

Columbia County New Building Permit Application

For Office Use Only		Application # <u>43856</u>	Date Received <u>10/21</u>	By <u>MG</u>	Permit # <u>38878/38879</u>
Zoning Official <u>LU/UA</u>	Date <u>10-23-19</u>	Flood Zone <u>X</u>	Land Use <u>Ag</u>	Zoning <u>A-3</u>	
FEMA Map # _____	Elevation _____	MFE <u>46'</u>	River _____	Plans Examiner <u>T.C.</u>	Date <u>10-31-19</u>
Comments					
<input checked="" type="checkbox"/> NOC <input checked="" type="checkbox"/> EH <input checked="" type="checkbox"/> Deed or PA <input checked="" type="checkbox"/> Site Plan <input type="checkbox"/> State Road Info <input type="checkbox"/> Well letter <input checked="" type="checkbox"/> 911 Sheet <input type="checkbox"/> Parent Parcel # _____					
<input type="checkbox"/> Dev Permit # _____ <input type="checkbox"/> In Floodway <input type="checkbox"/> Letter of Auth. from Contractor <input type="checkbox"/> F W Comp. letter					
<input type="checkbox"/> Owner Builder Disclosure Statement <input type="checkbox"/> Land Owner Affidavit <input type="checkbox"/> Ellisville Water <input checked="" type="checkbox"/> App Fee Paid <input checked="" type="checkbox"/> Sub VF Form					

Septic Permit No. 19-6780 **OR City Water** ☐ **Fax** 877-835-2575

Applicant (Who will sign/pickup the permit) Garrett Buzbee **Phone** 386-454-2555

Address 17774 US Hwy 441, High Springs, FL 32643

Owners Name Reginald & Regina Crutchfield **Phone** 850-758-0318

911 Address 816 SW Heflin Ave, Fort White, FL 32038

Contractors Name G Buzbee Inc. **Phone** 386-454-2555

Address 17774 US Hwy 441, High Springs, FL 32643

Contractor Email info@gbuzbee.com *****Include to get updates on this job.**

Fee Simple Owner Name & Address Reginald Crutchfield / 13622 NW 143rd Place, Alachua, FL 326165

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address G Buzbee Inc. / Schafer Engineering, LLC

Mortgage Lenders Name & Address N/A

Circle the correct power company ☐ **FL Power & Light** ☐ **Clay Elec.** ☐ **Suwannee Valley Elec.** ☒ **Duke Energy**

Property ID Number 30-7S-17-10058-675 **Estimated Construction Cost** \$250,000

Subdivision Name Santa Fe River Plantation **Lot** 85 **Block** - **Unit** - **Phase** -

Driving Directions from a Major Road From US 27 in High Springs.... North on US 27 across Santa Fe River, first left on SW Mapelton St, first right on SW Heflin Ave.... 2nd parcel on left.

Construction of New Single Family Res. **Commercial** ☐ **OR** ☒ **Residential**

Proposed Use/Occupancy Single Family Res. **Number of Existing Dwellings on Property** 0

Is the Building Fire Sprinkled? N/A **If Yes, blueprints included** N/A **Or Explain** N/A

Circle Proposed ☐ **Culvert Permit** **or** ☒ **Culvert Waiver** **or** ☐ **D.O.T. Permit** **or** ☐ **Have an Existing Drive**

Actual Distance of Structure from Property Lines - Front ~55' **Side** ~150' **Side** ~150' **Rear** ~60'

Number of Stories 1 **Heated Floor Area** 2110sf **Total Floor Area** 3285sf **Acreage** 1.63

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) _____

Columbia County Building Permit Application

CODE: Florida Building Code 2017 and the 2014 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: **YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

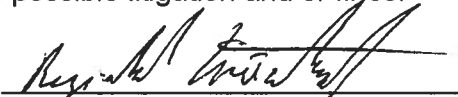
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

Reginald Crutchfield

Print Owners Name


Owners Signature

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.


Contractor's Signature

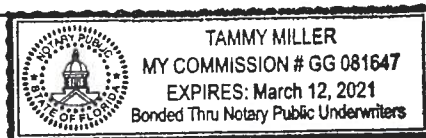
Contractor's License Number CGC 061980
Columbia County
Competency Card Number 672 ✓

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 18 day of Oct 2019

Personally known ☒ or Produced Identification


State of Florida Notary Signature (For the Contractor)

SEAL:



Legend

2018Aerials



Parcels

Roads

Roads
others

Dirt
Interstate
Main
Other
Paved
Private
Addresses

2018 Flood Zones

0.2 PCT ANNUAL CHANCE

A
AE
AH

Columbia County, FLA - Building & Zoning Property Map

Printed: Wed Nov 06 2019 13:23:21 GMT-0500 (Eastern Standard Time)



Parcel Information

Parcel No: 30-7S-17-10058-675

Owner: MCALHANY MICHAEL J &

Subdivision: SANTA FE RIVER PLANTATIONS REPLAT OF LOTS 38,45,46

Lot: 85

Acres: 1.6310221

Deed Acres: 1.63 Ac

District: District 2 Rocky Ford

Future Land Uses: Agriculture - 3

Flood Zones:

Official Zoning Atlas: A-3

sketch use for
911 Address
JMS

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Columbia County Property Appraiser

Jeff Hampton

2019 Preliminary Certified Values

updated: 8/14/2019

Parcel: << 30-7S-17-10058-675 >>

Owner & Property Info

Result: 1 of 1

Owner	CRUTCHFIELD REGINALD G & REGINA H CRUTHFIELD 13622 NW 143RD PLACE ALACHUA, FL 32615		
Site	816 HEFLIN AVE,		
Description*	LOT 85 SANTA FE RIVER PLANTATIONS REPLAT OF LOT 38. ORB 441-211,754-996, 816-1878, LE 821-339, DC 1056-223, WD 1056-224, WD 1388- 1596,		
Area	1.63 AC	S/T/R	30-7S-17
Use Code**	VACANT (000000)	Tax District	3

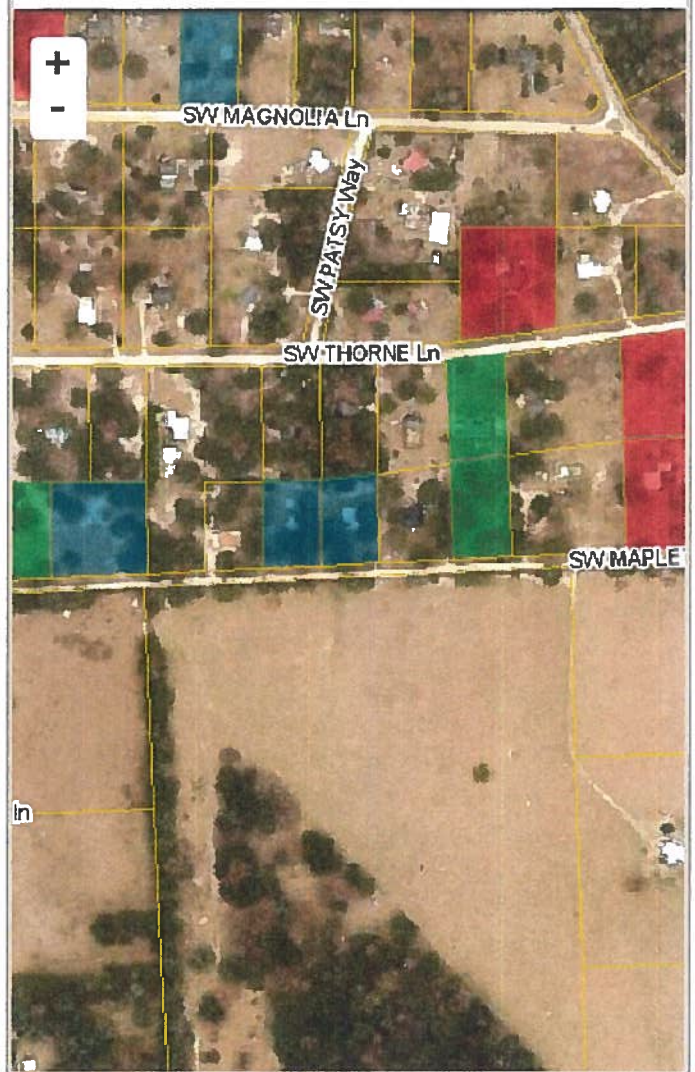
*The Description above is not to be used as the Legal Description for this parcel in any legal transaction.

**The Use Code is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.

Property & Assessment Values

2018 Certified Values		2019 Preliminary Certified	
Mkt Land (1)	\$17,930	Mkt Land (1)	\$17,930
Ag Land (0)	\$0	Ag Land (0)	\$0
Building (0)	\$0	Building (0)	\$0
XFOB (0)	\$0	XFOB (0)	\$0
Just	\$17,930	Just	\$17,930
Class	\$0	Class	\$0
Appraised	\$17,930	Appraised	\$17,930
SOH Cap [?]	\$0	SOH Cap [?]	\$0
Assessed	\$17,930	Assessed	\$17,930
Exempt	\$0	Exempt	\$0
Total Taxable	county:\$17,930 city:\$17,930 other:\$17,930 school:\$17,930	Total Taxable	county:\$17,930 city:\$17,930 other:\$17,930 school:\$17,930

Aerial Viewer Pictometry Google Maps

☒ 2019
 ☐ 2016
 ☐ 2013
 ☐ 2010
 ☐ 2007
 ☐ 2005
 ☒ Sales
**▼ Sales History**

Sale Date	Sale Price	Book/Page	Deed	V/I	Quality (Codes)	RCode
7/10/2019	\$40,000	1388/1596	WD	V	Q	01
12/8/2005	\$70,000	1068/2537	WD	V	U	09
8/19/2005	\$49,900	1056/0224	WD	V	Q	
1/22/1996	\$9,000	816/1878	WD	V	Q	
12/17/1991	\$8,000	754/0996	WD	V	Q	

▼ Building Characteristics

Bldg Sketch	Bldg Item	Bldg Desc*	Year Blt	Base SF	Actual SF	Bldg Value
NONE						

▼ Extra Features & Out Buildings (Codes)

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

▼ Land Breakdown

Land Code	Desc	Units	Adjustments	Eff Rate	Land Value
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PLAT BOOK 5 PAGE 13
SHEET 1 OF 5 SHEETS

81 MAR 20 P 1

Sheet Index

1. Legal Description, Dediciations, & Approvals

Report Map of the area for 33.

Server's Certification

Date _____

By _____
Harold E. Lingo
Reg. att'd. Rivers and Singer 3/25/50

Approvals of Columbia County
County Attorney's Certification

I hereby certify that I have examined the foregoing bill and find it correct in form with the California County Subdivision Laws and Chapter 19 of the General Statutes.

Date 11/25/11 County Attorney _____

The 10:00 a.m. meeting was approved by the following
 Jimmy Board in County Commissioners is accepted for
 this and recorded the 20th day of March 20 81
 1880 13-130

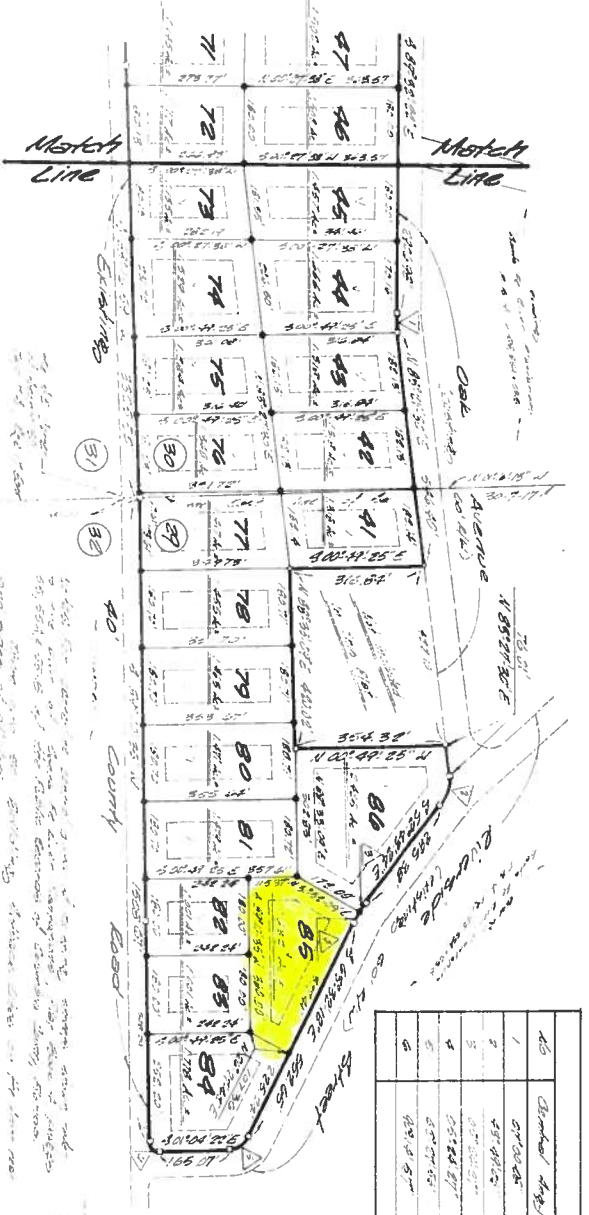
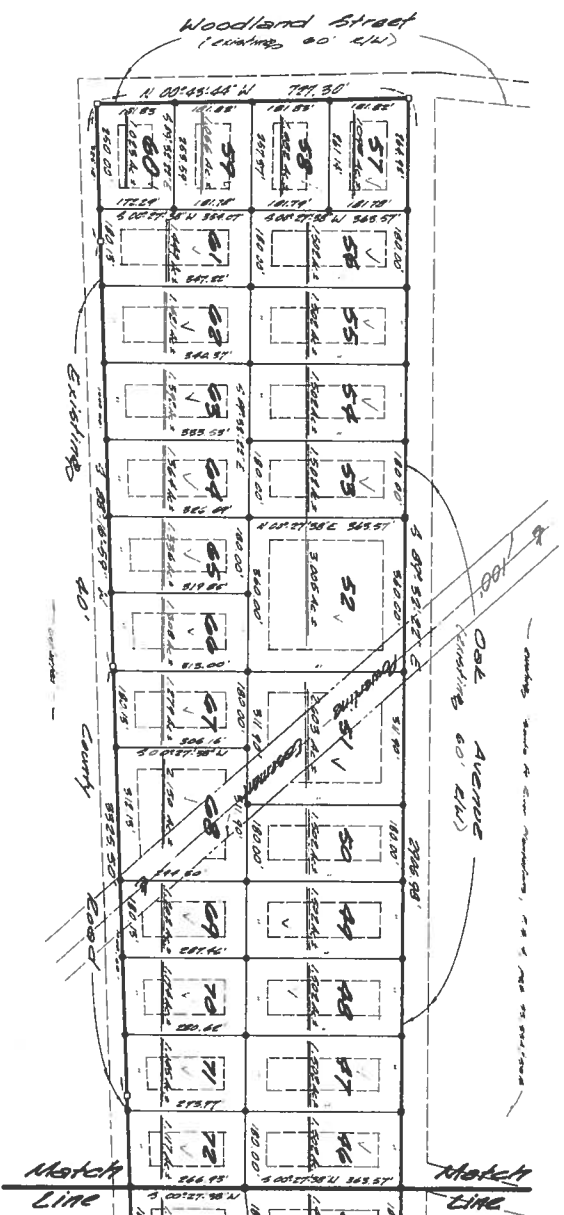
3. Amphipoda (continued) -
 none of the species being
 collected nearly fresh

For George W. -
Abelus Count and Owners, Mr
Geo W E Geo W E
Channing, Town

A REPEAT OF LOTS 38, 45 AND 46 OF

A SUBDIVISION SITUATED IN
B. 30, TOWNSHIP 7 SOUTH, R.
COLUMBIA COUNTY, FLORIDA

SHEET 5 OF 5 SHEETS



CIVILS 2010					
Roll No.	Personal File No.	Rank	Length of Service	Age	Pay Band
1	5700461	771/80	47.00	59.25	4, 63, 1, 100, 3, 54, 4
2	5700462	102/85	48.48	60.25	4, 63, 1, 100, 3, 57, 4
3	5700463	102/85	52.50	62.25	4, 63, 1, 100, 3, 57, 4
4	5700464	87/87	48.24	62.25	4, 63, 1, 100, 3, 57, 4
5	5700465	52/90	50.51	60.25	4, 63, 1, 100, 3, 57, 4
6	5700466	55/88	55.11	62.25	4, 63, 1, 100, 3, 57, 4

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Legend

2018 Flood Zones

0.2 PCT ANNUAL CHANCE

A

AE

AH

Lake City

Roads

Roads

others

Dirt

Interstate

Main

Other

Paved

Private

2018Aerials

SectionTownshipAndRange

LidarElevations

X

Columbia County, FLA - Building & Zoning Property Map

Printed: Wed Oct 23 2019 12:12:37 GMT-0400 (Eastern Daylight Time)



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SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # 43856 JOB NAME Crutchfield Residence

THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED

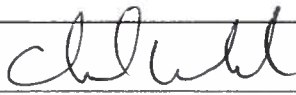

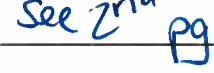
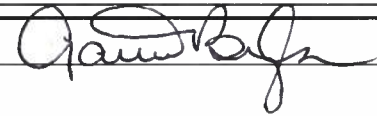
Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: <http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx>

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

ELECTRICAL <input checked="" type="checkbox"/> MECHANICAL/A/C CC# <u>543</u>	Print Name <u>Chad White</u> Signature <u></u> Company Name: <u>CK Electric, LLC</u> License #: <u>EC 13002222</u> Phone #: <u>352-538-5544</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
MECHANICAL/A/C CC# <u>380</u>	Print Name <u>Donnie Davis</u> Signature <u></u> Company Name: <u>High Springs Electric, LLC</u> License #: <u>CAC 1815367</u> Phone #: <u>386-623-0499</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/GAS CC# <u>767</u>	Print Name <u>Paul Kevin Coleman</u> Signature <u></u> Company Name: <u>Coleman's Plumbing, Inc.</u> License #: <u>CFC 1425624</u> Phone #: <u>352-472-4114</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING CC# <u>672</u>	Print Name <u>Garrett Buzbee</u> Signature <u></u> Company Name: <u>G Buzbee, Inc.</u> License #: <u>CGC 061980</u> Phone #: <u>386-454-2555</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
FIRE SYSTEM/SPRINKLER CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SOLAR CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
STATE SPECIALTY CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE

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43856

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ELECTRICAL <input checked="" type="checkbox"/> CC# 543	Print Name <u>Chad White</u> Signature _____ Company Name: <u>CK Electric, LLC</u> License #: <u>EC 13002222</u> Phone #: <u>352-538-5544</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
MECHANICAL/A/C <input checked="" type="checkbox"/> CC# 380	Print Name <u>Donnie Davis</u> Signature _____ Company Name: <u>High Springs Electric, LLC</u> License #: <u>CAC 1815367</u> Phone #: <u>386-623-0499</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/GAS <input checked="" type="checkbox"/> CC# 767	Print Name <u>Paul Kevin Coleman</u> Signature <u>Paul K. Coleman</u> Company Name: <u>Coleman's Plumbing, Inc.</u> License #: <u>CFC 1425624</u> Phone #: <u>352-472-4114</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING <input checked="" type="checkbox"/> CC# 672	Print Name <u>Garrett Buzbee</u> Signature _____ Company Name: <u>G Buzbee, Inc.</u> License #: <u>CGC 061980</u> Phone #: <u>386-454-2555</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL <input type="checkbox"/> CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
FIRE SYSTEM/SPRINKLER <input type="checkbox"/> CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SOLAR <input type="checkbox"/> CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
STATE SPECIALTY <input type="checkbox"/> CC# _____	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE

NOTICE OF COMMENCEMENT

Tax Parcel Identification Number:

30-7S-17-10058-675

Clerk's Office Stamp

Inst: 201912024276 Date: 10/21/2019 Time: 11:37AM
Page 1 of 1 B: 1396 P: 2198, P. DeWitt Cason, Clerk of Court
Columbia, County, By: BD
Deputy Clerk

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): LOT 85 SANTA FE RIVER PLANTATIONS REPLAT OF LOT 38.
a) Street (job) Address: 816 SW Heflin Ave. Fort White, FL 32038
2. General description of improvements: NEW SINGLE FAMILY RESIDENCE
3. Owner Information or Lessee information if the Lessee contracted for the improvements:
a) Name and address: REGINALD & REGINA CRUTCHFIELD / 13622 NW 143RD PLACE, ALACHUA, FL 32615
b) Name and address of fee simple titleholder (if other than owner) N/A
c) Interest in property FEES SIMPLE
4. Contractor Information
a) Name and address: G Buzbee Inc. / 17774 US Hwy 441, High Springs, FL 32643
b) Telephone No.: 388-454-2555
5. Surety Information (if applicable, a copy of the payment bond is attached):
a) Name and address: N/A
b) Amount of Bond: _____
c) Telephone No.: _____
6. Lender
a) Name and address: N/A
b) Phone No. _____
7. Person within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:
a) Name and address: _____
b) Telephone No.: _____
8. In addition to himself or herself, Owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name: _____ OF _____
b) Telephone No.: _____
9. Expiration date of Notice of Commencement (the expiration date will be 1 year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

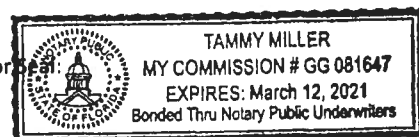
10. _____
Signature of Owner or Lessee, or Owner's or Lessee's Authorized Office/Director/Partner/Manager
Reginald Crutchfield
Printed Name and Signatory's Title/Office

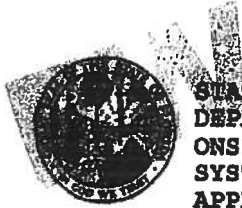
The foregoing instrument was acknowledged before me, a Florida Notary, this 18 day of OCT, 2019, by:
Reginald Crutchfield as _____ for _____
(Name of Person) (Type of Authority) (name of party on behalf of whom instrument was executed)

Personally Known _____ OR Produced Identification ☒ Type FLDL

Notary Signature Tammy Miller

Notary Stamp on Seal:





STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 14-0788
DATE PAID: 10/24/19
FEE PAID: 3000.00
RECEIPT #: 449988

APPLICATION FOR:

☒ New System ☐ Existing System ☐ Holding Tank ☐ Innovative
☐ Repair ☐ Abandonment ☐ Temporary ☐

