		Colun This Pe	rmit Expires	s One Ye	ear From the	_		PERMIT 000025220	
APPLICANT	BONNIE						454-8312		
ADDRESS	1501	SW COUNTY R	D 778		FT. WHITE			FL 32038	
OWNER	JEFF & B	ONNIE MORAN				PHONE	454.8312		
ADDRESS	1501	SW COUNTY R	D 778		FT. WHITE			<u>FL</u> 32038	
CONTRACTO	OR JEF	F & BONNIE MOR	lan		I	PHONE	386.454.83	12	
LOCATION C	F PROPER	TY <u>441S-S</u>	TR TO C-778,1	r and G	O APPROX. 1.	50 MILES A	ND IT'S		
		THE L.	AST DRIVEWA	Y ON R B	EFORE BETH	LEHEM AV	ENUE.		
TYPE DEVEL	OPMENT	SFD/UTILITY	,	ES'	TIMATED COS	ST OF CONS	STRUCTIO	N 108800.00	
HEATED FLO	OOR AREA	2176.00	ТС	OTAL ARE	EA <u>2560.00</u>		HEIGHT	22.80 STORIES	1
FOUNDATIO	N CONC	<u> </u>	ALLS FRAME	D F	ROOF PITCH	12'12		FLOOR CONC	
LAND USE &	ZONING	A-3				MAX. F	IEIGHT	35	
Minimum Set	Back Requi	rments: STRE	ET-FRONT	30.00	1	REAR 2	5.00	SIDE 25.00	
NO. EX.D.U.	0	_ FLOOD ZON	IE <u>X</u>	_	DEVELOPME	ENT PERMI	T NO.		
PARCEL ID	08-7S-17-	-09954-007	SU	BDIVISIO	N				
LOT	BLOCK	PHASE		UNIT _		TOTAL	ACRES _	13.30	-
					1	Do an an	· M		
Culvert Permit	No.	Culvert Waiver	Contractor's L	icanca Num	whore	win	u //	ner/Contractor	
EXISTING	NO.	03-0834-N	Contractor's L	BLK	noer	Ap JTH	_	ner/Contractor N	
Driveway Con	nection	Septic Tank Numb	ber I		ng checked by		ved for Issu		
COMMENTS:		ABOVE ROAD. NO							
COMMENTS.	110011	BOVE ROLD: NO	ZO OIVI IEEE.					·	
		-1		-		C	heck # or	Cash 1004	
		FOR							
		FOR	BUILDING 8	& ZONIN	NG DEPAR	MENT O	NLY	(footer/Slab))
Temporary Pov	wer		BUILDING &		NG DEPAR	MENT O	NLY Monolithic	(footer/Slab))
Temporary Pov	wer	date/app. by			date/app. by			•))
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NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

INSPECTORS OFFICE (

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

CLERKS OFFICE

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

THIS INSTRUMENT WAS PREPARED BY:

TERRY McDAVID POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

POST OFFICE BOX 1328 LAKE CITY, FL 32056-1328

00-05101

"00 MAR 24 PM 2: 14

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY. FL

RECORD VERIFIED

D.C.

Documentary Stamp Intangible Tax P DeWitt Cason Clerk of Court

File No. 00-124

TERRY McDAVID

RETURN TO:

Grantee #1 S.S. No. <u>258-27-6904</u>

Grantee #2 S.S. No. 261-93-7325

Property Appraiser's Parcel Identification No. 08-7S-17-09954-007

WARRANTY DEED

THIS INDENTURE, made this 24th day of March 2000, BETWEEN HOWARD A. SHEPARDSON and his wife, KRISTA S. SHEPARDSON, whose post office address is Route 3, Box 3127, Ft. White, Florida 32038, of the County of Columbia, State of Florida, grantor*, and JEFFREY A. MORAN and his wife, BONNIE J. MORAN, whose post office address is 4400 NW 74th Street, Pompano Beach, Florida 33073, of the County of Broward, State of Florida, grantee*.

that said grantor, for and in consideration of WITNESSETH: the sum of Ten Dollars (\$10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

TOWNSHIP 7 SOUTH - RANGE 17 EAST

SECTION 8: The East 1/3 of the West 2/3 of the SW 1/4 of
the SE 1/4 of Section 8, Township 7 South, Range 17 East, Columbia County, Florida.

SUBJECT TO: Restrictions, easements and outstanding mineral rights of record, if any, and taxes for the current year.

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons whomsoever.

*"Grantor" and "grantee" are used for singular or plural, as context requires.

BK 0899 PG 0989

OFFICIAL RECORDS

IN WITNESS WHEREOF, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(First Witness)
Terry McDavid

Printed Name

(Second Witness) DeEtte F. Brown

Printed Name

Shepardson

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 24th day of March 2000, by HOWARD A. SHEPARDSON and his wife, KRISTA S. SHEPARDSON, who are personally known to me and who did not take an oath.

My Commission Expires:



BK 0899 PG 0990

OFFICIAL RECORDS

Columbia County Building Permit Application

The state of the s	
For Office Use Only Application # 000 - 76 Date Rec	seived 1925 By TW Permit # 25220
Application Approved by - Zoning Official 62 Date	2,11.06 Plans Examiner <u>OK 77/f</u> Date 11-7-06
Flood Zone Development Permit Zoning_	
Comments	* RELEASE IN BOX
JOE: - 1 SET OF SPECS RECID -	FAX 386-454-4086
	386-588-4051 Cell
Applicants Name <u>Jeff + Bonnie Moran</u>	Phone 386-454-8312 hm
Address 1501 SW CR 778, FT White	FL 32038 386-454-2060 WK
Owners Name Same	Phone
911 Address Same	
Contractors Name OUN & BLDg	Phone
Address	56-56
Fee Simple Owner Name & Address	
Bonding Co. Name & Address	
Architect/Engineer Name & Address William Myers P	0 Box 1513, LK City 32056-386-758-8406
Mortgage Lenders Name & Address CASH	
Circle the correct power company - FL Power & Light - Clay	Elec. – Suwannee Valley Elec. – Progressive Fnergy
Property ID Number 08-75-17-09954-007	Estimated Cost of Construction / 6020001
Subdivision Name N/A	lot Rlock linit Phase
Driving Directions 75 S to Exit 414, S on	441 approx 7 m; to CR 778.
Ron 778 approx 1.5 mi, last	drive way on Right before
Bethlehem Ave.	S. TVE VVEY OTT TIGHT DETOTES
Type of Construction CBS - STD N	umber of Existing Dwellings on Property
Total Acreage 13.3 Lot Size 4440 Do you need a - Culve	ert Permit or Culvert Waiver of Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 1161.	Side W-144' Side E-234' Regr 248'
Total Building Height $22'8''$ Number of Stories 2 He	egted Floor Area 2174 Roof Pitch 12-12
	2560
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuance of a permit and	stallations as indicated. I certify that no work or
all laws regulating construction in this jurisdiction.	a triat all work be performed to meet the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information compliance with all applicable laws and regulating construction	mation is accurate and all work will be done in
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE	_
TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTILLENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF	END TO OBTAIN FINANCING CONSULT WITH YOUR
Owner Builder or Agent (Including Contractor)	Contractor Signature
	Contractors License Number
STATE OF FLORIDA COUNTY OF COLUMBIA DOROTHY L. SPENCER Notary Public, State of Florida My comm. expires Nov. 19, 2007	Competency Card Number NOTARY STAMP/SEAL
Sworn to (or affirmed) and subscribed before me	/
this 25th day of October 20 0/0.	Charles L. Snever
Personally known or Produced Identification	Notary Signature
Tu	Notary Signature A 1/6 1 Zomi E //. 8.06



OCCUPAZO

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in

accordance with the Columbia County Building Code.

Parcel Number 08-7S-17-09954-007

Fire: Building permit No. 000025220 97.68

Use Classification SFD/UTILITY

Permit Holder JEFF & BONNIE MORAN

Waste: 134.00

Owner of Building JEFF & BONNIE MORAN

Total: 231.68

Location: 1501 SW CR 778, FT. WHITE, FL

Date: 02/05/2008

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

IOTICE OF COMMENCEMENT FORM COLUMBIA COUNTY, FLORIDA

CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.***

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

	PERMIT NUMBER
escription of property: (legal description of the property)	orty and street address or 911 address
ORB 850-481, 853-125	5 Thru 1250
861-1757, 899-989	5 Thru 1259
ieneral description of improvement:Single	Family Home
	realities nome
Owner Name & Address Ucff + Bonnie	noran
1501 SWCK 178 FT White El	32038
lame & Address of Fee Simple Owner (if other than o	Mora Toperty
Contractor Name Jeff Moren	Phone Number 386 454 8312 Phone Number 386 454 8312
Address 1501 SW CR778 F	Tuber 9 9312
Burety Holders Name	DATE 10 52038 3
Address	. none unubet
Amount of Bond	nst:2006025398 Date:10/25/2006 Time:16:57 DC,P.Dewitt Cason,Columbia County B:1100 P:449
_ender Name In	nst:2006025398 Date:10/25/2006 Time:16:57
Address	DC,P.DeWitt Cason,Columbia County B:1100 P:449
red as provided by section 718.13 (1)(a) 7; Florida Star	wee;
Name	Phone Number
Address	Phone Number
in addition to himself/herself the owner designates _	of the Lienaria New of
to receive a copy	of the Lienor's Notice as provided in Section 713.13 (1) —
(a) 7 Phone Name and the control of	provided in Section 713.13 (1) -
(=) /- Frione number of the designee	
Expiration date of the Notice of Commencement (the	expiration date is 1 (one) were from the
Expiration date of the Notice of Commencement (the	expiration date is 1 (one) year from the date of recording
Expiration date of the Notice of Commencement (the (Unless a different date is specified)	expiration date is 1 (one) year from the date of recording,
Expiration date of the Notice of Commencement (the (Unless a different date is specified)	expiration date is 1 (one) year from the date of recording.
Expiration date of the Notice of Commencement (the (Unless a different date is specified)	expiration date is 1 (one) year from the date of recording.
Expiration date of the Notice of Commencement (the (Unless a different date is specified)	expiration date is 1 (one) year from the date of recording, one else may be permitted to sign in his/her stead.
Expiration date of the Notice of Commencement (the (Unless a different date is specified)	expiration date is 1 (one) year from the date of recording, one else may be permitted to sign in his/her stead. Sworn to (or affirmed) and subscribed before
Expiration date of the Notice of Commencement (the (Unless a different date is specified) ICE AS PER CHAPTER 713. Florida Statutes: owner must sign the notice of commencement and notice of commencement.	expiration date is 1 (one) year from the date of recording, one else may be permitted to sign in his/her stead. Sworn to (or affirmed) and subscribed before day of
Expiration date of the Notice of Commencement (the (Unless a different date is specified)	expiration date is 1 (one) year from the date of recording, one else may be permitted to sign in his/her stead. Sworn to (or affirmed) and subscribed before
Expiration date of the Notice of Commencement (the (Unless a different date is specified) ICE AS PER CHAPTER 713. Florida Statutes: owner must sign the notice of commencement and notice of commencement.	expiration date is 1 (one) year from the date of recording, one else may be permitted to sign in his/her stead. Sworn to (or affirmed) and subscribed before day of

EXPIRES: June 28/2008
Bonded Thru Notary Public Underwriters

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, 88. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

	TYPE OF CONSTRUCTION		
() Single Family Dwelling () Farm Outbuilding	Ö	Two-Family Res	ldence
	CONSTRUCTION OR IMPROVEMEN		
() New Construction	() Addition, Alteration, Modification	n or other impr	ovement
provided for in Florida Statutes ss.48	, have been advised of the as an owner/builder. I agree to comply 19.103(7) allowing this exception for the	with all require	ments
Columbia County Building Permit N	umber		
	10/30/06		
Owner Builder Signature	Date		
The above signer is personally known produced identification	a to-me-or	S.M. A.	DOROTHY L. SPENCER Notary Public, State of Florida My comm. expires Nov. 19, 2007 No. DD 269013
Notary Signature	Space Date 10/30/06	(Stamp/S	
	FOR BUILDING USE ONLY		
Statutes ss 489.103(7).	owner/builder has been notified of the d	lisclosure statem	ent in Florida



STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

03-0834N Permit Application Number ___ PART II - SITE PLAN-440 Ft. Scale: Each block represents 5 feet and 1 inch = 50 feet. OOVN 303 ft. Existing Well 78F# 200 ft to prop line house corner Notes: septic 130 A Distance from wdl Site Plan submitted by: Plan Appraved _ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

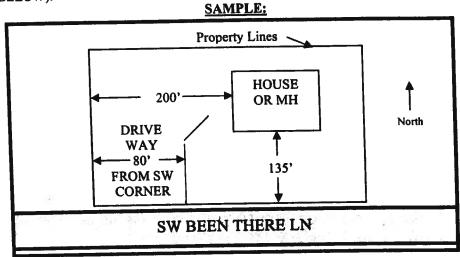
COLUMBIA COUNTY PUBLIC HEALTH UNIT ENVIRONMENTAL HEALTH

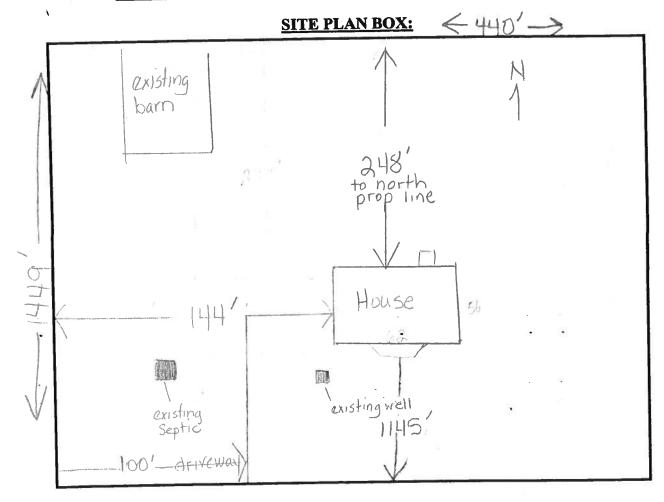
THE FOLLOWING INDIVIDUAL SEWAGE DISPOSAL SYSTEM HAS HAD A FINAL INSPECTION AND IS APPROVED FOR CONNECTION OF POWER. PERMIT 03-089

ADDRESS:_ OWNER:

PARCEL I.D.:

- 1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
- 2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
- 3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
- 4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).





