DATE 02/13/2009

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000027632

APPLICANT CHARLES NICKELSON PHONE 386.965.6906

ADDRESS POB 3248 FL 32056

OWNER CHARLES NICKELSON PHONE 386.965.6906

ADDRESS 718 SW MEADOW TERRACE LAKE CITY FL 32025

CONTRACTOR CHARLES NICKELSON PHONE 386.965.6906

LOCATION OF PROPERTY 47-S TO SOUTHWOOD EST,TL TO LITTLE,TL TO SOUTHWOOD MEADOWS,TR PAST CUL-DE-SAC, 2ND LOT ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 105200.00

HEATED FLOOR AREA 1496.00 TOTAL AREA 2104.00 HEIGHT 20.80 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 12-5S-16-03585-011 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 1.50

000001707				
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
WAIVER	09-0062	BLK	WR	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE. SECT. 14.9 SPECIAL FAMILY LOT PERMIT - BROTHER.
1 FOOT ABOVE ROAD.

Check # or Cash 1256

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE $ 530.00 CERTIFICATION FEE $ 10.52 SURCHARGE FEE $ 10.52

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 626.04

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0901-17 Date Received 1/13 By JW Permit # 1709 = 27632

Zoning Official BLK Date 29.01.09 Flood Zone X FEMA Map # N/A Zoning A-3

Land Use A-3 Elevation N/A MFE 1st floor Rd River N/A Plans Examiner WL Date 1-16-09

Comments Sect. 14.9 Special Family Lot Parent - Brother

☑ NOC ☐ EH ☑ Deed or PA ☑ Site Plan ☐ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor

☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 09-0062 Fax

Name Authorized Person Signing Permit Charles Nickelson Phone 386-965-6906

Address ~~226 SW Buttercup dr.~~ P.O. Box 3248 Lake City FL ~~32025-3256~~ 32056

Owners Name Charles Nickelson Phone 386-965-6906

911 Address ~~226 SW Buttercup dr.~~ 218 SW MEADOW TERRACE Lake City FL 32025

Contractors Name Charles Nickelson (owner) Phone 386-965-6906

Address PO Box 3248, Lake City. FL 32056

Fee Simple Owner Name & Address Charles Nickelson, PO Box 3248. L.C. 32056

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Nick Giesler, 1785 NW Brown rd. L.C. 32025

Mortgage Lenders Name & Address N/A

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 12-5S-16 03585-011 Estimated Cost of Construction $72,000

Subdivision Name See attached deed Lot ____ Block ____ Unit ____ Phase ____

Driving Directions US Hwy 47 south, to Swinwoodestates; turn left, into Southwood est, follow to Little rd., turn left, follow to Southwood Meadows turn right, follow post cul-de-sac, 2nd on R. Number of Existing Dwellings on Property Ø

Construction of Single family dwelling Total Acreage 1.50 Lot Size ____

Do you need a - Culvert Permit or (Culvert Waiver) or Have an Existing Drive Total Building Height 20' 8"

Actual Distance of Structure from Property Lines - Front 45' Side ~~80'~~ Side 57'8" Rear 63'8"

Number of Stories 1 Heated Floor Area 1496 Total Floor Area 2104 Roof Pitch 7/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

LEFT MESSAGE
1/29/09

18.50
33

SPECIAL WARRANTY DEED

THIS INDENTURE, made this 28th day of November, 2006, between JOSHUA NICKELSON and his wife DANETTE NICKELSON, whose address is 197 SW Waterford Court, Suite 105, Lake City, Florida 32025, Grantors, and SOUTHEAST DEVELOPERS GROUP, INC., a Florida corporation, whose address is 197 SW Waterford Court, Suite 105, Lake City, Florida 32025, Grantee,

W I T N E S S E T H:

That Grantors, for and in consideration of the sum of TEN AND NO/100 ($10.00) DOLLARS, and other good and valuable considerations to Grantors in hand paid by Grantee, the receipt whereof is hereby acknowledged, have granted, bargained and sold to Grantee and Grantee's heirs, successors and assigns forever, the following described land, situate and lying in COLUMBIA County, Florida:

SEE SCHEDULE A ATTACHED HERETO

(Tax parcel number 12-5S-[redacted] - cutout)

SUBJECT TO: Taxes for 2006 and subsequent years; restrictions and easements of record; easements shown by the plat of the property; and existing mortgage indebtedness on the property, if any.

Grantors hereby fully warrant the title to said land and will defend the same against the lawful claims of all persons claiming by, through, or under Grantors.

IN WITNESS WHEREOF, Grantors have hereunto set their hands and seals the day and year above written.

Signed, sealed and delivered
in the presence of:

Eddie M. Anderson
Print Name: Eddie M. Anderson

Julie Calloway
Print Name: Julie Calloway
Witnesses as to Grantors

JOSHUA NICKELSON

Danette Nickelson
DANETTE NICKELSON

This Instrument Prepared By:
EDDIE M. ANDERSON, P.A.
P. O. Box 1179
Lake City, Florida 32056-1179

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 28th day of November, 2006, by JOSHUA NICKELSON and DANETTE NICKELSON. They are personally known to me or they produced _____ as identification.

Julie Calloway
Notary Public
My commission expires: 1-23-2010

(Notarial Seal) Notary Public State of Florida
Julie Calloway
My Commission DD501123
Expires 01/23/2010

Inst:2006028027 Date:11/28/2006 Time:16:35
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1103 P:482

4

SCHEDULE A to Warranty Deed

Nickelson to Southeast Developers Group, Inc.

SHOWING TRACT "B" : THE WEST HALF OF LOT 3, RIVERS MANOR, UNIT 2, AN UNRECORDED SUBDIVISION IN THE SE 1/4 OF SECTION 1, AND THE NW 1/4 OF SECTION 12, TOWNSHIP 5 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA.

A PART OF THE NW 1/4 OF THE NE 1/4 OF SECTION 12, TOWNSHIP 5 SOUTH, RANGE 16 EAST, MORE PARTICULARLY DESCRIBED AS FOLLOWS: BEGIN AT THE NW CORNER OF SAID NW 1/4 OF THE NE 1/4 AND RUN N 89°20'28" E, ALONG THE NORTH LINE OF NW 1/4 OF THE NE 1/4, 333.21 FEET; THENCE S 00°08'38" W, 658.93 FEET; THENCE S 89°17'43" W, 331.01 FEET TO THE WEST LINE OF THE NE 1/4 OF SAID SECTION 12; THENCE N 00°02'51" W, 659.17 FEET TO THE POINT OF BEGINNING. CONTAINING 5.02 ACRES MORE OR LESS.

SUBJECT TO AN EASEMENT OVER AND ACROSS THE SOUTH 10.00 FEET AND THE EAST 15.00 FEET FOR UTILITIES AND DRAINAGE.

TOGETHER WITH A 60 FOOT ROAD EASEMENT FOR INGRESS, EGRESS AND UTILITIES IN THE EAST 1/2 OF SECTION 1 AND 12 OF TOWNSHIP 5 SOUTH, RANGE 16 EAST, THE CENTERLINE OF WITH IS PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE NORTHWEST CORNER OF THE NE 1/4 OF THE NW 1/4 OF THE SE 1/4 OF SAID SECTION 1 AND RUN N 89° 16' 03" E, ALONG THE NORTH LINE THEREOF, 529.90 FEET; THENCE S 00° 19' 13" E, 40.80 FEET TO THE SOUTH RIGHT-OF-WAY OF LITTLE ROAD ACCORDING TO THE PLAT OF RIVERS MANOR UNIT #1, AS RECORDED IN PLAT BOOK 5, PAGE 139, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. SAID POINT ALSO THE POINT OF BEGINNING FOR THE CENTERLINE OF SAID EASEMENT; THENCE S 00° 19' 13" W, ALONG SAID CENTERLINE, 698.13 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A CENTERLINE RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23'54"; THENCE SOUTHEASTERLY ALONG THE ARC OF SAID CURVE FOR AN ARC DISTANCE OF 134.07 FEET TO THE POINT OF REVERSE CURVE OF A CURVE TO THE RIGHT HAVING A RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23' 54"; THENCE SOUTHERLY ALONG THE ARC OF SAID CURVE AN ARC DISTANCE OF 134.07 FEET; THENCE S 00°19'13" E, 1336.16 FEET TO THE POINT OF CURVE TO THE RIGHT HAVING A CENTERLINE RADIUS OF 230.00 FEET AND AN INCLUDED ANGLE OF 89°39'41" ; THENCE SOUTHWESTERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 359.92 FEET; THENCE S 89°20'28" W, 119.25 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A RADIUS OF 230.00 FEET, AN INCLUDED ANGLE OF 89°21'35"; THENCE CONTINUE SOUTHERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 358.71 FEET; THENCE S 00°01'07" E, 565.48 FEET; THENCE N 89°58'53" E, 20.00 FEET TO THE RADIUS POINT OF A 50 FOOT CUL-DE-SAC AND THE END OF THE CENTERLINE OF SAID 60 FOOT EASEMENT. SAID EASEMENT INCLUDES A CUL-DE-SAC OF 50 FOOT RADIUS CENTERED ON THE ABOVE DEFINED RADIUS POINT WITH THE RETURN OF A 25 FOOT RADIUS AT THE INTERSECTION OF THE 50 FOOT ARC AND THE EAST RIGHT-OF-WAY EASEMENT.

TOGETHER WITH A 20 FOOT ROAD EASEMENT FOR INGRESS, EGRESS AND UTILITIES, THE CENTERLINE OF WITH IS PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE NORTHWEST CORNER OF THE NE 1/4 OF THE NW 1/4 OF THE SE 1/4 OF SAID SECTION 1 AND RUN N 89° 16' 03" E, ALONG THE NORTH LINE THEREOF, 529.90 FEET; THENCE S 00° 19' 13" E, 40.80 FEET TO THE SOUTH RIGHT-OF-WAY OF LITTLE ROAD ACCORDING TO THE PLAT OF RIVERS MANOR UNIT #1, AS RECORDED IN PLAT BOOK 5, PAGE 139, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. RUN THENCE S 00° 19' 13" W, ALONG SAID LINE, 698.13 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23'54"; THENCE SOUTHEASTERLY ALONG THE ARC OF SAID CURVE FOR AN ARC DISTANCE OF 134.07 FEET TO THE POINT OF REVERSE CURVE OF A CURVE TO THE RIGHT HAVING A RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23' 54"; THENCE SOUTHERLY ALONG THE ARC OF SAID CURVE AN ARC DISTANCE OF 134.07 FEET; THENCE S 00°19'13" E, 1336.16 FEET TO THE POINT OF CURVE TO THE RIGHT HAVING A RADIUS OF 230.00 FEET AND AN INCLUDED ANGLE OF 89°39'41" ; THENCE SOUTHWESTERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 359.92 FEET; THENCE S 89°20'28" W, 119.25 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A RADIUS OF 230.00 FEET, AN INCLUDED ANGLE OF 89°21'35"; THENCE CONTINUE SOUTHERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 358.71 FEET; THENCE S 00°01'07" E, 132.44 FEET, SAID POINT ALSO THE POINT OF BEGINNING FOR THE CENTERLINE OF SAID EASEMENT; RUN THENCE S 88°14'08" W, 359.81 FEET TO THE POINT OF TERMINATION OF SAID EASEMENT, LYING 10.00 FOOT ON EACH SIDE.

Columbia County Property Appraiser

DB Last Updated: 1/12/2009

2008 Tax Year

Tax Record | Property Card | Interactive GIS Map | Print

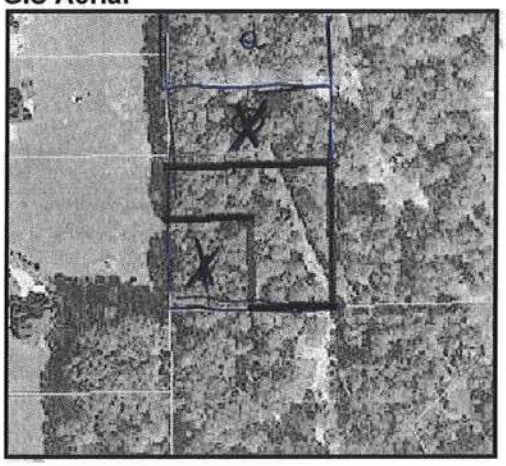
Parcel: 12-5S-16-03585-011

Search Result: 1 of 1

Owner & Property Info

Owner's Name	NICKELSON CHARLES		
Site Address			
Mailing Address	P O BOX 3248 LAKE CITY, FL 32056		
Use Desc. (code)	MOBILE HOM (000200)		
Neighborhood	12516.01	**Tax District**	3
UD Codes	MKTA01	**Market Area**	01
Total Land Area	1.500 ACRES		
Description	COMM NW COR OF NW1/4 OF NE1/4, S 367.24 FT FOR POB, E 331.98 FT, S 291.17 FT, W 165.21 FT, N 186.51 FT, W 165.80 FT TO W LINE OF NE1/4 OF SEC, N 105.31 FT TO POB. ORB 736-554, 801-2400, FJ DIV 998-593, QC 1011-2809. CORR QCD 1035-2075, WD 1035- 2078, SWD 1103-478,482, CWD 1162-213		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (2)	$20,700.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (1)	$22,640.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$43,340.00

Just Value	$43,340.00
Class Value	$0.00
Assessed Value	$43,340.00
Exempt Value	$0.00
Total Taxable Value	$43,340.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
11/13/2008	1162/213	WD	I	U	03	$17,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	MOBILE HME (000800)	1994	Vinyl Side (31)	960	1152	$22,640.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000200	MBL HM (MKT)	1.500 AC	1.00/1.00/1.00/1.00	$13,300.00	$19,950.00
009947	SEPTIC (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	$750.00	$750.00

Brian Kepner

From: Marlin Feagle [leagle@bellsouth.net]
Sent: Wednesday, January 28, 2009 6:23 PM
To: Brian Kepner
Subject: Re: Nickelson Special Family Lot Permit
Attachments: image001.gif

Brian,I think we can accept this,but let Mr.Nichelson know in writing this is being considered a deed from him and he will not be able to use this provision again ,either personally or through any corporate entity he may own, as to this same family member.Also, he should not assume we will approve any such transfers in the future from a corp or similiar entity using the family lot provision.It needs to come from an indiviual not another type entity.

----- Original Message -----
From: Brian Kepner
To: Marlin Feagle
Sent: Wednesday, January 28, 2009 2:10 PM
Subject: Nickelson Special Family Lot Permit

Marlin,

I received this afternoon a letter and a copy of a certificate that shows that Josh Nickelson is the sole share holder of Southeast Developers Group, Inc. As per our conversation yesterday, this should be adequate in order to issue the building permit for his brother Charles. You should have received a copy of the letter sent to Charles Nickelson dated yesterday. Please confirm, thank you.

Brian Kepner
Columbia County
Land Development
Regulation Administrator
386.758.1008
386.758.2160 FAX

SOUTHEAST
DEVELOPERS GROUP

484 NW Turner Ave, Ste 101 – Lake City, FL 32025

P386-955-3497
F866-943-4617

28, January 2009

Brian Kepner, Plans Review
Columbia County Building Dept.
Lake City, FL 32055

RE: Charles Nickelson Residence
Plan Review # 0901-17

Dear Sir,

Please be advised that I, Josh Nickelson, am the sole owner of Southeast Developers Group, Inc. and I own all authorized shares of corporate stock as evidenced by the attached stock certificate.

If there be any further questions please don't hesitate to call.

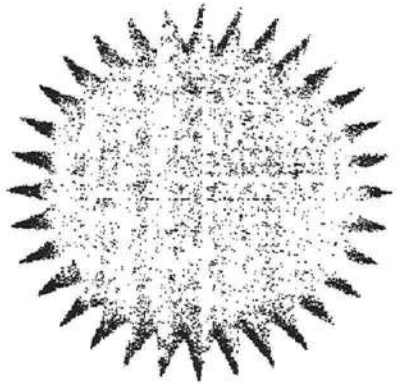
Sincerely,

Joshua A. Nickelson
President, Owner
Southeast Developers Group, Inc.

No. 001 _____ Shares 1000

Southeast Developers Group, Inc.

This Certifies that **Josh Nickelson** is the registered holder of **1000** Shares of the Capital Stock transferable only on the books of the Corporation by the holder hereof in person or by Attorney upon surrender of this Certificate properly endorsed.



In Witness Whereof, the said Corporation has caused this Certificate to be signed by its duly authorized officers and its Corporate Seal is to be hereunto affixed this 4th day of June, A.D. 2004

Electronic Articles of Incorporation For

P04000088478
FILED
June 07, 2004
Sec. Of State
jshivers

SOUTHEAST DEVELOPERS GROUP, INC

The undersigned incorporator, for the purpose of forming a Florida profit corporation, hereby adopts the following Articles of Incorporation:

Article I

The name of the corporation is:

SOUTHEAST DEVELOPERS GROUP, INC

Article II

The principal place of business address:

341 SW RING CT
SUITE 102
LAKE CITY, FL. US 32025

The mailing address of the corporation is:

341 SW RING CT
SUITE 102
LAKE CITY, FL. US 32025

Article III

The purpose for which this corporation is organized is:

ANY AND ALL LAWFUL BUSINESS.

Article IV

The number of shares the corporation is authorized to issue is:

1000

Article V

The name and Florida street address of the registered agent is:

JOSHUA A NICKELSON
341 SW RING CT
SUITE 102
LAKE CITY, FL. 32025

I certify that I am familiar with and accept the responsibilities of registered agent.

P04000088478
FILED
June 07, 2004
Sec. Of State
jshivers

Registered Agent Signature: JOSHUA A NICKELSON

Article VI

The name and address of the incorporator is:

JOSHUA A NICKELSON
341 SW RING CT
SUITE 102
LAKE CITY, FL. 32025

Incorporator Signature: JOSHUA A NICKELSON

Article VII

The initial officer(s) and/or director(s) of the corporation is/are:

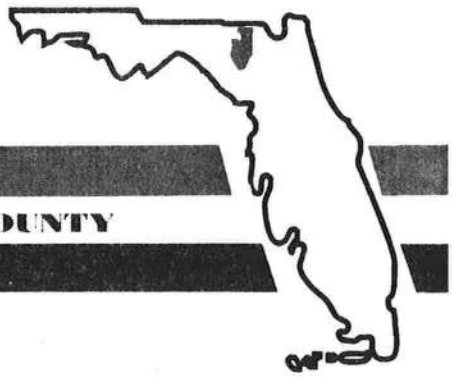
Title: P
JOSHUA A NICKELSON
341 SW RING CT , SUITE 102
LAKE CITY, FL. 32025 US

Title: VP
TREVOR W BLANK
341 SW RING CT, SUITE 102
LAKE CITY, FL. 32025 US

Article VIII

The effective date for this corporation shall be:

06/04/2004



27 January 2009

Charles Nickelson
P.O. Box 3248
Lake City, Florida 32056

RE: Building Permit Application 0901-17, Parcel #12-5S-16-03585-011

Dear Mr. Nickelson:

The above referenced parcel is zoned Agriculture-3 (A-3). Section 14.9 of the Columbia County Land Development Regulations (LDR's), entitled Special Family Lot Permits states as follows;

> A special family lot permit may be issued by the Land Development Regulation Administrator on land zoned Agricultural or Environmentally Sensitive Area within these land development regulations, for the purpose of conveying a lot or parcel to an individual who is the parent, grandparent, sibling, child or adopted child or grandchild of the person who conveyed the parcel to said individual, not to exceed two (2) dwelling units per one (1) acre and the lot complies with all other conditions from permitting development as set forth in these land development regulations. This provision is intended to promote the perpetuation of the family homestead in rural areas by making it possible for family members to reside on lots which exceed maximum density for such areas as their primary residence, provided that the lot complies with the following conditions for permitting:
>
> 1. The division of lots shall be by recorded separate deed and meet all other applicable land development regulations; and
>
> 2. The lot split or subdivision is for the establishment of a homestead of that relative and the lot so conveyed is at least one-half (1/2) acre in size and the remaining lot is at least one-half (1/2) acre in size; and
>
> 3. The family lot permit shall only be issued once for each relative of the parent tract owner. However, for purposes of this provision, if a lot is permitted under this provision to a daughter, for example, and was to be returned to the ownership of the owner of the parent tract, then the original use of this provision to provide the lot to the daughter shall not be counted as one of the one permitted per relative.
>
> 4. The lot complies with all other conditions for permitting and development as set forth in these land development regulations.

The special family lot permit cannot apply, as Southeast Developers Group, Inc. is a corporation and does not have a family. If your brother, Joshua A. Nickelson can provide proof that he is the sole owner and share holder of Southeast Developers Group, Inc. then I have enough documentation to allow the special family lot and building permit to be issued because of the family relationship and no other persons are involved.

If your brother, Joshua A. Nickelson cannot provide proof that he is the sole owner and share holder of Southeast Developers Group, Inc. then in order to meet the requirements of the special family lot. Your property and a minimum of a 1/2 acre of property from Southeast Developers Group, Inc. would need to be deed back to your brother Josh. Your brother then can deed to you the 1.5 acre parcel allowing him to retain a 1/2 acre. Once your brother has deeded you the 1.5 acres he can deed the remainder back to Southeast Developers Group, Inc. The above requested documentation will be required prior to the special family lot and building permit being issued.

If you have any questions concerning this matter, please do not hesitate to contact me at 386.758.1007.

Sincerely,

Brian L. Kepner
Land Development Regulation Administrator,
County Planner

xc: Joshua A. Nickelson, Southeast Developers Group, Inc.
Marlin M. Feagle, County Attorney

FLORIDA DEPARTMENT OF STATE
DIVISION OF CORPORATIONS

Detail by Entity Name

Florida Profit Corporation

SOUTHEAST DEVELOPERS GROUP, INC

Filing Information

Document Number	P04000088478
FEI Number	201227524
Date Filed	06/07/2004
State	FL
Status	ACTIVE
Effective Date	06/04/2004
Last Event	AMENDMENT
Event Date Filed	10/07/2004
Event Effective Date	10/10/2004

Principal Address

484 NW TURNER AVE
SUITE 101
LAKE CITY FL 32055 US

Changed 02/21/2008

Mailing Address

484 NW TURNER AVE
SUITE 101
LAKE CITY FL 32055 US

Changed 02/21/2008

Registered Agent Name & Address

NICKELSON, JOSHUA A
484 NW TURNER AVE
SUITE 101
LAKE CITY FL 32055 US

Address Changed: 02/21/2008

Officer/Director Detail

Name & Address

Title P

NICKELSON, JOSHUA A
484 NW TURNER AVE, STE 101
LAKE CITY FL 32055 US

Title VP

2008 FOR PROFIT CORPORATION ANNUAL REPORT

FILED
Feb 21, 2008
Secretary of State

DOCUMENT# P04000088478

Entity Name: SOUTHEAST DEVELOPERS GROUP, INC

Current Principal Place of Business:

197 SW WATERFORD CT
SUITE 105
LAKE CITY, FL 32025 US

New Principal Place of Business:

484 NW TURNER AVE
SUITE 101
LAKE CITY, FL 32055 US

Current Mailing Address:

197 SW WATERFORD CT
SUITE 105
LAKE CITY, FL 32025 US

New Mailing Address:

484 NW TURNER AVE
SUITE 101
LAKE CITY, FL 32055 US

FEI Number: 20-1227524 **FEI Number Applied For ( )** **FEI Number Not Applicable ( )** **Certificate of Status Desired ( )**

Name and Address of Current Registered Agent:

NICKELSON, JOSHUA A
197 SW WATERFORD CT
SUITE 105
LAKE CITY, FL 32025 US

Name and Address of New Registered Agent:

NICKELSON, JOSHUA A
484 NW TURNER AVE
SUITE 101
LAKE CITY, FL 32055 US

The above named entity submits this statement for the purpose of changing its registered office or registered agent, or both, in the State of Florida.

SIGNATURE: JOSH NICKELSON 02/21/2008
Electronic Signature of Registered Agent — Date

Election Campaign Financing Trust Fund Contribution ( ).

OFFICERS AND DIRECTORS:

Title: P ( ) Delete
Name: NICKELSON, JOSHUA A
Address: 197 SW WATERFORD CT , SUITE 105
City-St-Zip: LAKE CITY, FL 32025 US

Title: VP ( ) Delete
Name: KIRSCH, JACOB C
Address: 1030 SW ROSSBOROUGH CT #101
City-St-Zip: LAKE CITY, FL 32025

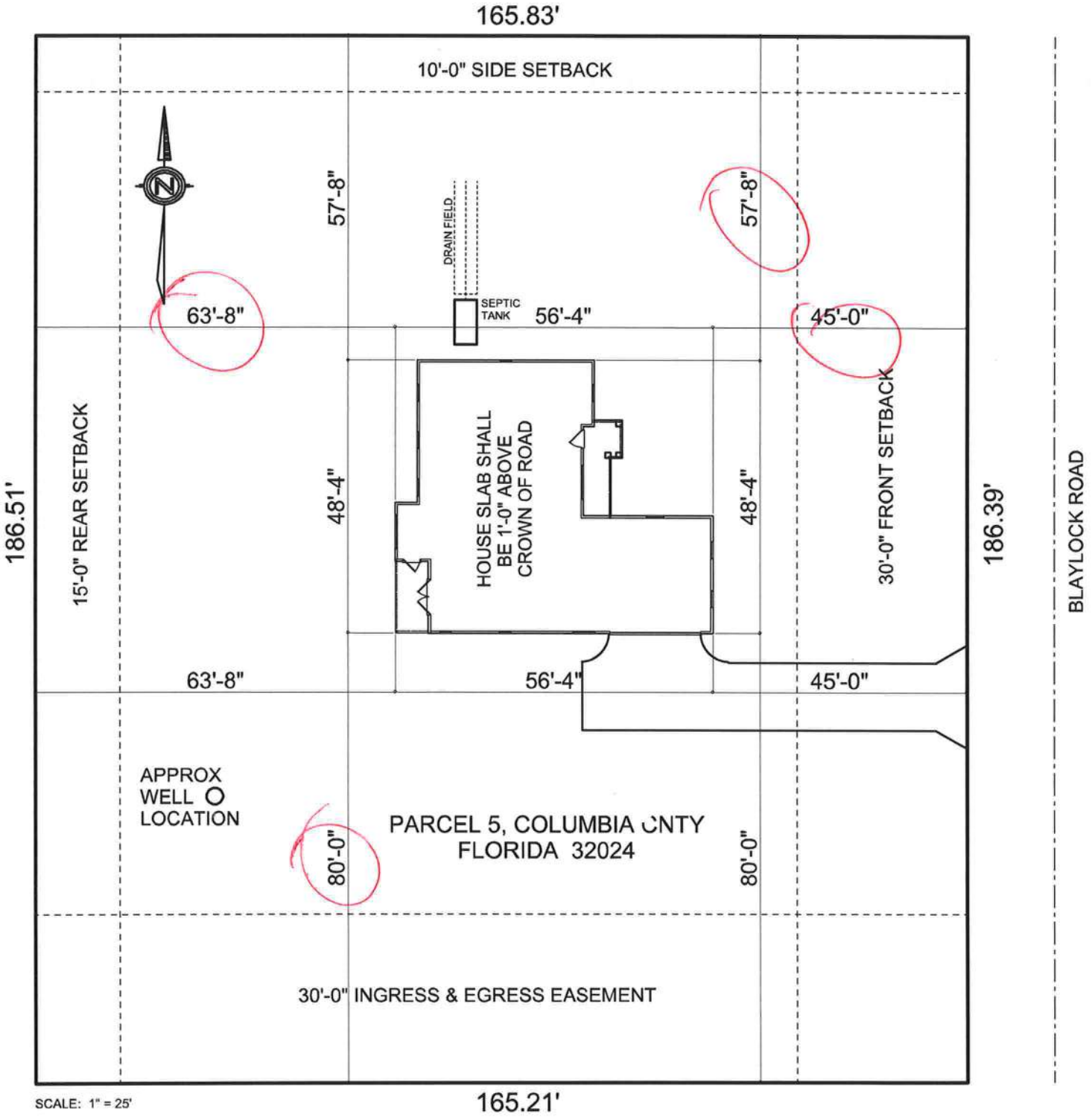
ADDITIONS/CHANGES TO OFFICERS AND DIRECTORS:

Title: P (X) Change ( ) Addition
Name: NICKELSON, JOSHUA A
Address: 484 NW TURNER AVE, STE 101
City-St-Zip: LAKE CITY, FL 32055 US

Title: VP (X) Change ( ) Addition
Name: KIRSCH, JACOB C
Address: PO BOX 1001
City-St-Zip: LAKE CITY, FL 32056

I hereby certify that the information supplied with this filing does not qualify for the exemption stated in Chapter 119, Florida Statutes. I further certify that the information indicated on this report or supplemental report is true and accurate and that my electronic signature shall have the same legal effect as if made under oath; that I am an officer or director of the corporation or the receiver or trustee empowered to execute this report as required by Chapter 607, Florida Statutes; and that my name appears above, or on an attachment with an address, with all other like empowered.

SIGNATURE: JOSH NICKELSON P 02/21/2008
Electronic Signature of Signing Officer or Director — Date



0901.17

This instrument prepared by
& return to
Charles Nickelson
163 SW Stonegate Ter
Lake City, FL 32024
REC:

Inst:200912000698 Date:1/15/2009 Time:3:22 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1165 P:1304

NOTICE OF COMMENCEMENT

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property -.
Parcel ID 03585-011 – Legal description attached as Exhibit "A"

2. General Description improvements – Residential New Construction, Single Family Dwelling

3. Owner Information:
a. Name & Address — Charles Nickelson, 163 SW Stonegate Ter, Lake City, FL 32024

b. Interest in Property — Fee Simple

c. Name & Address of Fee simple title holder (if other than owner) n/a

4. Contractor: — Charles Nickelson, 163 SW Stonegate Ter., Lake City, FL 32024

5. Lender: n/a

6. Additional persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by section 713.13(1)(a)7., Florida Statutes

7. In addition to himself, The owner designates the following persons to receive a copy of the Lienor's Notice as provided in section 713.13(1)(b). Florida Statutes

8. Expiration date of Notice of Commencement is one (1) year from date of recording.

Charles Nickelson

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 15th Day of January , 2009,
By Charles Nickelson who are personally known to me or did provide known as identification.

NOTARY PUBLIC

Name: Jacob C. Kirsch
My Commission Expires: 9/25/09

(NOC)

JACOB C. KIRSCH
MY COMMISSION # DD475233
EXPIRES: Sept. 25, 2009
Florida Notary Service.com
(407) 398-0153

FORM 600A-2004R

EnergyGauge® 4.5.2

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Charles Nickelson**	Builder:	
Address:	**718 SW Meadow Ter**	Permitting Office:	Columbia
City, State:	**Lake City, FL 32024**	Permit Number:	27632
Owner:	**Charles Nickelson**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 31.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	No	c. N/A	
6. Conditioned floor area (ft²)	1500 ft²	13. Heating systems	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		a. Electric Heat Pump	Cap: 31.0 kBtu/hr
a. U-factor: (or Single or Double DEFAULT)	7a. (Dble Default) 199.3 ft²		HSPF: 7.70
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 199.3 ft²	b. N/A	
8. Floor types		c. N/A	
a. Slab-On-Grade Edge Insulation	R=5.0, 175.0(p) ft	14. Hot water systems	
b. N/A		a. Electric Resistance	Cap: 50.0 gallons
c. N/A			EF: 0.90
9. Wall types		b. N/A	
a. Frame, Wood, Exterior	R=13.0, 1020.7 ft²	c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
b. Frame, Wood, Adjacent	R=13.0, 140.0 ft²	15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT,
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 1650.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 40.0 ft		
b. N/A			

Glass/Floor Area: 0.13

Total as-built points: 20875

Total base points: 21632

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 1/9/09

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: 1/9/08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 718 SW Meadow Terr, Lake City, FL, 32024 Permit #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1500.0	18.59	5019.0	1.Double, Clear	W	10.0	8.0	33.3	38.52	0.48	613.0	
				2.Double, Clear	S	14.2	8.0	20.0	35.87	0.46	327.0	
				3.Double, Clear	W	1.5	8.0	75.0	38.52	0.96	2768.0	
				4.Double, Clear	E	1.5	8.0	45.0	42.06	0.96	1812.0	
				5.Double, Clear	S	1.5	8.0	6.0	35.87	0.92	198.0	
				6.Double, Clear	S	1.5	8.0	20.0	35.87	0.92	662.0	
				As-Built Total:				**199.3**			**6380.0**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	140.0	0.70	98.0	1. Frame, Wood, Exterior			13.0	1020.7	1.50		1531.1	
Exterior	1020.7	1.70	1735.2	2. Frame, Wood, Adjacent			13.0	140.0	0.60		84.0	
Base Total:	**1160.7**		**1833.2**	**As-Built Total:**				**1160.7**			**1615.1**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	2.40	48.0	1.Exterior Insulated				20.0	4.10		82.0	
Exterior	20.0	6.10	122.0	2.Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**40.0**		**170.0**	**As-Built Total:**				**40.0**			**114.0**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1500.0	1.73	2595.0	1. Under Attic			30.0	1650.0	1.73 X 1.00		2854.5	
Base Total:	**1500.0**		**2595.0**	**As-Built Total:**				**1650.0**			**2854.5**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	175.0(p)	-37.0	-6475.0	1. Slab-On-Grade Edge Insulation			5.0	175.0(p	-36.20		-6335.0	
Raised	0.0	0.00	0.0									
Base Total:			**-6475.0**	**As-Built Total:**				**175.0**			**-6335.0**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1500.0	10.21	15315.0					1500.0	10.21		15315.0	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 718 SW Meadow Terr, Lake City, FL, 32024 Permit #:

BASE			AS-BUILT						
Summer Base Points: 18457.2			**Summer As-Built Points:**						**19943.6**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
			(sys 1: Central Unit 31000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)						
			19944	1.00	(1.09 x 1.147 x 1.00)	0.260	0.950	6158.7	
18457.2	**0.3250**	**5998.6**	**19943.6**	**1.00**	**1.250**	**0.260**	**0.950**	**6158.7**	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 718 SW Meadow Terr, Lake City, FL, 32024 Permit #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM'	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1500.0	20.17	5446.0	1.Double, Clear	W	10.0	8.0	33.3	20.73	1.19	822.0	
				2.Double, Clear	S	14.2	8.0	20.0	13.30	3.46	919.0	
				3.Double, Clear	W	1.5	8.0	75.0	20.73	1.01	1571.0	
				4.Double, Clear	E	1.5	8.0	45.0	18.79	1.02	862.0	
				5.Double, Clear	S	1.5	8.0	6.0	13.30	1.04	83.0	
				6.Double, Clear	S	1.5	8.0	20.0	13.30	1.04	276.0	
				As-Built Total:				**199.3**			**4533.0**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	140.0	3.60	504.0	1. Frame, Wood, Exterior			13.0	1020.7	3.40		3470.4	
Exterior	1020.7	3.70	3776.6	2. Frame, Wood, Adjacent			13.0	140.0	3.30		462.0	
Base Total:	**1160.7**		**4280.6**	**As-Built Total:**				**1160.7**			**3932.4**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	11.50	230.0	1.Exterior Insulated				20.0	8.40		168.0	
Exterior	20.0	12.30	246.0	2.Adjacent Insulated				20.0	8.00		160.0	
Base Total:	**40.0**		**476.0**	**As-Built Total:**				**40.0**			**328.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1500.0	2.05	3075.0	1. Under Attic			30.0	1650.0	2.05 X	1.00	3382.5	
Base Total:	**1500.0**		**3075.0**	**As-Built Total:**				**1650.0**			**3382.5**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	175.0(p)	8.9	1557.5	1. Slab-On-Grade Edge Insulation			5.0	175.0(p	7.60		1330.0	
Raised	0.0	0.00	0.0									
Base Total:			**1557.5**	**As-Built Total:**				**175.0**			**1330.0**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1500.0	-0.59	-885.0					1500.0	-0.59		-885.0	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 718 SW Meadow Terr, Lake City, FL, 32024 Permit #:

BASE			AS-BUILT					
Winter Base Points:		**13950.1**	**Winter As-Built Points:**					**12620.9**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 31000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
			12620.9	1.000	(1.069 x 1.169 x 1.00)	0.443	0.950	6635.4
13950.1	**0.5540**	**7728.4**	**12620.9**	**1.00**	**1.250**	**0.443**	**0.950**	**6635.4**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: 718 SW Meadow Terr, Lake City, FL, 32024 Permit #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X Multiplier	= Total		Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3	2635.00	7905.0		50.0	0.90	3	1.00	2693.56	1.00	8080.7	
				As-Built Total:							**8080.7**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
5999	**7728**	**7905**	**21632**	**6159**	**6635**	**8081**	**20875**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: 718 SW Meadow Terr, Lake City, FL, 32024 Permit #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.0

The higher the score, the more efficient the home.

