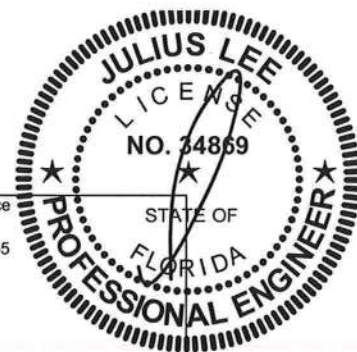




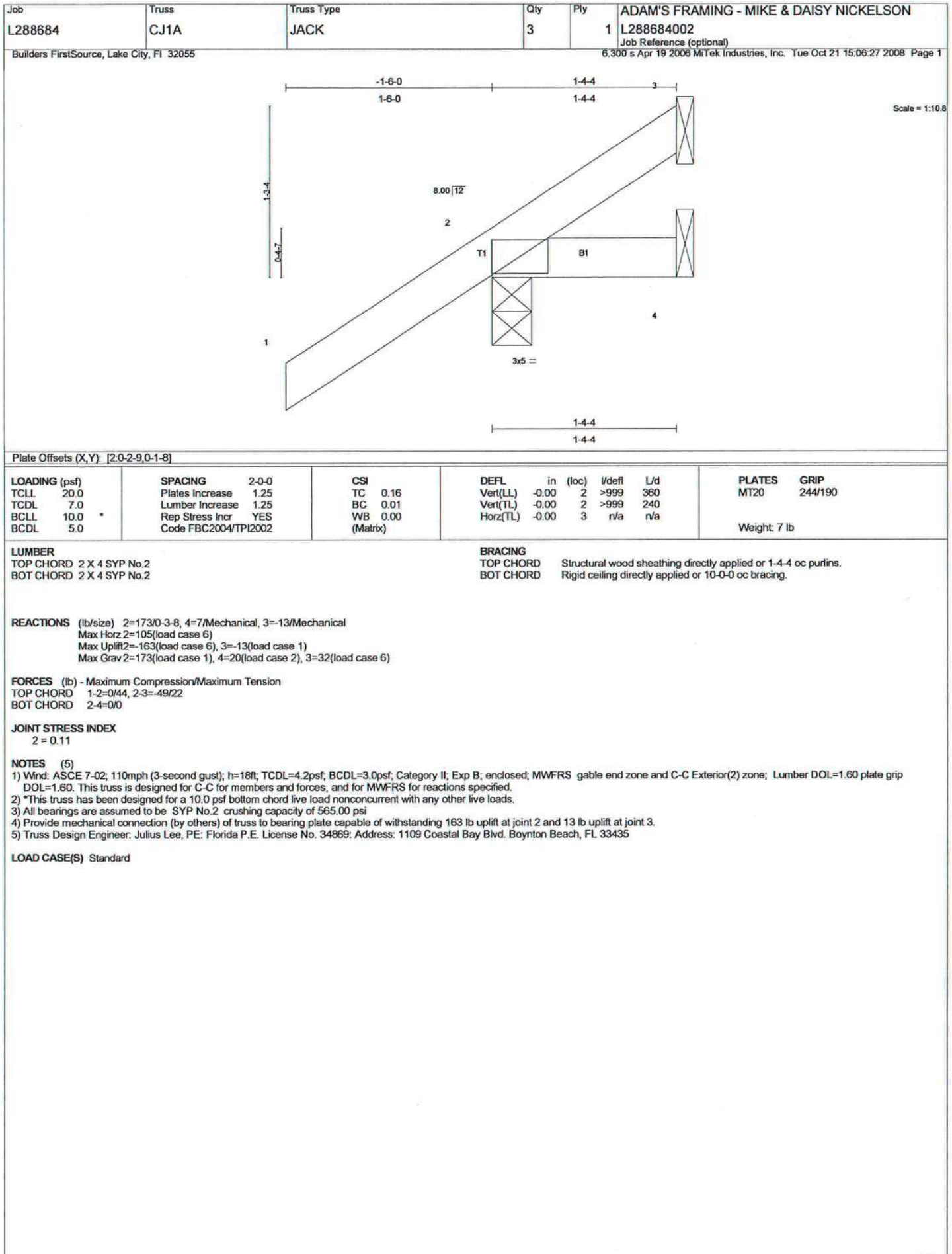
JULIUS LEE'S CONSULT. ENGR INC
1455 SW 4TH AVE , DELRAY BEACH
FLORIDA. 33444

Exposure: B

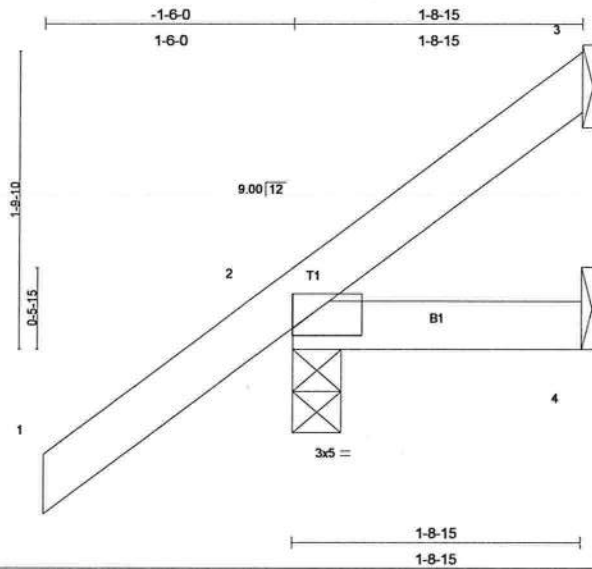
License # : CBC1253409
License # : 34869

Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:06:20 2008 Page 1

LOAD CASE(S) Standard



Job L288684	Truss CJ1B	Truss Type JACK	Qty 3	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684003 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:06:34 2008 Page 1		



Scale = 1:13.4

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.16	Vert(LL) -0.00 2 >999 360	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.02	Vert(TL) -0.00 2 >999 240	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a	
BCDL 5.0	Code FBC2004/TP12002	(Matrix)		Weight: 9 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 1-8-15 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

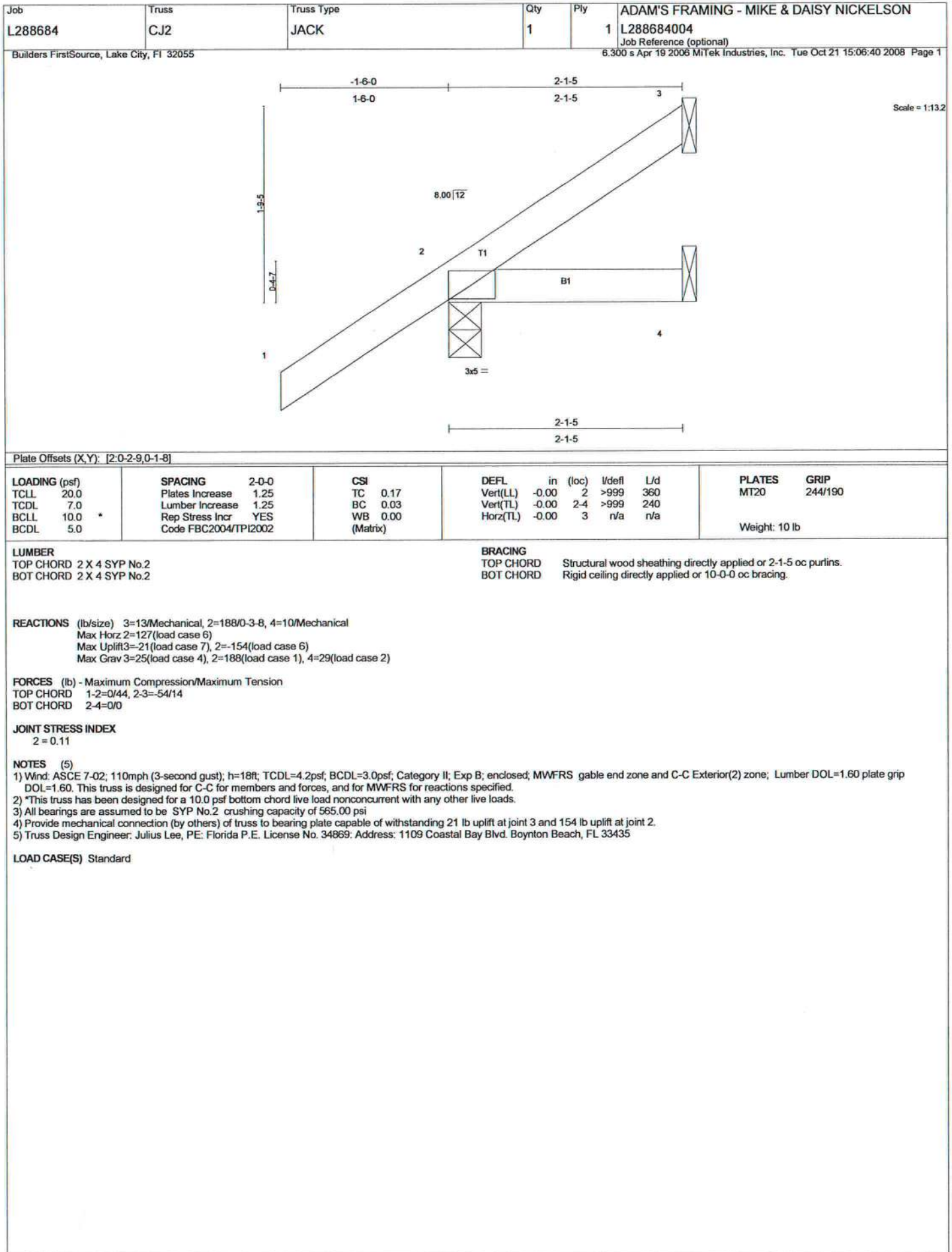
REACTIONS (lb/size) 2=174/0-3-8, 4=9/Mechanical, 3=9/Mechanical
Max Horz 2=129(load case 6)
Max Uplift 2=140(load case 6), 3=22(load case 7)
Max Grav 2=174(load case 1), 4=26(load case 2), 3=24(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-54/15
BOT CHORD 2-4=0/0

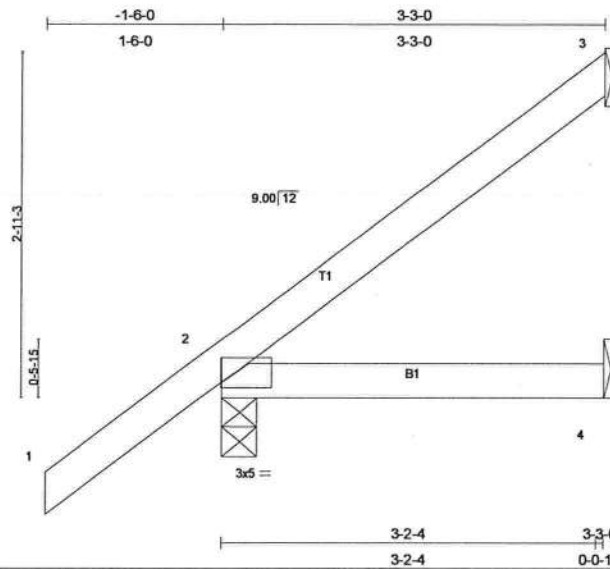
JOINT STRESS INDEX
2 = 0.20

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 2 and 22 lb uplift at joint 3.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job L288684	Truss CJ3	Truss Type JACK	Qty 1	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684005 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:06:45 2008 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.17	Vert(LL) -0.00 2-4 >999 360	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.07	Vert(TL) -0.01 2-4 >999 240	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 14 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

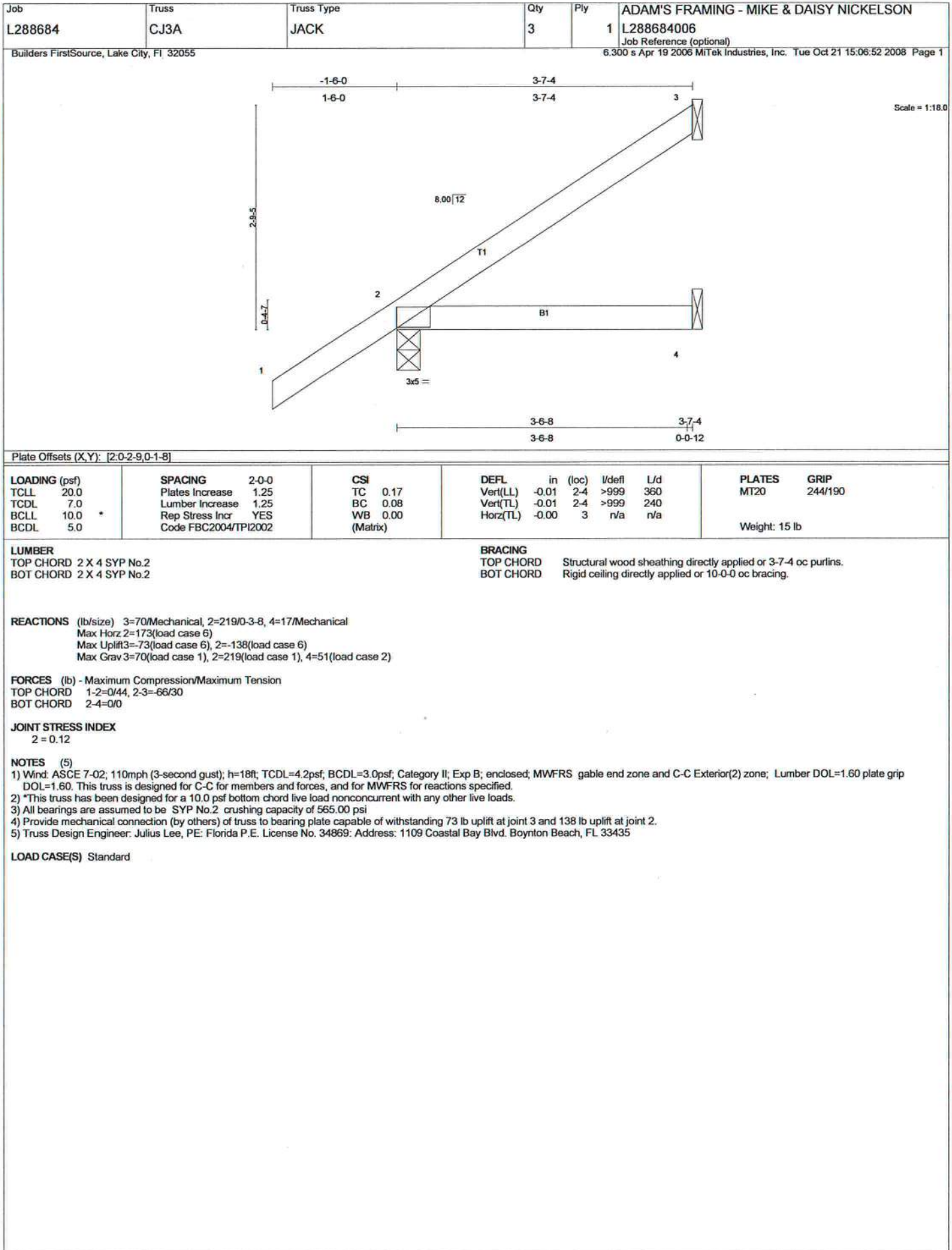
REACTIONS (lb/size) 3=58/Mechanical, 2=210/0-3-8, 4=15/Mechanical
Max Horz 2=180(load case 6)
Max Uplift 3=72(load case 6), 2=126(load case 6)
Max Grav 3=58(load case 1), 2=210(load case 1), 4=46(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-68/27
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.24

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 3 and 126 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job L288684	Truss CJ3B	Truss Type JACK	Qty 3	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684007 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:06:59 2008 Page 1		

Scale = 1:19.8

LOADING (psf) TCCL 20.0 TCDL 7.0 BCCL 10.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.17 BC 0.08 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.01 2-4 >999 360 Vert(TL) -0.01 2-4 >999 240 Horz(TL) -0.00 3 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 15 lb
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LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 3-6-4 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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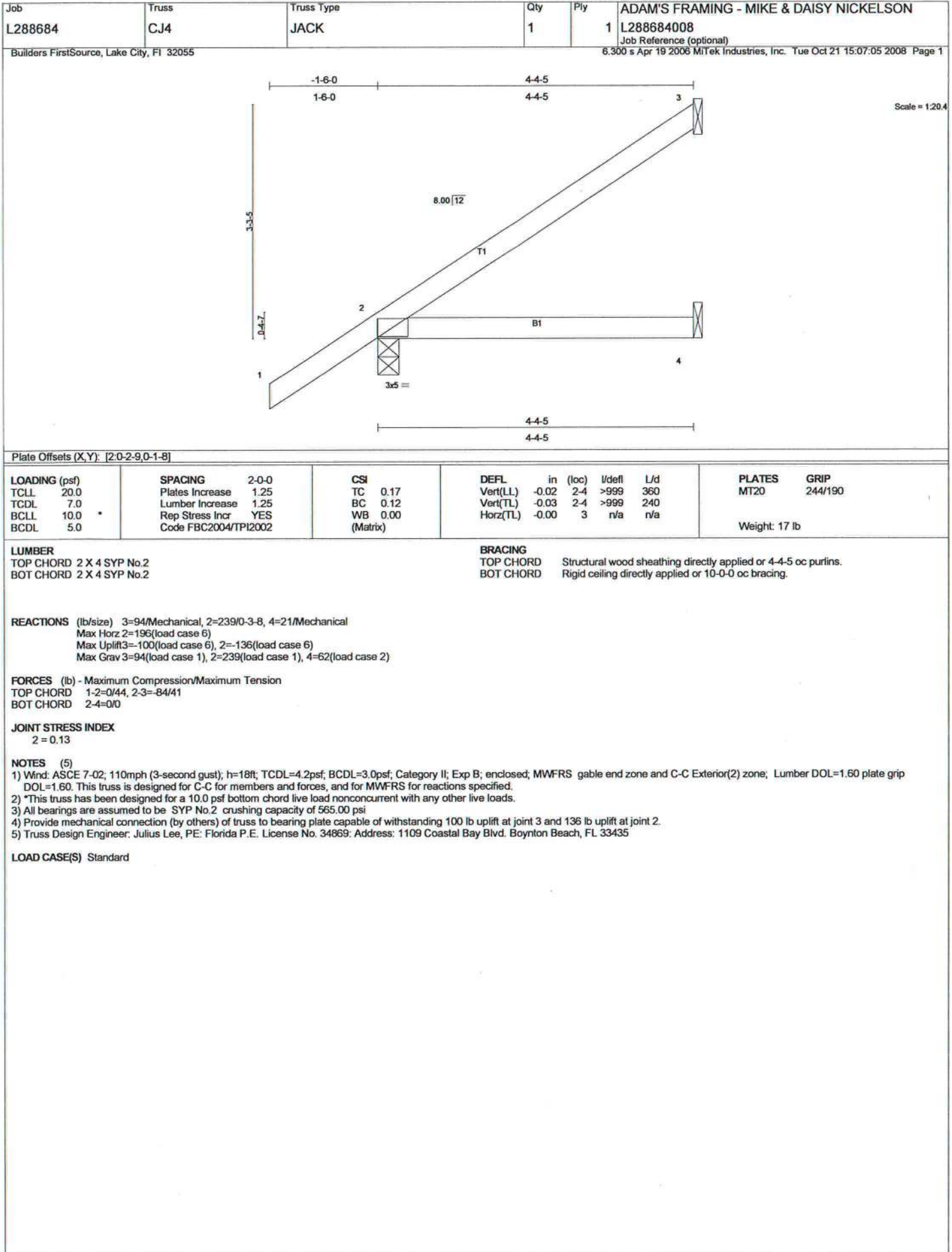
REACTIONS (lb/size) 3=67/Mechanical, 2=217/0-3-8, 4=17/Mechanical
 Max Horz 2=189(load case 6)
 Max Uplift 3=84(load case 6), 2=-124(load case 6)
 Max Grav 3=67(load case 1), 2=217(load case 1), 4=50(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-77/32
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX
 2 = 0.24

NOTES (5)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 124 lb uplift at joint 2.
 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job L288684	Truss CJ5A	Truss Type JACK	Qty 2	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684009 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:07:11 2008 Page 1		

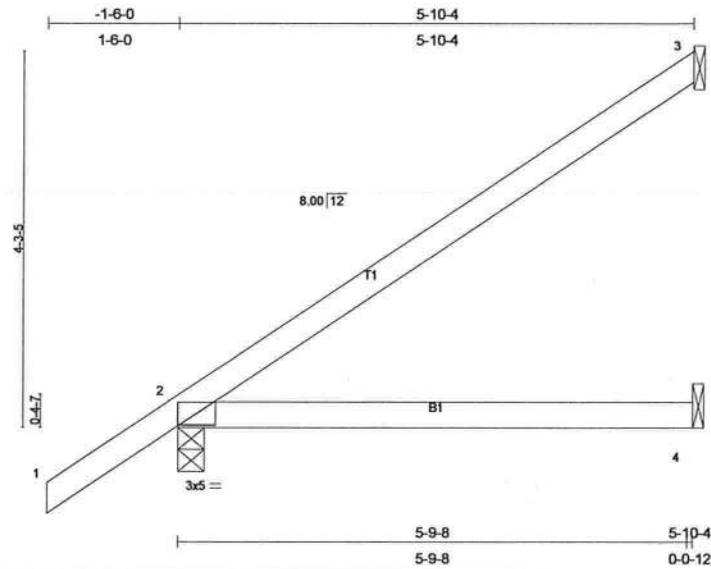


Plate Offsets (X,Y): [2.0-2.9,0-1.8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL)	-0.05	2-4	>999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.23	Vert(TL)	-0.09	2-4	>725		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TPI2002							
							Weight: 22 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-10-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=140/Mechanical, 2=283/0-3-8, 4=28/Mechanical
Max Horz 2=242(load case 6)
Max Uplift 3=150(load case 6), 2=138(load case 6)
Max Grav 3=140(load case 1), 2=283(load case 1), 4=85(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/44, 2-3=121/62
BOT CHORD 2-4=0/0