APPLICANT: Reginald CrutchfieldAGENT: Smith SepticTELEPHONE: 386-935-424MAILING ADDRESS: P.O. Box 838 Blue, FL 32619

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 85 BLOCK: _____ SUBDIVISION: Santa Fe River Platts PLATTED: 8/19/77PROPERTY ID #: 30-75-17-10058-675 ZONING: PC2 I/M OR EQUIVALENT: ☐ Y ☒ NPROPERTY SIZE: 1.63 ACRES WATER SUPPLY: ☒ PRIVATE PUBLIC ☐ ☐ ≤2000GPD ☐ >2000GPDIS SEWER AVAILABLE AS PER 381.0065, FS? ☐ Y ☒ N

DISTANCE TO SEWER: _____ FT

PROPERTY ADDRESS: 816 Heflin AveDIRECTIONS TO PROPERTY: Hwy 47 S TR e Hwy 27 TR - CR 18 TR e Heflin
Property - Right

BUILDING INFORMATION

☒ RESIDENTIAL ☐ COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	<u>Home</u>	<u>3</u>	<u>2110</u>	
2				
3				
4				

☐ Floor/Equipment Drains ☐ Other (Specify) _____

SIGNATURE: _____

DATE: 10-22-19

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC



COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018
AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES

Revised 7/1/18

Website: <http://www.columbiacountyfla.com/BuildingandZoning.asp>

Items to Include-
Each Box shall be
Circled as
Applicable

GENERAL REQUIREMENTS:

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Select From Drop down

1	Two (2) complete sets of plans containing the following:	<input checked="" type="checkbox"/>		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	<input checked="" type="checkbox"/>		
3	Condition space (Sq. Ft.) 2110sf	Total (Sq. Ft.) under roof 3285sf	Yes	No NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	- /		
5	Dimensions of all building set backs	- /		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	- /		
7	Provide a full legal description of property.	- /		

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA

Select From Drop down

9	Basic wind speed (3-second gust), miles per hour	- /		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	- /		
11	Wind importance factor and nature of occupancy	- /		
12	The applicable internal pressure coefficient, Components and Cladding	- /		
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifiically designed by the registered design professional.	- /		

Elevations Drawing including:

14	All side views of the structure	- /		
15	Roof pitch	- /		
16	Overhang dimensions and detail with attic ventilation	- /		
17	Location, size and height above roof of chimneys	- N/A		
18	Location and size of skylights with Florida Product Approval	- N/A		
19	Number of stories	- /		
20	Building height from the established grade to the roofs highest peak	- /		

Floor Plan Including:

21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	- /		
22	Raised floor surfaces located more than 30 inches above the floor or grade	- /		
23	All exterior and interior shear walls indicated	- /		
24	Shear wall opening shown (Windows, Doors and Garage doors)	- /		
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	✓ -		
26	Safety glazing of glass where needed	✓		
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	N/A		
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	N/A		
29	Identify accessibility of bathroom (see FBCR SECTION 320)	✓		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable
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FBCR 403: Foundation Plans

Select From Drop down

30	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	- /		
31	All posts and/or column footing including size and reinforcing	- /		
32	Any special support required by soil analysis such as piling.	N/A		
33	Assumed load-bearing value of soil <u>1500</u> Pound Per Square Foot	- /		
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	✓		

FBCR 506: CONCRETE SLAB ON GRADE

35	Show Vapor retarder (6mil. Polyethylene with joints taped 6 inches and sealed)	✓		
36	Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports	✓		

FBCR 318: PROTECTION AGAINST TERMITES

37	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides	- /		
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FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

38	Show all materials making up walls, wall height, and Block size, mortar type	✓		
39	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	✓		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	-		
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers	-		
42	Girder type, size and spacing to load bearing walls, stem wall and/or piers	-		
43	Attachment of joist to girder	-		
44	Wind load requirements where applicable	-		
45	Show required under-floor crawl space	-		
46	Show required amount of ventilation opening for under-floor spaces	-		
47	Show required covering of ventilation opening	-		
48	Show the required access opening to access to under-floor spaces	-		
49	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	-		
50	Show Draftstopping, Fire caulking and Fire blocking	-		
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	-		
52	Provide live and dead load rating of floor framing systems (psf).	-		

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
---	--	--	--	--

Select from Drop down

53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	- /		
54	Fastener schedule for structural members per table FBC-R602.3.2 are to be shown	- /		
55	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	- /		
56	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	- /		
57	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBC-R602.7.	- /		
58	Indicate where pressure treated wood will be placed	- /		
59	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	- /		
60	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	- /		

FBCR :ROOF SYSTEMS:

61	Truss design drawing shall meet section FBC-R 802.10.1 Wood trusses	- /		
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	- /		
63	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	- /		
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	- /		
65	Provide dead load rating of trusses	- /		

FBCR 802:Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing	- /		
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	-		
68	Valley framing and support details	-		
69	Provide dead load rating of rafter system	-		

FBCR 803 ROOF SHEATHING

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	- /		
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	- /		

ROOF ASSEMBLIES FRC Chapter 9

72	Include all materials which will make up the roof assemblies covering	- /		
73	Submit Florida Product Approval numbers for each component of the roof assemblies covering	- /		

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. **Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.**

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
Select from Drop Down				
74	Show the insulation R value for the following areas of the structure	-		
75	Attic space	-	/	
76	Exterior wall cavity	-	/	
77	Crawl space	-	N/A	

HVAC information

78	Submit two copies of a Manual J sizing equipment or equivalent computation study	- /		
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required	- /		
80	Show clothes dryer route and total run of exhaust duct	- /		

Plumbing Fixture layout shown

81	All fixtures waste water lines shall be shown on the foundation plan	- /		
82	Show the location of water heater	- /		

Private Potable Water

83	Pump motor horse power	- 1 HP		
84	Reservoir pressure tank gallon capacity	- 80 gal		
85	Rating of cycle stop valve if used	-		

Electrical layout shown including

86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	- /		
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	- /		
88	Show the location of smoke detectors & Carbon monoxide detectors	- /		
89	Show service panel, sub-panel, location(s) and total ampere ratings	- /		
90	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	- /		
91	Appliances and HVAC equipment and disconnects	- /		
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	- /		

Notice Of Commencement:

A notice of commencement form **RECORDED** in the Columbia County Clerk Office is required to be filed with the Building Department **BEFORE ANY INSPECTIONS** can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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****ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT.*****Select from Drop down*

93	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	-		
94	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	-		
95	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	-		
96	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031	-		
97	Toilet facilities shall be provided for all construction sites	-		
98	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	-		
99	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com)	-		
100	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	-		
101	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00	-		
102	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	-		
103	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	-		

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Disclosure Statement for Owner Builders:

If you as the Applicant will be acting as your own contractor or owner/builder under section 489.103(7) Florida Statutes, you must submit the required notarized Owner Builder Disclosure Statement form.

****This form can be printed from the Columbia County Website on the Building and Zoning page under Documents. Web address is - <http://www.columbiacountyfla.com/BuildingandZoning.asp>**

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date of issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

Notification:

When the application is approved for permitting the applicant will be notified by phone as to the status by the Columbia County Building & Zoning Department.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

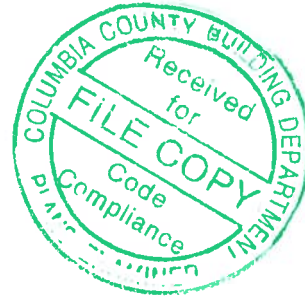
Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Plast Pro	Exterior Doors	FL 15213
B. SLIDING			
C. SECTIONAL/ROLL UP	Overhead Door Co.	Garage Door	FL 14170
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	YKK	Windows	FL 8114
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	Cemplank	Siding	FL 13192
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	Owens Corning	Asphalt Shingles	FL 10674
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS	Per Engineering		
B. WOOD ANCHORS	Per Engineering		
C. TRUSS PLATES	Per Engineering		
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

NOTES: _____

E



Prepared for:

G BUZBEE INCORPORATED
CRUTCHFIELD RESIDENCE
816 SW HEFLIN AVENUE
COLOMBIA COUNTY, FLORIDA

By:

Schafer Engineering, LLC ca9312

386-462-1340

NO COPIES ARE TO BE PERMITTED

SCHAFFER ENGINEERING, LLC
7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340

Trusses: Pre-engineered, pre-fabricated with the manufacturer's required bracing system installed.

Roof Sheathing: Type: OSB Size: 7/16" Fastener type nails: 8d / .113 Ring Shank
Interior zone spacing: Interior: 6" Periphery: 4"
Edge and end zone spacing: Interior: 6" Periphery: 4"

Double Top Plate: Type: Spruce Grade: #2 Size: 2 x 4 Nail Spacing: 8" o.c.

Stud Type: Spruce Grade: #2 Size: 2 x 4
Interior stud spacing: 16" End stud spacing: 16"

Required Shear Wall Siding: Type: OSB Thickness: 7/16"
49 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4"
50 ft Long: Fastener 8d/131 Spacing: Int: 8 Edge: 4"

Allowable Unit Shear on Shear Walls: 314 pounds per linear foot
Allowable Unit Shear Transferred from Diaphragm: Trans: 227 Long: 100

Wall Tension Transferred by: Siding Nails: 8d/131 @ 4" O.C. Edges

Foundation Anchor Bolts: Concrete Strength: 3000 psi Size: 1/2"

Washer: 2" Embedment: 7" Location of first anchor bolt from corner: 8"

Anchor Bolts @ 48" o.c. Model: A307 Loc. from corner: 8"

Type of Foundation: (1) - #5 rebar continuous required in bond beam.

Floor Slab: 4" Cmu size: 8" x 16" Height: 32" Rein.: #5 at 72" o.c.

Monolithic Footing: Depth: 20" Bottom Width: 12 Rein.: 2 #5 rebars

Stemwall Footing: Width: 20 Depth: 10 Rein.: 3 #5 rebar

Interior Footings 20" Wide X 16" Deep with 3-#5 rebar continuous

6 X 6 X 9' syp #2 pt @

Porch Columns: 11'-0" o.c. max. spacing Column Fasteners: Simpson PC66 \
PBS66 or equal

Special Comments: Install 2 ply 2 x 12 syp #2 with 7/16" osb fitch beam over
all doors and windows

Notes:

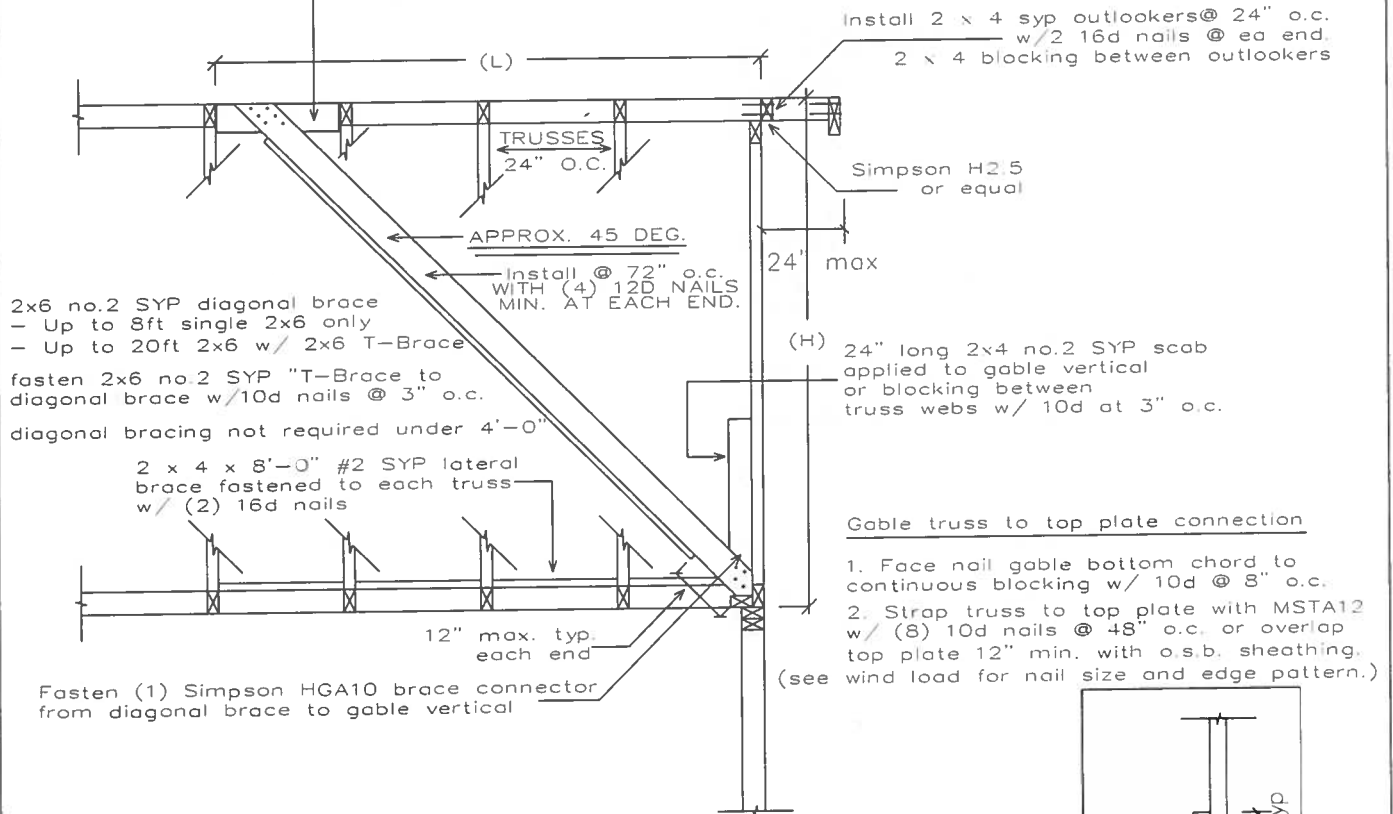
1. Balloon frame all gable ends unless accompanied by gable end detail
2. All walls to be nailed with same nailing pattern as the shear walls.
3. This wind load is not valid without a raised, embossed seal. (NO COPIES).
4. 1500 psf soil bearing pressure minimum.
5. Fiber mesh or WWM may be used in concrete slab. All steel must be grade 40 min. Install standard 10" ACI hook top and bottom.
6. Trusses must be installed and anchored in accordance to the truss engineering.
7. All headers spanning 12' and over must be pre-engineered.
8. This is a windload only. Not a structural analysis. Schaffer Engineering strongly recommends always having a structural analysis.
9. The foundation is for minimum design use, and may be increased.
10. Wind load is for one use only \ FBC-2017 \ No copies permitted
11. Install anchor bolts a 48" o.c., & Simpson SP1 at bottom plate and Simpson SP2 at top plate or equal @ 32" O.C. for all interior bearing walls.
12. Truss company to use all exterior porch walls for bearing when possible.

Bruce Schaffer, P. E. #48984 ca 9312
7104 NW 42ND LN
GAINESVILLE, FL. 32606

SCHAFER ENGINEERING, LLC

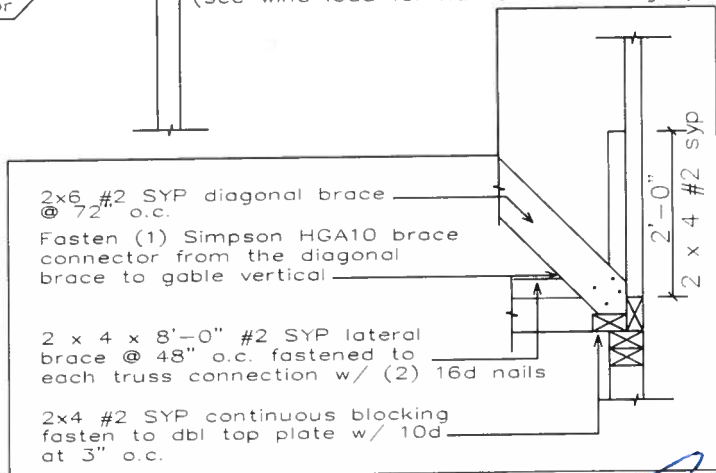
7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340

Toe-Nail min 2x6 No 2 SYP blocking
between truss top chords with
(3) 10d each end min.



Gable truss to top plate connection

1. Face nail gable bottom chord to continuous blocking w/ 10d @ 8" o.c.
2. Strap truss to top plate with MSTA12 w/ (8) 10d nails @ 48" o.c. or overlap top plate 12" min. with o.s.b. sheathing. (see wind load for nail size and edge pattern.)



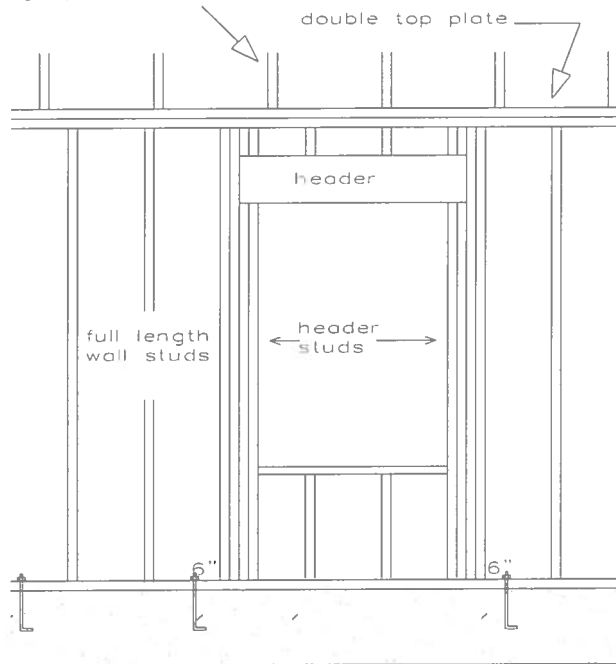
TYPICAL GABLE END BRACING

Bruce Schafer, P. E. #48984 CA #9312
7104 NW 42ND LN
GAINESVILLE, FL. 32606

SCHAFFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340

see truss engineering for required
anchorage from truss to top plate
and bracing system to be installed



total each truss uplift on the header and divide
by two for header and header stud anchorages

		Maximum Header Span (ft)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting End of Header					
		1	1	2	2	2	2
Unsupported Wall Height	Stud Spacing	Number of Full Length Studs at Each End of Header					
		2	2	3	3	3	3
10'-0" or less	12"	2	2	3	3	3	3
	16"	2	2	3	3	3	3
	24"	1	2	2	2	2	2
Greater than 10'-0"	12"	2	2	3	4	5	5
	16"	2	2	3	3	4	4
	24"	1	2	2	2	3	3

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PHONE: 386-462-1340

TIE-DOWN TABLES

HEADER STRAPPING				
Uplift Lbs	Top Connector	Rating Lbs	Bottom Connector	Rating Lbs
to 455	LSTA9	635	H3	320
to 910	LSTA12	795	2-H3	640
to 1265	LSTA18	1110	LTT19	1305
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2165
to 2865	3-LSTA18	3255	HD2A-3.5	2565
to 3700	3-LSTA24	3880	HD5A-3	3130

Total the uplift for each truss sitting on the header and divide by 2 to determine the uplift on the header. Use proper bolt anchors sufficient to support required uplift loads.

TRUSSES \ GIRDERS			
Uplift Lbs	Top Connector	Bottom Connector	Rating Lbs
to 535	H2.5A	NA	
to 1015	H10A	NA	
to 1215	TS22	LTT19	1305
to 1750	2-TS22	LTT20	1750
to 2570	2-TS22	HD2A	2565
to 3665	3-TS22	HD5A	3645
to 5420	2-MST37	HTT22	5250
to 9660	2-MST60	HD10A	8160

Two 12d common toenails are required per truss for each bearing point into top plate.
It is the contractors responsibility to provide a continuous load path from truss to foundation.

	TOP CONNECTOR	RATING LBS	BOTTOM CONNECTOR	RATING LBS
BEAM SEATS	LSTA18	1110	LTT19	1305
POSTS	2-LSTA18	2220	ABU44	2200

1. Simpson or equivalent hardware may be used
For nailing into spruce members,
multiply table values by .86
2. See truss engineering for anchor uplift values.
3. This schedule is not meant to be a
replacement to the specified values of
any manufactures values.