718 SW Meadow Terr, Lake City, FL, 32024

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	No	
6.	Conditioned floor area (ft²)	1500 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default)	Area 199.3 ft²
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear)	199.3 ft²
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=5.0, 175.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1020.7 ft²	
b.	Frame, Wood, Adjacent	R=13.0, 140.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1650.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 40.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 31.0 kBtu/hr	
		SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 31.0 kBtu/hr	
		HSPF: 7.70	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 50.0 gallons	
		EF: 0.90	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT,	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Charles Nesbitt Date: 1/9/09

Address of New Home: 718 SW Meadow ter. City/FL Zip: Lake City. FL 32024

GREAT SEAL OF THE STATE OF FLORIDA — IN GOD WE TRUST

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Residential System Sizing Calculation

Summary

Charles Nickelson
718 SW Meadow Ter
Lake City, FL 32024

Project Title:

Code Only
Professional Version
Climate: North

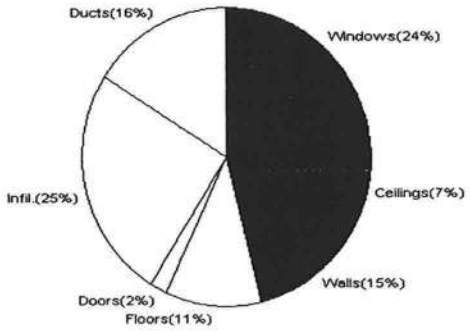
4/1/2008

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**26229**	**Btuh**	**Total cooling load calculation**	**35698**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	118.2	31000	Sensible (SHR = 0.75)	81.9	23250
Heat Pump + Auxiliary(0.0kW)	118.2	31000	Latent	105.8	7750
			Total (Electric Heat Pump)	86.8	31000

WINTER CALCULATIONS

Winter Heating Load (for 1500 sqft)

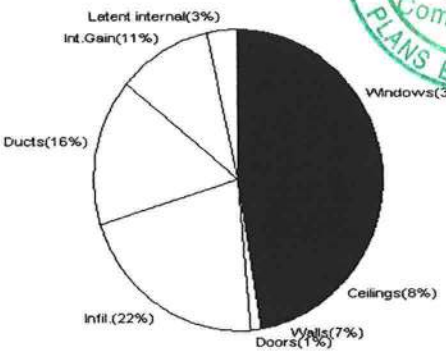
Load component			Load	
Window total	199	sqft	6416	Btuh
Wall total	1161	sqft	3812	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1650	sqft	1944	Btuh
Floor total	175	sqft	2862	Btuh
Infiltration	160	cfm	6481	Btuh
Duct loss			4196	Btuh
Subtotal			**26229**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**26229**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1500 sqft)

Load component			Load	
Window total	199	sqft	11840	Btuh
Wall total	1161	sqft	2340	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1650	sqft	2732	Btuh
Floor total			0	Btuh
Infiltration	140	cfm	2606	Btuh
Internal gain			3780	Btuh
Duct gain			4682	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**28372**	**Btuh**
Latent gain(ducts)			1010	Btuh
Latent gain(infiltration)			5116	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**7326**	**Btuh**
TOTAL HEAT GAIN			**35698**	**Btuh**



Version 8
For Florida residences only

EnergyGauge® System Sizing
PREPARED BY: _____
DATE: 1/9/09

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Charles Nickelson
718 SW Meadow Ter
Lake City, FL 32024

Project Title:

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

4/1/2008

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	33.3	32.2	1073 Btuh
2	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	W	75.0	32.2	2414 Btuh
4	2, Clear, Metal, 0.87	E	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
6	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
	Window Total		199(sqft)		6416 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1021	3.3	3352 Btuh
2	Frame - Wood - Adj(0.09)	13.0	140	3.3	460 Btuh
	Wall Total		1161		3812 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1650	1.2	1944 Btuh
	Ceiling Total		1650		1944Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	175.0 ft(p)	16.4	2862 Btuh
	Floor Total		175		2862 Btuh
			Envelope Subtotal:		15552 Btuh
Infiltration	Type	ACH X	Volume(cuft)	walls(sqft)	CFM=
	Natural	0.80	12000	1161	160.0	6481 Btuh
Ductload				(DLM of 0.190)	4196 Btuh
All Zones			**Sensible Subtotal All Zones**		**26229 Btuh**

WHOLE HOUSE TOTALS

	Subtotal Sensible	26229 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	26229 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Charles Nickelson
718 SW Meadow Ter
Lake City, FL 32024

Project Title:

Code Only
Professional Version
Climate: North

4/1/2008

EQUIPMENT

1. Electric Heat Pump	#	31000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Version 8
For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Charles Nickelson
718 SW Meadow Ter
Lake City, FL 32024

Project Title:

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

4/1/2008

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	33.3	32.2	1073 Btuh
2	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	W	75.0	32.2	2414 Btuh
4	2, Clear, Metal, 0.87	E	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	S	6.0	32.2	193 Btuh
6	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
	Window Total		199(sqft)		6416 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1021	3.3	3352 Btuh
2	Frame - Wood - Adj(0.09)	13.0	140	3.3	460 Btuh
	Wall Total		1161		3812 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1650	1.2	1944 Btuh
	Ceiling Total		1650		1944Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	175.0 ft(p)	16.4	2862 Btuh
	Floor Total		175		2862 Btuh
			Zone Envelope Subtotal:		15552 Btuh

Infiltration	Type	ACH X	Volume(cuft)	walls(sqft)	CFM=	
	Natural	0.80	12000	1161	160.0	6481 Btuh
Ductload	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic) (DLM of 0.190)					4196 Btuh
Zone #1	**Sensible Zone Subtotal**					**26229 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Charles Nickelson
718 SW Meadow Ter
Lake City, FL 32024

Project Title:

Code Only
Professional Version
Climate: North

4/1/2008

WHOLE HOUSE TOTALS

	Subtotal Sensible	26229 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	26229 Btuh

EQUIPMENT

1. Electric Heat Pump	#	31000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Version 8
For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Charles Nickelson
718 SW Meadow Ter
Lake City, FL 32024

Project Title:

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

4/1/2008

Component Loads for Whole House

	Type*		Overhang		Window Area(sqft)			HTM		Load
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
1	2, Clear, 0.87, None,N,N	W	10ft.	8ft.	33.3	33.3	0.0	29	80	965 Btuh
2	2, Clear, 0.87, None,N,N	S	14.1	8ft.	20.0	20.0	0.0	29	34	579 Btuh
3	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	75.0	0.0	75.0	29	80	5964 Btuh
4	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	45.0	0.0	45.0	29	80	3578 Btuh
5	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	6.0	6.0	0.0	29	34	174 Btuh
6	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	20.0	20.0	0.0	29	34	579 Btuh
	Window Total				199 (sqft)					11840 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1020.7	2.1	2129 Btuh
2	Frame - Wood - Adj	13.0/0.09	140.0	1.5	211 Btuh
	Wall Total		1161 (sqft)		2340 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Exterior	20.0	9.8	196 Btuh
2	Insulated - Adjacent	20.0	9.8	196 Btuh
	Door Total	40 (sqft)		392 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1650.0	1.7	2732 Btuh
	Ceiling Total		1650 (sqft)		2732 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	5.0	175 (ft(p))	0.0	0 Btuh
	Floor Total		175.0 (sqft)		0 Btuh

			Envelope Subtotal:		17304 Btuh

Infiltration	Type	ACH	Volume(cuft)	wall area(sqft)	CFM=	Load
	SensibleNatural	0.70	12000	1161	160.0	2606 Btuh

Internal gain	Occupants	Btuh/occupant		Appliance	Load
	6	X 230	+	2400	3780 Btuh

	Sensible Envelope Load:		23690 Btuh
Duct load	(DGM of 0.198)		4682 Btuh
	Sensible Load All Zones		**28372 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Charles Nickelson
718 SW Meadow Ter
Lake City, FL 32024

Project Title:

Code Only
Professional Version
Climate: North

4/1/2008

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**23690 Btuh**
	Sensible Duct Load	4682 Btuh
	Total Sensible Zone Loads	**28372 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**28372 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	5116 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1010 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**7326 Btuh**
	TOTAL GAIN	**35698 Btuh**

EQUIPMENT

1. Central Unit	#	31000 Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

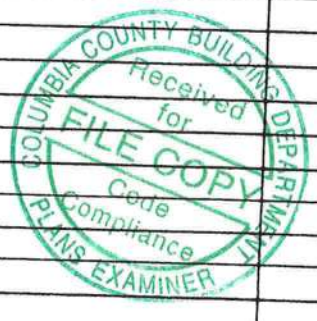
Version 8
For Florida residences only

PRODUCT APPROVAL SPECIFICATION SHEET

Location:_____ **Project Name:**_____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	Jeld-wen	Exterior door	FL-495-R1
2. Sliding			
3. Sectional			
4. Roll up	Raynor	Garage door	FL-4867
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	MI Products	SH	FL-5108
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding	James Hardie	Hardi-Plank	FL-989-R1
2. Soffits	Kaycan	Aluminum	FL-495-7
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles	Elk	Asphalt - Architectual	FL-586-R2
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			



Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13 Liquid Applied Roof Sys			
14 Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1 Accordion			
2 Bahama			
3 Storm Panels			
4 Colonial			
5. Roll-up			
6 Equipment			
7. Others			
F. SKYLIGHTS			
1 Skylight			
2 Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor	Simpson	Strongs	FL - 474. R1
2. Truss plates			
3 Engineered lumber			
4 Railing			
5. Coolers-freezers			
6 Concrete Admixtures			
7. Material			
8 Insulation Forms			
9. Plastics			
10. Deck-Roof			
11 Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products. the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with. 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Charles Nickelson

Contractor or Contractor's Authorized Agent Signature

Charles Nickelson 1/9/09

Print Name Date

Location

Permit # (FOR STAFF USE ONLY)

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

Website:

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 12/26/2009 **DATE ISSUED:** 1/7/2009

ENHANCED 9-1-1 ADDRESS:

718 SW MEADOW TER

LAKE CITY FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

12-5S-16-03585-011

Remarks:

Address Issued By: _____

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

1351

0901.17

COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Office 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT
FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

(✓) Single Family Dwelling ( ) Two-Family Residence ( ) Farm Outbuilding
( ) Other_____ ( ) Addition, Alteration, Modification or other Improvement

I Charles Nickelsen, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss 489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number 0901-17

Charles Nickelsen 1/14/09
Owner Builder Signature Date

FLORIDA NOTARY
The above signer is personally known to me or produced identification _____
Notary Signature Jacob C. Kirsch Date 1/15/09

FOR BUILDING DEPARTMENT USE ONLY
I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date_____ Building Official/Representative_____

NOTARY PUBLIC STATE OF FLORIDA (407) 398-0153
JACOB C. KIRSCH
MY COMMISSION # DD475233
EXPIRES: Sept. 25, 2009
Bonded Thru Notary Public Underwriters

0901.17

Exhibit "A"

A part of the NW ¼ of the NE ¼ of Section 12, Township 5 South, Range 16 East, more particularly described as follows: Commence at the NW corner of said NW ¼ of the NE ¼ and run S 00 degrees 02' 51" E, along the West line of the NE ¼ of said Section 12, a distance of 367.24 feet to the Point of Beginning; thence N 89 degrees 20'28" E, a distance of 331.98 feet; thence S 00 degrees 02'51" E, 291.17 feet; thence S 89 degrees 17'43" W, a distance of 165.21 feet, thence N 00 degrees 02'51" W, a distance of 186.51 feet, thence N 89 degrees 20'28" E, a distance of 165.80 feet to the West line of the NE ¼ of Section 12; thence N 00 degrees 20'51" W, along said West line a distance of 105.31 feet, to the Point of Beginning.

TOGETHER WITH AND SUBJECT TO A 60 FOOT ROAD EASEMENT FOR INGRESS, EGRESS AND UTILITIES, THE CENTERLINE OF WITCH IS MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE NORTHWEST CORNER OF THE NW 1/4 OF THE NE 1/4 OF SAID SECTION 12 AND RUN N 89° 20' 28" E. ALONG THE SOUTH LINE OF SOUTHWOOD MEADOWS A SUBDIVISION RECORDED IN PLAT BOOK IN PLAT BOOK 6, PAGE 49, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, A DISTANCE OF 662.35 FEET: THENCE S 00°01'07" E, 292.63 FEET, TO POINT "A" ALSO THE POINT OF BEGINNING FOR THE CENTERLINE OF SAID EASEMENT; RUN THENCE S 89°14'08" W, 330.33 FEET TO POINT "B"; FROM SAID POINT "B" THENCE N 00° 06' 38" E , 266.88 FEET TO THE POINT OF TERMINATION OF SAID EASEMENT; ALSO FROM SAID POINT "B" THENCE S 00° 06' 38" W A DISTANCE OF 392.08 FEET TO THE POINT OF TERMINATION. SAID EASEMENT LYING 30 FEET TO RIGHT AND 30 FEET TO LEFT OF SAID CENTERLINE.

0901.17

This instrument prepared by
& return to
Charles Nickelson
163 SW Stonegate Ter
Lake City, FL 32024
REC:

Inst 200912000696 Date 1/15/2009 Time 3:22 PM
DC,P DeWitt Cason Columbia County Page 1 of 2 B 1165 P 1304

NOTICE OF COMMENCEMENT

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property -.
Parcel ID 03585-011 - Legal description attached as Exhibit "A"

2. General Description improvements Residential New Construction, Single Family Dwelling

3. Owner Information:
   a. Name & Address

   Charles Nickelson
   163 SW Stonegate Ter
   Lake City, FL 32024

   b. Interest in Property Fee Simple

   c. Name & Address of Fee simple title holder (if other than owner) n/a

4. Contractor:

   Charles Nickelson
   163 SW Stonegate Ter.
   Lake City, FL 32024

5. Lender: n/a

6. Additional persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by section 713.13(1)(a)7., Florida Statutes

7. In addition to himself, The owner designates the following persons to receive a copy of the Lienor's Notice as provided in section 713.13(1)(b). Florida Statutes

8. Expiration date of Notice of Commencement is one (1) year from date of recording.

Charles Nickelson [signature]
Charles Nickelson

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 15th Day of January, 2009. By Charles Nickelson who are personally known to me or did provide Personal as identification.

NOTARY PUBLIC

[signature]
Name: Jacob C Kirsch
My Commission Expires: 9/25/09

(NOC)

JACOB C. KIRSCH
MY COMMISSION # DD475233
EXPIRES: Sept. 25, 2009
(407) 398-0153 Florida Notary Service.com
NOTARY PUBLIC STATE OF FLORIDA

CORPORATE WARRANTY DEED

Made this NOVEMBER 13, 2008 A.D. By **SOUTHEAST DEVELOPERS GROUP, INC. a Florida corporation**, whose post office address is: 484 NW Turner Avenue, Lake City, Florida 32055, hereinafter called the grantor, to **Charles Nickelson, a married man**, whose post office address is: P. O. Box 3248, Lake City, Florida 32024, hereinafter called the grantee:

(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth, that the grantor, for and in consideration of the sum of Ten Dollars, ($10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

SEE EXHIBIT "A" ATTACHED HERETO AND BY THIS REFERENCE MADE A PART HEREOF.

Said property is not the homestead of the Grantor(s) under the laws and constitution of the State of Florida in that neither Grantor(s) or any members of the household of Grantor(s) reside thereon.

Parcel ID Number: **Part of 03585-009**

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31, 2007.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Kathy R. Lord
Witness Printed Name Kathy R. Lord

Katrina Vercher
Witness Printed Name Katrina Vercher

SOUTHEAST DEVELOPERS GROUP, INC.

BY: JOSHUA A. NICKELSON
ITS: PRESIDENT

State of Florida
County of Columbia

The foregoing instrument was acknowledged before me this 13th day of NOVEMBER, 2008, by Joshua A. Nickelson, as President of SOUTHEAST DEVELOPERS GROUP, INC., a FLORIDA corporation, who is/are personally known to me or who has produced a Drivers License as identification.

F Voncile Dow
Notary Public
Print Name: F Voncile Dow

My Commission Expires: 10/03/2010

F. VONCILE DOW
MY COMMISSION # DD 592031
EXPIRES: October 3, 2010
Bonded Thru Notary Public Underwriters

Inst:200812020570 Date:11/13/2008 Time:2:12 PM
Doc Stamp-Deed:119.00
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1162 P:213

PREPARED BY
JOSHUA A. NICKELSON
484 NW Turner AVENUE
Lake City, FL 32055

Exhibit "A"

A part of the NW ¼ of the NE ¼ of Section 12, Township 5 South, Range 16 East, more particularly described as follows: Commence at the NW corner of said NW ¼ of the NE ¼ and run S 00 degrees 02' 51" E, along the West line of the NE ¼ of said Section 12, a distance of 367.24 feet to the Point of Beginning; thence N 89 degrees 20'28" E, a distance of 331.98 feet; thence S 00 degrees 02'51" E, 291.17 feet; thence S 89 degrees 17'43" W, a distance of 165.21 feet, thence N 00 degrees 02'51" W, a distance of 186.51 feet, thence N 89 degrees 20'28" E, a distance of 165.80 feet to the West line of the NE ¼ of Section 12; thence N 00 degrees 20'51" W, along said West line a distance of 105.31 feet, to the Point of Beginning.

TOGETHER WITH AND SUBJECT TO A 60 FOOT ROAD EASEMENT FOR INGRESS, EGRESS AND UTILITIES, THE CENTERLINE OF WITCH IS MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE NORTHWEST CORNER OF THE NW 1/4 OF THE NE 1/4 OF SAID SECTION 12 AND RUN N 89° 20' 28" E, ALONG THE SOUTH LINE OF SOUTHWOOD MEADOWS A SUBDIVISION RECORDED IN PLAT BOOK IN PLAT BOOK 6, PAGE 49, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, A DISTANCE OF 662.35 FEET: THENCE S 00°01'07" E, 260.63 FEET, TO POINT "A" ALSO THE POINT OF BEGINNING FOR THE CENTERLINE OF SAID EASEMENT; RUN THENCE S 88°14'08" W, 330.33 FEET TO POINT "B"; FROM SAID POINT "B" THENCE N 00° 08' 38" E , 266.88 FEET TO THE POINT OF TERMINATION OF SAID EASEMENT; ALSO FROM SAID POINT "B" THENCE S 00° 08' 38" W A DISTANCE OF 392.08 FEET TO THE POINT OF TERMINATION. SAID EASEMENT LYING 30 FEET TO RIGHT AND 30 FEET TO LEFT OF SAID CENTERLINE.

0901.17

AFFIDAVIT OF SUBDIVIDED REAL PROPERTY FOR USE OF IMMEDIATE FAMILY MEMBERS FOR PRIMARY RESIDENCE

STATE OF FLORIDA
COUNTY OF COLUMBIA

BEFORE ME the undersigned Notary Public personally appeared.

Josh ~~Charles~~ Nickelson, the Owner of the parent tract which has been subdivided for immediate family primary residence use, hereinafter the Owner, and Charles Nickelson, the family member of the Owner, who is the owner of the family parcel which is intended for immediate family primary residence use, hereafter the Family Member, and is related to the Owner as Siblings, and both individuals being first duly sworn according to law, depose and say:

1. Both the Owner and the Family Member have personal knowledge of all matters set forth in this Affidavit.

2. The Owner holds fee simple title to certain real property situated in Columbia County, and more particularly described by reference to the Columbia county Property Appraiser Tax Parcel No. 03585-009.

3. The Owner has divided his parent parcel for use of immediate family members for their primary residence and the parcel divided and the remaining parent parcel are at least ½ acre in size. Immediate family is defined as grandparent, parent, step-parent, adopted parent, sibling, child, step-child, adopted child or grandchild.

4. The Family Member is a member of the Owner's immediate family, as set forth above, and holds fee simple title to certain real property divided from the Owner's parcel situated in Columbia County and more particularly described by reference to the Columbia County Property Appraiser Tax Parcel No. 03585-011.

5. No person or entity other than the Owner and Family Member claims or is presently entitled to the right of possession or is in possession of the property, and there are no tenancies, leases or other occupancies that affect the Property.

6. This Affidavit is made for the specific purpose of inducing Columbia County to recognize a family division for a family member on the parcel divided in accordance with Section 14.9 of the Columbia County Land Development Regulations.

7. This Affidavit is made and given by Affiants with full knowledge that the facts contained herein are accurate and complete, and with full knowledge that the penalties under Florida law for perjury include conviction of a felony of the third degree.

We Hereby Certify that the information contained in this Affidavit are true and correct.

Owner

Family Member

Josh Nickelson
Typed or Printed Name

Charles Nickelson
Typed or Printed Name

Subscribed and sworn to (or affirmed) before me this 15th day of Jan, 2009, by Josh Nickelson (Owner) who is personally known to me or has produced known as identification.

Notary Public

JACOB C. KIRSCH
MY COMMISSION # DD475233
EXPIRES: Sept. 25, 2009
(407) 398-0153 Florida Notary Service.com

Subscribed and sworn to (or affirmed) before me this 15th day of Jan, 2009, by Charles Nickelson (Family Member) who is personally known to me or has produced known as identification.

Notary Public

JACOB C. KIRSCH
MY COMMISSION # DD475233
EXPIRES: Sept. 25, 2009
(407) 398-0153 Florida Notary Service.com

18.50
.70

SPECIAL WARRANTY DEED

THIS INDENTURE, made this 28th day of November, 2006, between TREVOR BLANK, who does not reside on the property, whose address is Post Office Box 3713, Lake City, Florida 32056-3713, Grantor, and JOSHUA NICKELSON, whose address is 197 SW Waterford Court, Suite 105, Lake City, Florida 32025, Grantee,

W I T N E S S E T H:

That Grantor, for and in consideration of the sum of TEN AND NO/100 ($10.00) DOLLARS, and other good and valuable considerations to Grantor in hand paid by Grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to Grantee and Grantee's heirs, successors and assigns forever, all of his undivided interest in the following described land, situate and lying in COLUMBIA County, Florida:

SEE SCHEDULE A ATTACHED HERETO

(Tax parcel number 12-5S-[redacted] - cutout)

SUBJECT TO: Taxes for 2006 and subsequent years; restrictions and easements of record; easements shown by the plat of the property; and existing mortgage indebtedness on the property, if any.

Grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons claiming by, through, or under Grantor.

IN WITNESS WHEREOF, Grantor has hereunto set his hand and seal the day and year above written.

Signed, sealed and delivered
in the presence of:

Print Name: Eddie M. Anderson

Print Name: Julie Calloway
Witnesses as to Grantor

TREVOR BLANK

This Instrument Prepared By:
EDDIE M. ANDERSON, P.A.
P. O. Box 1179
Lake City, Florida 32056-1179

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 28th day of November, 2006, by TREVOR BLANK. He is personally known to me or he produced FDL as identification.

(Notarial Seal)

Notary Public State of Florida
Julie Calloway
My Commission DD501123
Expires 01/23/2010

Notary Public
My commission expires: 1-23-2010

Inst:2006028025 Date:11/28/2006 Time:16:35
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1103 P:478

4

SCHEDULE A to Warranty Deed

Blank to Nickelson

SHOWING TRACT "B" : THE WEST HALF OF LOT 3, RIVERS MANOR, UNIT 2, AN UNRECORDED SUBDIVISION IN THE SE 1/4 OF SECTION 1, AND THE NW 1/4 OF SECTION 12, TOWNSHIP 5 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA.

A PART OF THE NW 1/4 OF THE NE 1/4 OF SECTION 12, TOWNSHIP 5 SOUTH, RANGE 16 EAST, MORE PARTICULARLY DESCRIBED AS FOLLOWS: BEGIN AT THE NW CORNER OF SAID NW 1/4 OF THE NE 1/4 AND RUN N 89°20'28" E, ALONG THE NORTH LINE OF NW 1/4 OF THE NE 1/4, 333.21 FEET; THENCE S 00°08'38" W, 658.93 FEET; THENCE S 89°17'43" W, 331.01 FEET TO THE WEST LINE OF THE NE 1/4 OF SAID SECTION 12; THENCE N 00°02'51" W, 659.17 FEET TO THE POINT OF BEGINNING. CONTAINING 5.02 ACRES MORE OR LESS.

SUBJECT TO AN EASEMENT OVER AND ACROSS THE SOUTH 10.00 FEET AND THE EAST 15.00 FEET FOR UTILITIES AND DRAINAGE.

TOGETHER WITH A 60 FOOT ROAD EASEMENT FOR INGRESS, EGRESS AND UTILITIES IN THE EAST 1/2 OF SECTION 1 AND 12 OF TOWNSHIP 5 SOUTH, RANGE 16 EAST, THE CENTERLINE OF WITH IS PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE NORTHWEST CORNER OF THE NE 1/4 OF THE NW 1/4 OF THE SE 1/4 OF SAID SECTION 1 AND RUN N 89° 16' 03" E, ALONG THE NORTH LINE THEREOF, 529.90 FEET; THENCE S 00° 19' 13" E, 40.80 FEET TO THE SOUTH RIGHT-OF-WAY OF LITTLE ROAD ACCORDING TO THE PLAT OF RIVERS MANOR UNIT #1, AS RECORDED IN PLAT BOOK 5, PAGE 139, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. SAID POINT ALSO THE POINT OF BEGINNING FOR THE CENTERLINE OF SAID EASEMENT; THENCE S 00° 19' 13" W, ALONG SAID CENTERLINE, 698.13 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A CENTERLINE RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23'54"; THENCE SOUTHEASTERLY ALONG THE ARC OF SAID CURVE FOR AN ARC DISTANCE OF 134.07 FEET TO THE POINT OF REVERSE CURVE OF A CURVE TO THE RIGHT HAVING A RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23' 54"; THENCE SOUTHERLY ALONG THE ARC OF SAID CURVE AN ARC DISTANCE OF 134.07 FEET; THENCE S 00°19'13" E, 1336.16 FEET TO THE POINT OF CURVE TO THE RIGHT HAVING A CENTERLINE RADIUS OF 230.00 FEET AND AN INCLUDED ANGLE OF 89°39'41" ; THENCE SOUTHWESTERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 359.92 FEET; THENCE S 89°20'28" W, 119.25 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A RADIUS OF 230.00 FEET, AN INCLUDED ANGLE OF 89°21'35"; THENCE CONTINUE SOUTHERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 358.71 FEET; THENCE S 00°01'07" E, 565.48 FEET; THENCE N 89°58'53" E, 20.00 FEET TO THE RADIUS POINT OF A 50 FOOT CUL-DE-SAC AND THE END OF THE CENTERLINE OF SAID 60 FOOT EASEMENT. SAID EASEMENT INCLUDES A CUL-DE-SAC OF 50 FOOT RADIUS CENTERED ON THE ABOVE DEFINED RADIUS POINT WITH THE RETURN OF A 25 FOOT RADIUS AT THE INTERSECTION OF THE 50 FOOT ARC AND THE EAST RIGHT-OF-WAY EASEMENT.

TOGETHER WITH A 20 FOOT ROAD EASEMENT FOR INGRESS, EGRESS AND UTILITIES, THE CENTERLINE OF WITH IS PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE NORTHWEST CORNER OF THE NE 1/4 OF THE NW 1/4 OF THE SE 1/4 OF SAID SECTION 1 AND RUN N 89° 16' 03" E, ALONG THE NORTH LINE THEREOF, 529.90 FEET; THENCE S 00° 19' 13" E, 40.80 FEET TO THE SOUTH RIGHT-OF-WAY OF LITTLE ROAD ACCORDING TO THE PLAT OF RIVERS MANOR UNIT #1, AS RECORDED IN PLAT BOOK 5, PAGE 139, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. RUN THENCE S 00° 19' 13" W, ALONG SAID LINE, 698.13 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23'54"; THENCE SOUTHEASTERLY ALONG THE ARC OF SAID CURVE FOR AN ARC DISTANCE OF 134.07 FEET TO THE POINT OF REVERSE CURVE OF A CURVE TO THE RIGHT HAVING A RADIUS OF 230.0 FEET AND AN INCLUDED ANGLE OF 33° 23' 54"; THENCE SOUTHERLY ALONG THE ARC OF SAID CURVE AN ARC DISTANCE OF 134.07 FEET; THENCE S 00°19'13" E, 1336.16 FEET TO THE POINT OF CURVE TO THE RIGHT HAVING A RADIUS OF 230.00 FEET AND AN INCLUDED ANGLE OF 89°39'41" ; THENCE SOUTHWESTERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 359.92 FEET; THENCE S 89°20'28" W, 119.25 FEET TO A POINT OF CURVE OF A CURVE TO THE LEFT HAVING A RADIUS OF 230.00 FEET, AN INCLUDED ANGLE OF 89°21'35"; THENCE CONTINUE SOUTHERLY ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 358.71 FEET; THENCE S 00°01'07" E, 132.44 FEET, SAID POINT ALSO THE POINT OF BEGINNING FOR THE CENTERLINE OF SAID EASEMENT; RUN THENCE S 88°14'08" W, 359.81 FEET TO THE POINT OF TERMINATION OF SAID EASEMENT, LYING 10.00 FOOT ON EACH SIDE.

Inst:2006028025 Date:11/28/2006 Time:16:35
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1103 P:479

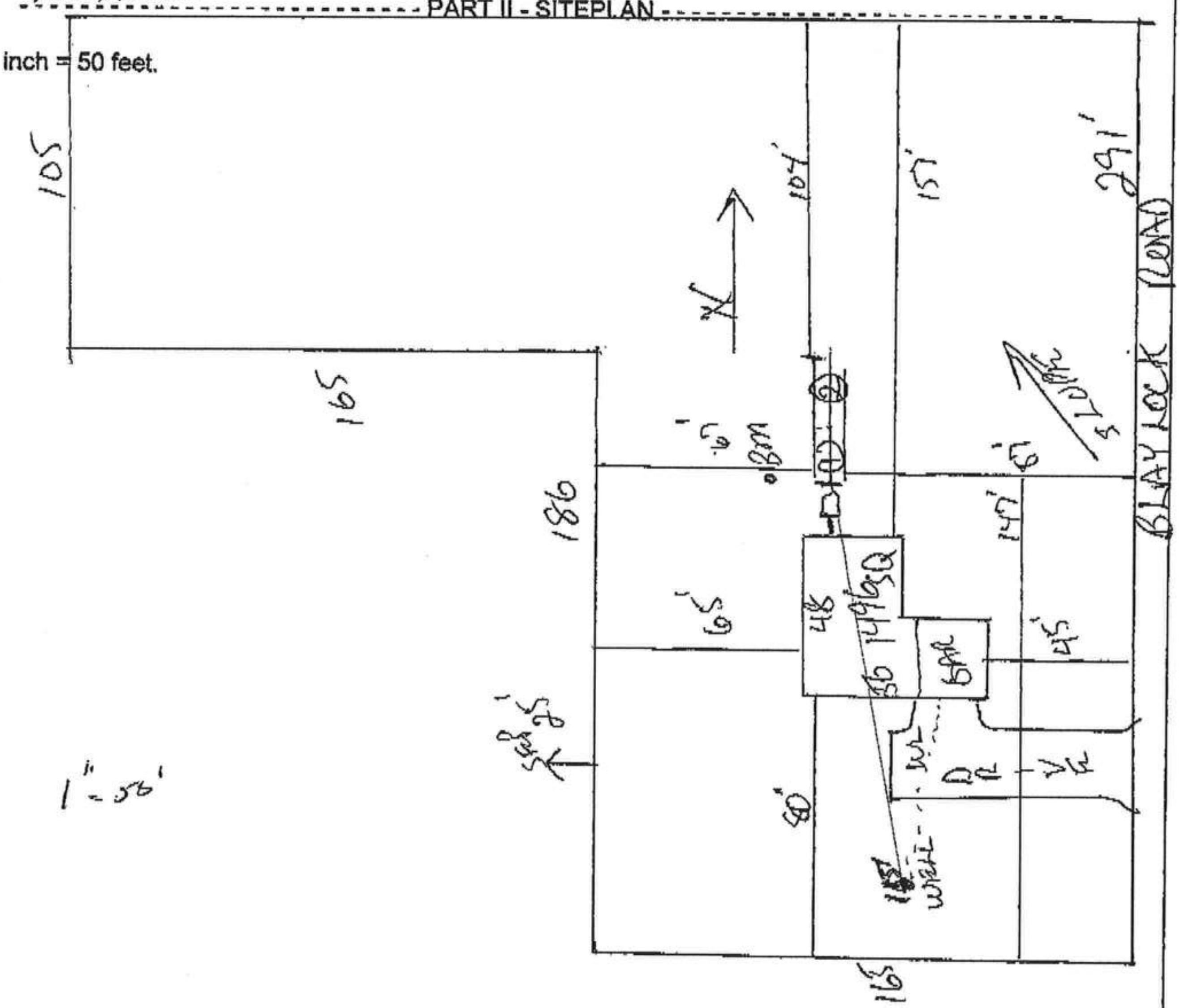
STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Nickelson 03585-011 331'

Permit Application Number 09-0062

PART II - SITEPLAN

Scale: 1 inch = 50 feet.