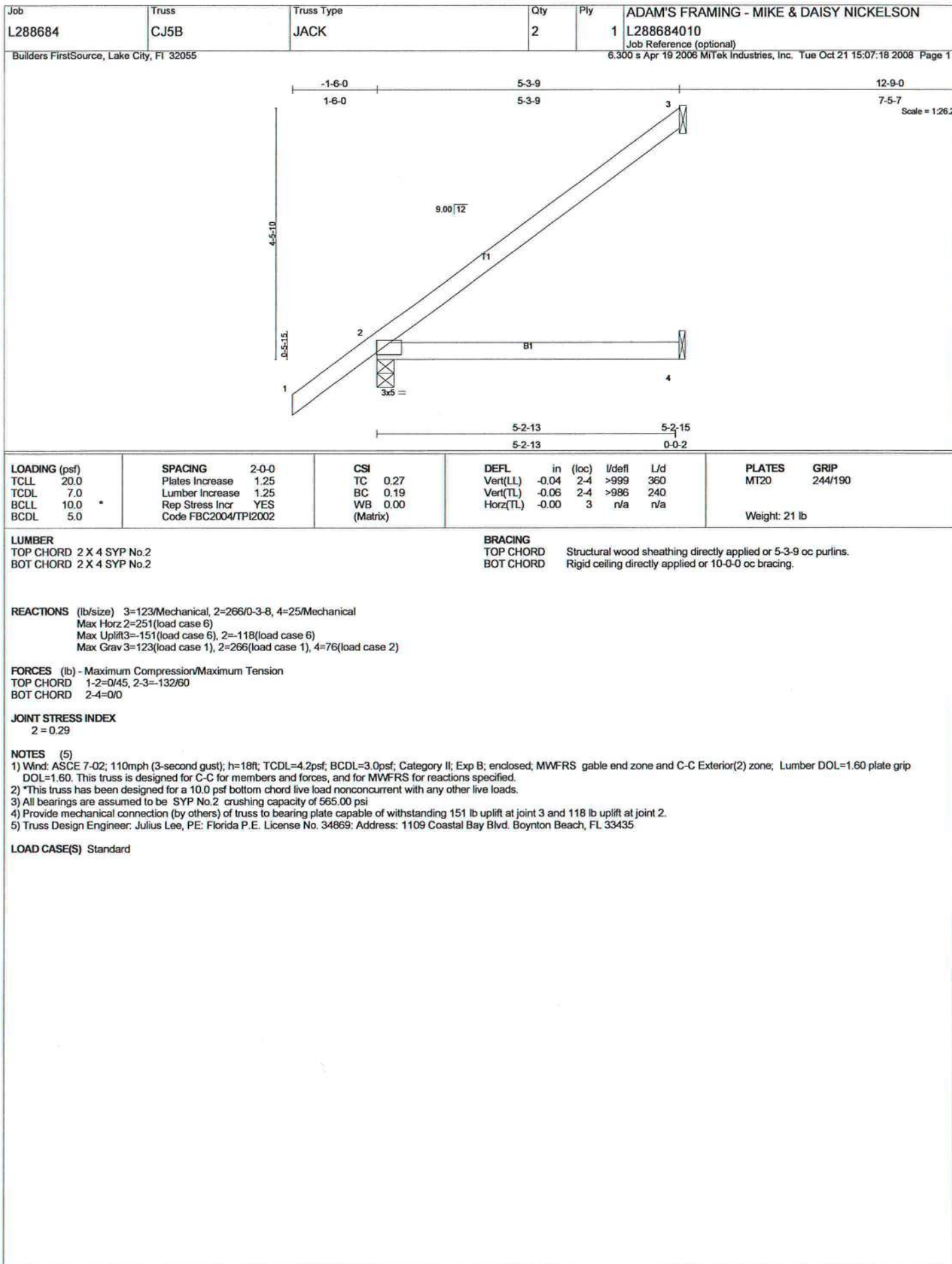
JOINT STRESS INDEX

2 = 0.15

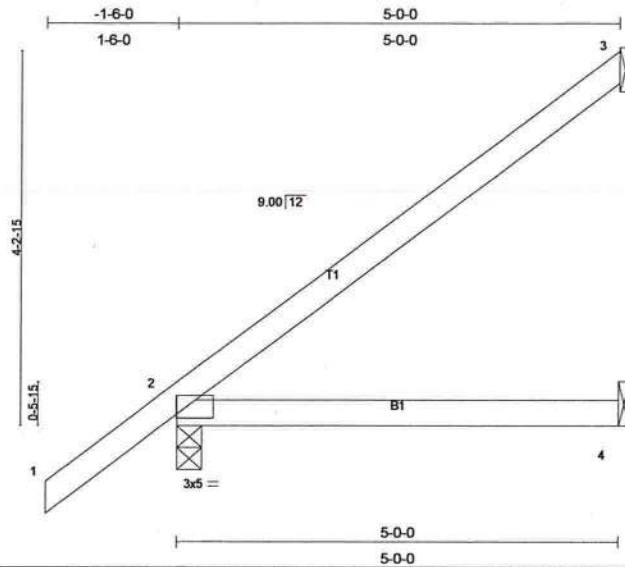
NOTES (5)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 3 and 138 lb uplift at joint 2.
- 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job L288684	Truss EJ5	Truss Type JACK	Qty 5	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684013 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:07:37 2008 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.23	Vert(LL) -0.03 2-4 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) -0.05 2-4 >999 240		
BCLL 10.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=257/0-3-8, 4=24/Mechanical
Max Horz 2=240(load case 6)
Max Uplift 3=140(load case 6), 2=118(load case 6)
Max Grav 3=114(load case 1), 2=257(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-123/55
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.28

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 3 and 118 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L288684	Truss EJ7	Truss Type MONO TRUSS	Qty 9	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684014 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2008 MiTek Industries, Inc. Tue Oct 21 15:07:43 2008 Page 1		

Scale = 1/32.3
Camber = 1/16 in

Plate Offsets (X,Y): [2-0-1-1,0-1-0]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl l/d
TCLL 20.0	Plates Increase	1.25	TC 0.43	Vert(LL) 0.11	2-4 >735 360
TCDL 7.0	Lumber Increase	1.25	BC 0.29	Vert(TL) -0.17	2-4 >468 240
BCLL 10.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL) -0.00	3 n/a n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)		
			PLATES GRIP MT20 244/190		
			Weight: 27 lb		

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS (lb/size) 3=157/Mechanical, 2=317/0-3-8, 4=50/Mechanical
 Max Horz 2=221(load case 6)
 Max Uplift 3=-120(load case 6), 2=-73(load case 6)
 Max Grav 3=157(load case 1), 2=317(load case 1), 4=96(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=-159/74
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX
 2 = 0.88

NOTES (5)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 3 and 73 lb uplift at joint 2.
 5) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L288684	Truss EJ11	Truss Type ROOF TRUSS	Qty 3	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684011 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:07:25 2008 Page 1		

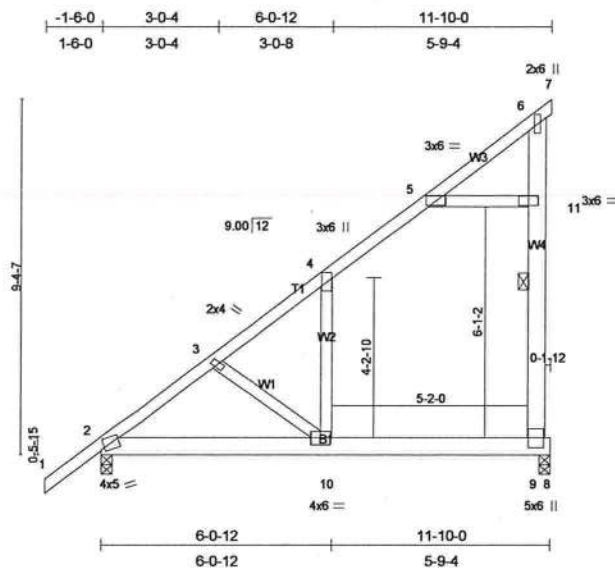


Plate Offsets (X,Y): [2,0-1-2,0-0-11]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.67	Vert(LL)	-0.18	10	>758	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.60	Vert(TL)	-0.34	10	>398		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.08	Horz(TL)	0.00	9	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TPI2002							
							Weight: 88 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-9

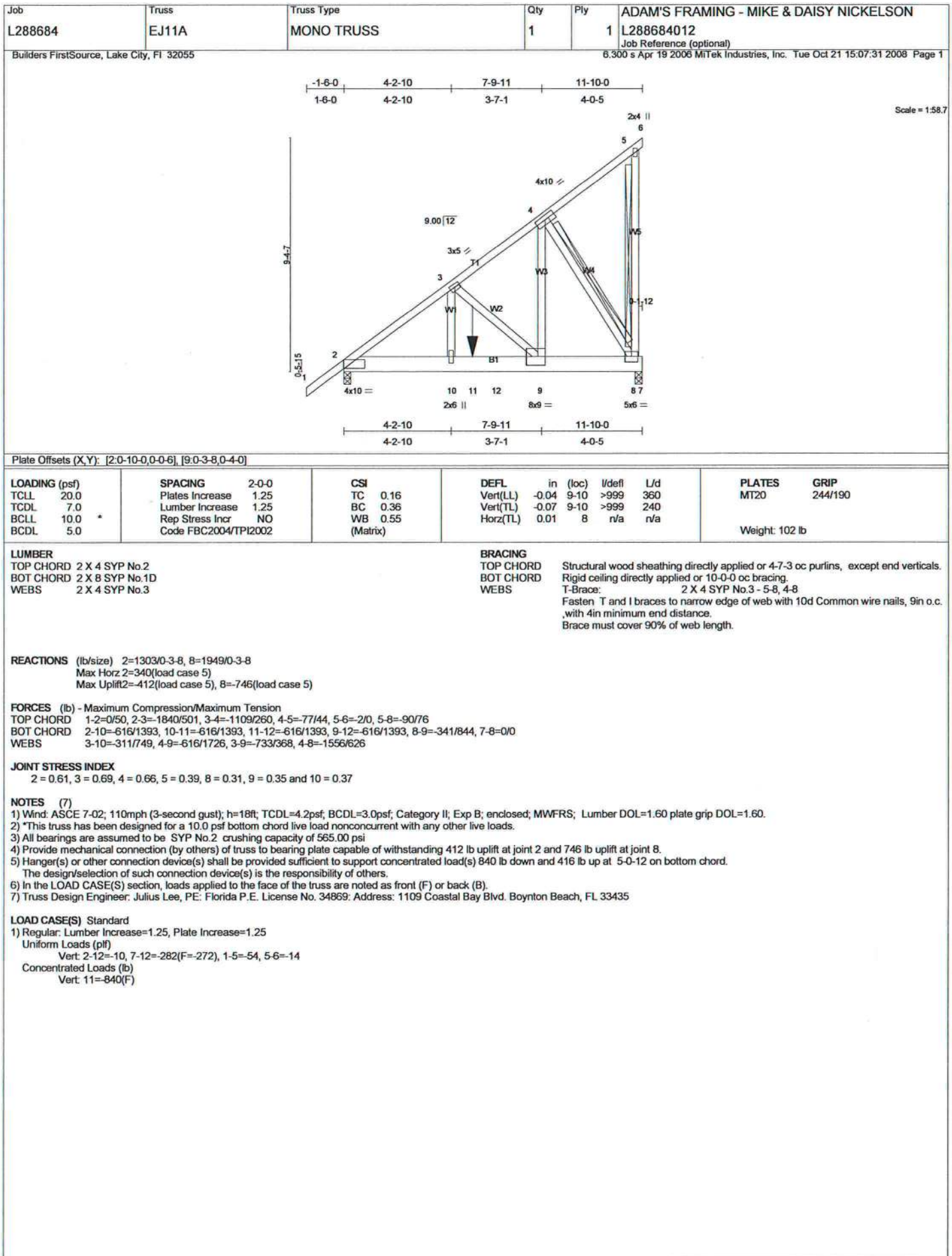
REACTIONS (lb/size) 9=863/0-3-8, 2=628/0-3-8
Max Horz 2=338(load case 6)
Max Uplift 9=-111(load case 6), 2=-18(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-602/0, 3-4=-435/0, 4-5=-292/0, 5-6=-338/432, 6-7=-12/0, 9-11=-287/297, 6-11=-272/306
BOT CHORD 2-10=-380/468, 9-10=-234/243, 8-9=0/0
WEBS 4-10=0/168, 5-11=-594/93, 3-10=-281/182

JOINT STRESS INDEX
2 = 0.75, 3 = 0.11, 4 = 0.08, 5 = 0.21, 6 = 0.77, 9 = 0.98, 10 = 0.09 and 11 = 0.19

NOTES (7)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed, MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-11; Wall dead load (5.0psf) on member(s). 4-10
4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 9-10
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 9 and 18 lb uplift at joint 2.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job L288684	Truss HJ7	Truss Type JACK	Qty 1	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684016 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:07:54 2008 Page 1		

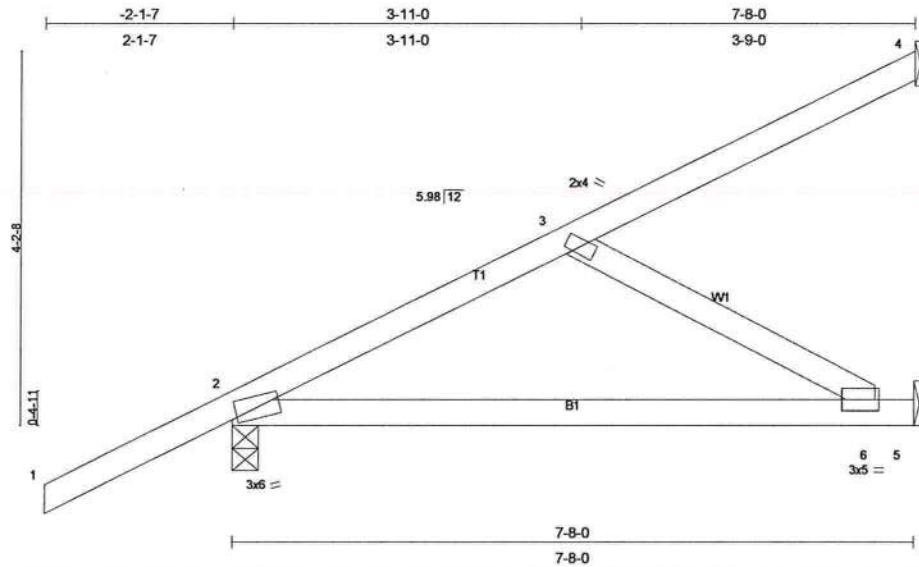


Plate Offsets (X,Y): [2.0-0-12.0-0-9]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.30	Vert(LL)	-0.10	2-6	>858	360	MT20
TCDL 7.0	Plates Increase 1.25	BC 0.34	Vert(TL)	-0.18	2-6	>510	240	244/190
BCLL 10.0	Lumber Increase 1.25	WB 0.07	Horz(TL)	0.00	5	n/a	n/a	
BCDL 5.0	Rep Stress Incr NO	(Matrix)						
	Code FBC2004/TP12002							Weight: 34 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=140/Mechanical, 2=292/0-3-8, 5=136/Mechanical

Max Horz 2=223(load case 5)
Max Uplift 4=145(load case 5), 2=152(load case 5), 5=47(load case 5)
Max Grav 4=140(load case 1), 2=292(load case 1), 5=148(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-231/12, 3-4=-85/46
BOT CHORD 2-6=-152/193, 5-6=0/0
WEBS 3-6=-221/174

JOINT STRESS INDEX

2 = 0.93, 3 = 0.11 and 6 = 0.07

NOTES (6)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 4, 152 lb uplift at joint 2 and 47 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

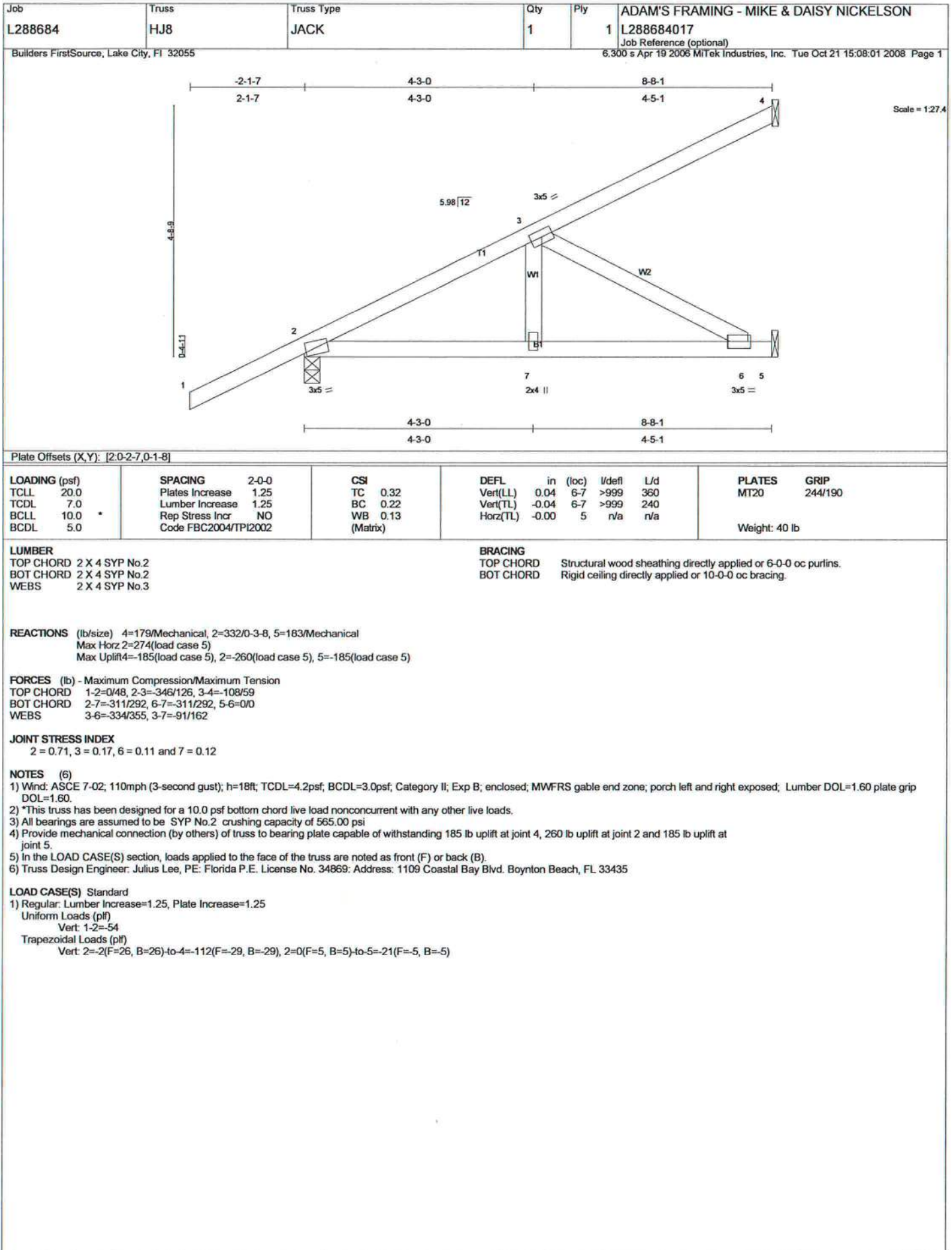
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-2(F=26, B=26)-to-4=-99(F=-23, B=-23), 2=0(F=5, B=5)-to-5=-18(F=-4, B=-4)



Job L288684	Truss HJ10	Truss Type JACK	Qty 2	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684015 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:07:48 2008 Page 1		

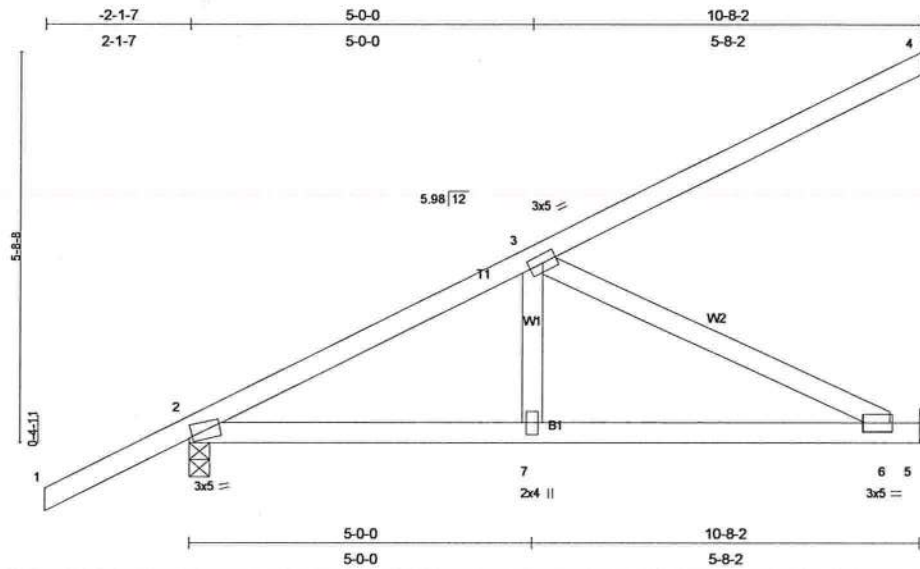


Plate Offsets (X,Y): [2-0-2-7,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.63	Vert(LL)	0.05	6-7	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.35	Vert(TL)	-0.12	6-7	>999	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.35	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 49 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-10-10 oc bracing.