Page 2 of 2

Gaylord Pump & Irrigation Inc.

F.O. Box 548

Branford, Fl. 32008
(386)-935-0932 Fax 386-935-0778

4" Steel Casing (schedule 40)

1 Hp Submersible Pump 20 GPM

1-1/4 Galvanize Drop Pipe
PC-244 Diaphragm Tank (Tank has 24.97 gallons of draw down, no cycle stop valve required)

This equipment meets or exceeds state codes of March 2002

8/27 motalise

... h.

Project Name:

Address:

Jeffrey & Bonnie Moran

1501 SW CR 778

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

Permitting Office: Own 5 in

City, State Owner: Climate Z	Moran Reside		Permitting Office: 4 000 No. 10 Permit Number: Jurisdiction Number: 22	
	onstruction or existing	New	12. Cooling systems	
-	family or multi-family	Single family	a. Central Unit	Cap: 49.0 kBtu/hr
	er of units, if multi-family	1 _		SEER: 11.00
	r of Bedrooms	3 _	b. N/A	
	a worst case?	No _		
	ioned floor area (ft²)	2176 ft ²	c. N/A	
	ype ¹ and area: (Label reqd. by 13	•	12 77 6	-
a. U-facto		Description Area	13. Heating systems	Cap: 49.0 kBtu/hr
b. SHGC:	ngle or Double DEFAULT) 7a(Single Default) 305.3 ft ²	a. Electric Heat Pump	HSPF: 6.80
	ear or Tint DEFAULT) 7b.	(Cl) 205 2 A2	b. N/A	11511.0.00
8. Floor t		(Clear) 305.3 ft ²	U. IVA	_
	n-Grade Edge Insulation	R=0.0, 204.0(p) ft	c. N/A	
b. N/A		11 010, 20 110(p) 11 =		
c. N/A			14. Hot water systems	_
9. Wall ty	pes	_	a. Electric Resistance	Cap: 80.0 gallons
a. Concre	te, Ext Insul, Exterior	R=5.0, 1414.7 ft ²		EF: 0.90
b. N/A		_	b. N/A	_
c. N/A		—		
d. N/A		_	c. Conservation credits	_
e. N/A		_	(HR-Heat recovery, Solar	
Ceiling		_	DHP-Dedicated heat pump)	
a. Under	Attic	R=30.0, 2800.0 ft ²	15. HVAC credits	PT,
b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c. N/A		 -	HF-Whole house fan,	
11. Ducts(•		PT-Programmable Thermostat,	
-	nc. Ret: Unc. AH: Interior	Sup. R=6.0, 50.0 ft	MZ-C-Multizone cooling,	
b. N/A			MZ-H-Multizone heating)	
		- 	×.	
	Glass/Floor Area: 0	.14 Total as-built p	points: 28090 PASS	

I hereby certify that the plans and specifications covered by Review of the plans and this calculation are in compliance with the Florida Energy specifications covered by this Code. calculation indicates compliance PREPARED BY: with the Florida Energy Code. Before construction is completed this building will be inspected for I hereby certify that this building, as designed, is in compliance compliance with Section 553.908 with the Florida Energy Code. Florida Statutes. OWNER/AGENT: **BUILDING OFFICIAL:** DATE:

Total base points: 29971

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **1501 SW CR 778, , FL, 32038-** PERMIT #:

BASE				AS-	BUI	LT				
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area	\$	Type/SC C	Ove Ornt	erhang		Area X	SPI	M X 9	SOF	= Points
.18 2176.0 20.04 784	9.3	Single, Clear	W	1.5	8.0	40.0 9.0	43.8 43.8		0.96	1680.1 378.0
8		Single, Clear Single, Clear	W W	1.5 1.5	8.0 8.0	9.0 16.0	43.8		0.96 0.96	672.0
		Single, Clear	N	1.5	8.0	16.0	21.7		0.97	336.3
		Single, Clear	N	1.5	8.0	30.0	21.7		0.97	630.5
		Single, Clear	Ε	9.5	8.0	80.0	47.9		0.47	1805.6
		Single, Clear	N	19.5	8.0	15.0	21.7	73	0.61	197.3
		Single, Clear	Ε	9.5	8.0	56.0	47.9	92	0.47	1263.9
		Single, Clear	S	19.5	8.0	13.3	40.8	31	0.44	238.7
		Single, Clear	S	1.5	8.0	30.0	40.8	31	0.92	1130.4
		As-Built Total:				305.3				8332.8
WALL TYPES Area X BSPM = P	oints	Туре		R	-Value	e Area	X	SPM	=	Points
Adjacent 0.0 0.00	0.0	Concrete, Ext Insul, Exterior	14		5.0	1414.7		0.50		707.3
•	405.0	, ,								
Base Total: 1414.7 2	405.0	As-Built Total:			_	1414.7				707.3
DOOR TYPES Area X BSPM = P	oints	Туре				Area	X	SPM	=	Points
Adjacent 0.0 0.00	0.0	Exterior Insulated				20.0		4.10		82.0
Exterior 40.0 4.10	164.0	Exterior Insulated				20.0		4.10		82.0
Base Total: 40.0	164.0	As-Built Total:				40.0				164.0
CEILING TYPES Area X BSPM = P	oints	Туре		R-Val	ue /	Area X :	SPM	X SC	M =	Points
Under Attic 2176.0 1.73 3	764.5	Under Attic	· - ··		30.0	2800.0	1.73)	K 1.00		4844.0
Base Total: 2176.0 3	764.5	As-Built Total:				2800.0				4844.0
FLOOR TYPES Area X BSPM = P	oints	Туре		R-	-Value	e Area	X	SPM	=	Points
Slab 204.0(p) -37.0 -7 Raised 0.0 0.00	548.0 0.0	Slab-On-Grade Edge Insulation	1		0.0	204.0(p	-	41.20		-8404.8
Base Total: -7	548.0	As-Built Total:				204.0				-8404.8
INFILTRATION Area X BSPM = P	oints		9			Area	X	SPM	=	Points
2176.0 10.21 22	217.0	×				2176.	0	10.21		22217.0

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **1501 SW CR 778, , FL, 32038-** PERMIT #:

	BASE		AS-BUILT						
Summer Ba	ase Points: 28	3851.7	Summer As-Built Points:	27860.3					
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Cooling Points					
28851.7	0.4266	12308.1	(sys 1: Central Unit 49000 btuh ,SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(I 27860 1.00 (1.09 x 1.000 x 0.91) 0.310 0.950 27860.3 1.00 0.992 0.310 0.950	NS) 8145.6 8145.6					

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 1501 SW CR 778, , FL, 32038-

PERMIT #:

BASE				AS-	BUI	LT				
GLASS TYPES .18 X Conditioned X BWPM = P Floor Area	oints	Type/SC C	Ove Ornt	rhang Len	Hgt	Area X	WF	РМ Х	WO	F = Points
.18 2176.0 12.74	4990.0	Single, Clear	w	1.5	8.0	40.0	28.	84	1.01	1166.4
		Single, Clear	W	1.5	8.0	9.0	28.	84	1.01	262.4
		Single, Clear	W	1.5	8.0	16.0	28.	84	1.01	466.6
		Single, Clear	N	1.5	8.0	16.0	33.	22	1.00	532.0
		Single, Clear	N	1.5	8.0	30.0	33.		1.00	997.5
		Single, Clear	E	9.5	8.0	80.0	26.		1.34	2824.5
		Single, Clear	N	19.5	8.0	15.0	33.		1.03	511.5
		Single, Clear	E	9.5	8.0	56.0	26.		1.34	1977.2
		Single, Clear	S	19.5	8.0	13.3	20.		3.60	968.2 633.1
		Single, Clear	S	1.5	8.0	30.0	20.	24	1.04	632.1
		As-Built Total:				305.3				10338.4
WALL TYPES Area X BWPM =	Points	Туре		R-	Value	Area	Х	WPN	1 =	Points
Adjacent 0.0 0.00	0.0	Concrete, Ext Insul, Exterior	63		5.0	1414.7		4.30		6083.2
Exterior 1414.7 3.70	5234.4									
Base Total: 1414.7	5234.4	As-Built Total:				1414.7				6083.2
DOOR TYPES Area X BWPM =	Points	Туре				Area	Х	WPN	1 =	Points
Adjacent 0.0 0.00	0.0	Exterior Insulated				20.0		8.40		168.0
Exterior 40.0 8.40	336.0	Exterior Insulated				20.0		8.40		168.0
Base Total: 40.0	336.0	As-Built Total:				40.0				336.0
CEILING TYPES Area X BWPM =	Points	Туре	R	-Value	. Ar	ea X W	PM	x wc	:M =	Points
Under Attic 2176.0 2.05	4460.8	Under Attic			30.0	2800.0 2	2.05	X 1.00		5740.0
Base Total: 2176.0	4460.8	As-Built Total:				2800.0				5740.0
FLOOR TYPES Area X BWPM =	Points	Туре		R-	Value	Area	Х	WPM	1 =	Points
Slab 204.0(p) 8.9 Raised 0.0 0.00	1815.6 0.0	Slab-On-Grade Edge Insulation	1		0.0	204.0(p		18.80		3835.2
Base Total:	1815.6	As-Built Total:				204.0				3835.2
INFILTRATION Area X BWPM =	Points					Area	X	WPM	1 =	Points
2176.0 -0.59	-1283.8					2176.0)	-0.59		-1283.8

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: 1501 SW CR 778, , FL, 32038- PERMIT #:

	BASE		AS-BUILT						
Winter Base	Points:	15553.0	Winter As-Built Points: 25049.0						
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)						
15553.0	0.6274	9757.9	(sys 1: Electric Heat Pump 49000 btuh ,EFF(6.8) Ducts:Unc(S),Unc(R),Int(AH),R6.0 25049.0 1.000 (1.069 x 1.000 x 0.93) 0.501 0.950 11863.7 25049.0 1.00 0.994 0.501 0.950 11863.7						

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: 1501 SW CR 778, , FL, 32038- PERMIT #:

BASE				AS-BUILT								
WATER HEA Number of Bedrooms	TING	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier	X Credit Multiplier	
3		2635.00		7905.0	80.0	0.90	3		1.00	2693.56	1.00	8080.7
					As-Built To	otal:						8080.7

	CODE COMPLIANCE STATUS									
	BAS	SE			AS-BUILT					
Cooling + Points	Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points			
12308 9758 7905 29971 8146 11864 8081										

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: 1501 SW CR 778, , FL, 32038-

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir	
		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools	
		must have a pump timer. Gas spa & pool heaters must have a minimum thermal	
		efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically	
		attached, sealed, insulated, and installed in accordance with the criteria of Section 610.	
		Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.	
		Common ceiling & floors R-11.	

Tested sealed ducts must be certified in this house.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.8

The higher the score, the more efficient the home.

Moran Residence, 1501 SW CR 778, , FL, 32038-

 New construction or existing Single family or multi-family Number of units, if multi-family Number of Bedrooms Is this a worst case? Conditioned floor area (ft²) Glass type¹ and area: (Label reqd. a. U-factor: (or Single or Double DEFAULT) SHGC:	Description Area 7a(Sngle Default) 305.3 ft ² 7b. (Clear) 305.3 ft ² R=0.0, 204.0(p) ft R=5.0, 1414.7 ft ² R=30.0, 2800.0 ft ²		Cap: 49.0 kBtu/hr SEER: 11.00 Cap: 49.0 kBtu/hr HSPF: 6.80 Cap: 80.0 gallons EF: 0.90 PT, PT,
I certify that this home has complic Construction through the above en in this home before final inspection based on installed Code compliant Builder Signature: Address of New Home:	ergy saving features which n. Otherwise, a new EPL I features.	will be installed (or exceeded)	THE STATE OF THE S

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

Energy Code Compliance

Duct System Performance Report

Project Name:

Jeffrey & Bonnie Moran

Address:

1501 SW CR 778

City, State: Owner: , FL 32038-Moran Residence

Climate Zone:

North

Builder:

Permitting Office: Permit Number: Jurisdiction Number:

Total Duct System Leakage Test Results

CFM25 Total Duct Leakage Test Values							
Line	System	Duct Leakage Total	Duct Leakage to Outdoors				
1	System1	cfm25(tot)	cfm25(out)				
2	System2	cfm25(tot)	cfm25(out)				
3	System3	cfm25(tot)	cfm25(out)				
4	System4	cfm25(tot)	cfm25(out)				
5	Total House Duct System Leakage	Sum lines 1-4 Divide by (Total Conditioned Floor Area) =(Q _n ,tot) Receive credit if Q _n ,tot≤ 0.03	Sum lines 1-4 Divide by (Total Conditioned Floor Area) =(Q _n ,out) Receive credit if Q _n ,out≤ 0.03 AND Q _n ,tot≤ 0.09				

I hereby certify that the above duct testing performance results demonstrate compliance with the Florida Energy Code requirements in accordance with Section 610.1.A.1, Florida Building Code, Building Volume, Chapter 13 for leak free duct system credit.