Notes: ______________________________

Site Plan submitted by: ____________ MASTER CONTRACTOR

Plan Approved ✓ Not Approved_____ Date 2-11-09

By____________ Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4016 which may be used)
(Stock Number: 5744-002-4015-6)

REVISED 2/11/09

09-0062

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 904601
DATE PAID: 1/30/08
FEE PAID: $ 1000
RECEIPT #: 1095978

LC

APPLICATION FO
[X] New Syst [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Nickelson, Charles

AGENT: ROCKY FORD, A & B CONSTRUCTION TELEPHONE: 386-497-2311

MAILING ADDRESS: P.O. BOX 39 FT. WHITE, FL, 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: na BLOCK: na SUB: na PLATTED: N/A

PROPERTY ID #: 12-5S-16-03538-~~010~~ 011 ZONING: N/A I/M OR EQUIVALENT: [Y / (N)]

PROPERTY SIZE: 1.90 ACRES WATER SUPPLY: [X] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [Y / (N)] DISTANCE TO SEWER: ____ FT

PROPERTY ADDRESS: SW Blaylock Road, Lake City, FL

DIRECTIONS TO PROPERTY: 47 South, TL on Walter Road, TL on Little Road, Follow around to Southwood Meadows, TR on Meadow Terr, To end.. go through gate combo is 2662, 3rd drive on right

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	SF Residential	3	1496	
2				
3				

[N] Floor/Equipment Drains [N] Other (Specify) _____

SIGNATURE: Rocky D Ford DATE: 1/7/2009

DH 4015, 10/97 (Previous Editions May Be Used)

FAX

To:	Building Dept	**From:**	Jake Kirsch
Fax:	386-758-2160	**Pages:**	2
Phone:		**Date:**	2.25.2009
Re:	Compaction Test Permit # 27632	**CC:**	

[X] Urgent [ ] For Review [ ] Please Comment [ ] Please Reply [ ] Please Recycle

Comments:

ATTN: Please find attached compaction Test for Permit # 27632

Thanks

Jake Kirsch

2.17.2009

Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456

4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 09-00087-01
DATE TESTED: 2/18/09
DATE REPORTED: 2/20/09

PROJECT:	Nickelson Residence, Lake City, FL
CLIENT:	Charles Nickelson, 718 SW Blalock Ct., Lake City, FL 32025
GENERAL CONTRACTOR:	Jake Kirsh
EARTHWORK CONTRACTOR:	Jake Kirsh
INSPECTOR:	Chad Day

ASTM METHOD	**SOIL USE**
(D-2922) Nuclear	BUILDING FILL

SPECIFIED REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft³)	MOISTURE PERCENT	DRY DENSITY (lb/ft³)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
1		SE Corner of Pad, 10' West x 10' North	12"	118.5	7.0	110.7	1	107.0	104%
2		SW Corner of Pad, 12' North x 10' East	12"	122.1	10.3	110.7	1	107.0	103%
3		NE Corner of Pad, 12' South x 12' West	12"	119.4	7.8	110.8	1	107.0	104%
4		NE Corner of Footing, 20' West	12"	117.2	9.4	107.1	1	107.0	100%
5		NW Corner of Footing, 20' South	12"	117.6	9.8	107.1	1	107.0	100%
6		SE Corner of Footing, 10' West	12"	118.0	10.2	107.1	1	107.0	100%
7		NE Corner of Footing, 20' South	12"	117.0	8.9	107.4	1	107.0	100%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft³)	OPT. MOIST.	TYPE
1		Tannish Gray Fine Sand	107.0	14.0	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Reviewed By:

Linda M. Creamer
President - CEO

Date:
Licensed, Florida No: 57842

ee

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company. 27632

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: P.O. Box 1795 City Lake City State FL Zip 32056
Company Business License No. JB103476 Company Phone No. 386-755-3611 • 352-494-5751
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Charles Nicholson Company Phone No. 965-6906

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 718 SW Meadow Terr. Lake City, FL 32024

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☒ Other Monolithic
Approximate Depth of Footing: Outside 11 Inside / Type of Fill /

Section 4: Treatment Information

Date(s) of Treatment(s) 2/26/09
Brand Name of Product(s) Used Bifen XTS
EPA Registration No. 53883-189
Approximate Final Mix Solution % .6%
Approximate Size of Treatment Area: Sq. ft. 2104 Linear ft. / Linear ft. of Masonry Voids /
Approximate Total Gallons of Solution Applied 210 gals.
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) S. Gregory Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature [signature] Date 2/26/09

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Charles Nielsen
Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Charles Nielsen
Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 9 **day of** Jan. **20** 09.

Personally known ✓ **or Produced Identification**_____

Jacob C. Kirsch
State of Florida Notary Signature (For the Contractor)

SEAL:

NOTARY PUBLIC STATE OF FLORIDA
(407) 398-0153
JACOB C. KIRSCH
MY COMMISSION # DD475233
EXPIRES: Sept. 25, 2009
Florida Notary Service.com

JHN: WEEG1, E

Columbia County Building Department
Culvert Waiver

Culvert Waiver No.
000001707

DATE: 02/13/2009 BUILDING PERMIT NO. 27632

APPLICANT CHARLES NICKELSON PHONE 386.965.6906

ADDRESS POB 3248 LAKE CITY FL 32056-3248

OWNER CHARLES NICKELSON PHONE 386.965.6906

ADDRESS 718 SW SOUTHWOOD MEADOW TERRACE LAKE CITY FL 32024

CONTRACTOR CHARLES NICKELSON PHONE 386.965.6906

LOCATION OF PROPERTY 47-S TO SOUTHWOOD EST,TL TO LITTLE,TL TO SOUTHWOOD MEADOWS,TR PAST CUL-DE-SAC,2ND LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT

PARCEL ID # 12-6S-16-03586-011

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Charles Nickelson

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

_____ APPROVED _____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: PRivate Road

SIGNED: Rocky Ford DATE: 2-20-09

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

JHN: WEEBIE

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000001707

DATE: 02/13/2009 BUILDING PERMIT NO. 27632

APPLICANT CHARLES NICKELSON PHONE 386.965.6906

ADDRESS POB 3248 LAKE CITY FL 32056-3248

OWNER CHARLES NICKELSON PHONE 386.965.6906

ADDRESS 718 SW SOUTHWOOD MEADOW TERRACE LAKE CITY FL 32024`

CONTRACTOR CHARLES NICKELSON PHONE 386.965.6906

LOCATION OF PROPERTY 47-S TO SOUTHWOOD EST,TL TO LITTLE,TL TO SOUTHWOOD MEADOWS,TR

PAST CUL-DE-SAC,2ND LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT ___ ___ ___ ___

PARCEL ID # 12-5S-16-03585-011

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: √ [signature]

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

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SIGNED: Rocky Gitch DATE: 2-20-09

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

COLUMBIA COUNTY GATEWAY TO FLORIDA

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 12-5S-16-03585-011 **Building permit No.** 000027632

Use Classification SFD/UTILITY **Fire:** 0.00

Permit Holder CHARLES NICKELSON **Waste:** 0.00

Owner of Building CHARLES NICKELSON **Total:** 0.00

Location: 718 SW MEADOW TERR., LAKE CITY, FL

Date: 11/10/2009

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Builders FirstSource

HLAKMF

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

Project Information:

Builder: [redacted]
Model: THE CHARLES MODEL
Builders FirstSource Job #: 296128
Street: 718 .SW Meadow Terrace
City: Lake City
County: Columbia
Building Code: FBC2004/TPI2002
Computer Program Used: MiTek 6.3

Builders FirstSource
2525 E. Duval St.
Lake City, FL 32055

1109 COASTAL BAY
BOYNTON BCH, FL.33435
ELLECTRONICALLY SEAL
IN ACCORDANCE TO
SS.668.001-668.006

Truss Design Information:

Gravity Loads
Roof: 32 psf Total
Floor: 55 psf Total

Wind
Wind Standard: ASCE 7-02
Wind Speed: 110 mph
Mean Roof Ht: 20 ft
Exposure: B

Note: Refer to individual truss design drawings for special loading conditions, design criteria, truss geometry, lumber, and plate information.

Design Professional Information:

Design Professional Of Record: [redacted]
License # : CE[redacted]75
Delegated Truss Engineer: Julius Lee
License # : 34869

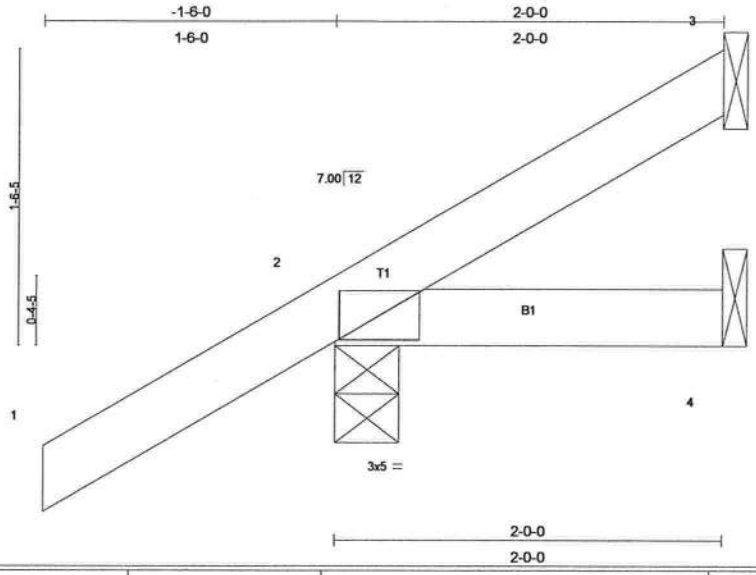
This truss specification package consists of this index sheet and 47 truss design drawings. This signed and sealed index sheet indicates acceptance of my professional engineering responsibility solely for listed truss design drawings. The suitability and use of each truss component for any particular building is the responsibility of the building designer per TPI.

Truss #	Truss Label	Drawing #	Seal Date	Truss #	Truss Label	Drawing #	Seal Date	Truss #	Truss Label	Drawing #	Seal Date
1	EJ2	296128001	1/13/2009	31	T05	296128031	1/13/2009				
2	EJ2A	296128002	1/13/2009	32	T06	296128032	1/13/2009				
3	EJ7	296128003	1/13/2009	33	T07	296128033	1/13/2009				
4	EJ7A	296128004	1/13/2009	34	T08	296128034	1/13/2009				
5	EJ7B	296128005	1/13/2009	35	T09	296128035	1/13/2009				
6	EJ7C	296128006	1/13/2009	36	T10	296128036	1/13/2009				
7	EJ7D	296128007	1/13/2009	37	T11	296128037	1/13/2009				
8	EJ7E	296128008	1/13/2009	38	T12	296128038	1/13/2009				
9	EJ7F	296128009	1/13/2009	39	T13	296128039	1/13/2009				
10	EJ7G	296128010	1/13/2009	40	T14	296128040	1/13/2009				
11	HJ2	296128011	1/13/2009	41	T15	296128041	1/13/2009				
12	HJ3	296128012	1/13/2009	42	T16	296128042	1/13/2009				
13	T01	296128013	1/13/2009	43	T17	296128043	1/13/2009				
14	T01A	296128014	1/13/2009	44	T18	296128044	1/13/2009				
15	T01B	296128015	1/13/2009	45	T18G	296128045	1/13/2009				
16	T01C	296128016	1/13/2009	46	T19	296128046	1/13/2009				
17	T01G	296128017	1/13/2009	47	T20	296128047	1/13/2009				
18	T02	296128018	1/13/2009								
19	T02A	296128019	1/13/2009								
20	T02B	296128020	1/13/2009								
21	T02C	296128021	1/13/2009								
22	T02D	296128022	1/13/2009								
23	T02E	296128023	1/13/2009								
24	T02F	296128024	1/13/2009								
25	T02G	296128025	1/13/2009								
26	T02H	296128026	1/13/2009								
27	T02I	296128027	1/13/2009								
28	T02J	296128028	1/13/2009								
29	T03	296128029	1/13/2009								
30	T04	296128030	1/13/2009								

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	EJ2	MONO TRUSS	3	1	296128001
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jan 13 09:02:36 2009 Page 1



LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 9 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=178/0-4-0, 4=10/Mechanical, 3=20/Mechanical
Max Horz 2=109(load case 6)
Max Uplift2=-145(load case 6), 3=-20(load case 7)
Max Grav 2=178(load case 1), 4=29(load case 2), 3=22(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-45/11
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 2 and 20 lb uplift at joint 3.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	EJ7	MONO TRUSS	9	1	296128003
					Job Reference (optional)

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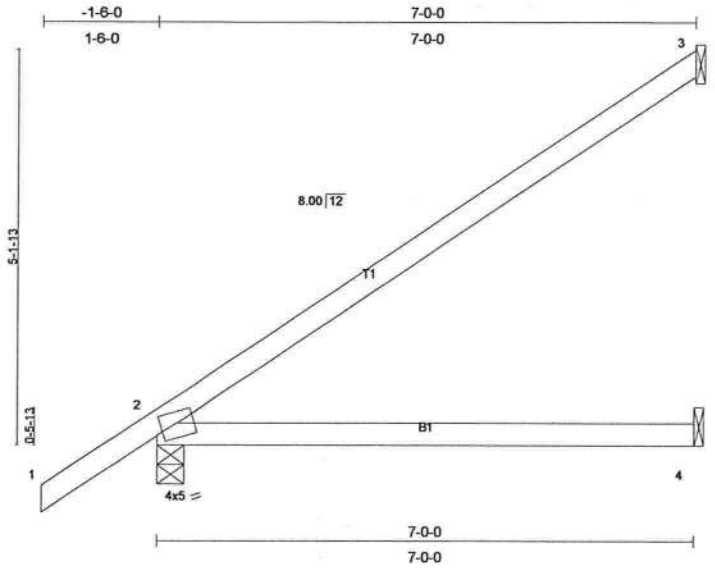

Plate Offsets (X,Y): [2:0-0-14,0-1-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.10	2-4	>834	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.17	2-4	>478	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=156/Mechanical, 2=318/0-4-0, 4=49/Mechanical
Max Horz 2=196(load case 6)
Max Uplift3=-109(load case 6), 2=-87(load case 6)
Max Grav 3=156(load case 1), 2=318(load case 1), 4=95(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-138/69
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.73

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 109 lb uplift at joint 3 and 87 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	EJ7B	MONO HIP	1	1	296128005
					Job Reference (optional)

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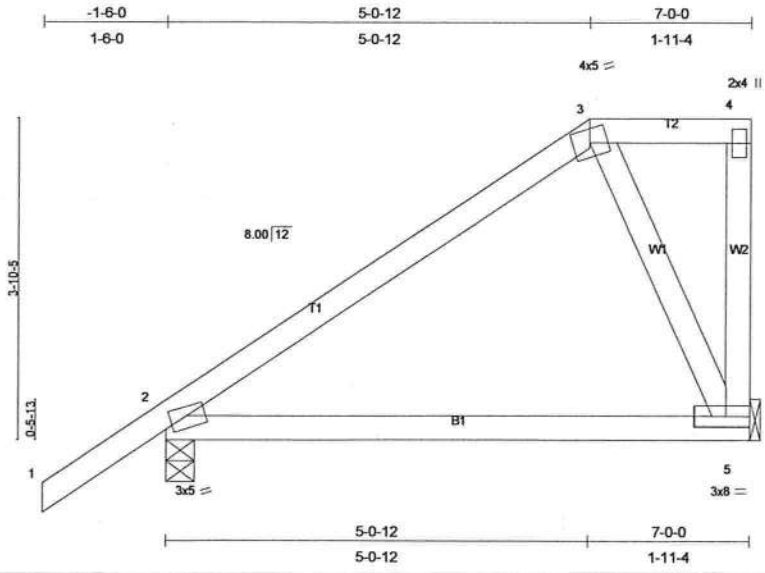

Plate Offsets (X,Y): [2:0-0-14,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.05	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.08	2-5	>962	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=203/Mechanical, 2=315/0-4-0
Max Horz 2=157(load case 6)
Max Uplift5=-61(load case 5), 2=-108(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-193/26, 3-4=-32/0, 4-5=-28/41
BOT CHORD 2-5=-96/96
WEBS 3-5=-188/227

JOINT STRESS INDEX
2 = 0.79, 3 = 0.48, 4 = 0.31 and 5 = 0.56

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 5 and 108 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted]7 THE CHARLES
296128	EJ7D	MONO HIP	1	1	296128007
					Job Reference (optional)

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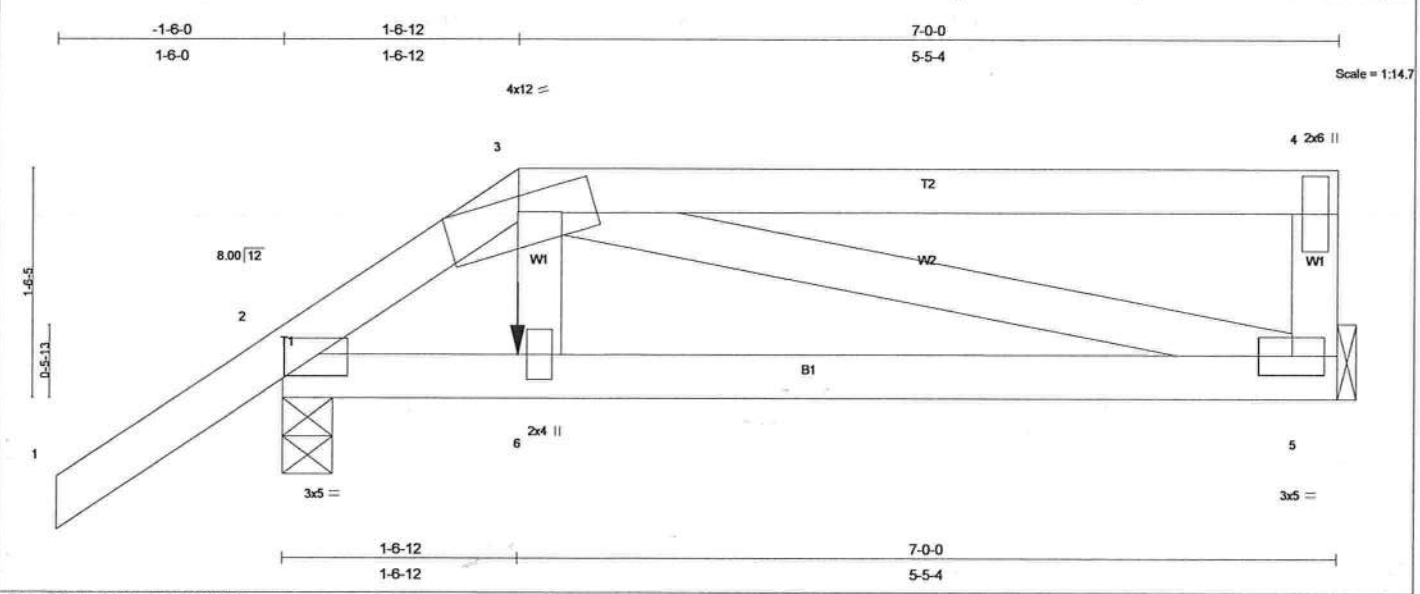

Plate Offsets (X,Y): [2:0-5-3,0-0-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.01	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.02	5-6	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=217/Mechanical, 2=333/0-4-0
Max Horz 2=82(load case 5)
Max Uplift5=-78(load case 3), 2=-124(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-306/76, 3-4=-135/51, 4-5=-155/85
BOT CHORD 2-6=-88/246, 5-6=-83/246
WEBS 3-6=-4/129, 3-5=-114/43

JOINT STRESS INDEX
2 = 0.65, 3 = 0.49, 4 = 0.72, 5 = 0.42 and 6 = 0.09

NOTES (9)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 5 and 124 lb uplift at joint 2.
6) Girder carries hip end with 0-0-0 right side setback, 1-6-12 left side setback, and 2-4-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 14 lb down and 9 lb up at 1-6-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-57(F=-3), 2-6=-10, 5-6=-11(F=-1)
Concentrated Loads (lb)
Vert: 6=-14(F)

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	EJ7F	MONO HIP	1	1	296128009
					Job Reference (optional)

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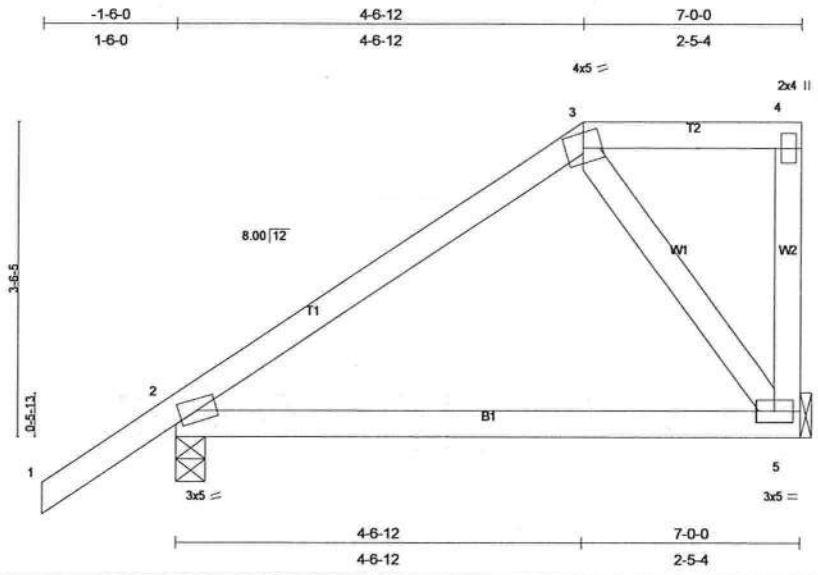

Plate Offsets (X,Y): [2:0-0-14,0-0-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.04	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.08	2-5	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=203/Mechanical, 2=315/0-4-0
Max Horz 2=146(load case 6)
Max Uplift5=-61(load case 5), 2=-111(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-203/53, 3-4=-35/0, 4-5=-33/43
BOT CHORD 2-5=-108/109
WEBS 3-5=-164/197

JOINT STRESS INDEX
2 = 0.80, 3 = 0.33, 4 = 0.24 and 5 = 0.57

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 5 and 111 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	HJ2	JACK	1	1	296128011
					Job Reference (optional)

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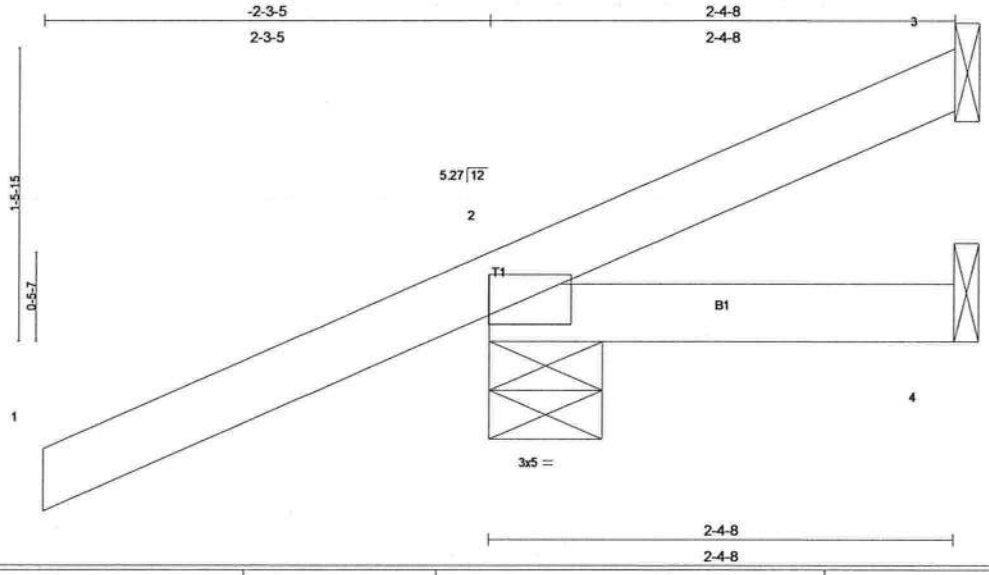

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.38	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-56/Mechanical, 2=248/0-6-15, 4=6/Mechanical
Max Horz 2=74(load case 5)
Max Uplift3=-56(load case 1), 2=-245(load case 5)
Max Grav 3=77(load case 5), 2=248(load case 1), 4=26(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-53/38
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 3 and 245 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-6(F=24, B=24)-to-3=-45(F=5, B=5), 2=0(F=5, B=5)-to-4=-8(F=1, B=1)

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T01	COMMON	8	1	296128013
					Job Reference (optional)

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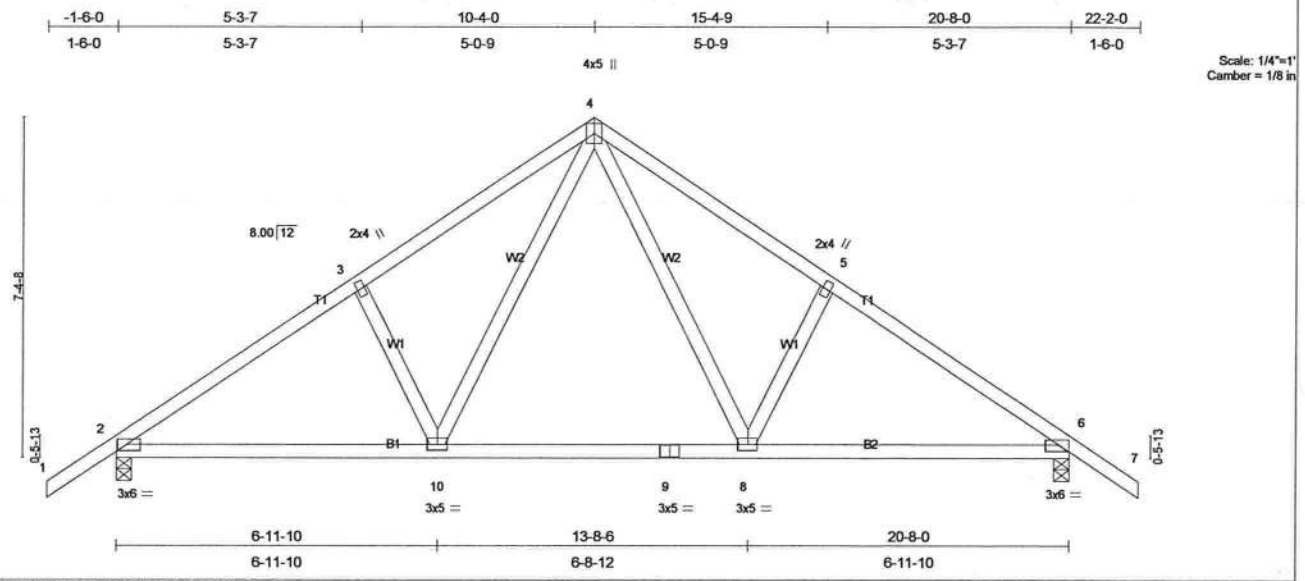

Plate Offsets (X,Y): [2:0-6-3,0-0-3], [6:0-6-3,0-0-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.15	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.27	8-10	>894	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.30	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=943/0-4-0, 6=943/0-4-0
Max Horz 2=194(load case 5)
Max Uplift2=-265(load case 6), 6=-265(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1299/571, 3-4=-1168/627, 4-5=-1168/627, 5-6=-1299/571, 6-7=0/40
BOT CHORD 2-10=-315/988, 9-10=-107/689, 8-9=-107/689, 6-8=-315/988
WEBS 3-10=-197/206, 4-10=-276/531, 4-8=-276/531, 5-8=-197/206

JOINT STRESS INDEX
2 = 0.70, 3 = 0.34, 4 = 0.66, 5 = 0.34, 6 = 0.70, 8 = 0.46, 9 = 0.62 and 10 = 0.46

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 2 and 265 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T01B	COMMON	2	1	296128015
					Job Reference (optional)

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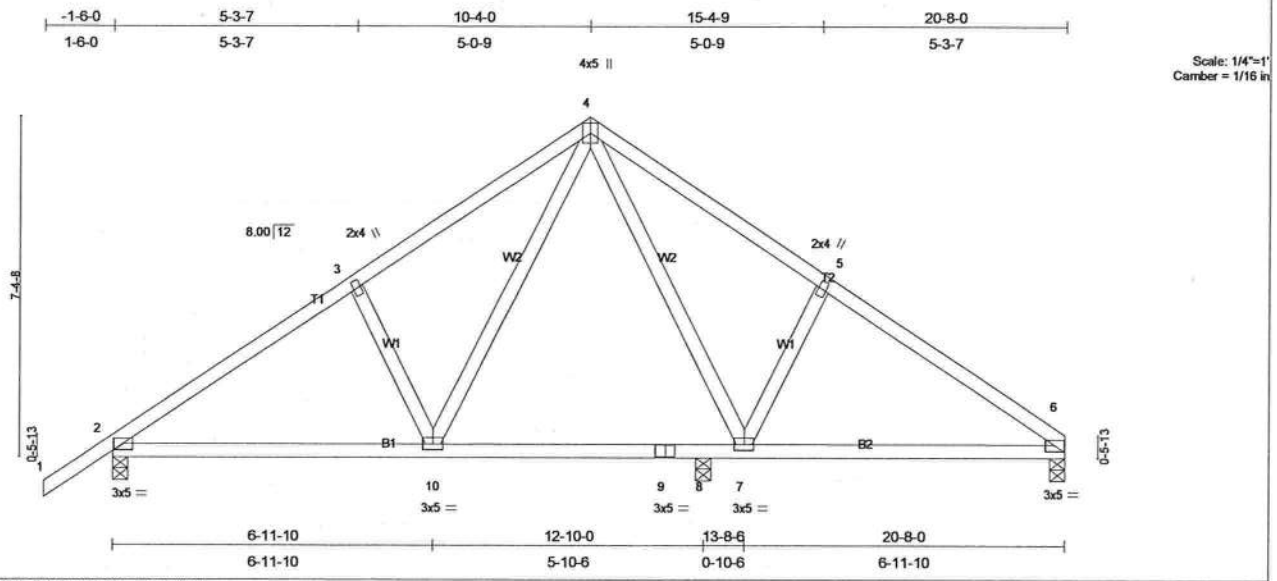

Plate Offsets (X,Y): [2:0-5-3,0-0-3], [6:0-5-3,0-0-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.06	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.12	6-7	>778	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=442/0-4-0, 2=702/0-4-0, 8=600/0-4-0
Max Horz 2=212(load case 5)
Max Uplift6=-96(load case 7), 2=-206(load case 6), 8=-144(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-867/385, 3-4=-740/443, 4-5=-450/325, 5-6=-556/263
BOT CHORD 2-10=-221/641, 9-10=-44/358, 8-9=-44/358, 7-8=-44/358, 6-7=-131/410
WEBS 3-10=-221/219, 4-10=-246/477, 4-7=-233/145, 5-7=-251/241

JOINT STRESS INDEX
2 = 0.66, 3 = 0.34, 4 = 0.60, 5 = 0.34, 6 = 0.66, 7 = 0.46, 9 = 0.59 and 10 = 0.46

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 6, 206 lb uplift at joint 2 and 144 lb uplift at joint 8.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T01G	GABLE	1	1	296128017
					Job Reference (optional)

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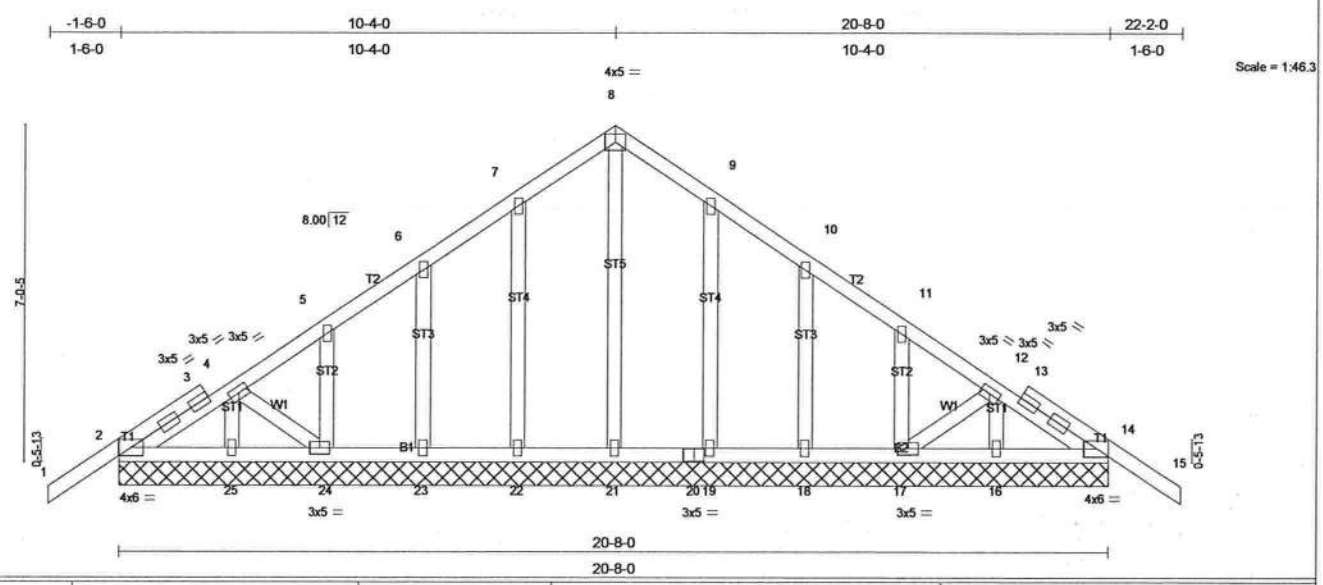