REACTIONS (lb/size)

4=279/Mechanical, 2=429/0-3-8, 5=284/Mechanical
Max Horz 2=398(load case 5)
Max Uplift 4=294(load case 5), 2=-164(load case 5), 5=-138(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-581/38, 3-4=-171/91
BOT CHORD 2-7=-361/505, 6-7=-361/505, 5-6=0/0
WEBS 3-7=0/218, 3-6=-563/402

JOINT STRESS INDEX

2 = 0.75, 3 = 0.19, 6 = 0.18 and 7 = 0.16

NOTES (6)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 294 lb uplift at joint 4, 164 lb uplift at joint 2 and 138 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

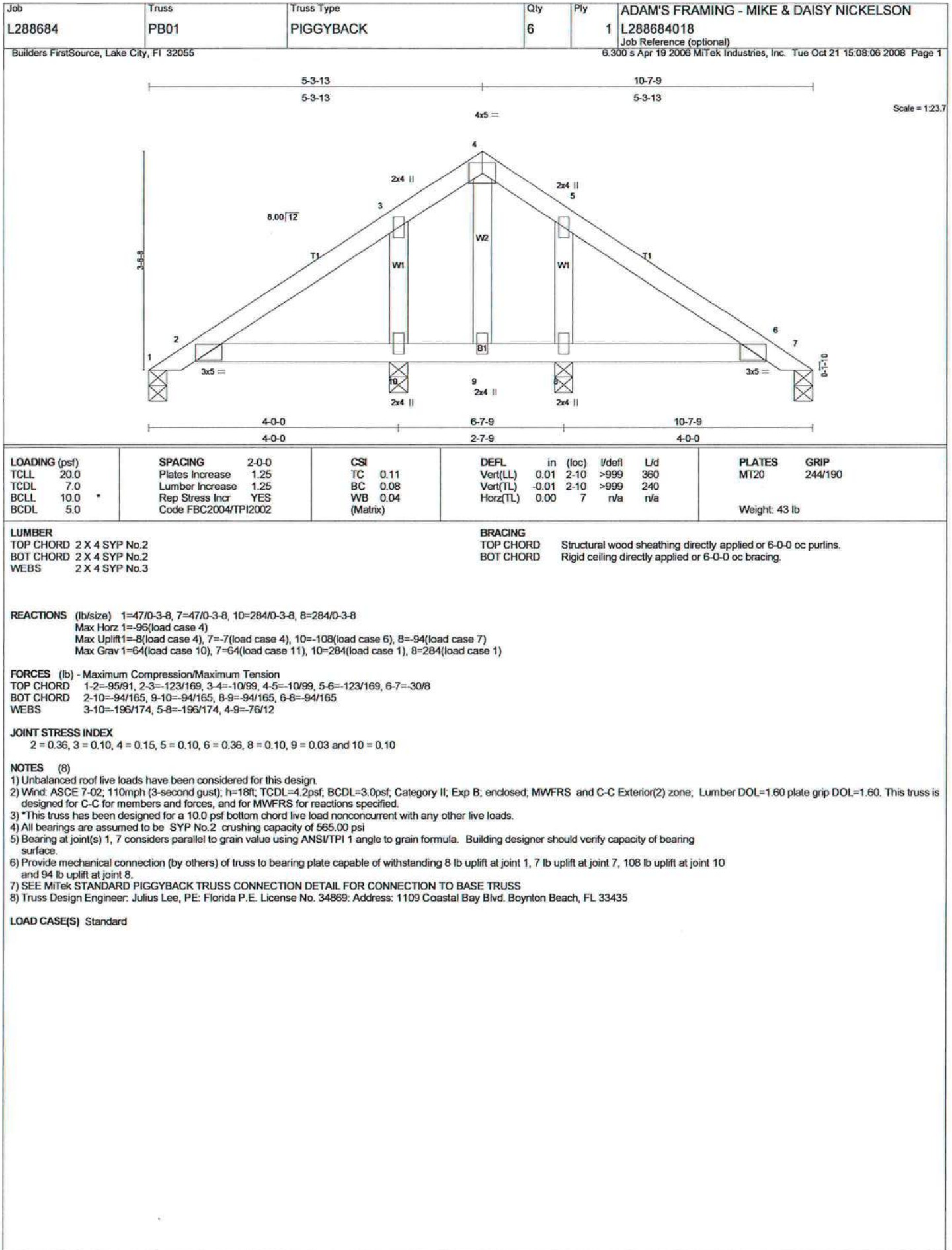
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

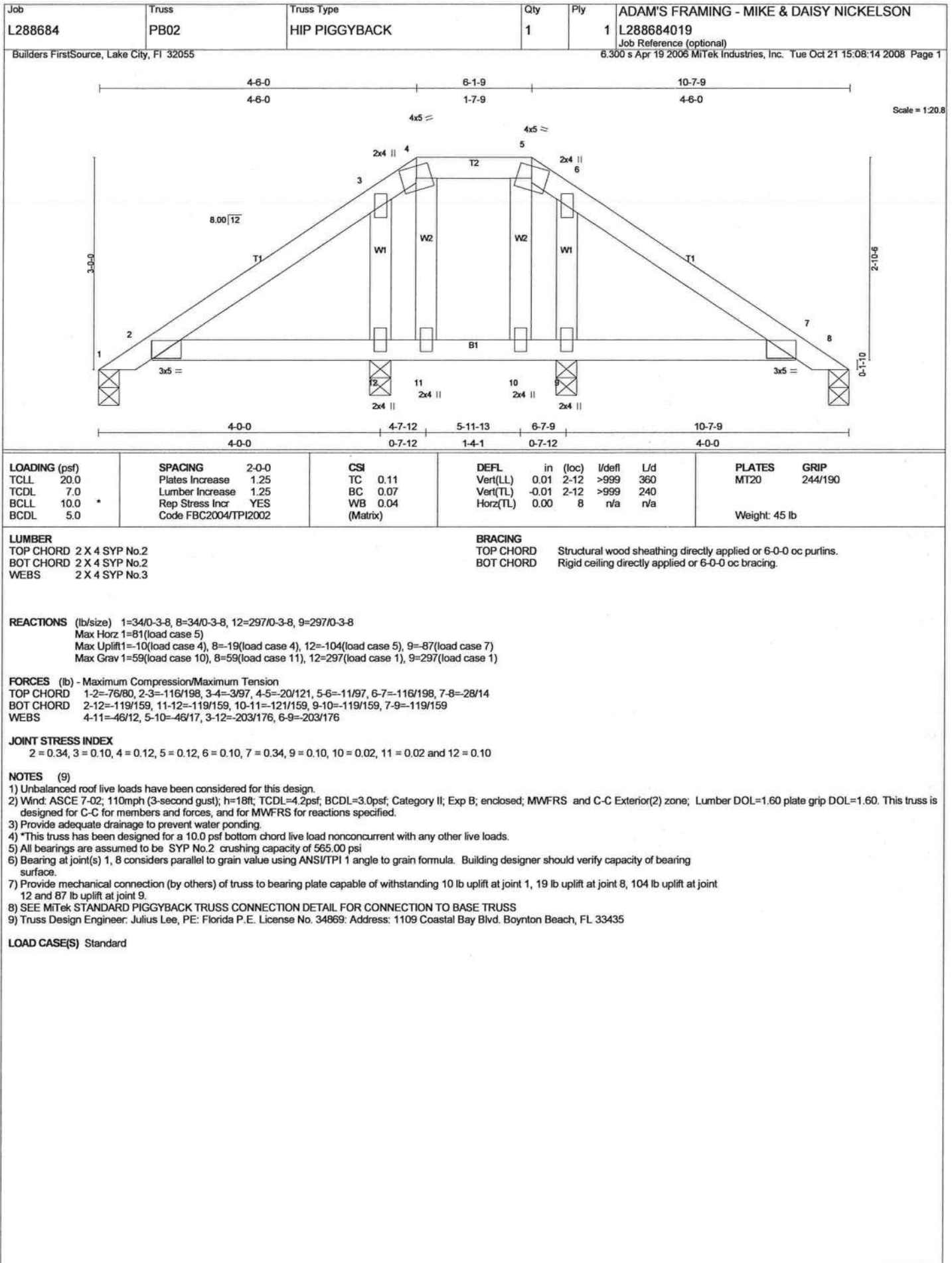
Uniform Loads (plf)

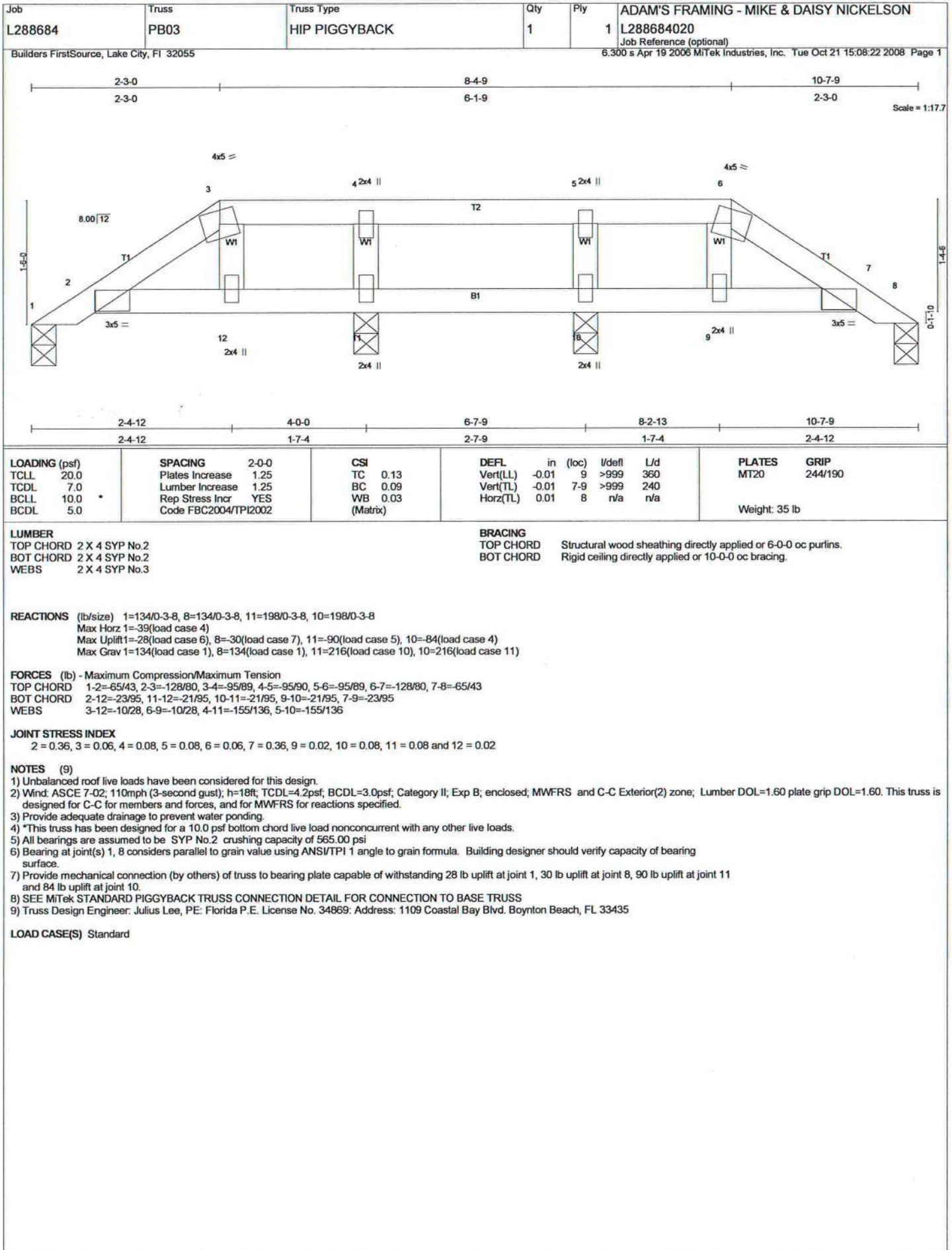
Vert: 1-2=-54

Trapezoidal Loads (plf)

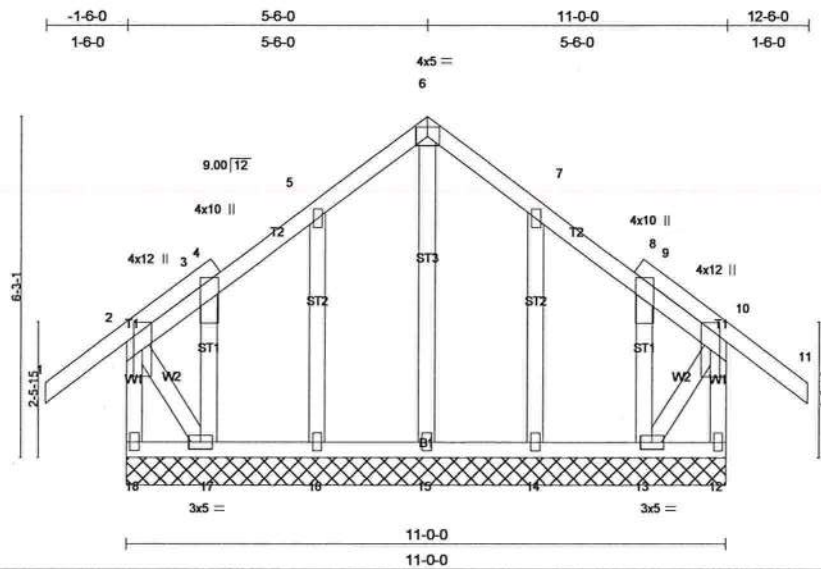
Vert: 2=-2(F=26, B=26)-to-4=-139(F=42, B=42), 2=0(F=5, B=5)-to-5=-26(F=8, B=8)







Job L288684	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684021 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:08:29 2008 Page 1		



Scale = 1:40.7

LOADING (psf)	SPACING 2'-0"	CSI	DEFL in (loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.17	Vert(LL) -0.01 11	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.04	Vert(TL) -0.02 11	n/r	90		
BCLL 10.0	Rep Stress Incr YES	WB 0.08	Horz(TL) 0.00 12	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)					
							Weight: 89 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6'-0" oc bracing.

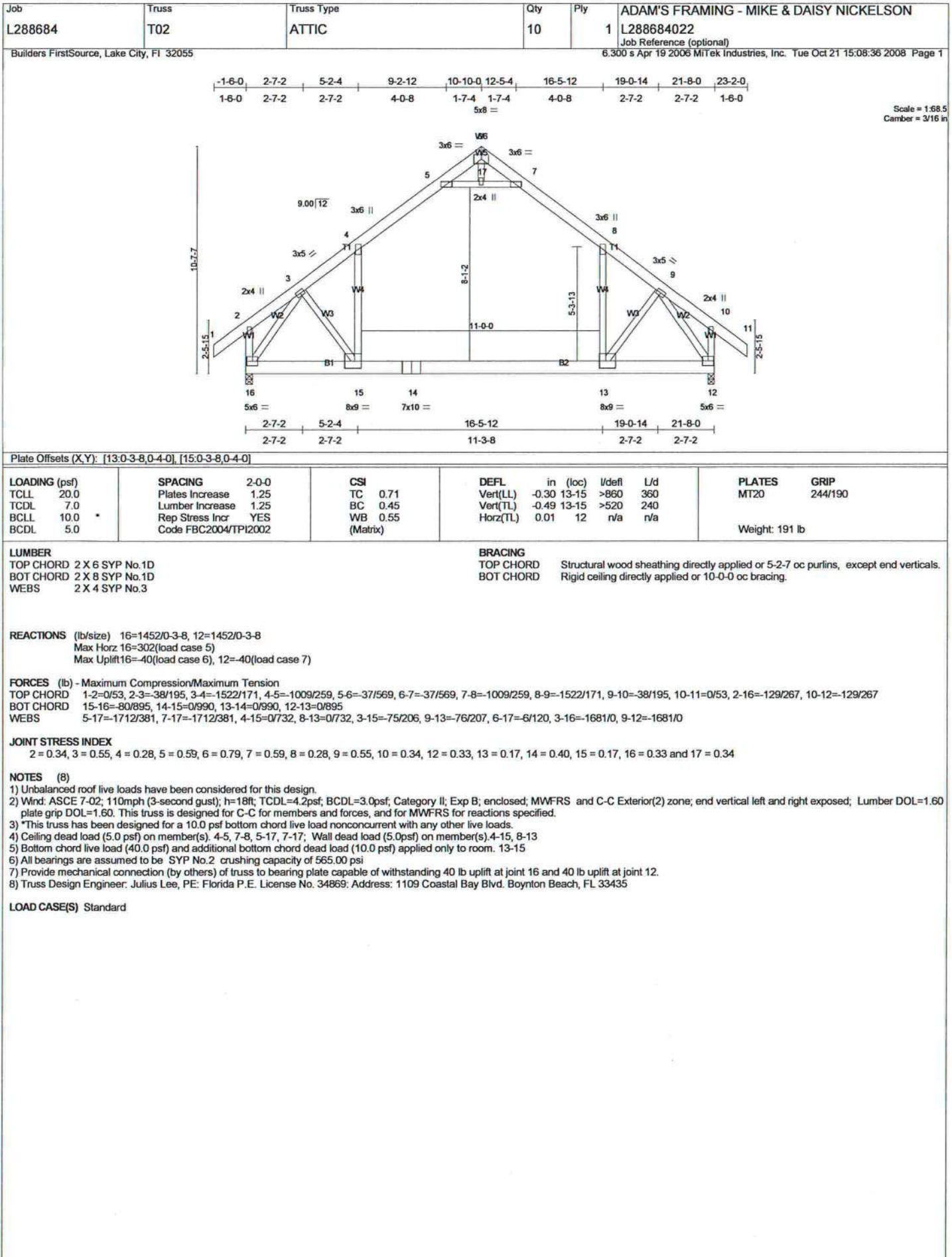
REACTIONS (lb/size) 18=141/11'-0", 12=141/11'-0", 15=146/11'-0", 16=129/11'-0", 17=88/11'-0", 14=129/11'-0", 13=88/11'-0"
Max Horz 18=244(load case 5)
Max Uplift 18=188(load case 4), 12=135(load case 5), 16=107(load case 6), 17=248(load case 5), 14=107(load case 7), 13=227(load case 7)
Max Grav 18=212(load case 5), 12=159(load case 4), 15=146(load case 1), 16=133(load case 10), 17=165(load case 4), 14=133(load case 11), 13=130(load case 5)

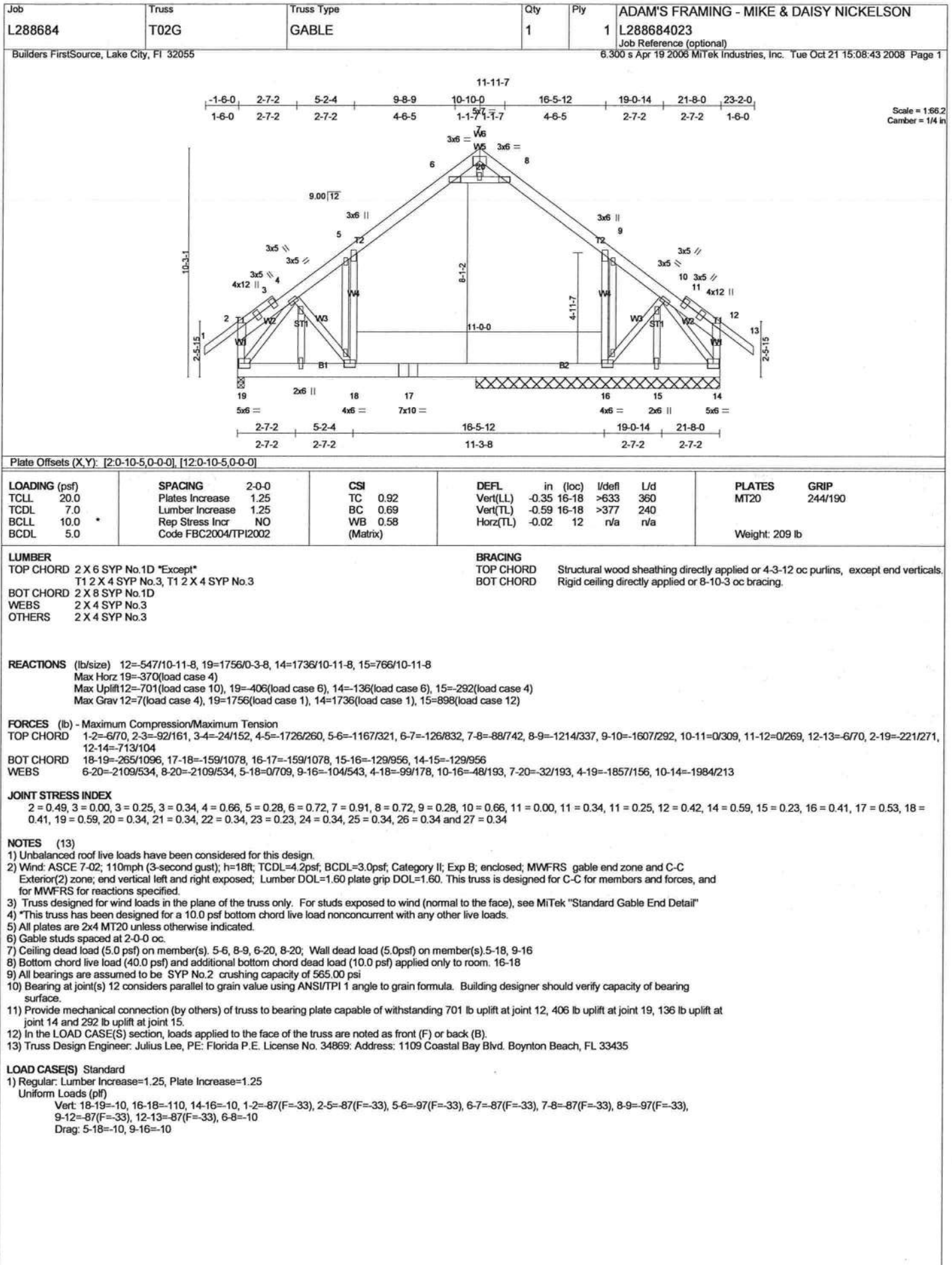
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-79/93, 3-4=-55/74, 4-5=-46/90, 5-6=-25/169, 6-7=-25/169, 7-8=-16/87, 8-9=-28/53, 9-10=-49/64, 10-11=0/47, 2-18=-193/181, 10-12=-149/130
BOT CHORD 17-18=-208/222, 16-17=-75/170, 15-16=-75/170, 14-15=-75/170, 13-14=-75/170, 12-13=-20/58
WEBS 6-15=-126/0, 5-16=-113/122, 3-17=-59/57, 7-14=-113/122, 9-13=-59/57, 2-17=-139/238, 10-13=-99/208

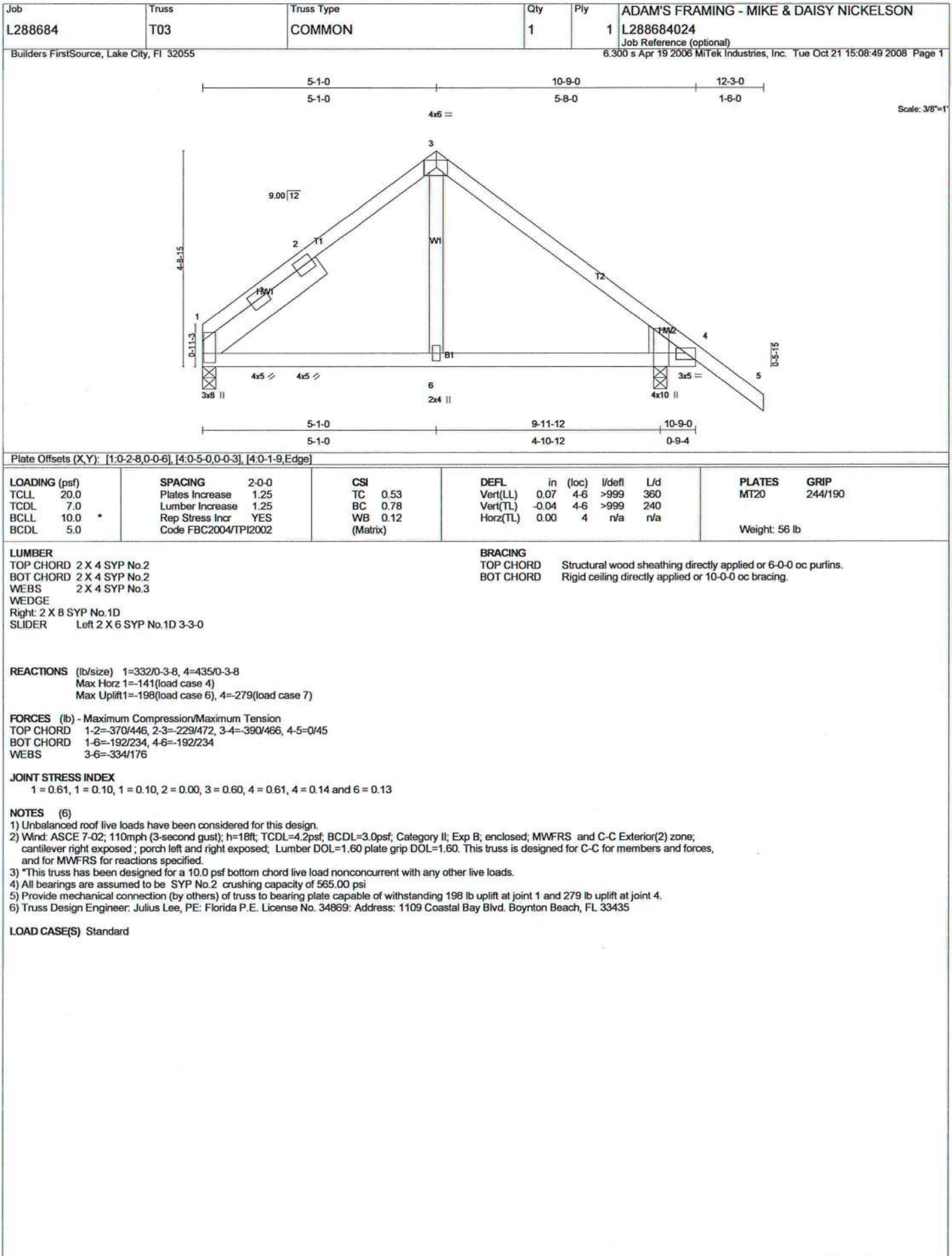
JOINT STRESS INDEX
2 = 0.90, 3 = 0.21, 4 = 0.00, 5 = 0.06, 6 = 0.15, 7 = 0.06, 8 = 0.00, 9 = 0.21, 10 = 0.90, 12 = 0.21, 13 = 0.15, 14 = 0.07, 15 = 0.05, 16 = 0.07, 17 = 0.15 and 18 = 0.21

