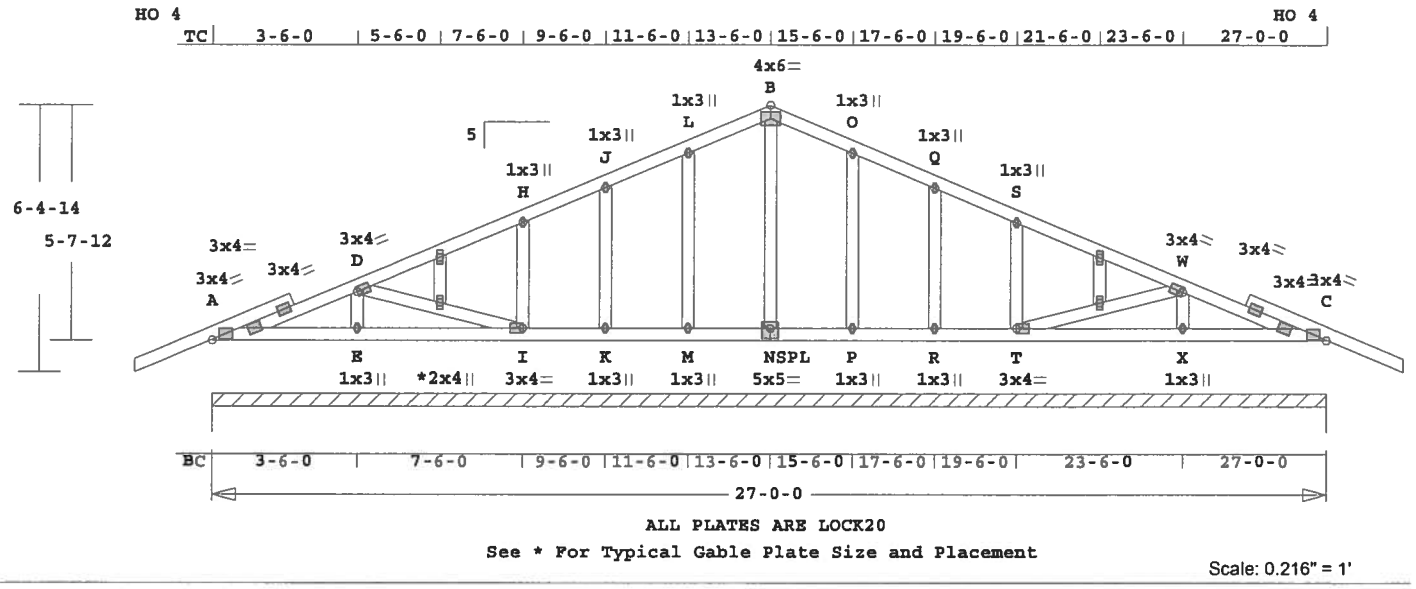
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 4/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-FULWOOD	A3	2	TR	270000	5	0	0	T06040791

U# J#HAYGOOD-FULWOOD FULWOOD RESIDENCE



ALL PLATES ARE LOCK20

See * For Typical Gable Plate Size and Placement

Scale: 0.216" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 190.3 LBS

Online Plus -- Version 19.0.011
RUN DATE: 07-APR-06

	CSI	-Size-	----Lumber----
TC	0.13	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
WB	0.01	2x 4	SP-#2
GW	0.03	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	27- 0- 0
BC	Cont.	0- 0- 0	27- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 6 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0	to 27- 0- 0		
	2160	288	Hz =	93

Membr	CSI	P	Lbs	Axl-CSI-Bnd
----------Top Chords----------				
A -D	0.13	95	C 0.00	0.13
D -H	0.13	39	C 0.00	0.13
H -J	0.11	40	C 0.00	0.11
J -L	0.03	41	T 0.00	0.03
L -B	0.03	69	T 0.00	0.03
B -O	0.03	69	T 0.00	0.03
O -Q	0.03	41	T 0.00	0.03
Q -S	0.11	40	C 0.00	0.11
S -W	0.13	39	C 0.00	0.13
W -C	0.13	95	C 0.00	0.13
--------Bottom Chords---------				
A -E	0.07	3	T 0.00	0.07
E -I	0.07	0	T 0.00	0.07
I -K	0.06	0	T 0.00	0.06
K -M	0.02	0	T 0.00	0.02
M -N	0.02	0	T 0.00	0.02
N -P	0.02	0	T 0.00	0.02
P -R	0.02	0	T 0.00	0.02
R -T	0.06	0	T 0.00	0.06
T -X	0.07	0	T 0.00	0.07
X -C	0.07	3	T 0.00	0.07
----------Webs----------				
D -I	0.01	76	C	
T -W	0.01	76	C	
----------Gable Webs----------				
E -D	0.01	188	C	
I -H	0.02	201	C	
K -J	0.01	91	C	
M -L	0.03	129	C	
N -B	0.03	86	C	
P -O	0.03	129	C	
R -Q	0.01	91	C	
T -S	0.02	201	C	
X -W	0.01	188	C	