Signature: _____
Printed Name: _____
Florida Rater Certification #: _____

DATE:

Florida Building Code requires that testing to confirm leak free duct systems be performed by a Class 1 Florida Energy Gauge Certified Energy Rater. Certified Florida Class 1 raters can be found at: http://energygauge.com/search.htp



BUILDING OFFICIAL:	
DATE:	

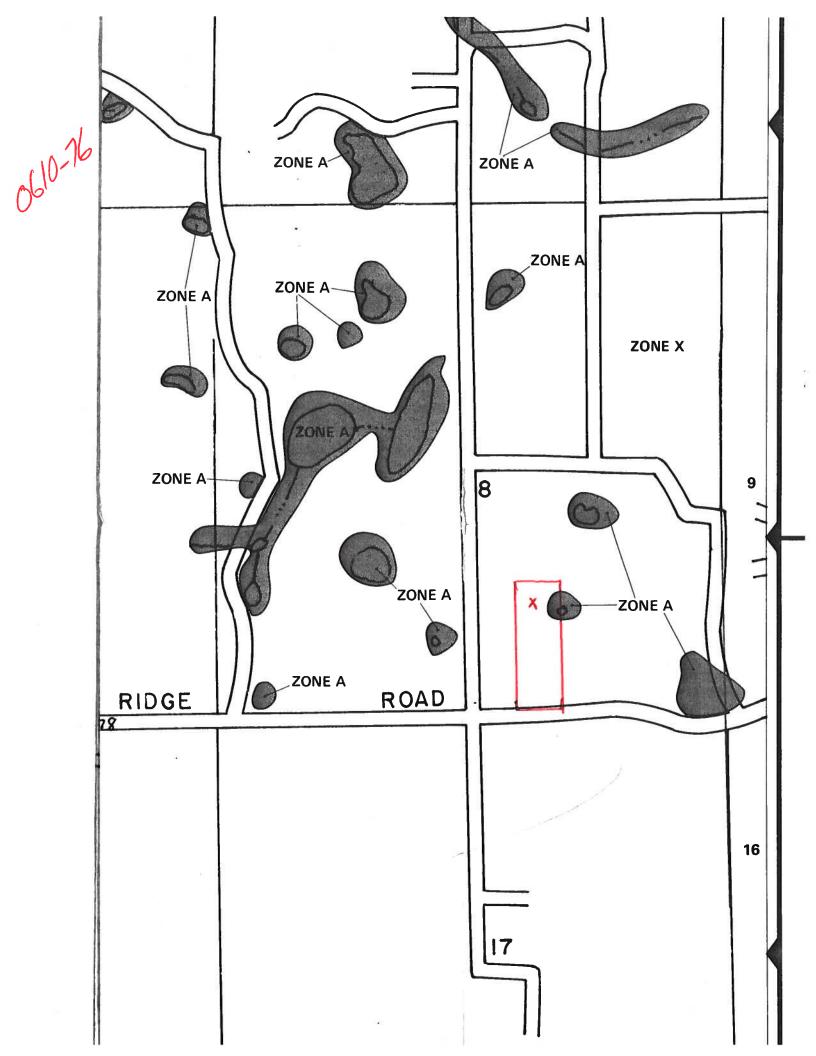
COLUMBIA COUNTY 9-1-1 ADDRESSING

263 NW Lake City Ave. * P. O. Box 2949 * Lake City, FL 32056-2949 PHONE: (386) 752-8787 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: 10-20-03
ENHANCED 9-1-1 ADDRESS:
1501 SW County Road 778 (Fr. white. Fr)
Addressed Location 911 Phone Number: NIP
OCCUPANT NAME: Jefferey + Bonnie Moran.
OCCUPANT CURRENT MAILING ADDRESS: 690 SW Bethleham Ave Fr. white. FL. 32038
PROPERTY APPRAISER MAP SHEET NUMBER: 106
PROPERTY APPRAISER PARCEL NUMBER: 08-75-17-09954-007
Other Contact Phone Number (If any):
Building Permit Number (If known):
ADDRESSING DEPARTMENT ID#: (Addressing Department Use Only, THIS IS NOT AN ADDRESS)
Remarks: None.
Address Issued By: Columbia County 9-1-1 Addressing Department





RE: MORANJ -

TRENCO

818 Soundside Road Edenton, NC 27932 Telephone 252/482-7000

Fax 252/482-7115

Site Information:

Project Customer: JEFF MORAN Project Name: MORANJ

Lot/Block:

Subdivision:

Address: 1501 SW CR 778

State: FLORIDA

City: HIGH SPRINGS

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):**

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.3

Wind Code: ASCE 7-02 Wind Speed: 110 mph

Design Method: Components/Cladding ASCE 7-02

Roof Load: 40.0 psf

Floor Load: 55.0 psf

This package includes 7 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date
1	E3606418	MORANJ	Α	10/31/06
2	E3606419	MORANJ	A1	10/31/06
3	E3606420	MORANJ	В	10/31/06
4	E3606421	MORANJ	DET	10/31/06
5	E3606422	MORANJ	DET1	10/31/06
6	E3606423	MORANJ	F1	10/31/06
7	E3606424	MORANJ	PB	10/31/06

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.

Marvin A. Strzyzewski Truss Engineering Con 818 Soundside Rd. Edenton, NC 27932 FL Cert.#7239

Strzyzewski, Marvin

October 31,2006

1 of 2

Truss Type Qty Job Truss F3606418 SPECIAL 14 MORANJ Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:19 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL 40-0-0 -2-0-0 13-11-1 18-0-15 26-9-12 35-5-2 A2-0-0, 2-0-0 7-1-7 4-1-15 4-4-6 4-10-4 3-9-2 4-6-14 2-0-0 6-9-9 6x6 MII20// Scale = 1:103.2 5x5 Mil20 ≫ 10.00 12 3x6 MII20// 5 6 3x5 MII20 >> 3x5 MII20 4 4.00 12 5x5 MII20≈ 23 3 3x6 Mil20~ 3x4 MII20≈ 9 3x5 MII20≈ 10 13 3x5 MII20= 3x5 MII20= 22 15 21 20 19 18 17 16 14 1 5x4 MII20 II 3x6 MII20= 3x4 MH20= 3x5 MH20= 3x4 MH20= 3x4 MII20= 3x6 MII20= 1 5x4 MII20 II 3v10 MH20= 13-11-1 31-8-0 35-5-2 40-0-0 18-0-15 7-1-7 6-9-9 4-1-15 4-4-6 4-4-6 4-10-4 3-9-2 4-6-14

Plate Offsets (2	/): [2:0-5-3,0-0-10], [5:0-2-12,0-1-8]						
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc) I/d	lefi L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.35	Vert(LL)	-0.05 21-22 >9	99 240	MII20	249/190
TCDL 10.0	Lumber Increase 1.25	BC 0.35	Vert(TL)	-0.16 21-22 >9	99 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.51	Horz(TL)	0.04 15 1	n/a n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)				Weight: 270 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WERS

BRACING TOP CHORD

Structural wood sheathing directly applied or 4-10-11 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

BOT CHORD WEBS 1 Row at midot 3-21, 5-19, 7-19

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss

fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 2=1346/0-8-0, 15=1812/0-8-0, 12=272/0-3-8

Max Horz 2=-423(load case 3)

Max Uplift2=-399(load case 5), 15=-315(load case 6), 12=-284(load case 4) Max Grav 2=1346(load case 1), 15=1812(load case 1), 12=294(load case 10)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/79, 2-3=-1579/296, 3-23=-1144/305, 4-23=-1026/306, 4-5=-1008/345, 5-6=-750/319, 6-7=-1059/357, TOP CHORD

7-8=-1228/301, 8-9=-883/201, 9-10=-937/193, 10-24=-71/592, 11-24=-83/586, 11-12=-49/175, 12-13=0/37

BOT CHORD 2-22=-142/1088, 21-22=-142/1088, 20-21=0/777, 19-20=0/777, 18-19=-8/881, 17-18=-58/870, 16-17=-58/870,

15-16=-525/192, 14-15=-151/82, 12-14=-151/82

3-22=0/306, 3-21=-422/269, 5-21=-117/435, 5-19=-216/174, 6-19=-148/397, 7-19=-291/176, 7-18=-45/164,

8-18=-19/127, 8-16=-735/174, 10-16=-245/1595, 10-15=-1543/304, 11-15=-434/159, 11-14=0/160

NOTES

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 2, 315 lb uplift at joint 15 and 284 lb uplift at joint 12.

LOAD CASE(S) Standard

October 31,2006

Truss Type Job Truss E3606419 SPECIAL 5 MORANJ Αı Job Reference (optional) 0 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:20 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL 6.300 13-11-1 18-0-15 22-5-6 26-9-12 31-8-0 35-5-2 40-0-0 42-0-0 7.1.7 4-1-15 4-10-4 3-9-2 4-6-14 2-0-0 6-9-9 4-4-6 4-4-6 6x6 MII20 // Scale = 1:101.0 5x5 MII20 > 10.00 18x6 MII20/ 5 3x5 MII20 > 3x5 MII20 / 4.00 12 5x5 MII20 2 3x6 MI120~ 3x4 MII20~ 8 3x5 MH20~ 9 10 12 3x5 MII20 = 4x6 MII20= 21 20 19 18 17 16 15 14 13 3x6 MII20= 3x4 MH20= 3x5 MH20= 1.5x4 MII20 II 3x4 MII20= 3x4 MII20= 3v6 MII20= 1.5x4 MII20 II 3x10 MH20 = 7-1-7 13-11-1 31-8-0 35-5-2 40-0-0 18-0-15 26-9-12 7-1-7 6-9-9 4-10-4 3-9-2 4-6-14 4-1-15 4-4-6 4-4-6 Plate Offsets (X,Y): [1:0-6-3,0-0-6], [4:0-2-12,0-1-8] LOADING (psf) **SPACING** DEFL L/d **PLATES GRIP** (loc) **Vdefl** 1-21 249/190 TCLL 20.0 Plates Increase 1.25 TC 0.34 Vert(LL) 0.10 >999 240 MI120 TCDL 10.0 Lumber Increase 1.25 BC 0.39 Vert(TL) -0.17 >999 180 1-21 WB 0.51 Rep Stress Incr YES 0.04 **BCLL** 0.0 Horz(TL) n/a 14 n/a Code FBC2004/TPI2002 (Matrix) BCDL 10.0 Weight: 266 lb LUMBER BRACING

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3

TOP CHORD **BOT CHORD WEBS**

Structural wood sheathing directly applied or 4-9-2 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. 2-20, 4-18, 6-18 1 Row at midpt

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 1=1200/0-8-0, 14=1819/0-8-0, 11=271/0-3-8

Max Horz 1=-452(load case 3)

Max Uplift1=-264(load case 5), 14=-318(load case 6), 11=-283(load case 4) Max Grav 1=1200(load case 1), 14=1819(load case 1), 11=293(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1607/337, 2-22=-1154/320, 3-22=-1013/321, 3-4=-959/359, 4-5=-756/327, 5-6=-1066/368, 6-7=-1234/310, TOP CHORD

7-8=-886/206, 8-9=-941/198, 9-23=-77/597, 10-23=-89/590, 10-11=-55/179, 11-12=0/37

BOT CHORD 1-21=-182/1117, 20-21=-182/1117, 19-20=0/783, 18-19=0/783, 17-18=-15/886, 16-17=-63/873, 15-16=-63/873,

14-15=-529/194, 13-14=-155/88, 11-13=-155/88

2-21=0/313, 2-20=-451/310, 4-20=-133/438, 4-18=-216/174, 5-18=-152/400, 6-18=-291/176, 6-17=-45/164,

7-17=-19/127, 7-15=-740/180, 9-15=-257/1604, 9-14=-1550/314, 10-14=-434/159, 10-13=0/160

NOTES

WEBS

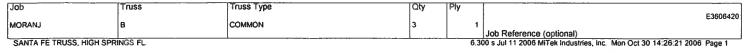
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

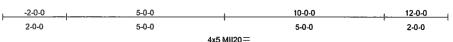
 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 1, 318 lb uplift at joint 14 and 283 lb uplift at joint 11.