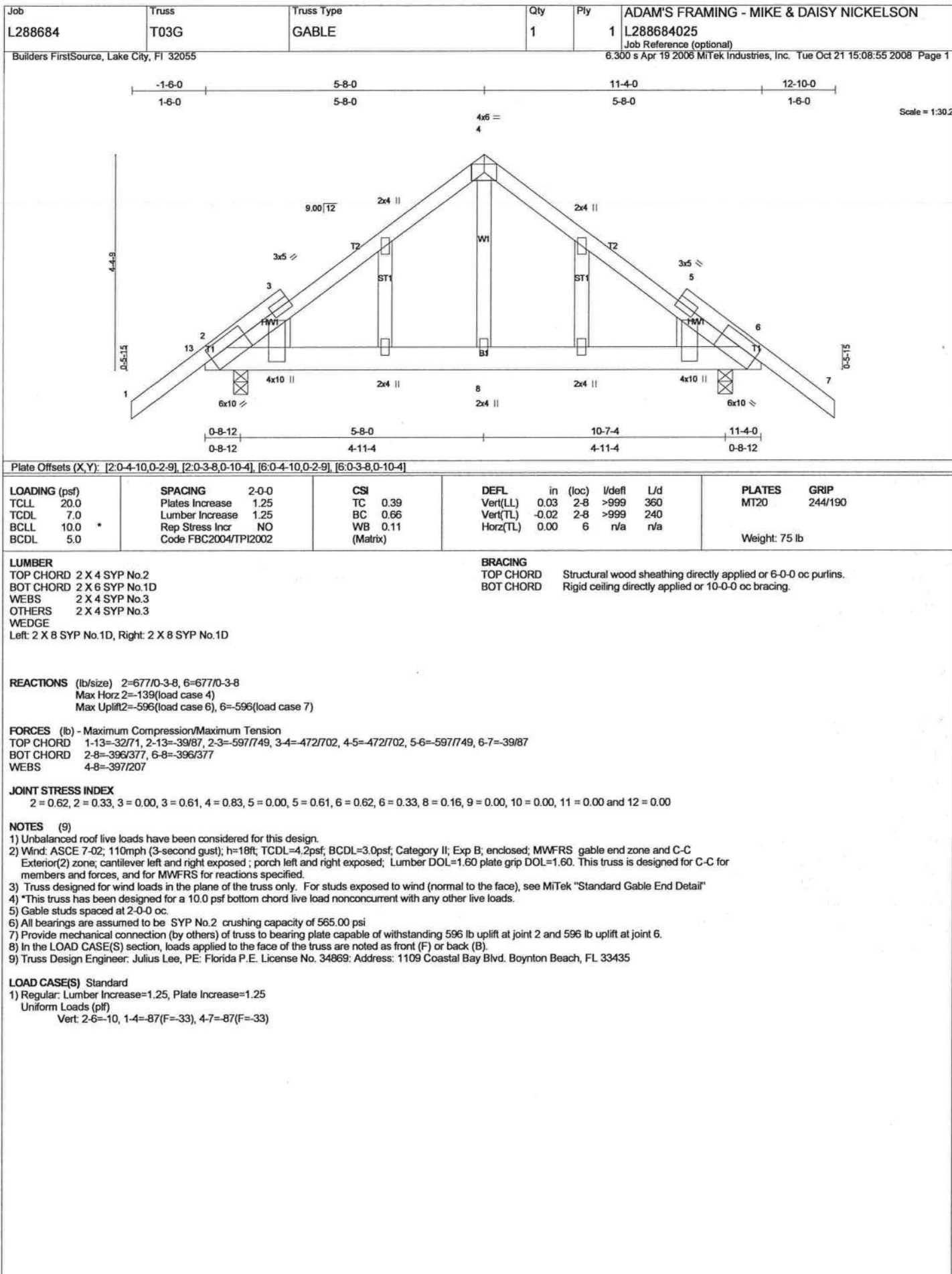
NOTES (11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
8) Gable studs spaced at 2'-0" oc.
9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 18, 135 lb uplift at joint 12, 107 lb uplift at joint 16, 248 lb uplift at joint 17, 107 lb uplift at joint 14 and 227 lb uplift at joint 13.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