TL Defl -0.01" in E -I L/999
LL Defl 0.00" in E -I L/999
Shear // Grain in D -H 0.14

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.91
D	LOCK	3.0x 4.0	Ctr	Ctr	0.64
H	LOCK	1.0x 3.0	Ctr	Ctr	0.79
J	LOCK	1.0x 3.0	Ctr	Ctr	0.79
L	LOCK	1.0x 3.0	Ctr	Ctr	0.79
B	LOCK	4.0x 6.0	Ctr	Ctr	0.65
O	LOCK	1.0x 3.0	Ctr	Ctr	0.79
Q	LOCK	1.0x 3.0	Ctr	Ctr	0.79
S	LOCK	1.0x 3.0	Ctr	Ctr	0.79
W	LOCK	3.0x 4.0	Ctr	Ctr	0.64
C	LOCK	3.0x 4.0	Ctr	Ctr	0.91
E	LOCK	1.0x 3.0	Ctr	Ctr	0.81
I	LOCK	3.0x 4.0	Ctr	Ctr	0.61
K	LOCK	1.0x 3.0	Ctr	Ctr	0.81
M	LOCK	1.0x 3.0	Ctr	Ctr	0.81
N	LOCK	5.0x 5.0	Ctr	-0.5	0.65
P	LOCK	1.0x 3.0	Ctr	Ctr	0.81
R	LOCK	1.0x 3.0	Ctr	Ctr	0.81
T	LOCK	3.0x 4.0	Ctr	Ctr	0.61
X	LOCK	1.0x 3.0	Ctr	Ctr	0.81

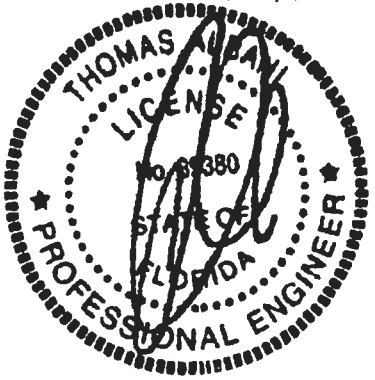
2 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Prevent truss rotation at all bearing locations.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 201 Lbs
Quality Control Factor 1.25

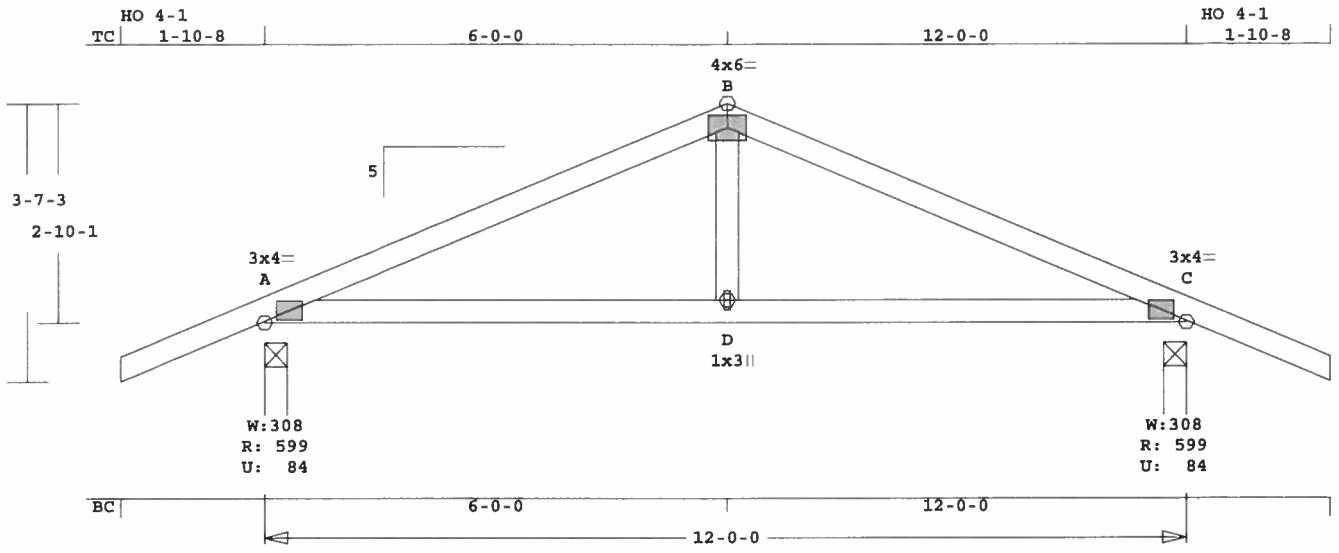
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 4/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-FULWOOD	B1	3	TR	120000	5	1-10- 8	1-10- 8	T06040791

