GENERAL NOTES

1. IT IS THE CONTRACTORS SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO , THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS OR TIE-DOWNS MAY BE NECESSARY.

CODES AND SPECIFICATIONS

A. GENERAL BUILDING CODE:
   1. FLORIDA BUILDING CODE 2020.

B. CONCRETE:
   1. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-14)
   2. SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301-16).

DESIGN LOADS

1. DESIGN LIVE LOADS

ROOF	20 PSF
PRIVATE ROOMS	40 PSF
BALCONY	60 PSF
CORRIDORS/LANDINGS	80 PSF
STAIRS	100 PSF
MECH ROOMS	100 PSF
PUBLIC ROOMS	100 PSF

2. DESIGN WIND LOAD SHALL BE BASED ON THE FLORIDA BUILDING CODE 2020.
   a) BASIC WIND SPEED = 140 MPH
   b) BUILDING CATAGORY II
   c) EXPOSURE 'C'
   d)INTERNAL PRESSURE COEFFICIENT ±0.18 FULLY ENCLOSED STRUCTURE
   e) COMPONENTS & CLADDING PRESSURES (ASD): +34.4 & -44.9 PSF

FOUNDATIONS

1. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 5000 PSF AS RECOMMENDED IN THE FOUNDATION INVESTIGATION PREPARED BY UNIVERSAL ENGINEERING SCIENCES (PROJECT NO. 1563044v2) APRIL 25, 2019.
2. PLACE FOOTINGS/SLAB ON COMPACTED SOIL. FOLLOW RECOMMENDATIONS OF SOILS REPORT.

CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:

SLAB ON GRADE, FOOTINGS	3000 PSI
REMAINING CONCRETE	4000 PSI

2. ALL CONCRETE SHALL HAVE A SLUMP OF 4" PLUS OR MINUS 1", AND HAVE 2 TO 4% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.58.
3. CONCRETE MIX DESIGN SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 CHAPTER 3, METHOD 1 OR METHOD 2. SUBMIT BACKUP DATA AS REQUIRED BY CHAPTER 5 SECTION 5.3. OF THE LATEST EDITION OF ACI 318.
4. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60.
5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. WWF SHALL BE LAPPED AT LEAST 8" AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 8".
6. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318 LATEST EDITION, AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS," ACI 301.
7. ALL REINFORCING DETAILS SHALL CONFORM TO "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" ACI 315 LATEST EDITION, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.
8. CONTRACTOR SHALL REVIEW ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF EMBEDDED ITEMS, SLEEVES, SLAB DEPRESSIONS, SLOPES, ETC. REQUIRED BY OTHER TRADES. THESE ITEMS SHALL BE FURNISHED AND INSTALLED PRIOR TO PLACEMENT OF CONCRETE.
9. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, ANCHOR BOLTS, INSERTS, ETC., AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED.
10. WHERE BAR LENGTHS ARE GIVEN ON THE DRAWINGS, THE LENGTH OF ANY HOOK, IF REQUIRED, IS NOT INCLUDED. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS AND AT SLABS EDGES.
11. CONTRACTOR SHALL PROVIDE SPACERS, CHAIRS, BOLSTERS, ETC. NECESSARY TO SUPPORT REINFORCING STEEL. SUPPORT ITEMS WHICH BEAR ON EXPOSED CONCRETE SURFACES SHALL HAVE ENDS WHICH ARE PLASTIC TIPPED OR STAINLESS STEEL.
12. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:
    - 3" CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.
    - 2" CONCRETE EXPOSED TO EARTH OR WEATHER, #6 THROUGH #18 BARS.
    - 1 1/2" CONCRETE EXPOSED TO EARTH OR WEATHER, #5 BAR AND SMALLER.
    - 1 1/2" CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH FOR THE PRIMARY REINFORCEMENT, TIES, STIRRUPS, AND SPIRALS IN BEAMS AND COLUMNS:
    - 3/4" CONCRETE NOT EXPOSED TO WEATHER NOR IN CONTACT WITH EARTH FOR SLABS, WALLS, AND JOISTS, #11 BAR AND SMALLER.
13. HORIZONTAL WALL AND FOOTING BARS SHALL BE BENT 1'-0" AROUND CORNERS OR CORNER BARS WITH 2'-0" LAP SHALL BE PROVIDED.
14. HORIZONTAL KEYWAYS IN CONSTRUCTION JOINTS SHALL BE PROVIDED IN BEAMS, SUPPORTED SLABS, AND WALL FOOTINGS WITH A DEPTH OF 1-1/2" AND HEIGHT EQUAL TO ONE-THIRD OF THE MEMBER'S DEPTH. REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS UNLESS OTHERWISE NOTED ON THE DRAWINGS. CONSTRUCTION JOINTS MAY BE USED ONLY AT LOCATIONS SHOWN ON THE DRAWINGS OR AT OTHER LOCATIONS APPROVED BY THE ARCHITECT.
15. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 48 BAR DIAMETERS TYP. EXCEPT WHERE OTHERWISE NOTED ON THE DRAWINGS. FOR BEAMS AND ELEVATED SLABS, LAP BOTTOM STEEL AT THE SUPPORT AND TOP STEEL OVER THE MIDSPAN, UNLESS OTHERWISE NOTED.
16. TESTING LABORATORY SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE ENGINEER.
17. REINFORCING STEEL TO BE WELDED: ASTM A 706.

STRUCTURAL STEEL

A. STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATION:
   1. W-SHAPES (U.N.O.) A572(GR50)
   2. STRUCTURAL STEEL PIPE.....A53 (FY-35)
   3. STRUCTURAL STEEL TUBE.....A500 (FY-46)
   4. BASE PLATES AND CONNECTION PLATES(U.N.O.).....A36 (FY=36)
   5. ALL OTHER STEEL.....A36 (FY=36)

B. BOLTED CONNECTIONS:
   1. LOAD-INDICATOR WASHERS MUST BE USED FOR FIELD CONNECTIONS.
   2. ALL BEAM TO COLUMN CONNECTIONS, BRACE CONNECTIONS AND MOMENT CONNECTED MEMBERS SHALL HAVE A325-N
   3. ALL OTHER CONNECTIONS (UNLESS NOTED OTHERWISE) 3/4" DIAMETER, A325 N
   4. OVERSIZED AND LONG-SLOTTED HOLES NOT ALLOWED UNLESS SHOWN ON STRUCTURAL DRAWINGS.
   5. THE SHOP DRAWINGS SHALL CLEARLY INDICATE THE TYPE OF BOLT USED IN EACH CONNECTION AND THE ALLOWABLE VALUES USED FOR THE VARIOUS BOLT TYPES.
   6. THE FOLLOWING MINIMUM STANDARDS APPLY:
      a) MINIMUM PLATE THICKNESS:.....3/8"
      b) MINIMUM BOLT DIAMETER:.....3/4"
      c) MINIMUM WELD:.....3/16" THICK THROAT
      d) MINIMUM DESIGN LOAD ON ANY CONNECTION:.10 KIPS

C. WELDED CONNECTIONS:
   1. ALL SHOP AND FIELD WELDING SHALL CONFORM TO ASW STRUCTURAL WELDING CODE-STEEL, ANSI/AWS D1.1
   2. MINIMUM WELD: 3/16" THICK THROAT.
   3. ALL STEEL TO STEEL CONNECTIONS NOT SHOWN BOLTED SHALL BE WELDED TO DEVELOP FULL SHEAR CAPACITY OF CONNECTING MEMBERS AS PER AISC SPECIFICATIONS. MINIMUM SIZE OF FILLET WELD (UNLESS NOTED OTHERWISE ON DRAWINGS.):

MATERIAL THICKNESS OF THICKER PART JOINED	MINIMUM SIZE OF FILLET WELD
1/4" TO 1/2"	3/16" ALL AROUND
OVER 1/2" TO 3/4"	1/4" ALL AROUND
OVER 3/4" TO 1 1/2"	5/16" ALL AROUND

D. SPLICING OF STRUCTURAL STEEL WHERE NOT DETAILED IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER.

E. BEAM CONNECTIONS:
   1. DESIGN CONNECTIONS FOR BEAMS TO SUPPORT HALF OF THE UNIFORM CAPACITY SHOWN IN AISC 'TABLES FOR ALLOWABLE LOADS ON BEAMS' FOR THE GIVEN SECTION AND SPAN UNLESS OTHERWISE SHOWN.

F. GALVANIZING:
   1. HOT-DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL ITEMS AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO THE OUTSIDE.
   2. EXAMINE DRAWINGS FOR OTHER ITEMS TO BE GALVANIZED.

G. ANCHOR BOLTS EXCEPT FOR STEEL TRUSSES:
   1. ALL ANCHOR BOLTS SHALL BE MADE FROM THREADED ROUND STOCK, ASTM A36. ALL ANCHOR BOLTS, NUTS AND WASHERS USED WITH GALVANIZED BASE PLATES SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.
   2. NUTS SHALL BE HEX HEAD, ASTM A563.
   3. WASHERS FOR ALL BASE PLATES SHALL BE 1/4" THICK PLATES EXTENDING MINIMUM 1" FROM EDGE OF BASE PLATE HOLES ON EACH SIDE WITH HOLES 1/16" LARGER THAN THE NOMINAL BOLT DIAMETER AND CONFORM TO ASTM A36.
   4. ALL ANCHOR BOLTS SET IN CONCRETE SHALL UTILIZE TEMPLATES. TEMPLATES SHALL BE DETAILED ON THE SHOP DRAWINGS.

H. NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES
   1. NON-METALLIC, SHRINKAGE RESISTANT, PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING PORTLAND CEMENT, SILICA SANDS, SHRINKAGE COMPENSATING AGENTS, AND FLUIDITY IMPROVING COMPOUNDS, AND SHALL CONFORM TO CORPS OF ENGINEERS SPECIFICATION FOR NON-SHRINK GROUT, CRD-C621-83.
   2. TWENTY-EIGHT DAY COMPRESSIVE STRENGTH SHALL BE A MINIMUM OF 6,000 PSI.

J. SHEAR CONNECTORS(HEADED STUDS.):
   1. MECHANICAL REQUIREMENTS STUD SHALL BE MADE FROM COLD DRAW BAR STOCK CONFORMING TO ASTM A 108, ' SPECIFICATION FOR STEEL BARS, CARBON COLD FINISHED, STANDARD QUALITY' GRADES 1010 THROUGH 1020 SEMI-KILLED OR FULLY KILLED. TENSILE STRENGTH SHALL BE 60,000 PSI MINIMUM WITH 20% MINIMUM ELONGATION IN 2 INCHES AND 50% MINIMUM AREA REDUCTION WHEN TESTED IN ACCORDANCE WITH APPLICABLE SECTIONS OF ASTM A370, "MECHANICAL TESTING OF STEEL PRODUCTS".
   2. DESIGN. THE DESIGN OF THE STUDS SHALL BE SUITABLE FOR ARC WELDING TO STEEL MEMBERS WITH AUTOMATICALLY TIMED STUD WELDING EQUIPMENT.
   3. FERRULE. AN ARC SHIELD (FERRULE) OF HEAT RESISTANT CERAMIC SHALL BE FURNISHED WITH EACH STUD. AFTER WELDING, ALL ARC SHIELDS SHALL BE REMOVED FROM EACH SHEAR CONNECTOR.
   4. DIMENSIONS. STUDS SHALL BE OF UNIFORM DIAMETER, HEADS SHALL BE CONCENTRIC AND NORMAL TO THE SHAFT, AND THE WELD END SHALL BE CHAMFERED. STANDARD DIMENSIONS AND TOLERANCES SHALL BE AS SPECIFIED IN AWS D1.1.
   5. SHEAR CONNECTORS MUST BE INSTALLED AFTER METAL DECK INSTALLATION.
   6. EPOXY ANCHOR FOR MISC. STEEL ATTACHMENT SHALL BE 3/4"DIAx6" HILTY HY-150.

STEEL ROOF DECK

1. ROOF DECK SHALL BE AS FOLLOWS:
   1). ALL METAL ROOF DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP 22 GAGE WIDE RIB, MIN PROPERTIES: Sp=0.186 IN3, Sn=.192 IN3 , I=0.169 IN4., GALVANIZED G90.
2. ROOF DECK SHALL BE PLACED IN AT LEAST TWO SPAN SEGMENTS. NO SINGLE SPAN CONDITIONS SHALL BE USED.
3. STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR GALVANIZED DECK, MINIMUM YIELD STRENGTH OF 33,000 PSI.
4. STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A525 G90 CLASS.

5. ATTACHMENT:

1) WELDING:
   a) ROOF DECK UNITS SHALL BE WELDED TO EACH STRUCTURAL SUPPORT MEMBER USING 5/8" DIAMETER PUDDLE WELDS AT ALL RIBS (36/7 FASTENER LAYOUT). WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND SIDE JOINTS AND SHALL BE COMPLETELY FUSED TO THE SUPPORTING MEMBERS.
   b) SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY WELDING (ON 18 GAUGE OR HEAVIER DECK ONLY) OR SHEET METAL SCREWS, SO THAT SPACING BETWEEN FASTENERS AND BETWEEN THE FIRST FASTENER AND SUPPORT DOES NOT EXCEED 18 INCHES.
   c) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, ROOF DECK UNITS SHALL BE WELDED TO EDGE STEEL USING 5/8"DIAMETER PUDDLE WELDS AT 6" O.C.

2) PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS.
3) END LAPS OF SHEETS SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS. ROOFS SHALL BE ERECTED BEGINNING AT THE LOW SIDE TO INSURE THAT END LAPS ARE SHINGLE FASHION.
4) ATTACH DECK TO EDGE SUPPORT WITH 5/8" DIA PUDDLE WELD @ 6" O.C.

PRE-ENGINEERED LIGHT GAGE METAL STUD

A. THE FABRICATOR SHALL FURNISH A STRUCTURAL SUBMITTAL BEARING THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. THIS SUBMITTAL SHALL BE CHECKED BY THE CONTRACTOR FOR COMPLETENESS AND CONTENT PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW. THE SUBMITTAL SHALL INCLUDE COMPONENT DETAILS AND SYSTEM LAYOUT DRAWINGS, IT SHALL IDENTIFY THE PROJECT LIST LOADING AND OTHER CRITERIA. THE DRAWINGS SHALL IDENTIFY AND LOCATE COMPONENTS AND SHALL SPECIFY MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS (SUCH AS STUDS TO CONC SLAB, TO CMU WALL, TO STL MEMBER, ETC.) & ALL OTHER NECESSARY FABRICATION AND ERECTION INFORMATION. THE SUBMITTAL SHALL INCLUDE CALCULATIONS VERIFYING ITS ADEQUACY TO RESIST THE LOADS INDICATED ON THE CONSTRUCTION DOCUMENTS. FABRICATION SHALL NOT COMMENCE UNTIL THIS REVIEW IS COMPLETED.

B. DESIGN OF LIGHTGAUGE METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF 'SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURAL STEEL MEMBERS(AISI).

C. MATERIALS: STUDS, RUNNERS AND ANGLES SHALL MEET THE REQUIREMENTS OF ASTM 1446 WITH MINIMUM YIELD STRENGTH AS FOLLOWS:

16, 14, AND 12 GAGE STUDS	50 KSI
22, 20, AND 18 GAGE STUDS	33 KSI
RUNNERS	33 KSI

D. METAL STUDS SHALL BE 'C' SHAPED STUDS

E. PROVIDE MANUFACTURER'S STANDARD STEEL RUNNERS, BLOCKING, LINTELS, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS AND ACCESSORIES AS RECOMMENDED BY MANUFACTURER AND/OR SHOWN ON DRAWINGS FOR APPLICATIONS INDICATED, AS NEEDED TO PROVIDE A COMPLETE STEEL FRAMING SYSTEM.

F. GALVANIZED FINISH SHALL COMPLY WITH ASTM A525 WITH A G90 COATING. ALL WELDS SHALL BE TOUCHED UP WITH A ZINC-RICH PROTECTIVE PAINT FOR CORROSION RESISTANCE.

G. ALL STUDS SHALL BE FULL LENGTH. NO SPLICING PERMITTED UNLESS SPECIFICALLY DETAILED.

H. ALL METAL TO METAL CONNECTIONS TO BE WELD OR SCREW ATTACHMENTS AS SHOWN ON DRAWINGS OR AS REQUIRED BY MANUFACTURER.

SHOP DRAWING SUBMITTALS

A. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL.

B. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS:
1. STRUCTURAL STEEL
2. REINFORCING STEEL
3. STEEL STAIRS (*)
4. CONCRETE MIX DESIGNS
5. LIGHT GAUGE METAL STUD AND CONNECTION WITH CALCULATIONS (*).
6. H.C. PLANK (*)

ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. ITEMS MARKED (*) SHALL BE SUBMITTED FOR ENGINEERS RECORD ONLY.

C. MANUFACTURER'S LITERATURE: SUBMIT TWO COPIES OF MANUFACTURER'S LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

D. DESIGN CALCULATIONS: THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW TWO SET OF DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA, FOR THE FOLLOWING ITEM:
1. STRUCTURAL STEEL CONNECTIONS.
2. STRUCTURAL STAIRS

MASONRY WALL CONSTRUCTION

A. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM COMPRESSIVE STRENGTH OF (f'm = 1500 PSI). FOR SECOND FLOOR ABOVE.
A MINIMUM COMPRESSIVE STRENGTH OF F'm = 2500 PSI FOR FIRST FLOOR (FOOTING TO SECOND FLOOR).

B. COURSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI FOR SECOND FLOOR ABOVE.
A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI FOR FIRST FLOOR (FOOTING TO SECOND FLOOR).

C. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.

D. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH CELLS FILLED WITH COARSE GROUT.

E. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 4'-0", REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL WITH DURO-WAL BAR POSITIONER D/A 811 TYPICAL UNLESS OTHERWISE NOTED. SEE TYPICAL GROUTING DETAILS FOR ADDITIONAL INFORMATION.

F. REINFORCING STEEL SHALL BE LAPPED MINIMUM 48 BAR DIAMETERS WHERE SPLICED UNLESS NOTED OTHERWISE ON THE DRAWINGS.

G. HORIZONTAL WALL REINFORCEMENT SHALL BE STANDARD TRUSS TYPE DUR-O-WAL (9 GA. SIDE X 9 GA. CROSS) @ 16" O.C., UNLESS SHOWN OTHERWISE ON DRAWINGS.

H. SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6". LAP WITH STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.