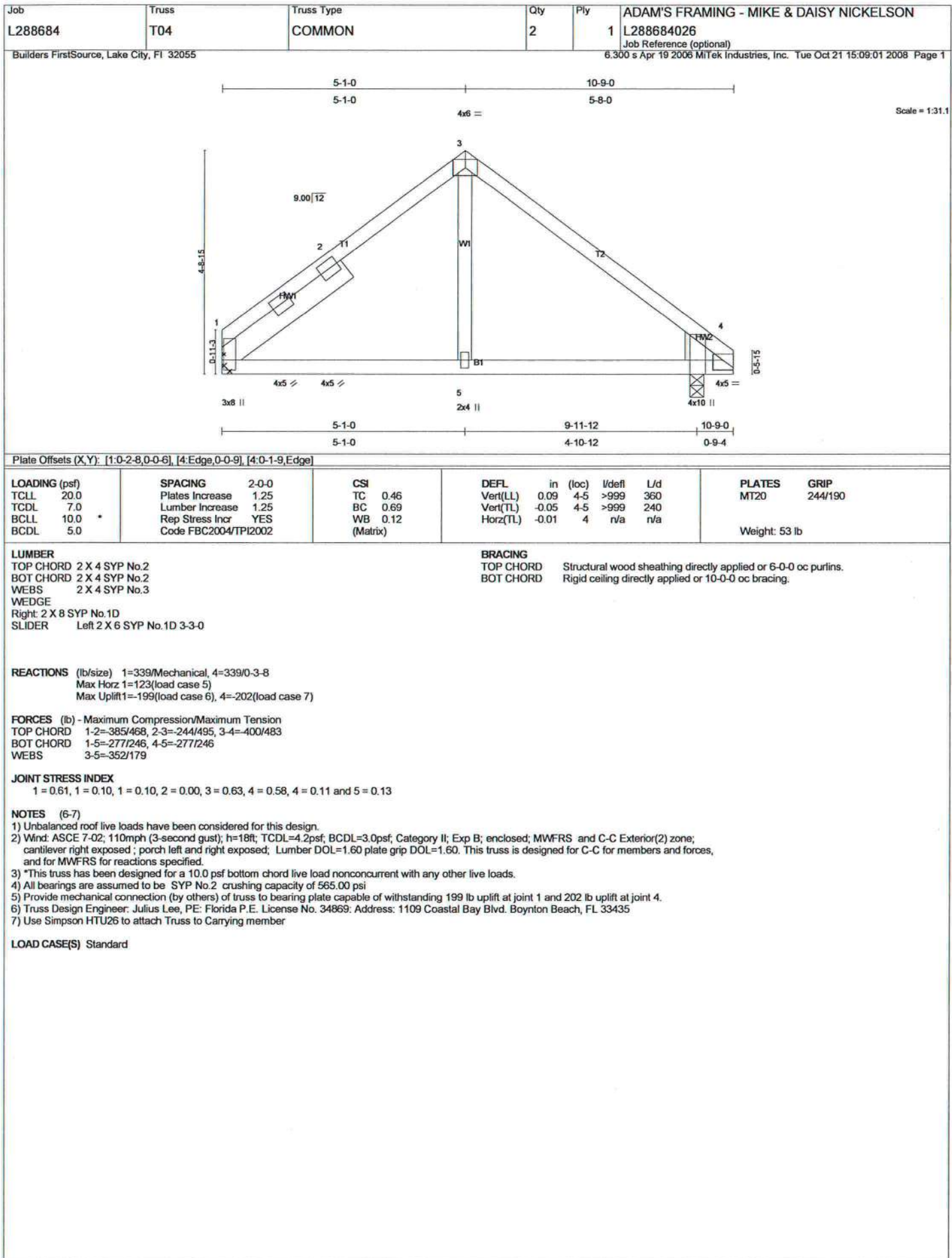
LOAD CASE(S) Standard

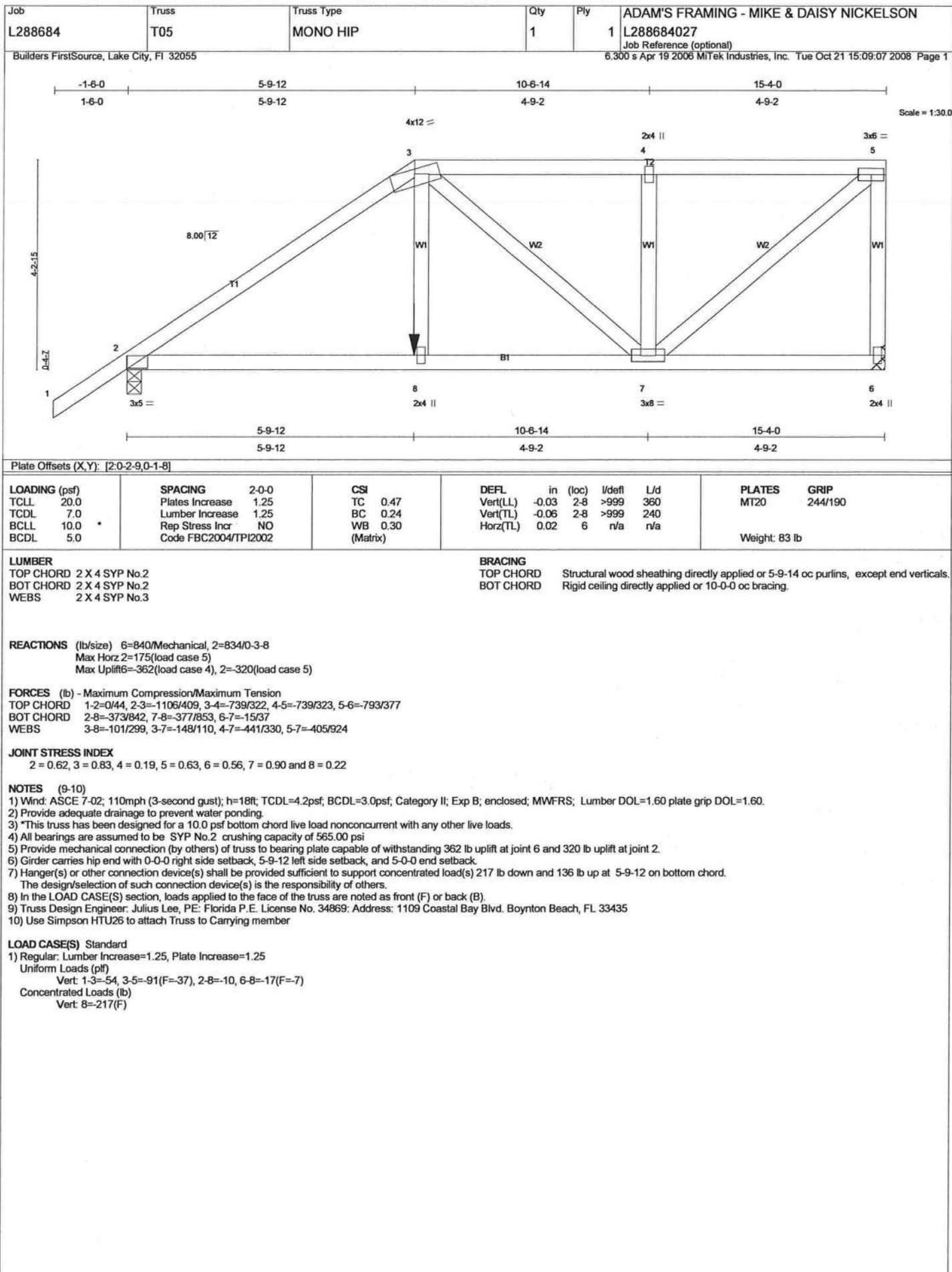


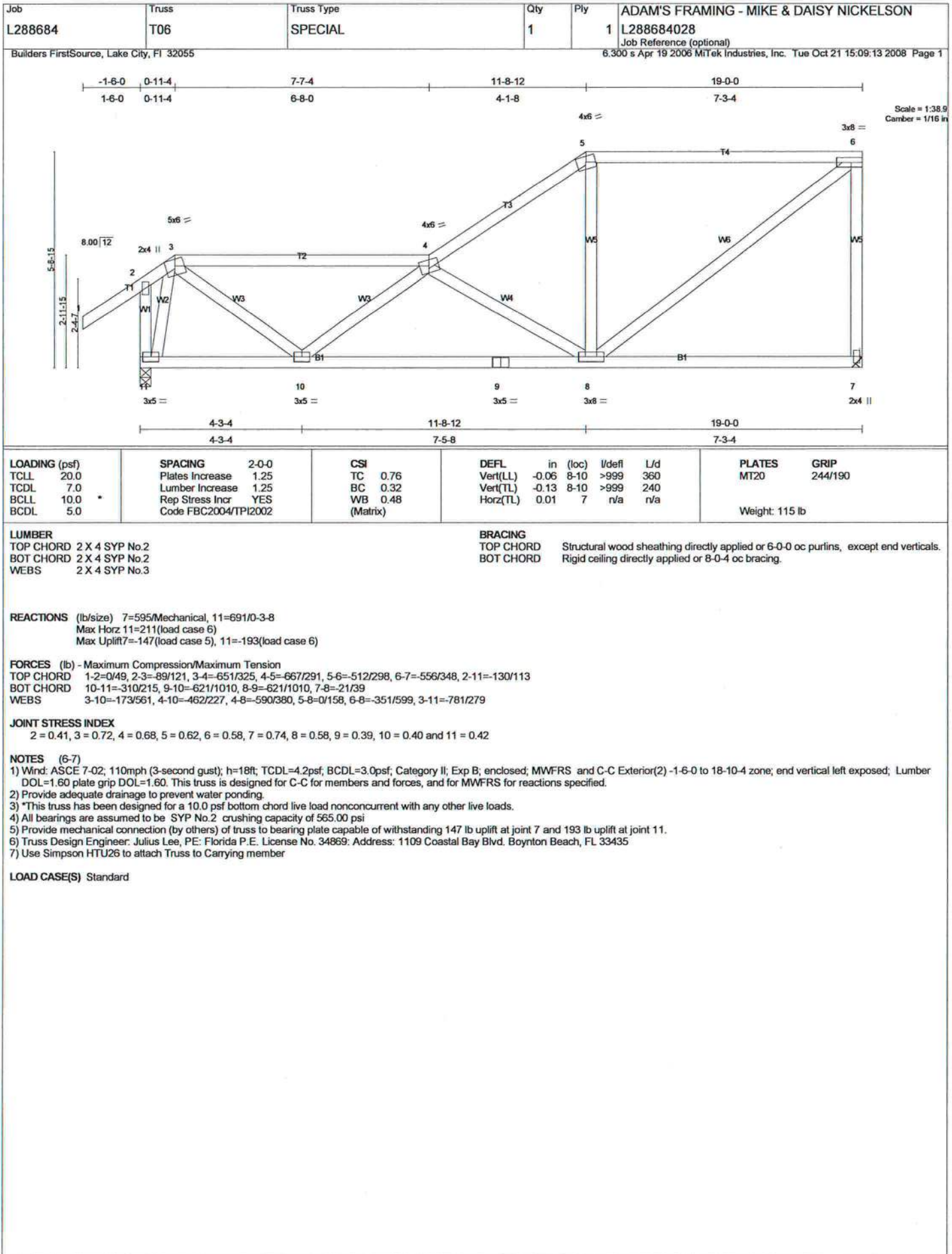




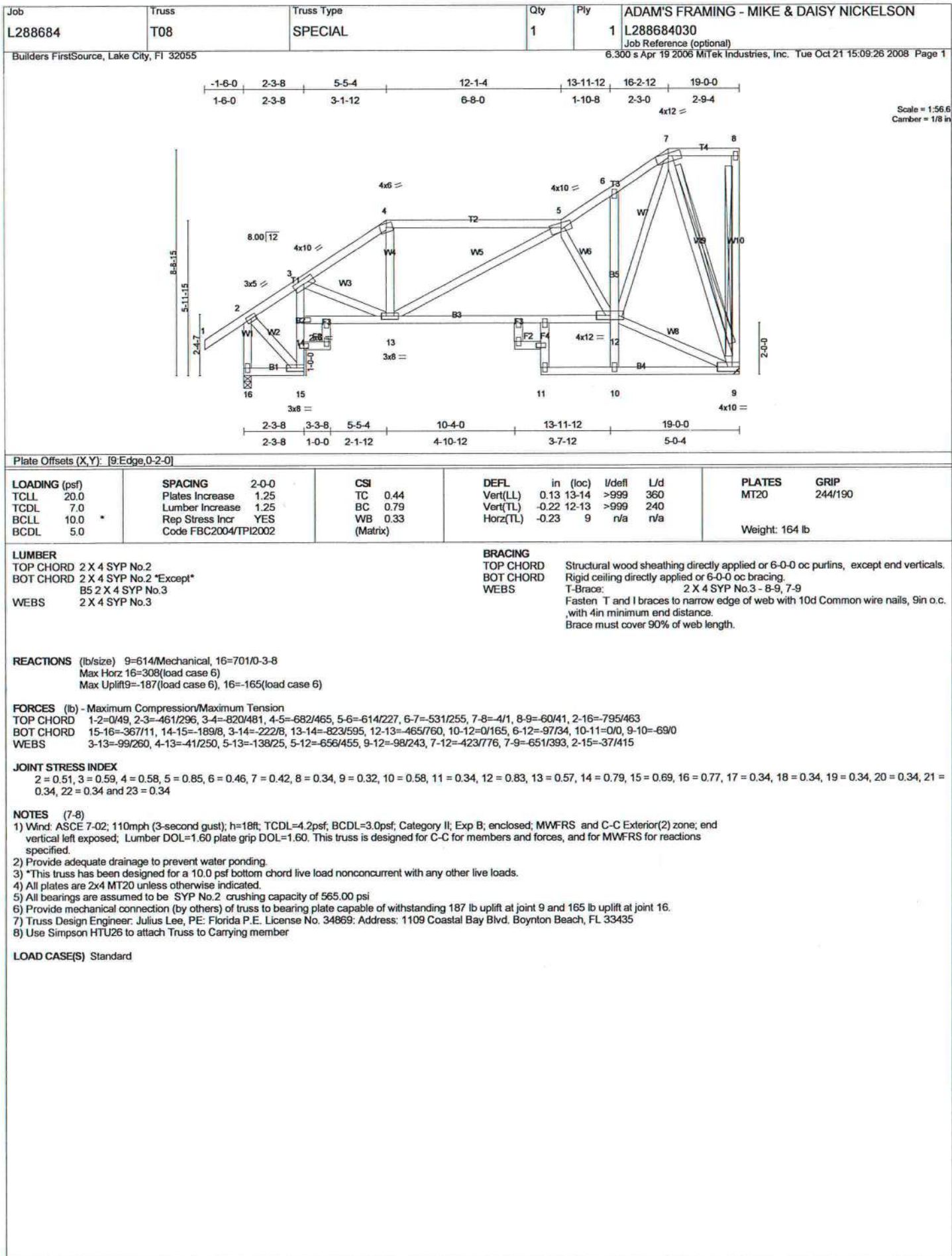


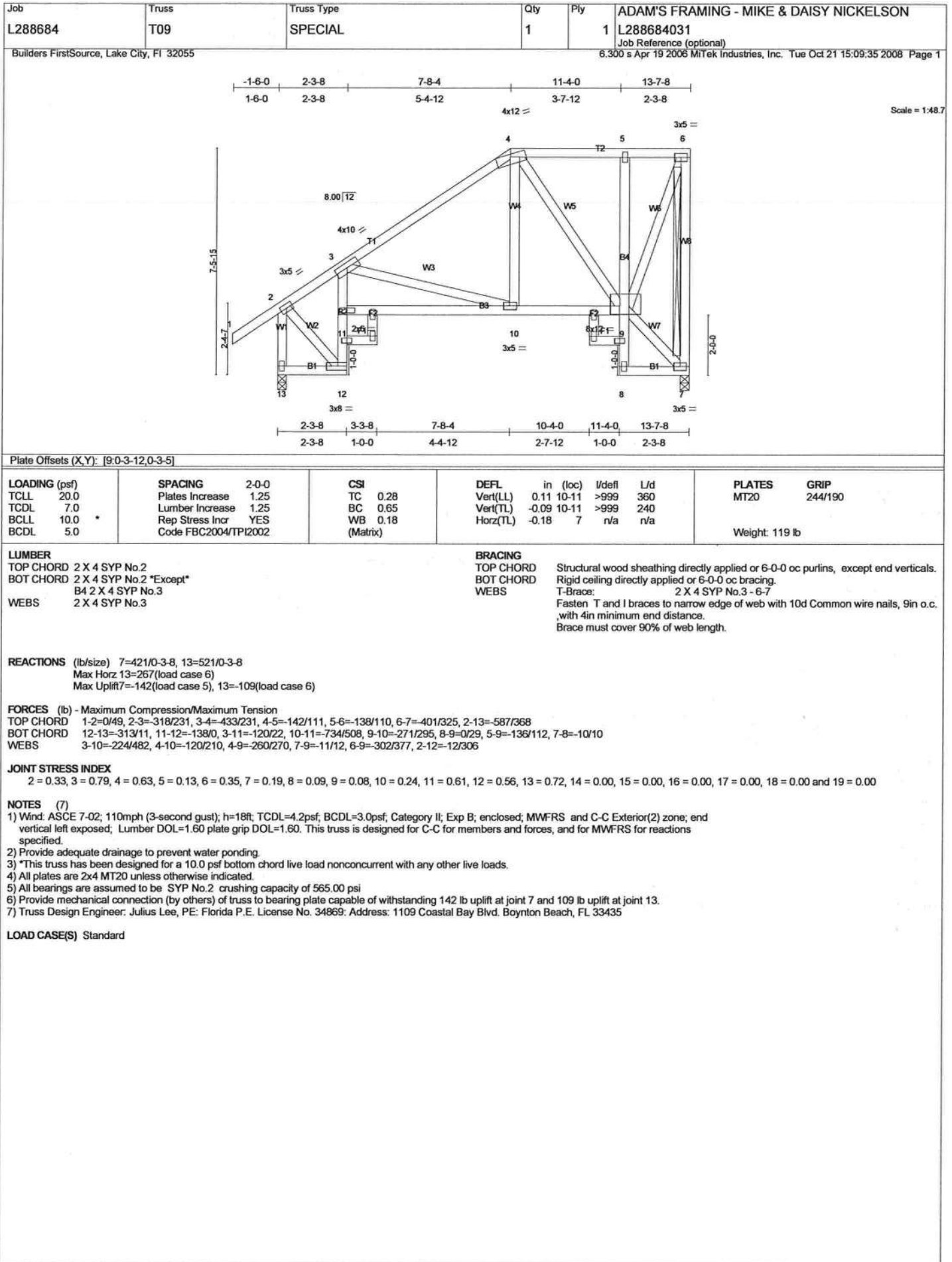


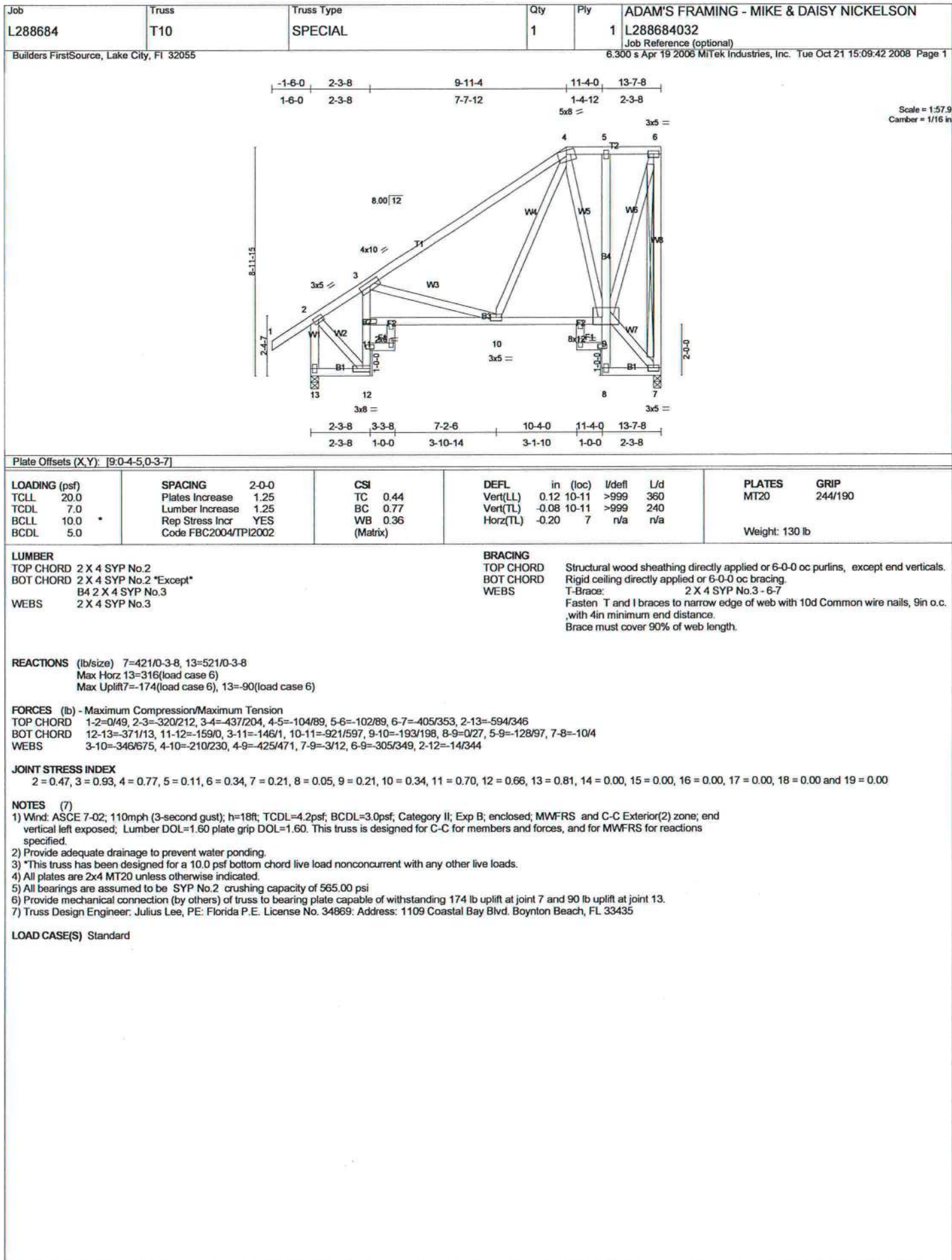


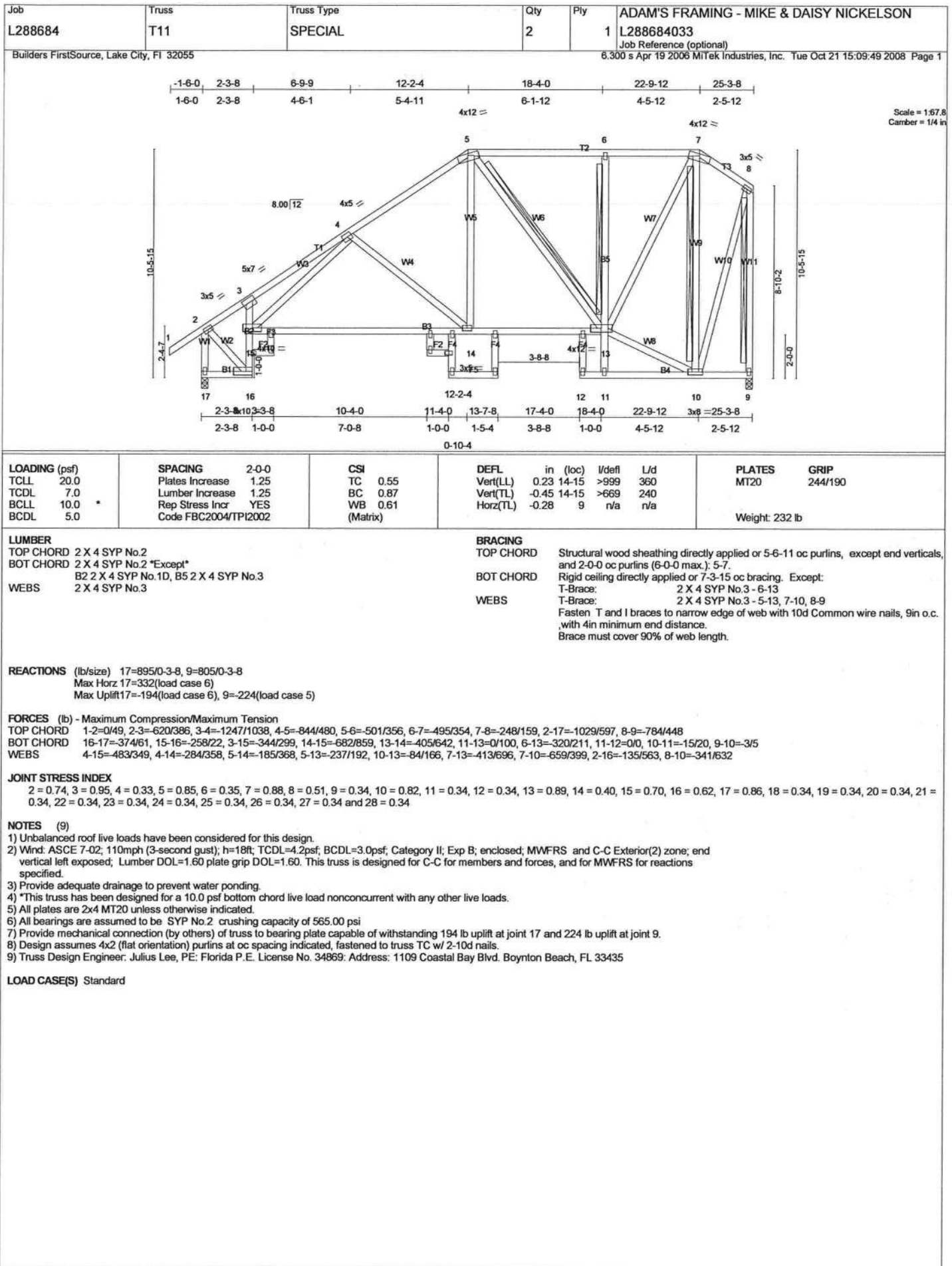


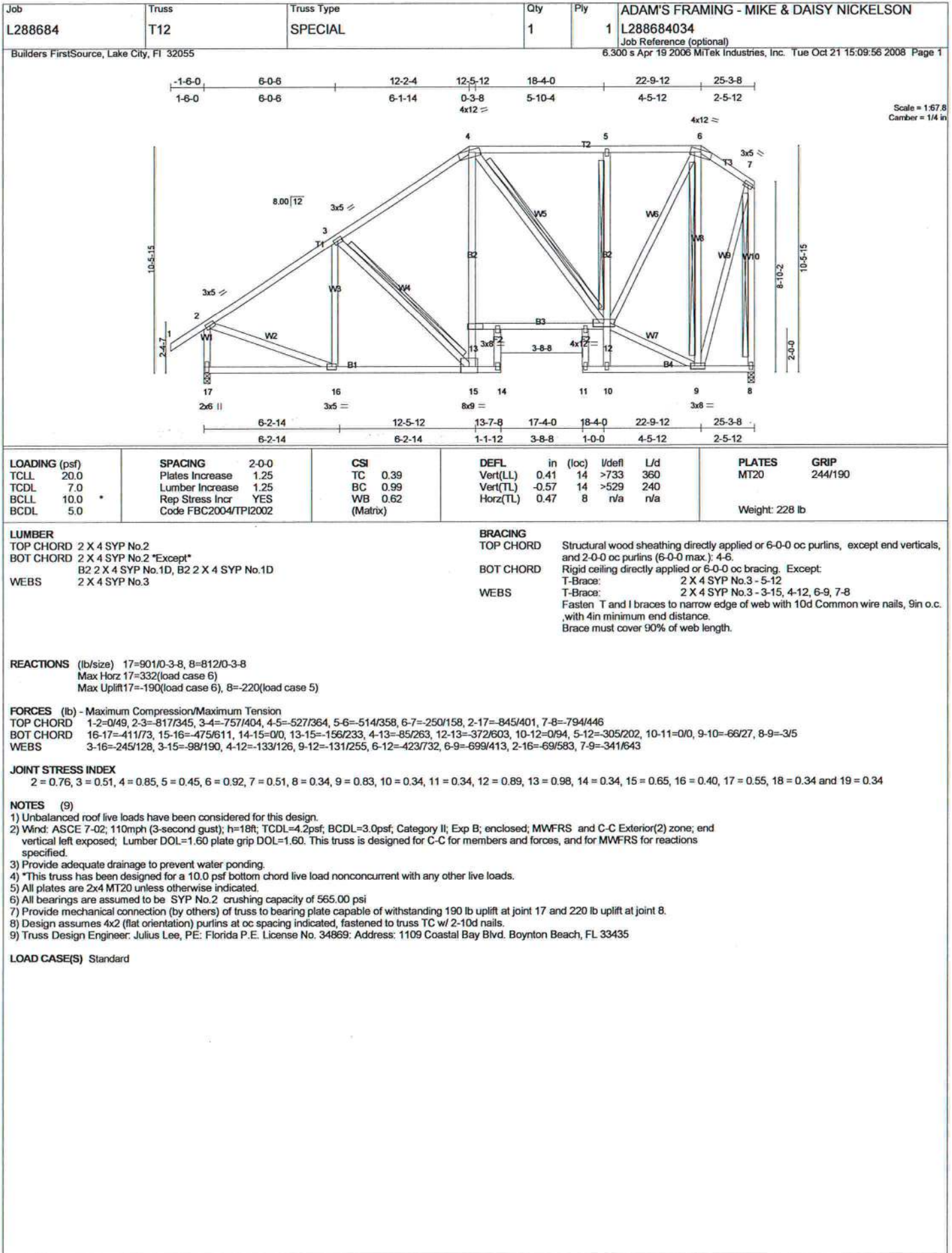
Job L288684	Truss T07	Truss Type SPECIAL	Qty 1	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684029 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:09:20 2008 Page 1					
Builders FirstSource, Lake City, FL 32055										
Plate Offsets (X,Y): [6-0-2,8,0-1-13]										
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TP12002	CSI TC 0.43 BC 0.72 WB 0.42 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) 0.14 12-13 >999 360 Vert(TL) -0.20 11-12 >999 240 Horz(TL) -0.22 8 n/a n/a	PLATES MT20	GRIP 244/190 Weight: 143 lb					
LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except* B5 2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 5-8-5 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. WEBS T-Brace: 2 X 4 SYP No.3 - 7-8 Fasten T and l braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.									
REACTIONS (lb/size) 8=614/Mechanical, 15=701/0-3-8 Max Horz 15=259(load case 6) Max Uplift 8=-146(load case 6), 15=-180(load case 6)										
FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/49, 2-3=-460/306, 3-4=-568/551, 4-5=-1099/633, 5-6=-629/247, 6-7=-472/258, 7-8=-602/347, 2-15=-793/473 BOT CHORD 14-15=-306/13, 13-14=-179/29, 3-13=-164/150, 12-13=-706/743, 11-12=-822/1344, 9-11=0/164, 6-11=0/199, 9-10=0/0, 8-9=-65/0 WEBS 4-13=-373/223, 4-12=-58/467, 5-12=-290/82, 5-11=-985/638, 8-11=0/78, 7-11=-366/667, 2-14=-43/399										
JOINT STRESS INDEX 2 = 0.52, 3 = 0.75, 4 = 0.37, 5 = 0.80, 6 = 0.70, 7 = 0.47, 8 = 0.40, 9 = 0.59, 10 = 0.34, 11 = 0.81, 12 = 0.40, 13 = 0.65, 14 = 0.62, 15 = 0.69, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34 and 22 = 0.34										
NOTES (7-8) 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 2) Provide adequate drainage to prevent water ponding. 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) All plates are 2x4 MT20 unless otherwise indicated. 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 8 and 180 lb uplift at joint 15. 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435 8) Use Simpson HTU26 to attach Truss to Carrying member										
LOAD CASE(S) Standard										





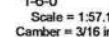






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Weight: 172 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-1-10 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 4-11-12 oc bracing.

Max Horiz 2=149(load case 3)
Max Uplift2=906(load case 4), 9=906(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=3341/1509, 3-4=3193/1518, 4-5=2662/1320, 5-6=3316/1582, 6-7=2662/1320, 7-8=3193/1518, 8-9=3341/1510, 9-10=0/44
BOT CHORD 2-13=1318/2675, 12-13=1613/3273, 11-12=1573/3273, 9-11=1169/2675
WEBS 3-13=135/87, 4-13=601/1326, 5-13=996/589, 5-12=0/174, 6-12=0/174, 6-11=996/589, 7-11=600/1326, 8-11=136/87

2 = 0.83, 3 = 0.34, 4 = 0.84, 5 = 0.46, 6 = 0.46, 7 = 0.84, 8 = 0.34, 9 = 0.83, 11 = 0.66, 12 = 0.78 and 13 = 0.66

NOTES (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDF=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 906 lb uplift at joint 2 and 906 lb uplift at joint 9.
- 7) Girder carries hip end with 8-0-12 right side setback, 8-0-12 left side setback, and 7-0-0 end setback.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 473 lb down and 297 lb up at 23-3-4, and 473 lb down and 297 lb up at 8-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

1) Regular: Lumber Increase=1.25. Plate Increase=1.25

Concentrated Loads (lb)
Vert: 13=-473(F) 11=-473(F)

Job	Truss	Truss Type	Qty	Ply	ADAM'S FRAMING - MIKE & DAISY NICKELSON
L288684	T14	HIP	1	1	L288684036 Job Reference (optional)

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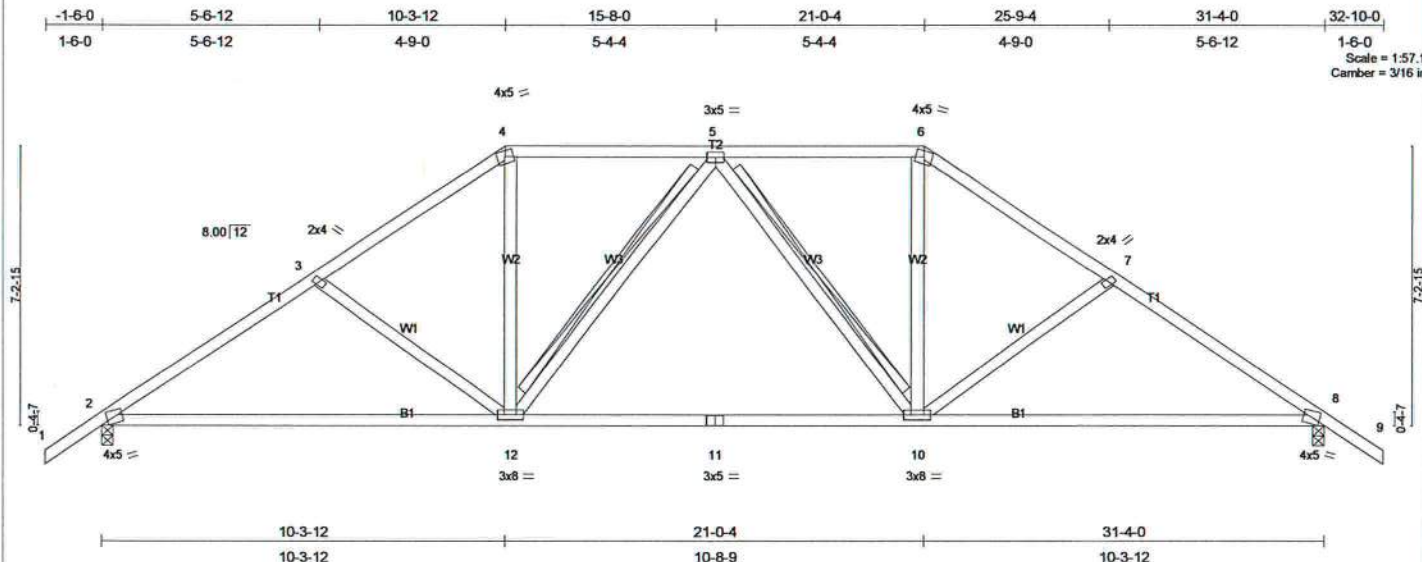


Plate Offsets (X,Y): [2:0-2-1,0-0-10], [8:0-2-1,0-0-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.27	Vert(LL)	-0.21 8-10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.52	Vert(TL)	-0.39 8-10	>957	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.16	Horz(TL)	0.06 8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 168 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 4-11-3 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 9-6-11 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-12, 5-10
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.
	,with 4in minimum end distance.
	Brace must cover 90% of web length.