LOAD CASE(S) Standard

October 31,2006

Edenton, NC 27932





Scale = 1:36.1

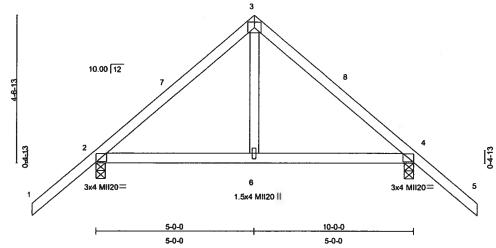


Plate Offsets (X,Y):	[2:0-2-1,0-1-8], [4:0-2-1,0-1-8]			_
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING 2-0-0 Plates increase 1.25 Lumber increase 1.25 Rep Stress incr YES	CSI TC 0.32 BC 0.14 WB 0.07	DEFL in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.02 2-6 >999 240 Mil20 249/190 Vert(TL) -0.03 4-6 >999 180	
BCLL 0.0 BCDL 10.0	Rep Stress Incr YES Code FBC2004/TPI2002	WB 0.07 (Matrix)	Horz(TL) 0.00 4 n/a n/a	

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=517/0-3-8, 4=517/0-3-8

Max Horz 2=-155(load case 3)

Max Uplift2=-251(load case 5), 4=-251(load case 6)

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/77, 2-7=-396/78, 3-7=-302/95, 3-8=-302/95, 4-8=-396/77, 4-5=0/77

BOT CHORD 2-6=0/228, 4-6=0/228

WEBS 3-6=0/217

NOTES

1) Unbalanced roof live loads have been considered for this design.

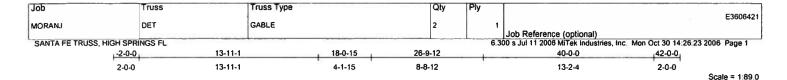
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint 4.

LOAD CASE(S) Standard

October 31,2006



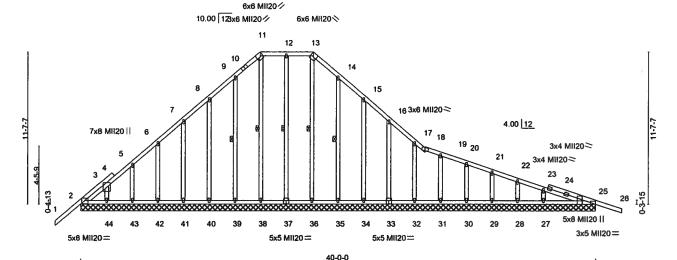


Plate Offsets (X,Y): [2:0-3-0,0-2-15], [11:0-2-4,Edge], [13:8	Edge,0-2-11], [18:0-3-0,0)-1-5], [25:0-4-12,E	dge], [25:0-	3-8,Edge], [33:0-2-8,	0-3-0], [37:0-2-8,0-3-0]
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.29 BC 0.07 WB 0.14 (Matrix)		in (loc) -0.02 26 -0.03 26 0.01 25	l/defl L/d n/r 120 n/r 120 n/a n/a	PLATES GRIP MII20 249/190 Weight: 300 lb

LUMBER TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3

OTHERS

BRACING TOP CHORD **BOT CHORD** WEBS

1 Row at midpt

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 14-35, 13-36, 12-37, 11-38, 9-39

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 25=301/40-0-0, 2=265/40-0-0, 35=156/40-0-0, 36=147/40-0-0, 37=165/40-0-0, 38=147/40-0-0, 39=156/40-0-0, 40=161/40-0-0, 41=160/40-0-0, 42=158/40-0-0, 43=171/40-0-0, 44=100/40-0-0, 34=161/40-0-0, 33=159/40-0-0, 32=166/40-0-0, 31=164/40-0-0, 30=158/40-0-0, 29=165/40-0-0, 28=140/40-0-0, 27=241/40-0-0

Max Horz 2=-408(load case 3)

Max Uplift25=-227(load case 4), 2=-185(load case 3), 35=-54(load case 5), 37=-59(load case 4), 39=-55(load case 5), 40=-75(load case 6), 41=-92(load case 5), 42=-117(load case 5), 43=-124(load case 5), 44=-24(load case 6), 34=-75(load case 6), 33=-69(load case 5), 32=-66(load case 6), 31=-105(load case 4), 30=-76(load case 4),

29=-79(load case 4), 28=-90(load case 4), 27=-72(load case 4)

Max Grav 25=301 (load case 1), 2=265 (load case 1), 35=156 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 38=147(load case 9), 39=156(load case 1), 40=161(load case 1), 41=160(load case 1), 42=158(load case 9), 43=171(load case 1), 44=108(load case 2), 34=161(load case 1), 33=159(load case 10), 32=166(load case 1), 31=164(load case 1), 30=158(load case 10), 29=165(load case 1), 28=140(load case 10), 27=241(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/75, 2-3=-237/296, 3-4=-199/273, 4-5=-189/281, 5-6=-140/287, 6-7=-72/276, 7-8=-59/268, 8-9=-60/262, TOP CHORD

9-10=-57/276, 10-11=-5/284, 11-12=-16/246, 12-13=-16/246, 13-14=-58/284, 14-15=-60/225, 15-16=-59/192, 16-17=-60/207, 17-18=-44/167, 18-19=-46/167, 19-20=-69/126, 20-21=-96/90, 21-22=-122/51, 22-23=-147/13,

23-24=-169/19, 24-25=-177/8, 25-26=0/36

BOT CHORD 2-44=0/209, 43-44=0/208, 42-43=0/208, 41-42=0/208, 40-41=0/208, 39-40=0/208, 38-39=0/208, 37-38=0/207,

36-37=0/207, 35-36=0/207, 34-35=0/207, 33-34=0/207, 32-33=0/207, 31-32=0/207, 30-31=0/207, 29-30=0/207,

28-29=0/207, 27-28=0/207, 25-27=0/207

14-35=-116/74, 13-36=-109/0, 12-37=-129/79, 11-38=-108/0, 9-39=-116/74, 8-40=-121/95, 7-41=-120/111, 6-42=-119/139, 5-43=-126/136, 3-44=-77/53, 15-34=-121/95, 16-33=-119/89, 17-32=-126/86, 19-31=-124/125,

20-30=-119/95, 21-29=-122/102, 22-28=-111/101, 23-27=-174/113

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

6) Provide adeguate drainage to prevent water ponding.

October 31,2006



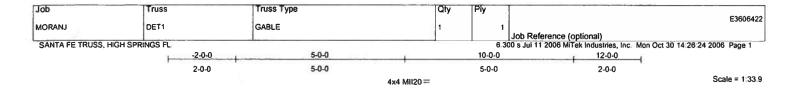
Job	Truss	Truss Type	Qty	Ply	
MORANJ	DET	GABLE	2	,	E3606421
			J		Job Reference (optional)
CANTA EF TRUSS HIGH SPRINGS EI				6.30	00 s bit 11 2006 MiTek Industries Inc. Mon Oct 30 14:26:23 2006 Page 2

NOTES

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 1.5x4 MII20 unless otherwise indicated.
- 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 25, 185 lb uplift at joint 2, 54 lb uplift at joint 35, 59 lb uplift at joint 37, 55 lb uplift at joint 39, 75 lb uplift at joint 40, 92 lb uplift at joint 41, 117 lb uplift at joint 42, 124 lb uplift at joint 43, 24 lb uplift at joint 44, 75 lb uplift at joint 34, 69 lb uplift at joint 33, 66 lb uplift at joint 32, 105 lb uplift at joint 31, 76 lb uplift at joint 30, 79 lb uplift at joint 29, 90 lb uplift at joint 28 and 72 lb uplift at joint 27.

LOAD CASE(S) Standard





4

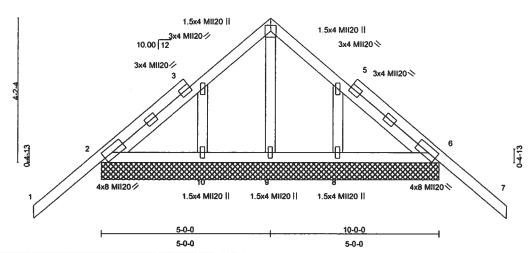


Plate Offsets (X,Y):	[2:0-3-4,0-2-3], [6:0-3-4,0-2-3]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.29 BC 0.09 WB 0.00 (Matrix)	DEFL in Vert(LL) -0.01 Vert(TL) -0.02 Horz(TL) 0.00	(loc) 7 7 6	Vdefl n/r n/r n/a	L/d 120 120 n/a	PLATES GRIP MII20 249/190 Weight: 62 lb

LUMBER TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 OTHERS

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 2=434/10-0-0, 6=434/10-0-0, 9=13/10-0-0, 10=79/10-0-0, 8=79/10-0-0

Max Horz 2=-142(load case 3)

Max Uplift2=-275(load case 5), 6=-275(load case 6)

Max Grav 2=434(load case 1), 6=434(load case 1), 9=54(load case 2), 10=120(load case 2), 8=120(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/75, 2-3=-328/126, 3-4=-233/143, 4-5=-233/143, 5-6=-328/125, 6-7=0/75

BOT CHORD 2-10=-15/189, 9-10=-15/189, 8-9=-15/189, 6-8=-15/189

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 275 lb uplift at joint 6.

LOAD CASE(S) Standard

October 31,2006

Job	Truss	Truss Type	W. 24402	Qty	Ply			E24	606423
MORANJ	F1	FLOOR		13	1	Job Reference (optional)		E30	100423
SANTA FE TRUSS, HIGH SPR	RINGS FL				6.3	00 s Jul 11 2006 MiTek Indus) stries, Inc. Mon Oct 30	0 14 26:24 2006 Pag	je 1
0-3-8									
<u> </u>		<u> </u>	0-8 2-0-6	<u> </u>	0-9-8			0-3-8 Scale =	4:00.5
								Scale =	1:22.5
1.5x4 MII20 II			1.5x4 MII20 II	1.5x4 Mil	20			1.5x4 MII20	
1	2	3	4	5	6		7	8	
			П			7			Ī
	(大)	/\\			/-	7	/X\		
		$//\setminus$			//		// \\		2-0-0
١ ١					/ /		/ \		2
		/	\ 1 1	1 1	/	\ \ / /		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	- 1

<u> </u>	5-6-0	, 6-	8-0 7-8-0		12-11-0	
'	5-6-0	<u> </u>	0-0 1-0-0		5-5-0	,
Plate Offsets (X,Y):	[1:0-2-0,0-1-0], [8:0-2-0,0-1-0]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc) Vdefl L/d	PLATES	GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.34	Vert(LL)	-0.03 12 >999 480	MII20	249/190
TCDL 10.0	Lumber Increase 1.00	BC 0.52	Vert(TL)	-0.05 11-12 >999 360)	
BCLL 0.0	Rep Stress Incr YES	WB 0.19	Horz(TL)	0.02 9 n/a n/a	1	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 72 lb	,

LUMBER

TOP CHORD 2 X 4 SYP No.3 BOT CHORD 2 X 4 SYP No.3

WEBS 2 X 4 SYP No.3 **BRACING**

11

TOP CHORD Structural wood sheathing directly applied or 5-1-15 oc purlins, except

4x12 Mil20=

end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

10

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 14=678/0-8-0, 9=678/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

1-14=-74/0, 8-9=-74/0, 1-2=-28/0, 2-3=-826/0, 3-4=-1189/0, 4-5=-1189/0, 5-6=-1189/0, 6-7=-826/0, 7-8=-29/0

TOP CHORD **BOT CHORD** 13-14=0/552, 12-13=0/1088, 11-12=0/1189, 10-11=0/1090, 9-10=0/552

13

WEBS

4-12=-249/0, 5-11=-264/0, 2-14=-782/0, 2-13=0/464, 3-13=-444/0, 3-12=0/376, 7-9=-781/0, 7-10=0/463, 6-10=-447/0,

12

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MII20 unless otherwise indicated.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

October 31,2006

Edenton, NC 27932

Truss Type Qty Job Truss F3606424 21 VALLEY MORANJ PB Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:25 2006 Page 1 SANTA FE TRUSS HIGH SPRINGS FL 2-0-15 2-0-15 2-1-0 4x4 MII20 = Scale = 1:10.3 10.00 12 ⁴ 5 2 2x4 MJi20= 1.5x4 My20 | 2x4 MII20= 4-1-15 CSI **GRIP PLATES LOADING** (psf) SPACING 2-0-0 DEFL in (loc) **Vdefl** Ľď TCLL 20,0 Plates Increase 1.25 TC 0.03 Vert(LL) n/a n/a 999 MII20 249/190 10.0 Lumber Increase 1.25 BC 0.02 Vert(TL) n/a n/a 999 TCDL. YES WB 0.01 0,00 **BCLL** 0.0 Rep Stress Incr Horz(TL) n/a

LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D OTHERS 2 X 4 SYP No.3

10,0

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-1-15 oc purlins.

Weight: 14 lb

Rigid ceiling directly applied or 10-0-0 oc bracing.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 1=-15/4-1-15, 5=-15/4-1-15, 2=122/4-1-15, 4=122/4-1-15, 6=87/4-1-15

Code FBC2004/TPI2002

Max Horz 1=54(load case 4)

Max Uplift1=-42(load case 3), 5=-15(load case 1), 2=-78(load case 5), 4=-78(load case 5)

Max Grav 1=56(load case 4), 5=27(load case 5), 2=122(load case 1), 4=122(load case 1), 6=89(load case 2)

(Matrix)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-76/73, 2-3=-48/27, 3-4=-48/27, 4-5=-15/29

BOT CHORD 2-6=-11/31, 4-6=-11/31

WEBS 3-6=-50/11

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS automatic zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 15 lb uplift at joint 5, 78 lb uplift at joint 2 and 78 lb uplift at joint 4.

7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4, 6.