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	15	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	15	n/r	90		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.00	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=188/20-8-0, 14=188/20-8-0, 21=109/20-8-0, 22=128/20-8-0, 23=127/20-8-0, 24=137/20-8-0, 25=108/20-8-0, 19=128/20-8-0, 18=127/20-8-0, 17=137/20-8-0, 16=109/20-8-0
Max Horz 2=237(load case 5)
Max Uplift2=-113(load case 6), 14=-148(load case 7), 22=-97(load case 6), 23=-106(load case 6), 24=-158(load case 6), 25=-8(load case 5), 19=-95(load case 7), 18=-107(load case 7), 17=-174(load case 7)
Max Grav 2=188(load case 1), 14=188(load case 1), 21=130(load case 7), 22=132(load case 10), 23=127(load case 1), 24=139(load case 10), 25=108(load case 1), 19=132(load case 11), 18=127(load case 1), 17=139(load case 11), 16=109(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-164/155, 3-4=-156/164, 4-5=-154/130, 5-6=-104/124, 6-7=-54/138, 7-8=-48/181, 8-9=-48/178, 9-10=-48/115, 10-11=-48/45, 11-12=-74/50, 12-13=-26/34, 13-14=-65/25, 14-15=0/39
BOT CHORD 2-25=-57/116, 24-25=-57/116, 23-24=-32/190, 22-23=-32/190, 21-22=-32/190, 20-21=-32/190, 19-20=-32/190, 18-19=-32/190, 17-18=-32/190, 16-17=-8/97, 14-16=-8/97
WEBS 8-21=-118/0, 7-22=-112/109, 6-23=-108/119, 5-24=-111/120, 4-25=-91/23, 9-19=-112/107, 10-18=-108/120, 11-17=-111/119, 12-16=-91/2, 4-24=-13/99, 12-17=-28/113

JOINT STRESS INDEX
2 = 0.68, 3 = 0.00, 3 = 0.18, 3 = 0.18, 4 = 0.51, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.28, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.51, 13 = 0.00, 13 = 0.18, 13 = 0.18, 14 = 0.68, 16 = 0.34, 17 = 0.40, 18 = 0.34, 19 = 0.34, 20 = 0.18, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.40 and 25 = 0.34

NOTES (10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 2, 148 lb uplift at joint 14, 97 lb uplift at joint 22, 106 lb uplift at joint 23, 158 lb uplift at joint 24, 8 lb uplift at joint 25, 95 lb uplift at joint 19, 107 lb uplift at joint 18 and 174 lb uplift at joint 17.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T02A	MONO HIP	1	1	296128019
					Job Reference (optional)

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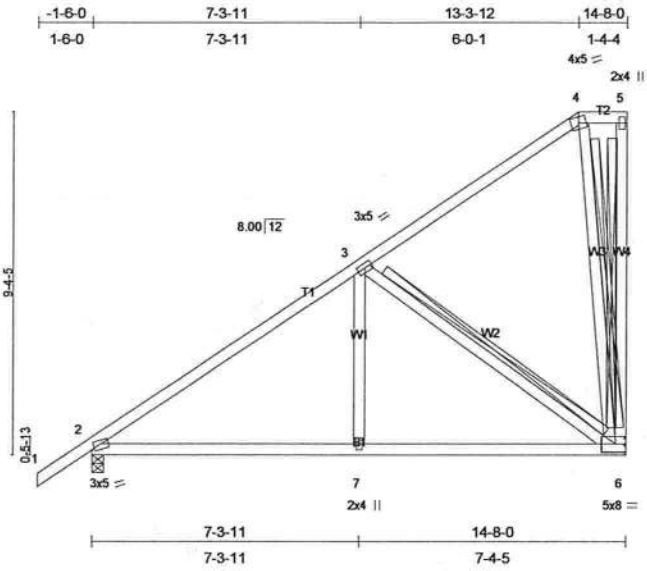

Plate Offsets (X,Y): [2:0-0-14,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.06	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.10	2-7	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-6, 3-6, 4-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 6=454/Mechanical, 2=555/0-4-0
Max Horz 2=333(load case 6)
Max Uplift6=-200(load case 6), 2=-110(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-570/24, 3-4=-142/38, 4-5=-9/0, 5-6=-71/49
BOT CHORD 2-7=-300/381, 6-7=-300/381
WEBS 3-7=0/251, 3-6=-423/327, 4-6=-235/252

JOINT STRESS INDEX
2 = 0.77, 3 = 0.23, 4 = 0.56, 5 = 0.23, 6 = 0.13 and 7 = 0.18

NOTES (6-7)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 200 lb uplift at joint 6 and 110 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T02C	SPECIAL	2	1	296128021
					Job Reference (optional)

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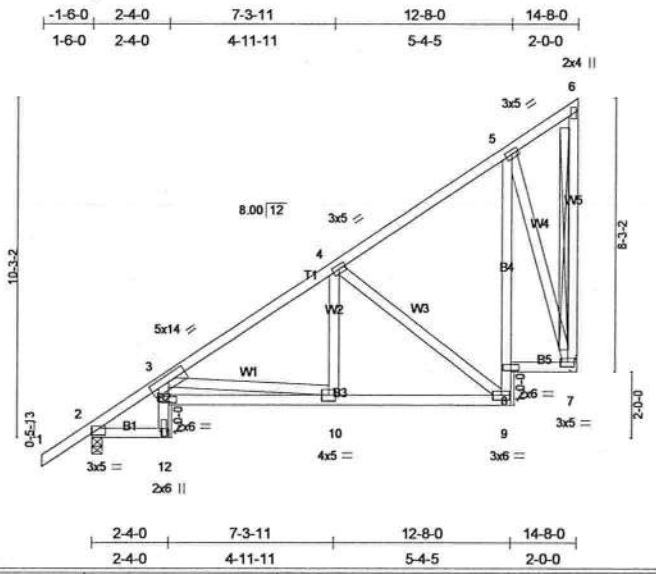

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.10	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.12	10-11	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 7=454/Mechanical, 2=555/0-4-0
Max Horz 2=358(load case 6)
Max Uplift7=-230(load case 6), 2=-94(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-670/49, 3-4=-600/82, 4-5=-230/0, 5-6=-51/9, 6-7=-21/20
BOT CHORD 2-12=-386/443, 11-12=-41/89, 3-11=-8/146, 10-11=-965/1133, 9-10=-357/444, 8-9=-190/277, 5-8=-140/260, 7-8=-95/119
WEBS 3-10=-693/612, 4-10=-56/241, 4-9=-409/329, 5-7=-396/317

JOINT STRESS INDEX
2 = 0.63, 3 = 0.80, 4 = 0.22, 5 = 0.44, 6 = 0.07, 7 = 0.24, 8 = 0.51, 9 = 0.64, 10 = 0.24, 11 = 0.87 and 12 = 0.80

NOTES (5-6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 7 and 94 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
6) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T02E	SPECIAL	2	1	296128023 Job Reference (optional)

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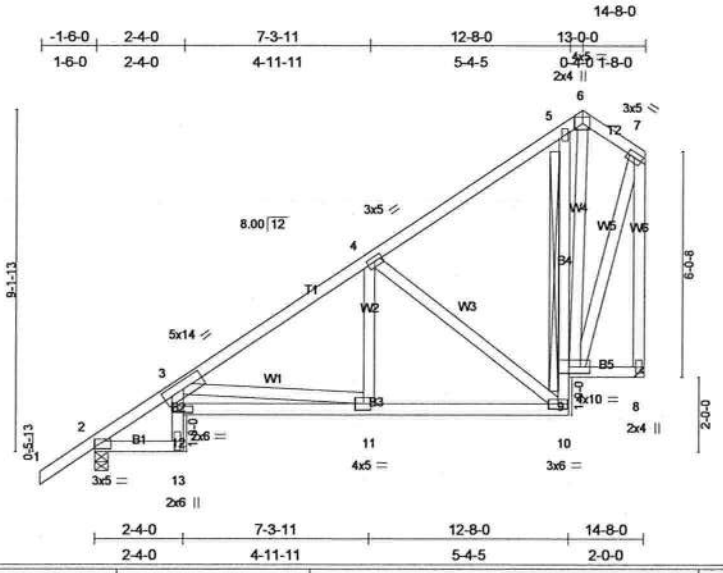

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	0.10	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.12	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-5-4 oc bracing: 11-12.
T-Brace: 2 X 4 SYP No.3 - 5-9

REACTIONS (lb/size) 2=555/0-4-0, 8=454/Mechanical
Max Horz 2=305(load case 6)
Max Uplift2=-126(load case 6), 8=-180(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-670/134, 3-4=-600/173, 4-5=-230/65, 5-6=-174/168, 6-7=-142/104, 7-8=-396/268
BOT CHORD 2-13=-365/443, 12-13=-36/89, 3-12=-6/146, 11-12=-919/1131, 10-11=-343/445, 9-10=-187/278, 5-9=-187/187, 8-9=-3/5
WEBS 3-11=-691/580, 4-11=-52/241, 4-10=-410/326, 6-9=-124/146, 7-9=-215/320

JOINT STRESS INDEX
2 = 0.63, 3 = 0.80, 4 = 0.22, 5 = 0.28, 6 = 0.22, 7 = 0.25, 8 = 0.23, 9 = 0.61, 10 = 0.65, 11 = 0.24, 12 = 0.87 and 13 = 0.80

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 2 and 180 lb uplift at joint 8.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T02G	SPECIAL	1	1	296128025
					Job Reference (optional)

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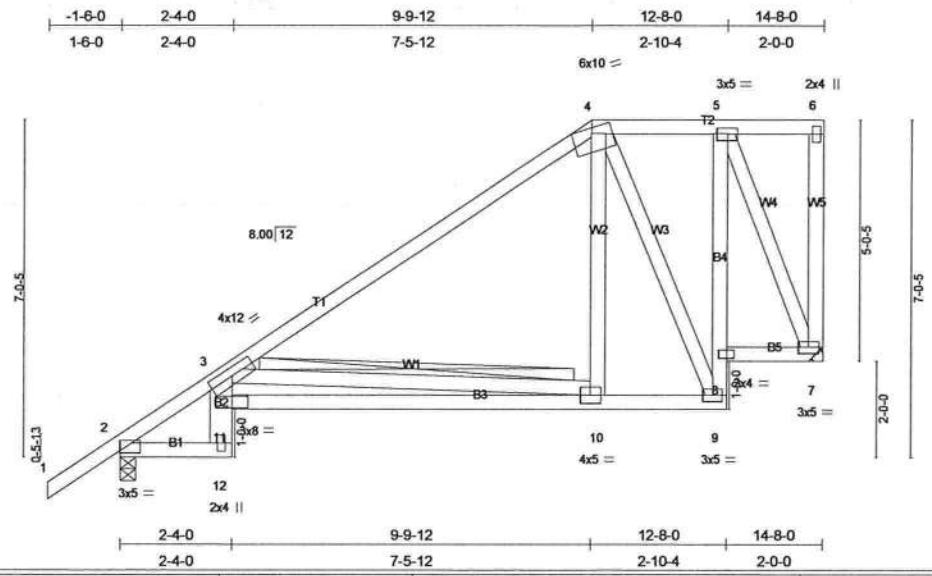

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	0.16	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.23	10-11	>761	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 103 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 6 SYP No.1D, B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
5-7-15 oc bracing: 10-11.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 7=454/Mechanical, 2=555/0-4-0
Max Horz 2=258(load case 6)
Max Uplift7=-130(load case 5), 2=-143(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-628/141, 3-4=-477/147, 4-5=-135/89, 5-6=-10/7, 6-7=-78/67
BOT CHORD 2-12=-347/437, 11-12=-1/50, 3-11=0/159, 10-11=-1138/1448, 9-10=-226/309, 8-9=-287/366, 5-8=-252/330, 7-8=-98/145
WEBS 3-10=-1146/916, 4-10=-47/274, 4-9=-409/319, 5-7=-347/233

JOINT STRESS INDEX
2 = 0.58, 3 = 0.90, 4 = 0.97, 5 = 0.46, 6 = 0.23, 7 = 0.20, 8 = 0.85, 9 = 0.67, 10 = 0.40, 11 = 0.63 and 12 = 0.54

NOTES (6-7)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 7 and 143 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T02I	SPECIAL	1	1	296128027
					Job Reference (optional)

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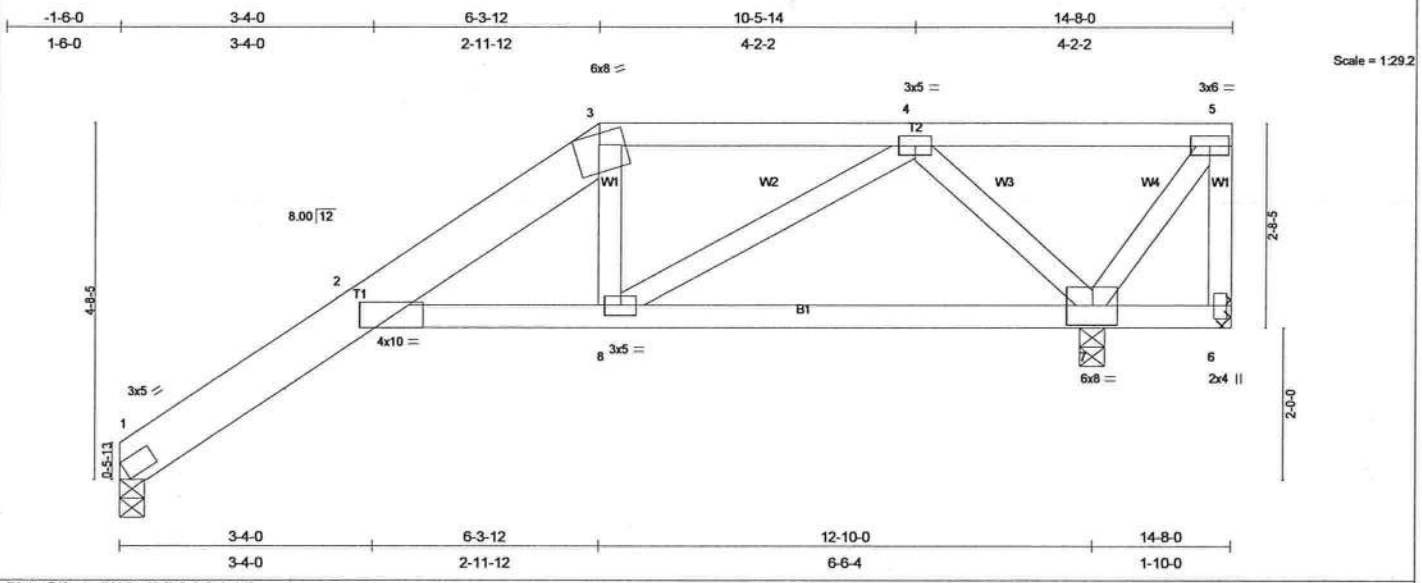

Plate Offsets (X,Y): [3:7-0-6,0-1-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.13	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.10	2-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.37	Horz(TL)	-0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=293/0-4-0, 6=-826/Mechanical, 7=1457/0-4-0
Max Horz 1=137(load case 6)
Max Uplift1=-80(load case 6), 6=-826(load case 1), 7=-872(load case 6)
Max Grav 1=293(load case 1), 6=560(load case 6), 7=1457(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-140/41, 2-3=-291/399, 3-4=-284/494, 4-5=-860/618, 5-6=-1122/796
BOT CHORD 2-8=-503/292, 7-8=-146/235, 6-7=-9/13
WEBS 3-8=-179/100, 4-8=-851/497, 4-7=-676/895, 5-7=-1090/1471

JOINT STRESS INDEX
2 = 0.20, 3 = 0.22, 3 = 0.00, 4 = 0.48, 5 = 0.84, 6 = 0.58, 7 = 0.32 and 8 = 0.33

NOTES (7-8)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 1, 826 lb uplift at joint 6 and 872 lb uplift at joint 7.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T03	HIP	1	1	296128029
					Job Reference (optional)

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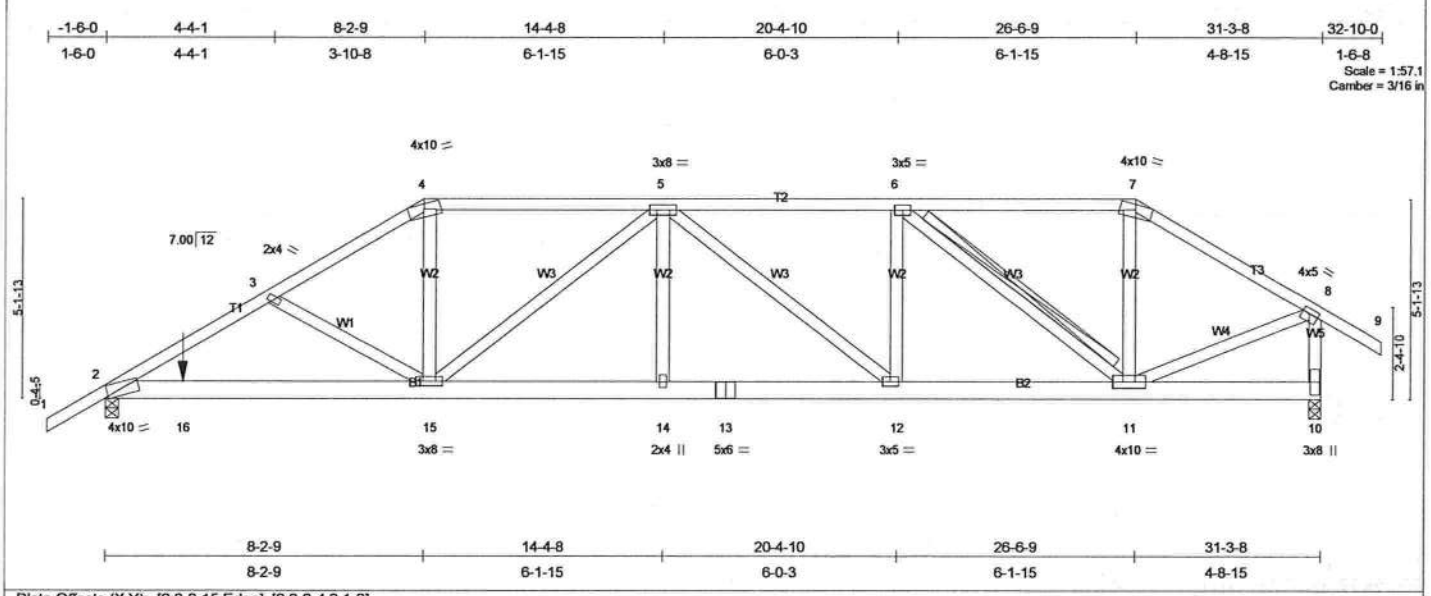

Plate Offsets (X,Y): [2:0-0-15,Edge], [8:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.18	12-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.34	2-15	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.09	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-10 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2348/0-4-0, 10=2296/0-3-8
Max Horz 2=161(load case 4)
Max Uplift2=-868(load case 4), 10=-883(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-3566/1405, 3-4=-3388/1397, 4-5=-2936/1260, 5-6=-3322/1463, 6-7=-1912/859, 7-8=-2236/941, 8-9=0/46, 8-10=-2092/824
BOT CHORD 2-16=-1313/3036, 15-16=-1313/3036, 14-15=-1624/3622, 13-14=-1624/3622, 12-13=-1624/3622, 11-12=-1479/3322, 10-11=0/42
WEBS 3-15=-158/98, 4-15=-399/1184, 5-15=-947/549, 5-14=0/195, 5-12=-401/185, 6-12=-27/386, 6-11=-1828/871, 7-11=-209/639, 8-11=-874/2020

JOINT STRESS INDEX
2 = 0.65, 3 = 0.34, 4 = 0.87, 5 = 0.57, 6 = 0.62, 7 = 0.87, 8 = 0.80, 10 = 0.34, 11 = 0.92, 12 = 0.40, 13 = 0.81, 14 = 0.34 and 15 = 0.59

NOTES (11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 868 lb uplift at joint 2 and 883 lb uplift at joint 10.
7) Girder carries tie-in span(s): 7-0-0 from 2-0-0 to 8-2-9; 7-0-0 from 26-6-9 to 31-3-8
8) Girder carries hip end with 4-8-15 right side setback, 8-2-9 left side setback, and 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 217 lb down and 79 lb up at 2-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-120(F=-66), 7-8=-54, 8-9=-54, 2-16=-10, 15-16=-85(F=-75), 11-15=-22(F=-12), 10-11=-85(F=-75)
Concentrated Loads (lb)
Vert: 16=-217(F)

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T05	HIP	1	1	296128031
					Job Reference (optional)

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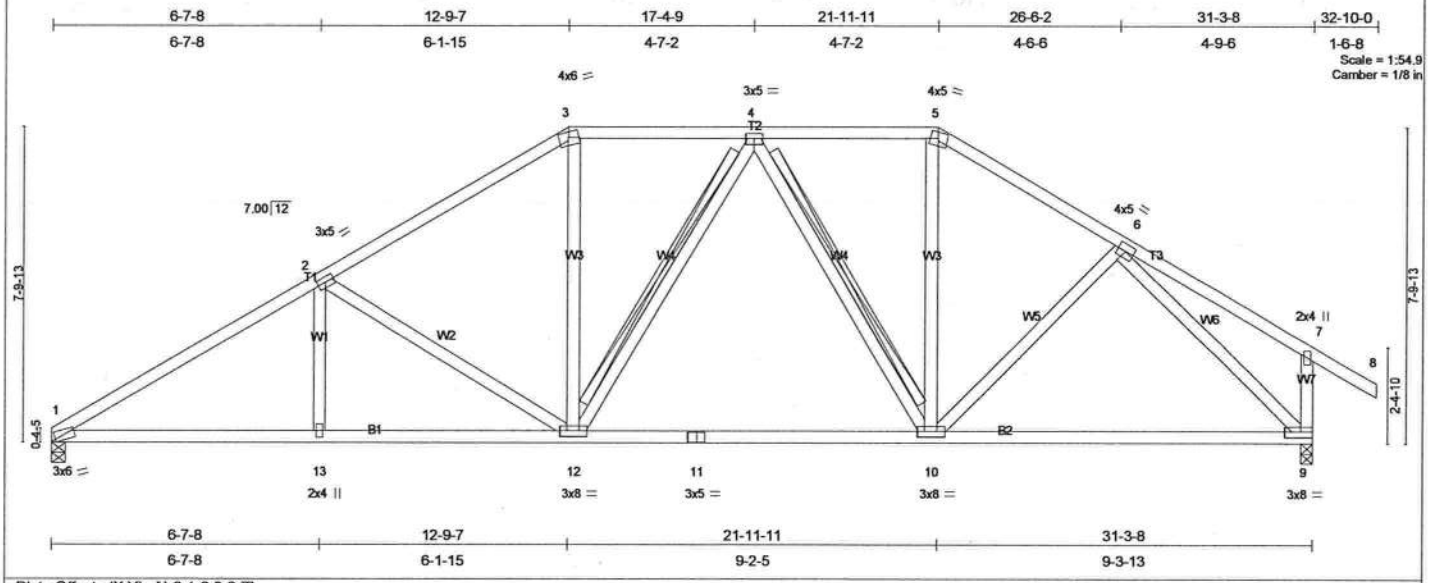

Plate Offsets (X,Y): [1:0-1-2,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.11	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.22	10-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-4-10 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 4-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=989/0-4-0, 9=1085/0-3-8
Max Horz 1=-221(load case 4)
Max Uplift1=-200(load case 6), 9=-244(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1665/783, 2-3=-1250/678, 3-4=-1008/650, 4-5=-874/583, 5-6=-1067/613, 6-7=-139/173, 7-8=0/46, 7-9=-252/264
BOT CHORD 1-13=-527/1355, 12-13=-527/1355, 11-12=-282/1006, 10-11=-282/1006, 9-10=-249/794
WEBS 2-13=0/195, 2-12=-419/304, 3-12=-130/330, 4-12=-141/172, 4-10=-335/194, 5-10=-123/286, 6-10=-101/191, 6-9=-1083/455

JOINT STRESS INDEX
1 = 0.85, 2 = 0.49, 3 = 0.60, 4 = 0.44, 5 = 0.62, 6 = 0.37, 7 = 0.47, 9 = 0.57, 10 = 0.60, 11 = 0.52, 12 = 0.60 and 13 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 200 lb uplift at joint 1 and 244 lb uplift at joint 9.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T07	SPECIAL	1	1	296128033
					Job Reference (optional)

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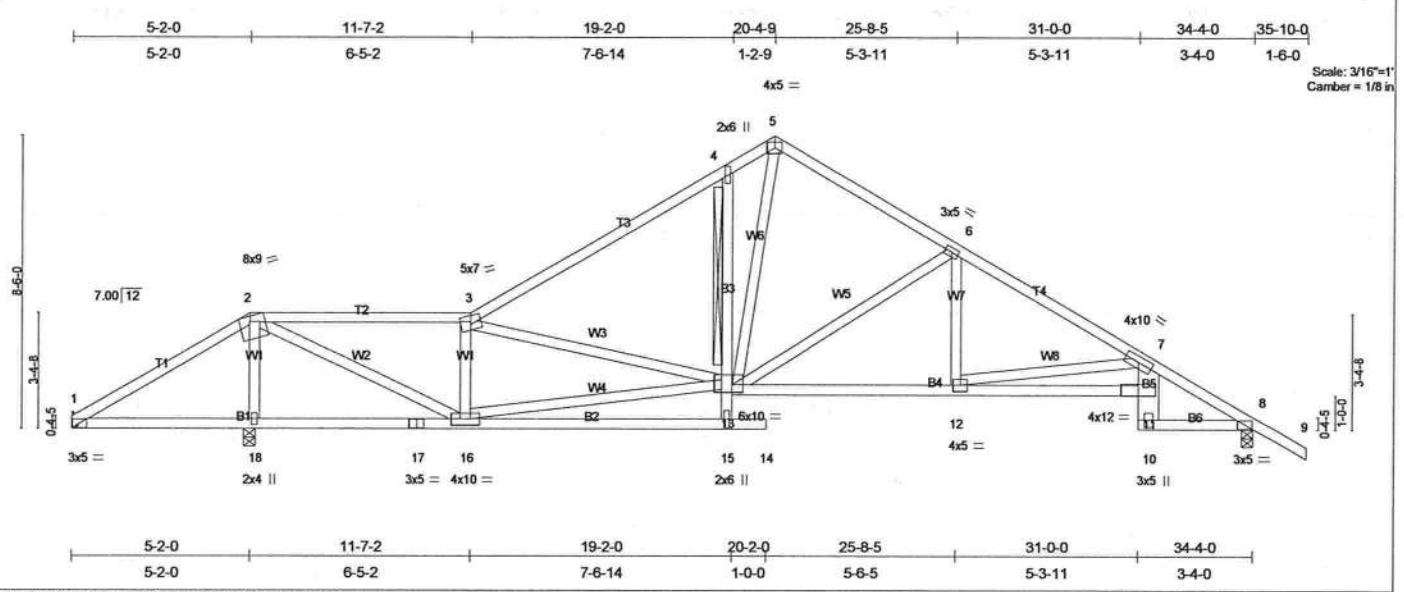

Plate Offsets (X,Y): [2:0-3-9,Edge], [13:0-4-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.13	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.26	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 204 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3, B5 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 4-13

REACTIONS (lb/size) 1=-465/Mechanical, 8=909/0-4-0, 18=1835/0-4-0
Max Horz 1=-243(load case 4)
Max Uplift1=-465(load case 1), 8=-254(load case 7), 18=-488(load case 6)
Max Grav 1=41(load case 6), 8=909(load case 1), 18=1835(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-456/1189, 2-3=-692/411, 3-4=-1041/491, 4-5=-963/610, 5-6=-848/449, 6-7=-1425/617, 7-8=-1360/561, 8-9=0/40
BOT CHORD 1-18=-943/492, 17-18=-1026/531, 16-17=-1026/531, 15-16=-9/83, 14-15=0/0, 13-15=0/153, 4-13=-325/251, 12-13=-321/1173, 11-12=-871/2398, 10-11=0/57, 7-11=0/114, 8-10=-363/1096
WEBS 2-18=-1745/827, 2-16=-838/1911, 3-16=-878/468, 13-16=-238/669, 3-13=-86/203, 5-13=-453/732, 6-13=-596/341, 6-12=-58/346, 7-12=-1238/556

JOINT STRESS INDEX
1 = 0.60, 2 = 0.69, 3 = 0.76, 4 = 0.42, 5 = 0.47, 6 = 0.49, 7 = 0.69, 8 = 0.75, 10 = 0.61, 11 = 0.78, 12 = 0.43, 13 = 0.38, 15 = 0.86, 16 = 0.88, 17 = 0.19 and 18 = 0.64

NOTES (7-8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 465 lb uplift at joint 1, 254 lb uplift at joint 8 and 488 lb uplift at joint 18.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T09	SPECIAL	1	1	296128035
					Job Reference (optional)

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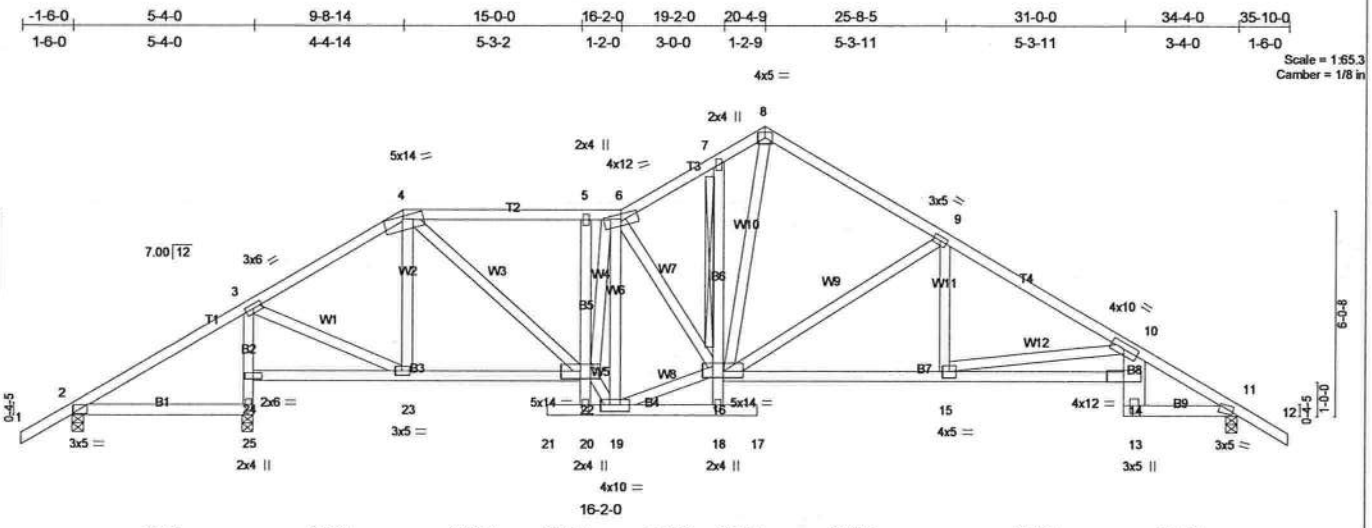

Plate Offsets (X,Y): [11:0-1-14,0-0-7], [16:0-7-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.15	14-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.29	14-15	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.14	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 233 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3, B6 2 X 4 SYP No.3, B8 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-0-1 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 7-16

REACTIONS (lb/size) 2=99/0-4-0, 25=1280/0-4-0, 11=1000/0-4-0
Max Horz 2=226(load case 5)
Max Uplift2=-195(load case 4), 25=-340(load case 6), 11=-276(load case 7)
Max Grav 2=146(load case 10), 25=1280(load case 1), 11=1000(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-109/353, 3-4=-818/478, 4-5=-1197/683, 5-6=-1166/671, 6-7=-1194/659, 7-8=-1113/666, 8-9=-1051/568, 9-10=-1643/744, 10-11=-1529/660, 11-12=0/40
BOT CHORD 2-25=-232/49, 24-25=-1265/488, 3-24=-1228/494, 23-24=-318/106, 22-23=-153/642, 20-22=-0/12, 5-22=-167/164, 20-21=0/0, 19-20=-79/162, 18-19=-51/18, 17-18=0/0, 16-18=0/63, 7-16=-71/79, 15-16=-431/1362, 14-15=-1044/2696, 13-14=0/59, 10-14=0/114, 11-13=-445/1236
WEBS 3-23=-288/1062, 4-23=-382/152, 4-22=-299/737, 19-22=-373/1306, 6-22=-197/685, 6-19=-1276/400, 16-19=-321/1092, 6-16=-180/220, 8-16=-445/805, 9-16=-610/348, 9-15=-68/360, 10-15=-1347/620

JOINT STRESS INDEX
2 = 0.42, 3 = 0.74, 4 = 0.69, 5 = 0.68, 6 = 0.40, 7 = 0.34, 8 = 0.55, 9 = 0.49, 10 = 0.74, 11 = 0.78, 13 = 0.68, 14 = 0.88, 15 = 0.47, 16 = 0.96, 18 = 0.34, 19 = 0.89, 20 = 0.60, 22 = 0.46, 23 = 0.70, 24 = 0.60 and 25 = 0.65