REACTIONS

(lb/size) 2=1082/0-3-8, 8=1082/0-3-8
Max Horz 2=-191(load case 4)
Max Uplift2=-254(load case 6), 8=-254(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/44, 2-3=-1511/719, 3-4=-1271/666, 4-5=-1003/618, 5-6=-1003/618, 6-7=-1271/666, 7-8=-1511/719, 8-9=0/44
BOT CHORD 2-12=433/181, 11-12=-327/1111, 10-11=-327/1111, 8-10=-433/181
WEBS 3-12=-235/228, 4-12=-170/414, 5-12=-275/196, 5-10=-275/196, 6-10=-170/414, 7-10=-235/228

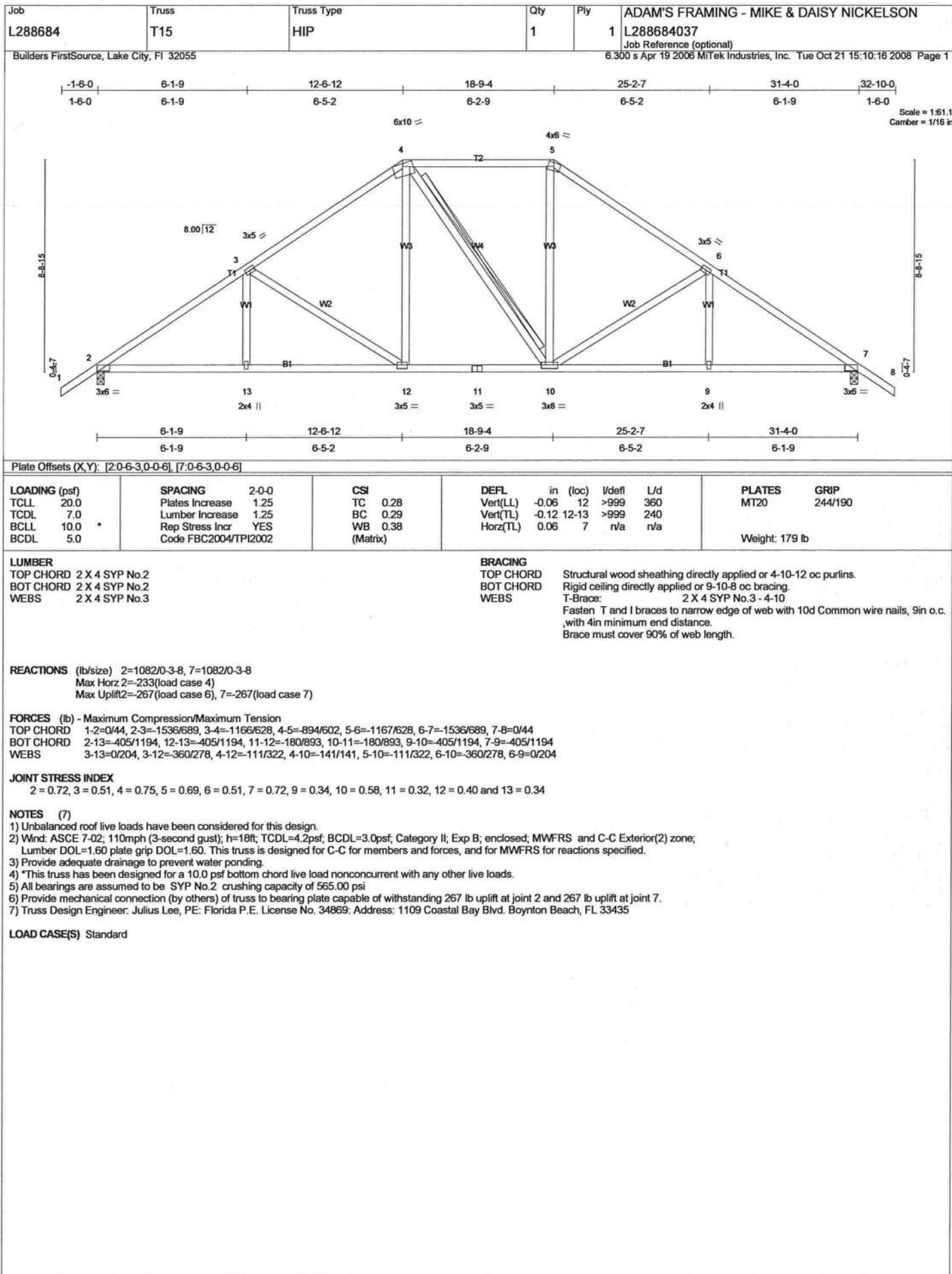
JOINT STRESS INDEX

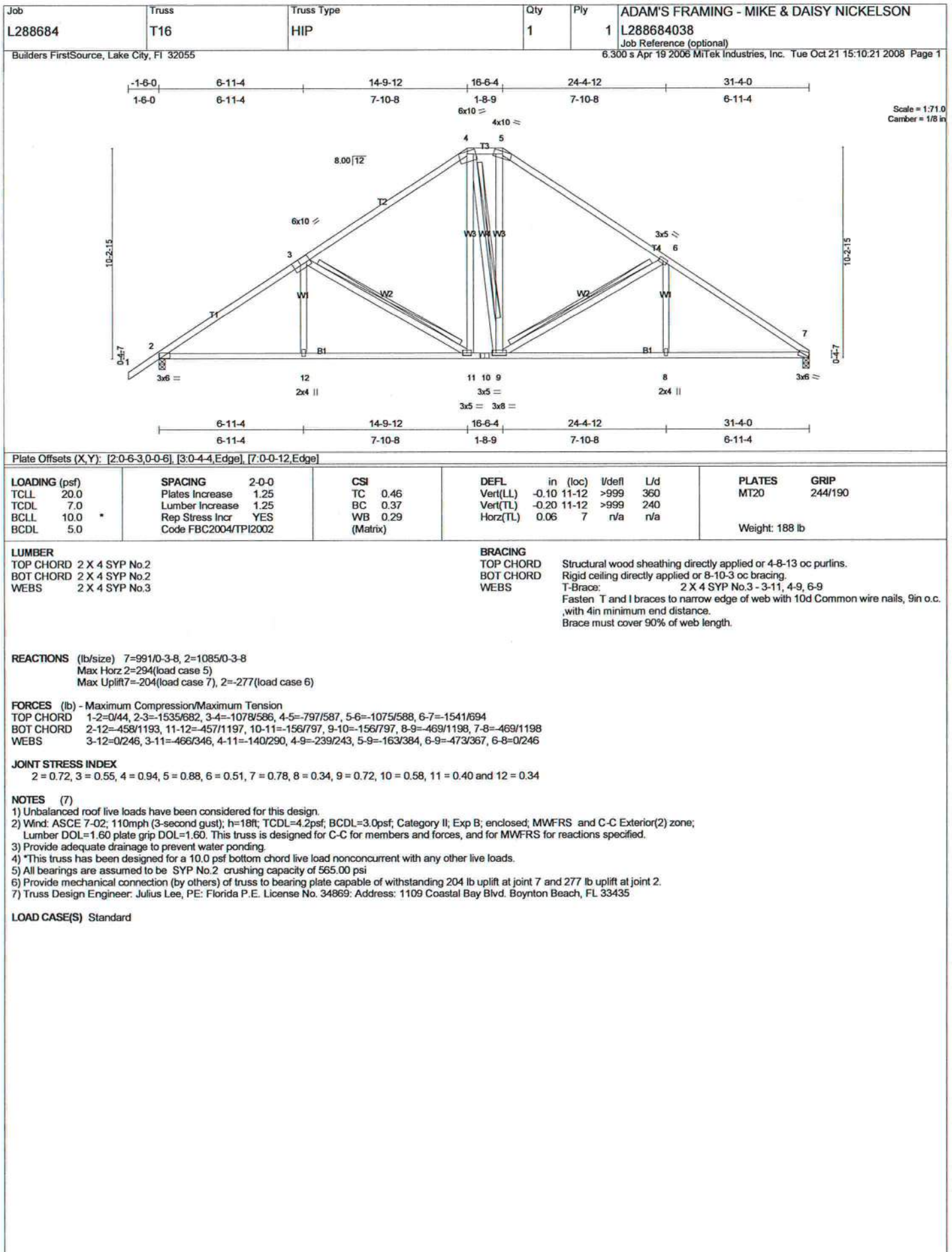
2 = 0.87, 3 = 0.34, 4 = 0.71, 5 = 0.42, 6 = 0.71, 7 = 0.34, 8 = 0.87, 10 = 0.58, 11 = 0.62 and 12 = 0.58

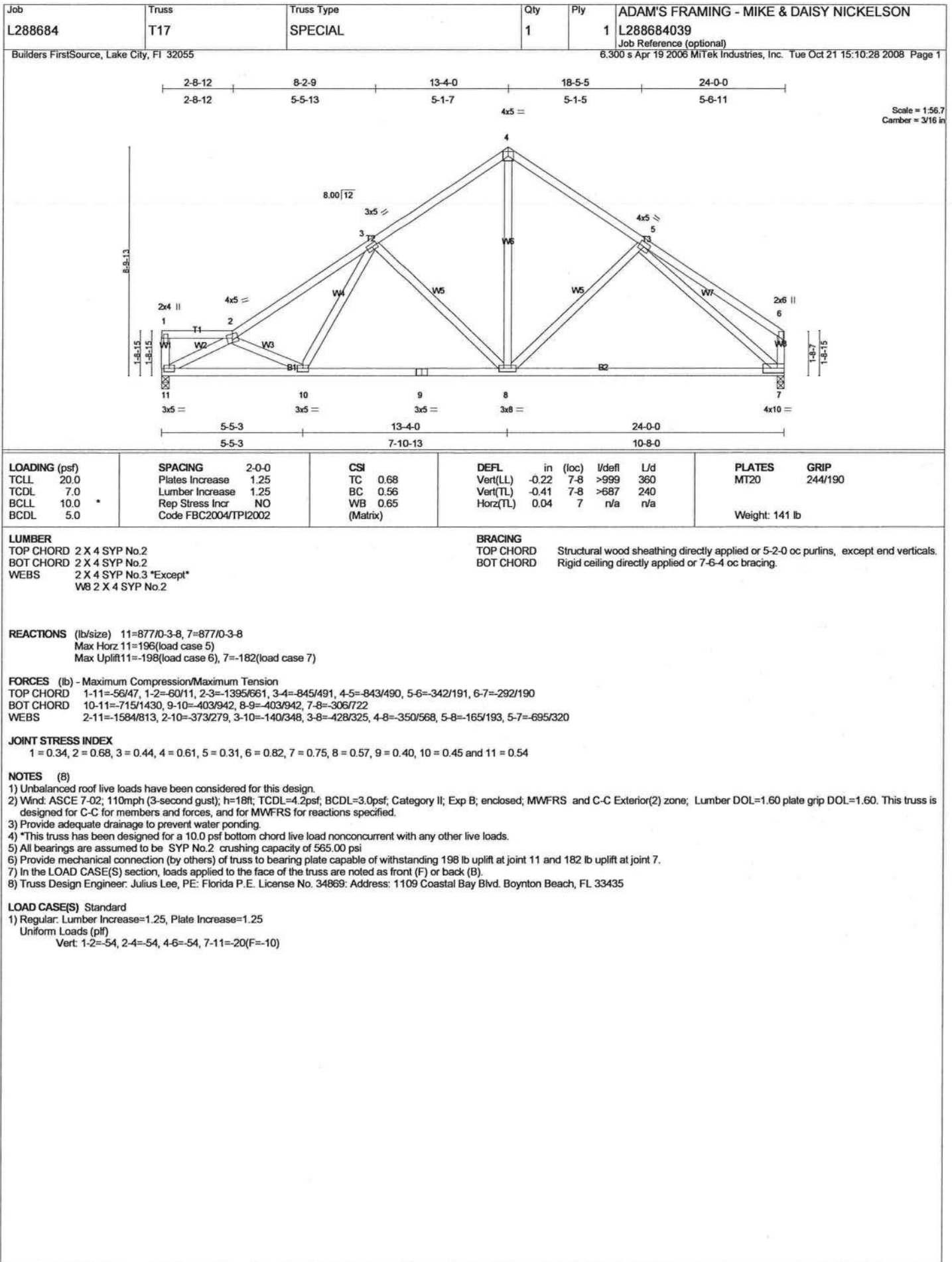
NOTES (7)

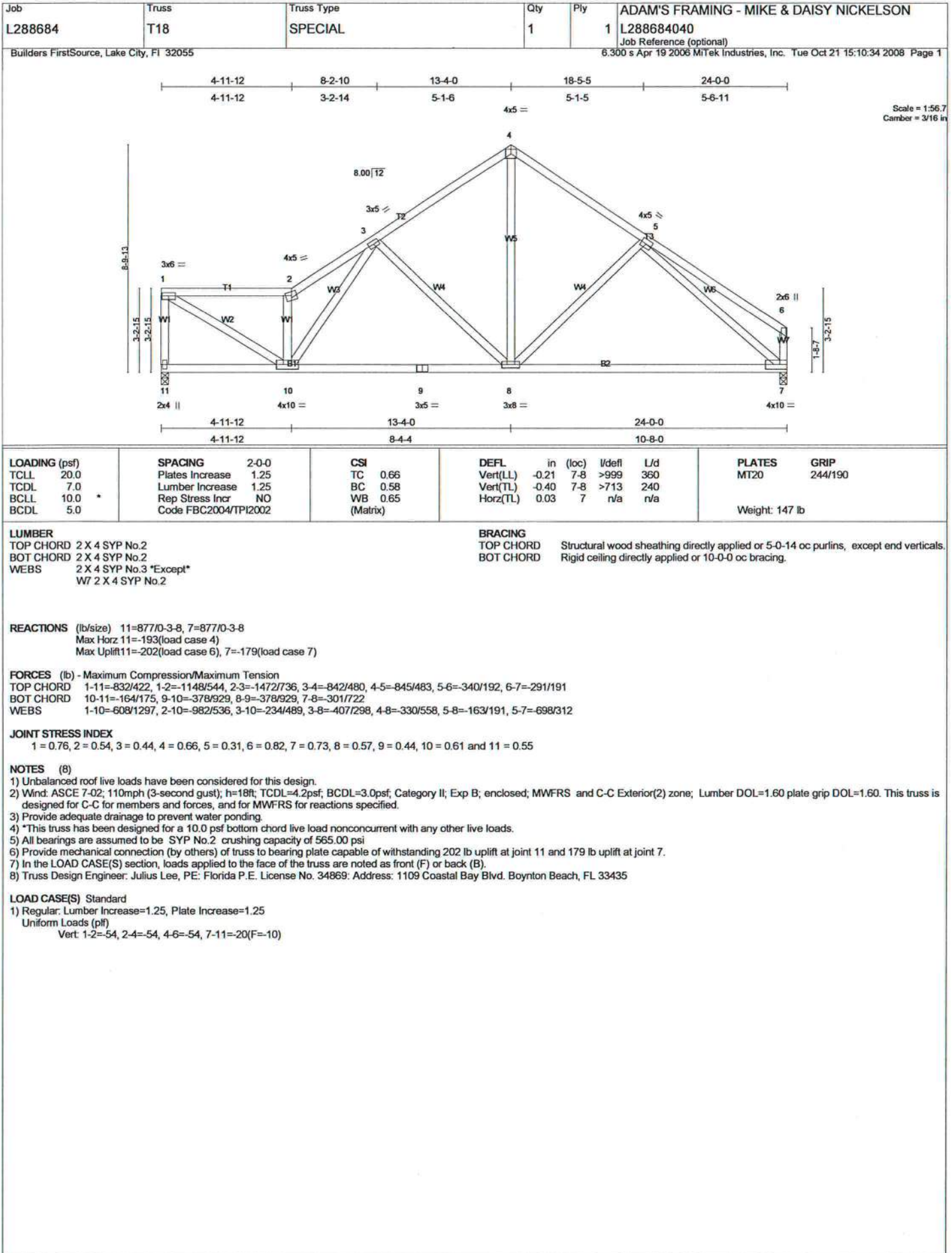
- 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph (3-second gust), $h=18ft$, $TCDL=4.2psf$, $BCDL=3.0psf$, Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 2 and 254 lb uplift at joint 8.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard









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LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-5=-54, 5-6=-54, 6-8=-54, 2-13=-10, 9-13=-20(F=-10)
 Concentrated Loads (lb)
 Vert: 14=-292(F)

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-6=-54, 6-7=-54, 7-8=-54, 8-9=-54, 2-15=-10, 10-15=-20(F=-10)

Job L288684	Truss T21	Truss Type SPECIAL	Qty 1	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684043 Job Reference (optional)
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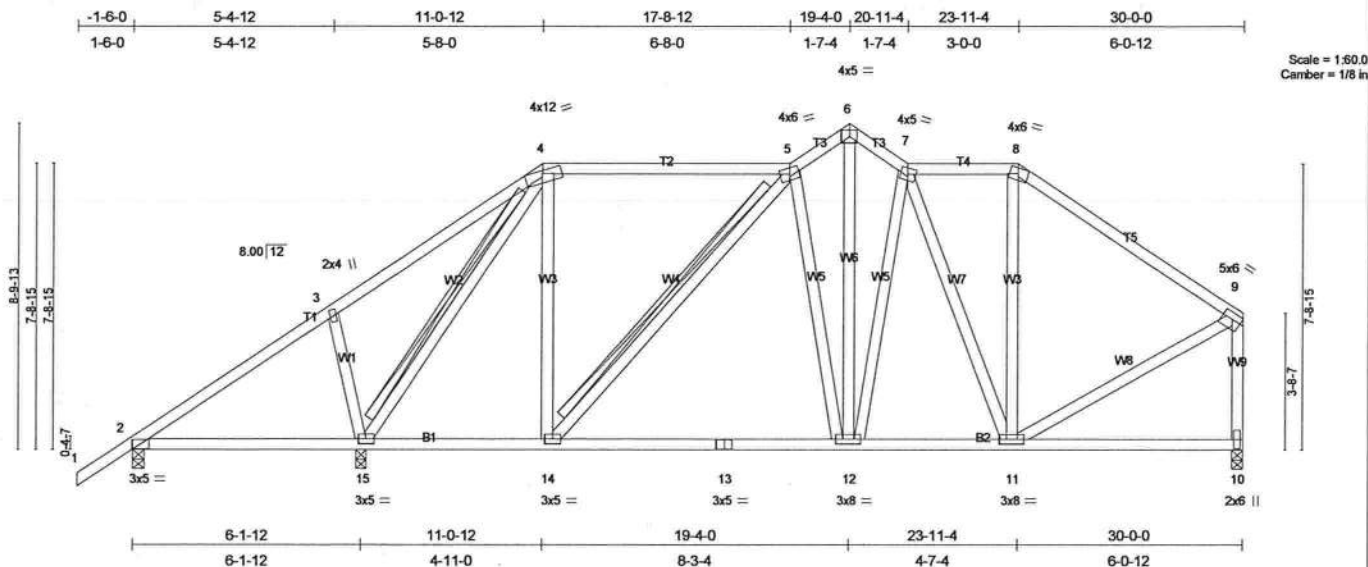


Plate Offsets (X,Y): [2.0-2.9,0-1.8], [9.0-2.12,0-2.0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.48	Vert(LL) -0.11 12-14	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.42	Vert(TL) -0.22 12-14	>999	240		
BCLL 10.0	Rep Stress Incr NO	WB 0.66	Horz(TL) 0.02 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)					
						Weight: 213 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
6-0-0 oc bracing: 2-15.
T-Brace: 2 X 4 SYP No.3 - 4-15, 5-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size)

2=252/0-3-8, 15=1109/0-3-8, 10=865/0-3-8
Max Horz 2=249(load case 5)
Max Uplift 2=117(load case 6), 15=472(load case 6), 10=225(load case 7)
Max Grav 2=258(load case 10), 15=1109(load case 1), 10=865(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/44, 2-3=-141/65, 3-4=-56/139, 4-5=-473/371, 5-6=-774/578, 6-7=-714/533, 7-8=-544/440, 8-9=-739/435, 9-10=-797/462
BOT CHORD 2-15=-84/116, 14-15=-195/465, 13-14=-342/693, 12-13=-342/693, 11-12=-309/638, 10-11=-45/55
WEBS 3-15=-263/289, 4-15=-921/502, 4-14=-186/412, 5-14=-335/218, 5-12=-434/319, 6-12=-519/691, 7-12=-188/160, 7-11=-273/118, 8-11=-58/163, 9-11=-256/562

JOINT STRESS INDEX

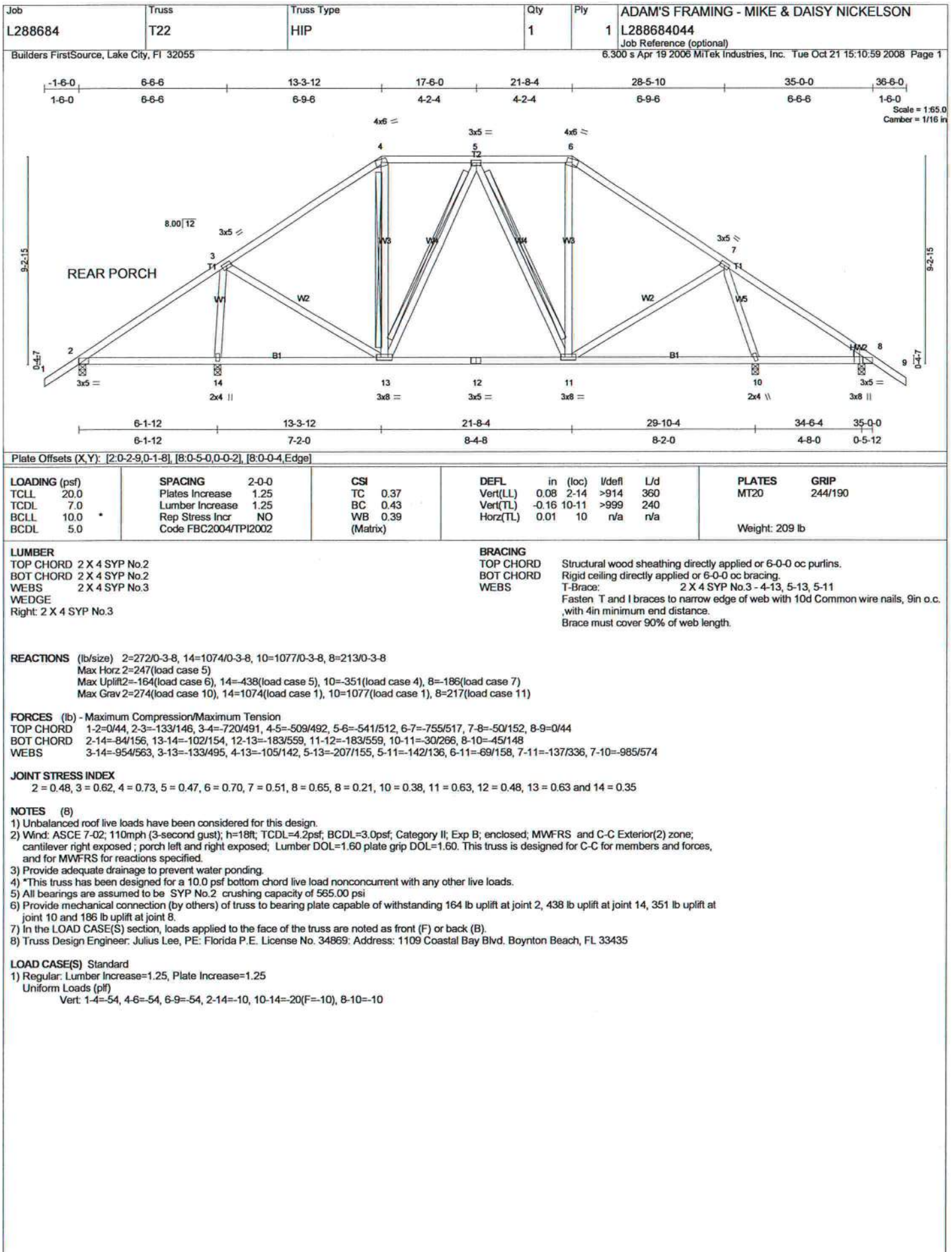
2 = 0.51, 3 = 0.34, 4 = 0.33, 5 = 0.69, 6 = 0.37, 7 = 0.41, 8 = 0.65, 9 = 0.89, 10 = 0.54, 11 = 0.65, 12 = 0.60, 13 = 0.65, 14 = 0.41 and 15 = 0.44