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	135	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.3	Deg
Type of Roof	Gabled	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	23.67	ft
Mean Roof Height (Ht)	16.34	ft
Width Perp. to Wind (B)	56.00	ft
Width Parallel to Wind (L)	64.67	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.29
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Non-Hurricane, Hurricane (v=85-100 mph) & Alaska		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
I =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	$Cc * (33/z)^{0.167}$	0.3048	
Lzm	$I * (zm/33)^{Epsilon}$	309.99	ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.8938	
Gust2	$0.925 * ((1+1.7 * Izm * 3.4 * Q)/(1+1.7 * 3.4 * Izm))$	0.8623	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	$V * (5280/3600)$	198.00	ft/s
Vzm	$bm * (zm/33)^{Am} * Vhref$	87.00	ft/s
NF1	$NatFreq * Lzm / Vzm$	3.56	Hz
Rn	$(7.47 * NF1) / (1 + 10.302 * NF1)^{1.667}$	0.0627	
Nh	$4.6 * NatFreq * Ht / Vzm$	0.86	
Nb	$4.6 * NatFreq * B / Vzm$	2.96	
Nd	$15.4 * NatFreq * Depth / Vzm$	11.45	
Rh	$1/Nh - (1/(2 * Nh^2) * (1 - Exp(-2 * Nh)))$	0.6066	
Rb	$1/Nb - (1/(2 * Nb^2) * (1 - Exp(-2 * Nb)))$	0.2809	
Rd	$1/Nd - (1/(2 * Nd^2) * (1 - Exp(-2 * Nd)))$	0.0835	
RR	$((1/Beta) * Rn * Rh * Rb * (0.53 + 0.47 * Rd))^{0.5}$	0.7800	
gg	$+(2 * LN(3600 * n1))^{0.5} + 0.577 / (2 * LN(3600 * n1))^{0.5}$	4.19	
Gust3	$0.925 * ((1 + 1.7 * Izm * (3.4^2 * Q^2 + GG^2 * RR^2))^{0.5}) / (1 + 1.7 * 3.4 * Izm)$	1.11	

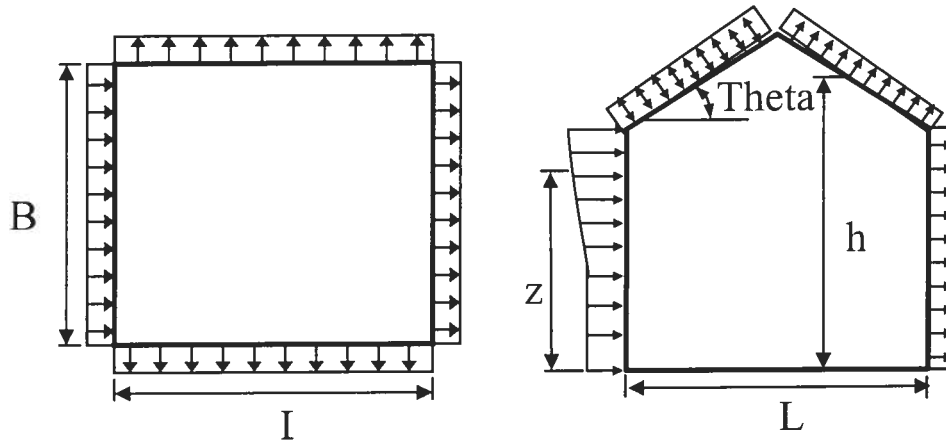
Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft ²	Pressure (lb/ft ²)	
					Windward Wall* +GCpi	-GCpi
23.67	0.70	1.00	1.00	32.69	17.60	27.49
20	0.70	1.00	1.00	32.69	17.60	27.49
16.34	0.70	1.00	1.00	32.69	17.60	27.49
15	0.70	1.00	1.00	32.69	17.60	27.49

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (Ht/zg)^{2/\alpha}$	0.59	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	27.48	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 56 ft wall)	-0.47	-16.06	-6.17
Leeward Walls (Wind Dir Parallel to 64.67 ft wall)	-0.50	-16.79	-6.90
Side Walls	-0.70	-21.53	-11.64
Roof - Normal to Ridge (Theta ≥ 10)			
Windward - Max Negative	-0.19	-9.40	0.49
Windward - Max Positive	0.30	2.28	12.17
Leeward Normal to Ridge	-0.60	-19.16	-9.27
Overhang Top	-0.19	-4.46	-4.46
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 8.17 ft	-0.90	-26.27	-16.38
Dist from Windward Edge: 8.17 ft to 16.34 ft	-0.90	-26.27	-16.38
Dist from Windward Edge: 16.34 ft to 32.68 ft	-0.50	-16.79	-6.90
Dist from Windward Edge: > 32.68 ft	-0.30	-12.05	-2.16

* Horizontal distance from windward edge

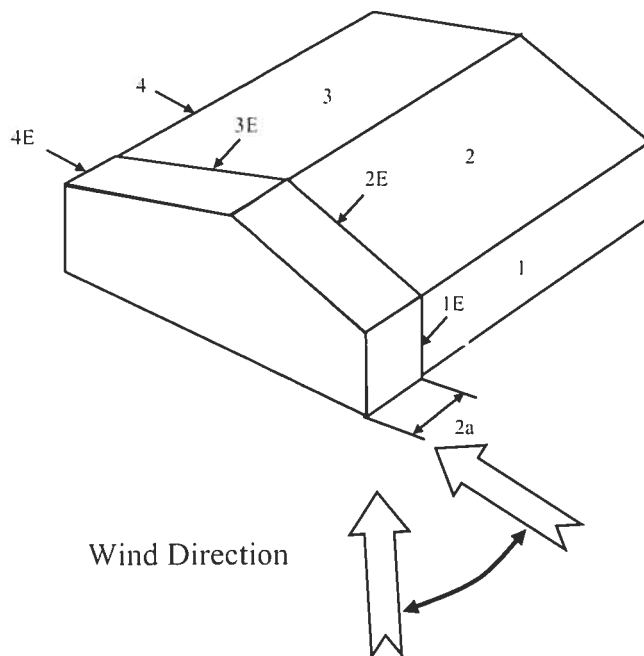
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (H_t/z_g)^{2/\alpha} &= & 0.59 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 27.48
 \end{aligned}$$

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	32.69	12.42	24.19
2	0.21	0.18	-0.18	32.69	0.98	12.75
3	-0.43	0.18	-0.18	32.69	-19.94	-8.17
4	-0.37	0.18	-0.18	32.69	-17.98	-6.21
5	0.00	0.18	-0.18	32.69	-5.88	5.88
6	0.00	0.18	-0.18	32.69	-5.88	5.88
1E	0.69	0.18	-0.18	32.69	16.67	28.44
2E	0.27	0.18	-0.18	32.69	2.94	14.71
3E	-0.53	0.18	-0.18	32.69	-23.21	-11.44
4E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
5E	0.00	0.18	-0.18	32.69	-5.88	5.88
6E	0.00	0.18	-0.18	32.69	-5.88	5.88

$$* p = q_h * (GCpf - GCpi)$$

**Figure 6-4 - External Pressure Coefficients, GCpf**

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (H_t/z_g)^{2/\alpha} &= & 0.59 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 27.48
 \end{aligned}$$

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	32.69	-20.59	-8.83
2	-0.69	0.18	-0.18	32.69	-28.44	-16.67
3	-0.37	0.18	-0.18	32.69	-17.98	-6.21
4	-0.45	0.18	-0.18	32.69	-20.59	-8.83
5	0.40	0.18	-0.18	32.69	7.19	18.96
6	-0.29	0.18	-0.18	32.69	-15.36	-3.60
1E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
2E	-1.07	0.18	-0.18	32.69	-40.86	-29.09
3E	-0.53	0.18	-0.18	32.69	-23.21	-11.44
4E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
5E	0.61	0.18	-0.18	32.69	14.06	25.82
6E	-0.43	0.18	-0.18	32.69	-19.94	-8.17

$p = qh * (GCpf - GCpi)$

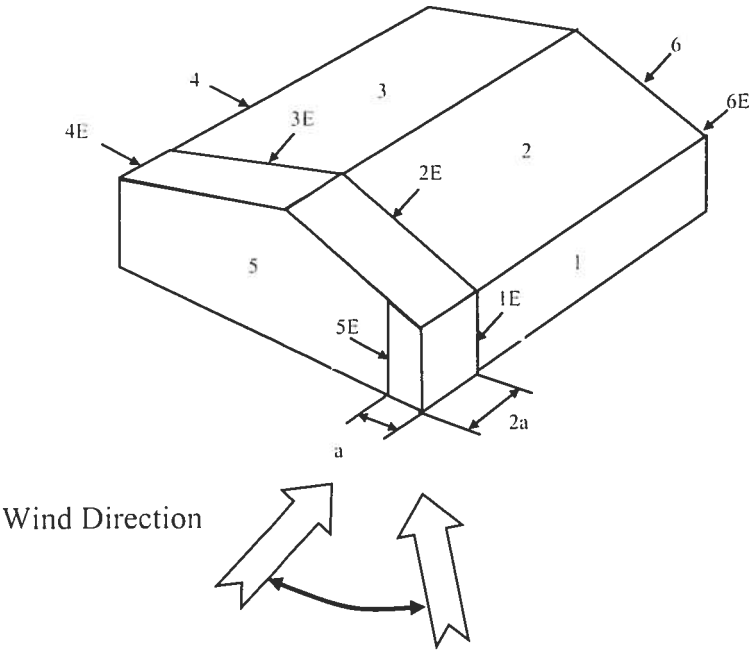
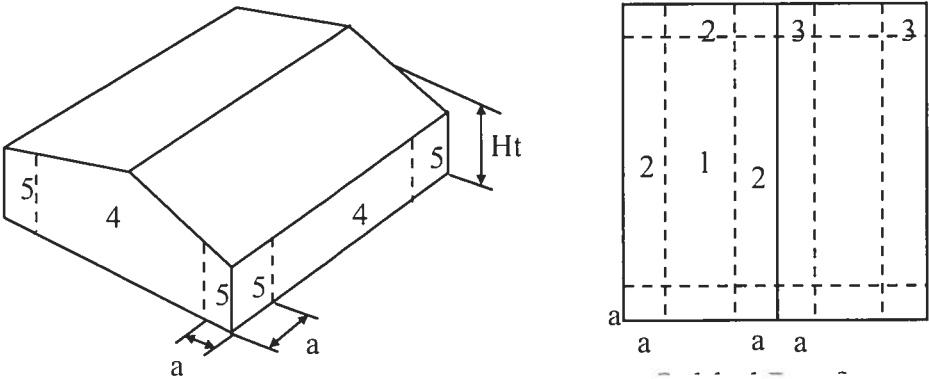


Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht ≤ 60 ft



$$a = 5.6 \implies \boxed{5.60 \text{ ft}}$$

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, C_{gpi}

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 2100698 - GBI - CRUTCHFIELD RES.

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: G. Buzbee, Inc. Project Name: Crutchfield Res. Model: Custom
Lot/Block: N/A Subdivision: N/A
Address: 816 SW Heflin Ave., N/A
City: Columbia City State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

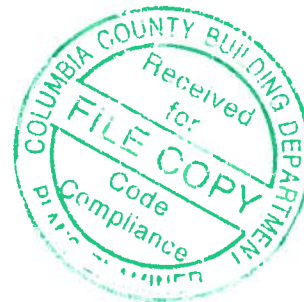
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 37.0 psf Floor Load: N/A psf

This package includes 23 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T18264480	CJ02	10/2/19	23	T18264502	V09	10/2/19
2	T18264481	CJ04	10/2/19				
3	T18264482	HJ06	10/2/19				
4	T18264483	PB01	10/2/19				
5	T18264484	PB01G	10/2/19				
6	T18264485	PB02	10/2/19				
7	T18264486	PB02G	10/2/19				
8	T18264487	T01	10/2/19				
9	T18264488	T01G	10/2/19				
10	T18264489	T02	10/2/19				
11	T18264490	T02G	10/2/19				
12	T18264491	T03	10/2/19				
13	T18264492	T03G	10/2/19				
14	T18264493	T04	10/2/19				
15	T18264494	V01	10/2/19				
16	T18264495	V02	10/2/19				
17	T18264496	V03	10/2/19				
18	T18264497	V04	10/2/19				
19	T18264498	V05	10/2/19				
20	T18264499	V06	10/2/19				
21	T18264500	V07	10/2/19				
22	T18264501	V08	10/2/19				

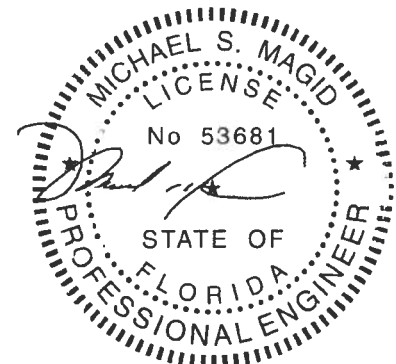


The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Magid, Michael

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Michael S. Magid PE No.53681
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

October 2, 2019

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264480
2100698	CJ02	Jack-Open	2	1	Job Reference (optional)	

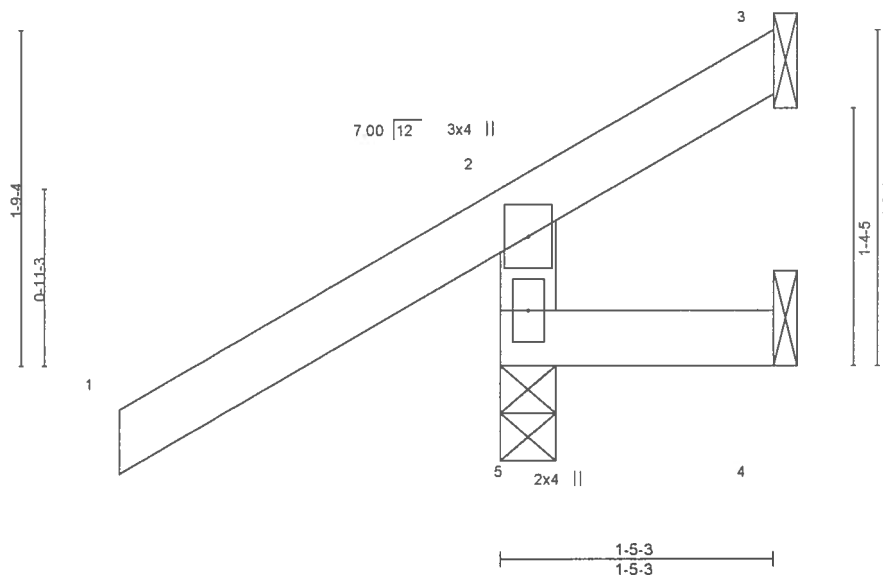
Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11 01 56 2019 Page 1

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Scale = 1/12 = 2



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.37	Vert(LL)	0.00	5	>999	240	MT20
TCDL 7.0	Lumber DOL	1.25	BC 0.07	Vert(CT)	0.00	5	>999	180	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2017/TP12014		Matrix-MR						
								Weight 9 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
WEBS 2x4 SP No 3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 1-5-3 oc purlins, except end verticals
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS.

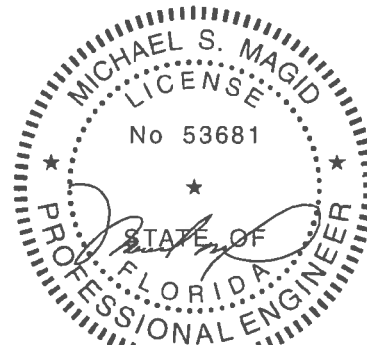
(lb/size) 5=261/0-3-8, 3=-36/Mechanical, 4=-15/Mechanical
Max Horz 5=88(LC 12)
Max Uplift 5=-120(LC 12), 3=-36(LC 1), 4=-19(LC 19)
Max Grav 5=261(LC 1), 3=14(LC 8), 4=17(LC 3)

FORCES.

(lb) - Max Comp /Max Ten - All forces 250 (lb) or less except when shown.
TOP CHORD 2-5=-221/252

NOTES-

- 1) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf; h=18ft, Cat. II; Exp C, Encl, GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- 5) Refer to girder(s) for truss to truss connections
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=ib) 5=120.



Michael S. Magid PE No.53681
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

October 2, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 33610



Weight: 13 lb FT = 20%

BRACING-	
TOP CHORD	Structural wood sheathing directly applied or 3-2-13 oc purlins
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing

FORCES. (lb) - Max Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- NOTES-**
- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp C, Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, porch left and right exposed, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - 3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - 5) Refer to girder(s) for truss to truss connections
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb)
- 2=235



October 2, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2013 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES.	T18264482
2100698	HJ06	Roof Special Girder	2	1		

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11 01 58 2019 Page 1

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Scale = 1/19.2

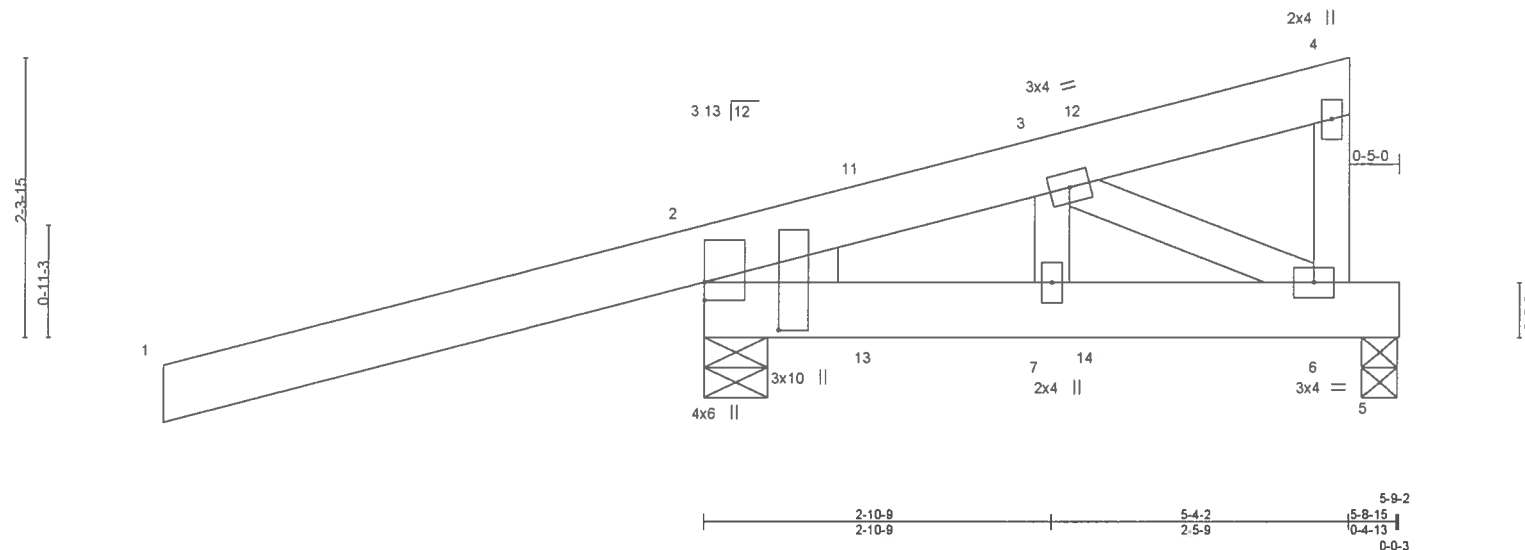


Plate Offsets (X,Y)-- [2 0-4-12,0-7-6]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.61	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.12	Vert(LL) 0.00 10 >999 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.06	Vert(CT) -0.00 6-7 >999 180		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-MP	Horz(CT) 0.00 2 n/a n/a	Weight 46 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No 2
BOT CHORD 2x6 SP No 2
WEBS 2x4 SP No 3
WEDGE
Left: 2x4 SP No 3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS.

(lb/size) 5=76/0-3-8, 2=535/0-6-5
Max Horz 2=142(LC 4)
Max Uplift 5=-100(LC 5), 2=-515(LC 4)
Max Grav 5=109(LC 3), 2=535(LC 1)

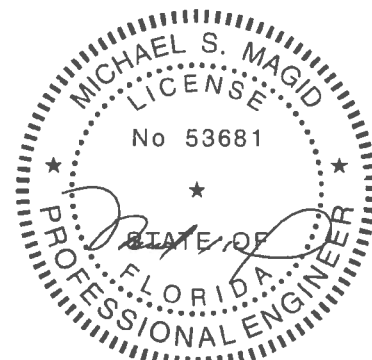
FORCES. (lb) - Max Comp /Max Ten - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp C, Encl. GCpi=0.18, MWFRS (envelope) gable end zone, porch left and right exposed, Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=515
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 23 lb down and 27 lb up at 1-5-2, and 73 lb down and 96 lb up at 3-3-3 on top chord, and 17 lb down and 33 lb up at 1-5-2, and 31 lb down and 46 lb up at 3-3-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced) Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert. 1-4=-54, 5-8=-20
Concentrated Loads (lb)
Vert. 12=24(B) 13=-1(F)



Michael S. Magid PE No.53681
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

October 2, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264483
2100698	PB01	Piggyback	11	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11:02:00 2019 Page 1
ID 1bYwwjYqtpHfMFFctmROVywFXb-CplK1JqTPEgGdLSfIWRqD4Jz79OV9IADF6IT6tyXTe5

2-0-0 2-0-0 4-0-0 2-0-0

Scale = 1/8"

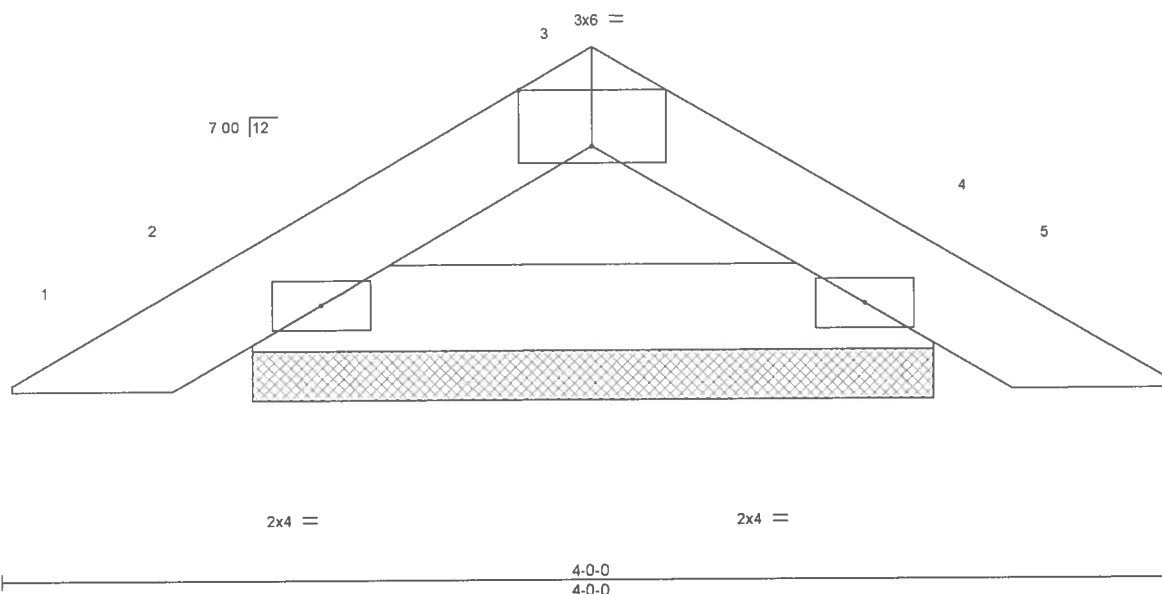


Plate Offsets (X,Y)-- [3 0-3-0,Edge]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1 25	TC 0 03	Vert(LL)	0 00	4	n/r	MT20	244/190
TCDL 7 0	Lumber DOL	1 25	BC 0 06	Vert(CT)	0 00	4	n/r		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0 00	Horz(CT)	0 00	4	n/a		
BCDL 10 0	Code FBC2017/TPI2014		Matrix-P					Weight 10 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

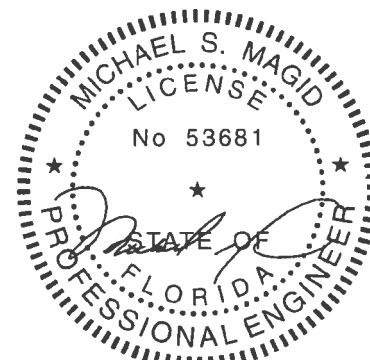
REACTIONS.