8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

October 31,2006



From: The Columbia County Building & Zoning Department

Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529

Reference to a building permit application Number: 0610-76

Applicant Jeff & Bonnie Moran, contractor Owner/Builder, Owners Jeff & Bonnie Moran, Property ID 08-7s-17-09954-007

On the date of October 27, 2006 application 0610-76 and plans for construction of single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0610-76 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

- **1.** Please submit the attached notarized disclosure statement to this department.
- 2. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.
- **3.** On the foundation plan a wood burning fireplace is shown please consult with Mr. Nicholas Geisler to confirm that no additional reinforcing steel is needed in the foundation to support the fireplace and chimney.
- 4. The structural plans show pre-engineered wood floor trusses to be used as floor joist for the loft area. Submit two set of embossed seal engineered design drawing with a truss layout plan. Show a detail which defines the type of sub-flooring material which will be placed onto the truss/joist system.
- 5. One set of truss design plans were submitted with the plans, please request Santa Fe Truss Company to supply a truss layout plan for these trusses. Please have Mr. Nicholas Geisler show a framing detail of the rafter framing around the chimney and covered porch area. Also show a design detail which shows the method of attachment of the 4"x10" ridge beam to the structural walls.
- **6.** The front elevation drawing show fix glass windows which sections R308.4 of the Florida residential Building Code required that areas of glass have safety glazing in an individual fixed or operable panel that meets all of the following conditions:
 - A. Exposed area of an individual pane greater than 9 square feet.
 - B. Bottom edge less than 18 inches (457 mm) above the floor.
- 7. A spiral stair case is shown too be used as an egress method to the loft area of the dwelling. Please show that the spiral stair will comply with sections R311.5.8.1 Spiral stairways are permitted provided the minimum width shall be 26 inches (660 mm) with each tread having a 7½-inches (190 mm) minimum tread depth at 12 inches from the narrower edge. All treads shall be identical, and the rise shall be no more than 9½ inches

- (241 mm). Minimum headroom of 6 feet 6 inches (1982 mm) shall be provided. Handrails shall be provided on one side.
- **8.** Show a detail of the porches and balconies showing compliance with section R312.1 of the Florida residential Building Code. Section R312.1 guards required:
 - A. Porches, balconies or raised floor surfaces located more than 30 inches (762 mm) above the floor or grade below shall have guards not less than 36 inches (914 mm) in height. Open sides of stairs with a total rise of more than 30 inches (762 mm) above the floor or grade below shall have guards not less than 34 inches (864 mm) in height measured vertically from the nosing of the treads.
 - B. Porches and decks which are enclosed with insect screening shall be provided with guards where the walking surface is located more than 30 inches (762 mm) above the floor or grade below. These guards shall also meet the requirements of sections R312.2 Guard opening limitations. Required guards on open sides of stairways, raised floor areas, balconies and porches shall have intermediate rails or ornamental closures which do not allow passage of a sphere 4 inches (102mm) or more in diameter.

Exceptions:

- 1. The triangular openings formed by the riser, tread and bottom rail of a guard at the open side of a stairway are permitted to be of such a size that a sphere 6 inches (152 mm) cannot pass through.
- 2. Openings for required guards on the sides of stair treads shall not allow a sphere 43/8 inches (107 mm) to pass through.

Thank You:

Joe Haltiwanger Plan Examiner

Columbia County Building

Department

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

	TYPE OF	CONSTRUCTION						
() Single Family Dwelling			() Two-Family Reside	nce				
() Farm Outbuilding		() Other						
	NEW CONSTRUC	CTION OR IMPRO	VEMENT					
() New Construction		() Addition, Alteration, Modification or other Improvement						
I	, have been advised of the above disclosure statement for or licensing as an owner/builder. I agree to comply with all requirements							
exemption from contractor li	censing as an owner	/builder. I agree to	comply with all requiremen	nts				
provided for in Florida Statu	ites ss.489.103(7) all	owing this exception	for the construction perm	itted by				
Columbia County Building P	ermit Number		F 02 - 12					
Owner Builder Signature	Date							
The above signer is personall produced identification				3 0				
Notary Signature		Date	(Stamp / Seal	1)				
				*				
9.51 18		LDING USE ONLY						
I hereby certify that the above	e listed owner/build	er has been notified	of the disclosure statemen	t in Florida				
Statutes ss 489.103(7).								
Date	Building Official	/Representative						

From: The Columbia County Building & Zoning Department Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529



Phone Number 386-758-1163 Fax Number 386-754-7088

FAX TRANSMITTAL

To: Jeff & Bonnie Moran

From: Joe Haltiwanger

Date Sent: 10/27/06

CC: Building permit application 0610-76

Number of Pages: Five pages including the cover page

Fax: (386) 454-4086

Message: Reference to building permit application Number: 0610-76

To the review of the party to whom it is addressed. It may contain proprietary and/or privileged information protected by law. If you are not the intended recipient, you may not use, copy or distribute this facsimile message or its attachments. If you have received this transmission in error, please immediately telephone the sender above to arrange for its return.

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Notice of Treatment						
Address: 116 N	Applicator: Florida Pest Control & Chemical Co. (www.flapest.com) Address:					
Site Location: Subdivision Lot # Block# Address 150) Sur CR	Permit #	25220				
Product used	Active Ingredient					
Premise	Imidacloprid	0.1%				
☐ <u>Termidor</u>	Fipronil	0.12%				
Bora-Care Dis	sodium Octaborate Te	etrahydrate 23.0%				
Type treatment: Area Treated		Wood ear feet Gallons Applied				
As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.						
If this notice is for the fin	al exterior treatment, i	nitial this line				
<u>/2-/9-06</u> Date	9:56 Time	Print Technician's Name				
Remarks:						
Applicator - White	Permit File - Canary	Permit Holder - Pink				



RE: MORANJ -

MiTek Industries, Inc.

1801 Massaro Blvd. Tampa, FI 33619

Phone: 813/675-1200

Fax: 813/675-1148

Site Information:

Project Customer: JEFF MORAN Project Name: MORANJ

Lot/Block:

Subdivision:

Address: 1501 SW CR 778

City: HIGH SPRINGS

State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.3 Design Method: User defined

Wind Code: ASCE 7/02 Wind Speed: 110 mph

Floor Load: 55.0 psf

Roof Load: 40 psf, nonconcurrent BCLL=10 psf

This package includes 7 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet

conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Na	me Date
1	T2391995	MORANJ	Α	10/11/06
2	T2391996	MORANJ	A1	10/11/06
3	T2391997	MORANJ	В	10/11/06
4	T2391998	MORANJ	DET	10/11/06
5	T2391999	MORANJ	DET1	10/11/06
6	T2392000	MORANJ	F1	10/11/06
7	T2392001	MORANJ	PB	10/11/06

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie

My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

Truss Type Qty Ply Job Truss T2391995 MORANJ SPECIAL 14 Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:23:55 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino 13-11-1 18-0-15 22-5-6 26-9-12 35-5-2 40-0-0 42-0-0 -2-0-0 2-0-0 7-1-7 4-1-15 4-4-R 4-10-4 3-9-2 4-6-14 2-0-0 Scale = 1:97.3 6x6 MII20 // 5x5 MII20 >> 10.00 12 3x6 MII20 // 6 3x5 MII20 >> 3x5 MII20/ 4.00 12 5x5 MII20≈ 3 3x6 MII20≈ я 9 3x5 Mil20 = 3x4 Mil20 = 10 24 3x5 MII20= 3x5 MII20= 15 14 22 21 20 19 18 17 16 1.5x4 MII20 II 3x6 MII20= 3x4 MII20= 3x5 MII20= 3x4 MII20= 3x6 MII20= 1.5x4 MII20 || 3x4 MII20= 3x10 MII20= 7-1-7 18-0-15 26-9-12 31-8-0 35-5-2 40-0-0 13-11-1 22-5-6 7-1-7 6-9-9 4-1-15 4-4-6 4-4-6 4-10-4 3-9-2 4-6-14 Plate Offsets (X,Y): [2:0-5-3,0-0-10], [5:0-2-12,0-1-8] GRIP DEFL L/d **PLATES** LOADING (psf) **SPACING** in (loc) I/def -0.05 21-22 TC >999 240 MII20 249/190 **TCLL** 20.Ó Plates Increase 1.25 0.35 Vert(LL) вс 0.35 -0.16 21-22 180 Lumber Increase 1.25 Vert(TL) >999 **TCDL** 10.0 WB 0.51 0.04 **BCLL** 10.0 Rep Stress Incr YES Horz(TL) 15 n/a n/a Code FBC2004/TPI2002 Weight: 270 lb **BCDL** (Matrix) 10.0 **BRACING** LUMBER Structural wood sheathing directly applied or 4-10-11 oc purlins. TOP CHORD 2 X 4 SYP No.2D TOP CHORD

BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3

BOT CHORD WEBS

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 3-21, 5-19, 7-19

REACTIONS (lb/size) 2=1346/0-8-0, 15=1812/0-8-0, 12=272/0-3-8

Max Horz 2=-423(load case 3)

Max Uplift2=-399(load case 5), 15=-315(load case 6), 12=-284(load case 4) Max Grav 2=1346(load case 1), 15=1812(load case 1), 12=294(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/79, 2-3=-1579/296, 3-23=-1144/305, 4-23=-1026/306, 4-5=-1008/345, 5-6=-750/319, 6-7=-1059/357, 7-8=-1228/301,

8-9=-883/201, 9-10=-937/193, 10-24=-71/592, 11-24=-83/586, 11-12=-49/175, 12-13=0/37

BOT CHORD 2-22=-142/1088, 21-22=-142/1088, 20-21=0/777, 19-20=0/777, 18-19=-8/881, 17-18=-58/870, 16-17=-58/870,

15-16=-525/192, 14-15=-151/82, 12-14=-151/82

WEBS 3-22=0/306, 3-21=-422/269, 5-21=-117/435, 5-19=-216/174, 6-19=-148/397, 7-19=-291/176, 7-18=-45/164, 8-18=-19/127,

8-16=-735/174, 10-16=-245/1595, 10-15=-1543/304, 11-15=-434/159, 11-14=0/160

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 2, 315 lb uplift at joint 15 and 284 lb uplift at joint 12.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634



Job Truss Truss Type Qty T2391996 MORANJ SPECIAL Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:23:57 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino 18-0-15 22-5-6 26-9-12 35-5-2 40-0-0 42-0-0 7-1-7 6-9-9 4-1-15 4-4-6 4-10-4 3-9-2 4-R-14 2-0-0 6x6 MII20 // Scale: 1/8"=1 10.00 18x6 MII20/ 5x5 MII20 >> 3x5 MII20 ◇ 3x5 MII20/ 4.00 12 5x5 MII20 ≈ 3x6 MII20≈ 3x5 MII20 = 3x4 MII20 = 9 10 12 4x6 MII20= 3x5 MII20= 21 20 19 18 17 16 15 13 14 1.5x4 MII20 || 3x6 MII20 == 3x4 MII20= 3x5 MII20= 3x4 MII20= 3x4 MII20= 3x10 MII20= 3x6 MII20= 1.5x4 MII20 | 13-11-1 18-0-15 22-5-6 26-9-12 31-8-0 35-5-2 40-0-0 7-1-7 6-9-9 4-1-15 4-4-6 4-4-6 4-10-4 3-9-2 4-6-14 Plate Offsets (X,Y): [1:0-6-3,0-0-6], [4:0-2-12,0-1-8] LOADING (psf) **SPACING** CSI DEFL **PLATES GRIP** (loc) l/defl L/d TCLL 20.0 Plates Increase 1.25 TC 0.34 Vert(LL) 0.10 1-21 >999 240 MII20 249/190 **TCDL** 1.25 BC 0.39 -0.17 180 10.0 Lumber Increase Vert(TL) 1-21 >999 WB **BCLL** 10.0 Rep Stress Incr YES 0.51 0.04 Horz(TL) n/a n/a **BCDL** 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 266 lb **BRACING** LUMBER TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purlins. **BOT CHORD** BOT CHORD 2 X 4 SYP No.2D Rigid ceiling directly applied or 6-0-0 oc bracing.