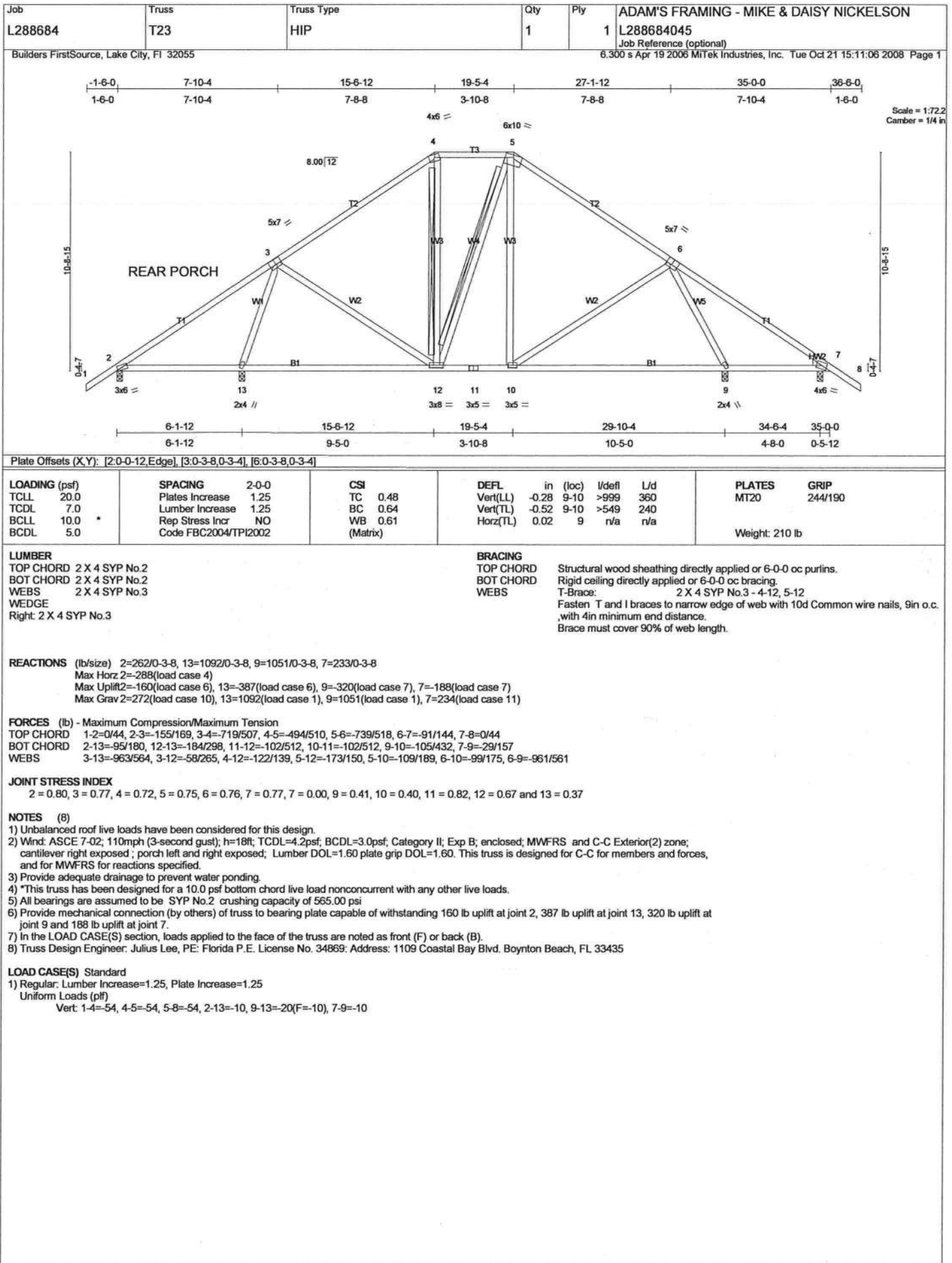
NOTES (8)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 2, 472 lb uplift at joint 15 and 225 lb uplift at joint 10.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-6=-54, 6-7=-54, 7-8=-54, 8-9=-54, 2-15=-10, 10-15=-20(F=-10)

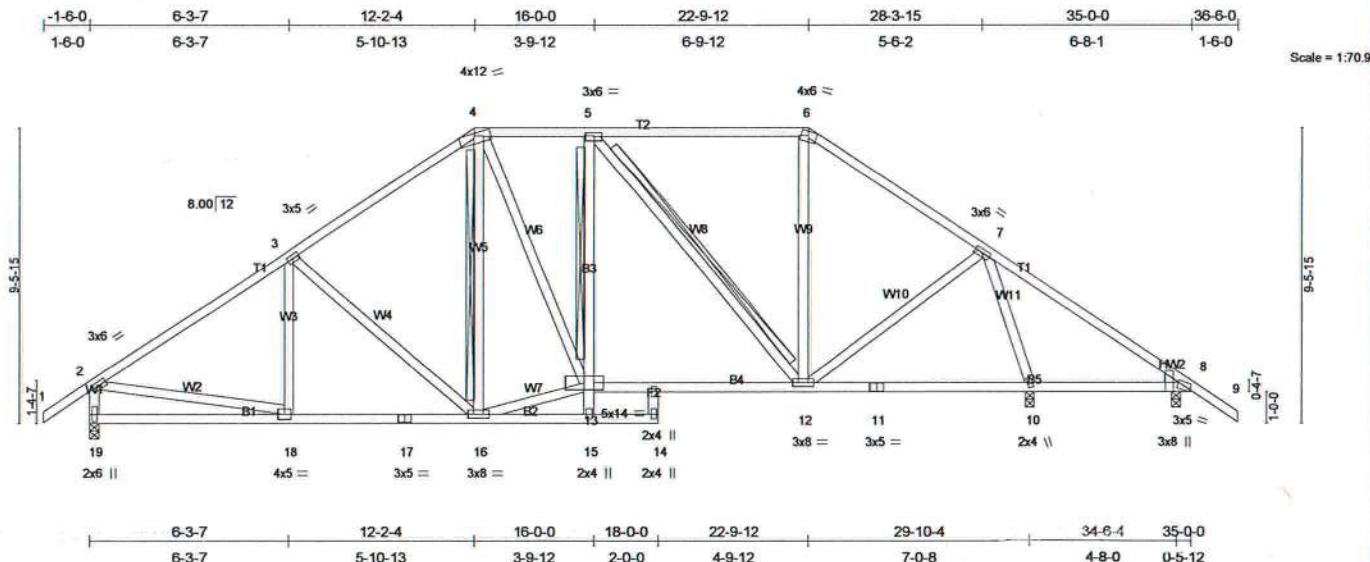




Job L288684	Truss T24	Truss Type SPECIAL	Qty 6	Ply 1	ADAM'S FRAMING - MIKE & DAISY NICKELSON L288684046 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Oct 21 15:11:13 2008 Page 1



#2 HIP OR COMMON TRUSS

Setback 7' or Less

120 MPH MAX

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

BRG LOC:

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

UPLIFT: 400# or Less

BRG LOC:

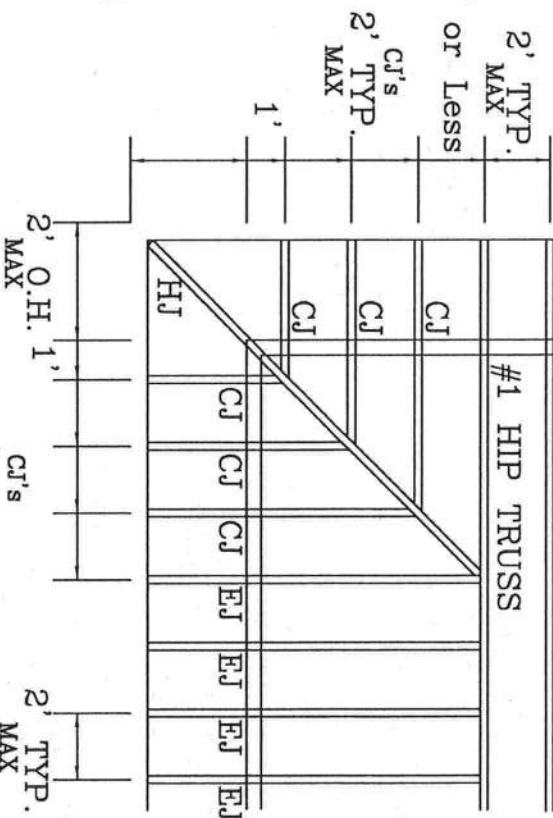
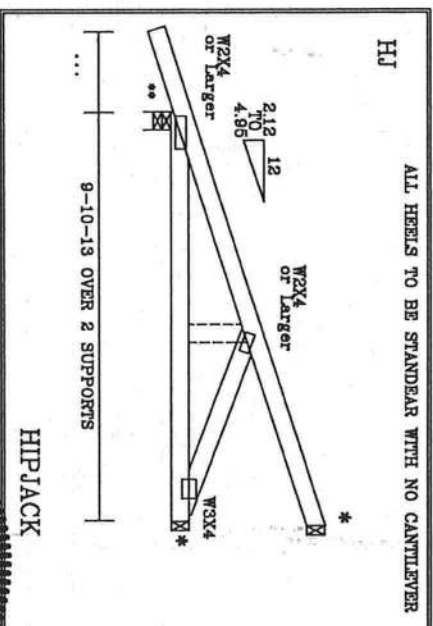
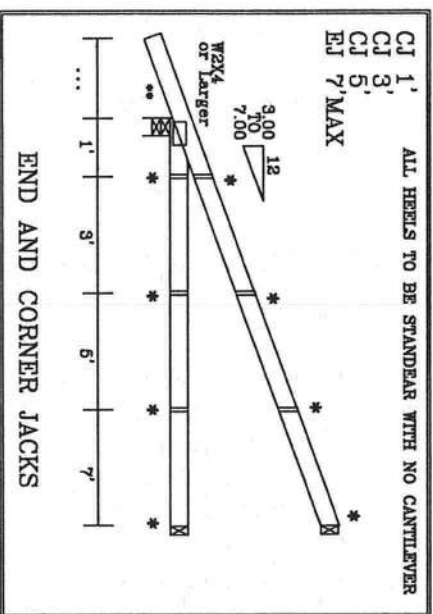
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND
SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

UPLIFT: 400# or Less

BRG LOC

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



*(3) 16d TOENAILS

SEE FOR THE DOWN
 看看

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET SETBACK

7'0" MAX

DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE BUILDING STRUCTURE AND THE BUILDING SYSTEMS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE BUILDING STRUCTURE AND THE BUILDING SYSTEMS. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE BUILDING STRUCTURE AND THE BUILDING SYSTEMS.

COXS. KENNELERS, P.
1405 ST 4th AVENUE
DELAWARE PLACES, 3444-210
NO. 34859

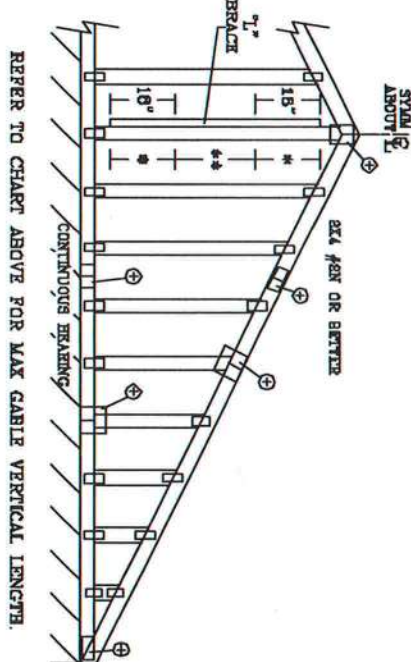
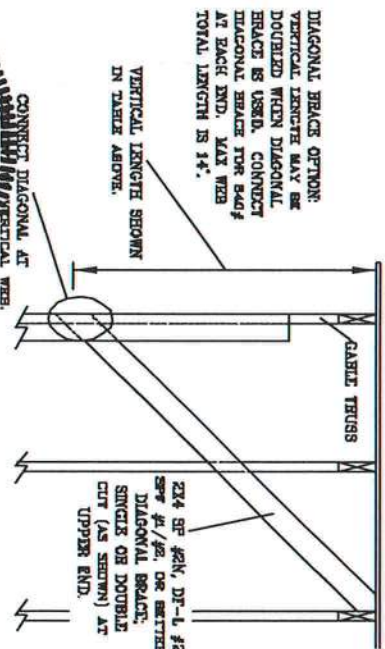
SERIAL		DATE
20	MAX PSF	TC LT
MAX	PSF	TC DL
10*	MAX PSF	BC LT
MAX	PSF	BC DL
5	MAX PSF	
20	MAX PSF	TC LT
MAX	PSF	TC DL
10*	MAX PSF	BC LT
MAX	PSF	BC DL
5	MAX PSF	
20	MAX PSF	TC LT
MAX	PSF	TC DL
10*	MAX PSF	BC LT
MAX	PSF	BC DL
5	MAX PSF	

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	

REVIEWED
By Julius Lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 16' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH			
CABLE VERTICAL SPACING / SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE
24" O.C.	SPF	#1 / #2	3' 4"
	STUD	#3	3' 3"
	HF	STANDARD	3' 3"
	SP	#1	3' 3"
16" O.C.	SPF	#1 / #2	3' 4"
	STUD	#3	3' 3"
	HF	STANDARD	3' 3"
	SP	#1	3' 3"
12" O.C.	SPF	#1 / #2	3' 4"
	STUD	#3	3' 3"
	HF	STANDARD	3' 3"
	SP	#1	3' 3"



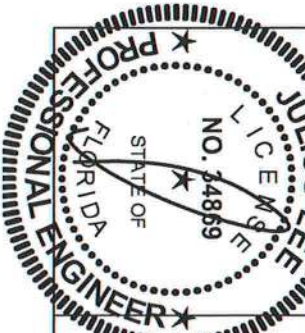
CABLE VERTICAL PLATE SIZES			
VERTICAL LENGTH	NO SPICE	LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4		
GREATER THAN 11' 6"	2X6		

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND HEBL PLATES.

BRACING GROUP SPECIES AND GRADES			
GROUP A:		GROUP B:	
SPICE - PINE - FIR	HEAVY - FIR	SPICE - PINE - FIR	HEAVY - FIR
#1 / #2	STUD	#1 / #2	STUD
STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD
DOUGLAS FIR - LARCH		DOUGLAS FIR - LARCH	
#1	STUD	#1	STUD
STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD

GROUP B: HEAVY - FIR #1 & HIR #1

SOUTHERN PINE DOUGLAS FIR - LARCH #1 #2



REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

No. 34869

STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE 7-02-CAB10015

DATE 11/26/03

DRWG. MTRX STD. CABLE IS E HT - ENG

JULIUS LEE'S

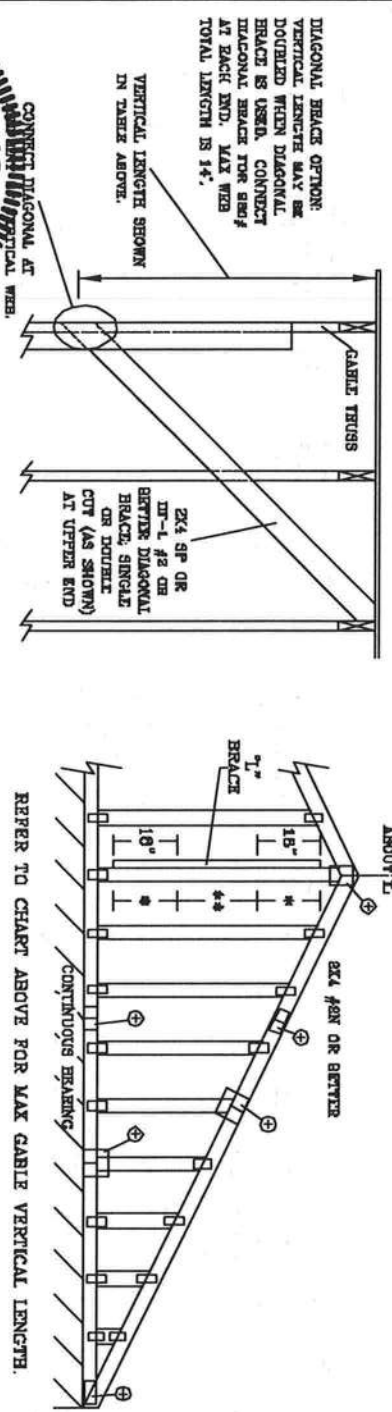
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE

DELRAY BEACH, FL 33441-8161

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1X4 T" BRACE *		(1) 2X4 T" BRACE *		(1) 2X6 T" BRACE *		(2) 2X8 T" BRACE **	
GABLE VERTICAL SPACING	SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
	HF	STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
	SP	STUD	2' 11"	3' 9"	3' 9"	6' 0"	6' 0"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
	DFL	STUD	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
16" O.C.	SPF	#1 / #2	3' 3"	4' 6"	4' 6"	6' 0"	6' 11"	7' 10"	8' 0"	9' 4"	9' 4"	12' 3"	12' 8"
	HF	STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
	SP	STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
	DFL	STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 5"	11' 4"	11' 4"	14' 0"	14' 0"
24" O.C.	SPF	#1 / #2	4' 0"	6' 11"	6' 11"	8' 3"	8' 3"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
	HF	STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP	STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	DFL	STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"



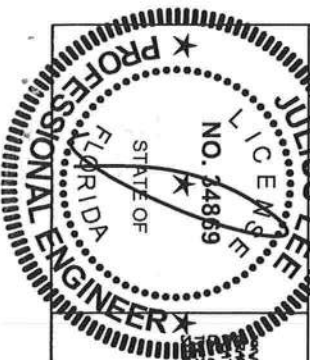
REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-YR	RED-FIR	SPRUCE-PINE-YR	RED-FIR
#1 / #2 STUD	#2 STUD	#1 / #2 STUD	#2 STUD
#3 STUD	#3 STUD	#3 STUD	#3 STUD
STANDARD	STANDARD	STANDARD	STANDARD

CABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEFLECTION CRITERIA IS L/240.
 PROVIDE UPLIFT CONNECTIONS FOR 150 PSF OVER CONTINUOUS BEARING (6 PSF TO DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 8' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
 ATTACH EACH T" BRACE WITH 104 NUTS.
 * FOR (1) T" BRACE, SPACE NUTS AT 8" O.C.
 ** FOR (2) T" BRACES, SPACE NUTS AT 3" O.C.
 OR 18" END ZONES AND 6" O.C. BETWEEN ZONES.
 T" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2X6

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND BEEL PLATES.



SWAYING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS INFORMATION, PUBLISHED BY THE TRUSS ASSOCIATION, 6800 ENTERPRISE DR., SUITE 200, MARIETTA, GA 30067 AND VITA (WOOD TRUSS COUNCIL) 1000 N. 10TH ST., SUITE 100, DENVER, CO 80202. UNLESS OTHERWISE INDICATED, TRUSS CHORDS SHALL HAVE PROPERLY ATTACHED BRACING FRAMES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

REVIEWED
 By Julius Lee at 12:00 pm, Jun 11, 2008

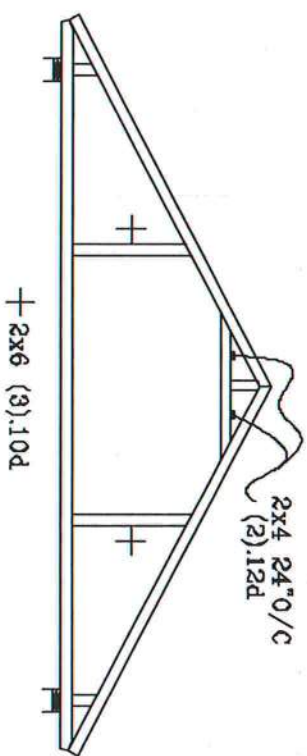
JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1456 SW 4th AVENUE
 DEER BEACH, FL 33441-6161

No. 34868
 STATE OF FLORIDA

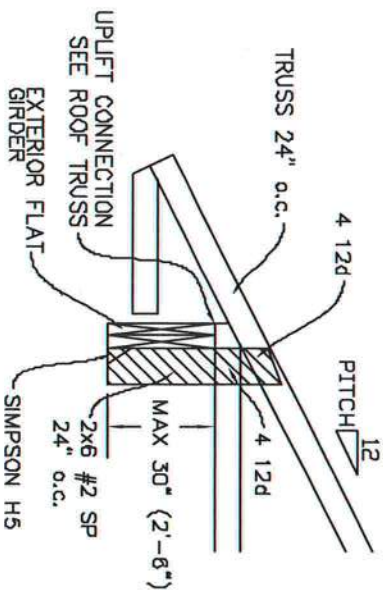
MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCE 7-02-GMB10090
 DATE 11/26/03
 DWG LATE STD GABLE 90' x 17'
 -ENG

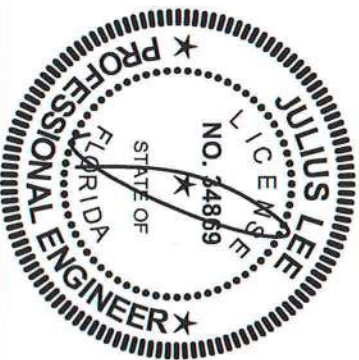
TYPICAL ATTIC TRUSS BRACING



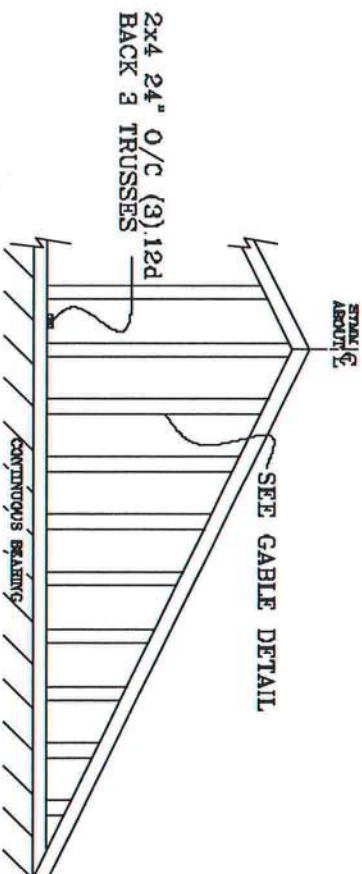
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

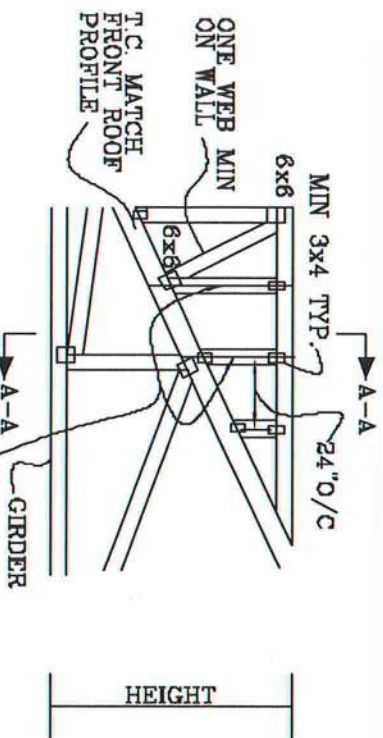


GABLE END TRUSS DETAIL



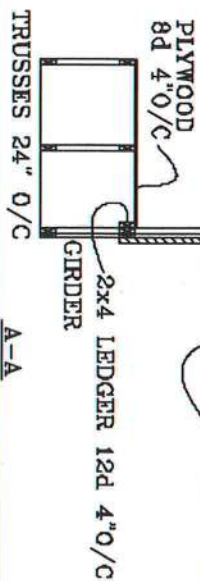
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT
ROOF 24" O/C

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



JULIUS LEE'S
CONS. ENGINEERS P.A.
1426 SW 4TH AVENUE
ODDART BRIDGE, FL 33444-2641

No. 34869
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPICES MUST BE STAGGERED SO THAT ONE SPICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

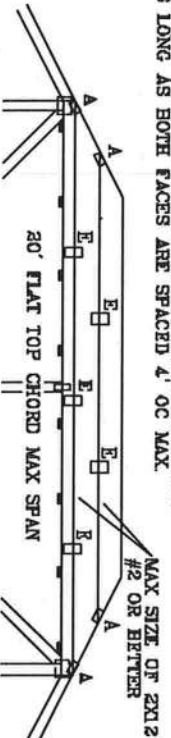
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, ENG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

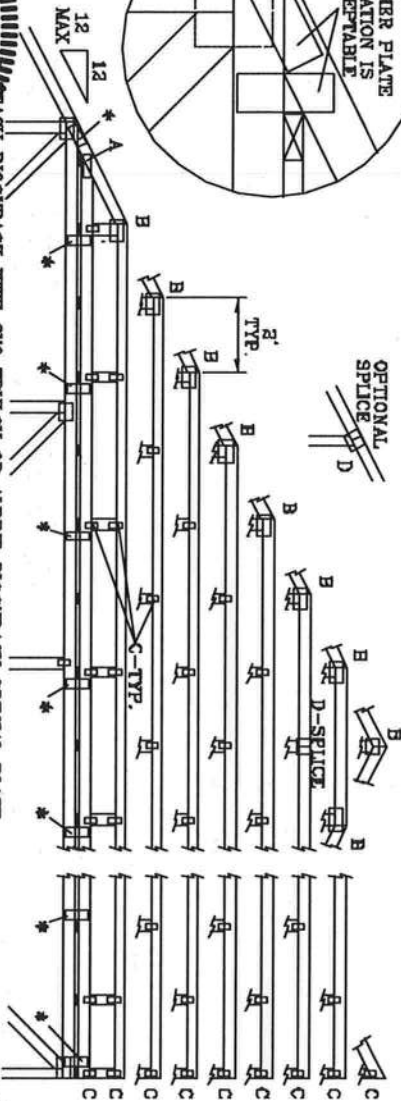
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



ETHER PLATE LOCATION IS ACCEPTABLE

OPTIONAL SPICE



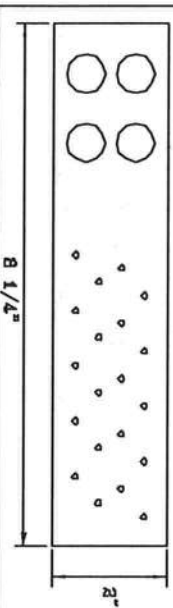
PIGGYBACK WITH 3X6 TRUSS OR ALPINE PIGGYBACK SPECIAL PLATE.