I. FOUNDATION DOWELS SHALL BE POSITIONED SUCH THAT IT IS ALIGNED WITH THE CENTER OF THE MASONRY UNIT'S VERTICAL CORE AND GROUTED SOLID. SHOULD THE FOUNDATION DOWEL NOT ALIGN WITH THE MASONRY UNIT'S VERTICAL CORE - NOTIFY THE ARCHITECT / STRUCTURAL ENGINEER FOR DIRECTION ON HOW TO CORRECT THE MIS ALIGNMENT.

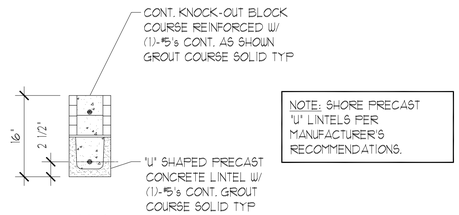
J. PROVIDE PRECAST CONCRETE LINTELS OVER ALL OPENINGS UNLESS NOTED OTHERWISE ON DRAWINGS. LINTELS SHALL BE OF SUFFICIENT SIZE AND REINFORCEMENT FOR THE GIVEN SPANS AND LOADING CONDITIONS. SUBMIT SHOP DRAWINGS WITH RATED LOAD CAPACITIES TO THE ARCHITECT FOR REVIEW.

K. PROVIDE A KNOCK OUT BLOCK OR U-BLOCK REINFORCED WITH (1)-#5 CONTINUOUS AT THE SILL OF ALL WINDOW OPENINGS. EXTEND 8' BEYOND EACH SIDE OF THE OPENING TYPICALLY.

MASONRY LINTEL SCHEDULE
(CAST-CRETE LINTEL)

MARK	LENGTH (L)	CAST-CRETE MARK	REMARK
MB-1	2' -10" ≤ 13'-4"	8F16-1B/1T PRECAST	

1. PROVIDE MASONRY LINTEL OVER ALL OPENINGS. IF NO LINTEL IS SPECIFIED, PROVIDE MB-1.
2. PROVIDE MINIMUM END BEARING OF 8". CUT OUT BOTTOM OF LINTEL AT END TO ALLOW CONTINUATION OF FILLED CELL REINFORCING.
3. MASONRY LINTEL SUBSTITUTIONS MUST BE APPROVED BY 'ADVANCED STRUCTURAL ENGINEERING' PRIOR TO INSTALLATION.



NOTE: SHORE PRECAST "U" LINTELS PER MANUFACTURER'S RECOMMENDATIONS.

MB-1

MASONRY LINTEL SCHEDULE

Columbia County Building Department — Plans Reviewed for Code Compliance — State of Florida

WINSHENG XIE LICENSE NO. 51161 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been electronically signed and sealed by Minsheng Xie, PE on the date shown on the time stamp using a digital signature.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Digitally signed by Minsheng Xie
DN: c=US, st=Florida, l=Orlando, o=Ase Engineering Services, Inc., cn=Minsheng Xie, email=mason@aseiicorp.com
Date: 2022.03.14 09:02:01 -04'00'
Adobe Acrobat DC version: 2015.006.30482

ASE ENGINEERING SERVICES, INC.
STRUCTURAL DESIGN GROUP
TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.
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Orlando, Florida 32817 ~ 407-677-5565 Fax 407-730-2999
Certificate of Authorization No. 25873
Minsheng Xie P.E.
Florida No. 51161

03-10-2022

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ARCHITECT OF RECORD :

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COURTYARD® Marriott®

REVISIONS

	REVISIONS	DATE
1	REVISIONS	03.10.2022

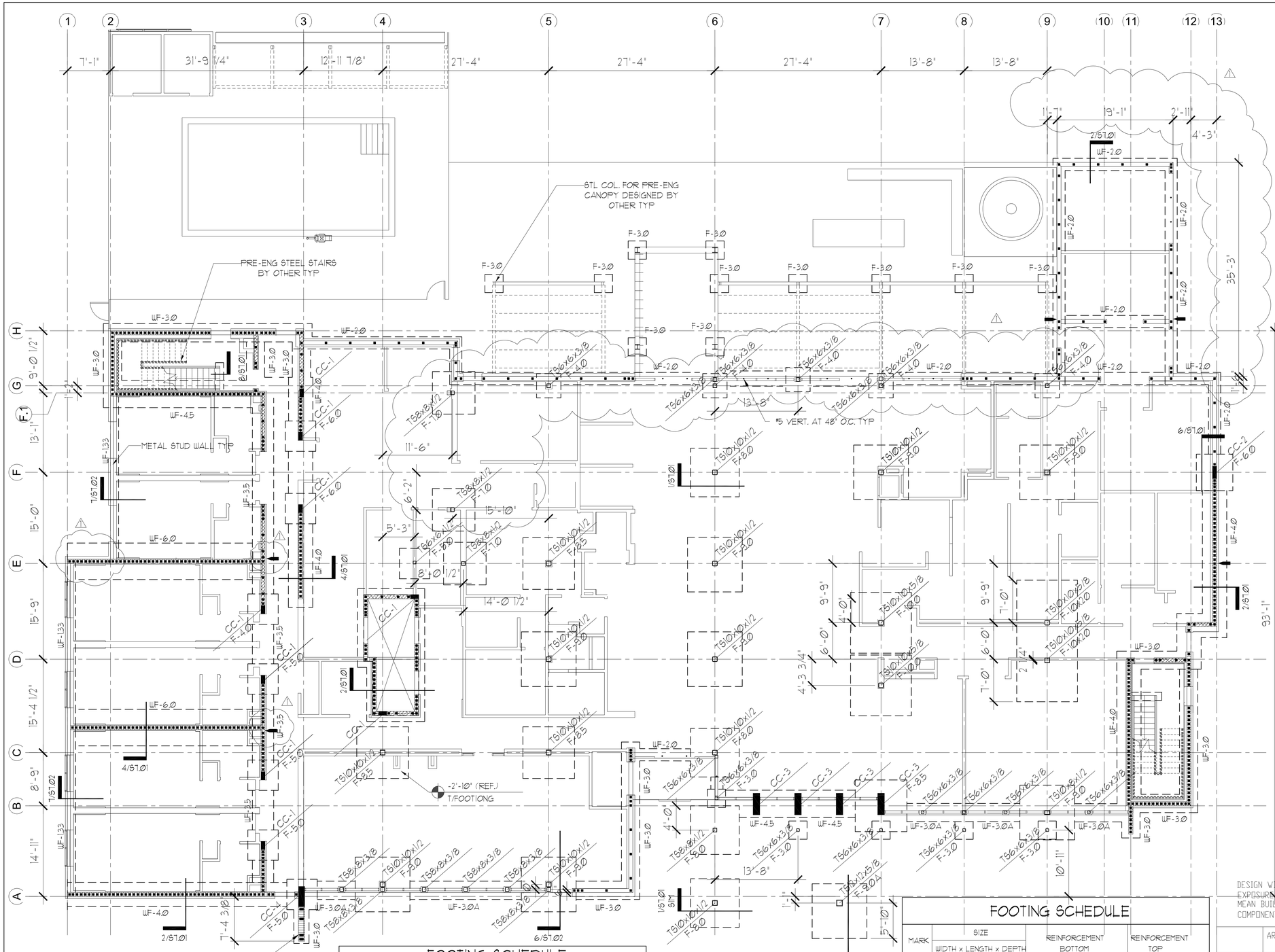
DRAWING NAME

GENERAL NOTES

PROJECT NAME

COURTYARD INN,®
Lake City, Florida

SEAL+SIGNATURE	DATE 07 DEC 2021
	PROJECT NUMBER 2K2101
	DRAWING NUMBER
S1.01	DECOR: XH
	PAGE NUMBER



PLAN NOTES:

1. SEE GENERAL NOTES ON SHEET S1.01.
2. DO NOT SCALE DRAWINGS. SEE ARCH'L. DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH ARCH'L. DRAWINGS PRIOR TO START OF CONSTRUCTION. IF DISCREPANCIES SHOULD OCCUR - CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
3. 4" (TOTAL) CONCRETE SLAB REINFORCED WITH 6x6 - W1.4xW1.4 WWF. OVER 8 MIL VAPOR BARRIER ON COMPACTED SUBGRADE. COORDINATE ANY AND ALL SLAB SLOPES, DEPRESSIONS AND LIMITS THERE OF WITH ARCH'L. DRAWINGS (FOR ACTUAL TOP OF SLAB ELEVATIONS, SEE ARCH'L. AND / OR CIVIL DRAWINGS)
5. SEE ARCH'L. DRAWINGS FOR LOCATIONS / LIMITS AND CONSTRUCTION INFORMATION OF INTERIOR NON-BEARING PARTITION WALLS NOT SHOWN ON PLAN. SEE GENERAL NOTES FOR ADDITIONAL WALL FRAMING INFORMATION.
6. THE MAX. SPACING OF CONTROL JOINT FOR ENCLOSURE SPACE SHALL BE 20'-0" O.C., AND FOR OPEN SPACE SHALL BE 8'-0" O.C. SEE CONTROL JOINT DETAIL.
7. PROVIDE CORNER BARS AT FOOTINGS. SEE SECTION FOR FURTHER INFORMATION.
8. STEPPED FOOTING: (AS REQUIRED) SEE DETAIL FOR ADDITIONAL INFORMATION. GENERAL CONTRACTOR TO COORD. STEP FOOTING LOCATIONS W/MECHANICAL.
8. ▪▪▪▪▪▪▪▪ INDICATES 8" MASONRY BEARING WALLS REINFORCED WITH (1)-#5 BAR (VERT.) AT 8' O.C. MAX. GROUT SOLID CONC. ALL THE FILLED CELLS. PROVIDE ADDITIONAL BARS AS SHOWN ON PLAN AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS AND ENDS OF WALLS. EXTEND VERTICAL REINFORCING BARS THRU ALL LEVELS (SPLICE / LAP AS REQ'D.) TO UPPER MOST CONC. TIE BEAM OR KNOCK-OUT BLK. BEAM AND TERMINATE BARS W/ 90° 9" HOOK.
9. ▪▪▪▪▪▪▪▪ INDICATES ONE STORY 8" MASONRY BEARING WALLS REINFORCED WITH (1)-#5 BAR (VERT.) AT 48" O.C. MAX. GROUT SOLID CONC. ALL THE CELLS. PROVIDE ADDITIONAL BARS AS SHOWN ON PLAN AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS AND ENDS OF WALLS. EXTEND VERTICAL REINFORCING BARS THRU ALL LEVELS (SPLICE / LAP AS REQ'D.) TO UPPER MOST CONC. TIE BEAM OR KNOCK-OUT BLK. BEAM AND TERMINATE BARS W/ 90° 9" HOOK.
10. ▪▪▪▪▪▪▪▪ INDICATES 8" MASONRY BEARING WALLS REINFORCED WITH (1)-#5 BAR (VERT.) AT 32" O.C. MAX. GROUT SOLID CONC. ALL THE CELLS. PROVIDE ADDITIONAL BARS AS SHOWN ON PLAN AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS AND ENDS OF WALLS. EXTEND VERTICAL REINFORCING BARS THRU ALL LEVELS (SPLICE / LAP AS REQ'D.) TO UPPER MOST CONC. TIE BEAM OR KNOCK-OUT BLK. BEAM AND TERMINATE BARS W/ 90° 9" HOOK.
11. ▪▪▪▪▪▪▪▪ INDICATES MASONRY CONTROL JOINT. SEE DETAIL.
12. ALL THE CELLS OF FIRST FLOOR 8" MASONRY BEARING WALLS SHALL BE GROUT SOLID WITH CONCRETE.

Ø'-0" (REF.) T/CONC. SLAB
-1'-4" MIN. (REF.) T/FOOTINGS AND ADJUST T/FOOTINGS TO BE 12" MIN BELOW FINISHED GRADE WHICH EVER IS LOWER U.N.O.

FOUNDATION PLAN
SCALE: 1/8"=1'-0"

FOOTING SCHEDULE

MARK	SIZE WIDTH x LENGTH x DEPTH	REINFORCEMENT BOTTOM	REINFORCEMENT TOP
WF-1.33	1'-4"xCONT.x2'-0"	(2)-#5 CONT.	(1)-#5 CONT.
WF-2.0	2'-0"xCONT.x1'-0"	(3)-#5 CONT. #5 @ 48" O.C. TRANSVERSE	
WF-3.0A	3'-0"xCONT.x1'-4"	(4)-#5 CONT. #5 @ 18" O.C. TRANSVERSE	
WF-3.0	3'-0"xCONT.x1'-0"	(4)-#5 CONT. #5 @ 18" O.C. TRANSVERSE	
WF-3.5	3'-6"xCONT.x1'-0"	(4)-#5 CONT. #5 @ 12" O.C. TRANSVERSE	
WF-4.0	4'-0"xCONT.x1'-0"	(4)-#5 CONT. #5 @ 9" O.C. TRANSVERSE	
WF-4.5	4'-6"xCONT.x1'-2"	(5)-#5 CONT. #5 @ 8" O.C. TRANSVERSE	
WF-8.5	6'-0"xCONT.x1'-4"	(7)-#5 CONT. #6 @ 9" O.C. TRANSVERSE	

FOOTING SCHEDULE

MARK	SIZE WIDTH x LENGTH x DEPTH	REINFORCEMENT BOTTOM	REINFORCEMENT TOP
F-3.0	3'-0"x3'-0"x1'-4"	(3)-#5's EACH WAY	
F-4.0	4'-0"x4'-0"x1'-4"	(5)-#5's EACH WAY	
F-5.0	5'-0"x5'-0"x1'-4"	(6)-#5's EACH WAY	
F-6.0	6'-0"x6'-0"x1'-6"	(6)-#6's EACH WAY	
F-7.0	7'-0"x7'-0"x1'-10"	(6)-#7's EACH WAY	
F-8.0	8'-0"x8'-0"x2'-0"	(8)-#7's EACH WAY	
F-8.5	8'-6"x8'-6"x2'-2"	(9)-#7's EACH WAY	
F-9.0 F-9.0A	9'-0"x9'-0"x2'-2"	(10)-#7's EACH WAY	(10)-#7's EACH WAY AT F-9.0A ONLY
F-10.0	10'-0"x10'-0"x2'-6"	(9)-#8's EACH WAY	
F-10x20	10'-0"x20'-0"x2'-6"	#8's AT 12" O.C. EACH WAY	

DESIGN WIND VELOCITY=140 MPH, BUILDING CATAGORY II
EXPOSURE "C",
MEAN BUILDING HEIGHT: 70'-0"
COMPONENT AND CLADDING PRESSURE (ASD) FOR OPENINGS IN WALLS

	AREA OF OPENINGS (SQ.FT)	COEFFICIENT	DESIGN PRESSURE
INTERIOR ZONE	UP TO 10	1.70	+36.01, -38.96 PSF
	10 TO 20	1.70	+34.37, -37.43 PSF
	20 TO 50	1.70	+32.23, -35.29 PSF
	50 TO 100	1.70	+30.60, -33.66 PSF
	100 TO 500	1.70	+26.83, -29.89 PSF
EDGE STRIP	UP TO 10	1.70	+36.01, -48.14 PSF
	10 TO 20	1.70	+34.37, -44.88 PSF
	20 TO 50	1.70	+32.23, -40.60 PSF
	50 TO 100	1.70	+30.60, -37.43 PSF
	100 TO 500	1.70	+26.83, -29.89 PSF

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	REVISIONS	03.10.2022
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DRAWING NAME

FOUNDATION PLAN

PROJECT NAME

COURTYARD INN,°
Lake City, Florida

SEAL+SIGNATURE

DATE 07 DEC 2021
PROJECT NUMBER 2K2101
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DECOR: XH

S2.01

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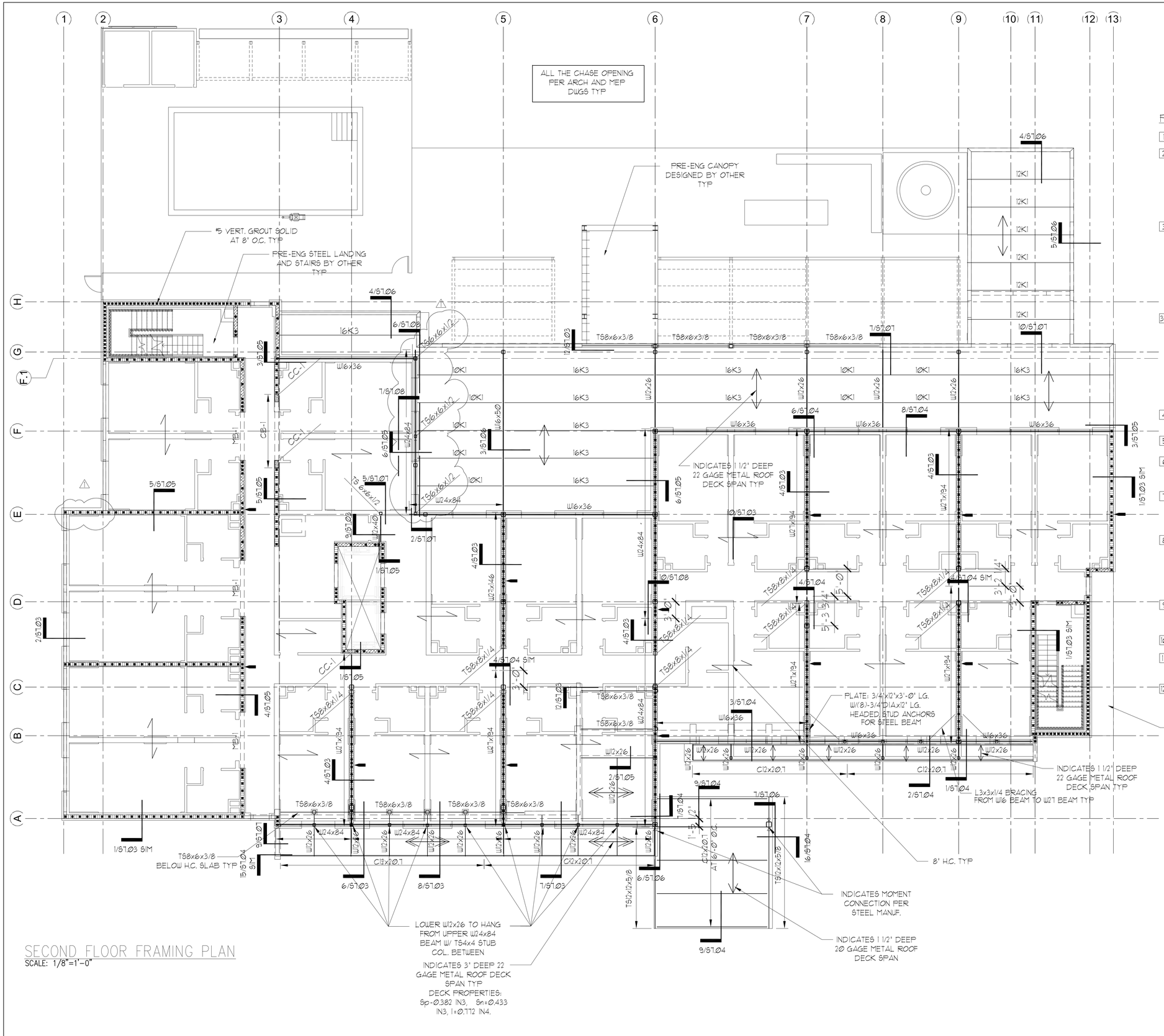

SECOND FLOOR FRAMING PLAN

SCALE: 1/8"=1'-0"

PLAN NOTES:

1. SEE GENERAL NOTES ON SHEET S1Ø1.
2. DO NOT SCALE DRAWINGS. SEE ARCH'L. DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH ARCH'L. DRAWINGS PRIOR TO START OF CONSTRUCTION. IF DISCREPANCIES SHOULD OCCUR - CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
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5. ⟷ HOLLOW CORE SPAN.
6. SEE ARCH'L. DRAWINGS FOR LOCATIONS / LIMITS AND CONSTRUCTION INFORMATION OF INTERIOR NON-BEARING PARTITION WALLS NOT SHOWN ON PLAN. SEE GENERAL NOTES.
7. COORDINATE ALL SLAB (TOPPING) SLOPES AND DEPRESSIONS WITH ARCH'L. DRAWINGS. (FOR ACTUAL TOP OF SLAB ELEVATIONS, SEE ARCH'L. DRAWINGS)
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DRAWING NAME

SECOND FLOOR FRAMING PLAN

PROJECT NAME

COURTYARD INN,® Lake City, Florida

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S3.01

PAGE NUMBER

PRECAST CONCRETE

1 DETAIL AND MANUFACTURE ALL PRECAST CONCRETE ELEMENTS PER APPLICABLE BUILDING CODES AND STANDARDS. MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS VERIFYING ALL DESIGN CONNECTIONS, DETAILS, PIECE MARKS AND PLACING PLANS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED, DATED AND BEAR THE EMBOSSED SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ALABAMA.