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 2, 340 lb uplift at joint 25 and 276 lb uplift at joint 11.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T11	SPECIAL	1	1	296128037
					Job Reference (optional)

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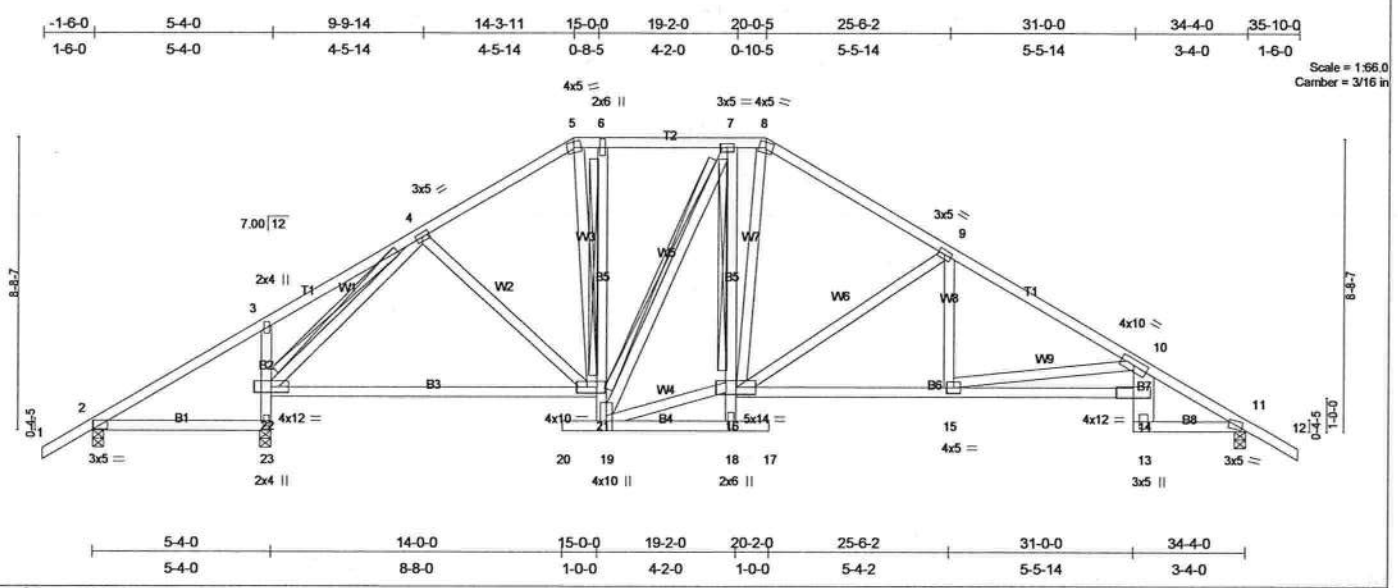

Plate Offsets (X,Y): [11:0-1-10,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.21	21-22	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.43	21-22	>808	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.22	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 235 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B7 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-7-14 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 6-21, 7-16
WEBS T-Brace: 2 X 4 SYP No.3 - 7-19, 4-22
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=56/0-4-0, 23=1330/0-4-0, 11=993/0-4-0
Max Horz 2=232(load case 5)
Max Uplift2=-217(load case 4), 23=-269(load case 6), 11=-282(load case 7)
Max Grav 2=104(load case 10), 23=1330(load case 1), 11=993(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-114/435, 3-4=-33/457, 4-5=-880/544, 5-6=-733/531, 6-7=-693/531, 7-8=-855/572, 8-9=-1028/581, 9-10=-1610/753, 10-11=-1515/671, 11-12=0/40
BOT CHORD 2-23=-314/13, 22-23=-1315/458, 3-22=-281/224, 21-22=-175/583, 19-21=-66/387, 6-21=-113/93, 19-20=0/0, 18-19=-147/6, 17-18=0/0, 16-18=-91/31, 7-16=-87/284, 15-16=-436/1332, 14-15=-1069/2681, 13-14=0/59, 10-14=0/120, 11-13=-454/1225
WEBS 7-19=-438/106, 16-19=-146/992, 8-16=-96/309, 9-16=-613/356, 9-15=-67/352, 10-15=-1362/639, 5-21=-118/286, 4-21=-12/242, 4-22=-1334/362

JOINT STRESS INDEX
2 = 0.46, 3 = 0.58, 4 = 0.52, 5 = 0.54, 6 = 0.63, 7 = 0.47, 8 = 0.69, 9 = 0.49, 10 = 0.75, 11 = 0.80, 13 = 0.68, 14 = 0.87, 15 = 0.47, 16 = 0.38, 18 = 0.62, 19 = 0.63, 21 = 0.97, 22 = 0.88 and 23 = 0.64

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 2, 269 lb uplift at joint 23 and 282 lb uplift at joint 11.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T13	SPECIAL	1	1	296128039
					Job Reference (optional)

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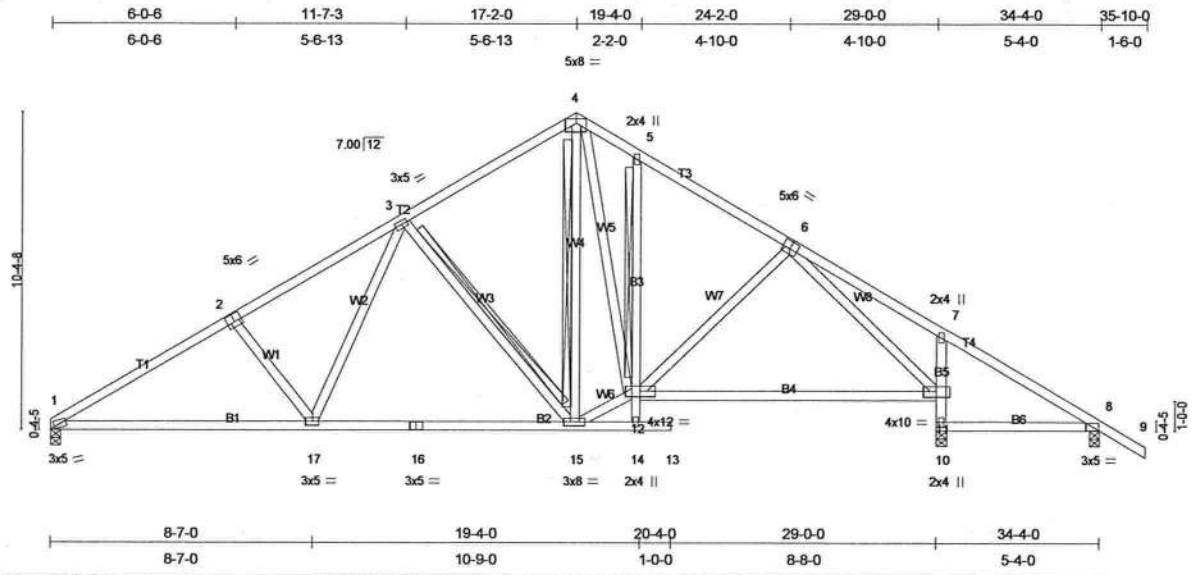

Plate Offsets (X,Y): [1:0-1-10,0-0-7], [2:0-3-0,0-3-0], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.14	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.26	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.05	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 212 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-3 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 5-12
WEBS T-Brace: 2 X 4 SYP No.3 - 3-15, 4-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=920/0-4-0, 10=1155/0-4-0, 8=203/0-4-0
Max Horz 1=-295(load case 4)
Max Uplift1=-210(load case 6), 10=-210(load case 7), 8=-239(load case 7)
Max Grav 1=920(load case 1), 10=1155(load case 1), 8=214(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1527/720, 2-3=-1338/718, 3-4=-834/549, 4-5=-883/627, 5-6=-963/556, 6-7=-46/281, 7-8=-24/184, 8-9=0/40
BOT CHORD 1-17=-477/1245, 16-17=-264/954, 15-16=-264/954, 14-15=-236/0, 13-14=0/0, 12-14=-30/0, 5-12=-185/132, 11-12=-198/733, 10-11=-1124/418, 7-11=-292/231, 8-10=-103/0
WEBS 2-17=-265/245, 3-17=-167/369, 3-15=-472/347, 6-12=-34/173, 6-11=-1097/304, 4-15=-227/166, 12-15=-9/776, 4-12=-205/489

JOINT STRESS INDEX
1 = 0.77, 2 = 0.48, 3 = 0.43, 4 = 0.44, 5 = 0.49, 6 = 0.40, 7 = 0.48, 8 = 0.39, 10 = 0.66, 11 = 0.83, 12 = 0.58, 14 = 0.70, 15 = 0.73, 16 = 0.46 and 17 = 0.47

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1, 210 lb uplift at joint 10 and 239 lb uplift at joint 8.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T15	SPECIAL	1	1	296128041
					Job Reference (optional)

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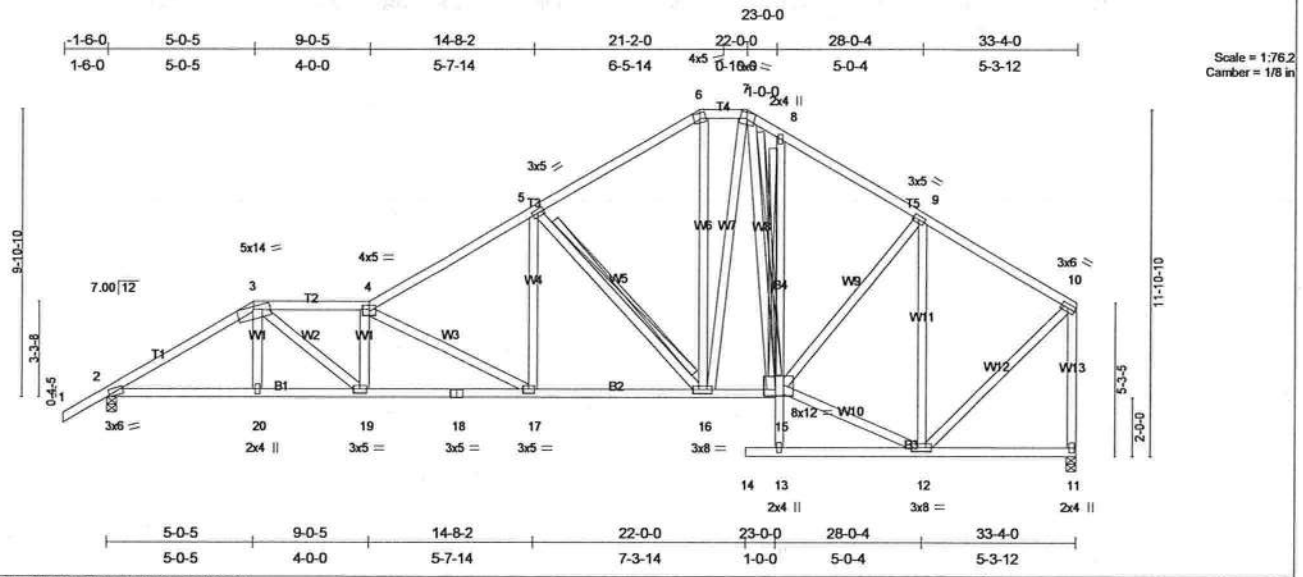

Plate Offsets (X,Y): [2:0-1-2,0-0-7], [15:0-4-8,0-2-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	0.15	17-19	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.25	17-19	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.10	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-5 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 8-15
WEBS T-Brace: 2 X 4 SYP No.3 - 5-16, 7-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1153/0-4-0, 11=1062/0-4-0
Max Horz 2=278(load case 5)
Max Uplift2=-314(load case 6), 11=-190(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1810/797, 3-4=-2281/1094, 4-5=-1691/809, 5-6=-1096/622, 7-8=-1015/688, 8-9=-1047/594, 9-10=-751/372, 10-11=-1032/495, 6-7=-880/597
BOT CHORD 2-20=-733/1484, 19-20=-732/1486, 18-19=-1112/2309, 17-18=-1112/2309, 16-17=-626/1402, 15-16=-280/820, 13-15=0/104, 8-15=-202/180, 13-14=0/0, 12-13=-5/27, 11-12=-14/22
WEBS 3-20=0/136, 3-19=-462/1005, 4-19=-580/315, 4-17=-1025/550, 5-17=-220/543, 12-15=-266/616, 9-15=-97/395, 9-12=-749/375, 10-12=-332/798, 6-16=-120/294, 5-16=-776/466, 7-16=-203/452, 7-15=-190/225

JOINT STRESS INDEX
2 = 0.81, 3 = 0.52, 4 = 0.73, 5 = 0.49, 6 = 0.70, 7 = 0.33, 8 = 0.34, 9 = 0.49, 10 = 0.77, 11 = 0.50, 12 = 0.79, 13 = 0.41, 15 = 0.20, 16 = 0.59, 17 = 0.41, 18 = 0.82, 19 = 0.69 and 20 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 190 lb uplift at joint 11.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T17	SPECIAL	1	2	296128043 Job Reference (optional)

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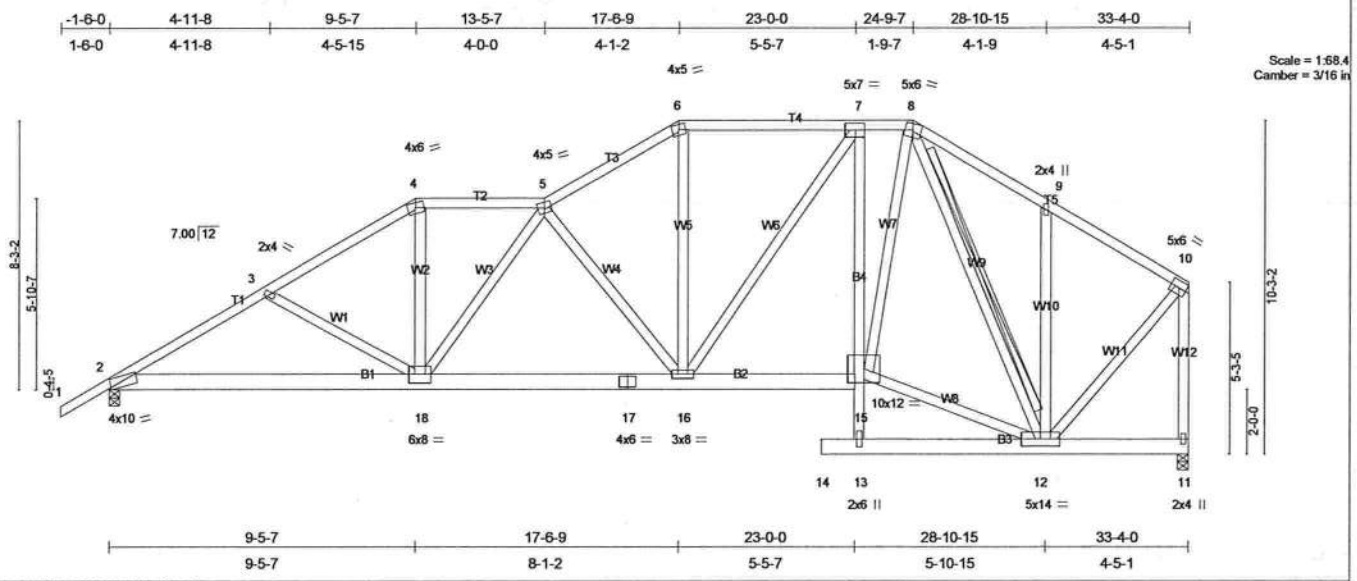

Plate Offsets (X,Y): [2:0-0-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.22	2-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.43	2-18	>927	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.78	Horz(TL)	0.13	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 547 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 8-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=4483/0-4-0, 11=4474/0-4-0
Max Horz 2=1383(load case 4)
Max Uplift2=-1418(load case 5), 11=-1520(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/43, 2-3=-6870/2046, 3-4=-6606/1979, 4-5=-5723/1765, 5-6=-5508/1557, 6-7=-4785/1399, 7-8=-4018/1526, 8-9=-2795/1067, 9-10=-2812/973, 10-11=-4103/1397
BOT CHORD 2-18=-3021/5902, 17-18=-2997/6324, 16-17=-2997/6324, 15-16=-2191/4045, 13-15=-263/791, 7-15=-1116/747, 13-14=0/0, 12-13=-98/195, 11-12=-17/41
WEBS 3-18=-302/175, 4-18=-883/3051, 5-18=-1046/238, 5-16=-2589/897, 6-16=-632/2419, 7-16=-552/1350, 12-15=-1114/3309, 8-15=-1631/3478, 8-12=-2531/985, 9-12=-208/160, 10-12=-1181/3492

JOINT STRESS INDEX
2 = 0.65, 3 = 0.34, 4 = 0.83, 5 = 0.74, 6 = 0.69, 7 = 0.80, 8 = 0.92, 9 = 0.34, 10 = 0.66, 11 = 0.87, 12 = 0.87, 13 = 0.90, 15 = 0.62, 16 = 0.70, 17 = 0.87 and 18 = 0.38

NOTES (10)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1418 lb uplift at joint 2 and 1520 lb uplift at joint 11.
9) Girder carries tie-in span(s): 14-8-0 from 0-0-0 to 33-4-0
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-6=-54, 6-8=-54, 8-10=-54, 2-15=-207(F=-197), 13-14=-207(F=-197), 11-13=-207(F=-197)

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T18G	GABLE	1	1	296128045
					Job Reference (optional)

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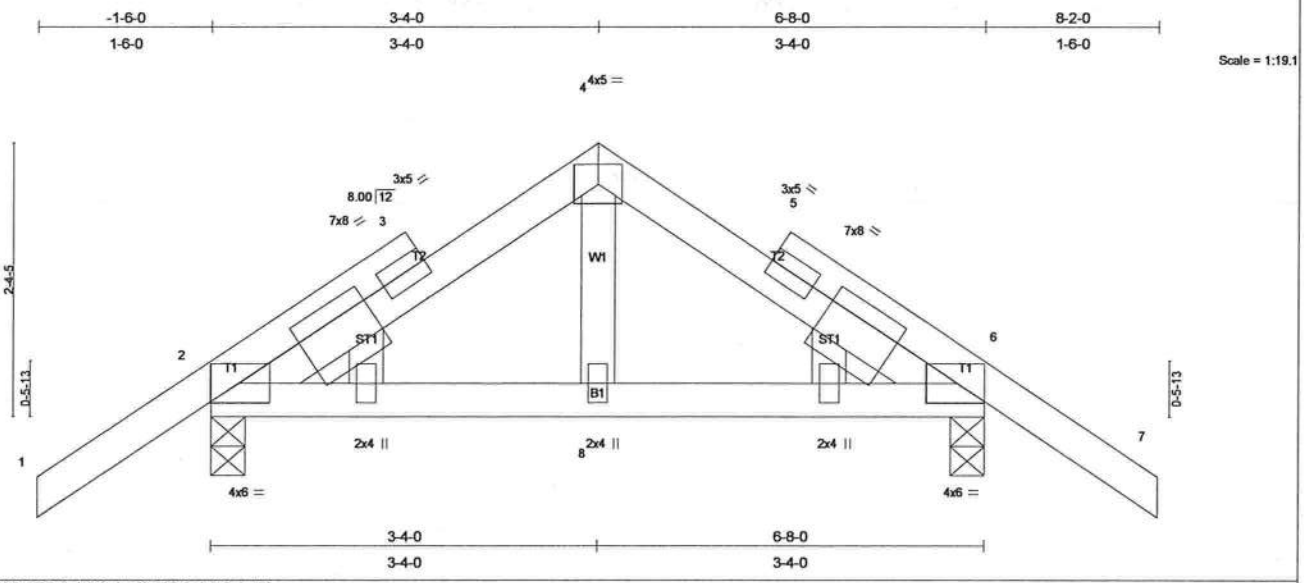

Plate Offsets (X,Y): [10:0-4-0,0-1-12], [12:0-4-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=293/0-3-8, 6=293/0-3-8
Max Horz 2=-75(load case 4)
Max Uplift2=-268(load case 6), 6=-268(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-209/272, 3-4=-154/285, 4-5=-154/285, 5-6=-209/272, 6-7=0/40
BOT CHORD 2-8=-109/128, 6-8=-109/128
WEBS 4-8=-220/117

JOINT STRESS INDEX
2 = 0.82, 3 = 0.00, 3 = 0.00, 3 = 0.23, 4 = 0.08, 5 = 0.00, 5 = 0.23, 5 = 0.00, 6 = 0.82, 8 = 0.09, 9 = 0.00, 10 = 0.15, 11 = 0.00 and 12 = 0.15

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 2 and 268 lb uplift at joint 6.
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T20	MONO TRUSS	1	1	296128047
					Job Reference (optional)

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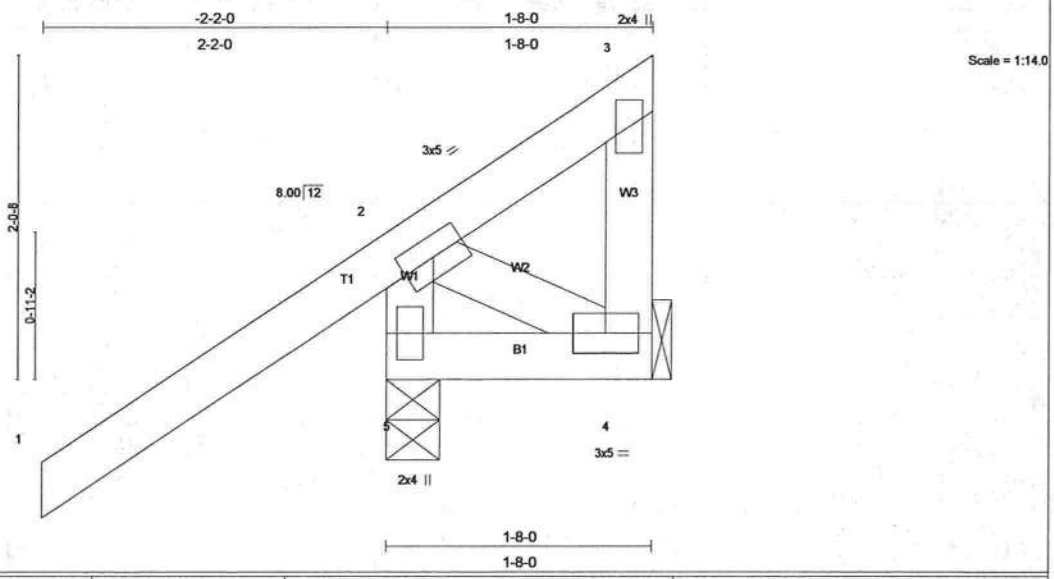


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.00	5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	5	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 14 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=274/0-4-0, 4=-61/Mechanical
Max Horz 5=142(load case 6)
Max Uplift5=-230(load case 6), 4=-61(load case 1)
Max Grav 5=274(load case 1), 4=57(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/69, 2-3=-79/80
BOT CHORD 4-5=-160/0
WEBS 3-4=-146/68, 2-5=-267/274, 2-4=-0/180

JOINT STRESS INDEX
2 = 0.19, 3 = 0.07, 4 = 0.09 and 5 = 0.16

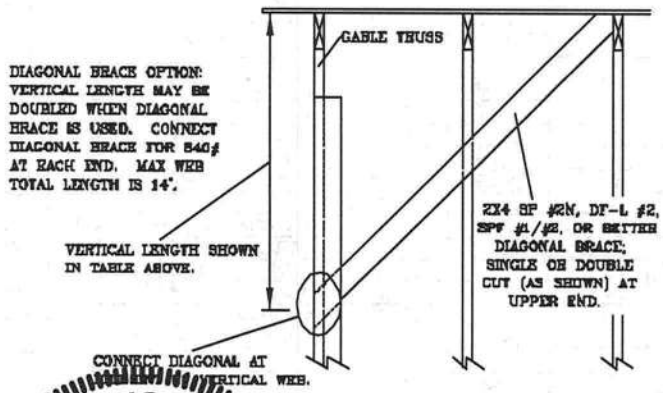
NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 5 and 61 lb uplift at joint 4.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

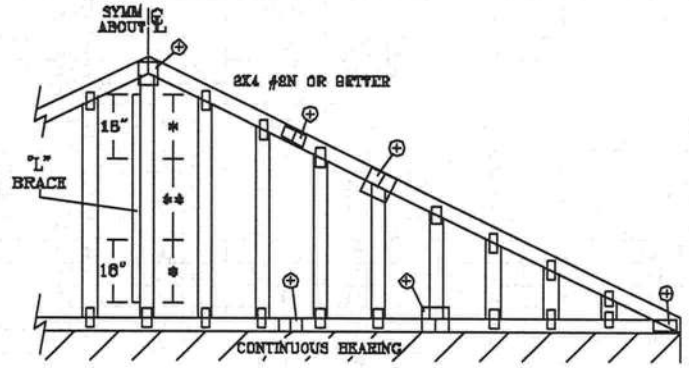
LOAD CASE(S) Standard

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	8' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 6"	10' 2"	12' 6"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 6"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 6"	13' 6"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 6"	10' 6"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 6"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

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WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

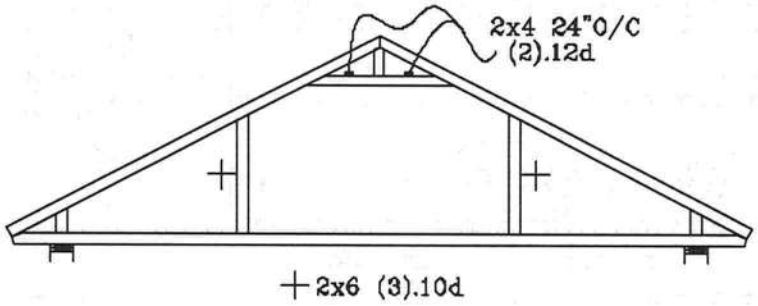
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

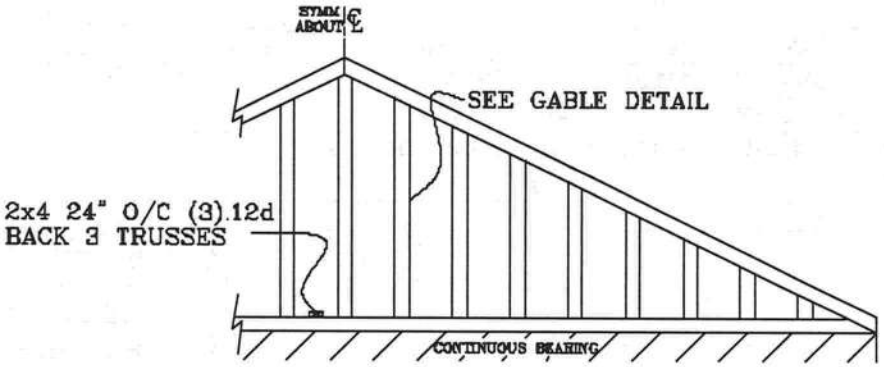
MAX. SPACING 24.0"

REF ASCE7-02-GAB13015
DATE 11/26/03
DRWG MITEK STD GABLE 15 E HT
-ENG

TYPICAL ATTIC TRUSS BRACING

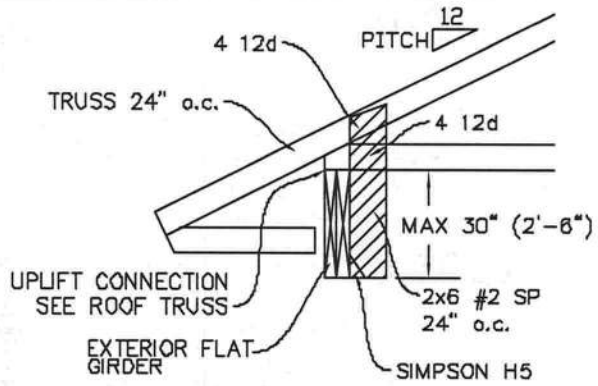


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

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LICENSE
NO. 34869
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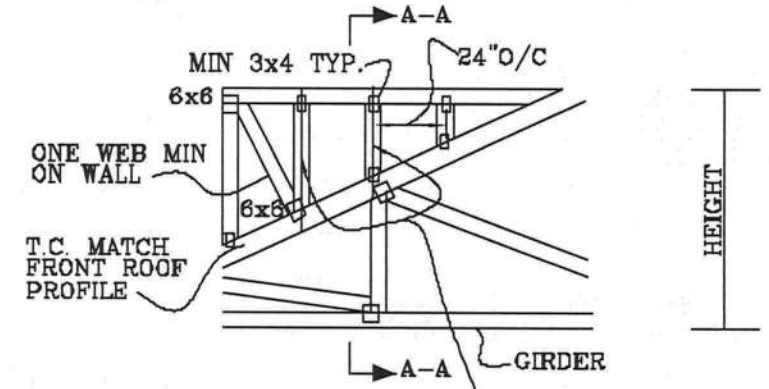
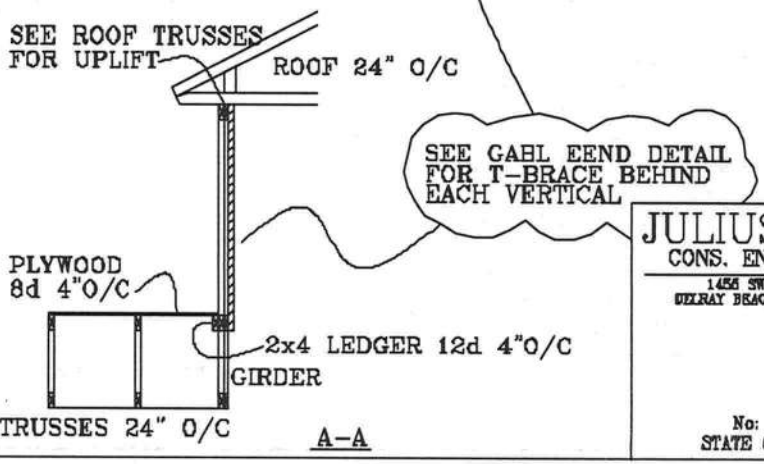
FLORIDA
PROFESSIONAL ENGINEER

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





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DELRAY BEACH, FL 33444-2161

No: 34869
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VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

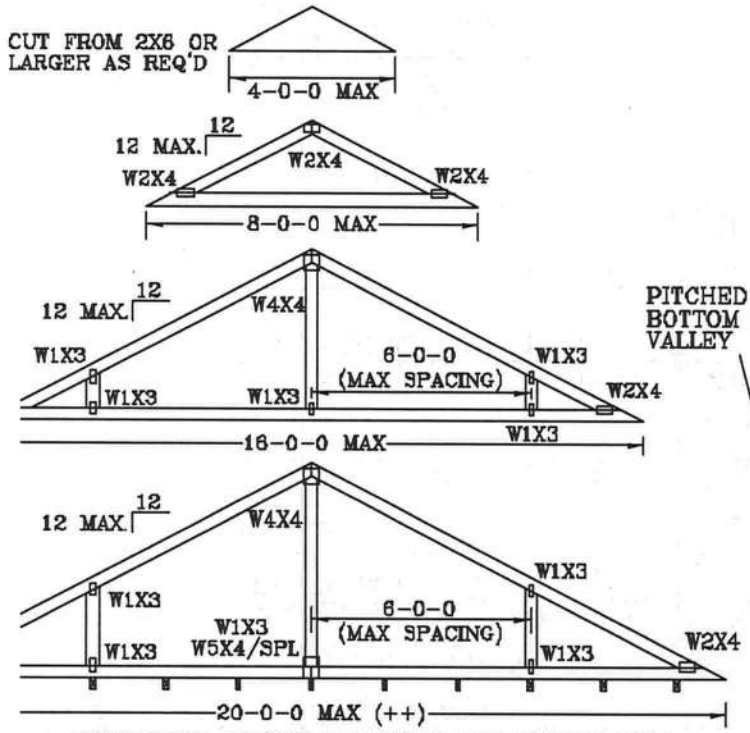
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

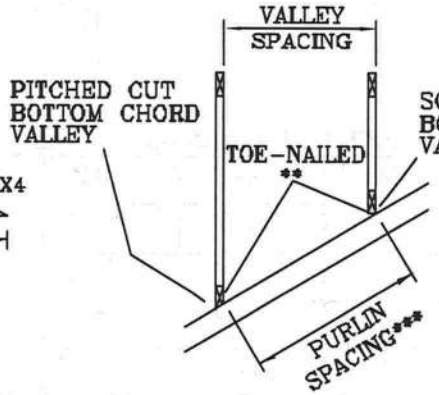
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

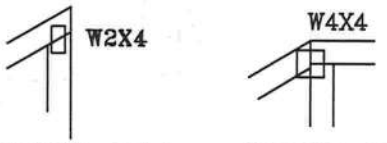
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

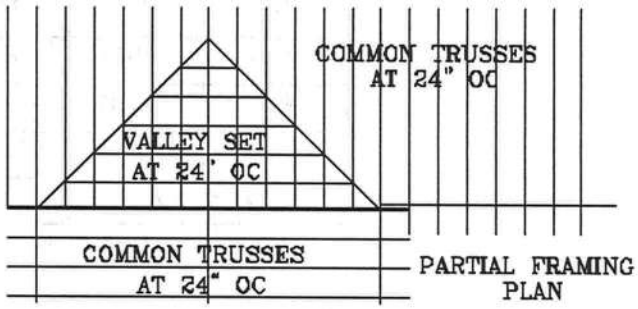
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

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WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DURFAC. 1.25		1.25		
SPACING		24"		