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

JOINT TYPE	SPANS UP TO		
	30'	36'	62'
A	2X4	2.6X4	3X6
B	4X6	6X6	6X6
C	1.5X3	1.6X4	1.5X4
D	5X4	6X6	6X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY		

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4" OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

JULIUS LEE'S
CONS. ENGINEERS P.A.
1440 SW 4th AVENUE
DIABLO BEACH, FL 33444-2161

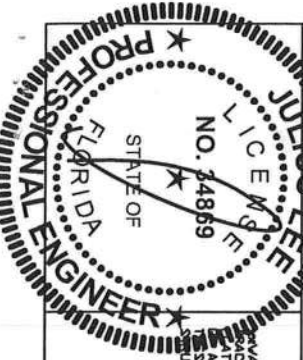
MAX LOADING

55 PSF AT
1.33 DUR. FAC.
50 PSF AT
1.25 DUR. FAC.

47 PSF AT
1.15 DUR. FAC.

REF	PIGGYBACK
DATE	09/12/07
DRWG/ITTEK	STD PIGGY
ENG	JL

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



No. 34869
STATE OF FLORIDA

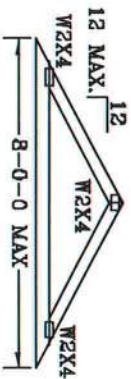
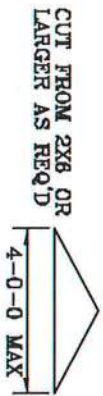
SPACING 24.0"

VALLEY TRUSS DETAIL

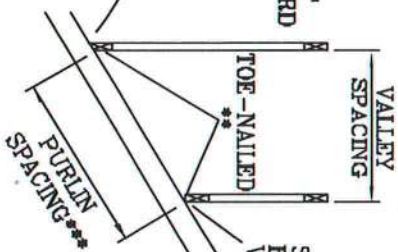
TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2/N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=6 PSF.



PITCHED CUT
BOTTOM CHORD
VALLEY

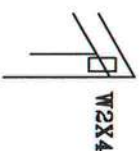


SQUARE CUT
BOTTOM CHORD
VALLEY

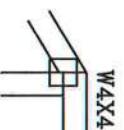
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

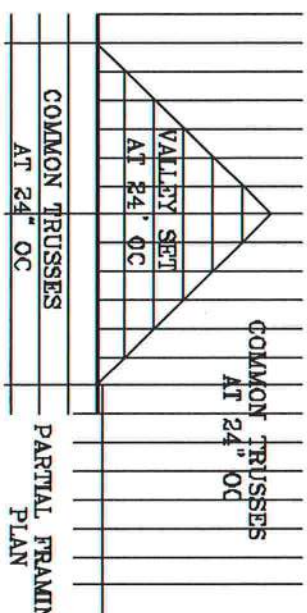
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.



OPTIONAL STUB
END DETAIL



OPTIONAL HIP
JOINT DETAIL



COMMON TRUSSES
AT 24" OC

PARTIAL FRAMING
PLAN

THIS DRAWING REPLACES DRAWING A105

JULIUS LEE'S
CONS. ENGINEERS P.A.
1655 SW 4th AVENUE
DELRAY BEACH, FL 33444-5101

TC IL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC IL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DUR.FAC.	1.25	1.25			
SPACING	24"				

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, DESTALING AND
ERECTING. REFER TO BEST PRACTICES FOR BUILDING DEPENDENT SAFETY INFORMATION. FOLLOWED BY THE TRUSS
MANUFACTURER'S INSTRUCTIONS. THE TRUSS MANUFACTURER'S INSTRUCTIONS MUST BE FOLLOWED FOR ALL
TRUSS FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED
STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

NO. 24869

STATE OF FLORIDA

PROFESSIONAL ENGINEER

REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

No. 24869
STATE OF FLORIDA

DUR.FAC. 1.25
SPACING 24"

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

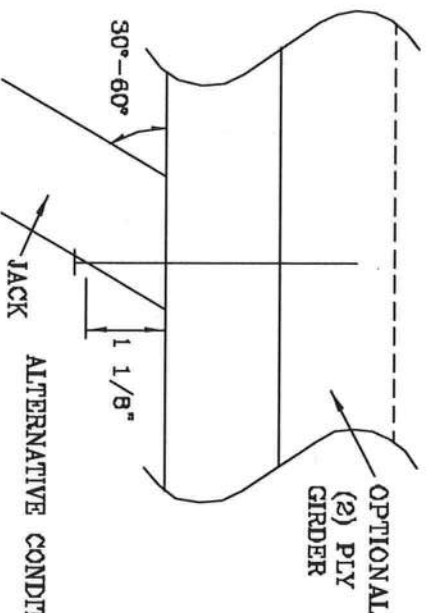
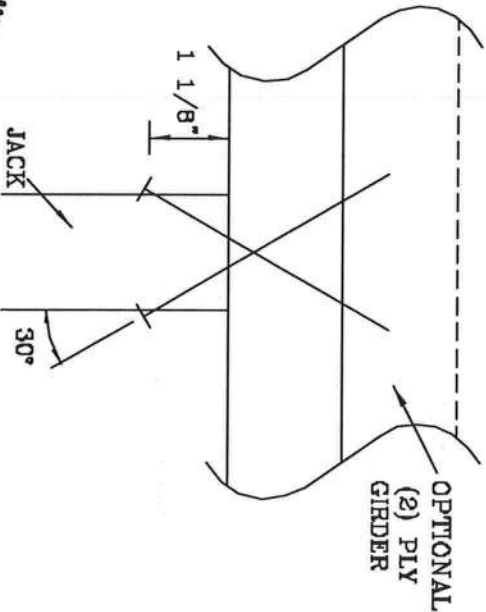
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

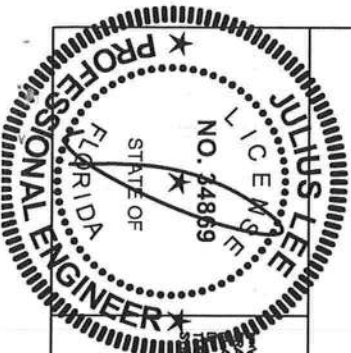
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES
2	197#	256#	181#	234#	156#	203#	154#	189#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND SPACING. REFER TO BEST PRACTICES FOR TRUSS CONSTRUCTION, PUBLISHED BY THE TRUSS ASSOCIATION, 388 JONES RD., SUITE 200, HANSON, VT 05719 AND VITA (WOOD) TRUSS COUNCIL, 10000 W. 10TH AVE., SUITE 100, DENVER, CO 80231. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED JOINTS AND BRACING SHALL HAVE A PROPERLY ATTACHED JOINT.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

1490 ST. 4TH AVENUE
DELRAY BEACH, FL 33444-2161

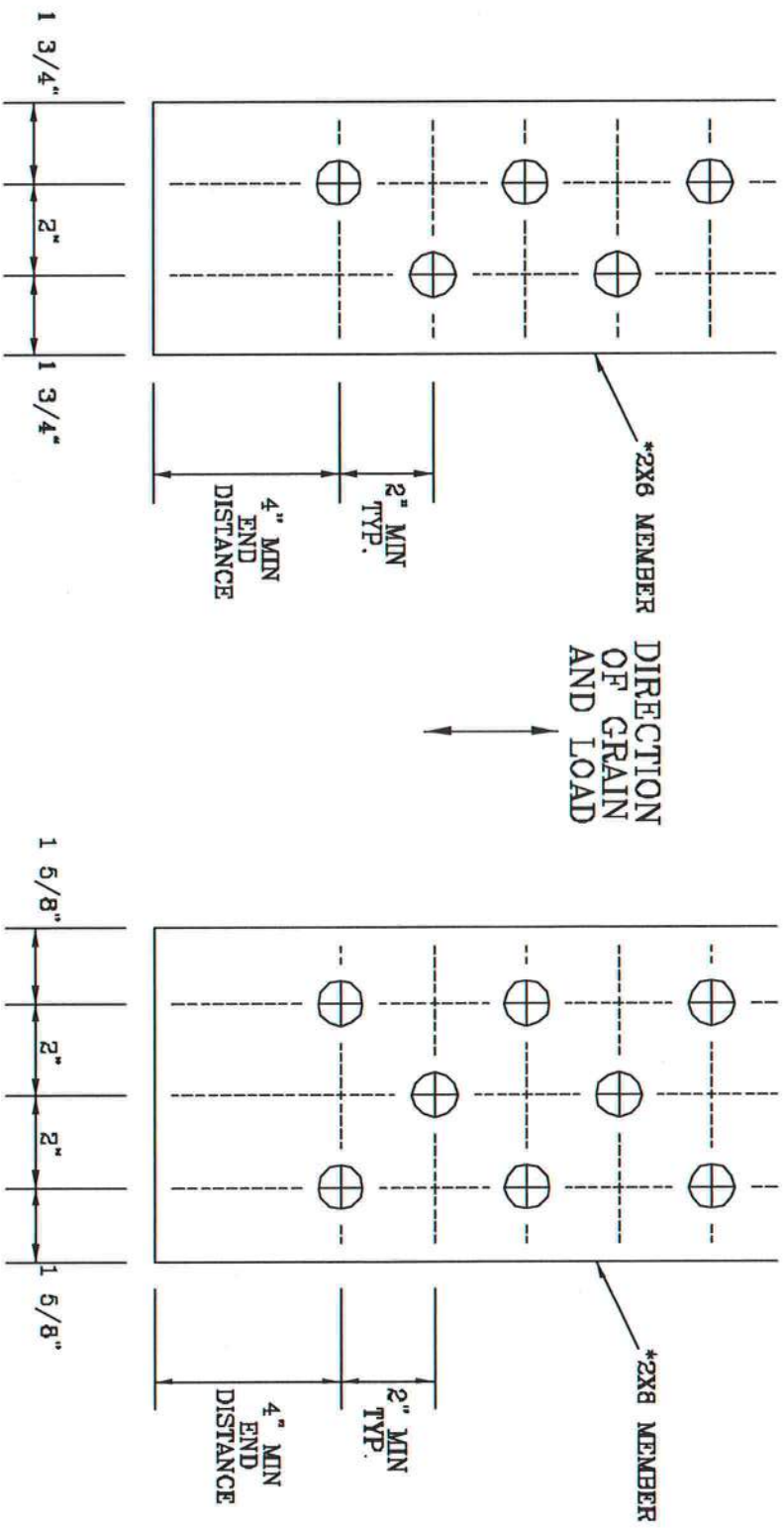
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

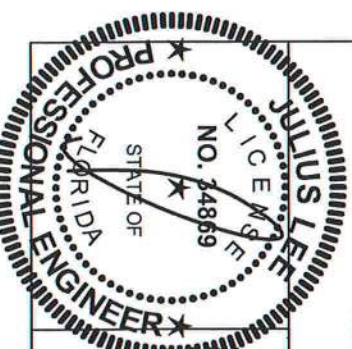
THIS DRAWING REPLACES DRAWING A628.016

VARIOUS TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 201 L-20 BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 3800 ENTERPRISE LN, MADISON, VT 05750 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 37 4TH AVENUE
DELMAR BEACH, FL 33444-2461

No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

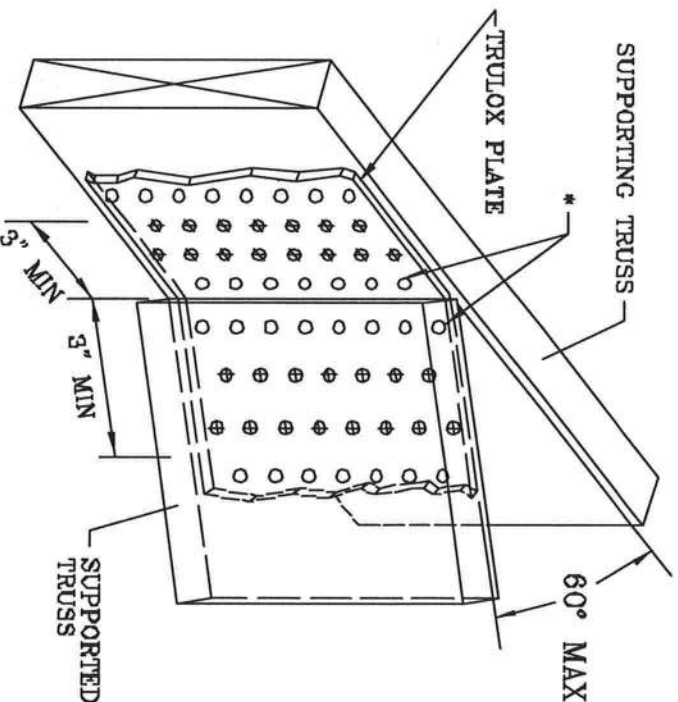
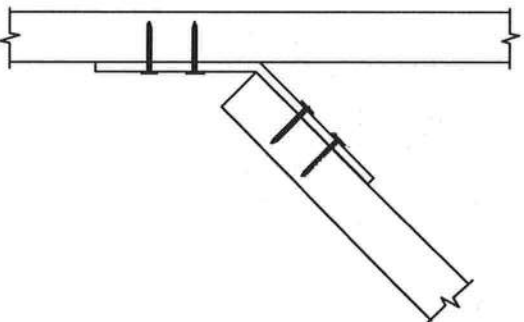
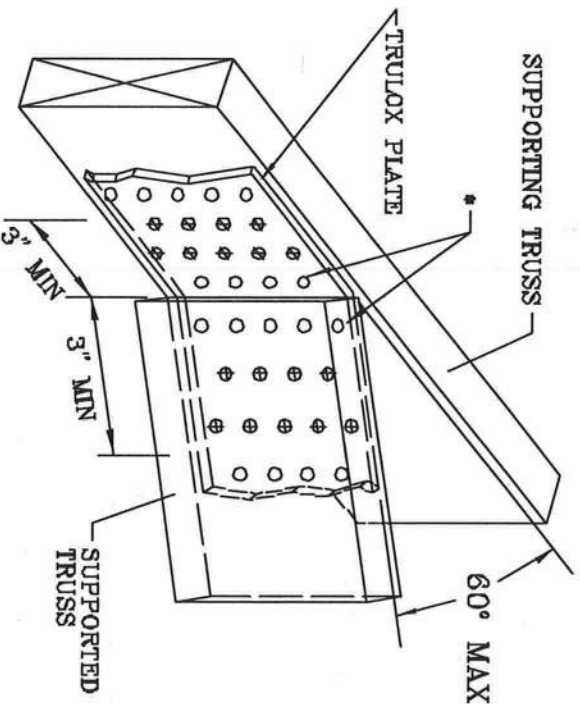
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FULL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



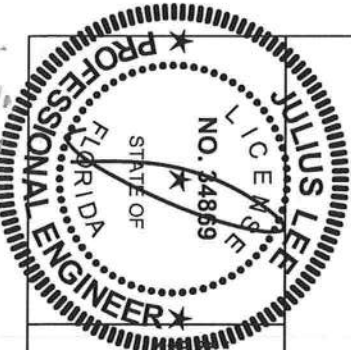
MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#

MINIMUM 5X6 TRULOX PLATE

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,988/R
1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SUPPORTING, INSTALLING AND BRACING. REFER TO ACI 308-1-03 (BUILDING COMPONENT SAFETY) FOR ADDITIONAL INFORMATION. TRUSSES ARE NOT TO BE USED IN ANY MANNER OTHER THAN AS SPECIFIED. TRUSSES ARE NOT TO BE USED IN ANY MANNER OTHER THAN AS SPECIFIED. TRUSSES ARE NOT TO BE USED IN ANY MANNER OTHER THAN AS SPECIFIED.

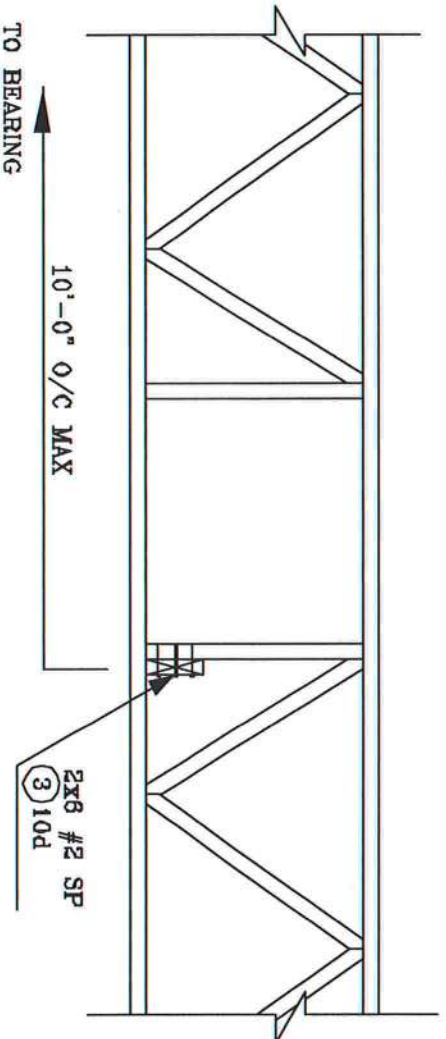
JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-5811

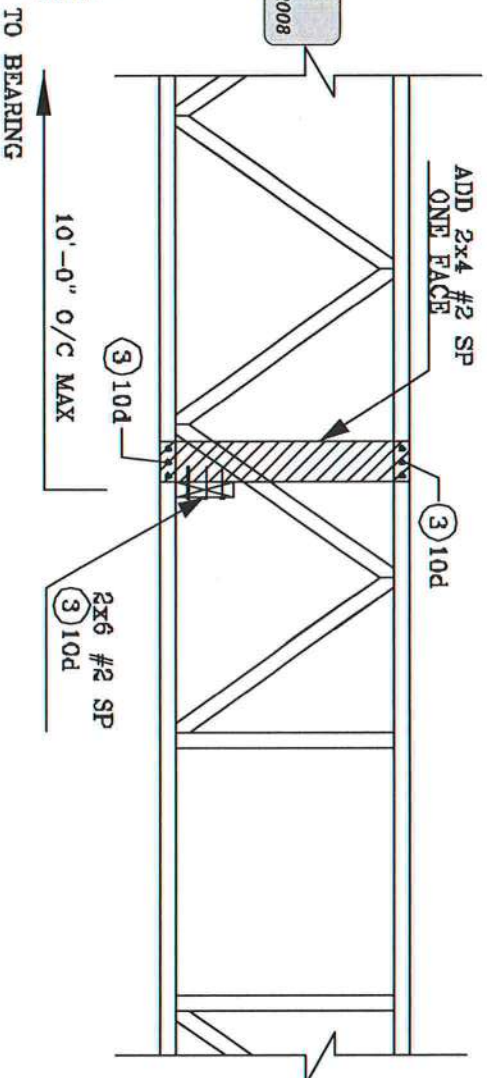
No: 34889
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

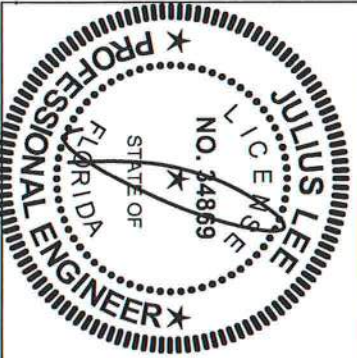
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



JULIUS LEE'S
CONS. ENGINEERS P.A.
1426 SW 4th AVENUE
DIKRAY BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

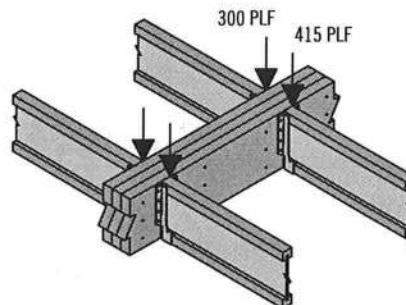
Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 1/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

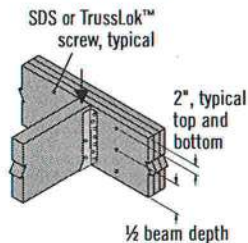
(2) 6" long screws required.

(3) 5" long screws required.

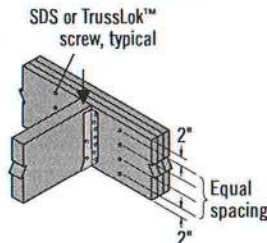
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections

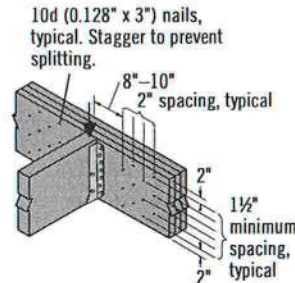
4 or 6 or Screw Connection



8 Screw Connection

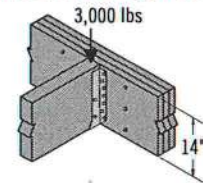


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

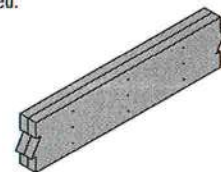
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6