(lb/size) 2=114/2-3-11, 4=114/2-3-11
Max Horz 2=-31(LC 10)
Max Uplift 2=-51(LC 12), 4=-51(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design
- Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf, BCDL=3 0psf, h=18ft, Cat II, Exp C, Encl, GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
- Gable requires continuous bottom chord bearing
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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MiTek

6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264484
2100698	PB01G	PIGGYBACK	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11 02:01 2019 Page 1
ID 1bYwwjYqtpHfIMFFctmROVywFXb-g?siEfr5AYo6EV1ssDz3mls8rZjTulQMUm11eKyXTe4

1-5-1 1-5-1 2-10-2 1-5-1

Scale = 1/6" = 2'

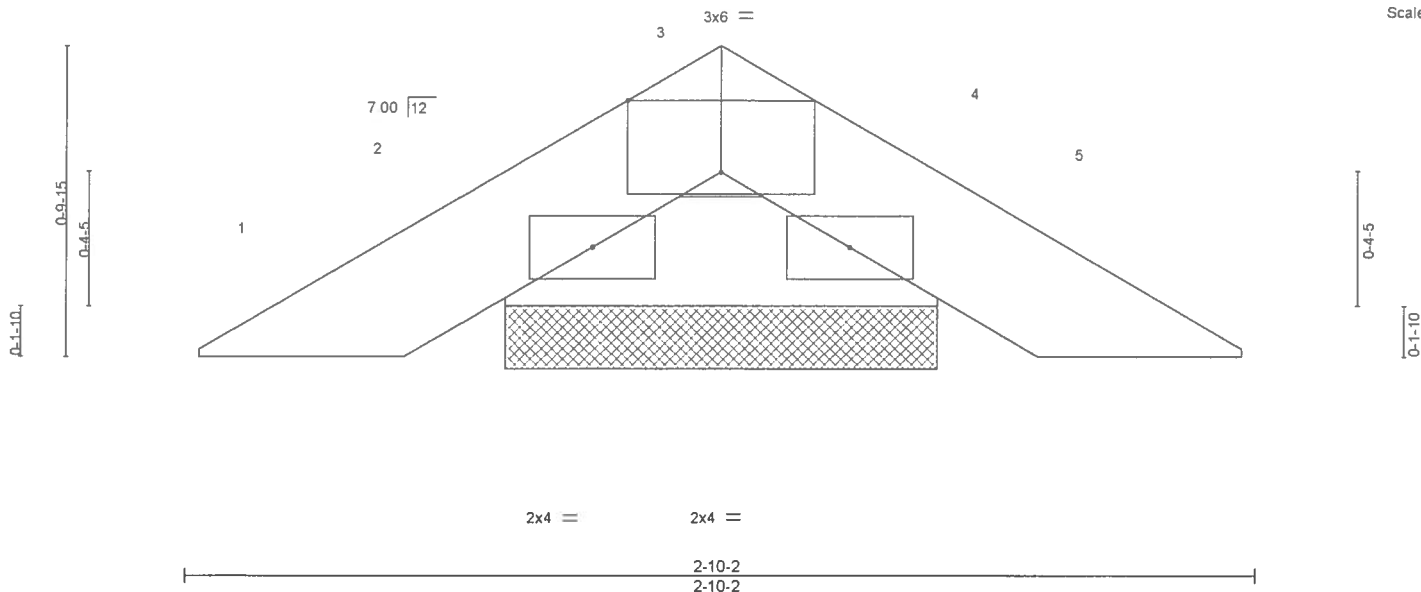


Plate Offsets (X,Y)-- [3 0-3-0,Edge]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1 25	TC 0 02	Vert(LL)	-0 00	4	n/r	120	MT20	244/190
TCDL 7 0	Lumber DOL	1 25	BC 0 01	Vert(CT)	-0 00	4	n/r	120		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0 00	Horz(CT)	0 00	4	n/a	n/a		
BCDL 10 0	Code FBC2017/TPI2014		Matrix-P						Weight 7 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-10-2 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

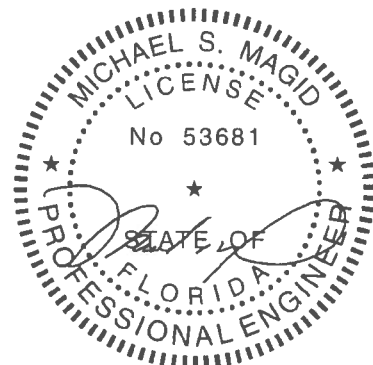
REACTIONS.

(lb/size) 2=72/1-1-13, 4=72/1-1-13
Max Horz 2=-20(LC 10)
Max Uplift 2=-37(LC 12), 4=-37(LC 13)

FORCES. (lb) - Max. Comp /Max. Ten. - All forces 250 (lb) or less except when shown

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4 2psf, BCDL=3 0psf, h=18ft, Cat II, Exp C, Encl., GCpi=0 18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
- 3) Gable requires continuous bottom chord bearing
- 4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Date:

October 2, 2019

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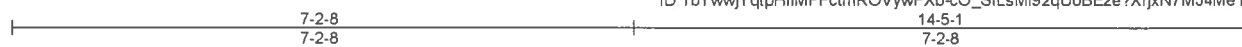
6904 Parke East Blvd
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264485
2100698	PB02	Piggyback	21	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11:02:03 2019 Page 1

ID 1bYwwjYqtpHfMFFctmROVywFXb-cO_SfLSMI92qUoBEze?XrpxN7MJ4MeTfx4W7jCyXTe2



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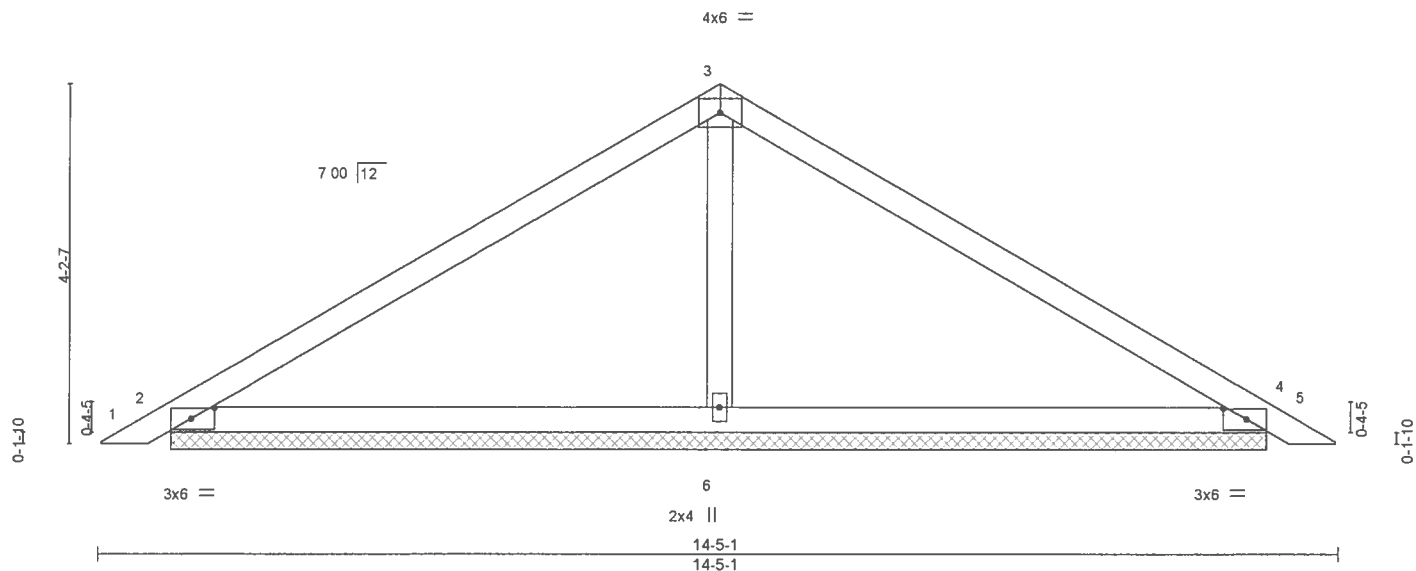


Plate Offsets (X,Y)-- [2 0-3-3,0-1-8], [4 0-3-3,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.48	Vert(LL)	0.02	5	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.39	Vert(CT)	0.04	5	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.09	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2017/TP12014		Matrix-S						Weight 49 lb	FT = 20%

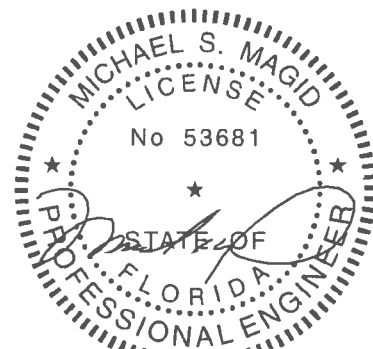
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (lb/size) 2=251/12-8-11, 4=251/12-8-11, 6=497/12-8-11
Max Horz 2=125(LC 11)
Max Uplift 2=-121(LC 12), 4=-138(LC 13), 6=-140(LC 12)
Max Grav 2=251(LC 23), 4=257(LC 20), 6=497(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-6=-306/181

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp. C, Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Gable requires continuous bottom chord bearing
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - 6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=121, 4=138, 6=140
 - 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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October 2, 2019

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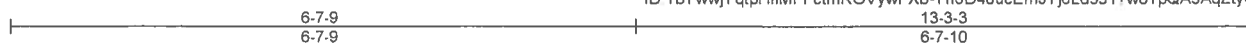


6904 Parke East Blvd
Tampa, FL 33610

Job 2100698	Truss PB02G	Truss Type GABLE	Qty 2	Ply 1	GBI - CRUTCHFIELD RES Job Reference (optional)	T18264486
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11 02 05 2019 Page 1
ID 1bYwYqtpHfMFFctmROVywFXb-Yn6D40ucEmJYj6Ld531?w81pQA5AqZtyOO?En5yXTe0



Scale = 1/24.7

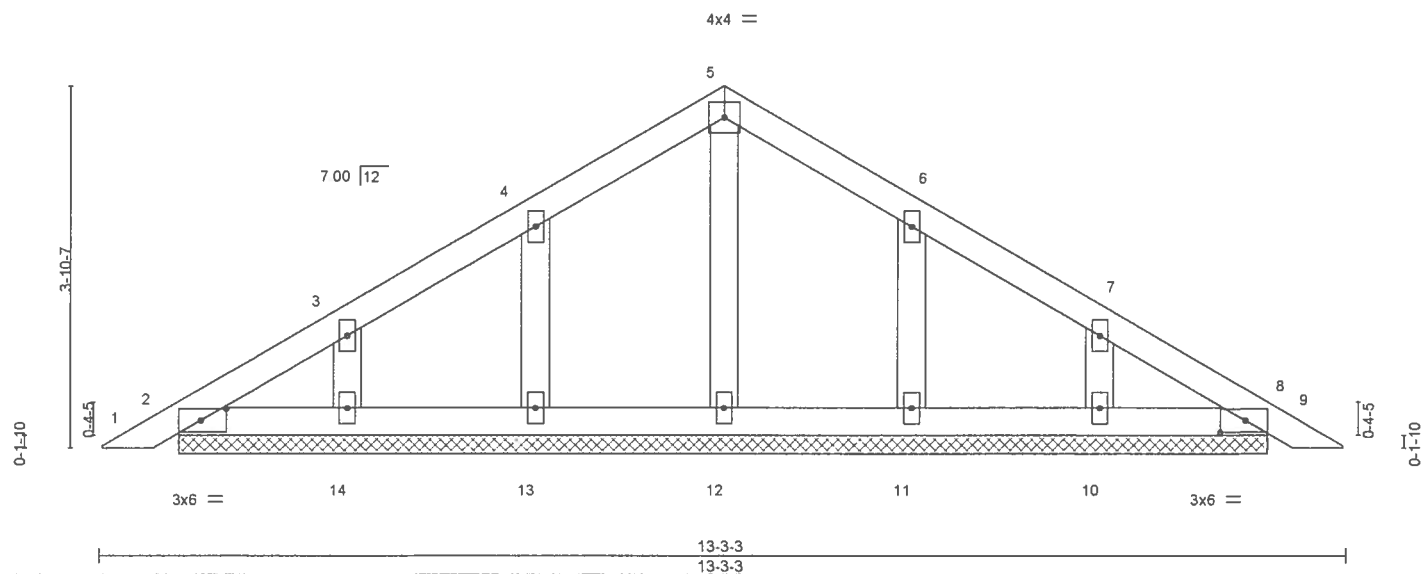


Plate Offsets (X,Y)-- [2-0-3-3,0-1-8], [8-0-3-3,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.05	Vert(LL)	0.00	8	n/r	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.03	Vert(CT)	0.00	8	n/r		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.04	Horz(CT)	0.00	8	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight 53 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No 3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

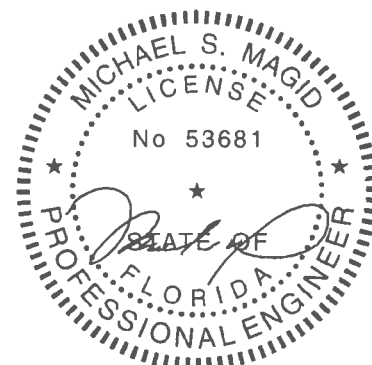
REACTIONS.

All bearings 11-6-14
(lb) - Max Horz 2=-115(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 8 except 13=-108(LC 12), 14=-104(LC 12), 11=-107(LC 13),
10=-103(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max Comp./Max Ten - All forces 250 (lb) or less except when shown

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II; Exp C, Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) 13=108, 14=104, 11=107, 10=103.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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October 2, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264487
2100698	T01	Piggyback Base	11	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

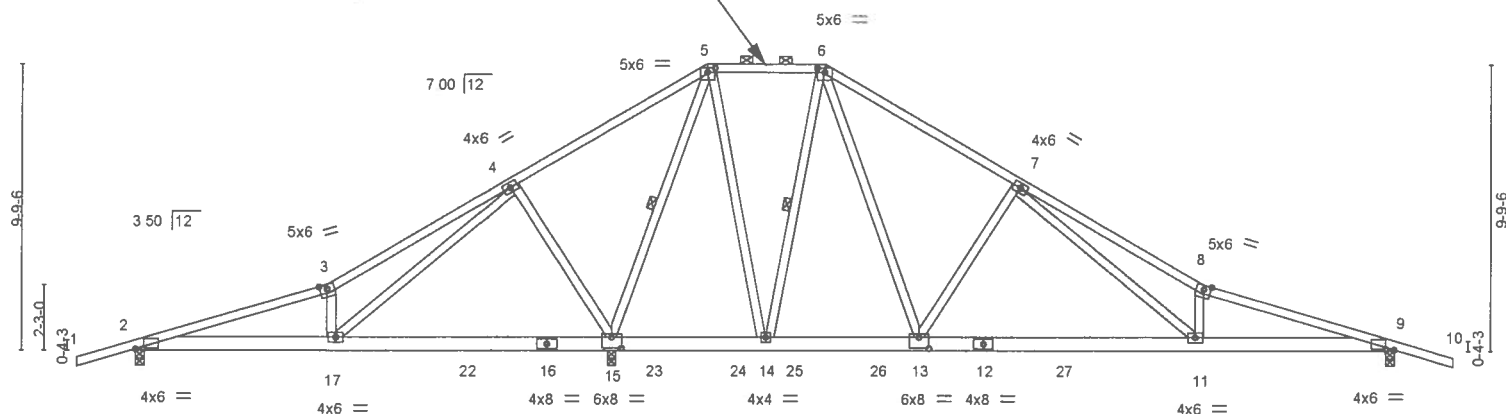
8 240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11 02 08 2019 Page 1

ID 1bYwwjYqtpHfMFFctmROVywFXb-zMnLi2wUXhh7aZ3CmBbYmfAZN_x1gO5MEuNqyXTdz

2-0-0	6-6-1	12-10-8	19-5-0	23-5-0	29-11-8	36-3-15	42-10-0	44-10-0	2-0-0
2-0-0	6-6-1	6-4-7	6-6-8	4-0-0	6-6-8	6-4-7	6-6-1	2-0-0	

TOP CHORD UNDER PIGGYBACKS TO BE Laterally
BRACED BY PURLINS AT 2-0-0 O.C. MAXIMUM
TYPICAL FOR ALL APPLICABLE TRUSS DESIGNS
IN THIS JOB.

Scale = 1/8" = 1'-0"



<div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div><div></div></div>									
Plate Offsets (X,Y)-- [2 0-3-7,0-0-4], [5 0-3-0,0-1-12], [6 0-3-0,0-1-12], [9 0-3-7,0-0-4], [13 0-4-0,0-4-8], [15 0-4-0,0-4-8]									
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.25		TC 0.69		Vert(LL) -0.16 11-13 >999 240		MT20 244/190	
TCDL	7.0	Lumber DOL 1.25		BC 0.54		Vert(CT) -0.30 11-13 >999 180			
BCLL	0.0 *	Rep Stress Incr YES		WB 0.93		Horz(CT) 0.01 9 n/a n/a			
BCDL	10.0	Code FBC2017/TPI2014		Matrix-MS				Weight 280 lb FT = 20%	

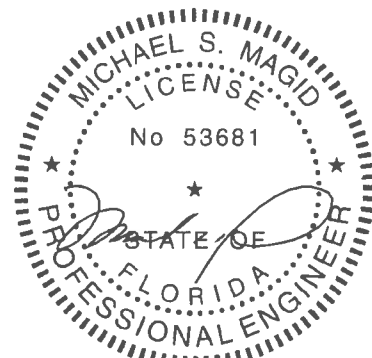
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x6 SP No 2
WEBS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins, except
2-0-0 oc purlins 5-6
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 5-15, 6-14

REACTIONS. (lb/size) 2=257/0-3-8, 15=2308/0-3-8, 9=821/0-3-8
Max Horz 2=-211(LC 13)
Max Uplift 2=-230(LC 8), 15=-770(LC 12), 9=-396(LC 13)
Max Grav 2=376(LC 23), 15=2308(LC 1), 9=880(LC 24)

FORCES. (lb) - Max Comp /Max Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-213/583, 3-4=-207/653, 4-5=-359/1212, 6-7=-529/326, 7-8=-2120/940,
8-9=-1948/763
BOT CHORD 2-17=-549/453, 15-17=-757/501, 14-15=-536/462, 13-14=-199/298, 11-13=-135/703,
9-11=-647/1833
WEBS 3-17=-310/251, 4-17=-339/773, 4-15=-537/436, 5-15=-1819/784, 5-14=-385/985,
6-14=-886/412, 6-13=-400/917, 7-13=-688/502, 7-11=-650/1440, 8-11=-775/463

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp C, Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - All bearings are assumed to be SP No 2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=230, 15=770, 9=396.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Michael S. Magid PE No.53681
MiTek USA, Inc. FL Cert 6634
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Date:

October 2, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264488
2100698	T01G	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11 02 12 2019 Page 1

ID 1bYwwjYqtpHfMFFctmROVywFXb-r71sYQz?awBZ3BNz?1feicpxL?QBj9?_?_C6WByXTdv

2-0-0	6-6-1	19-11-15	22-10-1	36-3-15	42-10-0	44-10-0
2-0-0	6-6-1	13-5-14	2-10-2	13-5-14	6-6-1	2-0-0

Scale = 1 81.5

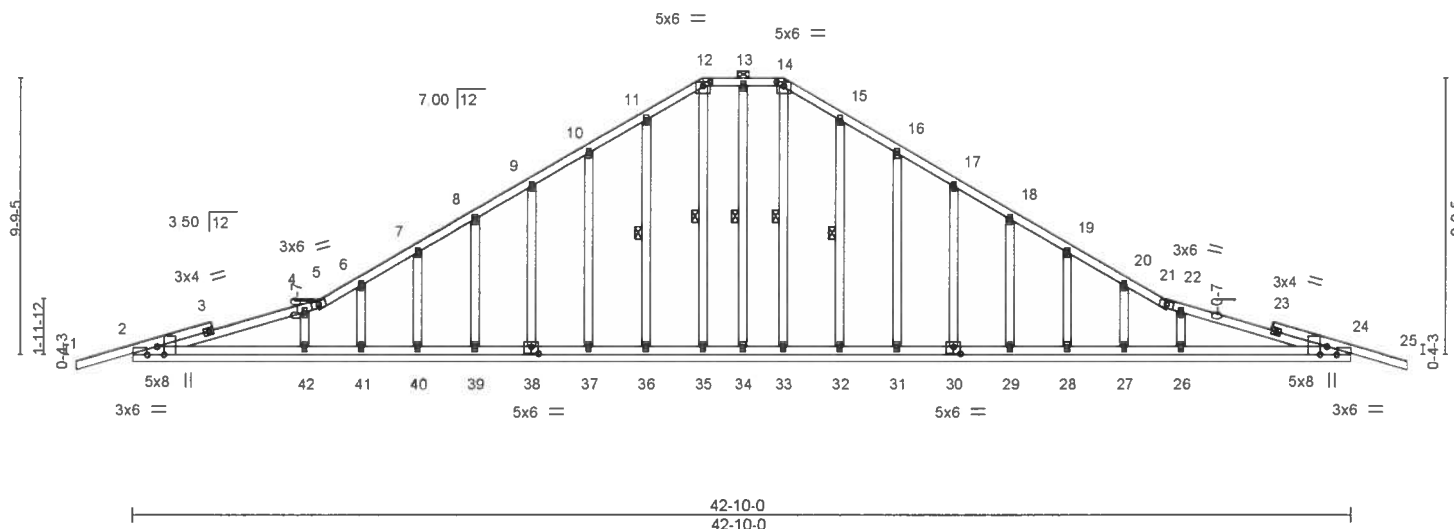


Plate Offsets (X,Y)--		[2 0-3-8,Edge], [2 0-4-1,Edge], [12 0-3-0,0-1-12], [14 0-3-0,0-1-12], [24 0-4-1,Edge], [24 0-3-8,Edge], [30 0-3-0,0-3-0], [38 0-3-0,0-3-0]	
LOADING (psf)	SPACING-	2-0-0	CSI.
TCLL 20 0	Plate Grip DOL	1 25	TC 0 32
TCDL 7 0	Lumber DOL	1 25	BC 0 26
BCLL 0 0 *	Rep Stress Incr	YES	WB 0 13
BCDL 10 0	Code	FBC2017/TPI2014	Matrix-S
			DEFL.
			in (loc) l/defl L/d
			Vert(LL) 0.01 25 n/r 120
			Vert(CT) 0.02 25 n/r 120
			Horz(CT) 0.01 24 n/a n/a
			PLATES
			MT20
			GRIP
			244/190
			Weight 284 lb FT = 20%