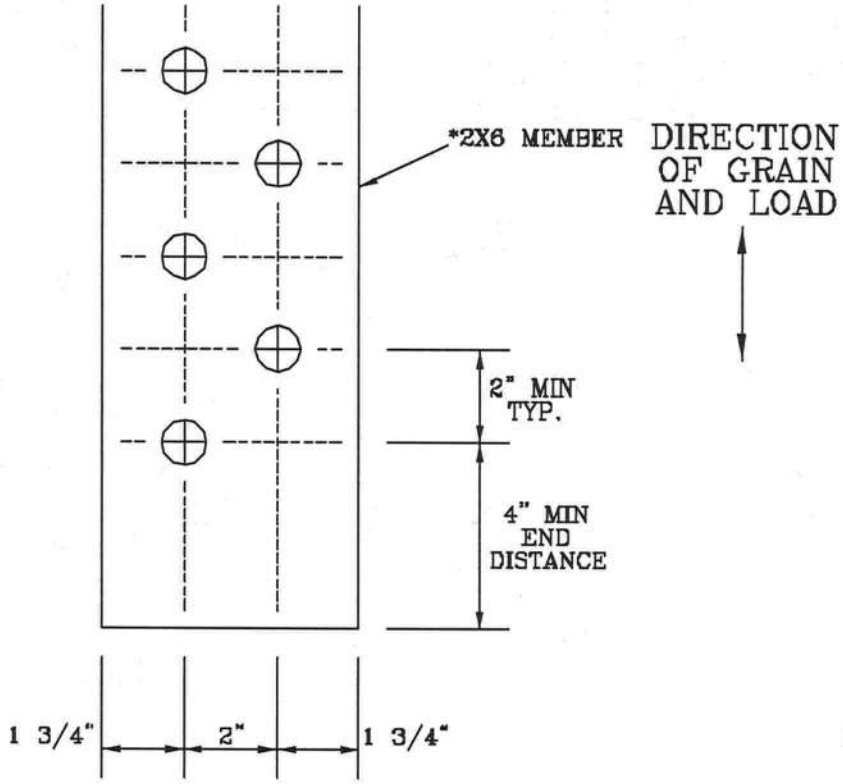
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

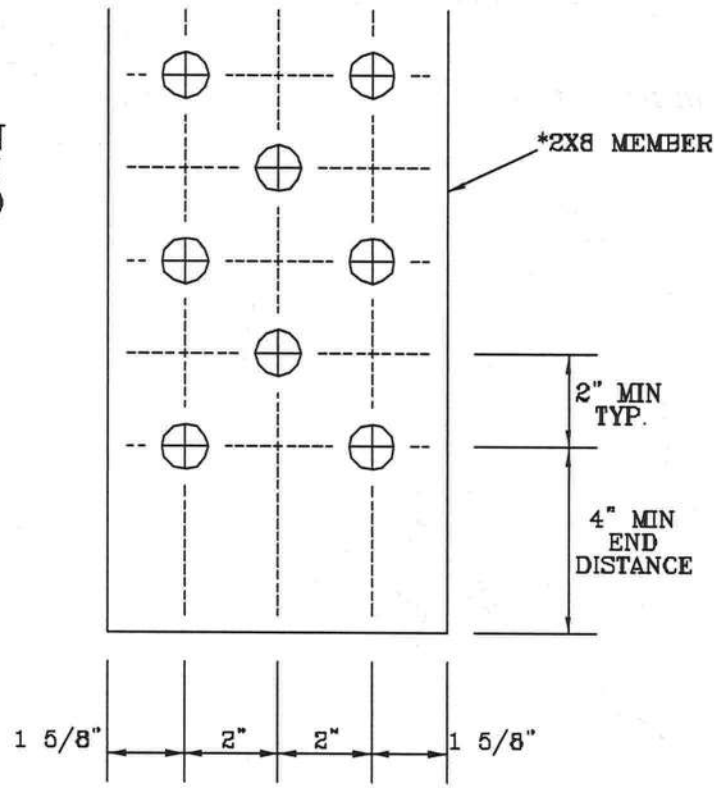
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828,016

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

=WARNING= TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

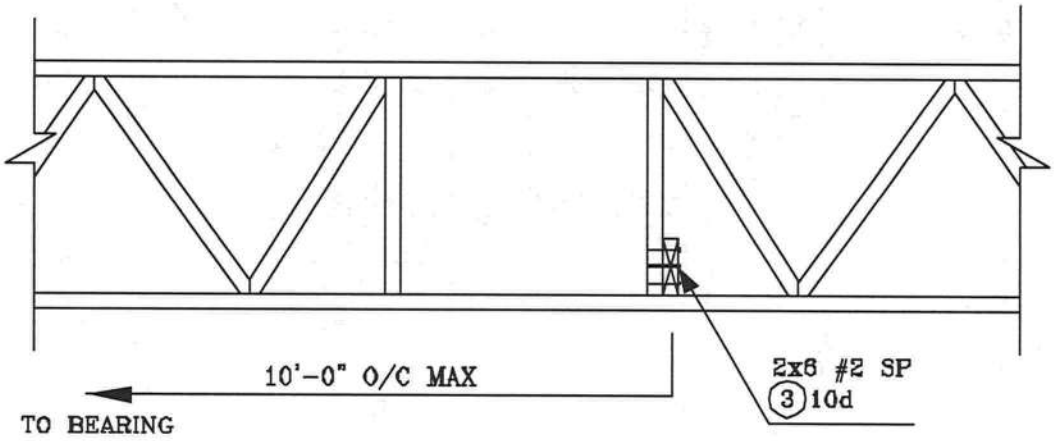
REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1450 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

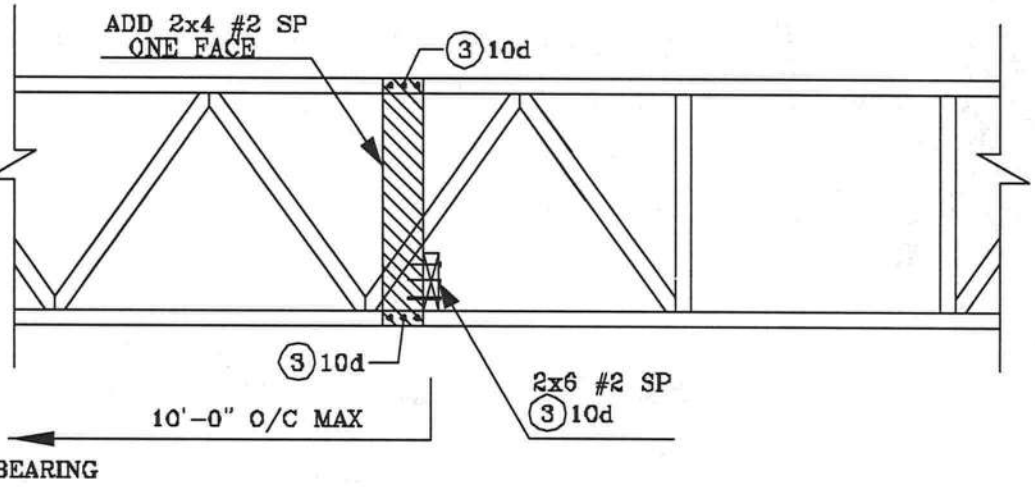
No: 34869
STATE OF FLORIDA

TC LL	PSF	REF BOLT SPACING
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG CNBOLTSP1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

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JULIUS LEE'S
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DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	***835***	835	***740***		
	12	2,225	***1,670***	1,670	***1,485***		
	18	3,335	***2,505***	2,505	***2,225***		
	24	4,450	***3,335***	3,335	***2,965***		
SDS Screws ¼" x 3½" or WS35 ¼" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	***1,275***	***1,860(2)***	***1,405(2)***
	6	2,870	2,150 (4)	2,150	***1,915***	***2,785(2)***	***2,110(2)***
	8	3,825	2,870 (4)	2,870	***2,550***	***3,715(2)***	***2,810(2)***
3⅜" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	***1,695***	***1,925(3)***	***1,775(3)***
	6	3,815	2,860 (4)	2,860	***2,545***	***2,890(3)***	***2,665(3)***
	8	5,090	3,815 (4)	3,815	***3,390***	***3,855(3)***	***3,550(3)***

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

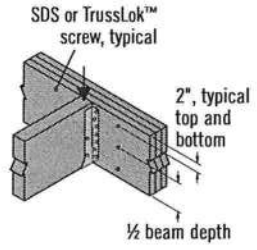
(2) 6" long screws required.

(3) 5" long screws required.

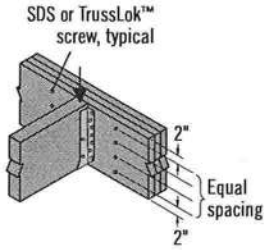
(4) 3½" and 3⅝" long screws must be installed on both sides.

Connections

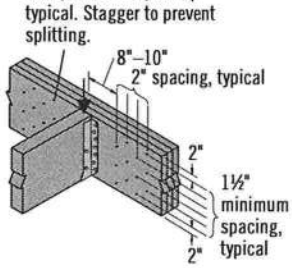
4 or 6 or Screw Connection



8 Screw Connection

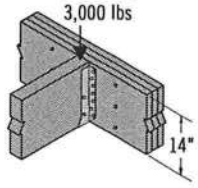


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3⅜" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

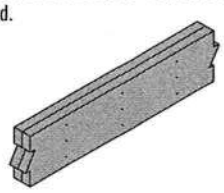
MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1¾" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3⅜" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3½" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.

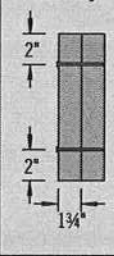
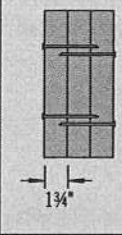
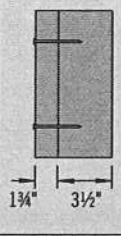
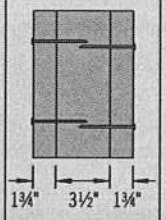
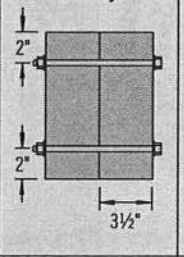
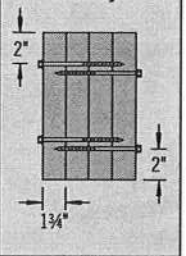


Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35 [4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6 [3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅜" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¾" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9/16" maximum.

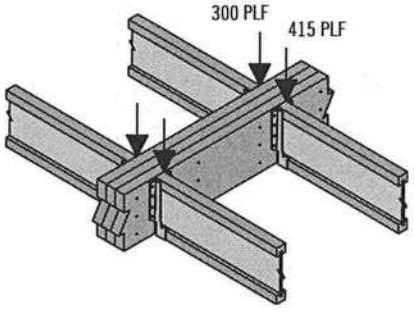
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.

TRULOX CONNECTION DETAIL

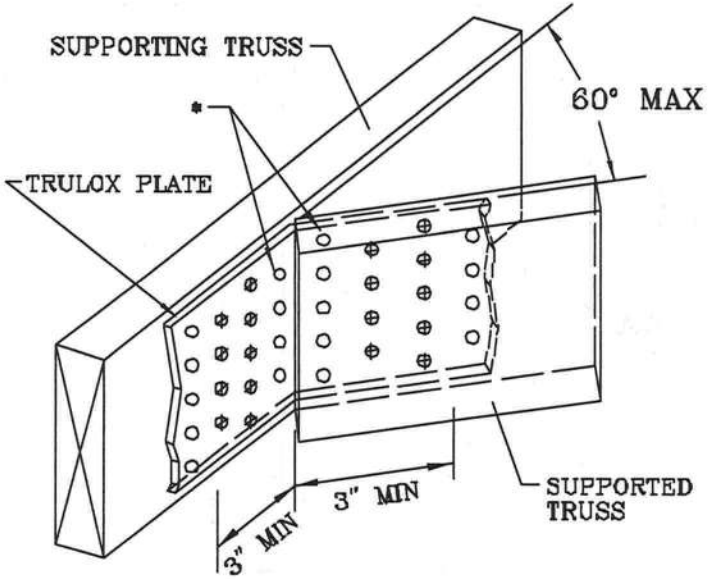
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

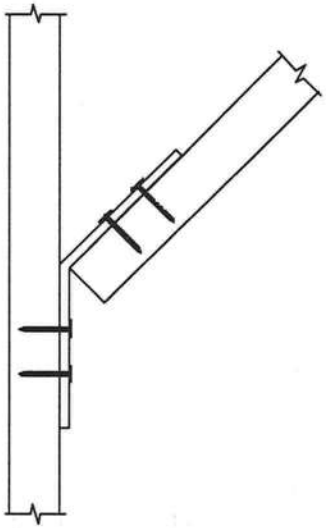
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

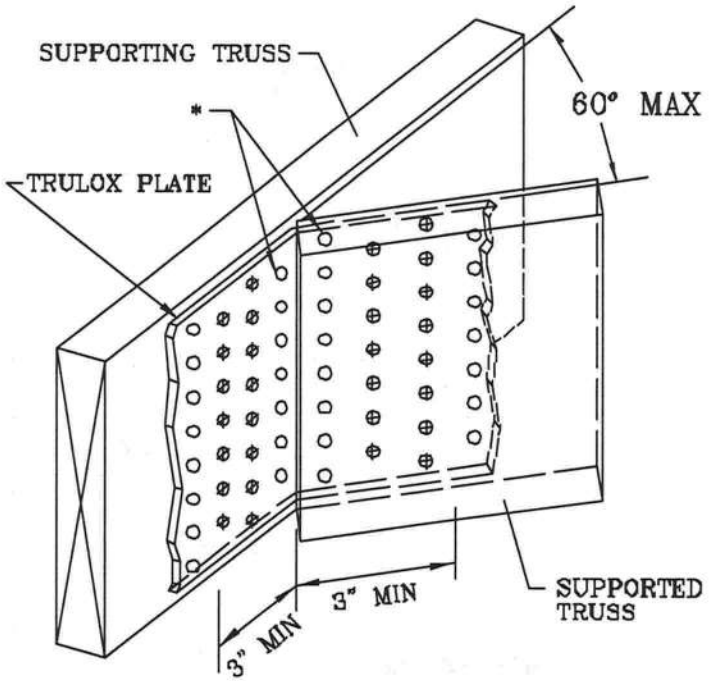
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

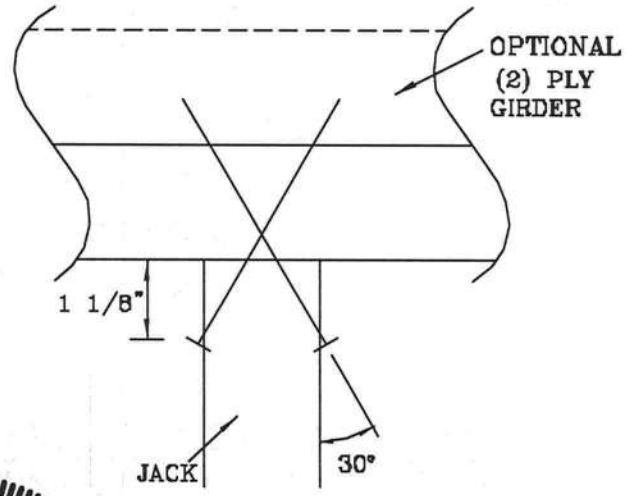
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

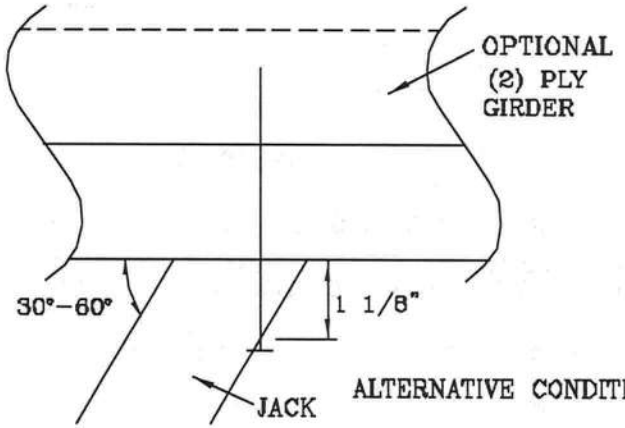
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

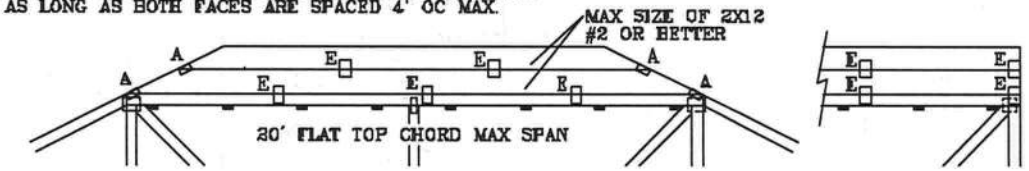
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

- 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF
- 130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

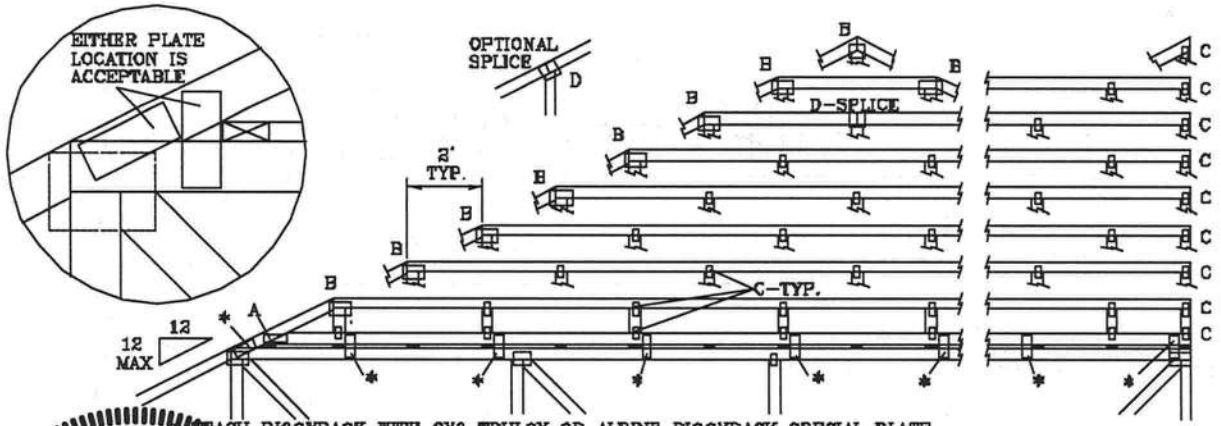
FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.



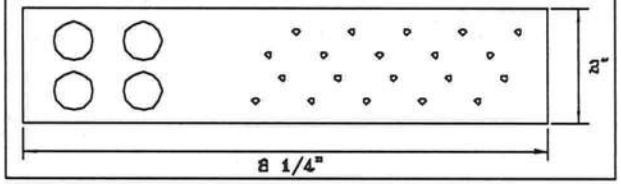
WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.



ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 6"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 5"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 5"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 5"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 6"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

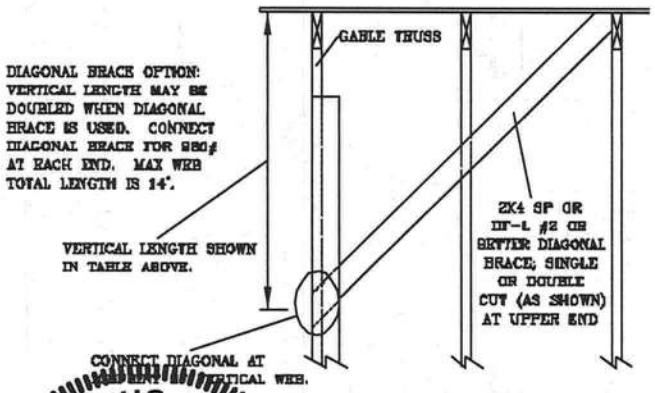
SOUTHERN PINE
#3
STUD
STANDARD

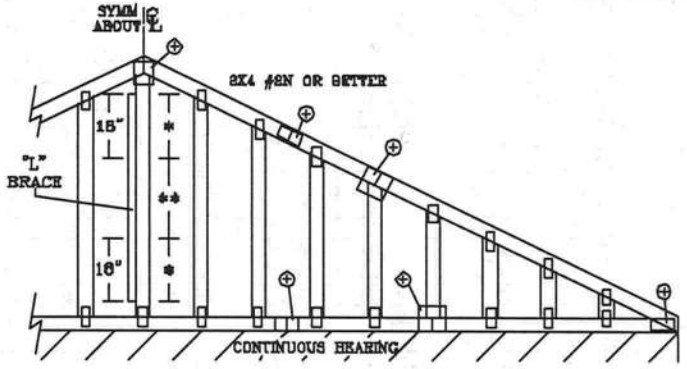
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	MID SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 90° E M7
-ENG

STEPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

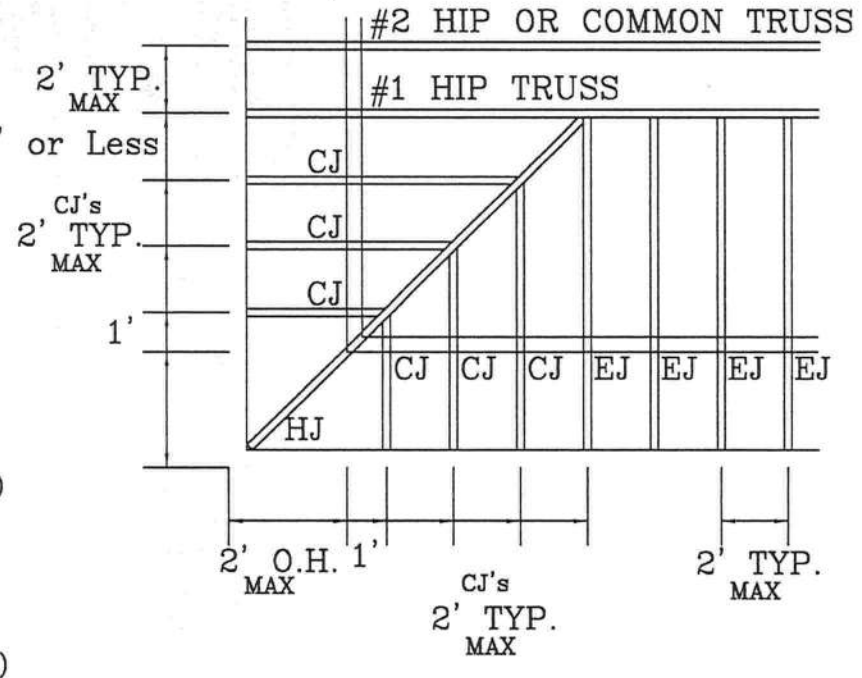
120 MPH MAX

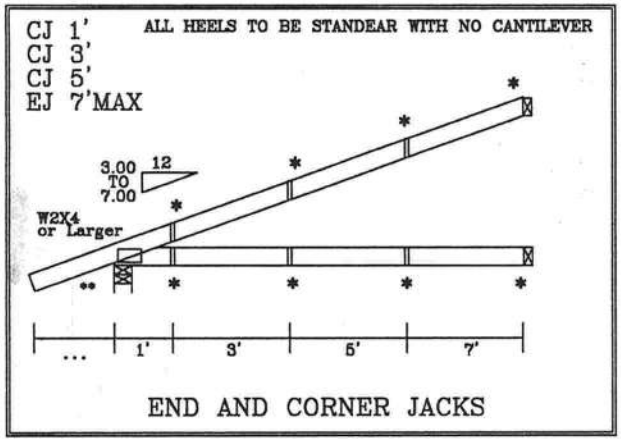
Setback 7' or Less

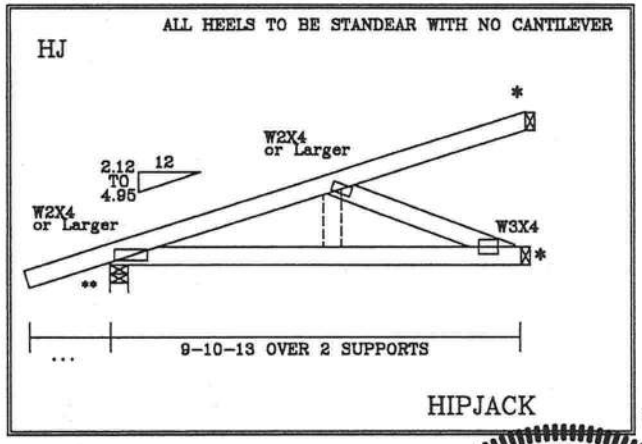
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)







* (3) 16d TOENAILS

** SEE EOR FOR TIE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET SETBACK
7'0" MAX

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H,S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

JULIUS LEE
CONS. ENGINEERS, P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2181

NO. 34869
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SHINGLE		
TC	LL	20 MAX PSF
TC	DL	7 MAX PSF
BC	LL	10* MAX PSF
BC	DL	5 MAX PSF
TILE		
TC	LL	20 MAX PSF
TC	DL	20 MAX PSF
BC	LL	10* MAX PSF
BC	DL	5 MAX PSF
DUR. FAC.		1.25
SPACING		2' MAX

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	

REVIEWED
By julius lee at 10:52 am, Jun 27, 2008

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T19	MONO TRUSS	1	1	296128046
					Job Reference (optional)

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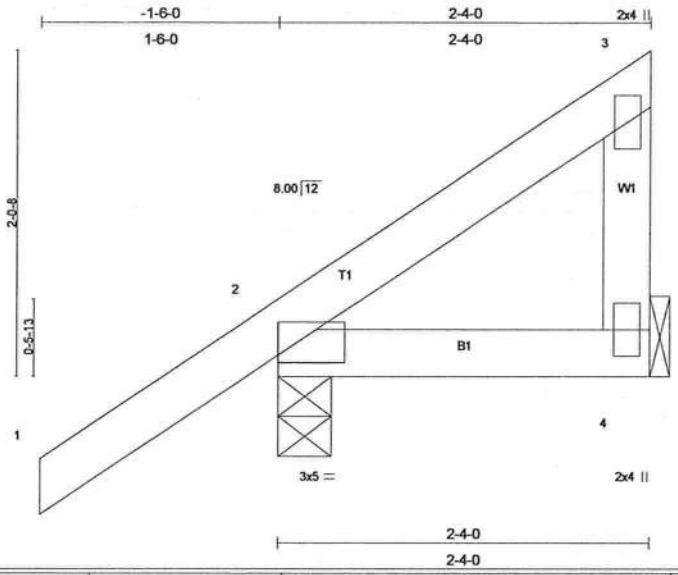

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.01	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=192/0-4-0, 4=28/Mechanical
Max Horz 2=129(load case 6)
Max Uplift2=-145(load case 6), 4=-23(load case 7)
Max Grav 2=192(load case 1), 4=35(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-57/14
BOT CHORD 2-4=0/0
WEBS 3-4=-25/29

JOINT STRESS INDEX
2 = 0.18, 3 = 0.02 and 4 = 0.02

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 2 and 23 lb uplift at joint 4.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T18	COMMON	1	2	296128044 Job Reference (optional)

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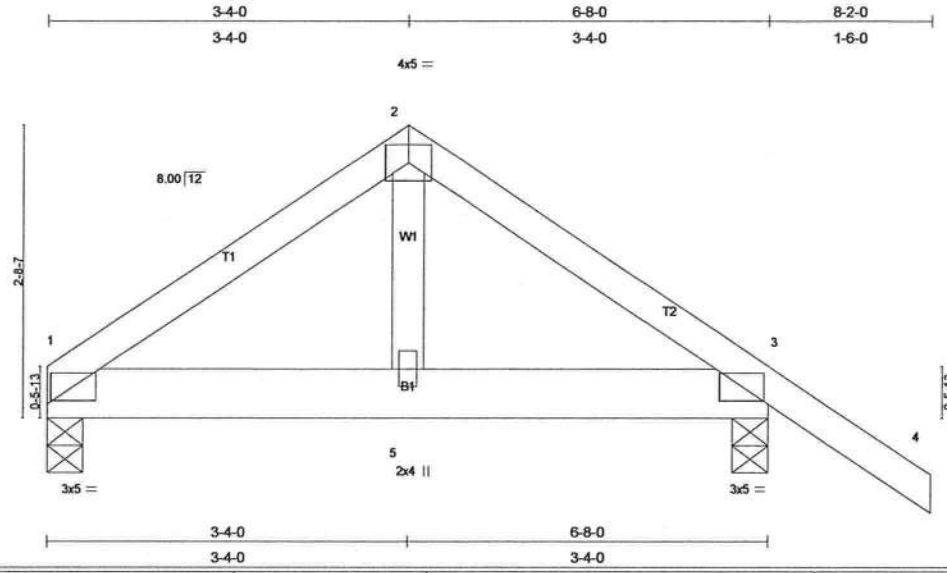

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	5	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	5	>999	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=410/0-4-0, 3=524/0-4-0
Max Horz 1=-81(load case 3)
Max Uplift1=-104(load case 5), 3=-189(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-413/87, 2-3=-437/106, 3-4=0/44
BOT CHORD 1-5=-43/301, 3-5=-43/301
WEBS 2-5=-55/293

JOINT STRESS INDEX
1 = 0.14, 2 = 0.08, 3 = 0.14 and 5 = 0.11

NOTES (9)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 1 and 189 lb uplift at joint 3.
8) Girder carries tie-in span(s): 6-8-0 from 0-0-0 to 6-8-0
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-54, 1-3=-79(F=-69)

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T16	SPECIAL	1	1	296128042
					Job Reference (optional)

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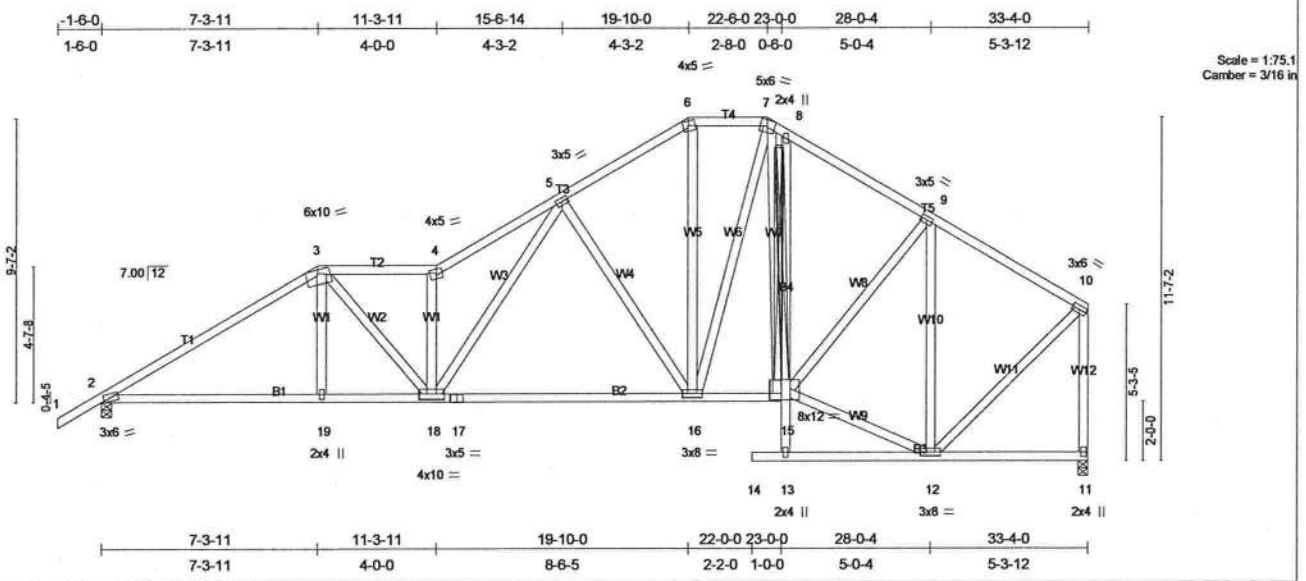

Plate Offsets (X,Y): [2:0-1-2,0-0-7], [15:0-4-11,0-2-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.16	16-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.34	16-18	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.08	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 257 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-4-12 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 8-15
WEBS T-Brace: 2 X 4 SYP No.3 - 7-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1153/0-4-0, 11=1062/0-4-0
Max Horz 2=270(load case 5)
Max Uplift2=-312(load case 6), 11=-187(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1757/783, 3-4=-1796/895, 4-5=-2158/1122, 5-6=-1122/650, 6-7=-918/605, 7-8=-992/670, 8-9=-1046/599, 9-10=-751/374, 10-11=-1031/498
BOT CHORD 2-19=-694/1423, 18-19=-694/1425, 17-18=-576/1289, 16-17=-576/1289, 15-16=-296/830, 13-15=0/105, 8-15=-159/140, 13-14=0/0, 12-13=-2/40, 11-12=-14/22
WEBS 3-19=0/164, 3-18=-219/557, 4-18=-1213/657, 5-18=-525/1007, 5-16=-691/451, 6-16=-198/352, 7-16=-145/413, 7-15=-154/160, 12-15=-272/612, 9-15=-108/394, 9-12=-748/379, 10-12=-335/798

JOINT STRESS INDEX
2 = 0.83, 3 = 0.74, 4 = 0.76, 5 = 0.81, 6 = 0.40, 7 = 0.32, 8 = 0.34, 9 = 0.49, 10 = 0.77, 11 = 0.50, 12 = 0.79, 13 = 0.45, 15 = 0.21, 16 = 0.68, 17 = 0.44, 18 = 0.66 and 19 = 0.34

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 2 and 187 lb uplift at joint 11.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T14	SPECIAL	1	1	296128040
					Job Reference (optional)