WEBS 2 X 4 SYP No.3

WEBS 1 Row at midpt 2-20, 4-18, 6-18

REACTIONS (lb/size) 1=1200/0-8-0, 14=1819/0-8-0, 11=271/0-3-8

Max Horz 1=-452(load case 3)

Max Uplift1=-264(load case 5), 14=-318(load case 6), 11=-283(load case 4) Max Grav 1=1200(load case 1), 14=1819(load case 1), 11=293(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1607/337, 2-22=-1154/320, 3-22=-1013/321, 3-4=-959/359, 4-5=-756/327, 5-6=-1066/368, 6-7=-1234/310,

7-8=-886/206, 8-9=-941/198, 9-23=-77/597, 10-23=-89/590, 10-11=-55/179, 11-12=0/37

BOT CHORD 1-21=-182/1117, 20-21=-182/1117, 19-20=0/783, 18-19=0/783, 17-18=-15/886, 16-17=-63/873, 15-16=-63/873,

14-15=-529/194, 13-14=-155/88, 11-13=-155/88

2-21=0/313, 2-20=-451/310, 4-20=-133/438, 4-18=-216/174, 5-18=-152/400, 6-18=-291/176, 6-17=-45/164, 7-17=-19/127, **WEBS**

7-15=-740/180, 9-15=-257/1604, 9-14=-1550/314, 10-14=-434/159, 10-13=0/160

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 1, 318 lb uplift at joint 14 and 283 lb uplift at joint 11.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634



Qty Job Ply Truss Truss Type T2391997 3 MORANJ COMMON Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:23:59 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino -2-0-0 10-0-0 12-0-0 5-0-0 5-0-0 2-0-0 2-0-0 Scale = 1:34.2 4x5 MII20= 3 10.00 12 ا ا 04-13 6 3x4 MII20= 3x4 MII20 1.5x4 MII20 || 5-0-0 10-0-0 5-0-0 5-0-0 Plate Offsets (X,Y): [2:0-2-1,0-1-8], [4:0-2-1,0-1-8] LOADING (psf)

LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WEBS

20.0

10.0

10.0

10.0

BRACING

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

in (loc)

2-6

4-6

-0.02

-0.03

0.00

I/defl

>999

>999

n/a

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

L/d

240

180

n/a

PLATES

Weight: 48 lb

MII20

GRIP

249/190

REACTIONS (lb/size) 2=517/0-3-8, 4=517/0-3-8

Max Horz 2=-155(load case 3)

SPACING

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

Max Uplift2=-251 (load case 5), 4=-251 (load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/77, 2-7=-396/78, 3-7=-302/95, 3-8=-302/95, 4-8=-396/77, 4-5=0/77 2-6=0/228, 4-6=0/228 TOP CHORD

2-0-0

1.25

1.25

YES

BOT CHORD

WEBS 3-6=0/217

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

CSI

TC BC

WB 0.07

(Matrix)

0.32

0.14

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634



Qty Job Truss Truss Type Ply T2391998 2 DET GABLE MORANJ Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:24:02 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino 42-0-0 26-9-12 40-0-0 -2-0-0 13-11-1 18-0-15 13-2-4 2-0-0 8-8-12 2-0-0 13-11-1 4-1-15 Scale = 1:84.1 6x6 MII20 / 10.00 123x6 MII20// 6x6 MII20 >> 11 12 13 10 9 14 15 16 3x8 MII20> 4.00 12 17 7x8 MII20 [] 1819 3x4 MII20~ 20 21 3x4 MII20~ 23 24 26 5x8 MII20 || 33 32 30 29 42 41 40 37 44 43 3x5 MII20= 5x5 MII20= 5x5 MII20= 5x6 MII20= 40-0-0 40-0-0 Plate Offsets (X,Y): [2:0-3-0,0-2-15], [11:0-2-4,Edge], [13:Edge,0-2-11], [18:0-3-0,0-1-5], [25:0-4-12,Edge], [25:0-3-8,Edge], [33:0-2-8,0-3-0], [37:0-2-8,0-3-0] GRIP LOADING (psf) CSI DEFL 1/d**PLATES SPACING** in (loc) 1/deft 249/190 Plates Increase 1.25 TC 0.29 Vert(LL) -0.0226 n/r 120 MII20 TCLL 20.0 1.25 BC 0.07 Vert(TL) -0.0326 n/r 120

LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **OTHERS** 2 X 4 SYP No.3

10.0

10.0

10.0

BRACING

Horz(TL)

TOP CHORD **BOT CHORD** WFBS

0.01

25

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 300 lb

14-35, 13-36, 12-37, 11-38, 9-39 1 Row at midpt

n/a

REACTIONS (lb/size) 25=301/40-0-0, 2=265/40-0-0, 35=156/40-0-0, 36=147/40-0-0, 37=165/40-0-0, 38=147/40-0-0, 39=156/40-0-0,

n/a

40=161/40-0-0, 41=160/40-0-0, 42=158/40-0-0, 43=171/40-0-0, 44=100/40-0-0, 34=161/40-0-0, 33=159/40-0-0, 32=166/40-0-0, 31=164/40-0-0, 30=158/40-0-0, 29=165/40-0-0, 28=140/40-0-0, 27=241/40-0-0

WB

(Matrix)

0.14

Max Horz 2=-408(load case 3)

Lumber Increase

Rep Stress Incr

Code FBC2004/TPI2002

Max Uplift25=-227(load case 4), 2=-185(load case 3), 35=-54(load case 5), 37=-59(load case 4), 39=-55(load case 5),

40=-75(load case 6), 41=-92(load case 5), 42=-117(load case 5), 43=-124(load case 5), 44=-24(load case 6), 34=-75(load case 6), 33=-69(load case 5), 32=-66(load case 6), 31=-105(load case 4), 30=-76(load case 4),

29=-79(load case 4), 28=-90(load case 4), 27=-72(load case 4)

YES

Max Grav 25=301 (load case 1), 2=265 (load case 1), 35=156 (load case 10), 36=149 (load case 10), 37=169 (load case 10),

38=147(load case 9), 39=156(load case 1), 40=161(load case 1), 41=160(load case 1), 42=158(load case 9),

43=171 (load case 1), 44=108 (load case 2), 34=161 (load case 1), 33=159 (load case 10), 32=166 (load case 1)

31=164(load case 1), 30=158(load case 10), 29=165(load case 1), 28=140(load case 10), 27=241(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/75, 2-3=-237/296, 3-4=-199/273, 4-5=-189/281, 5-6=-140/287, 6-7=-72/276, 7-8=-59/268, 8-9=-60/262, 9-10=-57/276, TOP CHORD

10-11=-5/284, 11-12=-16/246, 12-13=-16/246, 13-14=-58/284, 14-15=-60/225, 15-16=-59/192, 16-17=-60/207,

17-18=-44/167, 18-19=-46/167, 19-20=-69/126, 20-21=-96/90, 21-22=-122/51, 22-23=-147/13, 23-24=-169/19, 24-25=-177/8 25-26=0/36

2-44=0/209, 43-44=0/208, 42-43=0/208, 41-42=0/208, 40-41=0/208, 39-40=0/208, 38-39=0/208, 37-38=0/207, 36-37=0/207, 35-36=0/207, 34-35=0/207, 33-34=0/207, 32-33=0/207, 31-32=0/207, 30-31=0/207, 29-30=0/207, 28-29=0/207, 27-28=0/207

25-27=0/207

14-35=-116/74, 13-36=-109/0, 12-37=-129/79, 11-38=-108/0, 9-39=-116/74, 8-40=-121/95, 7-41=-120/111, 6-42=-119/139,

5-43=-126/136, 3-44=-77/53, 15-34=-121/95, 16-33=-119/89, 17-32=-126/86, 19-31=-124/125, 20-30=-119/95,

21-29=-122/102, 22-28=-111/101, 23-27=-174/113

WEBS

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002

4) Provide adequate drainage to prevent water ponding.

5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All plates are 1.5x4 MII20 unless otherwise indicated.

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

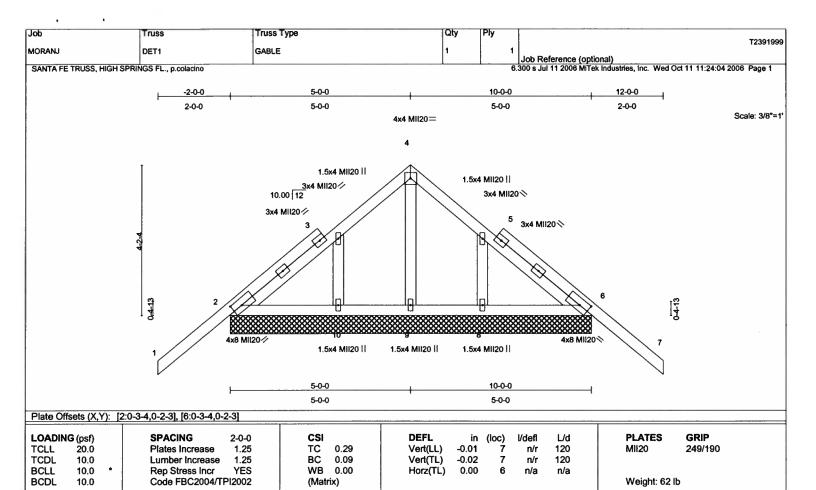


Јор	Truss	Truss Type	Qty	Ply	-	
MORANJ	DET	GABLE	2	1	1	72391998
<u> </u>	E			1	Job Reference (optional)	
SANTA FE TRUSS, HIGH SPR	INGS FL., p.colacino		172	6	.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:24:02 2006 P	age 2

NOTES

- 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 25, 185 lb uplift at joint 2, 54 lb uplift at joint 35, 59 lb uplift at joint 37, 55 lb uplift at joint 39, 75 lb uplift at joint 40, 92 lb uplift at joint 41, 117 lb uplift at joint 42, 124 lb uplift at joint 43, 24 lb uplift at joint 44, 75 lb uplift at joint 34, 69 lb uplift at joint 33, 66 lb uplift at joint 32, 105 lb uplift at joint 31, 76 lb uplift at joint 30, 79 lb uplift at joint 29, 90 lb uplift at joint 28 and 72 lb uplift at joint 27.

LOAD CASE(S) Standard



LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D OTHERS 2 X 4 SYP No.3 **BRACING**

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=434/10-0-0, 6=434/10-0-0, 9=13/10-0-0, 10=79/10-0-0, 8=79/10-0-0

Max Horz 2=-142(load case 3)

Max Uplift2=-275(load case 5), 6=-275(load case 6)

Max Grav 2=434(load case 1), 6=434(load case 1), 9=54(load case 2), 10=120(load case 2), 8=120(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/75, 2-3=-328/126, 3-4=-233/143, 4-5=-233/143, 5-6=-328/125, 6-7=0/75

BOT CHORD 2-10=-15/189, 9-10=-15/189, 8-9=-15/189, 6-8=-15/189

NOTES

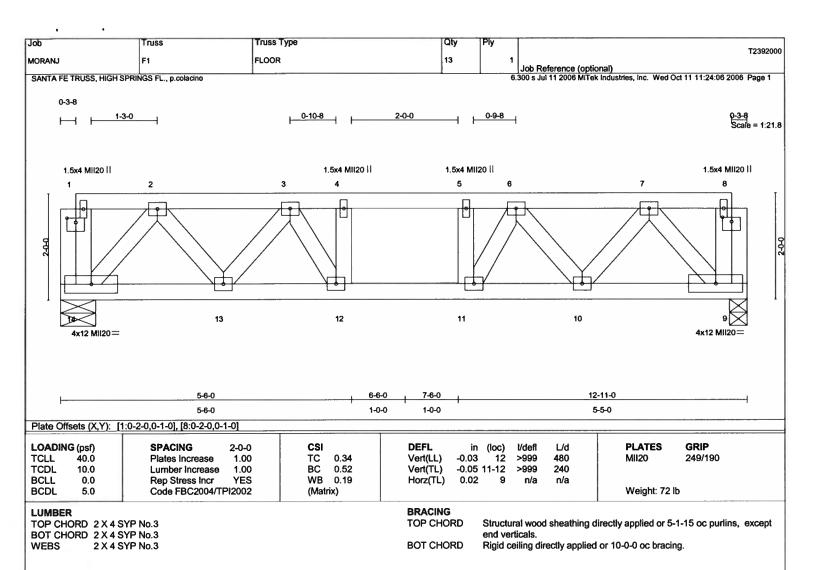
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 275 lb uplift at joint 6

LOAD CASE(S) Standard

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REACTIONS (lb/size) 14=678/0-8-0, 9=678/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

1-14=-74/0, 8-9=-74/0, 1-2=-28/0, 2-3=-826/0, 3-4=-1189/0, 4-5=-1189/0, 5-6=-1189/0, 6-7=-826/0, 7-8=-29/0 TOP CHORD

BOT CHORD 13-14=0/552, 12-13=0/1088, 11-12=0/1189, 10-11=0/1090, 9-10=0/552

4-12=-249/0, 5-11=-264/0, 2-14=-782/0, 2-13=0/464, 3-13=-444/0, 3-12=0/376, 7-9=-781/0, 7-10=0/463, 6-10=-447/0. **WEBS**

6-11=0/390

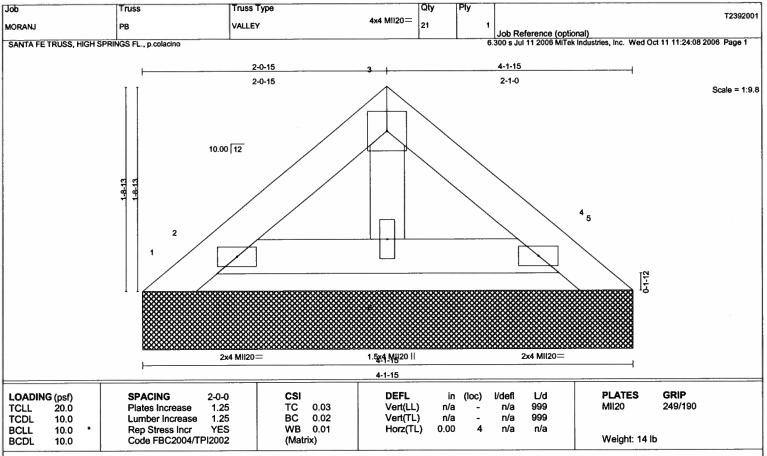
NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MII20 unless otherwise indicated.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

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LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-1-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-15/4-1-15, 5=-15/4-1-15, 2=122/4-1-15, 4=122/4-1-15, 6=87/4-1-15

Max Horz 1=54(load case 4)

Max Uplift1=-42(load case 3), 5=-15(load case 1), 2=-78(load case 5), 4=-78(load case 5)

Max Grav 1=56(load case 4), 5=27(load case 5), 2=122(load case 1), 4=122(load case 1), 6=89(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-76/73, 2-3=-48/27, 3-4=-48/27, 4-5=-15/29

BOT CHORD 2-6=-11/31, 4-6=-11/31

WEBS 3-6=-50/11

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS automatic zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Gable requires continuous bottom chord bearing.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 15 lb uplift at joint 5, 78 lb uplift at joint 2 and 78 lb uplift at joint 4.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4, 6.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in. SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006



MITEK POWER TO PERFORM.