U# J#HAYGOOD-FULWOOD FULWOOD RESIDENCE



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 59.6 LBS

Online Plus -- Version 19.0.011
RUN DATE: 07-APR-06

	CSI	-Size-	----Lumber----
TC	0.24	2x 4	SP-#2
BC	0.30	2x 4	SP-#2
WB	0.04	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 0- 0
BC	Cont.	0- 0- 0	12- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
A	600	85	3- 8	1- 8
C	600	85	3- 8	1- 8

Membr	CSI	P Lbs	Axl	CSI	Bnd
Top Chords					
A -B	0.24	683	C	0.00	0.24
B -C	0.24	683	C	0.00	0.24
Bottom Chords					
A -D	0.30	634	T	0.10	0.20
D -C	0.30	634	T	0.10	0.20
Webs					
D -B	0.04	265	T		

TL Defl -0.05" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in A -B 0.20

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.66
B	LOCK	4.0x 6.0	Ctr	Ctr	0.47
C	LOCK	3.0x 4.0	Ctr	Ctr	0.66
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

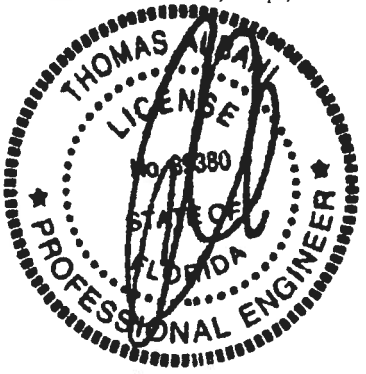
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.

Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 683 Lbs
Quality Control Factor 1.25