2 PICK APPROPRIATE SYSTEM

DESIGN PRECAST HOLLOW CORE SLAB SYSTEM FOR THE FOLLOWING SUPERIMPOSED DEAD LOADS IN ADDITION TO THE DESIGN LIVE LOADS.

TYPICAL FLOORS 25 PSF

USE UNIT LOAD OF 600 PLF FOR ALL NON-BEARING MASONRY WALLS. COORDINATE LOCATION OF WALLS WITH ARCHITECTURAL/STRUCTURAL DRAWINGS.

3 CONCRETE FOR PRECAST CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI AT 28 DAYS. USE MINIMUM CONCRETE STRENGTH OF 3,500 PSI AT RELEASE.

4 PRESTRESSING STRAND SHALL BE 7-WIRE, 1/2" DIAMETER, CONFORMING TO ASTM A-416 AND DEVELOPING A MINIMUM YIELD STRENGTH OF 270,000 PSI.

5 DESIGN PRECAST FLOOR MEMBERS FOR FIRE RATING OF 1 HOUR.

6 MAINTAIN UNIFORM SLAB THICKNESS NOTED DURING CONCRETE PLACEMENT AS REQUIRED TO MAINTAIN FIRE RATING ABOVE.

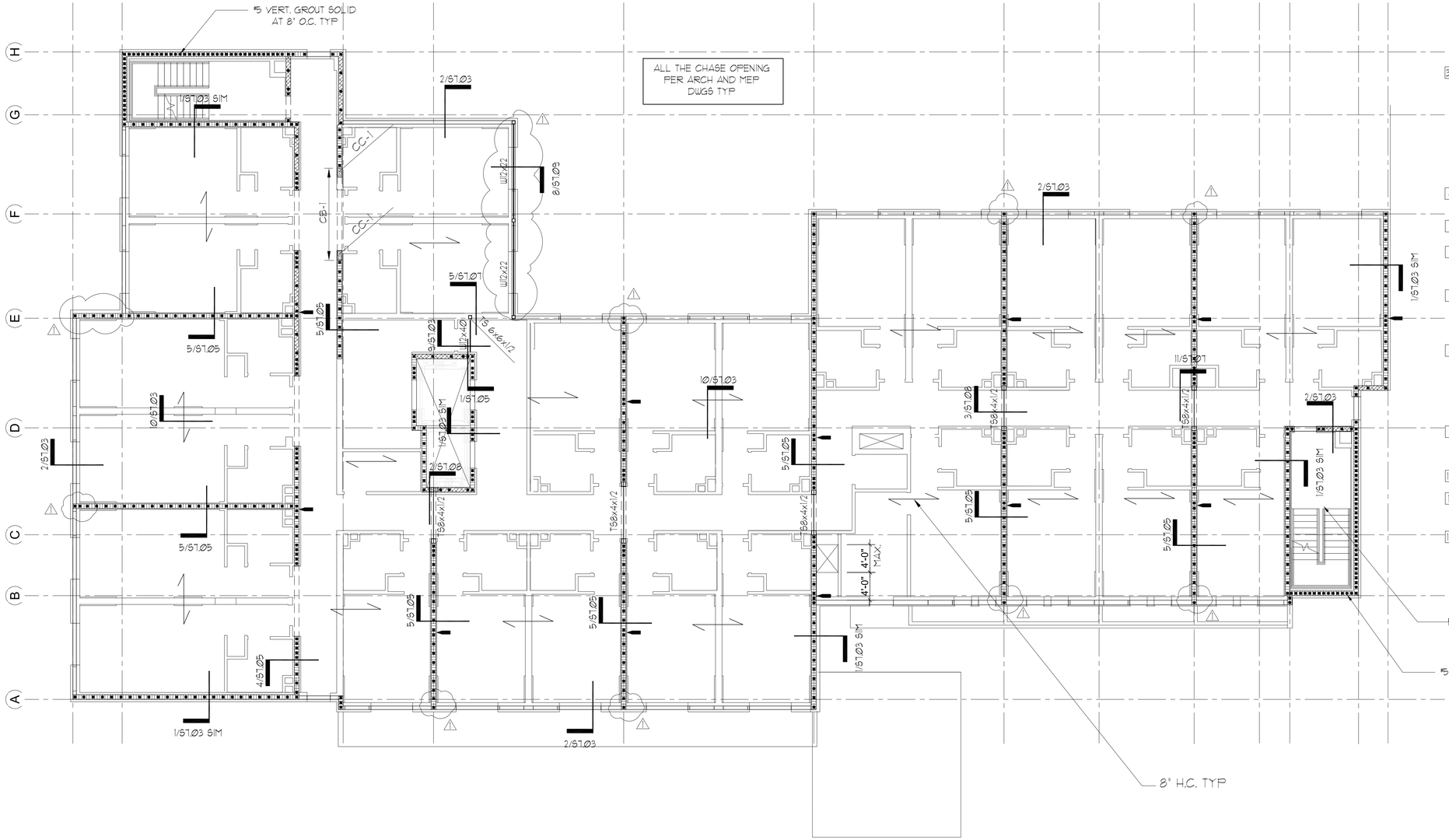
7 SHOP DRAWINGS SHALL BE COORDINATED BY THE CONTRACTOR WITH ARCHITECTURAL, MECHANICAL, PLUMBING AND OTHER DRAWINGS AS REQUIRED FOR EQUIPMENT WEIGHTS, PADS, OPENINGS CONSTRUCTION JOINTS AND OTHER DETAILS PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER FOR REVIEW.

8 DESIGN IN ACCORDANCE WITH ACI 318. LIVE LOAD DEFLECTION SHALL NOT EXCEED THE SPAN /360.

9 GROUT PRECAST UNITS WITH 1:3 CEMENT-SAND GROUT MIX PROPERLY FILING SHEAR KEYWAYS BETWEEN UNITS.

10 CONCRETE TOPPING OR LOADING SHALL NOT BE DONE PRIOR TO GROUTING OF KEYWAYS. TOPPING SHALL BE NORMAL WEIGHT CONCRETE OF 3000 PSI COMPRESSIVE STRENGTH TO A MINIMUM CONCRETE THICKNESS OF 2". ADEQUATE BOND TO BE ACCOMPLISHED BY CLEANING AND WETTING PLANKS BEFORE POURING CONCRETE.

11 GENERAL CONTRACTOR SHALL PROVIDE A LEVEL AND ADEQUATE BEARING SURFACE FOR ALL PRECASTS UNITS.



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3RD AND 4TH FLOOR FRAMING PLAN

SCALE: 1/8"=1'-0"

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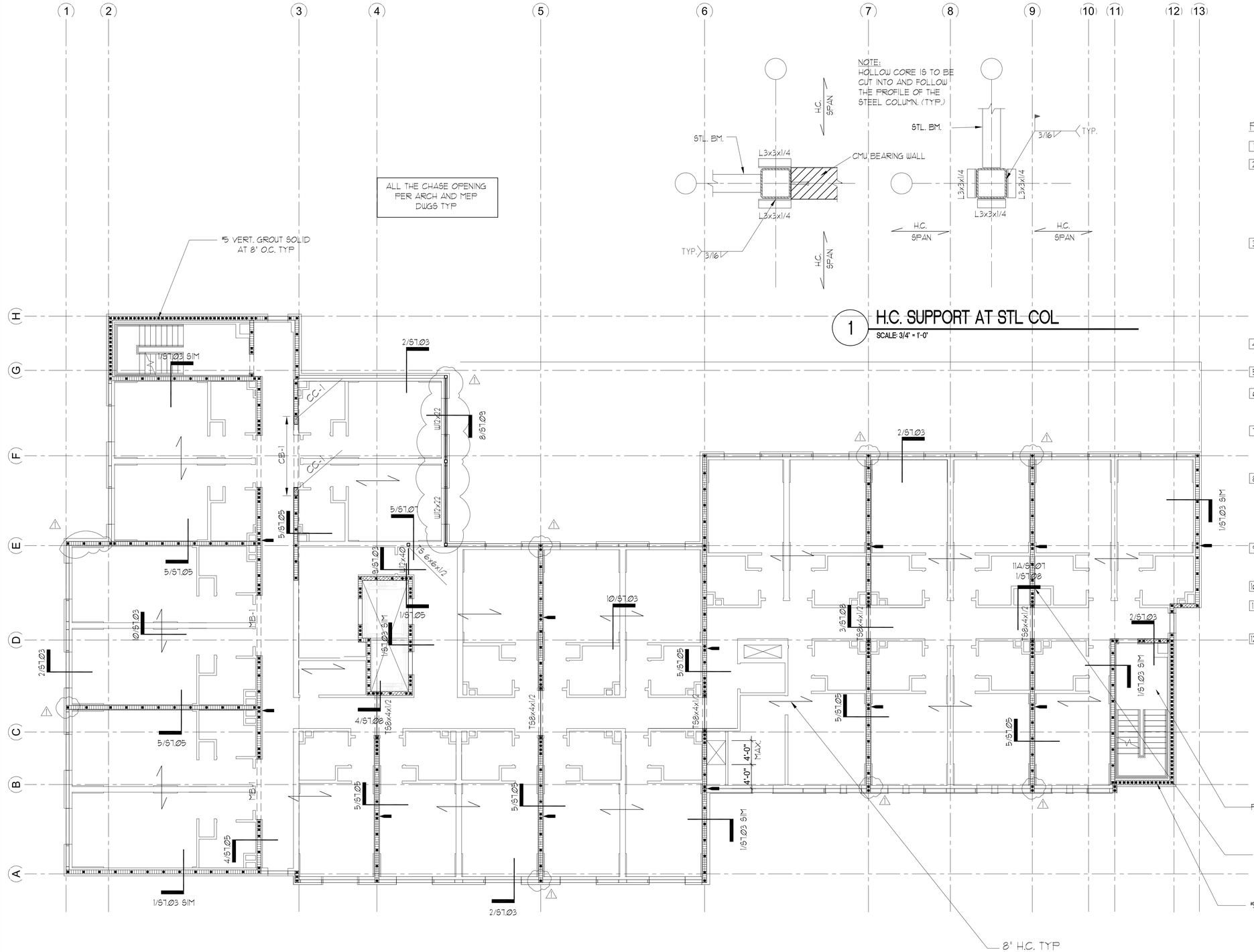

1 **H.C. SUPPORT AT STL COL**
SCALE: 3/4" = 1'-0"

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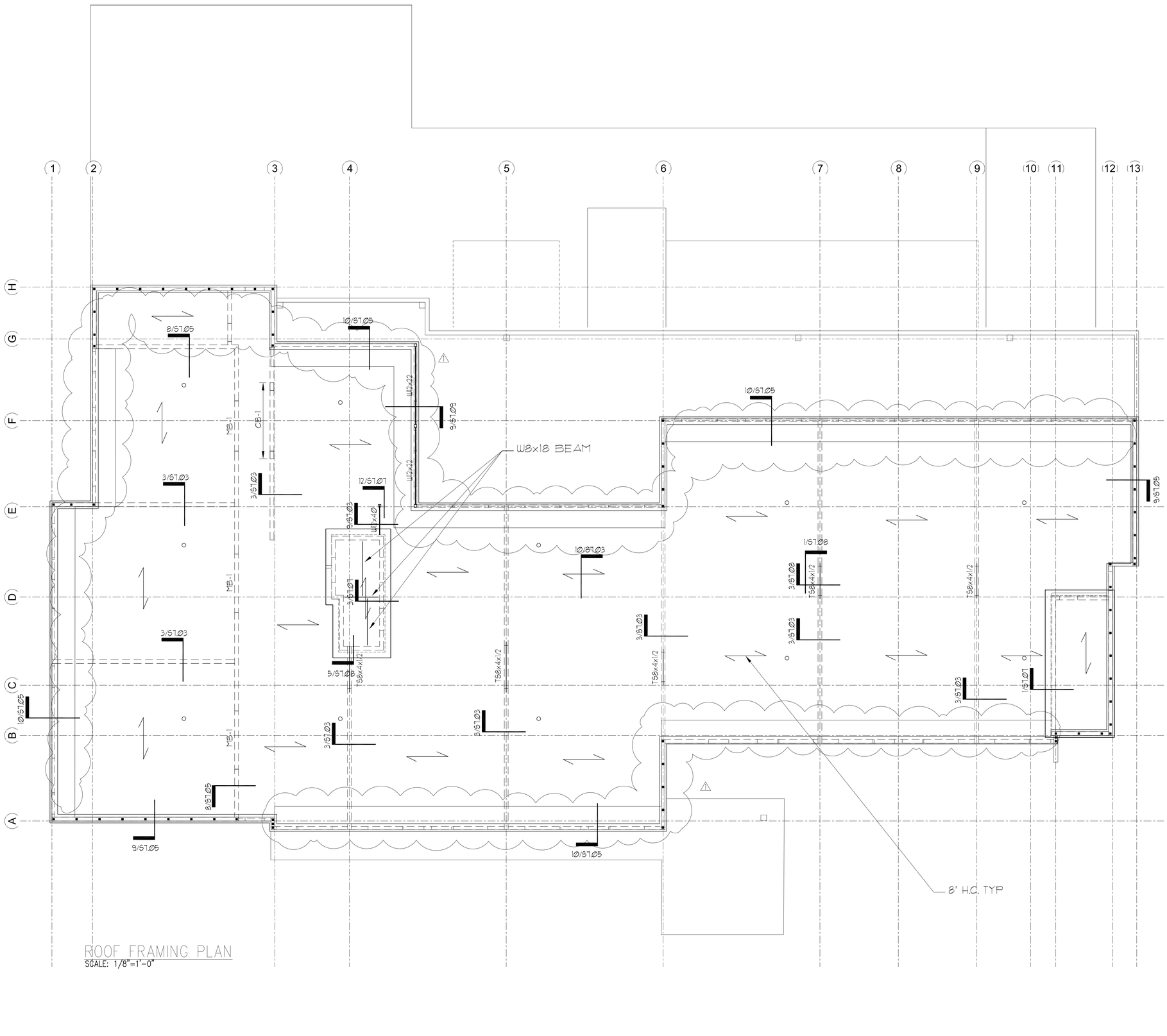

ROOF FRAMING PLAN
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PLAN NOTES:

1. SEE GENERAL NOTES ON SHEET S1.01.
2. DO NOT SCALE DRAWINGS. SEE ARCH'L. DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH ARCH'L. DRAWINGS PRIOR TO START OF CONSTRUCTION. IF DISCREPANCIES SHOULD OCCUR - CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
3. INDICATES 8" MASONRY PARAPET WALLS REINFORCED WITH (1)-#5 BAR (VERT.) AT 48" O.C. MAX. GROUT SOLID CONC. ALL THE FILLED CELLS. PROVIDE ADDITIONAL BARS AS SHOWN ON PLAN AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS AND ENDS OF WALLS.
4. 8" PRESTRESSED HOLLOW CORE SLAB U.N.O.
5. HOLLOW CORE SPAN.
6. SEE ARCH'L. DRAWINGS FOR LOCATIONS / LIMITS AND CONSTRUCTION INFORMATION OF INTERIOR NON-BEARING PARTITION WALLS NOT SHOWN ON PLAN. SEE GENERAL NOTES
7. COORDINATE ALL SLAB (TOPPING) SLOPES AND DEPRESSIONS WITH ARCH'L. DRAWINGS. (FOR ACTUAL TOP OF SLAB ELEVATIONS, SEE ARCH'L. DRAWINGS)
8. PROVIDE MASONRY LINTEL OVER ALL OPENINGS. IF NO LINTEL TYPE IS INDICATED, PROVIDE TYPE MB-1 FOR 8" CMU WALL. SEE MASONRY LINTEL SCHEDULE ON SHEET S1.01. PROVIDE SHORING DURING CONSTRUCTION, IF LINTEL SPAN MORE THAN SIX FEET.
9. PLACE THE PLUMBING SLEEVES INTO H.C. PLANKS, CONCRETE BEAMS AND COLUMNS. SEE PLUMBING DRAWINGS FOR THE LOCATION OF SLEEVES.
10. INDICATES MASONRY CONTROL JOINT. SEE DETAIL.
11. PROVIDE DOUBLE K.O. BOND BEAM AT ROOF LEVEL WITH (1) #5 AT EACH COURSE.
12. CB-1: 8x16 W/ (2) #7 TOP AND BOTTOM AND #3 TIES AT 6' O.C.