RE: MORANJ -

MiTek Industries, Inc.

1801 Massaro Blvd. Tampa, FI 33619

Phone: 813/675-1200

Fax: 813/675-1148

Site Information:

Project Customer: JEFF MORAN Project Name: MORANJ

Lot/Block:

Subdivision:

Address: 1501 SW CR 778

City: HIGH SPRINGS

State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.3

Wind Code: ASCE 7/02 Wind Speed: 110 mph

Design Method: User defined

Roof Load: 40 psf, nonconcurrent BCLL=10 psf

Floor Load: 55.0 psf

This package includes 7 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Na	me Date
1	T2391995	MORANJ	Α	10/11/06
2	T2391996	MORANJ	A1	10/11/06
3	T2391997	MORANJ	В	10/11/06
4	T2391998	MORANJ	DET	10/11/06
5	T2391999	MORANJ	DET1	10/11/06
6	T2392000	MORANJ	F1	10/11/06
7	T2392001	MORANJ	PB	10/11/06

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie

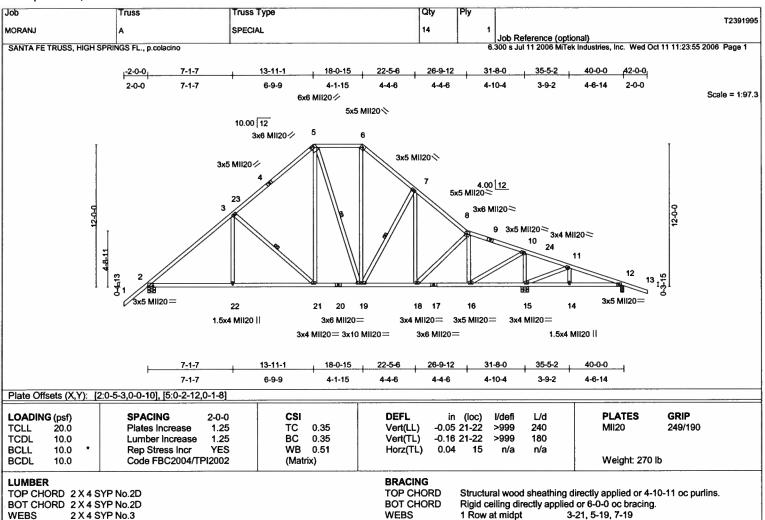
My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006

Zhang, Guo-jie



WEBS

1 Row at midpt 3-21, 5-19, 7-19

REACTIONS (lb/size) 2=1346/0-8-0, 15=1812/0-8-0, 12=272/0-3-8

Max Horz 2=-423(load case 3)

Max Uplift2=-399(load case 5), 15=-315(load case 6), 12=-284(load case 4) Max Grav 2=1346(load case 1), 15=1812(load case 1), 12=294(load case 10)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/79, 2-3=-1579/296, 3-23=-1144/305, 4-23=-1026/306, 4-5=-1008/345, 5-6=-750/319, 6-7=-1059/357, 7-8=-1228/301,

8-9=-883/201, 9-10=-937/193, 10-24=-71/592, 11-24=-83/586, 11-12=-49/175, 12-13=0/37

2-22=-142/1088, 21-22=-142/1088, 20-21=0/777, 19-20=0/777, 18-19=-8/881, 17-18=-58/870, 16-17=-58/870, **BOT CHORD**

15-16=-525/192, 14-15=-151/82, 12-14=-151/82

3-22=0/306, 3-21=-422/269, 5-21=-117/435, 5-19=-216/174, 6-19=-148/397, 7-19=-291/176, 7-18=-45/164, 8-18=-19/127, 8-16=-735/174, 10-16=-245/1595, 10-15=-1543/304, 11-15=-434/159, 11-14=0/160

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

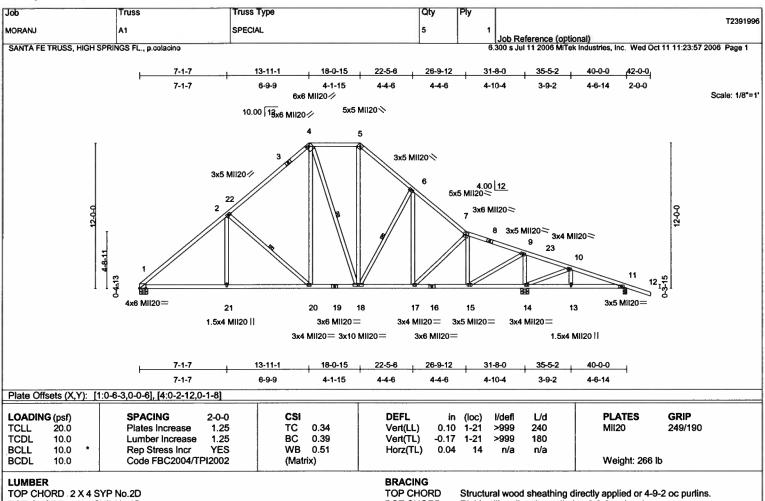
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 2, 315 lb uplift at joint 15 and 284 lb uplift at joint 12.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634





BOT CHORD 2 X 4 SYP No.2D

WEBS 2 X 4 SYP No.3 **BOT CHORD** WEBS

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 2-20, 4-18, 6-18

REACTIONS: (lb/size) 1=1200/0-8-0, 14=1819/0-8-0, 11=271/0-3-8

Max Horz 1=-452(load case 3)

Max Uplift1=-264(load case 5), 14=-318(load case 6), 11=-283(load case 4) Max Grav 1=1200 (load case 1), 14=1819 (load case 1), 11=293 (load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1607/337, 2-22=-1154/320, 3-22=-1013/321, 3-4=-959/359, 4-5=-756/327, 5-6=-1066/368, 6-7=-1234/310, TOP CHORD

7-8=-886/206, 8-9=-941/198, 9-23=-77/597, 10-23=-89/590, 10-11=-55/179, 11-12=0/37

BOT CHORD 1-21=-182/1117, 20-21=-182/1117, 19-20=0/783, 18-19=0/783, 17-18=-15/886, 16-17=-63/873, 15-16=-63/873,

14-15=-529/194, 13-14=-155/88, 11-13=-155/88

2-21=0/313, 2-20=-451/310, 4-20=-133/438, 4-18=-216/174, 5-18=-152/400, 6-18=-291/176, 6-17=-45/164, 7-17=-19/127,

7-15=-740/180, 9-15=-257/1604, 9-14=-1550/314, 10-14=-434/159, 10-13=0/160

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

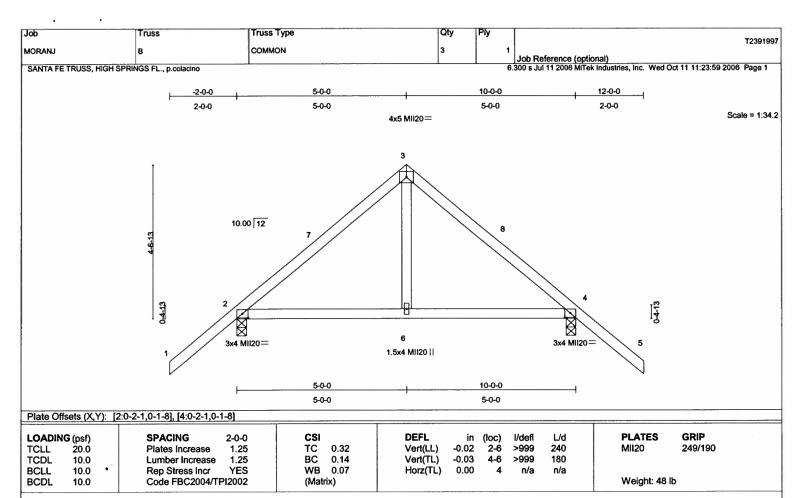
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 1, 318 lb uplift at joint 14 and 283 lb uplift at joint 11.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634





LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **WEBS**

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=517/0-3-8, 4=517/0-3-8

Max Horz 2=-155(load case 3)

Max Uplift2=-251 (load case 5), 4=-251 (load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/77, 2-7=-396/78, 3-7=-302/95, 3-8=-302/95, 4-8=-396/77, 4-5=0/77 TOP CHORD

BOT CHORD 2-6=0/228, 4-6=0/228

WEBS 3-6=0/217

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634



Job Truss Truss Type Qty T2391998 GABLE 2 DET MORANJ | Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:24:02 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino -2-0-0 13-11-1 18-0-15 26-9-12 40-0-0 42-0-0 13-11-1 4-1-15 8-8-12 13-2-4 2-0-0 2-0-0 Scale = 1:84.1 6x6 MII20 // 10.00 125x6 MII20// 6x6 MII20 > 11 12 13 10 9 14 15 16 3x6 MII20≈ 4.00 12 7x8 MII20 || 17 1819 20 3x4 MII20~ 21 3x4 MII20~ 23 25 26 5x8 MII20 | 37 33 32 40 39 36 35 34 44 43 42 41 38 3x5 MII20= 5x5 MII20= 5x5 MII20= 5x6 MII20= 40-0-0 Plate Offsets (X,Y): [2:0-3-0,0-2-15], [11:0-2-4,Edge], [13:Edge,0-2-11], [18:0-3-0,0-1-5], [25:0-4-12,Edge], [25:0-3-8,Edge], [33:0-2-8,0-3-0], [37:0-2-8,0-3-0] SPACING GRIP LOADING (psf) CSI **DEFL** l/defl **PLATES** TC Vert(LL) -0.02 120 MII20 249/190 TCLL 20.0 Plates Increase 1.25 0.29 n/r 1.25 BC 0.07 Vert(TL) -0.03n/r 120 Lumber Increase TCDL 10.0 WB 0.01 25 n/a n/a 0.14 Horz(TL) BCLL 10.0 Rep Stress Incr YES Weight: 300 lb Code FBC2004/TPI2002 BCDL 10.0 (Matrix) BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **OTHERS**

BOT CHORD WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing. 14-35, 13-36, 12-37, 11-38, 9-39 1 Row at midpt

REACTIONS (lb/size) 25=301/40-0-0, 2=265/40-0-0, 35=156/40-0-0, 36=147/40-0-0, 37=165/40-0-0, 38=147/40-0-0, 39=156/40-0-0, 40=161/40-0-0, 41=160/40-0-0, 42=158/40-0-0, 43=171/40-0-0, 44=100/40-0-0, 34=161/40-0-0, 33=159/40-0-0, 32=166/40-0-0, 31=164/40-0-0, 30=158/40-0-0, 29=165/40-0-0, 28=140/40-0-0, 27=241/40-0-0

Max Horz 2=-408(load case 3)

Max Uplift25=-227(load case 4), 2=-185(load case 3), 35=-54(load case 5), 37=-59(load case 4), 39=-55(load case 5),

40=-75(load case 6), 41=-92(load case 5), 42=-117(load case 5), 43=-124(load case 5), 44=-24(load case 6). 34=-75(load case 6), 33=-69(load case 5), 32=-66(load case 6), 31=-105(load case 4), 30=-76(load case 4),

29=-79(load case 4), 28=-90(load case 4), 27=-72(load case 4)

Max Grav 25=301 (load case 1), 2=265 (load case 1), 35=156 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 36=149 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 36=149 (load cas 38=147(load case 9), 39=156(load case 1), 40=161(load case 1), 41=160(load case 1), 42=158(load case 9),

43=171(load case 1), 44=108(load case 2), 34=161(load case 1), 33=159(load case 10), 32=166(load case 1), 31=164(load case 1), 30=158(load case 10), 29=165(load case 1), 28=140(load case 10), 27=241(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

 $1-2=0/75,\ 2-3=-237/296,\ 3-4=-199/273,\ 4-5=-189/281,\ 5-6=-140/287,\ 6-7=-72/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 7-8=-59/268,\ 8-9=-60/262,\ 9-10=-57/276,\ 9-10=$ TOP CHORD

10-11=-5/284, 11-12=-16/246, 12-13=-16/246, 13-14=-58/284, 14-15=-60/225, 15-16=-59/192, 16-17=-60/207, 17-18=-44/167, 18-19=-46/167, 19-20=-69/126, 20-21=-96/90, 21-22=-122/51, 22-23=-147/13, 23-24=-169/19, 24-25=-177/8

2-44=0/209, 43-44=0/208, 42-43=0/208, 41-42=0/208, 40-41=0/208, 39-40=0/208, 38-39=0/208, 37-38=0/207, 36-37=0 35-36=0/207, 34-35=0/207, 33-34=0/207, 32-33=0/207, 31-32=0/207, 30-31=0/207, 29-30=0/207, 28-29=0/207, 27-28=0/207

25-27=0/207

14-35=-116/74, 13-36=-109/0, 12-37=-129/79, 11-38=-108/0, 9-39=-116/74, 8-40=-121/95, 7-41=-120/111, 6-42=-119/139,

5-43=-126/136, 3-44=-77/53, 15-34=-121/95, 16-33=-119/89, 17-32=-126/86, 19-31=-124/125, 20-30=-119/95,

21-29=-122/102, 22-28=-111/101, 23-27=-174/113

NOTES

WEBS

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

4) Provide adequate drainage to prevent water ponding.