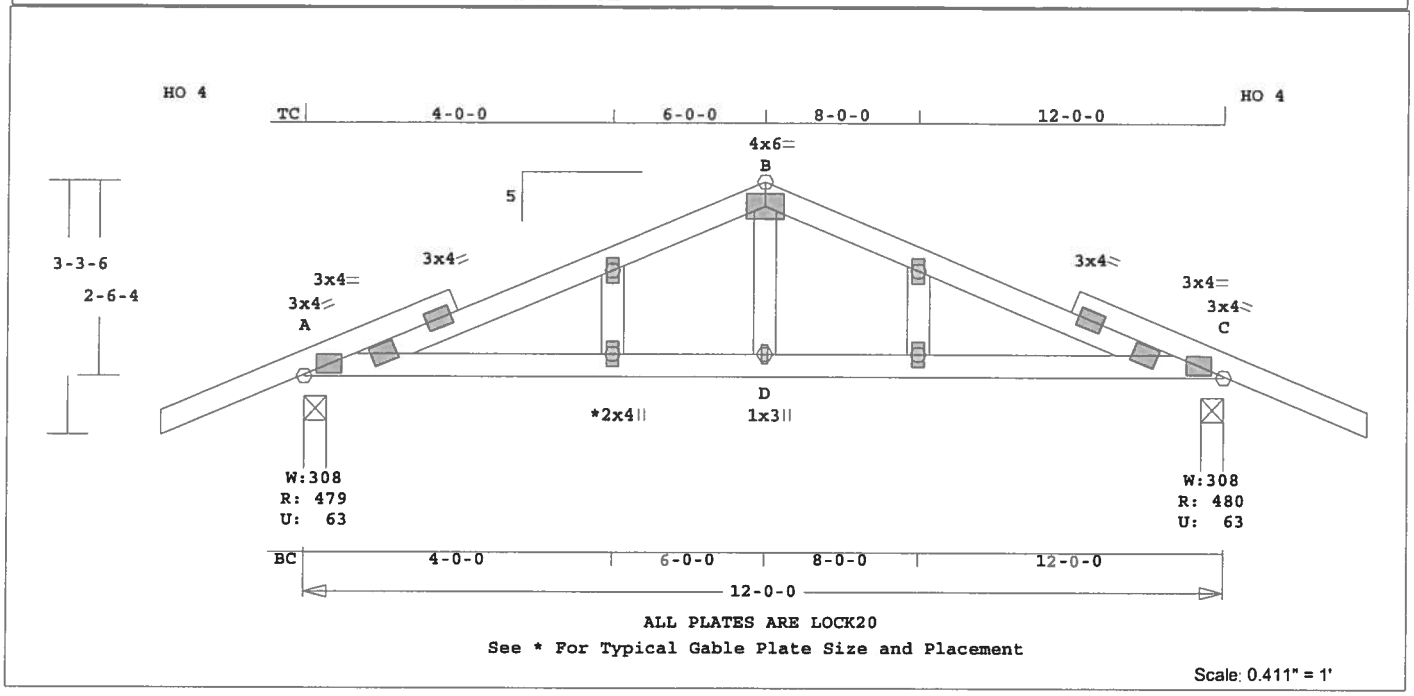
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 4/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-FULWOOD	B2	1	TR	120000	5	0	0	T06040791

U# J#HAYGOOD-FULWOOD FULWOOD RESIDENCE



ALL PLATES ARE LOCK20

See * For Typical Gable Plate Size and Placement

Scale: 0.411" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 68.3 LBS

Online Plus -- Version 19.0.011
RUN DATE: 07-APR-06

	CSI	-Size-	----Lumber----
TC	0.20	2x 4	SP-#2
BC	0.31	2x 4	SP-#2
WB	0.03	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	12- 0- 0
BC	Cont.	0- 0- 0	12- 0- 0

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	Size Req'd In-Sx
A	480	64	3- 8	1- 8
C	480	64	3- 8	1- 8

Membr	CSI	P Lbs	Axl	CSI	Bnd
----------Top Chords----------					
A -B	0.20	753	C	0.00	0.20
B -C	0.20	753	C	0.00	0.20
--------Bottom Chords---------					
A -D	0.31	717	T	0.12	0.19
D -C	0.31	717	T	0.12	0.19
------------Webs------------					
D -B	0.03	246	T		

TL Defl -0.05" in D -C L/999
LL Defl -0.02" in D -C L/999
Shear // Grain in A -B 0.19

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	3.0x 4.0	Ctr	Ctr	0.66
B	LOCK	4.0x 6.0	Ctr	Ctr	0.47
C	LOCK	3.0x 4.0	Ctr	Ctr	0.66
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

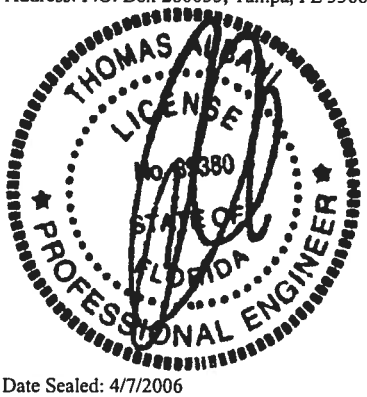
2 Gable studs to be attached with 2.0x4.0 plates each end.