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COURTYARD® Marriott®

REVISIONS

1	REVISIONS	03.10.2022

DRAWING NAME

ROOF FRAMING PLAN

PROJECT NAME

COURTYARD INN,®
Lake City, Florida

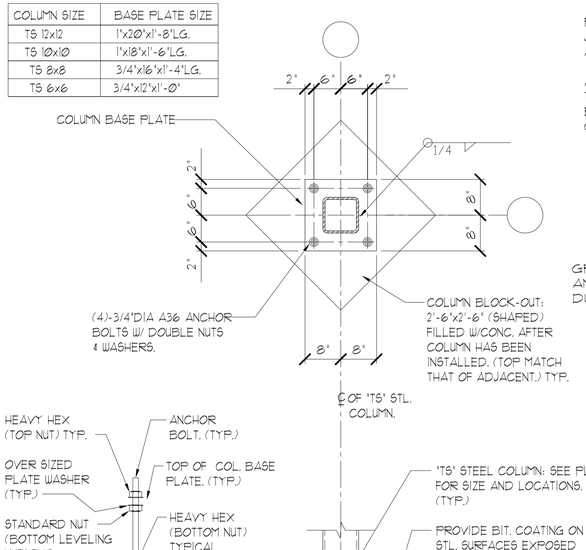
SEAL+SIGNATURE

DATE 07 DEC 2021
PROJECT NUMBER 2K2101
DRAWING NUMBER
DECOR: XH

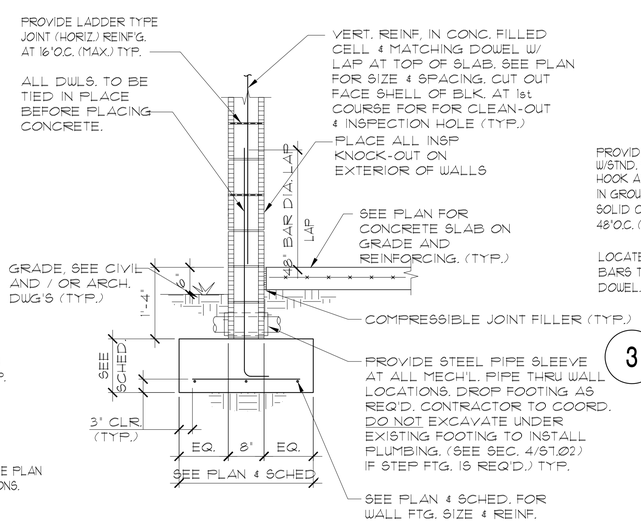
S6.01

PAGE NUMBER

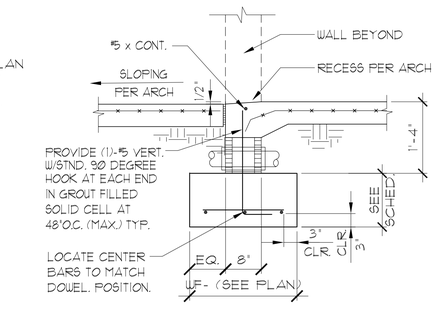
COLUMN SIZE	BASE PLATE SIZE
TS 12x12	1"x20"x1'-8"LG.
TS 10x10	1"x18"x1'-6"LG.
TS 8x8	3/4"x16"x1'-4"LG.
TS 6x6	3/4"x12"x1'-0"



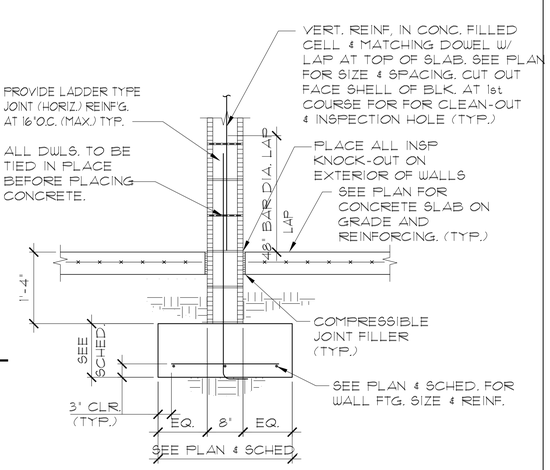
1 STEEL COL. AT FOOTING
SCALE: 3/4" = 1'-0"



2 EXTERIOR CMU WALL AT FOOTING
SCALE: 3/4" = 1'-0"



3 SECTION @ CMU WALL OPENING
SCALE: 3/4" = 1'-0"



4 INTERIOR CMU WALL AT FOOTING
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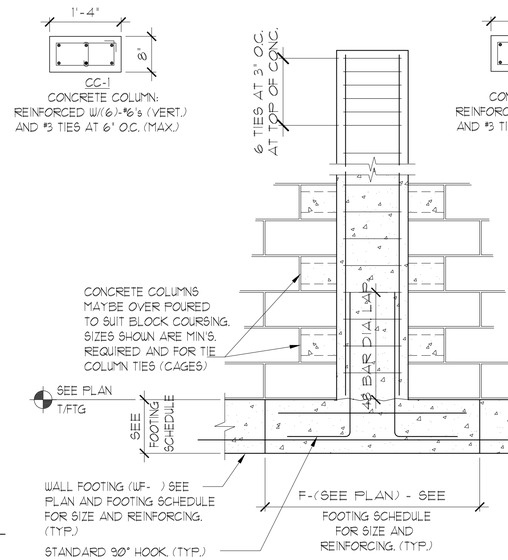
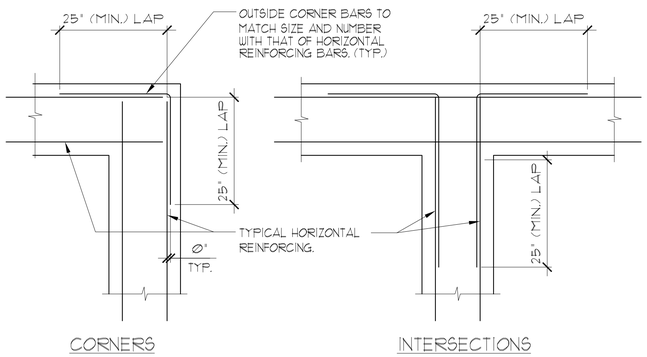
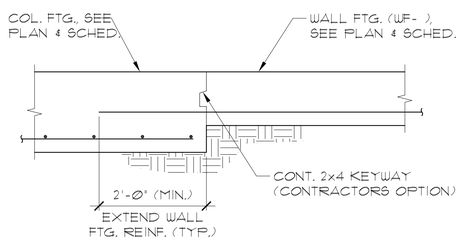


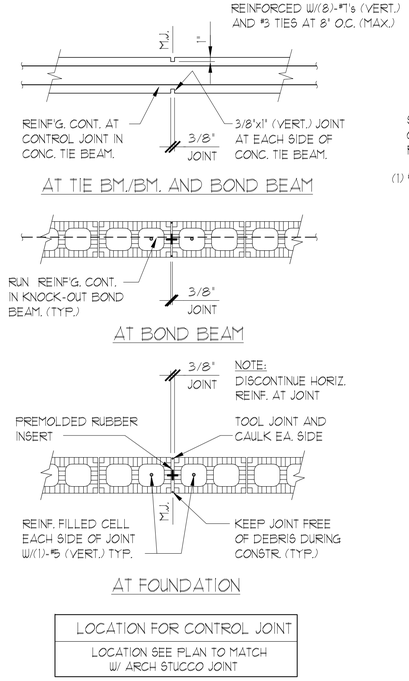
6 CONCRETE TIE COLUMN AT FOOTING AND TYPES
SCALE: 3/4" = 1'-0"



5 HORIZONTAL REINFORCING AT BOND BEAMS MASONRY WALLS AND FOOTINGS
SCALE: 3/4" = 1'-0"



10 TYP. WALL FOOTING AT COLUMN FOOTING
SCALE: 3/4" = 1'-0"



7 MASONRY JOINTS
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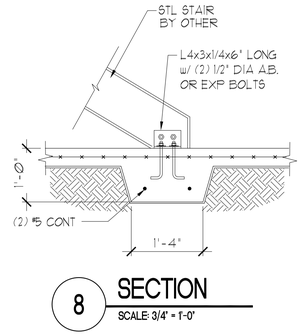

8 SECTION
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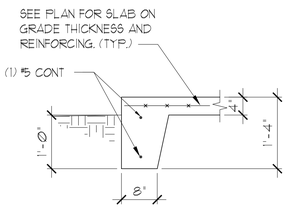

9 SLAB EDGE DETAIL
SCALE: 3/4" = 1'-0"

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COURTYARD INN,°
Lake City, Florida

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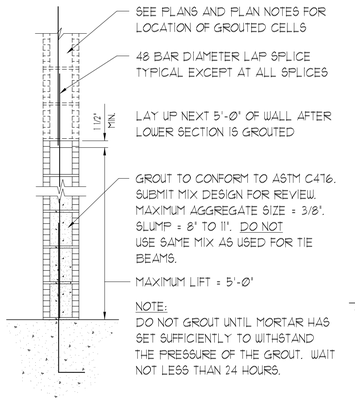
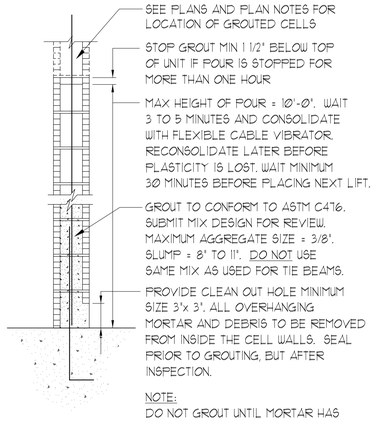
S7.01	DECOR:	XH
	PAGE NUMBER	



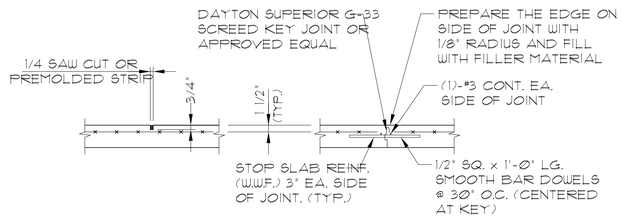
1 GROUTING SECTION TYPICAL LOW LIFT

SCALE: 3/4" = 1'-0"



2 GROUTING SECTION TYPICAL HIGH LIFT

SCALE: 3/4" = 1'-0"



CONTROL JOINT CONSTRUCTION JOINT

SAW CONTROL JOINT NOTES:

1. MAKE SAW CUT AS SOON AS SLAB IS ABLE TO SUPPORT WEIGHT OF WORKERS AND SAWING EQUIPMENT WITHOUT DAMAGE TO FINISH SURFACE OF SLAB.
2. ALL SAW CUT CONTROL JOINTS SHALL BE "SOFT CUT" WITHIN 2 HOURS AFTER FINAL FINISHING.
3. CLEAN JOINT PRIOR TO FILLING THE JOINT.

JOINT FILLER MATERIAL NOTES:

1. FILLER MATERIAL USED SHALL HAVE A MINIMUM SHORE A HARDNESS OF 35, AND SHALL CONFORM TO ASTM D2240. JOINT FILLER SHALL BE APPROVED BY ENGINEER PRIOR TO APPLICATION. APPROVED JOINT FILLER IS VULKEM 245 AS MANUFACTURED BY MAMECO INTERNATIONAL OR APPROVED EQUAL.
2. WHERE POSSIBLE, FILLER MATERIAL SHALL BE APPLIED WHEN BUILDING IS UNDER PERMANENT TEMPERATURE CONTROL. THIS SHALL BE EITHER AT THE END OF CONSTRUCTION OF THE COMPLETE BUILDING SHELL, OR A MINIMUM OF 90 DAYS AFTER SLAB CONSTRUCTION.
3. FOLLOW STRICTLY THE MANUFACTURERS RECOMMENDED PROCEDURES FOR APPLYING THE JOINT FILLER.

CONSTRUCTION JOINT NOTES:

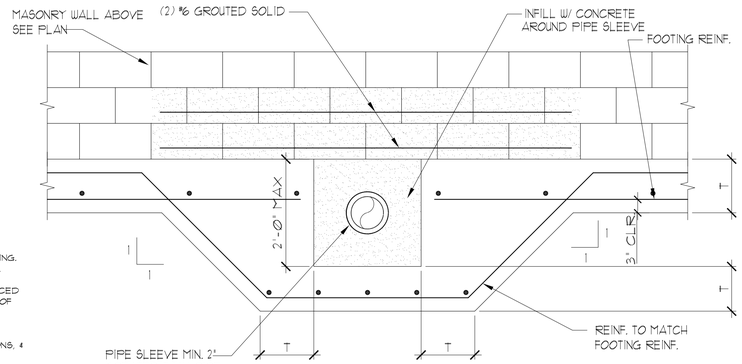
1. SEE PLAN FOR SLAB THICKNESS (T) AND REINFORCING.
2. SLAB REINFORCEMENT SHALL BE CHAIRED BY SOIL SUPPORTED SLAB BOLSTERS 3'-0" O.C. EA. WAY.
3. BREAK BOND BETWEEN NEW AND PREVIOUSLY PLACED SLAB BY SPRAYING OR PAINTING EXPOSED SIDE OF SLAB AND DOWEL WITH A CURING COMPOUND. ASPHALTIC EMULSION, OR FORM OIL.
4. REFER TO GENERAL NOTES, GENERAL SPECIFICATIONS, & DRAWINGS FOR SUB-FLOOR DRAINAGE SYSTEM. SUBGRADE PREPARATION AND/OR MUD SLAB REQUIREMENTS.
5. SUBGRADE SHALL BE FREE OF STANDING WATER AT THE TIME OF CONCRETE PLACEMENT.

JOINT LOCATION & SPACING NOTES:

1. GENERAL CONTRACTOR TO VERIFY THE ARCHITECHURAL FLOOR FINISH AND COORDINATE THE LOCATIONS OF ALL SLAB CONTROL AND/OR CONSTRUCTION JOINTS WITH THE ARCHITECHURAL FLOOR FINISH (TYPICAL).
2. PROVIDE CONTROL AND/OR CONSTRUCTION JOINTS AT EVERY COLUMN LINE & BETWEEN THE COLUMN LINES SUCH THAT THE JOINT SPACING DOES NOT EXCEED 20'-0" O.C. OR SLAB AREA DOSE NOT EXCEED 400 SQ. FEET, AND SLAB PROPORTIONS DO NOT EXCEED A 2:1 RATIO. (TYPICAL UNLESS OTHERWISE SHOWN ON THE DRAWINGS).

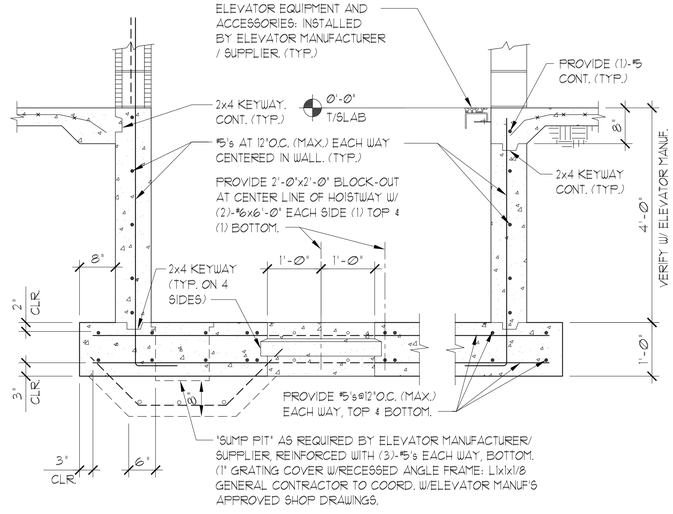
3 TYPICAL SLAB ON GRADE CONTROL AND CONSTRUCTION JOINT DETAIL

SCALE: 3/4" = 1'-0"



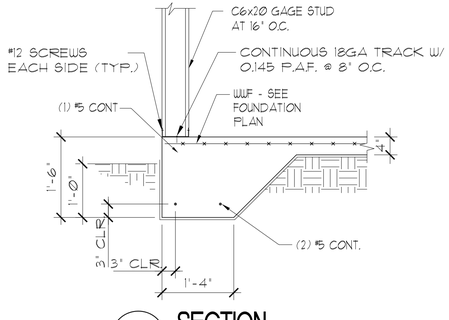
4 TYP PIPE PENETRATION AT FOOTING

SCALE: 3/4" = 1'-0"



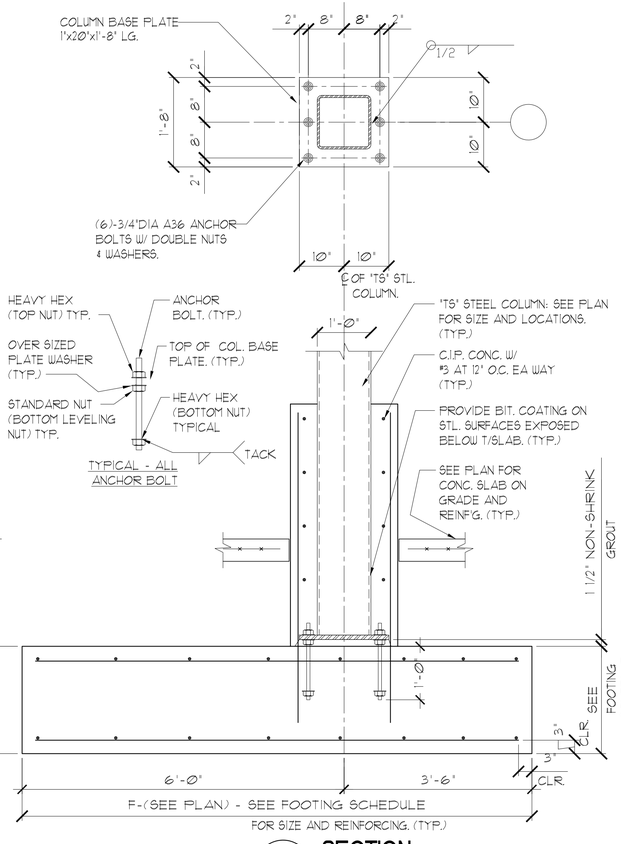
5 ELEVATOR PIT DETAIL

SCALE: 3/4" = 1'-0"