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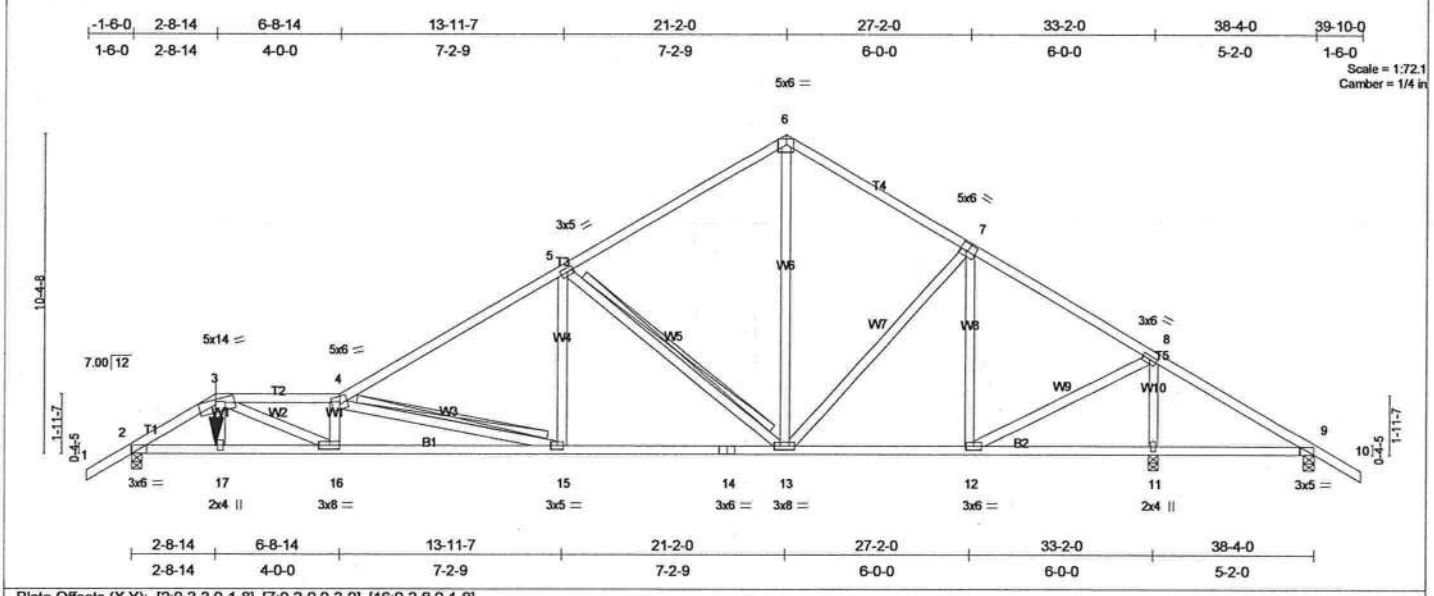

Plate Offsets (X,Y): [2:0-3-3,0-1-8], [7:0-3-0,0-3-0], [16:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.21	15-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.43	15-16	>913	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.64	Horz(TL)	0.07	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 220 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-15, 5-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1115/0-4-0, 11=1877/0-4-0, 9=-319/0-4-0
Max Horz 2=-278(load case 3)
Max Uplift2=-329(load case 5), 11=-444(load case 5), 9=-428(load case 9)
Max Grav 2=1115(load case 1), 11=1877(load case 1), 9=62(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1760/399, 3-4=-3065/731, 4-5=-1604/357, 5-6=-850/256, 6-7=-824/260, 7-8=-610/175, 8-9=-270/1111, 9-10=0/40
BOT CHORD 2-17=-404/1448, 16-17=-403/1455, 15-16=-804/3123, 14-15=-287/1311, 13-14=-287/1311, 12-13=0/457, 11-12=-880/259, 9-11=-880/259
WEBS 3-17=-10/100, 3-16=-416/1752, 4-16=-662/209, 4-15=-1858/531, 5-15=-76/519, 5-13=-858/320, 6-13=-124/417, 7-13=-142/371, 7-12=-630/174, 8-12=-283/1499, 8-11=-1802/404

JOINT STRESS INDEX
2 = 0.75, 3 = 0.53, 4 = 0.58, 5 = 0.49, 6 = 0.45, 7 = 0.36, 8 = 0.93, 9 = 0.46, 11 = 0.66, 12 = 0.86, 13 = 0.57, 14 = 0.45, 15 = 0.63, 16 = 0.70 and 17 = 0.34

NOTES (10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 2, 444 lb uplift at joint 11 and 428 lb uplift at joint 9.
7) Girder carries hip end with 31-7-2 right side setback, 2-8-14 left side setback, and 2-8-14 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 36 lb down and 20 lb up at 2-8-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-59(F=-5), 4-6=-54, 6-10=-54, 2-17=-10, 16-17=-11(F=-1), 9-16=-10
Concentrated Loads (lb)
Vert: 17=-36(F)

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T12	SPECIAL	1	1	296128038
					Job Reference (optional)

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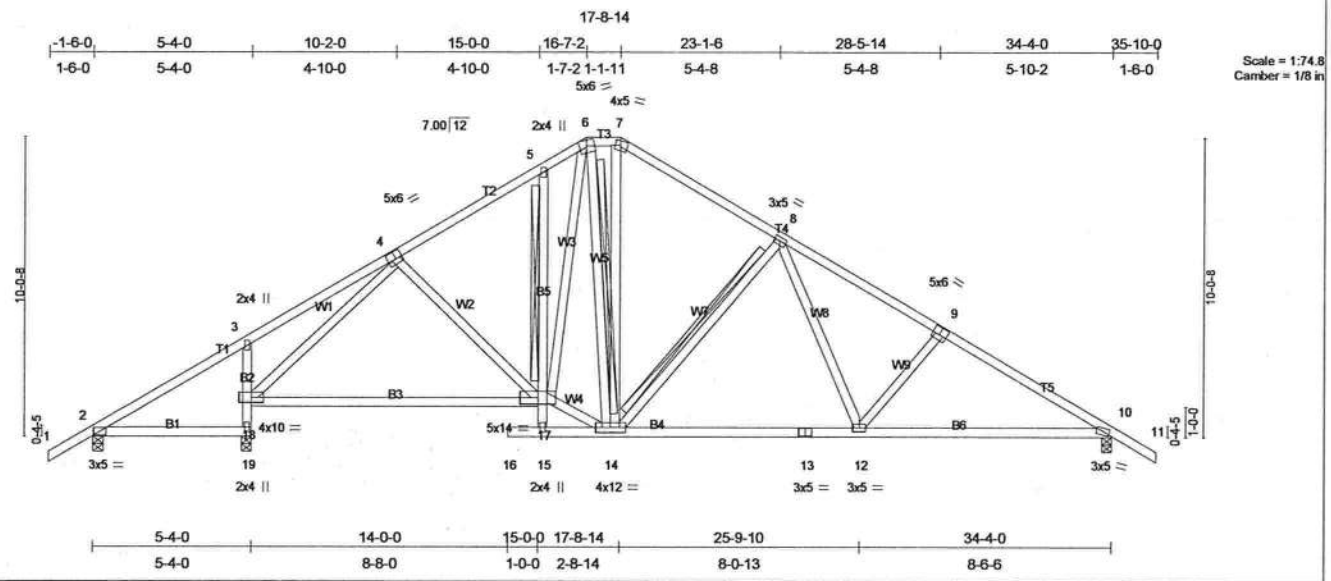

Plate Offsets (X,Y): [4:0-2-12,0-3-0], [9:0-3-0,0-3-0], [10:0-1-10,0-0-7]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0		Plates Increase 1.25	TC 0.23	Vert(LL) -0.14 17-18 >999 360	MT20 244/190
TCDL 7.0		Lumber Increase 1.25	BC 0.57	Vert(TL) -0.26 17-18 >999 240	
BCLL 10.0	*	Rep Stress Incr YES	WB 0.92	Horz(TL) 0.05 10 n/a n/a	
BCDL 5.0		Code FBC2004/TPI2002	(Matrix)		Weight: 227 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-1 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 5-17
WEBS T-Brace: 2 X 4 SYP No.3 - 6-14, 8-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=201/0-4-0, 19=1153/0-4-0, 10=1013/0-4-0
Max Horz 2=-269(load case 4)
Max Uplift2=-234(load case 4), 19=-294(load case 6), 10=-299(load case 7)
Max Grav 2=212(load case 10), 19=1153(load case 1), 10=1013(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-143/348, 3-4=-59/293, 4-5=-958/554, 5-6=-917/658, 6-7=-671/534, 7-8=-845/549, 8-9=-1321/692, 9-10=-1514/702, 10-11=0/40
BOT CHORD 2-19=-106/0, 18-19=-1123/417, 3-18=-293/232, 17-18=-196/729, 15-17=-12/2, 5-17=-208/198, 15-16=0/0, 14-15=-226/0, 13-14=-265/960, 12-13=-265/960, 10-12=-458/1231
WEBS 14-17=0/771, 6-17=-264/533, 6-14=-220/138, 7-14=-71/185, 8-14=-453/326, 8-12=-133/355, 9-12=-249/226, 4-17=-33/150, 4-18=-1094/301

JOINT STRESS INDEX
2 = 0.43, 3 = 0.48, 4 = 0.47, 5 = 0.44, 6 = 0.34, 7 = 0.69, 8 = 0.49, 9 = 0.42, 10 = 0.80, 12 = 0.48, 13 = 0.32, 14 = 0.57, 15 = 0.64, 17 = 0.76, 18 = 0.83 and 19 = 0.66

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 2, 294 lb uplift at joint 19 and 299 lb uplift at joint 10.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T10	SPECIAL	1	1	296128036

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Jan 13 09:03:08 2009 Page 1

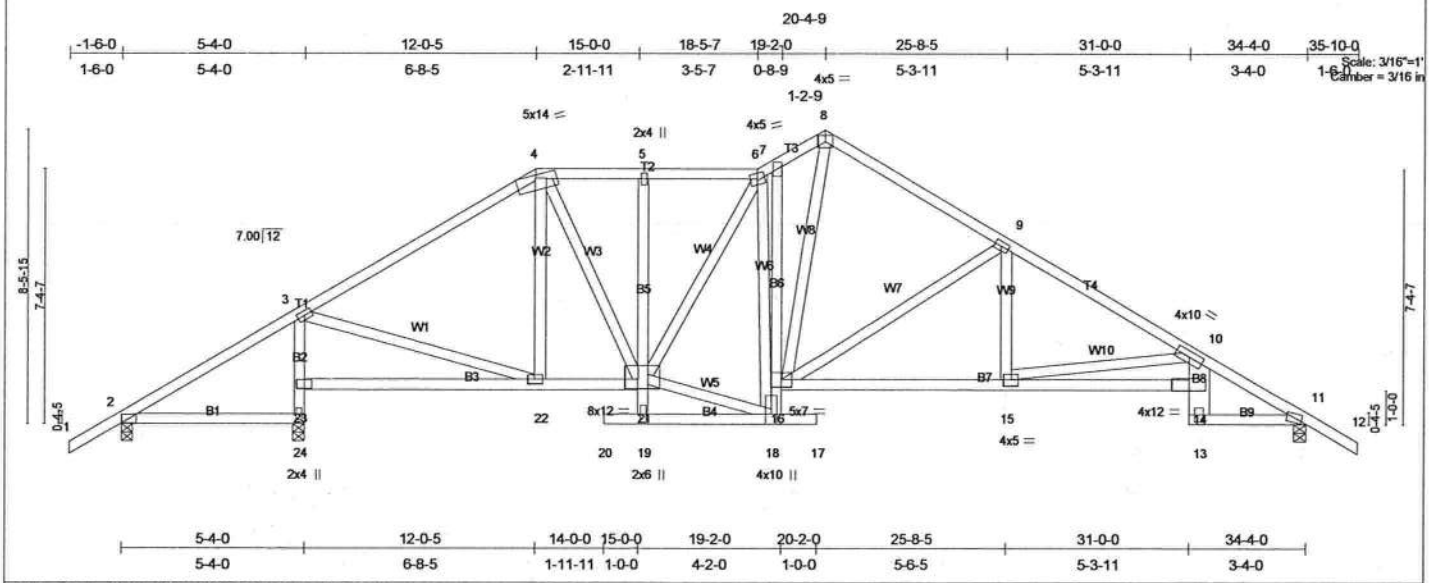

Plate Offsets (X,Y): [11:0-1-14,0-0-7], [21:0-4-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.21	17	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.41	17	>842	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.21	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3, B6 2 X 4 SYP No.1D, B8 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-0-10 oc bracing.

REACTIONS (lb/size) 2=92/0-4-0, 24=1287/0-4-0, 11=999/0-4-0
Max Horz 2=-226(load case 4)
Max Uplift2=-202(load case 4), 24=-331(load case 6), 11=-278(load case 7)
Max Grav 2=142(load case 10), 24=1287(load case 1), 11=999(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-97/358, 3-4=-950/522, 4-5=-917/586, 5-6=-919/589, 6-7=-1230/701, 7-8=-1058/668, 8-9=-1024/572, 9-10=-1655/756, 10-11=-1527/667, 11-12=0/40
BOT CHORD 2-24=-232/20, 23-24=-1276/475, 3-23=-1227/491, 22-23=-259/42, 21-22=-156/730, 19-21=-89/40, 5-21=-117/123, 19-20=0/0, 18-19=-66/14, 17-18=0/0, 16-18=-396/943, 7-16=-88/421, 15-16=-441/1373, 14-15=-1052/2665, 13-14=0/59, 10-14=0/103, 11-13=-451/1235
WEBS 3-22=-200/1037, 4-22=-245/98, 4-21=-144/409, 18-21=-264/1056, 6-21=-144/53, 6-18=-946/435, 8-16=-445/742, 9-16=-650/356, 9-15=-76/391, 10-15=-1306/617

JOINT STRESS INDEX
2 = 0.47, 3 = 0.64, 4 = 0.94, 5 = 0.34, 6 = 0.48, 7 = 0.72, 8 = 0.55, 9 = 0.49, 10 = 0.73, 11 = 0.78, 13 = 0.68, 14 = 0.91, 15 = 0.45, 16 = 0.78, 18 = 0.67, 19 = 0.57, 21 = 0.20, 22 = 0.67, 23 = 0.82 and 24 = 0.63

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x5 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 2, 331 lb uplift at joint 24 and 278 lb uplift at joint 11.
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T08	SPECIAL	1	1	296128034

Job Reference (optional)

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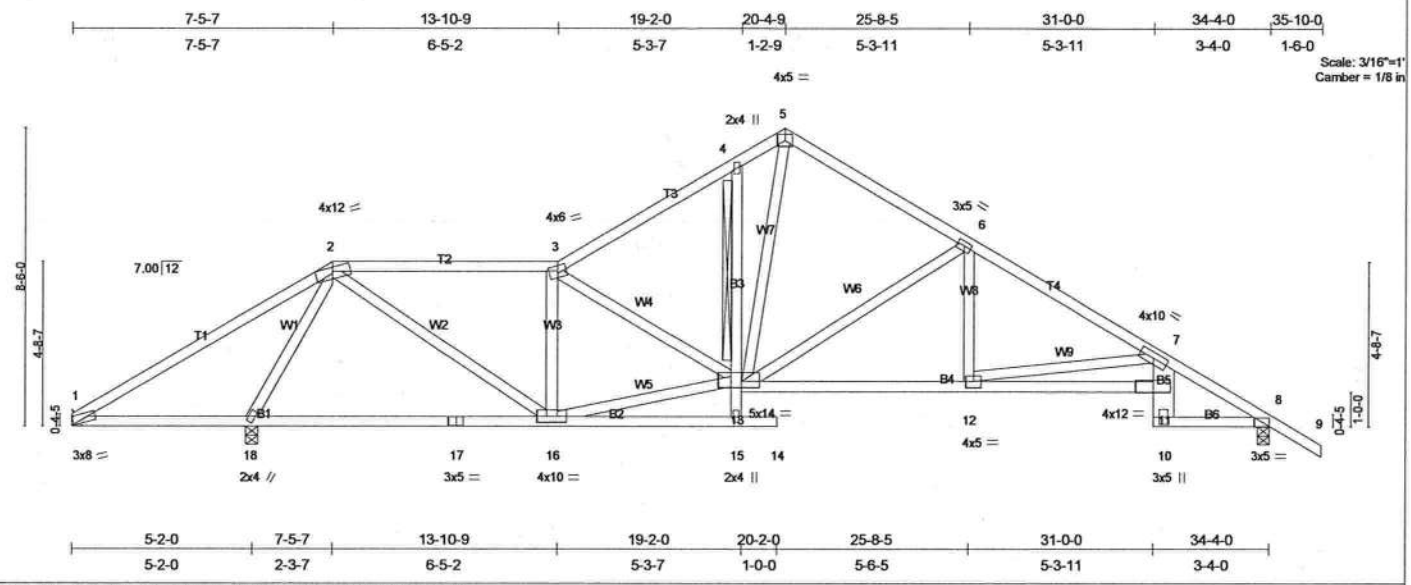

Plate Offsets (X,Y): [1:0-0-14,0-0-3], [8:Edge,0-0-2], [13:0-6-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	0.14	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.26	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3, B5 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 4-13

REACTIONS (lb/size) 1=-355/Mechanical, 8=942/0-4-0, 18=1690/0-4-0
Max Horz 1=-243(load case 4)
Max Uplift1=-355(load case 1), 8=-260(load case 7), 18=-425(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-381/1091, 2-3=-920/541, 3-4=-1081/556, 4-5=-988/613, 5-6=-923/497, 6-7=-1504/668, 7-8=-1422/600, 8-9=0/40
BOT CHORD 1-18=-841/456, 17-18=-180/134, 16-17=-180/134, 15-16=-43/45, 14-15=0/0, 13-15=0/95, 4-13=-169/131, 12-13=-365/1242, 11-12=-940/2508, 10-11=0/58, 7-11=0/114, 8-10=-395/1147
WEBS 2-18=-1768/824, 3-16=-762/374, 13-16=-268/916, 3-13=-194/178, 5-13=-419/717, 6-13=-600/343, 6-12=-62/348, 7-12=-1279/582, 2-16=-440/1127

JOINT STRESS INDEX
1 = 0.82, 2 = 0.51, 3 = 0.72, 4 = 0.42, 5 = 0.51, 6 = 0.49, 7 = 0.71, 8 = 0.74, 10 = 0.63, 11 = 0.82, 12 = 0.44, 13 = 0.54, 15 = 0.42, 16 = 0.53, 17 = 0.42 and 18 = 0.76

NOTES (7-8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 355 lb uplift at joint 1, 260 lb uplift at joint 8 and 425 lb uplift at joint 18.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T06	SPECIAL	1	1	296128032
					Job Reference (optional)

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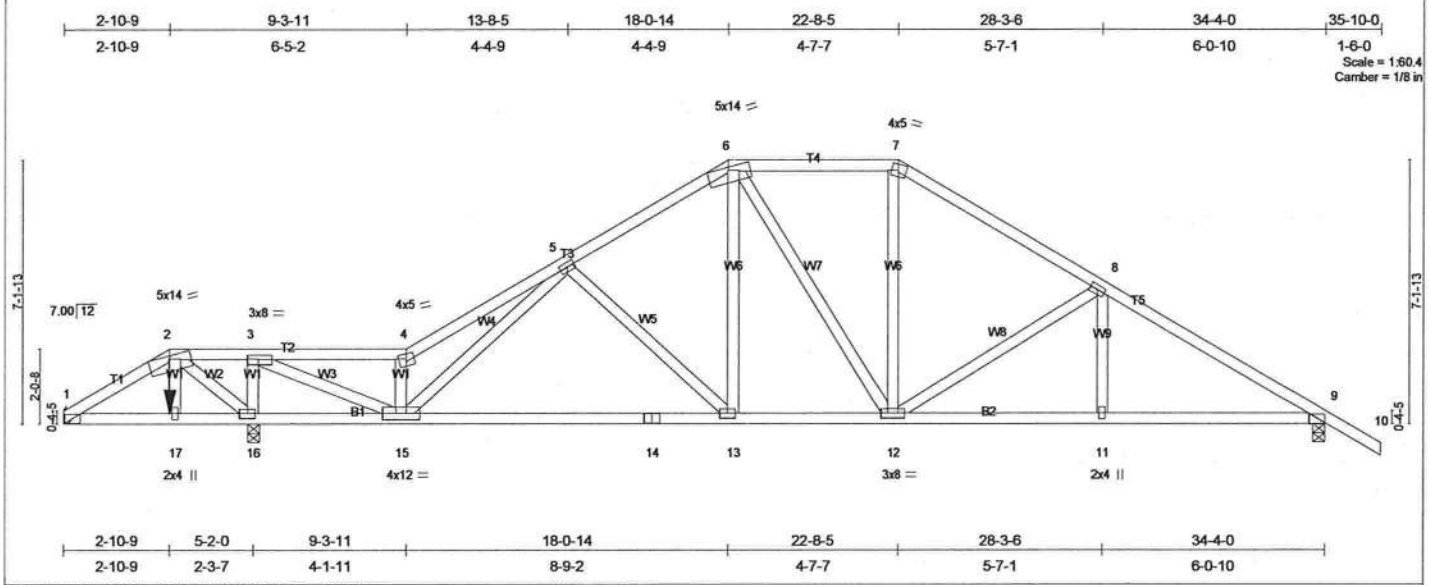

Plate Offsets (X,Y): [3:0-3-8,0-1-8], [9:Edge,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.15	13-15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.30	13-15	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.70	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-9 oc bracing.

REACTIONS (lb/size) 1=-311/Mechanical, 9=940/0-4-0, 16=1758/0-4-0
Max Horz 1=-206(load case 3)
Max Uplift1=-311(load case 1), 9=-249(load case 6), 16=-499(load case 5)
Max Grav 1=4(load case 5), 9=940(load case 1), 16=1758(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-108/767, 2-3=-291/1308, 3-4=-706/126, 4-5=-936/202, 5-6=-979/213, 6-7=-816/234, 7-8=-1020/226, 8-9=-1389/258, 9-10=0/40
BOT CHORD 1-17=-614/152, 16-17=-610/145, 15-16=-1308/294, 14-15=-188/919, 13-14=-188/919, 12-13=-120/797, 11-12=-113/1120, 9-11=-113/1120
WEBS 2-17=-87/92, 4-15=-674/213, 5-15=-216/40, 5-13=-190/139, 6-13=-53/264, 6-12=-113/144, 7-12=-41/251, 8-12=-366/164, 8-11=0/198, 3-16=-1149/290, 3-15=-438/2195, 2-16=-880/303

JOINT STRESS INDEX
1 = 0.34, 2 = 0.47, 3 = 0.87, 4 = 0.57, 5 = 0.49, 6 = 0.57, 7 = 0.58, 8 = 0.49, 9 = 0.73, 11 = 0.34, 12 = 0.59, 13 = 0.40, 14 = 0.41, 15 = 0.83, 16 = 0.43 and 17 = 0.34

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x5 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 1, 249 lb uplift at joint 9 and 499 lb uplift at joint 16.
8) Girder carries hip end with 25-0-5 right side setback, 2-10-9 left side setback, and 2-10-9 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 42 lb down and 24 lb up at 2-10-9 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-64(F=-10), 4-6=-54, 6-7=-54, 7-10=-54, 1-17=-10, 15-17=-12(F=-2), 9-15=-10
Concentrated Loads (lb)
Vert: 17=-42(F)

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T04	HIP	1	1	296128030
					Job Reference (optional)

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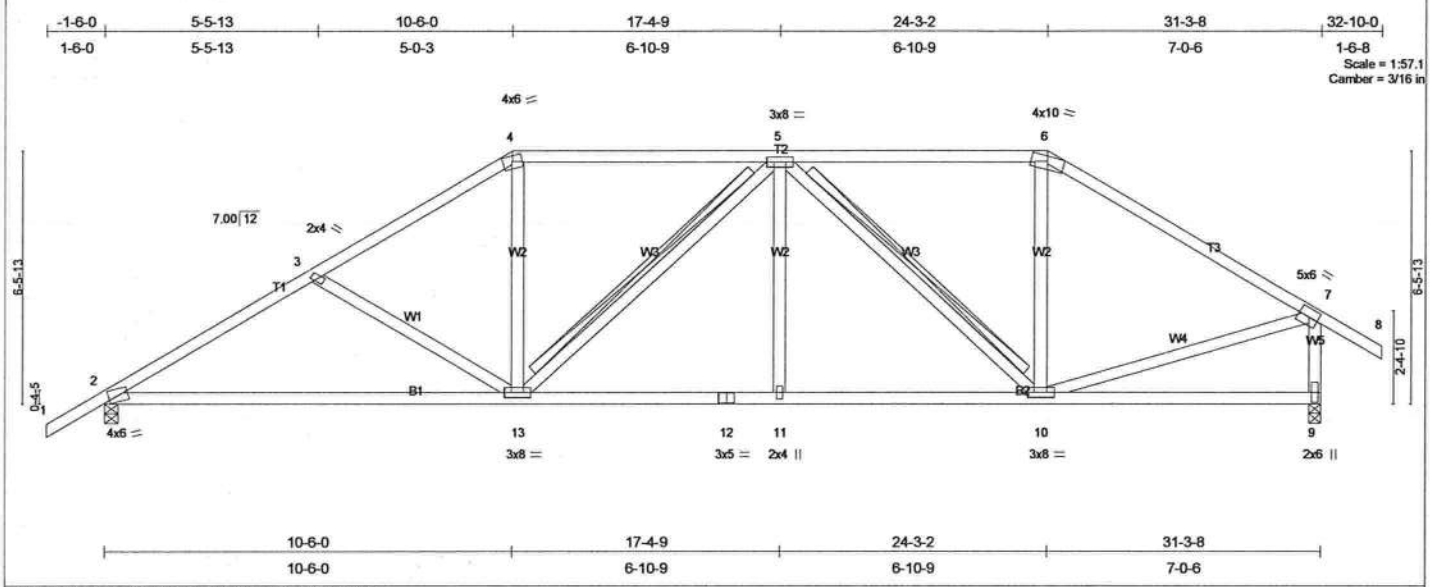

Plate Offsets (X,Y): [2:0-1-10,0-0-11], [7:0-2-9,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.25	2-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.47	2-13	>794	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-6 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-13, 5-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1081/0-4-0, 9=1083/0-3-8
Max Horz 2=200(load case 5)
Max Uplift2=-259(load case 6), 9=-227(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1635/796, 3-4=-1373/714, 4-5=-1137/680, 5-6=-896/582, 6-7=-1126/587, 7-8=0/46, 7-9=-1045/591
BOT CHORD 2-13=-544/1340, 12-13=-434/1250, 11-12=-434/1250, 10-11=-434/1250, 9-10=0/79
WEBS 3-13=-249/219, 4-13=-124/390, 5-13=-266/197, 5-11=0/168, 5-10=-547/245, 6-10=-49/267, 7-10=-281/850

JOINT STRESS INDEX
2 = 0.74, 3 = 0.34, 4 = 0.70, 5 = 0.57, 6 = 0.75, 7 = 0.74, 9 = 0.60, 10 = 0.78, 11 = 0.34, 12 = 0.42 and 13 = 0.57

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 259 lb uplift at joint 2 and 227 lb uplift at joint 9.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T02J	SPECIAL	1	1	296128028
					Job Reference (optional)

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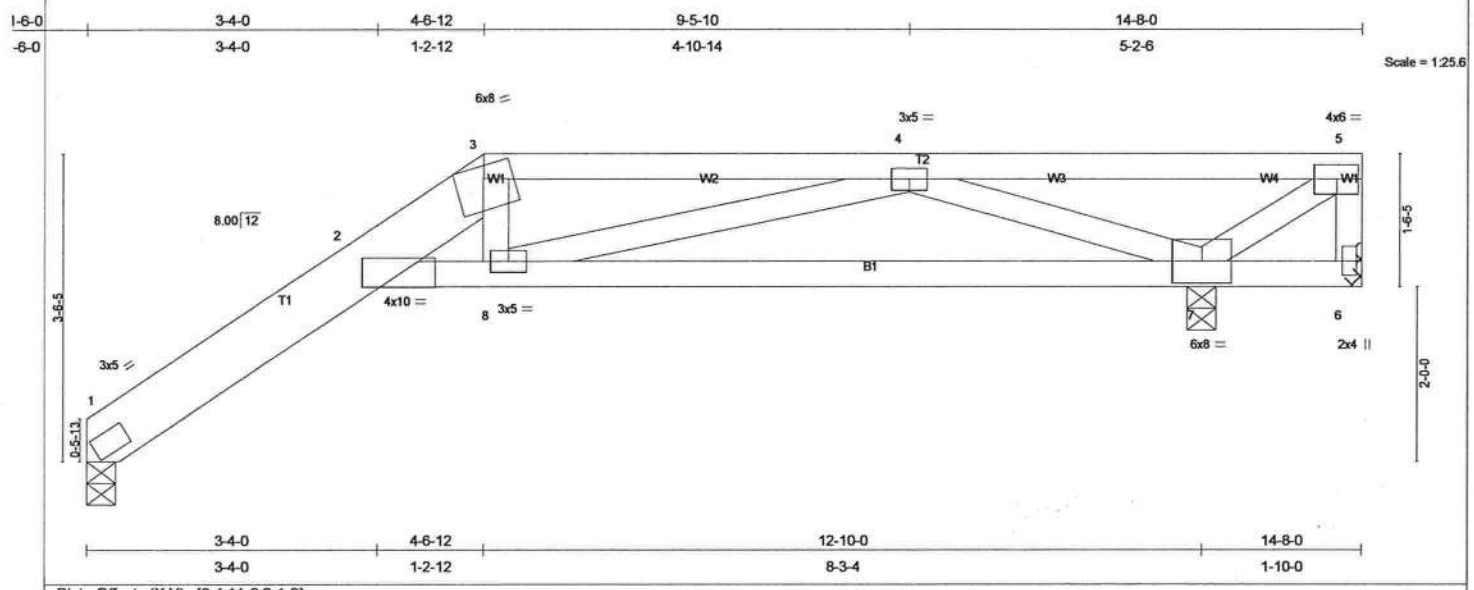

Plate Offsets (X,Y): [3:4-11-0,0-1-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	0.27	7-8	>555	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.15	7-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.46	Horz(TL)	-0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=332/0-4-0, 6=-531/Mechanical, 7=1123/0-4-0
Max Horz 1=100(load case 6)
Max Uplift1=-110(load case 6), 6=-531(load case 1), 7=-684(load case 5)
Max Grav 1=332(load case 1), 6=344(load case 5), 7=1123(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-160/58, 2-3=-726/1236, 3-4=-814/1324, 4-5=-1023/776, 5-6=-628/483
BOT CHORD 2-8=-1286/813, 7-8=-328/305, 6-7=-18/65
WEBS 3-8=-315/190, 4-8=-1032/523, 4-7=-1151/1439, 5-7=-1041/1257

JOINT STRESS INDEX
2 = 0.41, 3 = 0.39, 3 = 0.00, 4 = 0.82, 5 = 0.73, 6 = 0.75, 7 = 0.29 and 8 = 0.33

NOTES (7-8)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 1, 531 lb uplift at joint 6 and 684 lb uplift at joint 7.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T02H	SPECIAL	1	1	296128026
					Job Reference (optional)

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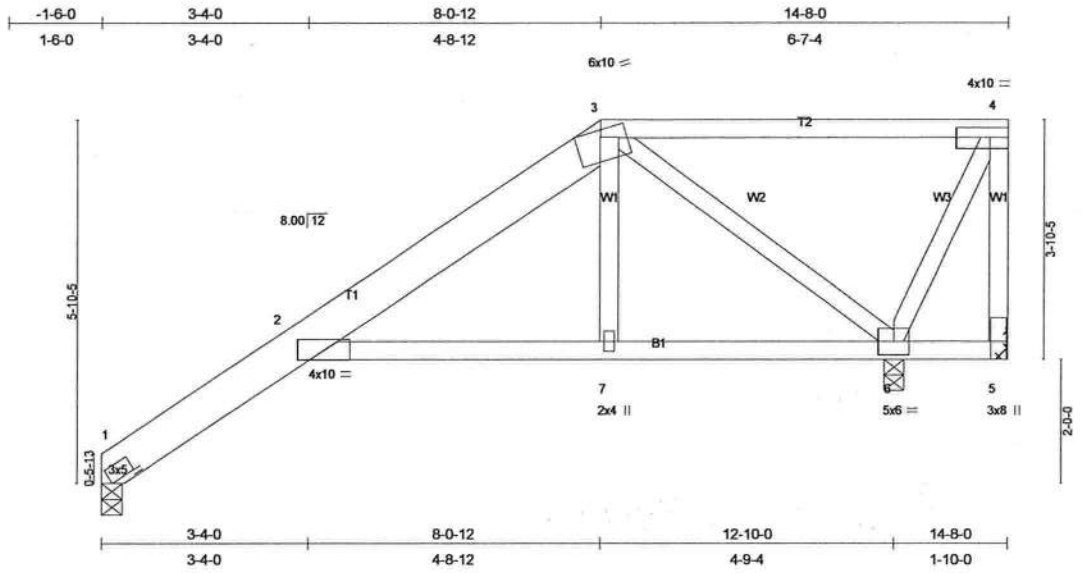

Plate Offsets (X,Y): [5:12-8-14,6-0-14]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.66	Vert(LL)	0.10	2-7	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.12	2-7	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.07	5	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 81 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D *Except*
T2 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=285/0-4-0, 5=-886/Mechanical, 6=1525/0-4-0
Max Horz 1=175(load case 6)
Max Uplift1=-9(load case 6), 5=-886(load case 1), 6=-555(load case 6)
Max Grav 1=285(load case 1), 5=389(load case 6), 6=1525(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-185/4, 2-3=-152/101, 3-4=-354/462, 4-5=-662/879
BOT CHORD 2-7=-120/140, 6-7=-119/143, 5-6=-30/32
WEBS 3-7=0/153, 3-6=-744/580, 4-6=-1150/890

JOINT STRESS INDEX
2 = 0.13, 3 = 0.66, 4 = 0.65, 5 = 0.31, 5 = 0.00, 6 = 0.31 and 7 = 0.11

NOTES (7-8)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1, 886 lb uplift at joint 5 and 555 lb uplift at joint 6.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	THE CHARLES
296128	T02F	SPECIAL	1	1	296128024

Job Reference (optional)

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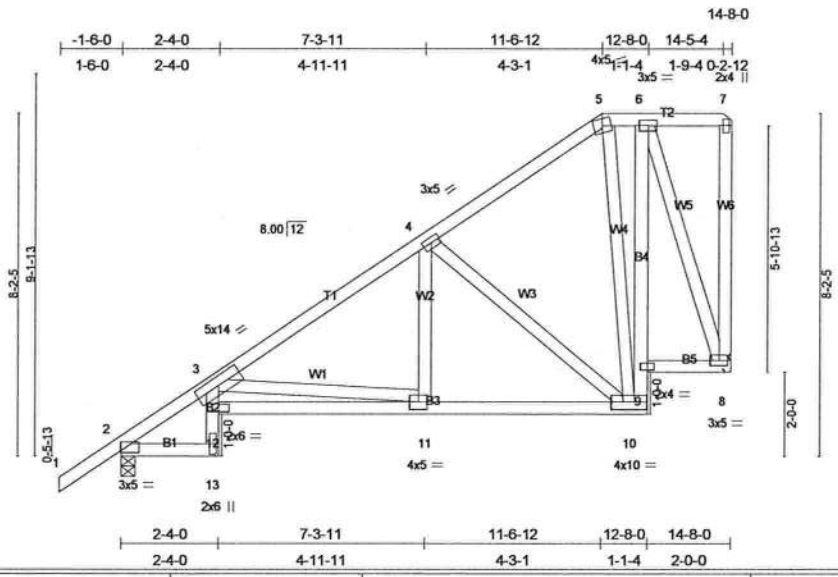

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.10	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.12	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-4-6 oc bracing: 11-12.