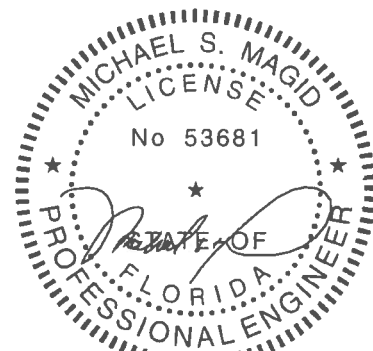
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max). 12-14
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 13-34, 15-32, 14-33, 11-36, 12-35

REACTIONS. All bearings 42-10-0
(lb) - Max Horz 2=210(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 34, 27, 29, 41, 39, 35 except 2=-252(LC 8), 24=-234(LC 9), 26=-177(LC 9), 28=-119(LC 13), 30=-102(LC 13), 31=-104(LC 13), 32=-101(LC 13), 42=-199(LC 8), 40=-120(LC 12), 38=-102(LC 12), 37=-103(LC 12), 36=-103(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 34, 27, 28, 29, 30, 31, 32, 33, 41, 40, 39, 38, 37, 36, 35 except 2=312(LC 23), 24=312(LC 24), 26=413(LC 24), 42=413(LC 23)

FORCES. (lb) - Max. Comp /Max Ten - All forces 250 (lb) or less except when shown
TOP CHORD 10-11=-177/272, 11-12=-230/323, 12-13=-213/302, 13-14=-213/302, 14-15=-230/323, 15-16=-177/261
WEBS 22-26=-282/232, 4-42=-282/234

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4 2psf, BCDL=3 0psf, h=18ft, Cat II, Exp C, Encl., GCp=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing
 - Gable studs spaced at 2-0-0 oc
 - This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No 2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 34, 27, 29, 41, 39, 35 except (jt=lb) 2=252, 24=234, 26=177, 28=119, 30=102, 31=104, 32=101, 42=199, 40=120, 38=102, 37=103, 36=103.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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October 2, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264489
2100698	T02	Piggyback Base	4	1	Job Reference (optional)	

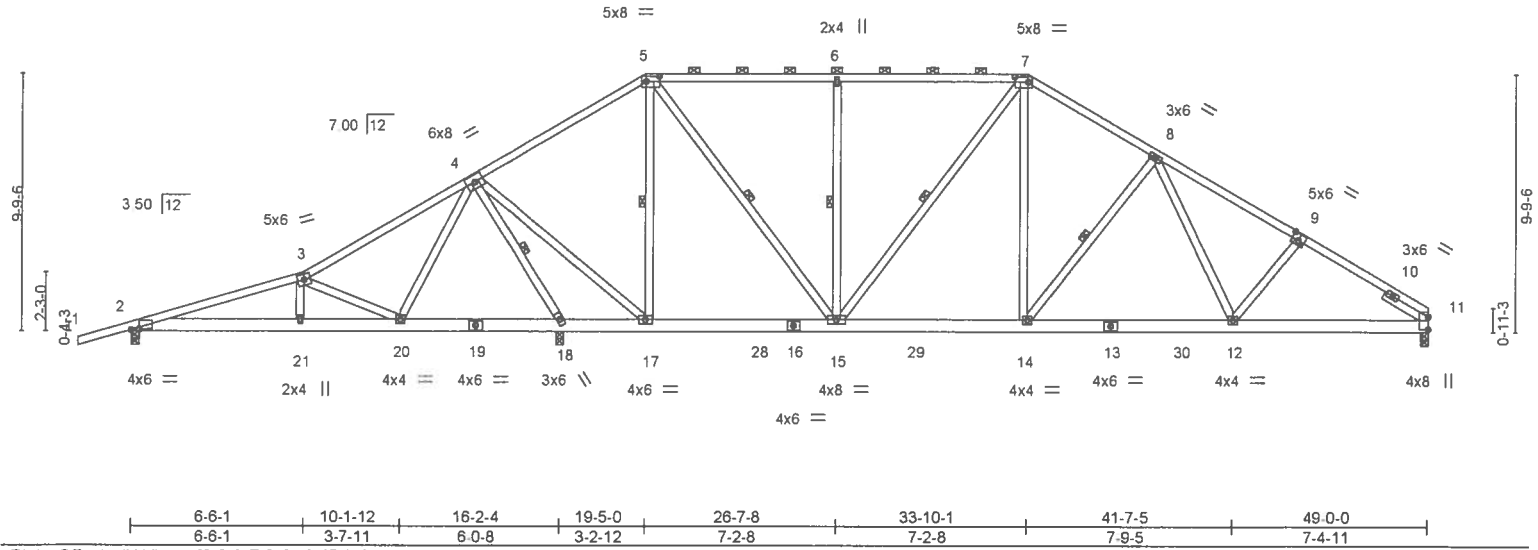
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ID 1bYwwjYqtpHfMFFctmROVywFXb-oW8dz5?F6XRGUUXM7Si6n1vD9o1dQPzHTlhDa3yXTdt

2-0-0	6-6-1	12-11-6	19-5-0	26-7-8	33-10-1	38-8-0	44-1-0	49-0-0
2-0-0	6-6-1	6-5-5	6-5-10	7-2-8	7-2-8	4-10-0	5-5-0	4-11-0

Scale = 1 87.5



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20 0	2-0-0	TC 0.62	in (loc) l/defl L/d	MT20	244/190
TCDL 7 0	Plate Grip DOL 1.25	BC 0.52	Vert(LL) -0 11 12-14 >999 240		
BCLL 0 0 *	Lumber DOL 1.25	WB 1.00	Vert(CT) -0 19 12-14 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MS	Horz(CT) 0 04 11 n/a n/a		
	Code FBC2017/TPI2014			Weight 341 lb	FT = 20%

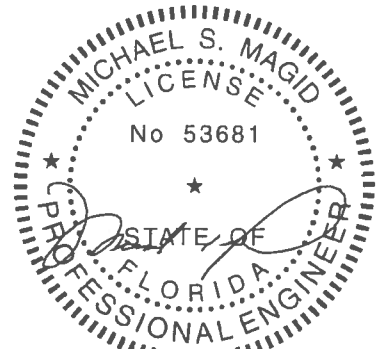
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x6 SP No 2
WEBS 2x4 SP No 3
SLIDER Right 2x4 SP No 3 1-11-8

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins, except
2-0-0 oc purlins: 5-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 4-18, 5-17, 5-15, 6-15, 7-15, 8-14

REACTIONS. (lb/size) 2=544/0-3-8, 18=2068/0-3-8, 11=1122/0-3-8
Max Horz 2=315(LC 9)
Max Uplift 2=-275(LC 8), 18=-696(LC 12), 11=-423(LC 13)
Max Grav 2=561(LC 23), 18=2068(LC 1), 11=1156(LC 24)

FORCES. (lb) - Max Comp /Max Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-847/331, 4-5=-305/222, 5-6=-854/552, 6-7=-854/552, 7-8=-1240/663,
8-9=-1591/760, 9-11=-1704/760
BOT CHORD 2-21=-386/777, 20-21=-384/763, 17-18=-1213/707, 15-17=-179/276, 14-15=-218/1027,
12-14=-406/1240, 11-12=-570/1419
WEBS 3-20=-703/416, 4-20=-126/403, 4-18=-2165/1078, 4-17=-713/1744, 5-17=-1102/534,
5-15=-500/1062, 6-15=-453/343, 7-15=-441/173, 7-14=-246/704, 8-14=-518/358,
8-12=-128/324

- NOTES-**
1) Unbalanced roof live loads have been considered for this design.
2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf, BCDL=3 0psf; h=18ft, Cat II, Exp C, Encl, GCpi=0 18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf.
6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=275, 18=696, 11=423.
8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Date:

October 2,2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264490
2100698	T02G	GABLE	1	1		

Builders FirstSource, Jacksonville, FL - 32244

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ID 1bYwwjYqtpHfMFFctmROVywFXb-gHO7pT2mAmxin6q7MHm2yt3vkPS0MEPsOvfQkryXTdp

-2-0-0	6-6-1	12-11-6	19-5-0	26-7-8	33-3-2	38-8-0	44-1-0	49-0-0	51-0-0
2-0-0	6-6-1	6-5-5	6-5-10	7-2-8	6-7-10	5-4-14	5-5-0	4-11-0	2-0-0

Scale = 1 88 3

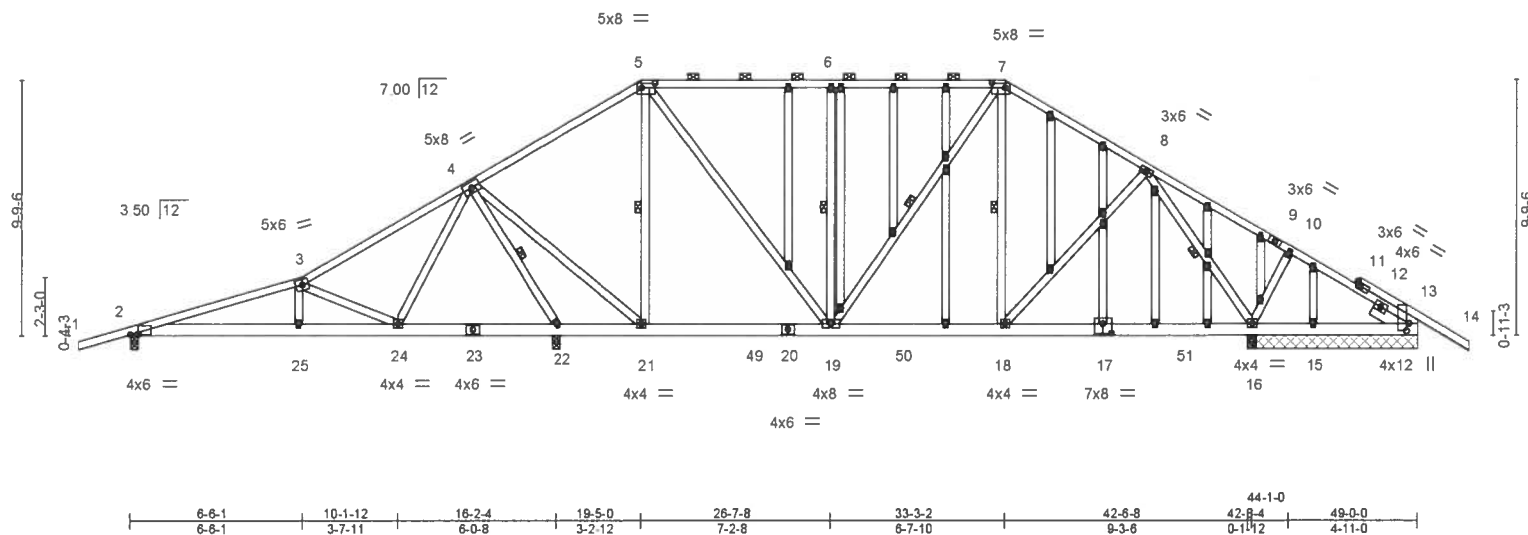


Plate Offsets (X,Y)-- [2 0-3-7,0-0-4], [5 0-6-0,0-2-4], [7 0-6-0,0-2-4], [13 0-0-0,0-2-1], [13 0-3-8,0-1-0], [17 0-4-0,0-4-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1 25	TC 0 58	Vert(LL)	-0 05 19-21	>999	240	MT20	244/190
TCDL 7 0	Lumber DOL	1 25	BC 0 36	Vert(CT)	-0 10 16-18	>999	180		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0 85	Horz(CT)	0 02 16	n/a	n/a		
BCDL 10 0	Code FBC2017/TPI2014		Matrix-MS					Weight 433 lb	FT = 20%

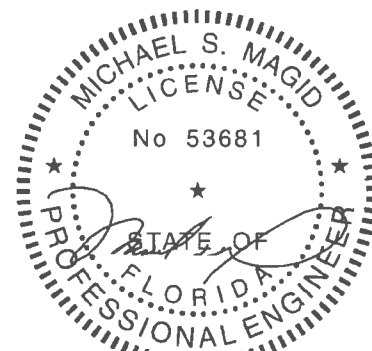
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x6 SP No 2
WEBS 2x4 SP No 3
OTHERS 2x4 SP No 3
SLIDER Right 2x6 SP No 2 1-5-13

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins, except 2-0-0 oc purlins: 5-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 4-22, 5-21, 6-19, 7-19, 7-18, 8-16

REACTIONS. All bearings 6-5-8 except (jt=length) 2=0-3-8, 22=0-3-8.
(lb) - Max Horz 2=321(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 15 except 2=-293(LC 8), 22=-626(LC 12), 16=-462(LC 13), 13=-123(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 15 except 2=600(LC 1), 22=1728(LC 1), 16=1394(LC 2), 16=1315(LC 1), 13=265(LC 24)

FORCES. (lb) - Max Comp /Max Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-983/449, 3-4=-350/177, 4-5=-310/230, 5-6=-631/456, 6-7=-631/456, 7-8=-762/437, 8-10=-65/331
BOT CHORD 2-25=-405/908, 24-25=-404/895, 21-22=-897/608, 19-21=-190/312, 18-19=-141/605, 16-18=-44/425
WEBS 3-24=-737/445, 4-24=-149/433, 4-22=-1796/892, 4-21=-501/1315, 5-21=-813/392, 5-19=-325/693, 6-19=-433/329, 8-18=-149/338, 8-16=-1082/519, 10-16=-298/230

- NOTES-**
- Unbalanced roof live loads have been considered for this design
 - Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf, BCDL=3 0psf, h=18ft, Cat II, Exp C, Encl, GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone,C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1 60 plate grip DOL=1 60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Provide adequate drainage to prevent water ponding
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable studs spaced at 2-0-0 oc
 - This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
 - All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15 except (jt=lb) 2=293, 22=626, 16=462, 13=123.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord



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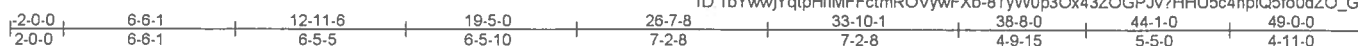
October 2,2019

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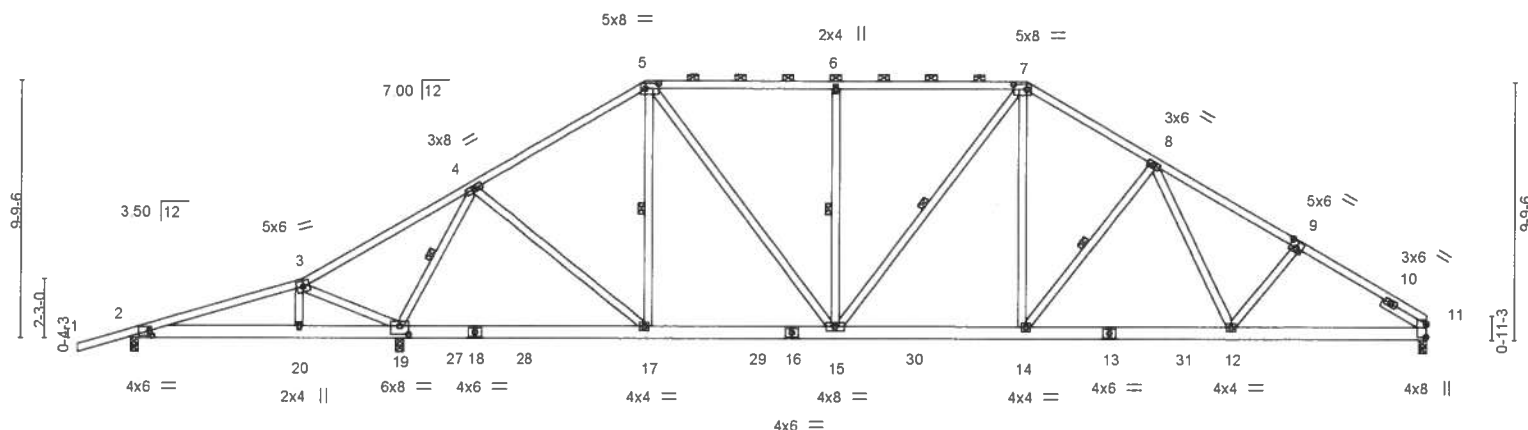
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Scale = 1.875



	6-6-1	10-1-12	19-5-0	26-7-8	33-10-1	41-7-5	49-0-0
Plate Offsets (X Y) --	6-6-1	3-7-11	9-3-4	7-2-8	7-2-8	7-9-5	7-4-11
	[2 0-1-0 0-2-0]	[5 0-6-0 0-2-4]	[7 0-6-0 0-2-4]	[9 0-3-0 0-3-0]	[11 0-5-15 0-0-4]	[19 0-4-0 0-3-12]	

LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d				PLATES GRIP		
TCLL	20.0	Plate Grip DOL	1.25	TC	0.63	Vert(LL)	-0.13	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.73	Vert(CT)	-0.23	12-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.97	Horz(CT)	0.06	11	n/a	n/a		
BCDL	10.0	Code FBC2017/TP12014		Matrix-MS							Weight 332 lb	FT = 20%

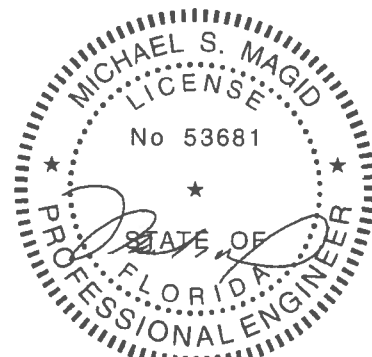
LUMBER-		BRACING-	
TOP CHORD	2x4 SP No 2	TOP CHORD	Structural wood sheathing directly applied or 3-1-5 oc purlins, except
BOT CHORD	2x6 SP No 2		2-0-0 oc purlins 5-7
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing
SLIDER	Right 2x4 SP No.3 1-11-8	WEBS	1 Row at midpt 4-19 5-17 6-15 7-15 8-14

REACTIONS. (lb/size) 2=292/0-3-8, 19=2057/0-3-8, 11=1385/0-3-8
 Max Horz 2=316(LC 9)
 Max Uplift 2=-333(LC 8), 19=-701(LC 12), 11=-482(LC 13)
 Max Grav 2=317(LC 23), 19=2057(LC 1), 11=1385(LC 1)

FORCES. (lb) - Max. Comp/Max. Ten - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-89/358, 3-4=-456/679, 4-5=-1230/600, 5-6=-1408/800, 6-7=-1408/800,
7-8=-1685/859, 8-9=-1994/938, 9-11=-2093/937
BOT CHORD 2-20=-368/199, 19-20=-366/198, 17-19=-197/530, 15-17=-312/989, 14-15=-387/1410
12-14=-570/1610, 11-12=-719/1749
WEBS 3-20=-290/180, 3-19=-418/610, 4-19=-1932/1081, 4-17=-298/807, 5-17=-321/243,
5-15=-371/470, 6-15=-452/343, 7-15=-244/707, 8-14=-496/353, 8-12=-118/281

NOTES-

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDF=4.2psf, BCDL=3.0psf, h=18ft, Cat II, Exp C; Encl . GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members, with BCDL = 10.0psf
- 6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=333, 19=701, 11=482
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord



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October 2, 2019

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6904 Parke East Blvd
Tampa, FL 36610

Scale = 1.914

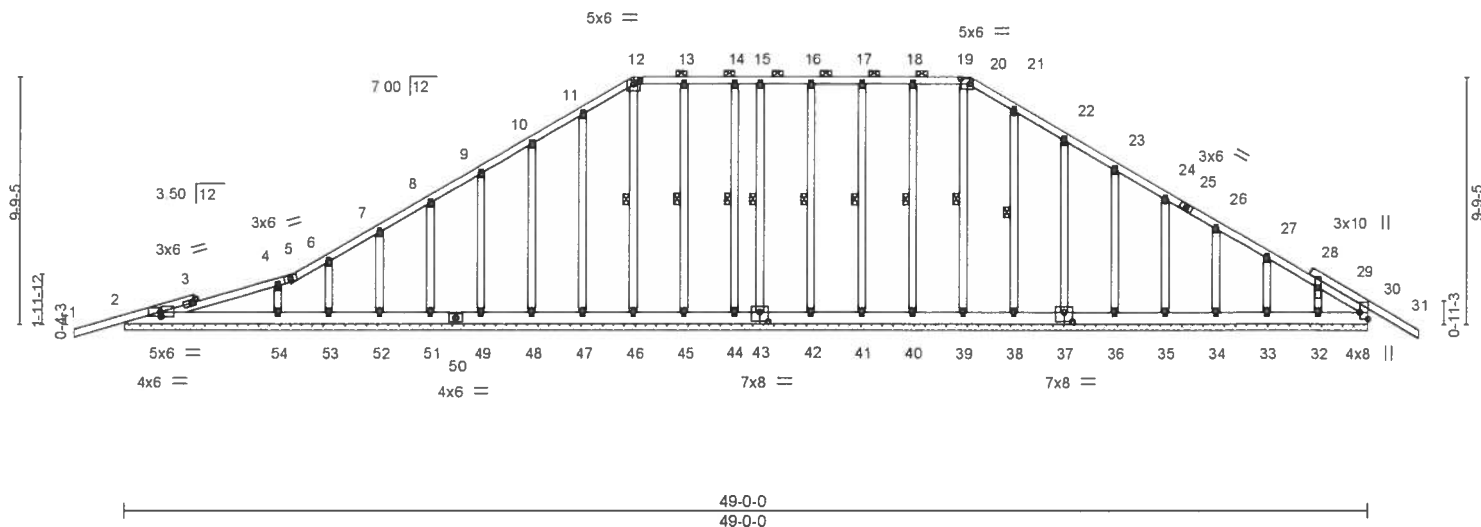


Plate Offsets (X,Y)-- [2 0-0-4,0-2-4], [2 0-0-4,0-2-0], [12 0-3-0,0-1-12], [19 0-1-12,0-0-0], [20 0-0-0,0-1-12], [20 0-4-8,0-2-8], [30 Edge,0-4-3], [37 0-4-0,0-4-8], [43 0-4-0,0-4-8]												
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d				PLATES GRIP		
TCLL	20 0	Plate Grip DOL	1.25	TC	0.28	Vent(LL)	-0.02	31	n/r	120	MT20	244/190
TCDL	7 0	Lumber DOL	1.25	BC	0.12	Vent(CT)	-0.02	31	n/r	120		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0.17	Horz(CT)	0.01	30	n/a	n/a		
BCDL	10 0	Code FBC2017/TPI2014		Matrix-S							Weight 408 lb	FT = 20%