7 SECTION

SCALE: 3/4" = 1'-0"



8 SECTION

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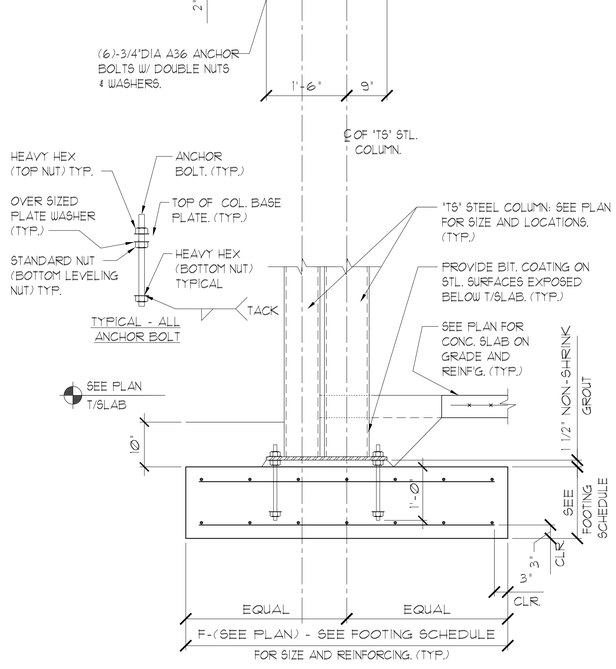

6 SECTION

SCALE: 3/4" = 1'-0"

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COURTYARD INN,® Lake City, Florida

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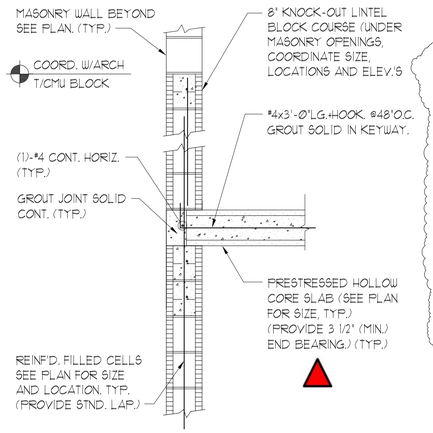
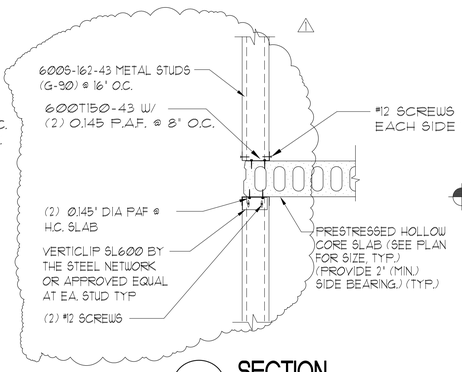


1 H.C. SLAB EXTER. END BRG.
SCALE: 3/4" = 1'-0"



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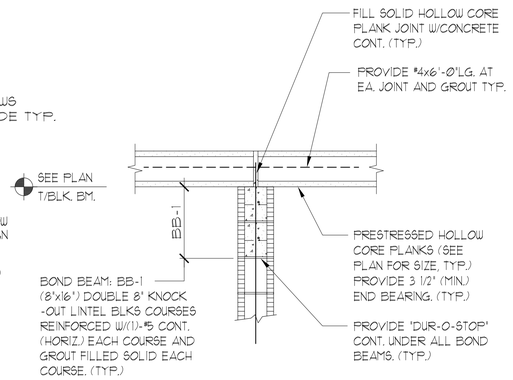

3 H.C. PLANK COMMON END BRG.
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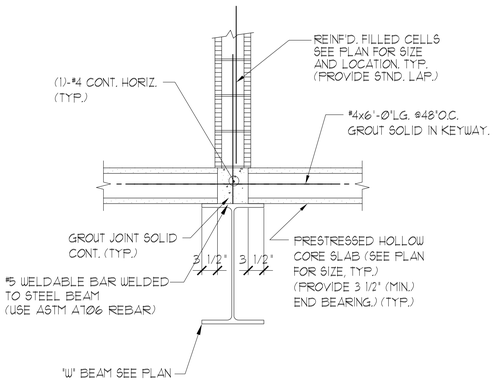

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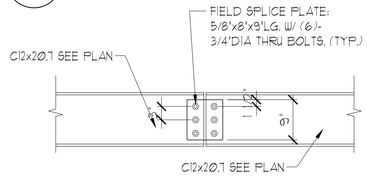
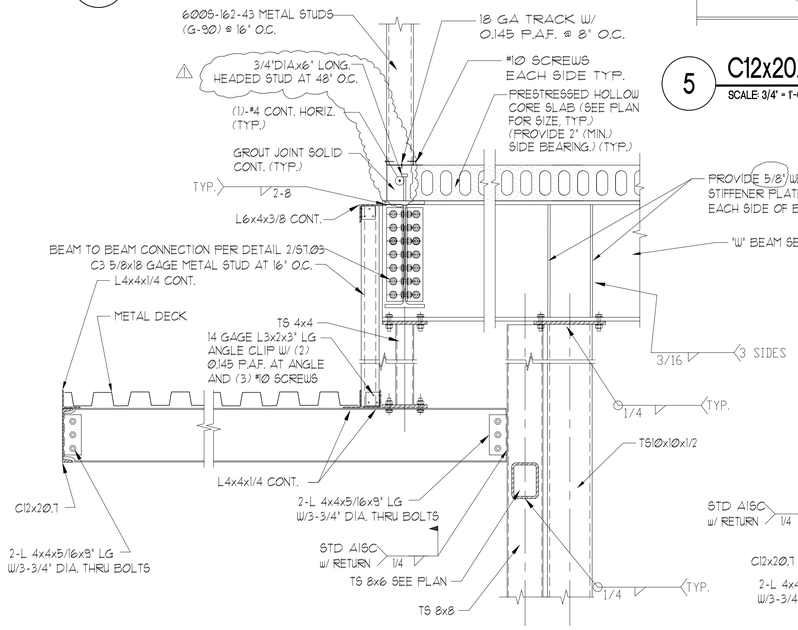


5 C12x20.7 FIELD SPLICE DETAIL
SCALE: 3/4" = 1'-0"



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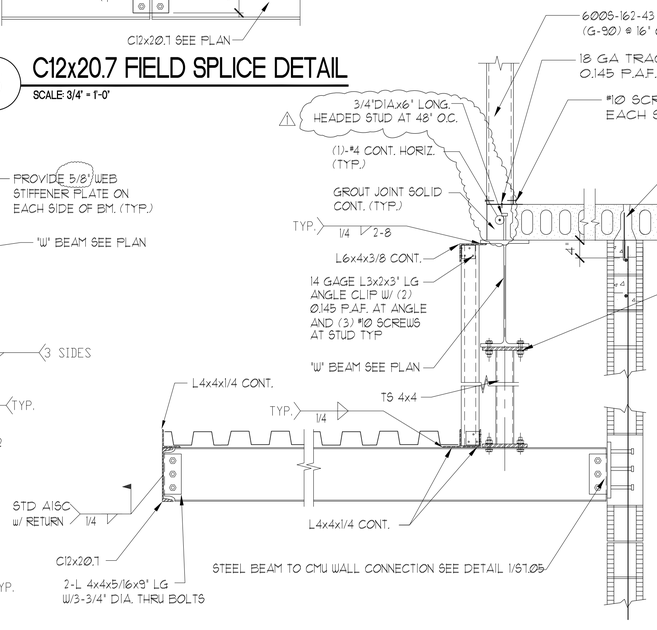

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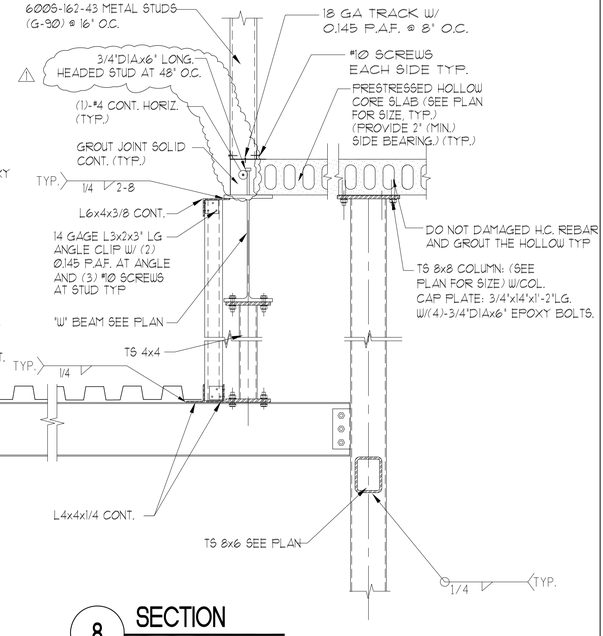

8 SECTION
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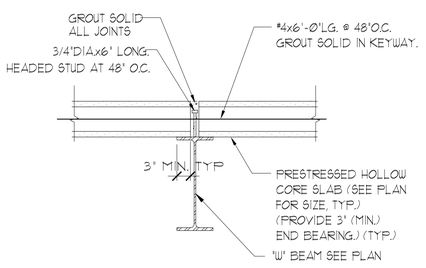

9 SECTION
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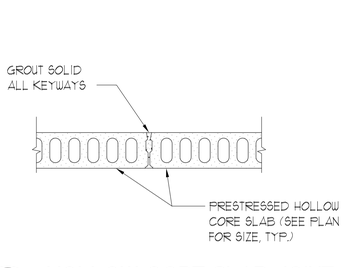
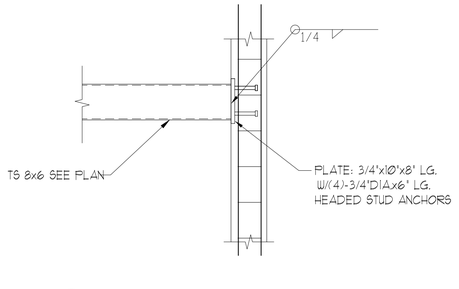
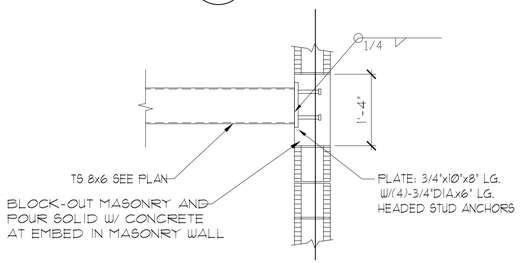


10 HOLLOW CORE SLAB JOINT
SCALE: 3/4" = 1'-0"



11 TS8x6 TO CONCRETE COL. DETAIL
SCALE: 3/4" = 1'-0"



12 TS8x6 TO CMU WALL DETAIL
SCALE: 3/4" = 1'-0"

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COURTYARD® Marriott®

REVISIONS

1	REVISIONS	03.10.2022

DRAWING NAME

SECTIONS AND DETAILS

PROJECT NAME

COURTYARD INN,°
Lake City, Florida

SEAL+SIGNATURE

DATE 07 DEC 2021
PROJECT NUMBER 2K2101
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DECOR: XH

S7.03

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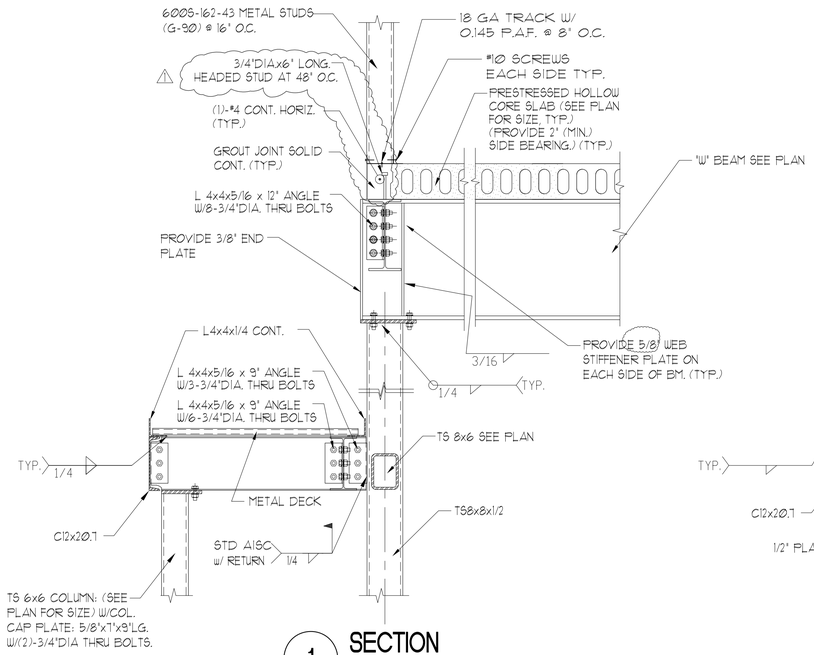

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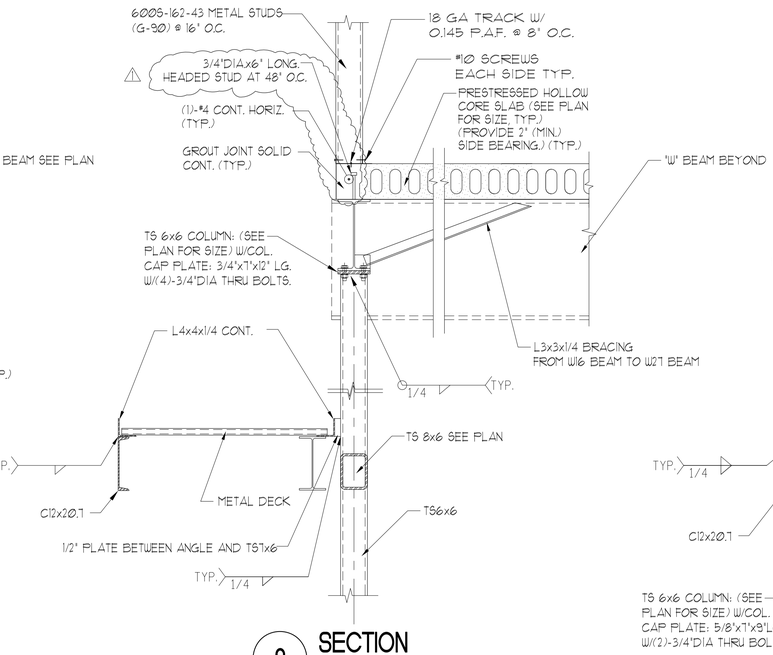

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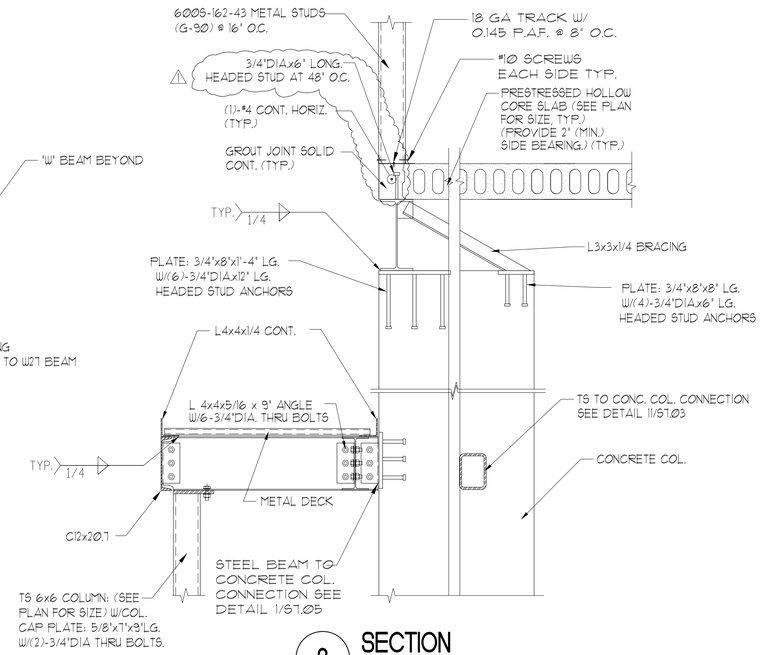

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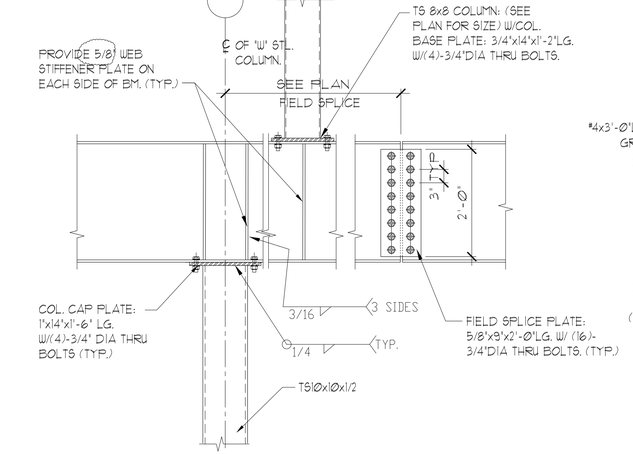

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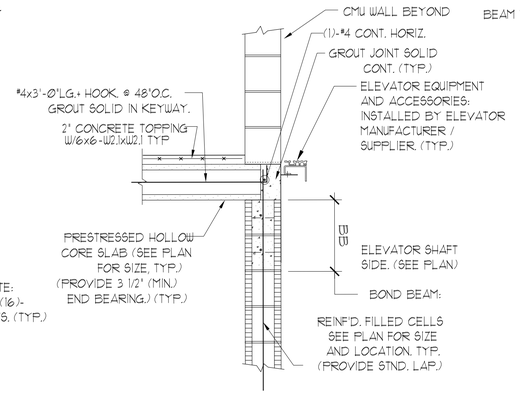
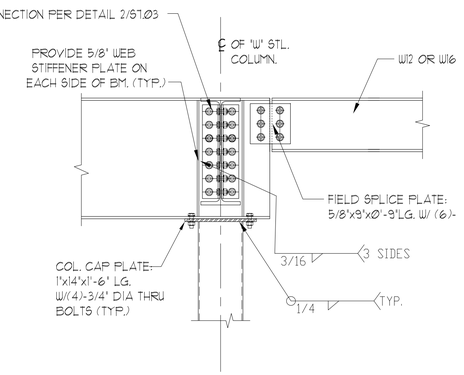