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
WARNING Do Not Cut overframe member between outside of truss and first tie-plate to inside of heel plate.
Design checked for 10 psf non-concurrent LL on BC.
Refer to Gen Det 3 series for web bracing and plating.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 753 Lbs
Quality Control Factor 1.25

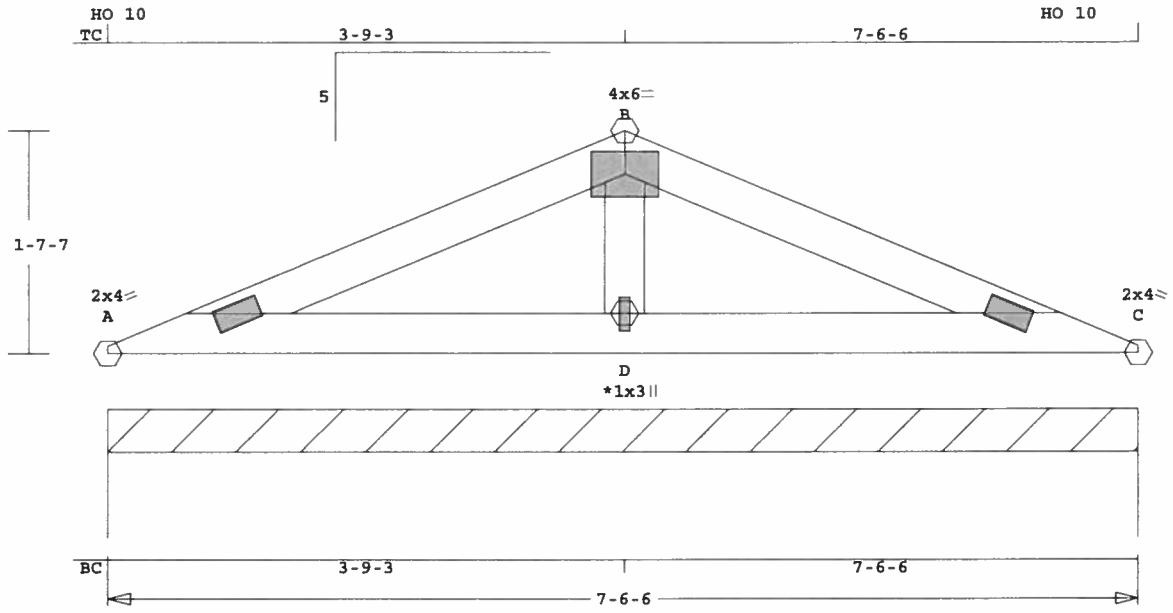
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 4/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-FULWOOD	V1	1	VL.SB	70606	5	0	0	T06040791

U# J#HAYGOOD-FULWOOD FULWOOD RESIDENCE



ALL PLATES ARE LOCK20
See Joint D For Typical Gable Plate Size and Placement

Scale: 0.733" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 29.2 LBS

Online Plus -- Version 19.0.011
RUN DATE: 07-APR-06

	CSI	-Size-	----Lumber----
TC	0.07	2x 4	SP-#2
BC	0.07	2x 4	SP-#2
GW	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	7- 6- 6
BC	Cont.	0- 0- 0	7- 6- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"

Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to		7- 6- 6	
	628	86	Hz =	47

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.07	43 C	0.00	0.07
B -C	0.07	43 C	0.00	0.07
--------Bottom Chords---------				
A -D	0.07	2 C	0.00	0.07
D -C	0.07	2 C	0.00	0.07
----------Gable Webs----------				
D -B	0.01	152 C		

TL Defl 0.00" in A -D L/999
LL Defl 0.00" in A -D L/999
Shear // Grain in A -B 0.10

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.74
B	LOCK	4.0x 6.0	Ctr	Ctr	0.42
C	LOCK	2.0x 4.0	Ctr	Ctr	0.74
D	LOCK	1.0x 3.0	Ctr	Ctr	0.75

REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02

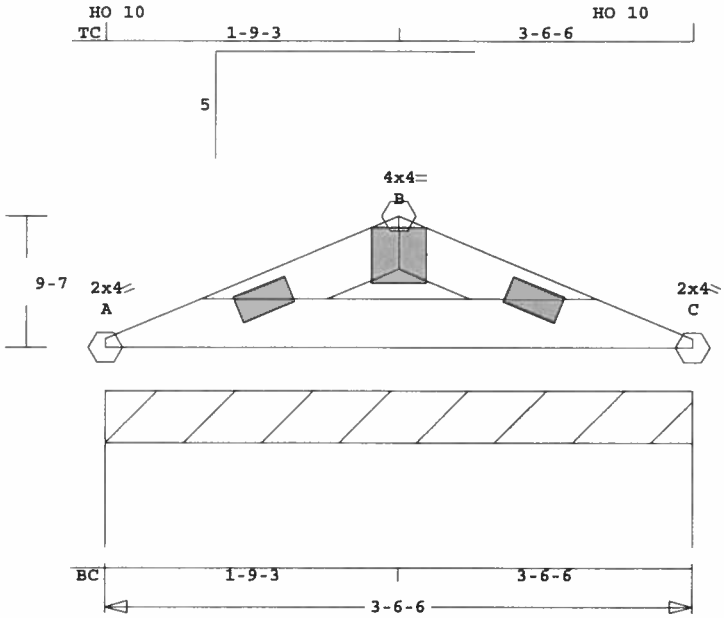
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 152 Lbs
Quality Control Factor 1.25

Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682

Date Sealed: 4/7/2006

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
HAYGOOD-FULWOOD	V2	1	VL.SB	30606	5	0	0	T06040791

U# J#HAYGOOD-FULWOOD FULWOOD RESIDENCE



Scale: 0.890" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 11.5 LBS

Online Plus -- Version 19.0.011
RUN DATE: 07-APR-06

	CSI	-Size-	----Lumber----
TC	0.01	2x 4	SP-#2
BC	0.01	2x 4	SP-#2

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	3- 6- 6
BC	Cont.	0- 0- 0	3- 6- 6

Loading	Live	Dead	(psf)
TC	20.0	10.0	
BC	0.0	10.0	
Total	20.0	20.0	40.0
Spacing			24.0"
Lumber Duration Factor			1.25
Plate Duration Factor			1.25
TC Fb=1.15	Fc=1.10	Ft=1.10	
BC Fb=1.10	Fc=1.10	Ft=1.10	

Plus 4 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Jt	React Lbs	Uplft Lbs	Size Req'd In-Sx	In-Sx
Cont.	Brg 0- 0- 0 to		3- 6- 6	
	186	25	Hz =	0

Membr	CSI	P Lbs	Axl-CSI-Bnd	
----------Top Chords----------				
A -B	0.01	107 C	0.00	0.01
B -C	0.01	107 C	0.00	0.01
--------Bottom Chords---------				
A -C	0.01	0 T	0.00	0.01

TL Defl 0.00" in B -B L/999
LL Defl 0.00" in B -B L/999
Shear // Grain in B -B 0.04

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORT: NER 691
ROBBINS ENGINEERING, INC.
BASED ON SP LUMBER
USING GROSS AREA TEST.
Plate - LOCK 20 Ga, Gross Area
Plate - RHS 20 Ga, Gross Area

Jt	Type	Plt Size	X	Y	JSI
A	LOCK	2.0x 4.0	Ctr	Ctr	0.68
B	LOCK	4.0x 4.0	Ctr	-0.9	0.33
C	LOCK	2.0x 4.0	Ctr	Ctr	0.68

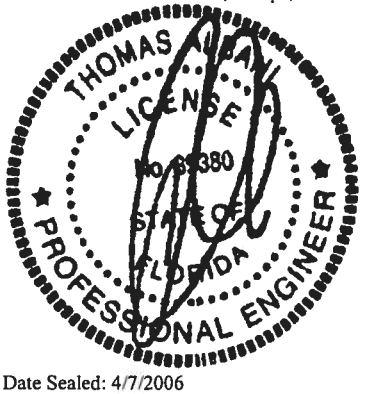
REVIEWED BY:
Robbins Engineering, Inc.
PO Box 280055
Tampa, FL 33682

REFER TO ROBBINS ENG. GENERAL NOTES AND SYMBOLS SHEET FOR ADDITIONAL SPECIFICATIONS.