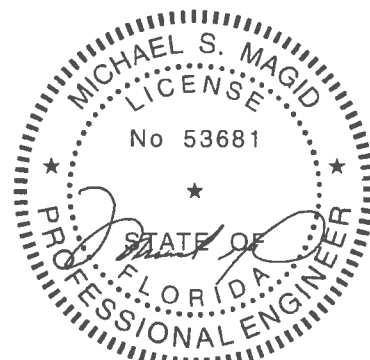
LUMBER-		BRACING-	
TOP CHORD	2x4 SP No 2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD	2x6 SP No 2		2-0-0 oc purlins: 12-20
OTHERS	2x4 SP No 3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing
		WEBS	1 Row at midpt 21-38, 19-39, 18-40, 17-41, 16-42, 15-43
			12-46 13-45 14-44

REACTIONS. All bearings 49-0-0.
(lb) - Max Horz 2=322(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 33, 38, 39, 40, 41, 42, 43, 53, 51, 46, 45, 44, 30 except
2=-243(LC 8), 32=-101(LC 13), 34=-102(LC 13), 35=-102(LC 13), 36=-100(LC 13), 37=-108(LC 13), 54=-193(LC
8), 52=-120(LC 12), 49=-102(LC 12), 48=-103(LC 12), 47=-105(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 53, 52, 51,
49, 48, 47, 46, 45, 44, 30 except 2=312(LC 1), 54=420(LC 1)

FORCES. (lb) - Max. Comp /Max Ten - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-273/149, 11-12=-208/255, 20-21=-209/255
 2-54=-184/254, 53-54=-184/254, 52-53=-184/254, 51-52=-184/254, 49-51=-184/254,
 48-49=-184/254, 47-48=-184/254, 46-47=-184/254, 45-46=-184/254, 44-45=-184/254
 43-44=-184/254, 42-43=-184/254, 41-42=-184/254, 40-41=-184/254, 39-40=-184/254
 38-39=-184/254, 37-38=-184/254, 36-37=-184/254, 35-36=-184/254, 34-35=-184/254
 33-34=-184/254, 32-33=-184/254, 30-32=-184/254
 WEBS 4-54=-264/207

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDF=4.2psf, BCDL=3.0psf, h=18ft, Cat II, Exp C, Encl , GCp=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2'-0" oc
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 33, 38, 39, 40, 41, 42, 43, 53, 51, 46, 45, 44, 30 except (J=Ib) 2=243, 32=101, 34=102, 35=102, 36=100, 37=108, 54=193, 52=120, 49=102, 48=103, 47=105
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264493
2100698	T04	Monopitch	17	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11 02 23 2019 Page 1

ID 1bYwwjYqtpHfIMFFctmROVywFXb-1EB0sA6v7la?ttj48rMDfxmnTQ8r1tbXBMBP2yXTdk

-2-0-0
2-0-0

7-3-8
7-3-8

Scale = 1:17.3

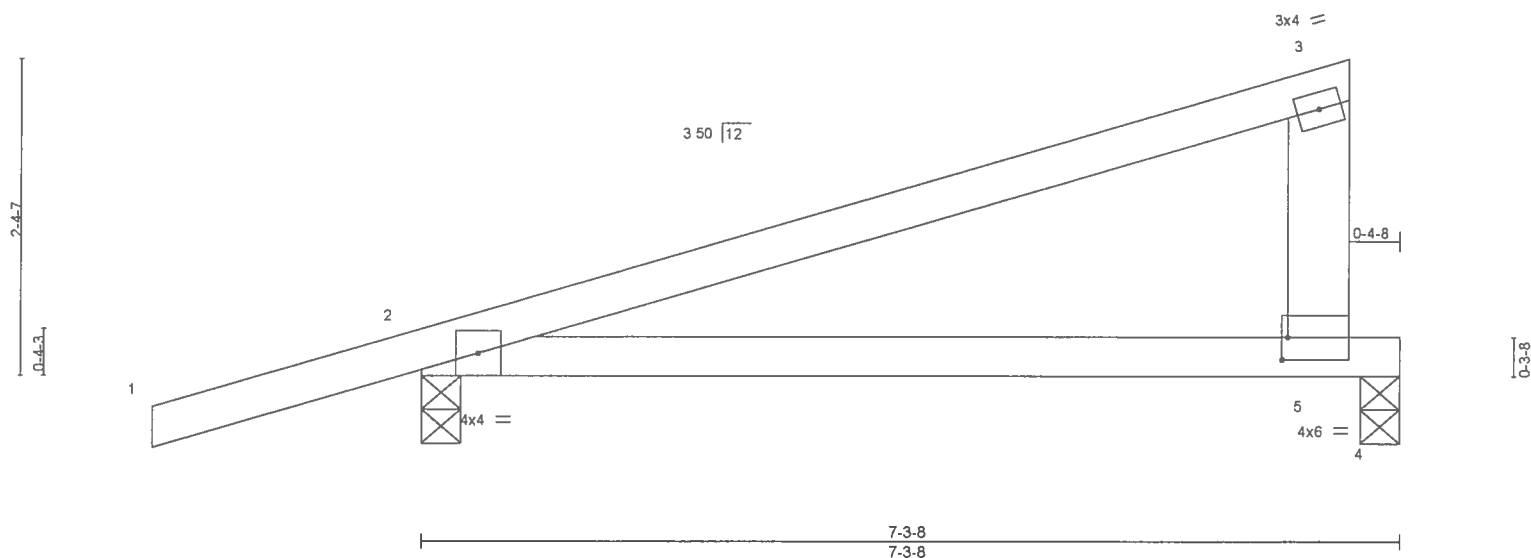


Plate Offsets (X, Y)-- [5 0-0-8, 0-2-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1.25	TC 0.46	Vert(LL)	0.14	5-8	>594	240	MT20	244/190
TCDL 7 0	Lumber DOL	1.25	BC 0.45	Vert(CT)	0.12	5-8	>690	180		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	2	n/a	n/a		
BCDL 10 0	Code FBC2017/TPI2014		Matrix-MS						Weight 29 lb	FT = 20%

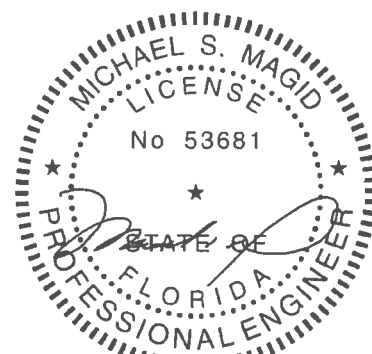
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
WEBS 2x6 SP No 2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals
BOT CHORD Rigid ceiling directly applied or 9-1-14 oc bracing

REACTIONS. (lb/size) 2=387/0-3-8, 4=225/0-3-8
Max Horz 2=133(LC 8)
Max Uplift 2=-321(LC 8), 4=-184(LC 8)

FORCES. (lb) - Max. Comp /Max. Ten. - All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-219/265
BOT CHORD 2-5=-316/171

- NOTES-**
- 1) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp C, Encl, GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone, porch left and right exposed; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - 4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=321, 4=184.



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Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264494
2100698	V01	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244

8 240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11 02 26 2019 Page 1
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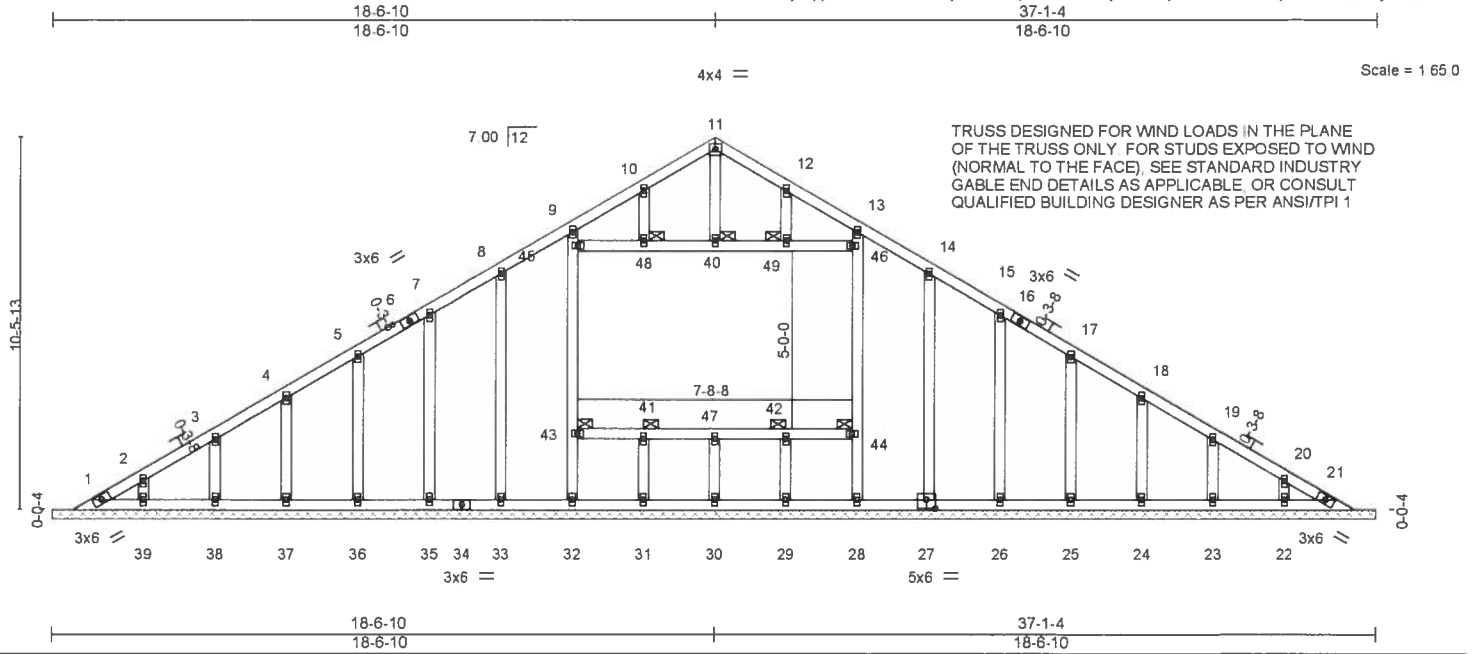


Plate Offsets (X,Y)-- [27 0-3-0,0-3-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	l/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.10	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.05	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.18	Horz(CT)	0.01	21	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight 238 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 40, 41, 42, 43, 44, 48, 49

REACTIONS. All bearings 37-1-4.
(lb) - Max Horz 1=-317(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 33, 39, 27, 22, 21 except 32=-142(LC 12), 35=-104(LC 12), 36=-101(LC 12), 37=-101(LC 12), 38=-103(LC 12), 28=-118(LC 13), 26=-103(LC 13), 25=-102(LC 13), 24=-101(LC 13), 23=-103(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 31, 33, 35, 36, 37, 38, 39, 29, 27, 26, 25, 24, 23, 22, 21, 30 except 32=285(LC 19), 28=258(LC 20)

FORCES. (lb) - Max. Comp /Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-325/182, 2-3=-296/161, 3-4=-271/136, 4-5=-258/123, 9-10=-260/163, 12-13=-260/163, 19-20=-266/100, 20-21=-285/124
BOT CHORD 1-39=-115/260, 38-39=-115/260, 37-38=-115/260, 36-37=-115/260, 35-36=-115/260, 33-35=-115/260, 32-33=-115/260, 31-32=-117/265, 30-31=-117/265, 29-30=-117/265, 28-29=-117/265, 27-28=-115/260, 26-27=-114/259, 25-26=-114/259, 24-25=-114/259, 23-24=-114/259, 22-23=-114/259, 21-22=-114/259

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp. C, Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 2x4 MT20 unless otherwise indicated
 - Gable requires continuous bottom chord bearing
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - All bearings are assumed to be SP No 2 crushing capacity of 565 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 33, 39, 27, 22, 21 except (if=lb) 32=142, 35=104, 36=101, 37=101, 38=103, 28=118, 26=103, 25=102, 24=101, 23=103



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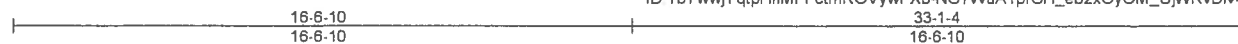


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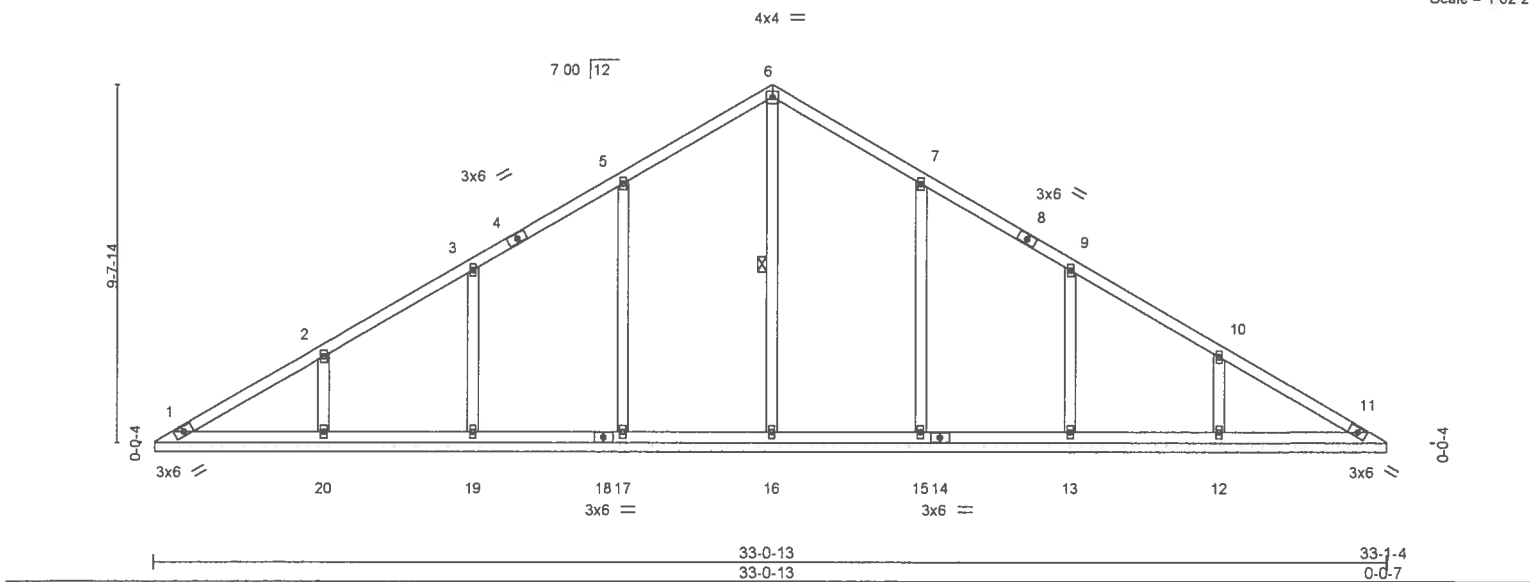
Job 2100698	Truss V02	Truss Type Valley	Qty 1	Ply 1	GBI - CRUTCHFIELD RES T18264495
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Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11 02 28 2019 Page 1
ID 1bYwwjYqtpHfMFFctmROVywFXb-NC7vuuA1prCH_eb2xOyOM_UjWRvDiv4LhT4y4FyXTdf



Scale = 1/62.2



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1 25	TC 0.18	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7 0	Lumber DOL	1 25	BC 0.18	Vert(CT)	n/a	-	n/a		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0.26	Horz(CT)	0 01	11	n/a		
BCDL 10 0	Code FBC2017/TPI2014		Matrix-S					Weight: 157 lb	FT = 20%

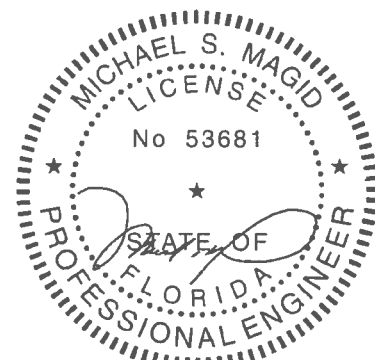
LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
WEBS 1 Row at midpt 6-16

REACTIONS. All bearings 33-0-7
(lb) - Max Horz 1=-291(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 11 except 17=-221(LC 12), 19=-192(LC 12), 20=-232(LC 12), 15=-221(LC 13), 13=-193(LC 13), 12=-232(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 16=401(LC 22), 17=478(LC 19), 19=369(LC 19), 20=365(LC 19), 15=477(LC 20), 13=370(LC 20), 12=364(LC 20)

FORCES. (lb) - Max Comp /Max Ten - All forces 250 (lb) or less except when shown.
TOP CHORD 5-6=-249/278, 6-7=-249/275
WEBS 5-17=-276/245, 3-19=-250/218, 2-20=-289/249, 7-15=-276/244, 9-13=-250/218, 10-12=-289/249

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp. C, Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - 3) All plates are 2x4 MT20 unless otherwise indicated.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0 psf.
 - 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11 except (jt=lb) 17=221, 19=192, 20=232, 15=221, 13=193, 12=232.



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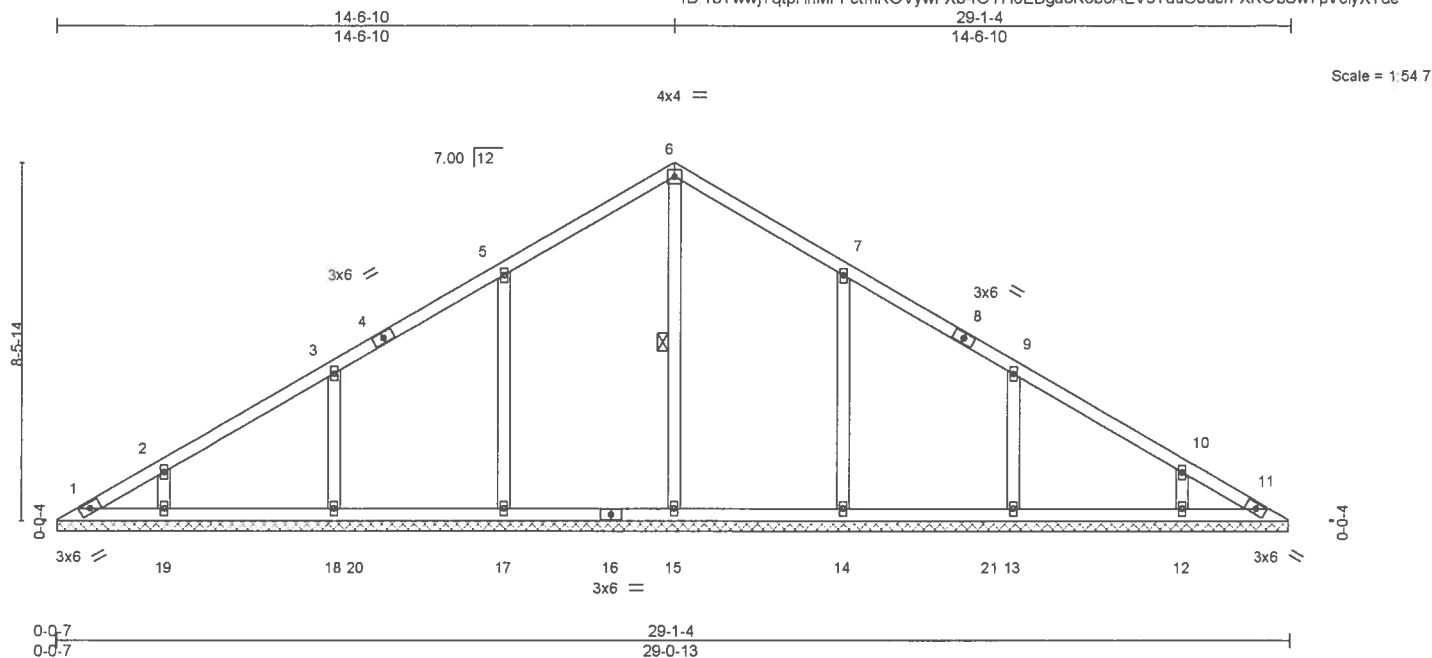


6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264496
2100698	V03	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244

8 240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11 02 29 2019 Page 1
ID 1bYwwjYqtpHfiMFFctmROVywFXb-rOYH6EBga8K8boAEV5TduC0ucrFXRObUw7pVciyXTde



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1.25	TC 0 16	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7 0	Lumber DOL	1.25	BC 0 17	Vert(CT)	n/a	-	n/a		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0 18	Horz(CT)	0 01	11	n/a	Weight 132 lb	FT = 20%
BCDL 10 0	Code FBC2017/TPI2014		Matrix-S						

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
WEBS 1 Row at midpt 6-15

REACTIONS. All bearings 29-0-7
(lb) - Max Horz 1=255(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1, 11 except 17=-218(LC 12), 18=-205(LC 12), 19=-175(LC 12), 14=-218(LC 13), 13=-206(LC 13), 12=-175(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 15=385(LC 22), 17=471(LC 19), 18=376(LC 19), 19=272(LC 19), 14=470(LC 20), 13=376(LC 20), 12=272(LC 20)

FORCES. (lb) - Max Comp./Max. Ten - All forces 250 (lb) or less except when shown
WEBS 5-17=-273/242, 3-18=-265/230, 7-14=-273/241, 9-13=-265/230

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design
 - 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4 2psf, BCDL=3 0psf, h=18ft, Cat. II, Exp C, Encl., GCpi=0 18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1 60
 - 3) All plates are 2x4 MT20 unless otherwise indicated.
 - 4) Gable requires continuous bottom chord bearing
 - 5) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
 - 6) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf.
 - 7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11 except (jt=lb) 17=218, 18=205, 19=175, 14=218, 13=206, 12=175.