5 H.C. PLANK AT ELEVATOR
SCALE: 3/4" = 1'-0"



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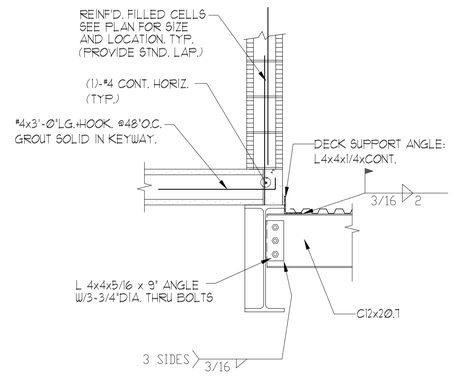

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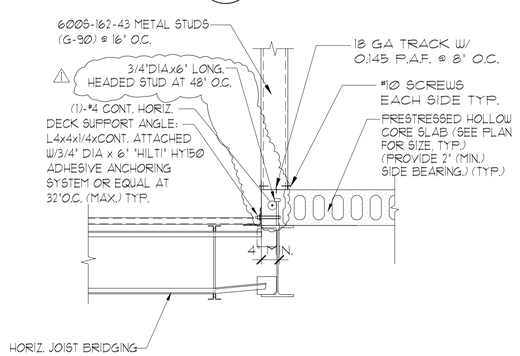

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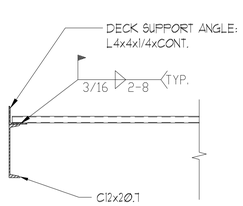

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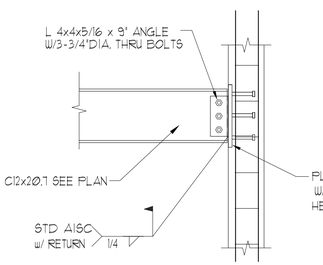
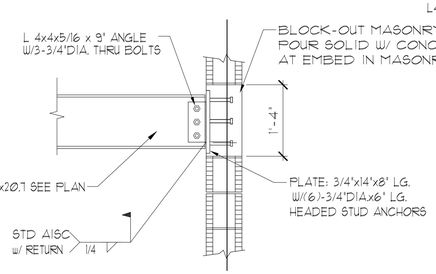


14 C12x20.7 TO CONCRETE COL. DETAIL
SCALE: 3/4" = 1'-0"



15 C12x20.7 TO CMU WALL DETAIL
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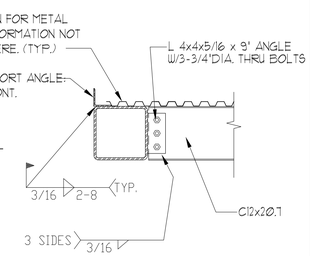

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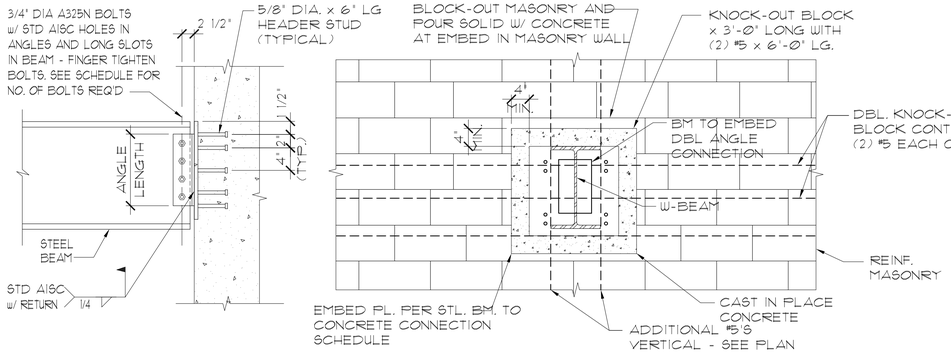


CONNECTION TO CONCRETE (UNLESS NOTED OTHERWISE)

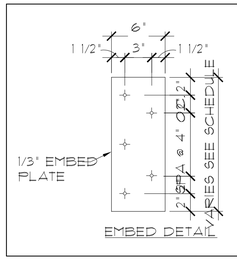
CONNECTION TO MASONRY (UNLESS NOTED OTHERWISE)

CONNECTION TO CONCRETE AT NARROW FACE

STL BM TO CONC CONNECTION SCHEDULE

STEEL BEAM SIZE	STEEL CLIP ANGLE CONNECTION		CONCRETE EMBED CONNECTION	
	2-L 4x3x5/16 ANGLE LENGTH (IN)	BOLTS	EMBED PLATE	HEADED STUDS (6" LG MIN)
W24, W27	18	6	1/2 x 15 x 2'-0"	(12) 3/4" DIA IN (4) ROWS
W18, W21	15	5	1/2 x 15 x 1'-6"	(9) 3/4" DIA IN (3) ROWS
W14, W16,	12	4	1/2 x 15 x 1'-3"	(9) 3/4" DIA IN (2) ROWS
W12	9	3	1/2 x 15 x 1'-3"	(6) 3/4" DIA IN (2) ROWS
STL JOIST TO CONC	SEE SECT	N/A	1/2 x 12 x 1'-0"	(4) 3/4" DIA IN (2) ROWS

NOTE: 1 ALL STUDS ARE 6" AND EQUALLY SPACED
2 MINIMUM EDGE DISTANCE FROM CL OF STUDS TO FACE OF CONCRETE SHALL NOT BE LESS THAN 4" WITHOUT APPROVAL BY ENGINEER
3 PROVIDE NELSON TYPE STUDS OR APPROVED EQUAL BY ARCHITECT



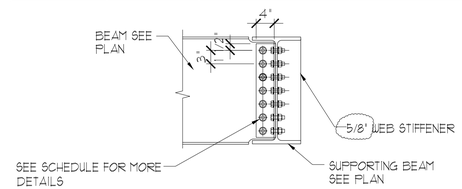
EMBED DETAIL

STL BM TO CONCRETE CONNECTION SCHEDULE AT END OF WALLS AND NARROW COLUMN FACE

STEEL BEAM SIZE	# OF BOLTS	SINGLE PLATE SIZE t	SINGLE PLATE SIZE l	SINGLE PLATE WELD TO EMBED	# OF STUDS	EMBED PLATE LENGTH
W10, W12	(3)	5/16"	9"	1/4" FILLET	(4)	12"
W14, W16	(4)	5/16"	12"	1/4" FILLET	(5)	16"
W18	(5)	3/8"	15"	5/16" FILLET	(6)	20"
W21	(6)	3/8"	18"	5/16" FILLET	(7)	24"
W24,W27	(7)	3/8"	21"	5/16" FILLET	(8)	28"

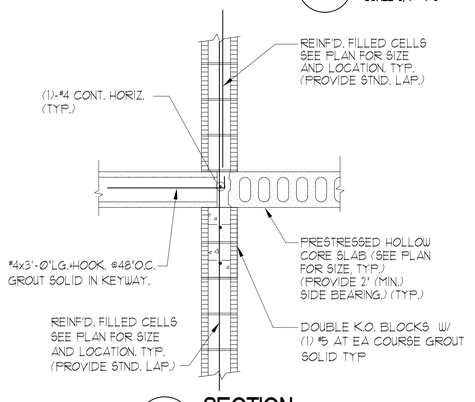
W-BEAM TO W-BEAM TYPICAL BOLTING CONNECTION SCHEDULE (U.N.O.)

SUPPORTING BEAM DEPTH	NUMBER OF A325-N BOLTS	NUMBER OF 3/4" DIA. (EA. SIDE) THRU BOLTS	2-L 4x4x5/16 x LENGTH ANGLES	
12, 14	3-3/4" DIA.	3	9"	
16	4-3/4"DIA.	4	12"	
18	5-3/4"DIA.	5	15"	
21	6-3/4"DIA.	6	18"	
24	7-3/4"DIA.	7	21"	
27	8-3/4"DIA.	8	24"	



2 TYPICAL BEAM TO BEAM CONNECTION
SCALE: N.T.S.

1 STL BM TO CONC CONNECTION SCHEDULE
SCALE: 3/4" = 1'-0"



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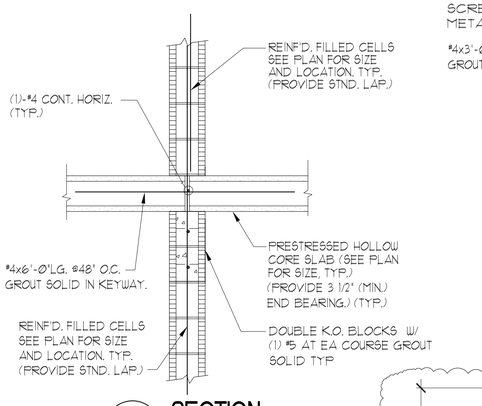
SCALE: 3/4" = 1'-0"



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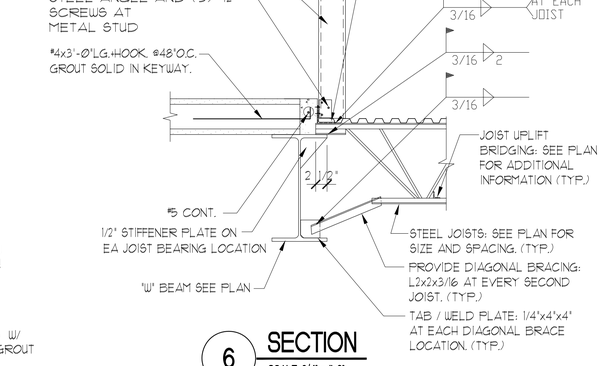

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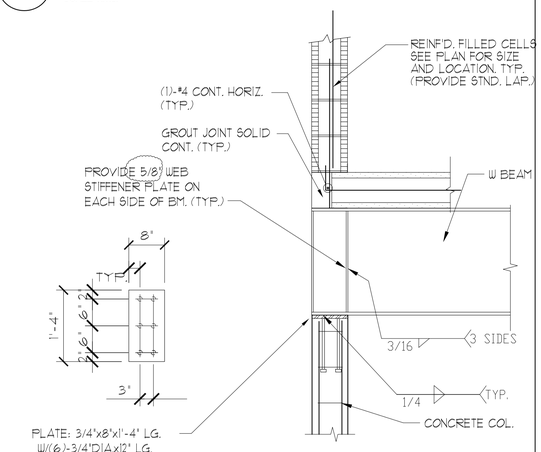

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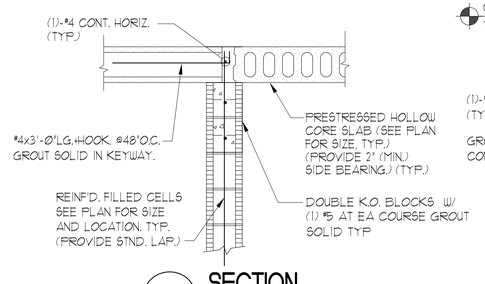

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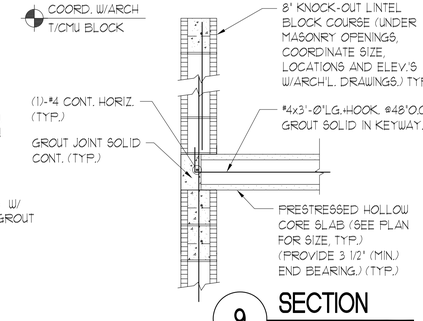

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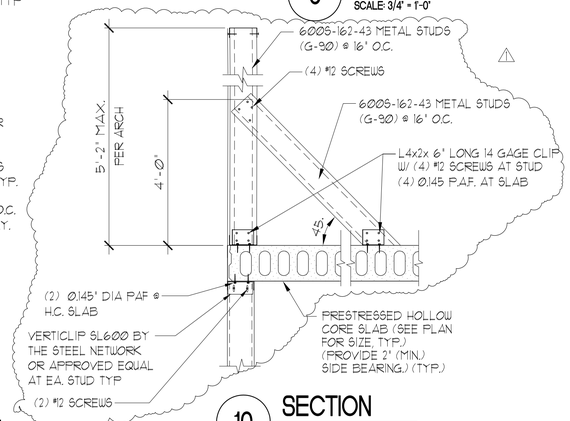
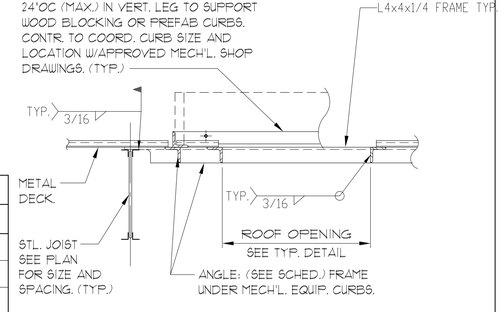


10 SECTION
SCALE: 3/4" = 1'-0"

ROOF TOP UNIT FRAME SCHEDULE	
UNIT WEIGHT	ANGLE SIZE:
0 - 675 LBS.	L4x3x1/4 (L.L.H.)
676 - 1500 LBS.	L4x4x5/16
1501 - 3000 LBS.	L6x3x3/8 (L.L.H.)
3001 - 6000 LBS.	L6x6x3/8
6001 - 10000 LBS.	L8x6x3/4

NOTE:
SEE SECTION 3/69 FOR ADDITIONAL JOIST REINFORCING INFORMATION, (AS REQUIRED)



7 ROOF MECH UNIT FRAMING
SCALE: 3/4" = 1'-0"

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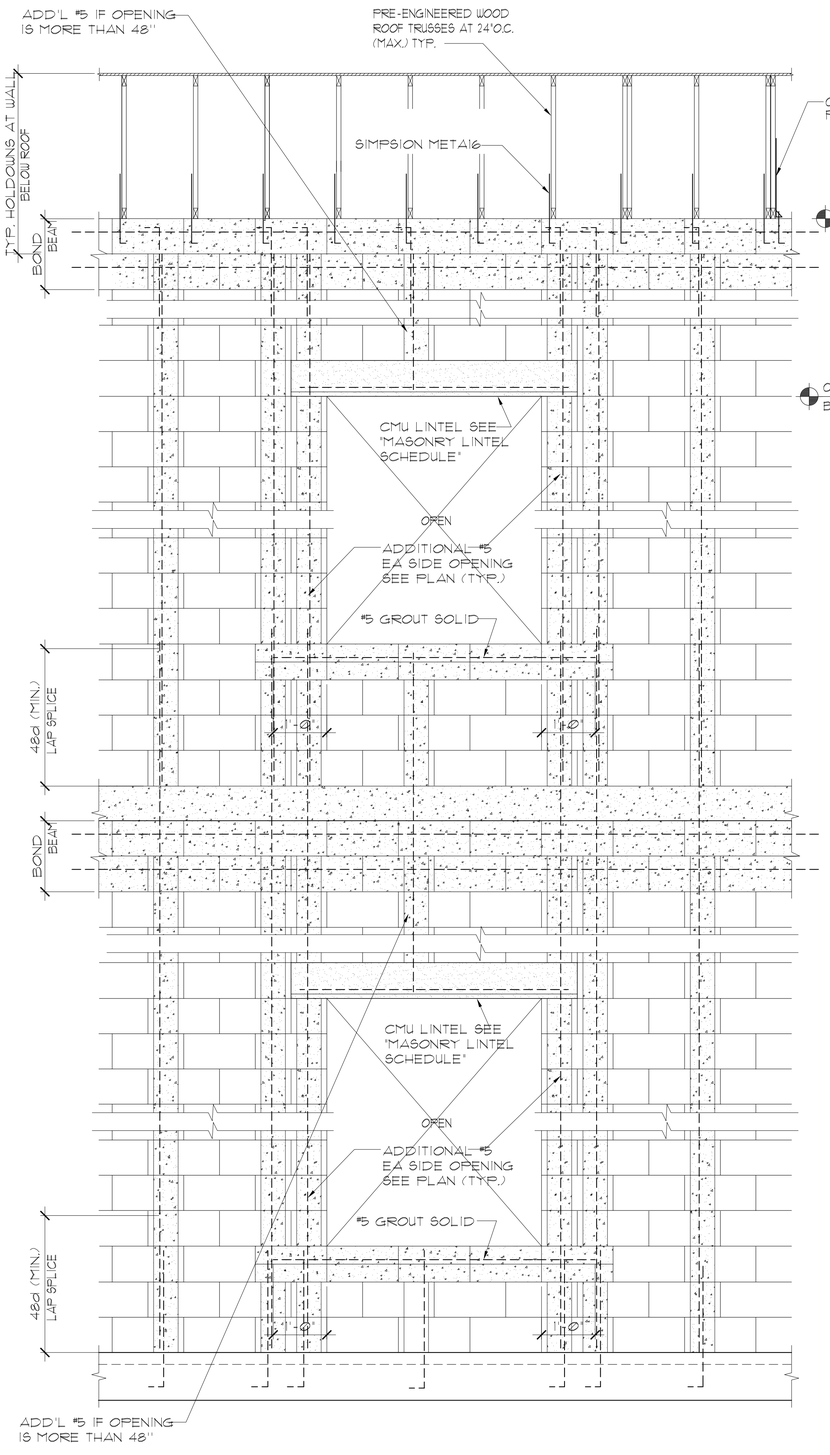
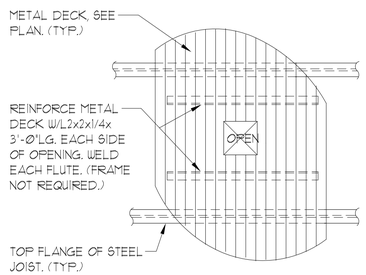


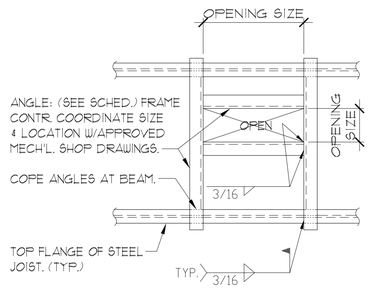
1 TYPICAL CMU BEARING WALL ELEVATION

SCALE: 3/4" = 1'-0"



SMALL OPENING DETAIL

SMALLER THAN 12" SQ. & LARGER THAN 6' SQ.