NOTES:
Trusses Manufactured by:
Mayo Truss Co. Inc.
Analysis Conforms To:
FBC2004
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as a Main Wind-Force Resistance System.
Wind Speed: 110 mph
Mean Roof Height: 15-0
Exposure Category: B

Occupancy Factor : 1.00
Building Type: Enclosed
Zone location: Exterior
TC Dead Load : 5.0 psf
BC Dead Load : 5.0 psf
Max comp. force 107 Lbs
Quality Control Factor 1.25

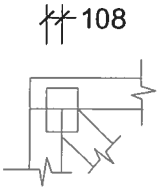
Truss Design Engineer: Thomas A. Albani
License #: 39380
Address: P.O. Box 280055, Tampa, FL 33682



Date Sealed: 4/7/2006

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE (3X2, 4X2, 6X2)

(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

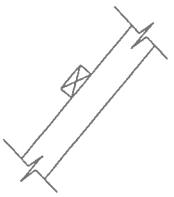
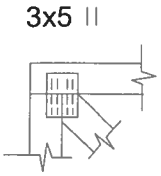


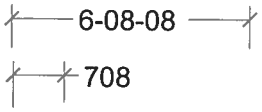
PLATE SIZE AND ORIENTATION

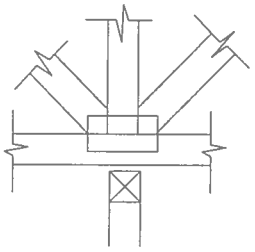


The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).





W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted.
The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA)," National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.

Robbins Engineering, Inc.

6904 Parke East Blvd.
Tampa, Fl 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbinseng.com

** LAMAR BOOZER **
900 EAST PUTNAM STREET
LAKE CITY, FL 32055

PROJECT:
CLIENT: HAYGOOD fulwood)
DATE: 4 24 06

RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS

DESIGNER: LAMAR BOOZER

CLIENT INFORMATION:

NAME: HAYGOOD fulwood)
ADDRESS:
CITY, STATE: LAKE CITY, FLORIDA

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	131	4,274	0	7,892	7,89
9-I FRENCH DOOR DBL CLR GLS METL FR	73	2,477	0	3,668	3,66
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,342	4,831	0	2,639	2,63
11-C DOOR METAL POLYSTYRENE CORE	40	846	0	462	46
16-G CEILING R-30 INSULATION	1,080	2,489	0	2,047	2,04
22-A SLAB ON GRADE NO EDGE INSUL	187	6,816	0	0	
SUBTOTALS FOR STRUCTURE:	2,853	21,733	0	16,708	16,70
PEOPLE	16	0	0	4,800	4,80
APPLIANCES	0	0	1,800	1,500	3,30
DUCTWORK	0	1,087	0	2,302	2,30
INFILTRATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	
SENSIBLE GAIN TOTAL				25,310	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		22,820	1,800	25,310	27,11

SUPPLY CFM AT 20 DEG DT: 1,150
SQUARE FT. OF ROOM AREA: 1,080

CFM PER SQUARE FOOT: 0.6E
SQUARE FOOT PER TON: 741.8E

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 22.820 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2.259 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
NO. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
7 JUNE 2002

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

ALLEN N. REEVES
CERTIFICATE
NO. 19356
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
7 JUNE 2002

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 13364 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.15"	0.41" max.
	@ 15.0 psf (negative)	0.29"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.29" max.
	@ 22.5 psf (negative)	0.01"	0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs		
	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
CERTIFICATE NO. 15334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
	Optional Performance		
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC

Mark A. Hess
Technician

MAH:nlb
01-41641.01

Allen N. Reeves, P.E.
Director - Engineering Services
7 JUNE 2002

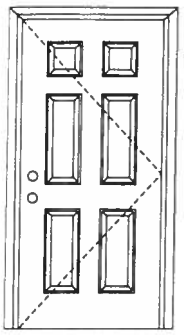
ALLEN N. REEVES
CERTIFICATE NO. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

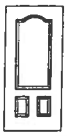
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

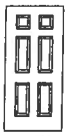
Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

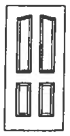

Flush

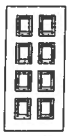

Arch Top 3-panel


3-panel


6-panel

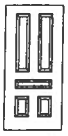

New England 4-panel

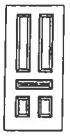

Eyebrow 4-panel

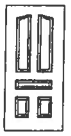

8-panel

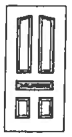

9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOOR® Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 28-4S-17-08835-002 **Building permit No.** 000024535

Use Classification SFD/UTILITY **Fire:** 18.17

Permit Holder PAT HAYGOOD **Waste:** 0.00

Owner of Building CHRISTOPHER FULWOOD **Total:** 18.17

Location: 980 SW WENDY TERR

Date: 09/13/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)