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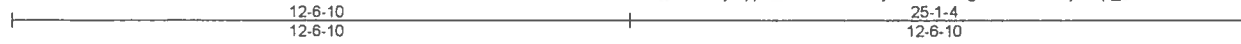


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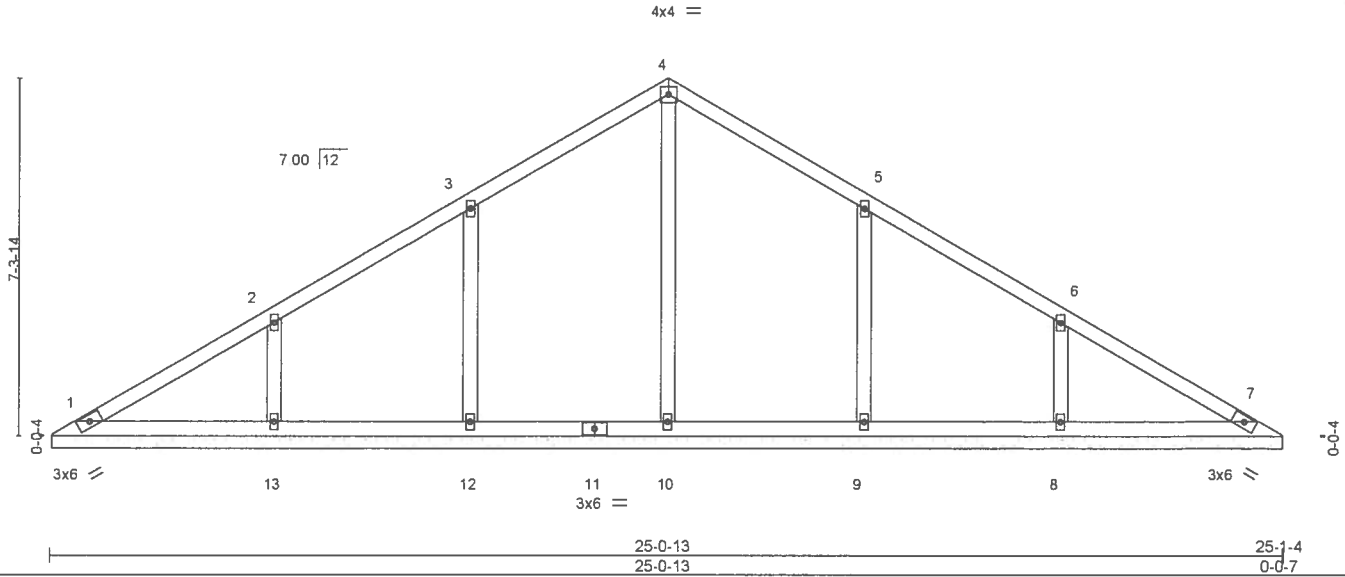
Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264497
2100698	V04	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244

8 240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11 02 30 2019 Page 1
ID: 1bYwwjYqtpHfIMFFctmROVywFXb-Kb6gKZBILSS?DylR3p_sRPZ3EFbTArd8nZ398yXTdd



Scale = 1/47.1



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1 25	TC 0.17	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7 0	Lumber DOL	1 25	BC 0.19	Vert(CT)	n/a	-	n/a		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0.16	Horz(CT)	0 01	7	n/a		
BCDL 10 0	Code FBC2017/TPI2014		Matrix-S					Weight 108 lb	FT = 20%

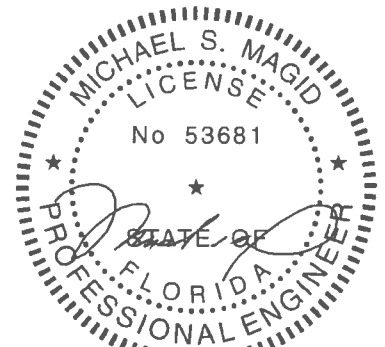
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 25-0-7
(lb) - Max Horz 1=218(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 12=-213(LC 12), 13=-227(LC 12), 9=-212(LC 13), 8=-227(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=389(LC 22), 12=392(LC 19), 13=359(LC 19), 9=391(LC 20), 8=359(LC 20)

FORCES. (lb) - Max Comp /Max Ten - All forces 250 (lb) or less except when shown.
WEBS 3-12=-268/239, 2-13=-285/244, 5-9=-268/238, 6-8=-285/244

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft, Cat II, Exp C, Encl, GCp=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - 3) All plates are 2x4 MT20 unless otherwise indicated.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 7) All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 12=213, 13=227, 9=212, 8=227.



Michael S. Magid PE No.53681
MiTek USA, Inc. FL Cert 6634
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Date:

October 2, 2019

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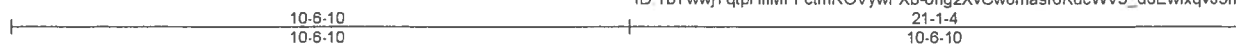


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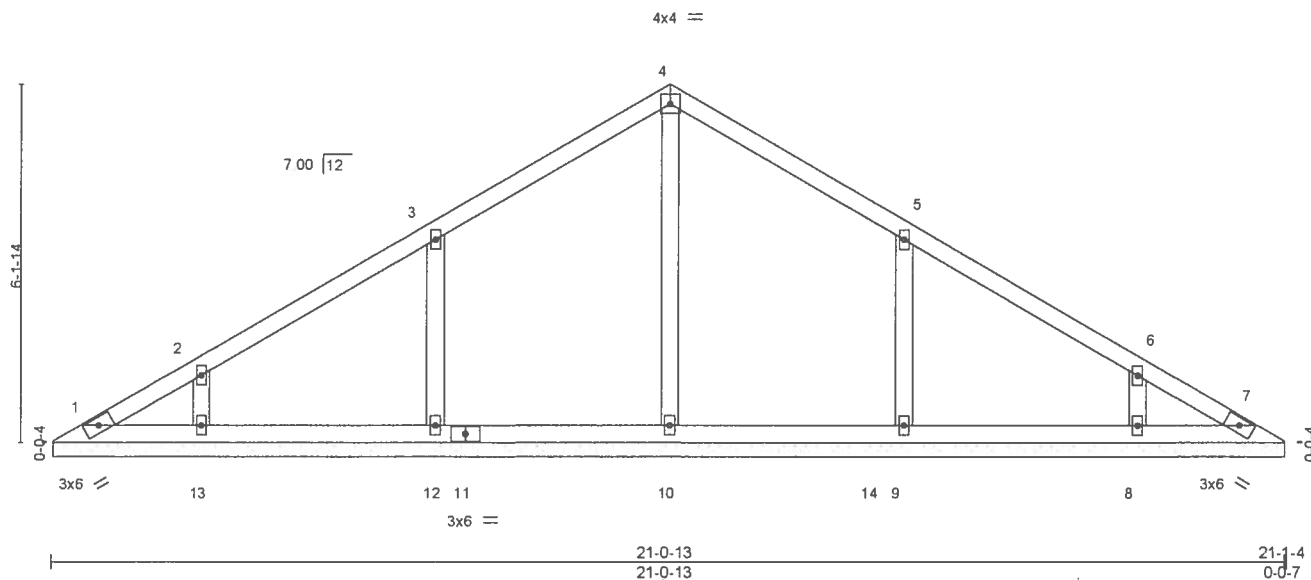
Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264498
2100698	V05	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11 02 31 2019 Page 1
ID 1bYwwjYqtpHfIMFFctmROVywFXb-ong2XvCw6masr6KdcWV5_d6EwfxqvJ9nNRichayXTdc



Scale = 1/32 = 1/39 6



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL 1.25	TC 0 17	Vert(LL) n/a	-	n/a	999	MT20	244/190
TCDL 7 0	Lumber DOL 1.25	BC 0 18	Vert(CT) n/a	-	n/a	999		
BCLL 0 0 *	Rep Stress Incr YES	WB 0 11	Horz(CT) 0.00	7	n/a	n/a		
BCDL 10 0	Code FBC2017/TPI2014	Matrix-S					Weight 87 lb	FT = 20%

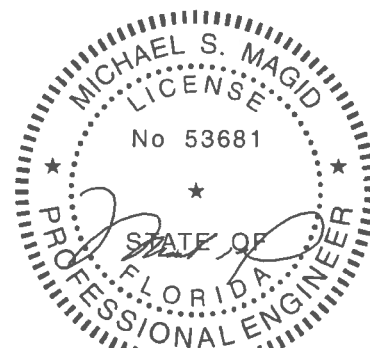
LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 21-0-7.
(lb) - Max Horz 1=-182(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 12=-227(LC 12), 13=-169(LC 12), 9=-226(LC 13), 8=-169(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=372(LC 19), 12=400(LC 19), 13=264(LC 19), 9=399(LC 20), 8=264(LC 20)

FORCES. (lb) - Max Comp /Max. Ten - All forces 250 (lb) or less except when shown
WEBS 3-12=-284/252, 5-9=-284/251

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4 2psf, BCDL=3 0psf, h=18ft, Cat. II; Exp C, Encl, GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - 3) All plates are 2x4 MT20 unless otherwise indicated
 - 4) Gable requires continuous bottom chord bearing
 - 5) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
 - 6) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf.
 - 7) All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 12=227, 13=169, 9=226, 8=169.



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Date:

October 2, 2019

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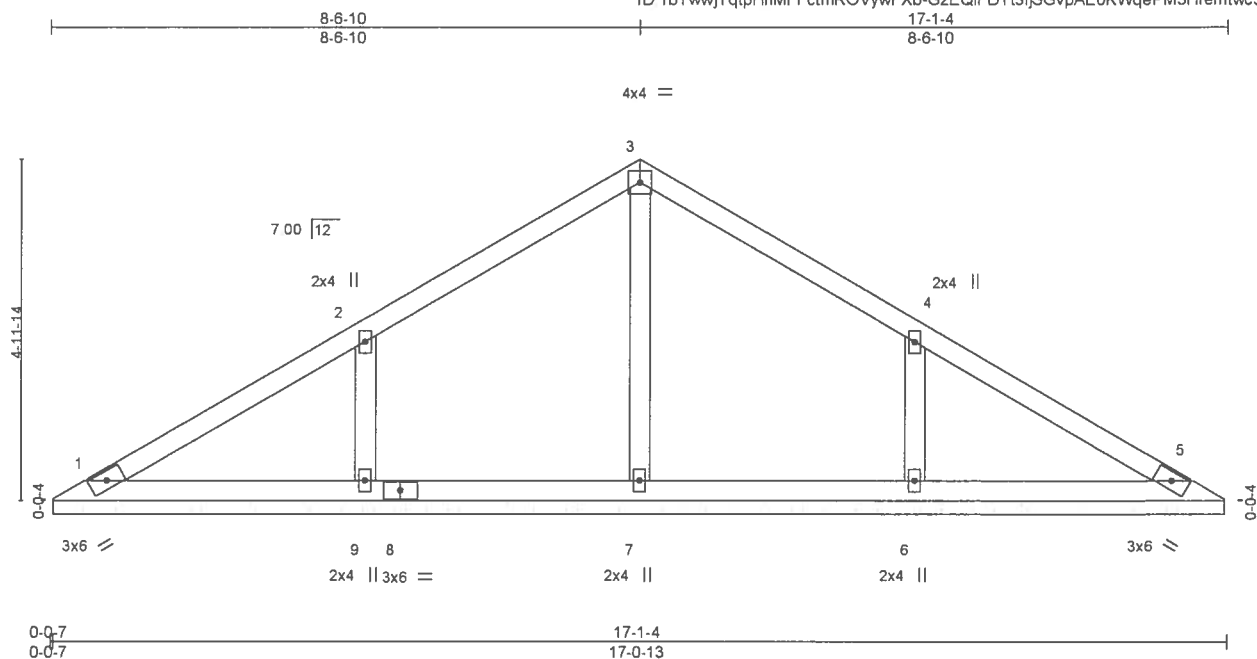


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Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264499
2100698	V06	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244,

8 240 s Jul 14 2019 MITek Industries, Inc. Wed Oct 2 11 02 32 2019 Page 1
ID 1bYwwjYqtpHfMFFctmROVywFXb-GzEQlFDYt3jSGvpAE0KWqePM3Hremtwc52AD1yXTdb



Scale = 1/32" = 1'

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20 0	Plate Grip DOL	1 25	TC 0 19	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7 0	Lumber DOL	1 25	BC 0 13	Vert(CT)	n/a	-	n/a		
BCLL 0 0 *	Rep Stress Incr	YES	WB 0 08	Horz(CT)	0 00	5	n/a		
BCDL 10 0	Code FBC2017/TPI2014		Matrix-S					Weight 66 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS.

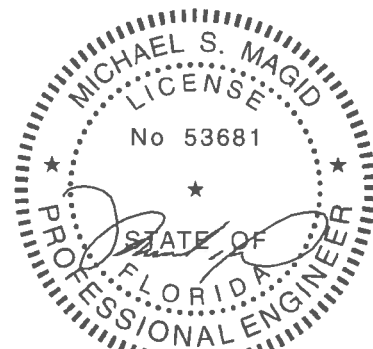
All bearings 17-0-6
(lb) - Max Horz 1=-146(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-247(LC 12), 6=-247(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 9=390(LC 19), 6=389(LC 20)

FORCES.

(lb) - Max Comp /Max. Ten - All forces 250 (lb) or less except when shown.
WEBS 2-9=-304/265, 4-6=-304/265

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3 0psf, h=18ft; Cat II; Exp C, Encl., GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=247, 6=247.



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October 2, 2019

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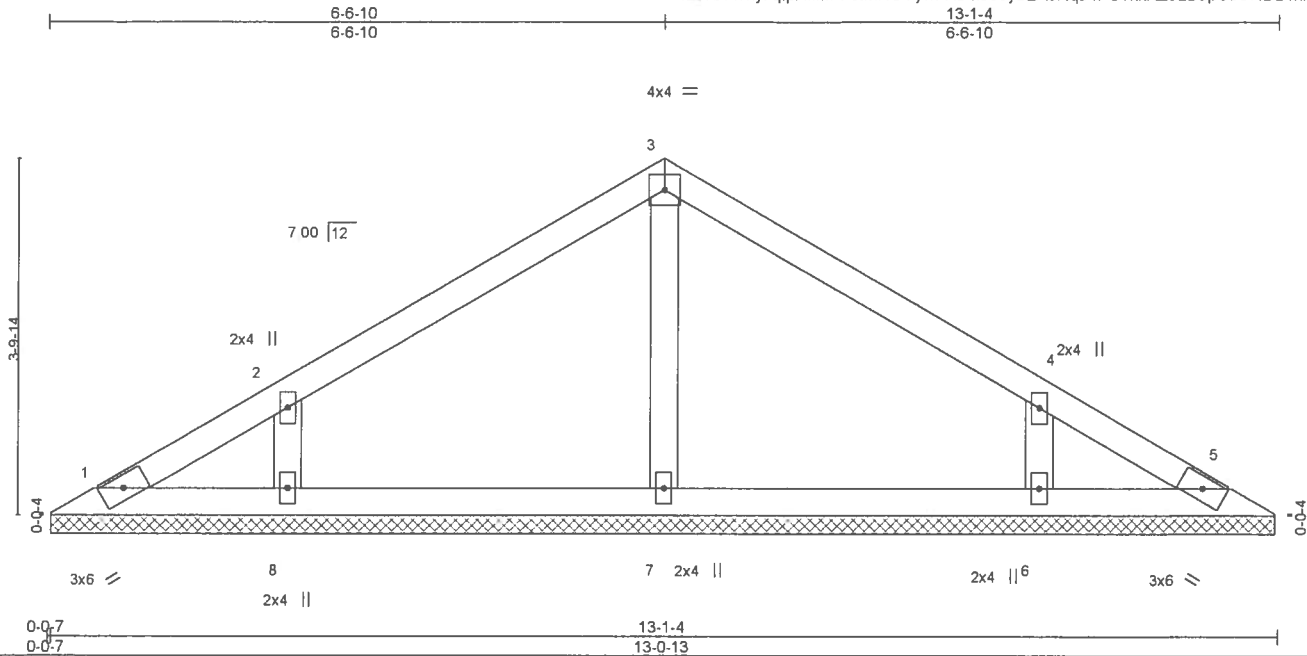


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Job 2100698	Truss V07	Truss Type Valley	Qty 1	Ply 1	GBI - CRUTCHFIELD RES Job Reference (optional)	T18264500
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8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11 02 33 2019 Page 1
ID 1bYwwjYqtPhfMFFctmROVywFXb-k9ooybEAeNqa4PU?kxXZ32BapSdGNDL4rinJlTyXTda



Scale = 1/24.7

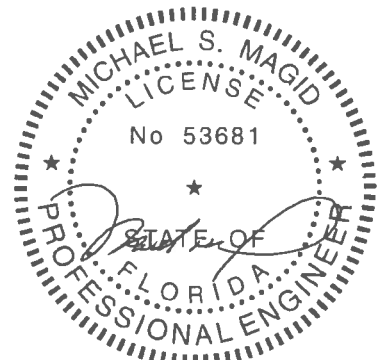
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.14	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.12	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.06	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 48 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No 2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD 2x4 SP No 2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
OTHERS 2x4 SP No 3	

REACTIONS. All bearings 13-0-6
(lb) - Max Horz 1=-109(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=-197(LC 12), 6=-197(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=258(LC 1), 8=304(LC 19), 6=303(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II; Exp. C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 - Gable requires continuous bottom chord bearing
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
 - All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (jt=lb) 8=197, 6=197.



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October 2, 2019

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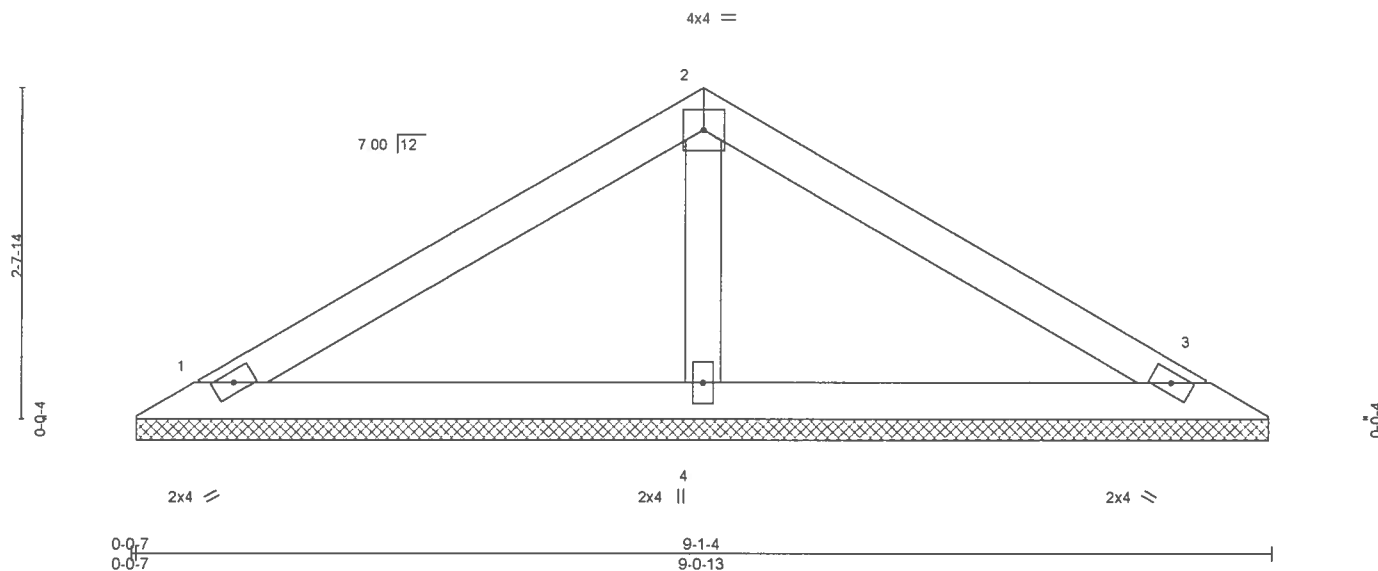
Job	Truss	Truss Type	Qty	Ply	GBI - CRUTCHFIELD RES	T18264501
2100698	V08	Valley	1	1	Job Reference (optional)	

Builders FirstSource, Jacksonville, FL - 32244

8.240 s Jul 14 2019 MiTek Industries, Inc Wed Oct 2 11:02:34 2019 Page 1
ID: 1bYwwjYqtpHfiMFFctmROVywFXb-CMMAAxEOPhyRiZ2CI3obFkvsyx6gzD3PXGIvyXTdZ



Scale = 1" = 18.5'



LOADING (psf)	SPACING-	2'-0"	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.19	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.16	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.04	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						Weight 30 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No 3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS.

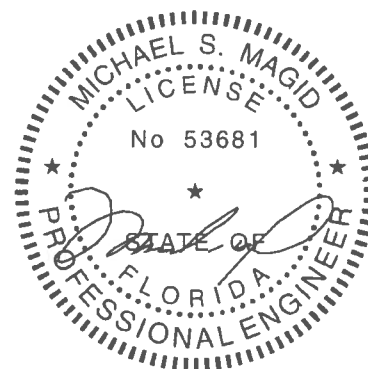
(lb/size) 1=140/9'-0"-6, 3=140/9'-0"-6, 4=314/9'-0"-6
Max Horz 1=-73(LC 8)
Max Uplift 1=-65(LC 12), 3=-75(LC 13), 4=-88(LC 12)
Max Grav 1=140(LC 1), 3=144(LC 20), 4=314(LC 1)

FORCES.

(lb) - Max Comp /Max Ten - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat II; Exp C; Encl, GCp=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.



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October 2, 2019

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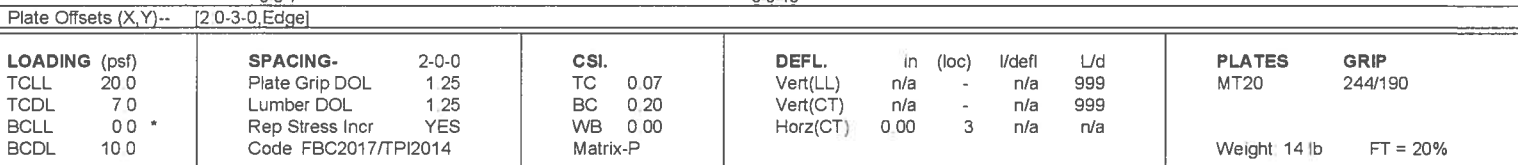
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Oct 2 11 02 35 2019 Page 1
ID:1bYwwYqtpHfMFFctmROVvwFXb-gYwZNHFQA 4IjdOrMa18TGxWGHw7rMI3GgdLyXTdY



LUMBER-
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2

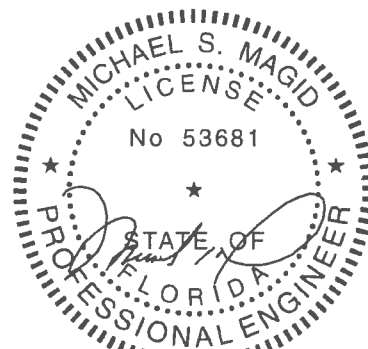
BRACING-	
TOP CHORD	Structural wood sheathing directly applied or 5-1-4 ac purlins
BOT CHORD	Rigid ceiling directly applied or 10-0-0 ac bracing

REACTIONS. (lb/size) 1=149/5-0-6, 3=149/5-0-6
Max Horz 1=37(LC 11)
Max Uplift 1=-55(LC 12), 3=-55(LC 13)

FORCES. (lb) - Max. Comp /Max. Ten - All forces 250 (lb) or less except when shown

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, Vult=130mph (3-second gust) Vasd=101mph, TCDF=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp. C, Encl. GCpi=0.18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone, C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gird DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Michael S. Magid PE No.53681
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

October 2.2019

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE

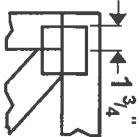
Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



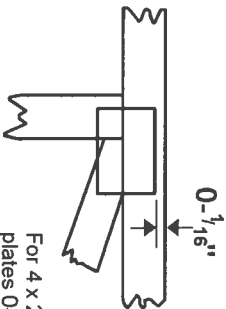
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Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MITek 20/20 software or upon request.