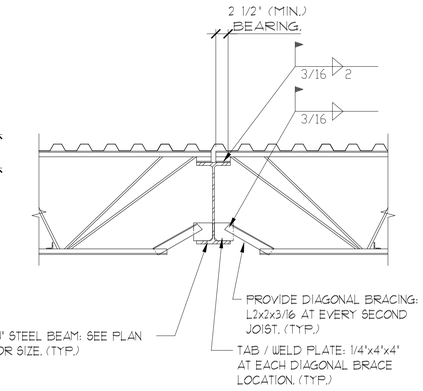


LARGE OPENING DETAIL

LARGER THAN 12" SQUARE OR ROUND

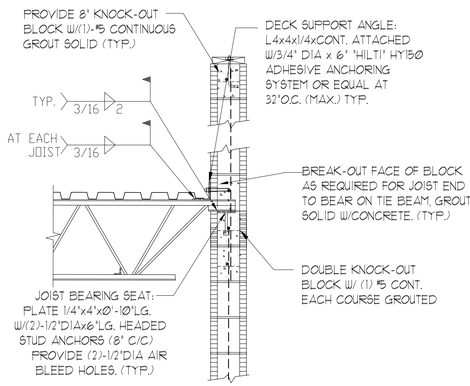
2 ROOF OPENING DETAILS

SCALE: 3/4" = 1'-0"



3 JOIST BEARING AT STEEL BEAM

SCALE: 3/4" = 1'-0"



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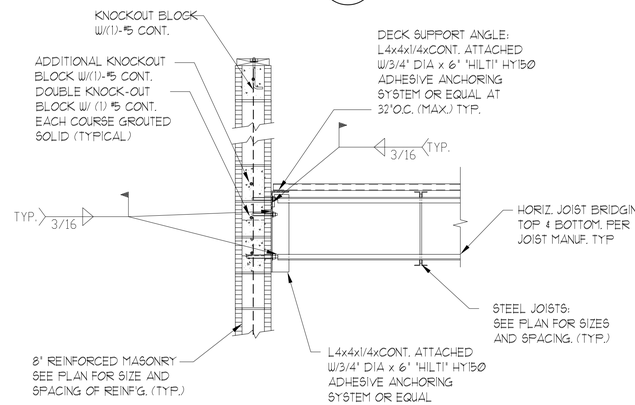
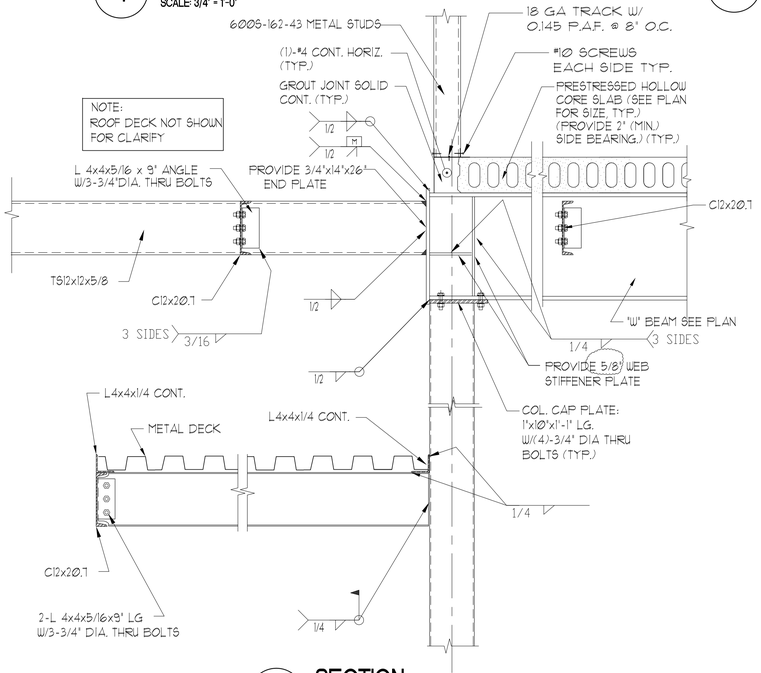


5 JOIST BRIDGING AT EXTER. WALL

SCALE: 3/4" = 1'-0"



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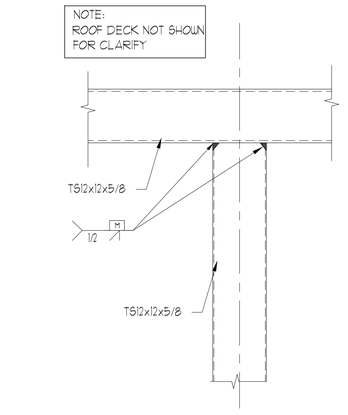

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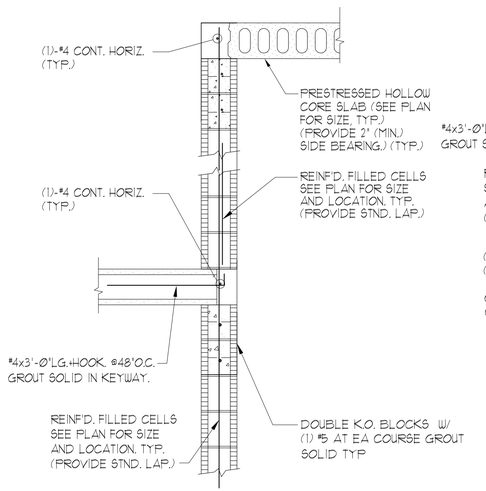

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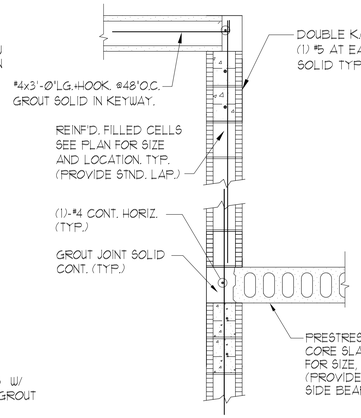

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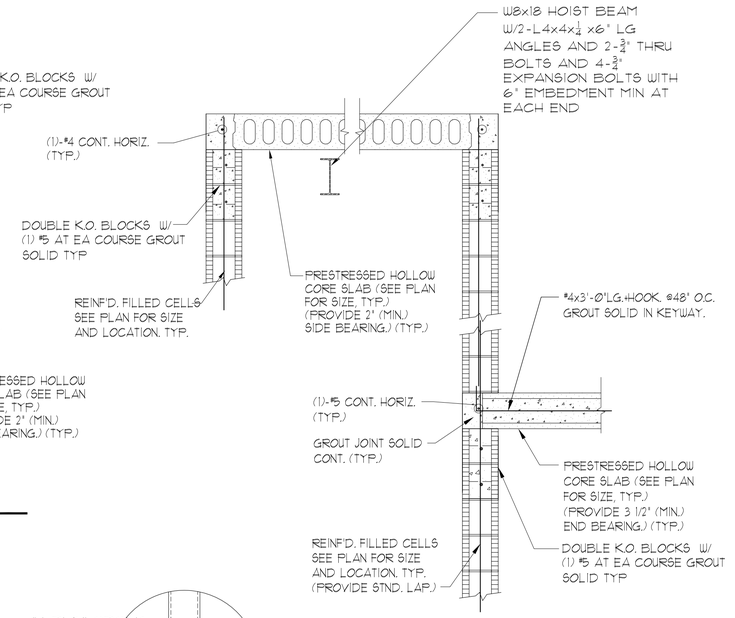

3 SECTION AT ELEVATOR
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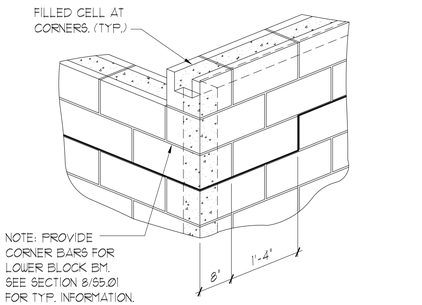
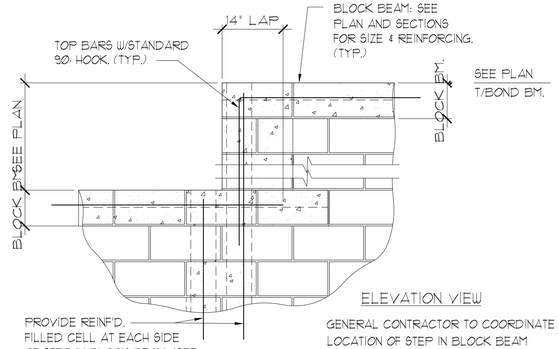
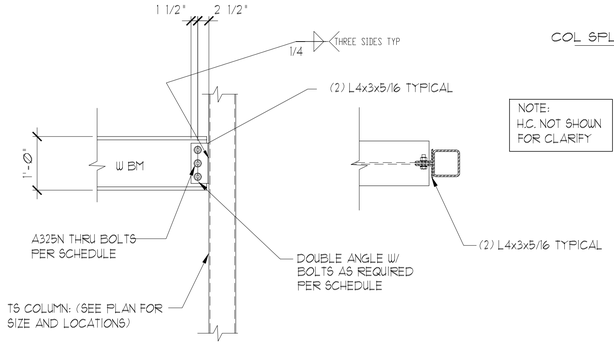




ELEVATION VIEW

GENERAL CONTRACTOR TO COORDINATE LOCATION OF STEP IN BLOCK BEAM (CHANGE IN ELEVATIONS) WITH TRUSS FRAMING (PRE-ENGINEERED TRUSS MANUFACTURER'S APPROVED SHOP DRAWINGS) AND TO SUIT BLOCK COURSING.

4 STEPPED BLOCK BEAM DETAIL
SCALE: 3/4" = 1'-0"



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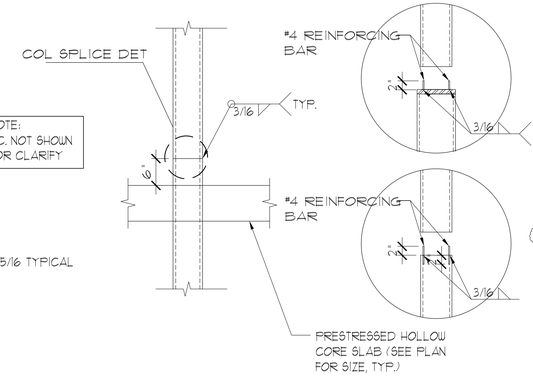
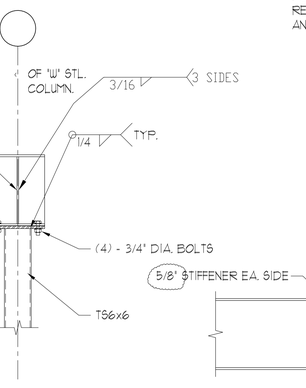


6 TYP TS COL SPLICE
SCALE: 3/4" = 1'-0"



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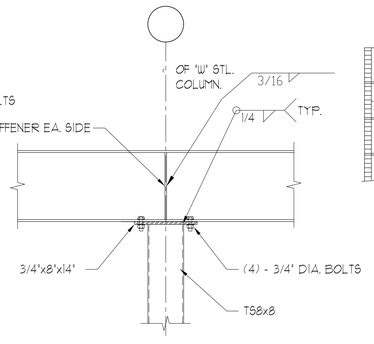

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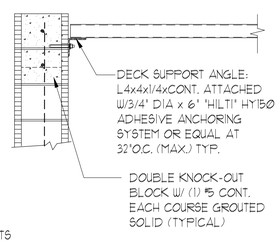

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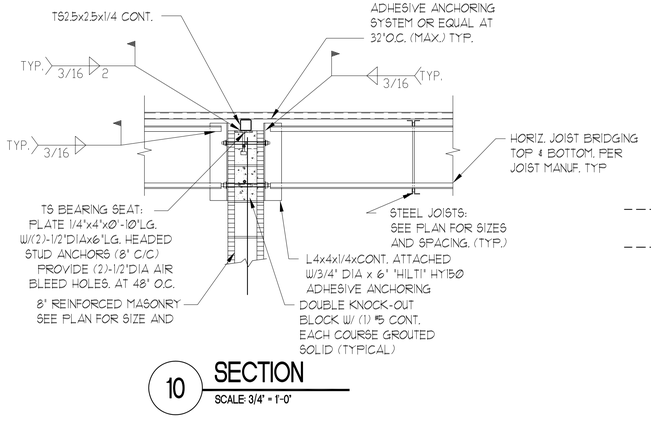

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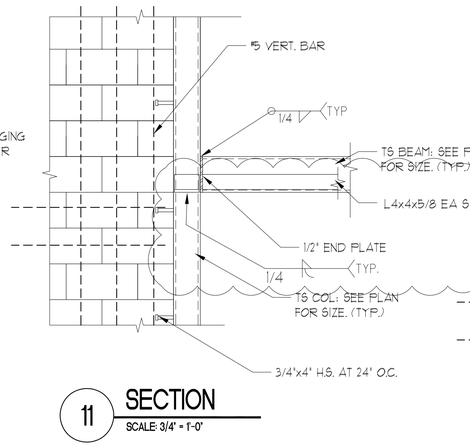

11 SECTION
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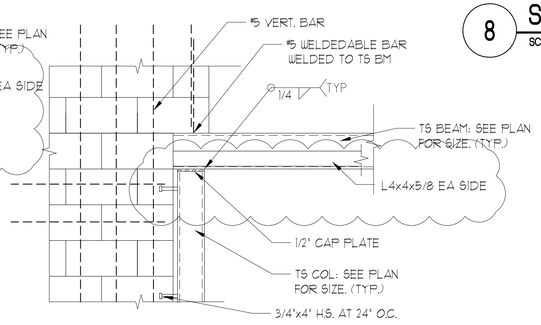

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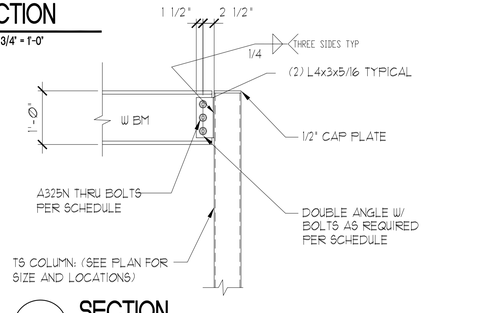

12 SECTION
SCALE: 3/4" = 1'-0"

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COURTYARD® Marriott®

REVISIONS

1	REVISIONS	03.10.2022

DRAWING NAME

SECTIONS AND DETAILS

PROJECT NAME

COURTYARD INN,® Lake City, Florida

SEAL+SIGNATURE

DATE 07 DEC 2021
PROJECT NUMBER 2K2101
DRAWING NUMBER
DECOR: XH
PAGE NUMBER

S7.07

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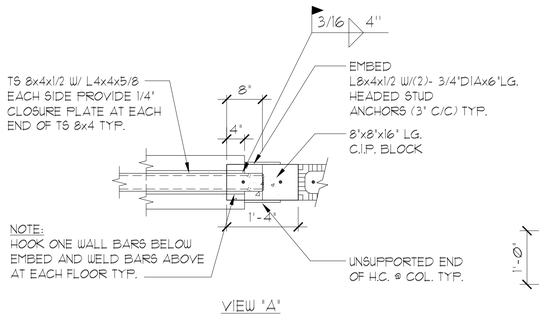
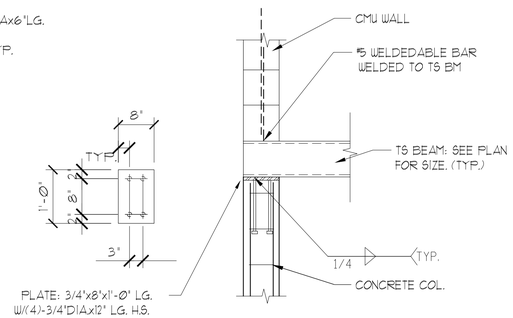
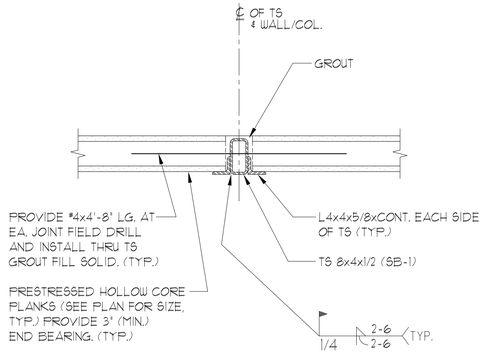
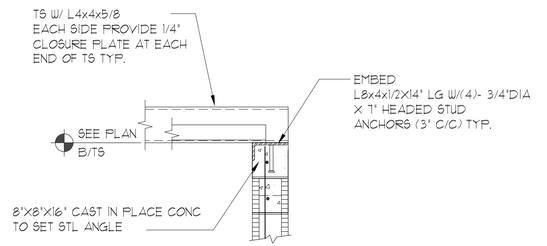


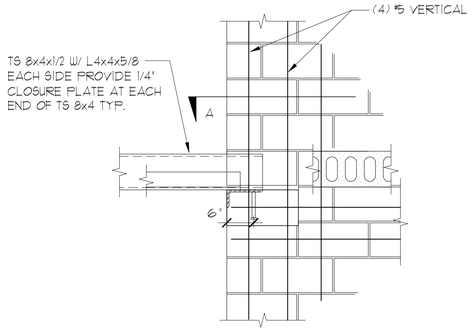
VIEW "A"







5 TS CONN. AT CMU WALL
SCALE: N.T.S.



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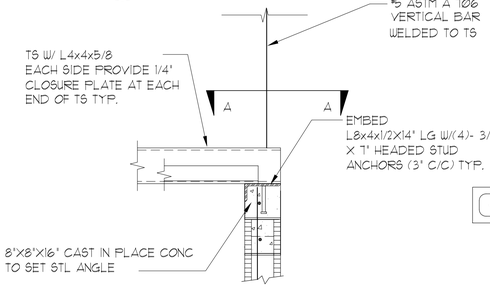
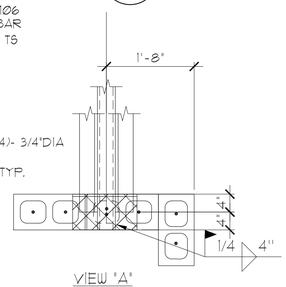


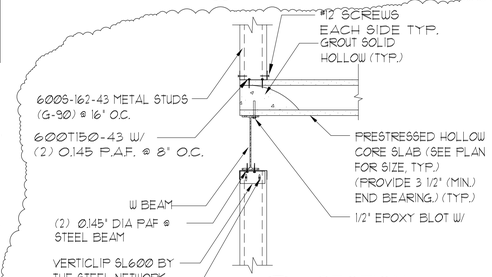
3 H.C. PLK. END BRG. AT TS 8x4
SCALE: 3/4" = 1'-0"



VIEW "A"

NOTE: ONLY ONE SIDE OF STEEL ANGLES AND TUBE SHALL BE WELDED TO STEEL EMBEDDED PLATE DURING HOLLOWCORE ERECTION. SHORING SHALL BE PROVIDED DURING AND AFTER HOLLOWCORE ERECTION. CONTRACTOR SHALL WELD OTHER SIDE OF STEEL MEMBERS AFTER HOLLOWCORE ERECTION IS COMPLETE FOR THE ENTIRE BUILDING.