REACTIONS (lb/size) 2=554/0-4-0, 8=452/Mechanical
Max Horz 2=296(load case 6)
Max Uplift2=-130(load case 6), 8=-161(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-670/135, 3-4=-599/174, 4-5=-238/68, 5-6=-131/96, 6-7=-5/4, 7-8=-52/42
BOT CHORD 2-13=-370/443, 12-13=-36/89, 3-12=-2/146, 11-12=-936/1139, 10-11=-345/441, 9-10=-281/344, 6-9=-240/308, 8-9=-95/129
WEBS 4-11=-63/241, 3-11=-703/596, 5-10=-53/86, 4-10=-394/311, 6-8=-388/284

JOINT STRESS INDEX
2 = 0.62, 3 = 0.83, 4 = 0.22, 5 = 0.29, 6 = 0.23, 7 = 0.14, 8 = 0.21, 9 = 0.38, 10 = 0.33, 11 = 0.24, 12 = 0.88 and 13 = 0.79

NOTES (6-7)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 2 and 161 lb uplift at joint 8.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T02D	SPECIAL	1	1	296128022
					Job Reference (optional)

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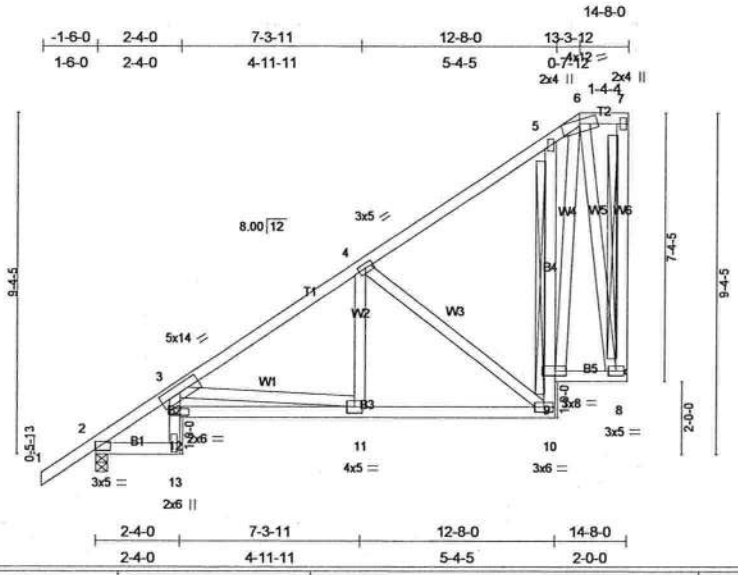

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	0.10	11-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.12	11-12	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 116 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-14 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 5-9
WEBS T-Brace: 2 X 4 SYP No.3 - 7-8
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=454/Mechanical, 2=555/0-4-0
Max Horz 2=333(load case 6)
Max Uplift8=-200(load case 6), 2=-110(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-670/84, 3-4=-601/121, 4-5=-229/10, 5-6=-167/119, 6-7=-5/4, 7-8=-28/18
BOT CHORD 2-13=-380/442, 12-13=-39/89, 3-12=-7/146, 11-12=-950/1130, 10-11=-356/446, 9-10=-190/279, 5-9=-175/195, 8-9=-53/68
WEBS 3-11=-690/599, 4-11=-55/241, 4-10=-412/330, 6-9=-331/414, 6-8=-375/290

JOINT STRESS INDEX
2 = 0.63, 3 = 0.80, 4 = 0.22, 5 = 0.33, 6 = 0.19, 7 = 0.15, 8 = 0.21, 9 = 0.85, 10 = 0.64, 11 = 0.24, 12 = 0.87 and 13 = 0.80

NOTES (7-8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 200 lb uplift at joint 8 and 110 lb uplift at joint 2.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted]THE CHARLES
296128	T02B	MONO TRUSS	3	1	296128020
					Job Reference (optional)

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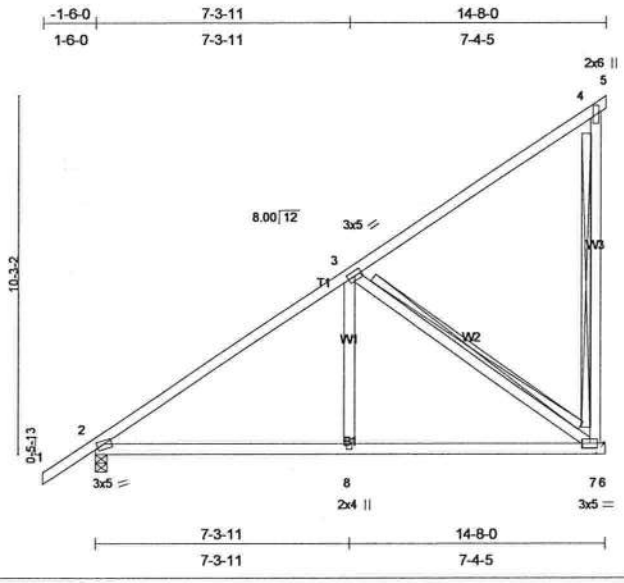

Plate Offsets (X,Y): [2:0-0-14,0-0-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.05	2-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.10	2-8	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-7, 3-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 7=456/Mechanical, 2=550/0-4-0
Max Horz 2=360(load case 6)
Max Uplift7=-234(load case 6), 2=-91(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-565/0, 3-4=-137/74, 4-5=-2/0, 4-7=-160/160
BOT CHORD 2-8=-304/381, 7-8=-304/381, 6-7=0/0
WEBS 3-8=0/247, 3-7=-450/359

JOINT STRESS INDEX
2 = 0.63, 3 = 0.22, 4 = 0.59, 7 = 0.34 and 8 = 0.18

NOTES (5-6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 7 and 91 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
6) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	T02	MONO HIP	1	1	296128018
					Job Reference (optional)

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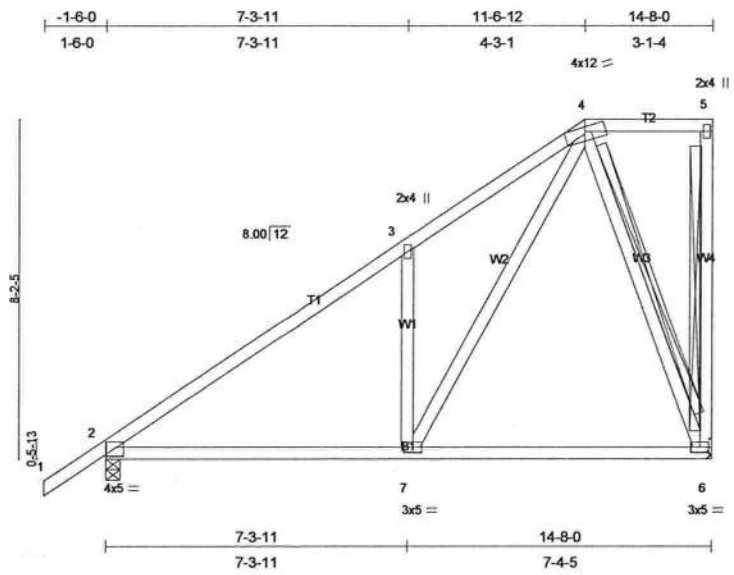

Plate Offsets (X,Y): [2:0-0-3,0-0-9]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.36	Vert(LL)	0.07	2-7	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.11	2-7	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 93 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-6, 4-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 6=454/Mechanical, 2=555/0-4-0
Max Horz 2=296(load case 6)
Max Uplift6=-160(load case 6), 2=-129(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-565/75, 3-4=-536/300, 4-5=-11/0, 5-6=-75/53
BOT CHORD 2-7=-287/374, 6-7=-109/137
WEBS 3-7=-339/361, 4-6=-375/312, 4-7=-376/500

JOINT STRESS INDEX
2 = 0.60, 3 = 0.19, 4 = 0.22, 5 = 0.27, 6 = 0.38 and 7 = 0.41

NOTES (6-7)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 160 lb uplift at joint 6 and 129 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted]/ THE CHARLES
296128	T01C	COMMON	2	1	296128016 Job Reference (optional)

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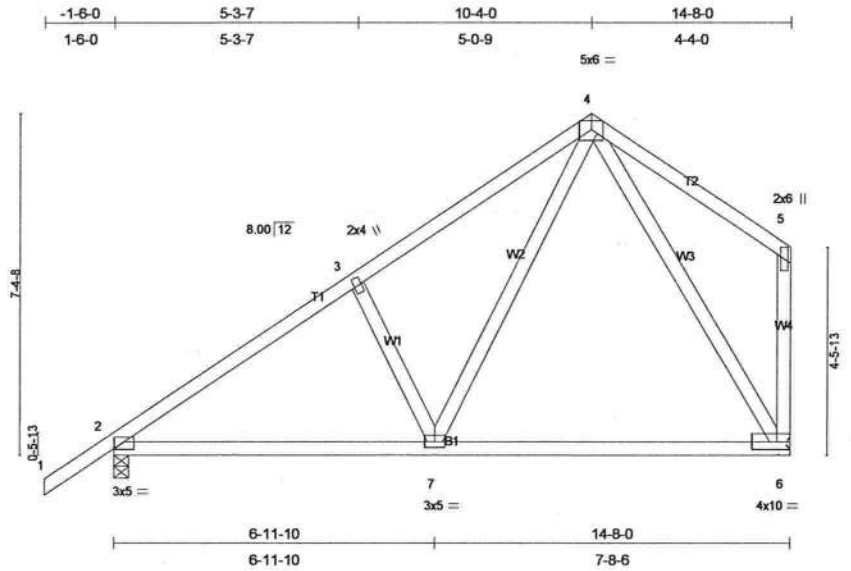

Plate Offsets (X,Y): [2:0-0-3,0-0-1]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.97	Vert(LL)	0.17	6-7	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.33	6-7	>528	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 86 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=674/0-4-0, 6=788/Mechanical
Max Horz 2=212(load case 6)
Max Uplift2=-195(load case 6), 6=-205(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-853/338, 3-4=-725/397, 4-5=-175/121, 5-6=-186/129
BOT CHORD 2-7=-370/626, 6-7=-141/292
WEBS 3-7=-212/226, 4-7=-305/584, 4-6=-406/205

JOINT STRESS INDEX
2 = 0.73, 3 = 0.14, 4 = 0.32, 5 = 0.65, 6 = 0.59 and 7 = 0.49

NOTES (7-8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 2 and 205 lb uplift at joint 6.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 2-7=-10, 6-7=-70(F=-60)

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	T01A	COMMON	1	1	296128014 Job Reference (optional)

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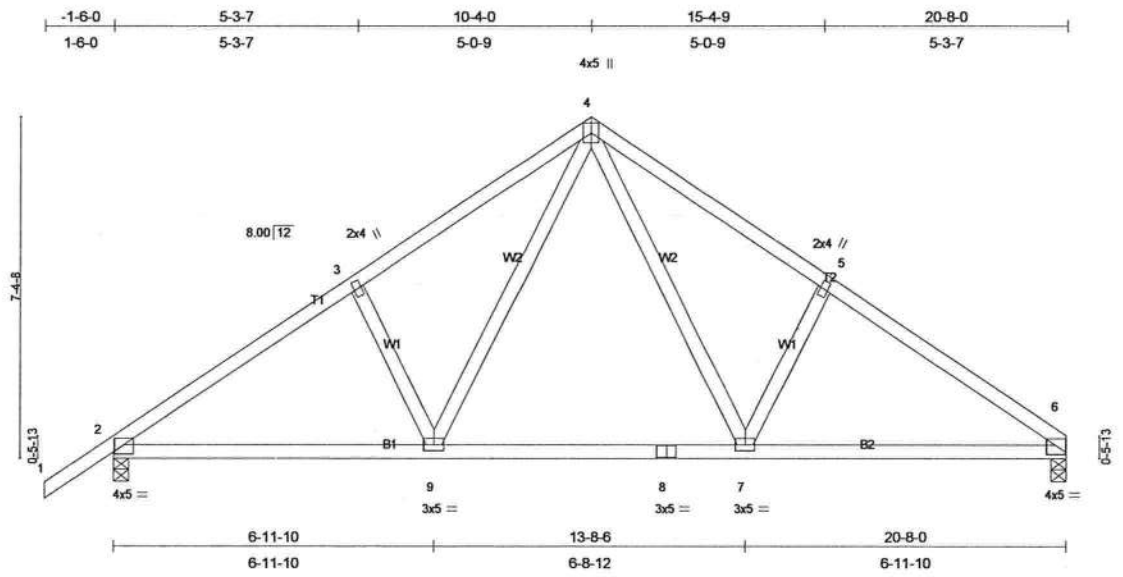

Plate Offsets (X,Y): [2:0-0-3,0-0-9], [6:0-0-3,0-0-9]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.24	Vert(LL)	0.15	7-9	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.27	7-9	>907	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.33	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-2 oc bracing.

REACTIONS (lb/size) 6=849/0-4-0, 2=946/0-4-0
Max Horz 2=212(load case 5)
Max Uplift6=-191(load case 7), 2=-265(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1305/579, 3-4=-1174/636, 4-5=-1185/652, 5-6=-1315/593
BOT CHORD 2-9=-377/993, 8-9=-170/695, 7-8=-170/695, 6-7=-395/1006
WEBS 3-9=-197/207, 4-9=-273/531, 4-7=-302/550, 5-7=-207/219

JOINT STRESS INDEX
2 = 0.69, 3 = 0.34, 4 = 0.64, 5 = 0.34, 6 = 0.69, 7 = 0.46, 8 = 0.62 and 9 = 0.46

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 6 and 265 lb uplift at joint 2.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-70(F=-60), 6-7=-10

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	HJ3	JACK	1	1	296128012
					Job Reference (optional)

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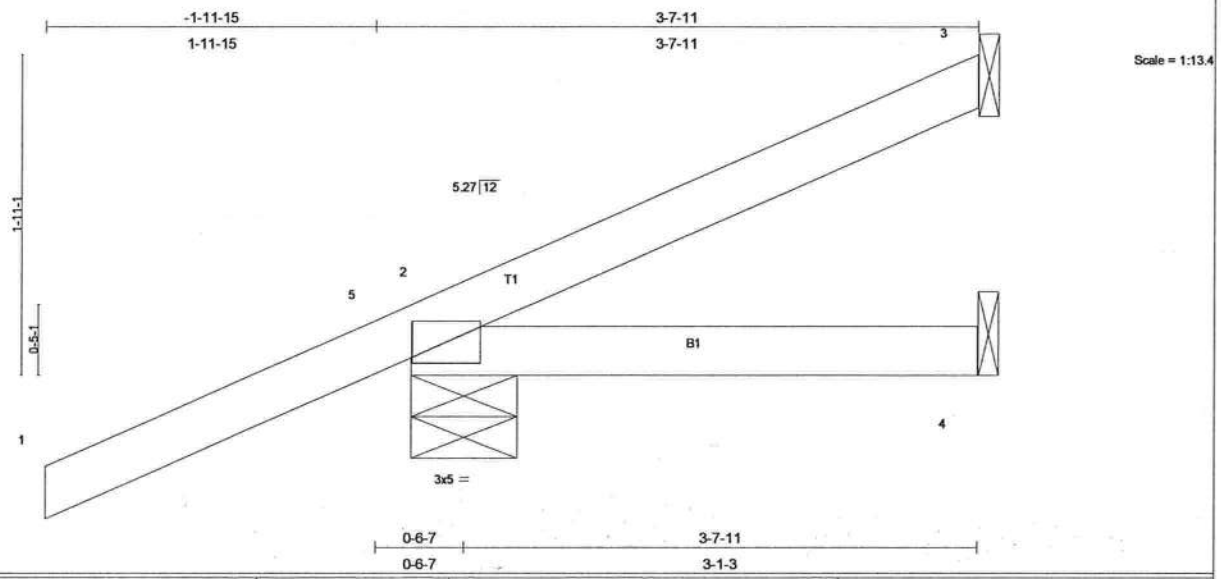

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.36	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 14 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-7/Mechanical, 2=209/0-7-13, 4=9/Mechanical
Max Horz 2=76(load case 5)
Max Uplift3=-7(load case 1), 2=-191(load case 5)
Max Grav 3=42(load case 7), 2=209(load case 1), 4=39(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-5=0/38, 2-5=0/38, 2-3=-39/19
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 3 and 191 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54
Trapezoidal Loads (plf)
Vert: 5=0(F=27, B=27)-to-3=-46(F=4, B=4), 2=-1(F=5, B=5)-to-4=-9(F=1, B=1)

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	EJ7G	GABLE	1	1	296128010 Job Reference (optional)

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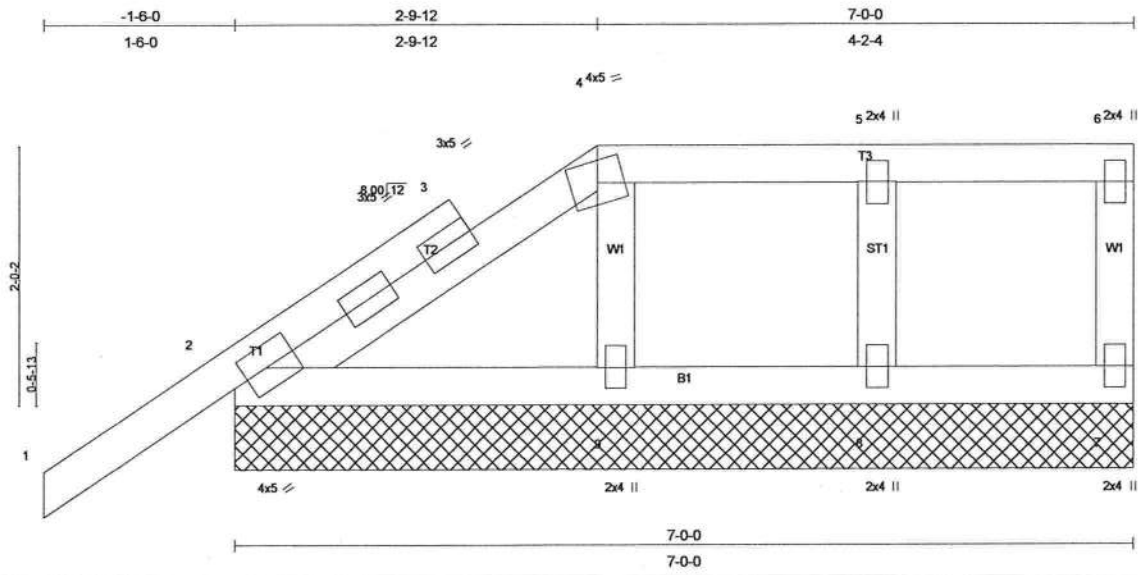

Plate Offsets (X,Y): [2:0-2-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	1	n/r	90		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=193/7-0-0, 7=53/7-0-0, 8=132/7-0-0, 9=141/7-0-0
Max Horz 2=133(load case 6)
Max Uplift2=-138(load case 6), 7=-29(load case 4), 8=-100(load case 4), 9=-55(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/39, 2-3=-62/9, 3-4=-24/18, 4-5=-7/5, 5-6=-6/5, 6-7=-43/46
BOT CHORD 2-9=-15/16, 8-9=-5/6, 7-8=-5/6
WEBS 5-8=-115/130, 4-9=-116/112

JOINT STRESS INDEX
2 = 0.76, 3 = 0.00, 3 = 0.18, 3 = 0.19, 4 = 0.13, 5 = 0.07, 6 = 0.09, 7 = 0.05, 8 = 0.07 and 9 = 0.06

NOTES (9)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2, 29 lb uplift at joint 7, 100 lb uplift at joint 8 and 55 lb uplift at joint 9.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	EJ7E	MONO HIP	1	1	296128008
					Job Reference (optional)

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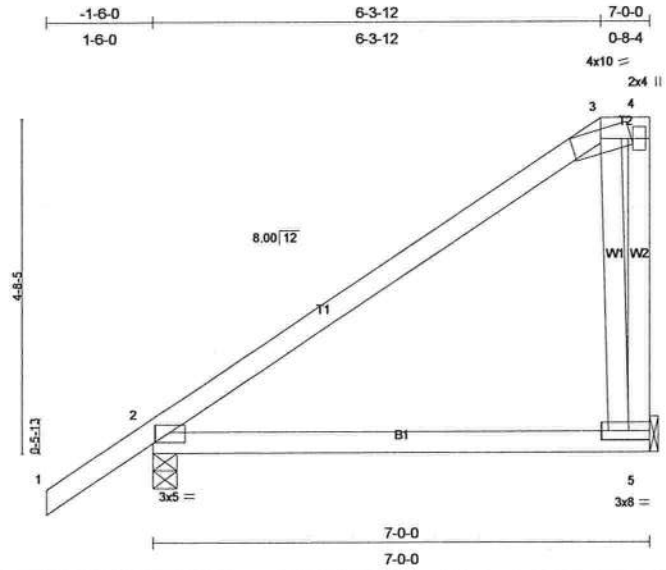

Plate Offsets (X,Y): [2:0-0-7,0-0-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.05	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.09	2-5	>867	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=203/Mechanical, 2=315/0-4-0
Max Horz 2=183(load case 6)
Max Uplift5=-88(load case 6), 2=-94(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-174/19, 3-4=-32/14, 4-5=-284/230
BOT CHORD 2-5=-65/67
WEBS 3-5=-389/476

JOINT STRESS INDEX
2 = 0.69, 3 = 0.64, 4 = 0.53 and 5 = 0.59

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 5 and 94 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	EJ7C	MONO HIP	1	1	296128006

Job Reference (optional)

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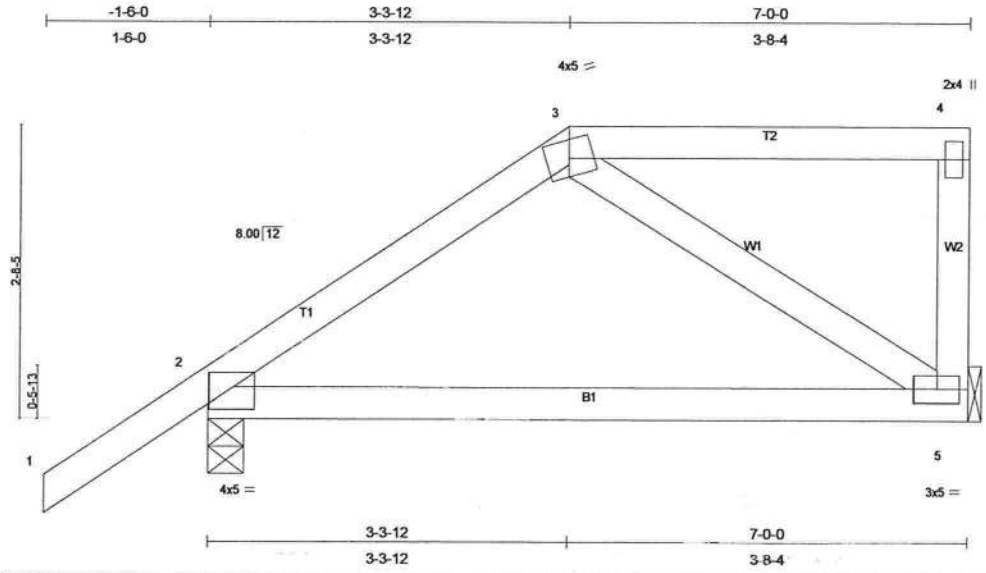

Plate Offsets (X,Y): [2:0-0-3,0-0-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.04	2-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.07	2-5	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=203/Mechanical, 2=315/0-4-0
Max Horz 2=119(load case 6)
Max Uplift5=-59(load case 5), 2=-116(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-232/123, 3-4=-49/6, 4-5=-92/101
BOT CHORD 2-5=-142/146
WEBS 3-5=-140/164

JOINT STRESS INDEX
2 = 0.61, 3 = 0.32, 4 = 0.40 and 5 = 0.59

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 5 and 116 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] / THE CHARLES
296128	EJ7A	MONO HIP	1	1	296128004
					Job Reference (optional)

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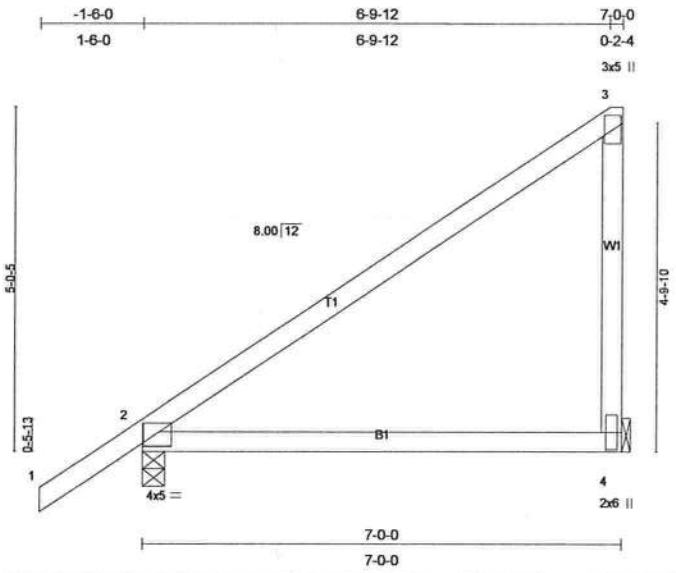

Plate Offsets (X,Y): [2:0-0-3,0-0-9]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.58	Vert(LL)	-0.05	2-4	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.09	2-4	>845	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=203/Mechanical, 2=315/0-4-0
Max Horz 2=194(load case 6)
Max Uplift4=-102(load case 6), 2=-87(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-166/42, 3-4=-145/176
BOT CHORD 2-4=-45/51

JOINT STRESS INDEX
2 = 0.70, 3 = 0.70 and 4 = 0.56

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 4 and 87 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	[redacted] THE CHARLES
296128	EJ2A	JACK	1	1	296128002
					Job Reference (optional)

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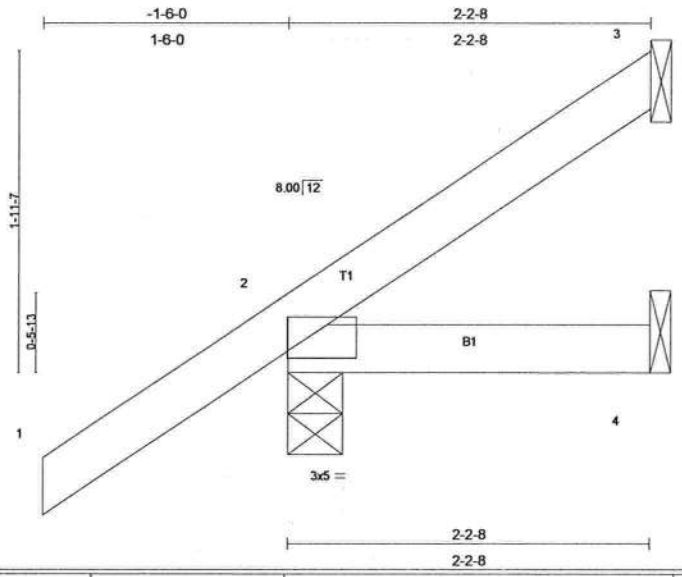


LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.17	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 10 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=16/Mechanical, 2=191/0-4-0, 4=10/Mechanical
Max Horz 2=128(load case 6)
Max Uplift3=-28(load case 7), 2=-146(load case 6)
Max Grav 3=24(load case 4), 2=191(load case 1), 4=30(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-57/14
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.18

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 146 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

.00 CODE 210 UNIT 1

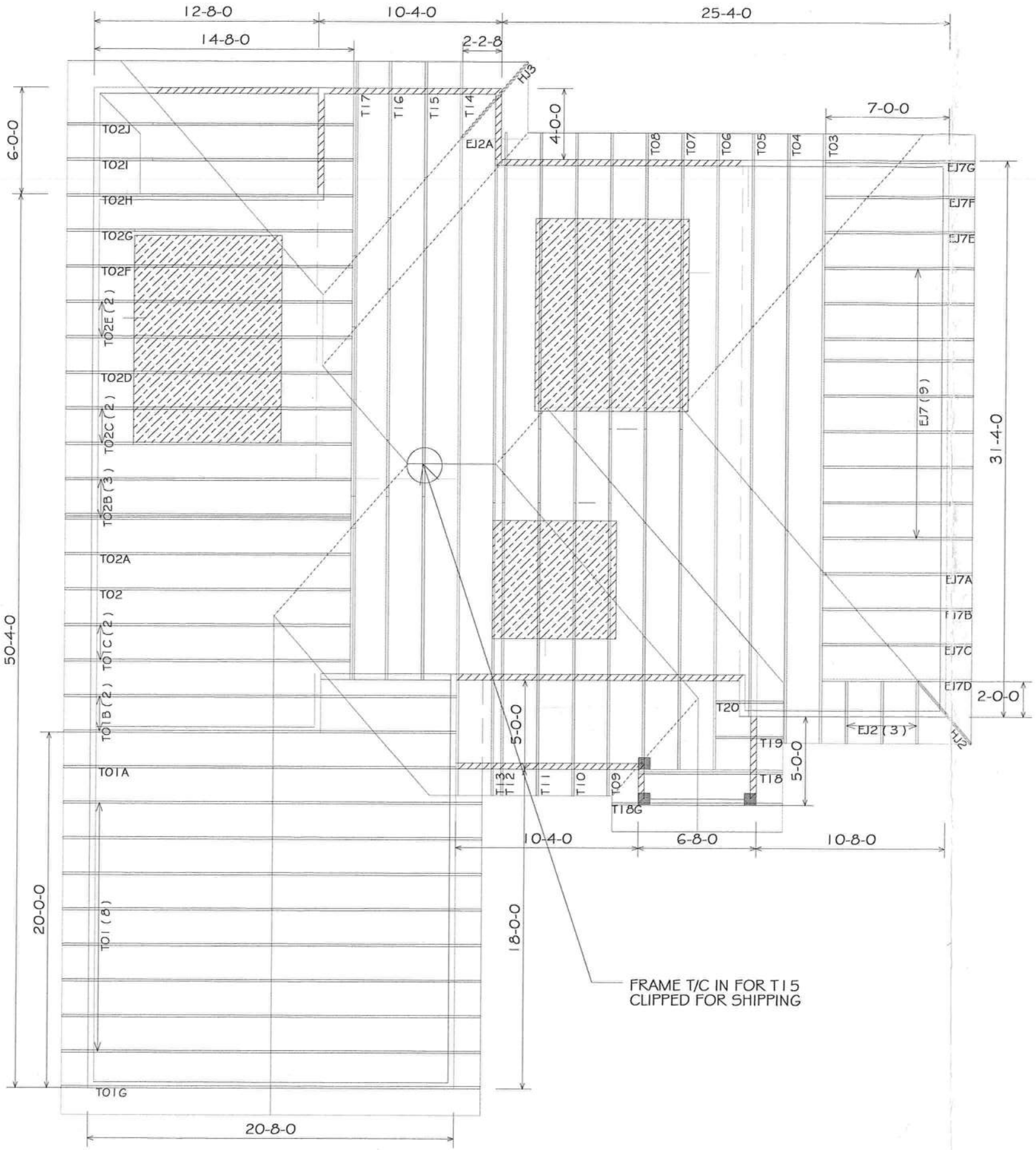
.88

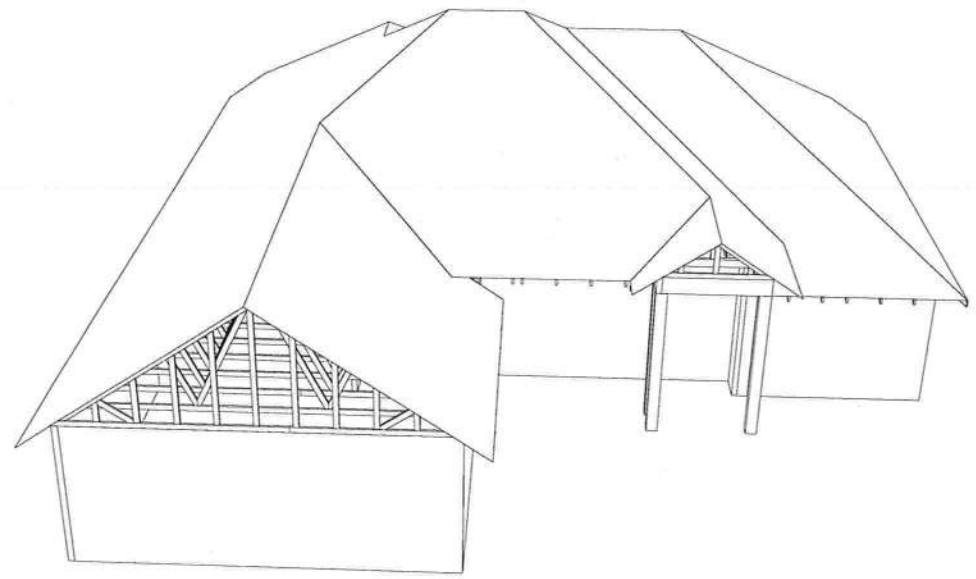
$78.63

$409.16

500.00

$3,063.67 CHECK NUMBER 1253





TRUSS INFORMATION:

ROOF PITCH: 7-8/12

CEILING: TRAYS @ MASTER/LIVING/OFFICE

OVERHANG: 1' 6"

HANGER LIST:
19 - HTU26

VALLEY:
FRAMED BY OTHERS

NOTE: ALL BEAMS AND HEADERS BY OTHERS

BEARING HEIGHT SCHEDULE

8' 0"

10' 0"

INDICATES TRAY CLG.

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: [redacted]

LEGAL ADDRESS:
COLUMBIA COUNTY, FL

MODEL:		REVISION:
CHARLES		SCALE: NTS
DATE: 01/13/09	DRAWN BY: JP	JOB #: 296128