PLATE SIZE

4 X 4

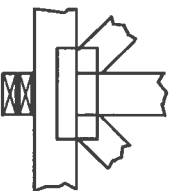
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



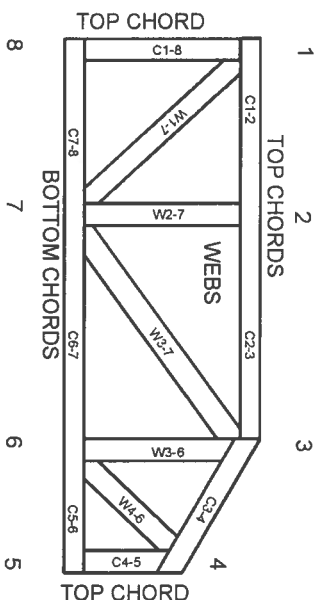
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MIL-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1 Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- 2 Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered
- 3 Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4 Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5 Cut members to bear tightly against each other.
- 6 Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7 Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1
- 8 Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9 Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
- 10 Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11 Plate type, size, orientation and location dimensions indicated are minimum plating requirements
- 12 Lumber used shall be of the species and size, and in all respects, equal to or better than that specified
- 13 Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14 Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 15 Connections not shown are the responsibility of others
- 16 Do not cut or alter truss member or plate without prior approval of an engineer
- 17 Install and load vertically unless indicated otherwise
- 18 Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use
- 19 Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20 Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



Load Short Form
Entire House
G.Buzbee

Job:
Date: Oct 03, 2019
By: Donna Brackeen

Project Information

For: Crutchfield
816 SW Heflin Ave., Ft. White, FL

Design Information

	Htg	Clg	Infiltration	Simplified
Outside db (°F)	33	92	Method	Average
Inside db (°F)	68	75	Construction quality	
Design TD (°F)	35	17	Fireplaces	
Daily range	-	M		
Inside humidity (%)	50	50		
Moisture difference (gr/lb)	29	47		

HEATING EQUIPMENT

Make Goodman Mfg.
Trade GOODMAN; JANITROL; AMANA DISTI...
Model GSZ140301K
AHRI ref 7995113

Efficiency 8.2 HSPF
Heating input
Heating output 27800 Btuh @ 47°F
Temperature rise 0 °F
Actual air flow 0 cfm
Air flow factor 0 cfm/Btuh
Static pressure 0.50 in H2O
Space thermostat
Capacity balance point = 31 °F

COOLING EQUIPMENT

Make Goodman Mfg.
Trade GOODMAN; JANITROL; AMANA DISTI...
Model GSZ140301K
AHRI ref 7995113

Efficiency 12.0 EER, 14 SEER
Sensible cooling 19600 Btuh
Latent cooling 8400 Btuh
Total cooling 28000 Btuh
Actual air flow 933 cfm
Air flow factor 0.050 cfm/Btuh
Static pressure 0.50 in H2O
Load sensible heat ratio 0.82

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
laund	88	2055	1592	0	79
bd3	156	2767	1777	0	88
c3	21	0	0	0	0
bth	48	986	416	0	21
shwr	18	0	0	0	0
Ln	7	0	0	0	0
c2	19	0	0	0	0
bd2	156	2821	2032	0	101
H	42	0	0	0	0
kit	217	924	2627	0	130
living	417	2908	4149	0	206
dining	300	2661	2416	0	120
pantry	45	0	0	0	0
entry	60	0	0	0	0

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.



m bd	264	3044	2675	0	132
wic	104	1236	469	0	23
m bth	97	1009	414	0	20
wc	28	1116	269	0	13
m shwr	23	0	0	0	0
Entire House	2110	21528	18837	0	933
Other equip loads		2275	1111		
Equip. @ 1.00 RSM			19949		
Latent cooling			4313		
TOTALS	2110	23804	24262	0	933

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.



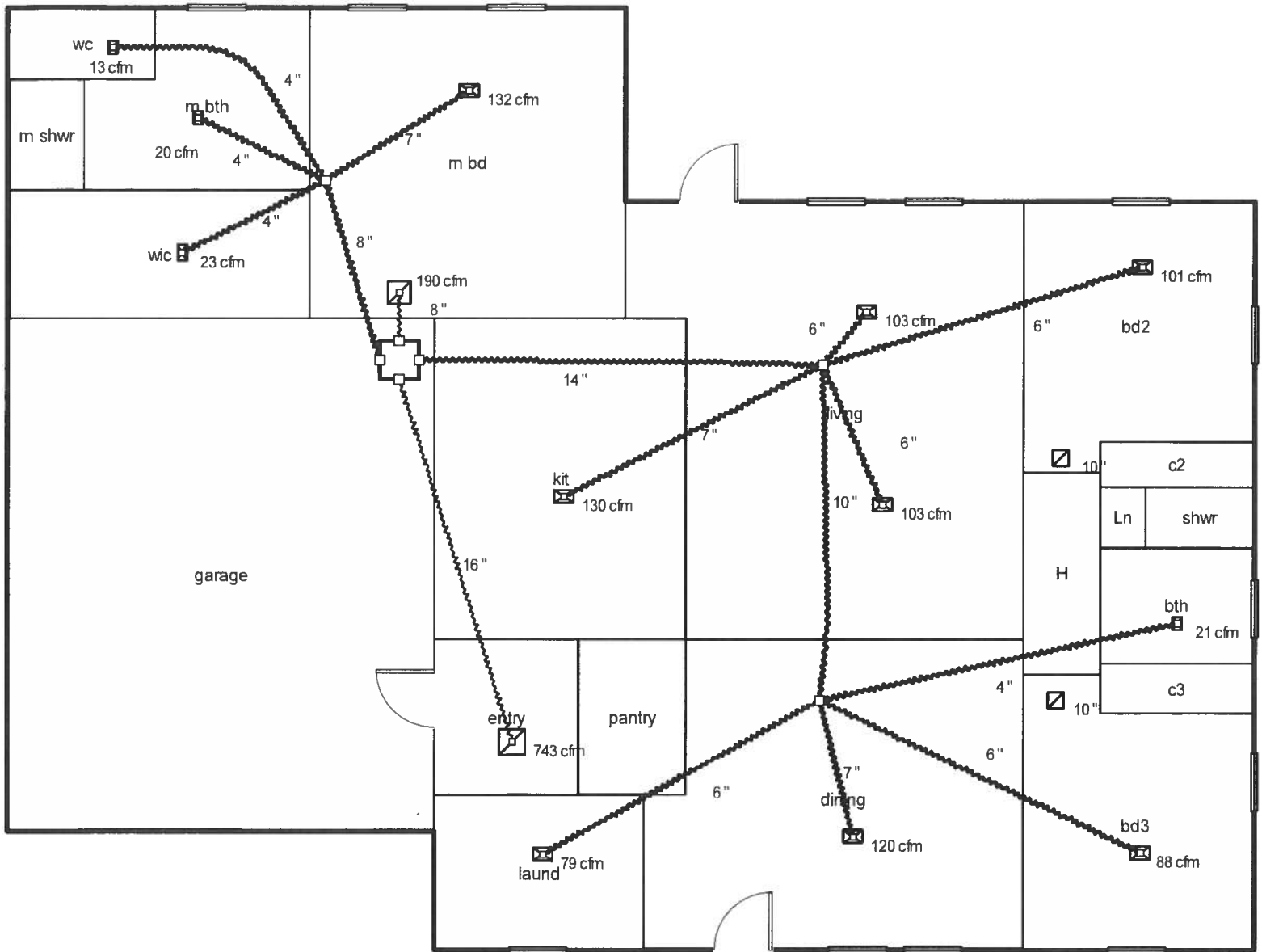
wrightsoft[®]
A McQuay-Norris Company

Right-Suite® Universal 2018 18.0.08 RSU07253

...ngs Electric\Buzbee\Crutchfield\Crutchfield.rup Calc = MJ8 Front Door faces: NE

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Page 2





Duct System Summary

Entire House

G.Buzbee

Job:
Date: Oct 03, 2019
By: Donna Brackeen

Project Information

For: Crutchfield
816 SW Heflin Ave., Ft. White, FL

	Heating	Cooling
External static pressure	0.50 in H ₂ O	0.50 in H ₂ O
Pressure losses	0.18 in H ₂ O	0.18 in H ₂ O
Available static pressure	0.32 in H ₂ O	0.32 in H ₂ O
Supply / return available pressure	0.250 / 0.070 in H ₂ O	0.250 / 0.070 in H ₂ O
Lowest friction rate	0.141 in/100ft	0.141 in/100ft
Actual air flow	0 cfm	933 cfm
Total effective length (TEL)	227 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	H x W (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
bd2	c 2032	0	101	0.188	6.0	0x0	VIFx	38.3	95.0	st1
bd3	c 1777	0	88	0.141	6.0	0x0	VIFx	56.8	120.0	st3
bth	c 416	0	21	0.141	4.0	0x0	VIFx	57.4	120.0	st3
dining	c 2416	0	120	0.151	7.0	0x0	VIFx	45.7	120.0	st3
kit	c 2627	0	130	0.191	7.0	0x0	VIFx	36.0	95.0	st1
laund	c 1592	0	79	0.143	6.0	0x0	VIFx	54.8	120.0	st3
living	c 2075	0	103	0.209	6.0	0x0	VIFx	24.5	95.0	st1
living-A	c 2075	0	103	0.202	6.0	0x0	VIFx	28.8	95.0	st1
m bd	c 2675	0	132	0.230	7.0	0x0	VIFx	18.5	90.0	st2
m bth	c 414	0	20	0.233	4.0	0x0	VIFx	17.1	90.0	st2
wc	c 269	0	13	0.209	4.0	0x0	VIFx	24.6	95.0	st2
wic-A	c 469	0	23	0.231	4.0	0x0	VIFx	18.1	90.0	st2

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
st3	Peak AVF	0	307	0.141	563	10.0	0 x 0	VinIFlx	st1
st1	Peak AVF	0	743	0.141	695	14.0	0 x 0	VinIFlx	
st2	Peak AVF	0	190	0.209	543	8.0	0 x 0	VinIFlx	



Return Branch Detail Table

Name	Grille Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x 0	0	743	49.7	0.141	532	16.0	0x 0		VIFx	
rb2	0x 0	0	190	32.5	0.216	543	8.0	0x 0		VIFx	

Project Name: Crutchfield Street: 816 SW Hefflin Ave. City, State, Zip: Ft. White , FL , Owner: Design Location: FL, Gainesville	Builder Name: G.Buzbee Permit Office: Permit Number: Jurisdiction: County: Columbia (Florida Climate Zone 2)
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Glass/Floor Area: 0.101	Total Proposed Modified Loads: 51.71 Total Baseline Loads: 52.01	PASS
-------------------------	---	------

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: Donna Brackeen DATE: 10/4/2019 I hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT: DATE: 10/17/19	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes. <div style="text-align: center;"> </div> BUILDING OFFICIAL: _____ DATE: _____
--	--

INPUT SUMMARY CHECKLIST REPORT

PROJECT

Title:	Crutchfield	Bedrooms:	3	Address Type:	Street Address
Building Type:	User	Conditioned Area:	2110	Lot #	
Owner Name:		Total Stories:	1	Block/Subdivision:	
# of Units:	1	Worst Case:	No	PlatBook:	
Builder Name:	G.Buzbee	Rotate Angle:	0	Street:	816 SW Hellin Ave.
Permit Office:		Cross Ventilation:		County:	Columbia
Jurisdiction:		Whole House Fan:		City, State, Zip:	Ft. White , FL ,
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

✓	Design Location	TMY Site	Design Temp 97.5 %	2.5 %	Int Design Temp Winter	Summer	Heating Degree Days	Design Moisture	Daily Temp Range
_____	FL, Gainesville	FL_GAINESVILLE_REGI	32	92	70	75	1305.5	51	Medium

BLOCKS

Number	Name	Area	Volume
1	Block1	2110	18990

SPACES

Number	Name	Area	Volume	Kitchen	Occupants	Bedrooms	Infil ID	Finished	Cooled	Heated
1	Main	2110	18990	Yes	4	3	1	Yes	Yes	Yes

FLOORS

✓	#	Floor Type	Space	Perimeter	R-Value	Area		Tile	Wood	Carpet
_____	1	Slab-On-Grade Edge Insulatio	Main	178.7 ft	0	2110 ft²	----	0.25	0.75	0

ROOF

✓	#	Type	Materials	Roof Area	Gable Area	Roof Color	Rad Barr	Solar Absor.	SA Tested	Emitt Tested	Emitt Tested	Deck Insul.	Pitch (deg)
_____	1	Gable or shed	Composition shingles	2444 ft²	616 ft²	Light	N	0.8	No	0.9	No	0	30.3

ATTIC

✓	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
_____	1	Full attic	Vented	300	2110 ft²	N	N

CEILING

✓	#	Ceiling Type	Space	R-Value	Ins Type	Area	Framing Frac	Truss Type
_____	1	Under Attic (Vented)	Main	38	Blown	2110 ft²	0.11	Wood

INPUT SUMMARY CHECKLIST REPORT

WALLS

✓	#	Omt	Adjacent To	Wall Type	Space	Cavity R-Value	Width Ft	In	Height Ft	In	Area	Sheathing R-Value	Framing Fraction	Solar Absor.	Below Grade%
✓	1	NE	Exterior	Frame - Wood	Main	19	42	6	9		382.5 ft²	0.69	0.23	0.5	0
✓	2	SE	Exterior	Frame - Wood	Main	19	22	6	9		202.5 ft²	0.69	0.23	0.5	0
✓	3	NW	Exterior	Frame - Wood	Main	19	49	0	9		441.0 ft²	0.69	0.23	0.5	0
✓	4	SW	Exterior	Frame - Wood	Main	19	32	4	9		291.0 ft²	0.69	0.23	0.5	0
✓	5	SW	Exterior	Frame - Wood	Main	19	32	4	9		291.0 ft²	0.69	0.23	0.5	0

DOORS

✓	#	Omt	Door Type	Space	Storms	U-Value	Width Ft	In	Height Ft	In	Area
✓	1	NE	Wood	Main	None	.29	3		6	8	20 ft²

WINDOWS

Orientation shown is the entered, Proposed orientation.

✓	#	Omt	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	Imp	Area	Overhang Depth	Separation	Int Shade	Screening
✓	1	NE	1	Vinyl	Low-E Double	Yes	0.33	0.23	N	72.0 ft²	7 ft 0 in	1 ft 0 in	Drapes/blinds	Exterior 5
✓	2	NW	3	Vinyl	Low-E Double	Yes	0.33	0.23	N	30.0 ft²	1 ft 0 in	1 ft 0 in	Drapes/blinds	None
✓	3	NW	3	Vinyl	Low-E Double	Yes	0.33	0.23	N	3.0 ft²	1 ft 0 in	1 ft 0 in	None	None
✓	4	SW	4	Vinyl	Low-E Double	Yes	0.33	0.23	N	36.0 ft²	1 ft 0 in	1 ft 0 in	Drapes/blinds	None
✓	5	SW	4	Vinyl	Low-E Double	Yes	0.33	0.23	N	3.0 ft²	1 ft 0 in	1 ft 0 in	None	None
✓	6	SW	5	Vinyl	Low-E Double	Yes	0.33	0.23	N	54.0 ft²	10 ft 0 in	1 ft 0 in	Drapes/blinds	Exterior 1
✓	7	SW	5	Vinyl	Low-E Double	Yes	0.33	0.23	N	15.0 ft²	10 ft 0 in	1 ft 0 in	Drapes/blinds	Exterior 1

GARAGE

✓	#	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
✓	1	589.625 ft²	589.625 ft²	48.7 ft	9 ft	1

INFILTRATION

#	Scope	Method	SLA	CFM 50	ELA	EqLA	ACH	ACH 50
1	Wholehouse	Proposed ACH(50)	.0004	2215.5	121.63	228.74	.1579	7

HEATING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Block	Ducts
✓	1	Electric Heat Pump/	Split	HSPF:8.2	27.8 kBtu/hr	1	sys#1

COOLING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Block	Ducts
✓	1	Central Unit/	Split	SEER: 14	28 kBtu/hr	840 cfm	0.75	1	sys#1

INPUT SUMMARY CHECKLIST REPORT

HOT WATER SYSTEM

✓	#	System Type	SubType	Location	EF	Cap	Use	SetPnt	Conservation
✓	1	Electric	None	Garage	0.95	50 gal	60 gal	120 deg	None

SOLAR HOT WATER SYSTEM

✓	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
✓	None	None			ft ²		

DUCTS

✓	#	--- Supply --- Location	R-Value	Area	--- Return --- Location	Area	Leakage Type	Air Handler	CFM 25 TOT	CFM25 OUT	QN	RLF	HVAC # Heat	Cool
✓	1	Attic	6	422 ft ²	Attic	105.5 ft	Default Leakage	Garage	(Default)	(Default)			1	1

TEMPERATURES

Programable Thermostat: Y					Ceiling Fans:									
Cooling	<input type="checkbox"/> Jan	<input type="checkbox"/> Feb	<input type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input checked="" type="checkbox"/> Jun	<input checked="" type="checkbox"/> Jul	<input checked="" type="checkbox"/> Aug	<input checked="" type="checkbox"/> Sep	<input type="checkbox"/> Oct	<input type="checkbox"/> Nov	<input checked="" type="checkbox"/> Dec		
Heating	<input checked="" type="checkbox"/> Jan	<input checked="" type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input checked="" type="checkbox"/> Dec		
Venting	<input type="checkbox"/> Jan	<input type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input checked="" type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input checked="" type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input checked="" type="checkbox"/> Dec		
Thermostat Schedule: FloridaCode 2017														
Schedule Type		1	2	3	4	5	6	7	8	9	10	11	12	
Cooling (WD)	AM	75	75	75	75	75	75	75	75	75	75	75	75	
	PM	75	75	75	75	75	75	75	75	75	75	75	75	
Cooling (WEH)	AM	75	75	75	75	75	75	75	75	75	75	75	75	
	PM	75	75	75	75	75	75	75	75	75	75	75	75	
Heating (WD)	AM	72	72	72	72	72	72	72	72	72	72	72	72	
	PM	72	72	72	72	72	72	72	72	72	72	72	72	
Heating (WEH)	AM	72	72	72	72	72	72	72	72	72	72	72	72	
	PM	72	72	72	72	72	72	72	72	72	72	72	72	

MASS

Mass Type	Area	Thickness	Furniture Fraction	Space
Default(8 lbs/sq.ft.)	0 ft ²	0 ft	0.3	Main

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 99

The lower the Energy Performance Index, the more efficient the home.

1. New home or, addition	1. <u>New (From Plans)</u>	12. Ducts, location & insulation level
2. Single-family or multiple-family	2. <u>Single-family</u>	a) Supply ducts R <u>6.0</u>
3. No. of units (if multiple-family)	3. <u>1</u>	b) Return ducts R <u>6.0</u>
4. Number of bedrooms	4. <u>3</u>	c) AHU location <u>Garage</u>
5. Is this a worst case? (yes/no)	5. <u>No</u>	13. Cooling system: Capacity <u>28.0</u>
6. Conditioned floor area (sq. ft.)	6. <u>2110</u>	a) Split system SEER <u>14.0</u>
7. Windows, type and area		b) Single package SEER <u> </u>
a) U-factor:(weighted average)	7a. <u>0.330</u>	c) Ground/water source SEER/COP <u> </u>
b) Solar Heat Gain Coefficient (SHGC)	7b. <u>0.230</u>	d) Room unit/PTAC EER <u> </u>
c) Area	7c. <u>213.0</u>	e) Other <u> </u>
8. Skylights		14. Heating system: Capacity <u>27.8</u>
a) U-factor:(weighted average)	8a. <u>NA</u>	a) Split system heat pump HSPF <u>8.2</u>
b) Solar Heat Gain Coefficient (SHGC)	8b. <u>NA</u>	b) Single package heat pump HSPF <u> </u>
9. Floor type, insulation level:		c) Electric resistance COP <u> </u>
a) Slab-on-grade (R-value)	9a. <u>0.0</u>	d) Gas furnace, natural gas AFUE <u> </u>
b) Wood, raised (R-value)	9b. <u> </u>	e) Gas furnace, LPG AFUE <u> </u>
c) Concrete, raised (R-value)	9c. <u> </u>	f) Other <u> </u>
10. Wall type and insulation:		15. Water heating system
A. Exterior:		a) Electric resistance EF <u>0.95</u>
1. Wood frame (Insulation R-value)	10A1. <u>19.0</u>	b) Gas fired, natural gas EF <u> </u>
2. Masonry (Insulation R-value)	10A2. <u> </u>	c) Gas fired, LPG EF <u> </u>
B. Adjacent:		d) Solar system with tank EF <u> </u>
1. Wood frame (Insulation R-value)	10B1. <u> </u>	e) Dedicated heat pump with tank EF <u> </u>
2. Masonry (Insulation R-value)	10B2. <u> </u>	f) Heat recovery unit HeatRec% <u> </u>
11. Ceiling type and insulation level		g) Other <u> </u>
a) Under attic	11a. <u>38.0</u>	16. HVAC credits claimed (Performance Method)
b) Single assembly	11b. <u> </u>	a) Ceiling fans <u>Yes</u>
c) Knee walls/skylight walls	11c. <u> </u>	b) Cross ventilation <u>No</u>
d) Radiant barrier installed	11d. <u>No</u>	c) Whole house fan <u>No</u>
		d) Multizone cooling credit <u> </u>
		e) Multizone heating credit <u> </u>
		f) Programmable thermostat <u>Yes</u>

*Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

I certify that this home has complied with the Florida Building Code, Energy Conservation, through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL display card will be completed based on installed code compliant features.

Builder Signature: _____

Date: _____

Address of New Home: 816 SW Heflin Ave.

City/FL Zip: Ft. White, FL