4 TS CONN. AT CMU WALL
SCALE: N.T.S.



1 TS 8x4 CONN. AT CMU WALL
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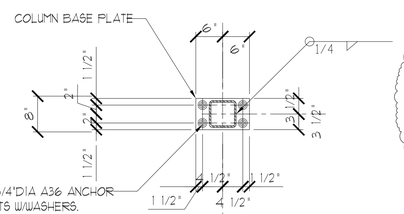

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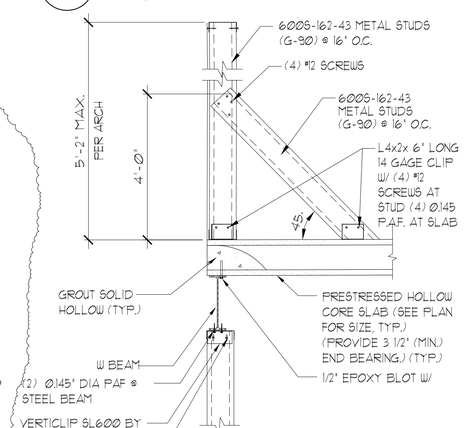


9 SECTION
SCALE: 3/4" = 1'-0"

'TS' STEEL COLUMN: SEE PLAN FOR SIZE AND LOCATIONS.

℄ OF 'TS' STL. COLUMN.

COL. BASE PLATE: 3/4"x1'x1'-0" LG. W/(4)-3/4"DIA THRU BOLTS (TYP.)

5/8" STIFFENER EA. SIDE

5/8" STIFFENER EA. SIDE

3/16

1/4 TYP.

3/16

3 SIDES

3/4"x8"x14"

(4) - 3/4" DIA. BOLTS

W BEAM

TS8x8

7 SECTION
SCALE: 3/4" = 1'-0"

OTHERS

1. WATERPROOFING DESIGN IS NOT IN THE STRUCTURAL SCOPE OF WORK, HENCE NO DETAILS OF FLASHING, FLOOR, SLOPE AND STEP, NOR ANY OTHER WATERPROOFING MEASURES HAVE BEEN INCLUDED IN OUR DRAWINGS. IF SLOPE AND FLOOR STEPS ARE SHOWN ON OUR DRAWINGS, THEY SHALL BE USED ONLY AS A REFERENCE. FLOOR SLOPE AND STEPS SHALL FOLLOW ARCHITECTURAL DRAWINGS.

2. CONTRACTOR SHALL VERIFY THAT THE TRUSS LAYOUT SUPPLIED BY THE TRUSS MANUFACTURER MATCHES THE TRUSS LAYOUT SHOWN ON THE STRUCTURAL DRAWINGS. IF THERE IS ANY DISCREPANCY BETWEEN TRUSS LAYOUT SHOWN ON TRUSS SHOP DRAWINGS AND TRUSS LAYOUT SHOWN ON STRUCTURAL DRAWINGS, CONTRACTOR SHALL STOP CONSTRUCTION AND INFORM IN WRITING THE ENGINEER OF RECORD. CONTRACTOR SHALL WAIT FOR ENGINEER OF RECORD RESPONSE IN WRITING PRIOR TO START OF CONSTRUCTION.

3. ALL STUCCO AND/OR VENEER DESIGNS ARE NOT IN THE SCOPE OF WORK FOR THE STRUCTURAL ENGINEER.

4. PAVER AND SIDE WALK DESIGNS ARE NOT IN THE SCOPE OF WORK FOR THE STRUCTURAL ENGINEER.

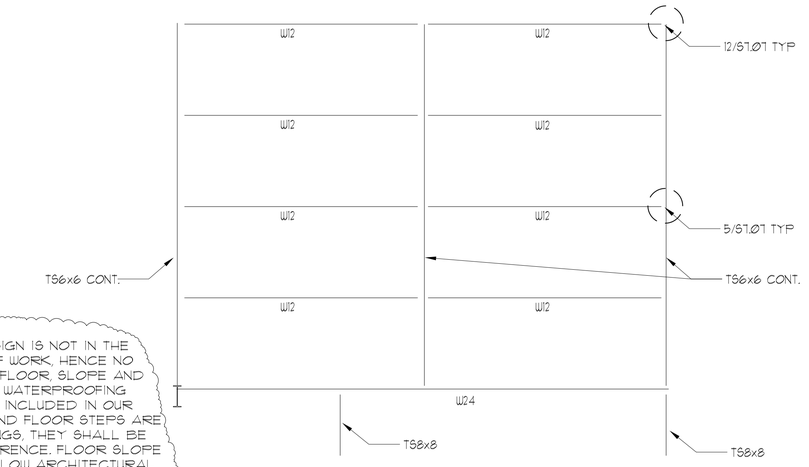
5. CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS. INFORM ARCH/ENG IF DIMENSIONS DO NOT MATCH. CONTRACTOR SHALL GET A RESPONSE FROM ARCHITECT/ENGINEER IN WRITING TO RESOLVE THE PROBLEMS PRIOR TO STARTING CONSTRUCTION.

6. BUILDING MAINTENANCE SUCH AS PAINTING AND SEALING ETC. IS THE HOME OWNER SOLELY RESPONSIBLE.

7. CONTRACTOR TO PROVIDE CRACK ISOLATION MEMBRANE OVER SLAB AT AREAS TO BE TILED PRIOR TO INSTALLATION OF FLOOR TILE.

8. ALL REBAR SHOWN ON ALL DRAWING DETAILS AND SECTIONS IS CONTINUOUS UNLESS NOTICE OTHERWISE.

9. THE SECTIONS SHOWN ON THE DESIGN DRAWINGS INDICATE THE TYPICAL SECTIONS WHICH ARE APPLIED TO ALL SIMILAR BUILDING CONDITIONS.



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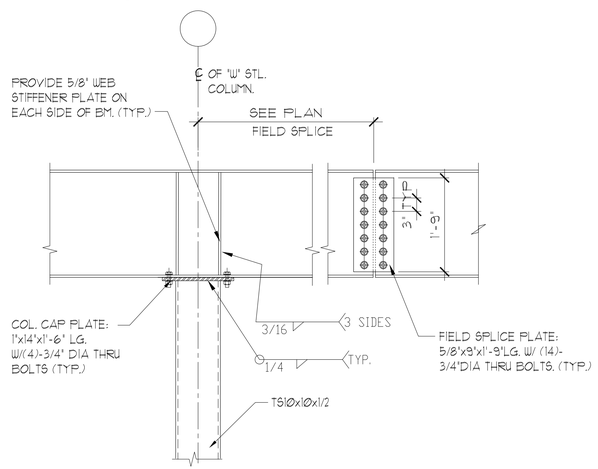

10 SECTION
SCALE: 3/4" = 1'-0"

03-10-2022

ASE ENGINEERING SERVICES, INC.
STRUCTURAL DESIGN GROUP
"TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE, THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS."
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COURTYARD® Marriott®

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COURTYARD INN,°
Lake City, Florida

SEAL+SIGNATURE

DATE 07 DEC 2021
PROJECT NUMBER 2K2101
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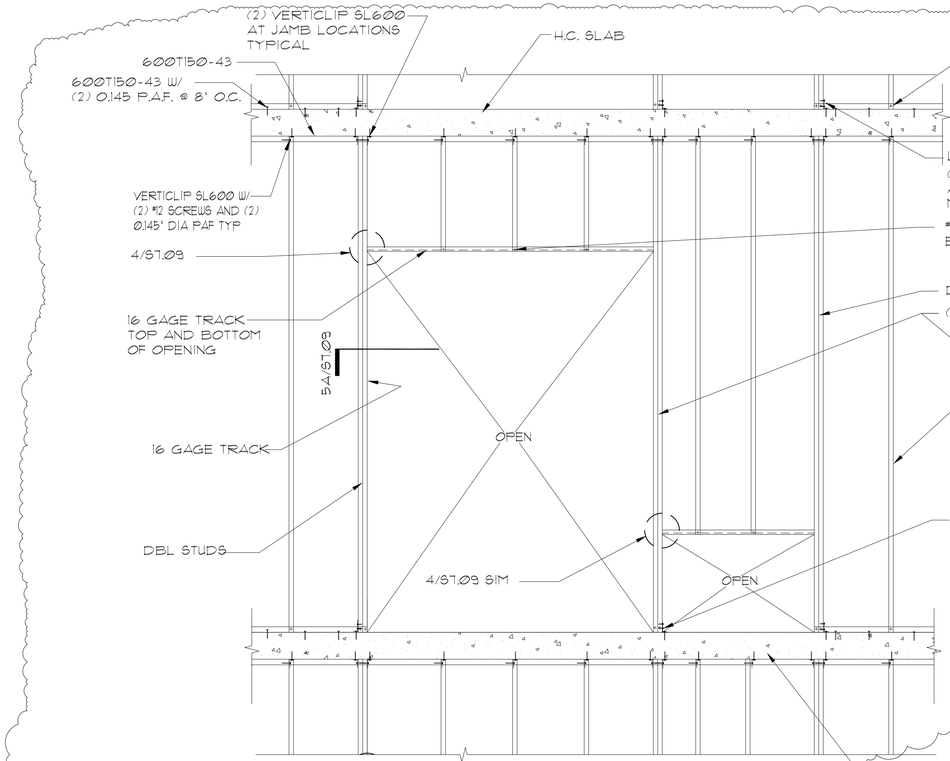
DECOR: XH

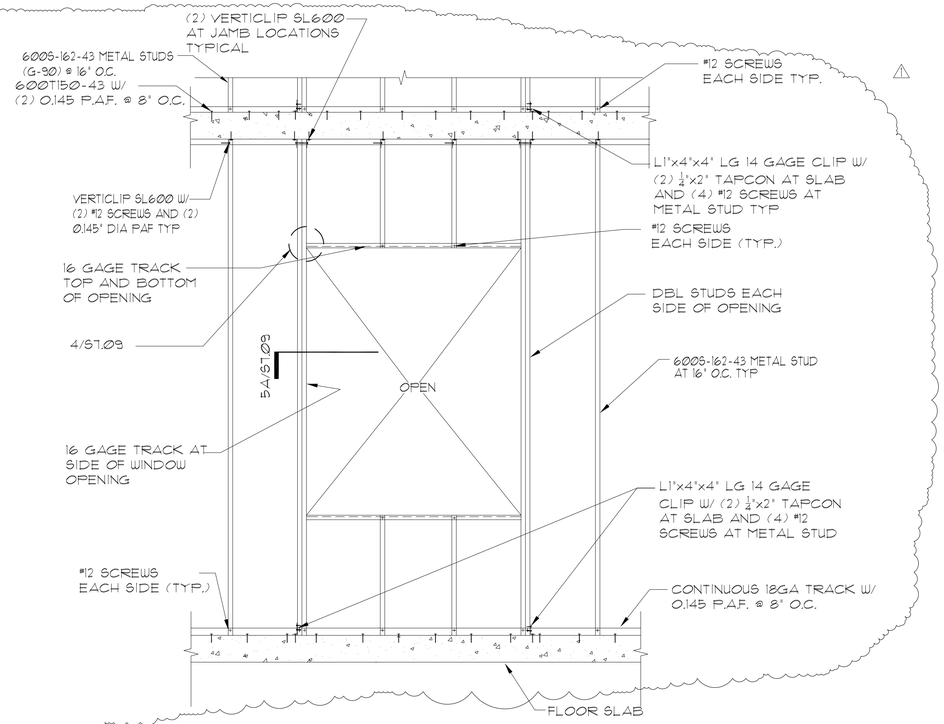
S7.08

PAGE NUMBER



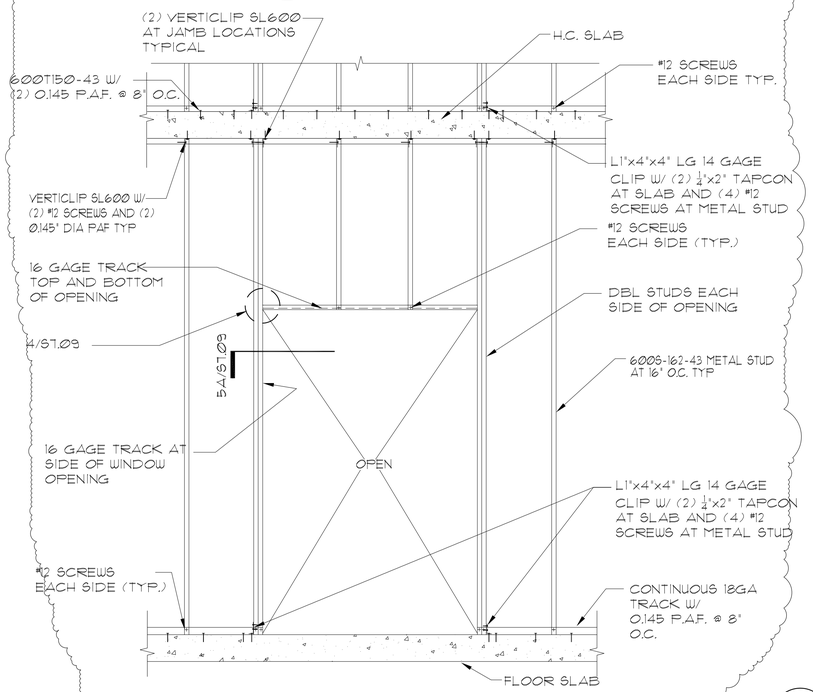
TYPICAL EXTER. METAL STUD WALL WINDOW OPENING W/ AC OPENING

1 SCALE: 3/4" = 1'-0"



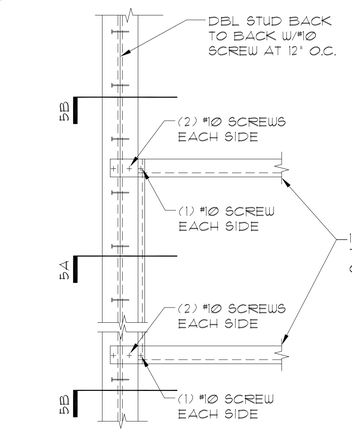
TYPICAL EXTER. METAL STUD WALL WINDOW OPENING < 6'-0"

2 SCALE: 3/4" = 1'-0"



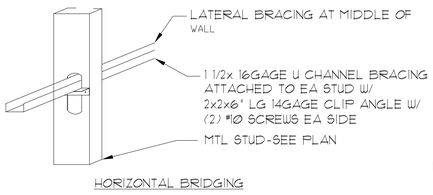
TYPICAL EXTER. METAL STUD WALL DOOR OPENING < 6'-0"

3 SCALE: 3/4" = 1'-0"



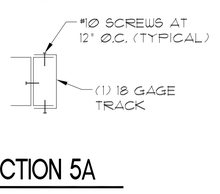
OPENING < 5'-0"

4 SCALE: 3/4" = 1'-0"

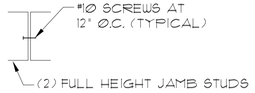


TYP. WALL LATERAL BRACING DETAIL

6 SCALE: 3/4" = 1'-0"



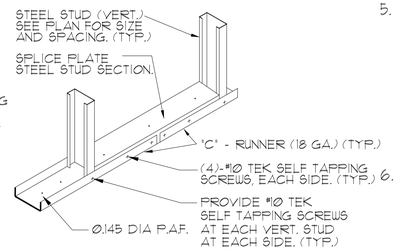
SECTION 5A



SECTION 5B

TYPICAL SECTION

5 SCALE: N.T.S.



TYP. "C"-RUNNER SPLICE

7 SCALE: 3/4" = 1'-0"

EXTERIOR METAL STUD FRAMING SPECIFICATIONS

FRAMING CONTRACTOR:

1. ALL LIGHT STEEL FRAMING SHALL BE COLD-FORMED GALVANIZED FORMED STEEL, WITH G-50 MINIMUM COALTING CONFORMING WITH WITH ASTM A446. ALL STEEL THAT IS 18 GAGE OR LIGHTER SHALL BE GRADE A, WITH MINIMUM YEILD STRENGTH OF 33 KSI. ALL STEEL THAT IS 16 GAGE OR HEAVIER SHALL BE GRADE D, WITH A MINIMUM YIELD STRENGTH OF 50 KSI.

2. SCREW FASTENERS SHALL BE OF THE NUMBER AND SPACING AS SHOWN IN THE DETAILS, USING #12-16 SELF-DRILLING SCREWS AT EXTERIOR FRAMING CONNECTIONS. FRAMING SCREWS SHALL BE ZINC OR CAD-PLATED, OR OTHERWISE TREATED WITH A CORROSION INHIBITOR.

3. ALL POWDER-ACTUATED FASTENERS (DENOTED AS P.A.F.'S IN THE DETAIL DRAWINGS) TO STRUCTURAL STEEL SHALL BE Ø.145" DIAMETER STEEL PINS. ALL P.A.F.'S TO CONCRETE SHALL BE Ø.145" DIA. X 1-1/4" LONG PINS.

4. ALL STUDS SHALL BE FULL LENGTH. NO SPLICING PERMITTED UNLESS SPECIFICALLY DETAILED.

5. MATERIALS: STUDS, RUNNERS AND ANGLES SHALL MEET THE REQUIREMENTS OF ASTM 1446 WITH MINIMUM YIELD STRENGTH AS FOLLOWS:

16, 14, AND 12 GAGE STUDS	50 KSI
22, 20, AND 18 GAGE STUDS	33 KSI
RUNNERS	33 KSI

6. CONTRACTOR SHALL VERIFY THE LENGTH OF METAL STUDS IN FIELD PRIOR TO FABRICATE THEM.

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SECTIONS AND DETAILS

PROJECT NAME

COURTYARD INN,°
Lake City, Florida

SEAL+SIGNATURE

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