5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All plates are 1.5x4 MII20 unless otherwise indicated.

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

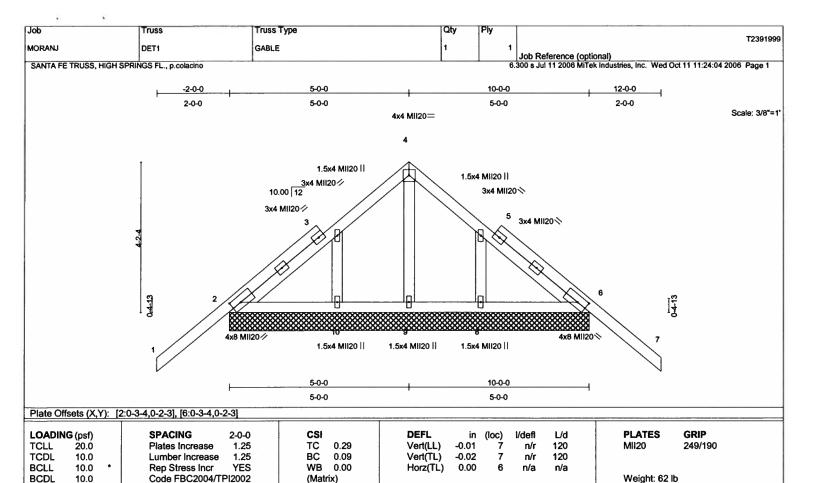


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Job	Truss	Truss Type	Qty	Pły	T0004000
MORANJ	DET	GABLE	2	1 1	T2391998
			_		Job Reference (optional)
SANTA FE TRUSS, HIGH SPRI	NGS FL., p.colacino			6.	300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:24:02 2006 Page 2

SANTA FE TRUSS, HIGH SPRINGS FL., p. colacino

- 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 25, 185 lb uplift at joint 2, 54 lb uplift at joint 35, 59 lb uplift at joint 37, 55 lb uplift at joint 39, 75 lb uplift at joint 40, 92 lb uplift at joint 41, 117 lb uplift at joint 42, 124 lb uplift at joint 43, 24 lb uplift at joint 44, 75 lb uplift at joint 34, 69 lb uplift at joint 39, 75 lb uplift at joint 44, 75 lb uplift at joint 39, 75 lb uplift at joint 40, 92 lb uplift 40, 33, 66 lb uplift at joint 32, 105 lb uplift at joint 31, 76 lb uplift at joint 30, 79 lb uplift at joint 29, 90 lb uplift at joint 28 and 72 lb uplift at joint 27.

LOAD CASE(S) Standard



LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=434/10-0-0, 6=434/10-0-0, 9=13/10-0-0, 10=79/10-0-0, 8=79/10-0-0

Max Horz 2=-142(load case 3)

Max Uplift2=-275(load case 5), 6=-275(load case 6)

Max Grav 2=434(load case 1), 6=434(load case 1), 9=54(load case 2), 10=120(load case 2), 8=120(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/75, 2-3=-328/126, 3-4=-233/143, 4-5=-233/143, 5-6=-328/125, 6-7=0/75

BOT CHORD 2-10=-15/189, 9-10=-15/189, 8-9=-15/189, 6-8=-15/189

NOTES

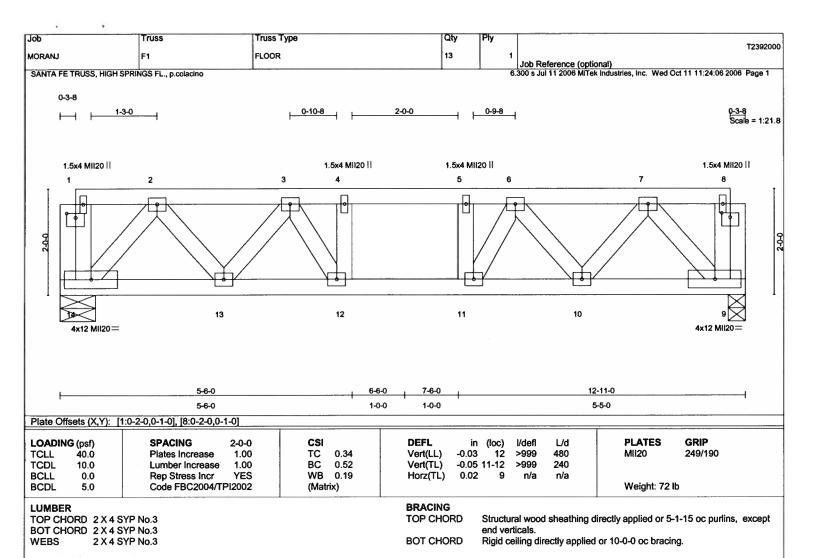
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 275 lb uplift at joint 6

LOAD CASE(S) Standard

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REACTIONS (lb/size) 14=678/0-8-0, 9=678/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

1-14=-74/0, 8-9=-74/0, 1-2=-28/0, 2-3=-826/0, 3-4=-1189/0, 4-5=-1189/0, 5-6=-1189/0, 6-7=-826/0, 7-8=-29/0 TOP CHORD

BOT CHORD 13-14=0/552, 12-13=0/1088, 11-12=0/1189, 10-11=0/1090, 9-10=0/552

4-12=-249/0, 5-11=-264/0, 2-14=-782/0, 2-13=0/464, 3-13=-444/0, 3-12=0/376, 7-9=-781/0, 7-10=0/463, 6-10=-447/0, WEBS

6-11=0/390

NOTES

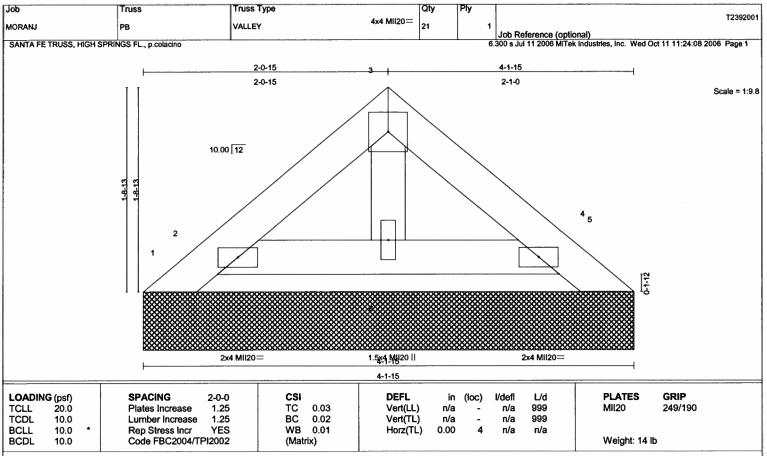
- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MII20 unless otherwise indicated.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006

14515 N. Outer Forty, Suite #300 Chesterfield, MO 63017



LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **OTHERS** 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 4-1-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-15/4-1-15, 5=-15/4-1-15, 2=122/4-1-15, 4=122/4-1-15, 6=87/4-1-15

Max Horz 1=54(load case 4)

Max Uplift1=-42(load case 3), 5=-15(load case 1), 2=-78(load case 5), 4=-78(load case 5)

Max Grav 1=56(load case 4), 5=27(load case 5), 2=122(load case 1), 4=122(load case 1), 6=89(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-76/73, 2-3=-48/27, 3-4=-48/27, 4-5=-15/29

BOT CHORD 2-6=-11/31, 4-6=-11/31

3-6=-50/11 **WEBS**

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS automatic zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Gable requires continuous bottom chord bearing.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 15 lb uplift at joint 5, 78 lb uplift at joint 2 and 78 lb uplift at joint 4.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4, 6.

8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in. SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634





RE: MORANJ -

MiTek Industries, Inc.

1801 Massaro Blvd.

Tampa, FI 33619

Phone: 813/675-1200

Fax: 813/675-1148

Site Information:

Project Customer: JEFF MORAN Project Name: MORANJ

Lot/Block:

Subdivision:

Address: 1501 SW CR 778

City: HIGH SPRINGS

State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.3

Wind Code: ASCE 7/02 Wind Speed: 110 mph

Design Method: User defined

Roof Load: 40 psf, nonconcurrent BCLL=10 psf

Floor Load: 55.0 psf

This package includes 7 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Nar	neDate
1	T2391995	MORANJ	Α	10/11/06
2	T2391996	MORANJ	A1	10/11/06
3	T2391997	MORANJ	В	10/11/06
4	T2391998	MORANJ	DET	10/11/06
5	T2391999	MORANJ	DET1	10/11/06
6	T2392000	MORANJ	F1	10/11/06
7	T2392001	MORANJ	PB	10/11/06

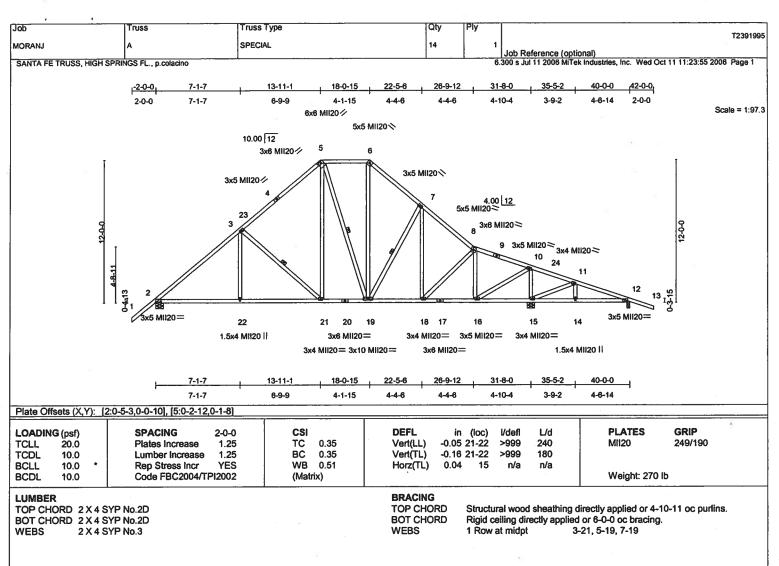
The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie

My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634



REACTIONS (lb/size) 2=1346/0-8-0, 15=1812/0-8-0, 12=272/0-3-8

Max Horz 2=-423(load case 3)

Max Uplift2=-399(load case 5), 15=-315(load case 6), 12=-284(load case 4)

Max Grav 2=1346(load case 1), 15=1812(load case 1), 12=294(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/79, 2-3=-1579/296, 3-23=-1144/305, 4-23=-1026/306, 4-5=-1008/345, 5-6=-750/319, 6-7=-1059/357, 7-8=-1228/301,

8-9=-883/201, 9-10=-937/193, 10-24=-71/592, 11-24=-83/586, 11-12=-49/175, 12-13=0/37

BOT CHORD 2-22=-142/1088, 21-22=-142/1088, 20-21=0/777, 19-20=0/777, 18-19=-8/881, 17-18=-58/870, 16-17=-58/870,

15-16=-525/192, 14-15=-151/82, 12-14=-151/82

3-22=0/306, 3-21=-422/269, 5-21=-117/435, 5-19=-216/174, 6-19=-148/397, 7-19=-291/176, 7-18=-45/164, 8-18=-19/127,

8-16=-735/174, 10-16=-245/1595, 10-15=-1543/304, 11-15=-434/159, 11-14=0/160

NOTES

WERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 2, 315 lb uplift at joint 15 and 284 lb uplift at joint 12.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not huss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANTPIT Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Qty Job Truss russ Type T2391996 5 SPECIAL MORANJ Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:23:57 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino 40-0-0 18-0-15 22-5-6 35-5-2 42-0-0 4-1-15 2-0-0 7-1-7 4-10-4 4-6-14 6x6 MII20 // Scale: 1/8"=1 5x5 MII20 ◇ 10.00 13x6 MII20 // 5 3x5 MII20 >> 3x5 MII20/ 4.00 12 5x5 MII20≈ 3x6 MII20 > 8 3x5 MII20 = 3x4 MII20 = 23 10 3x5 Mil20= 4x6 MII20= 19 15 13 21 20 18 17 16 14 3x6 MII20= 3x4 MII20= 3x5 MII20= 3x4 MII20= 1.5x4 MII20 II 3x6 MII20= 1.5x4 MH20 | 3x4 MII20= 3x10 MII20= 7-1-7 13-11-1 18-0-15 22-5-8 26-9-12 31-8-0 35-5-2 40-0-0 7-1-7 6-9-9 4-1-15 4-4-6 4-4-6 4-10-4 3-9-2 4-8-14 Plate Offsets (X,Y): [1:0-6-3,0-0-6], [4:0-2-12,0-1-8] LOADING (psf) **PLATES GRIP SPACING** CSI DEFL in (loc) Vdeft L/d 1.25 TC 0.34 Vert(LL) 0.10 1-21 >999 240 **MII20** 249/190 **TCLL** 20.0 Plates Increase 180 BC 0.39 -0.17>999 TCDL 10.0 Lumber Increase 1.25 Vert(TL) 1-21 **BCLL** 10.0 Rep Stress Incr YES WB 0.51 Horz(TL) 0.04 14 n/a n/a **BCDL** Code FBC2004/TPI2002 (Matrix) Weight: 266 lb 10.0 LUMBER **BRACING TOP CHORD** Structural wood sheathing directly applied or 4-9-2 oc purlins. TOP CHORD 2 X 4 SYP No.2D **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.2D **WEBS** 1 Row at midpt 2-20, 4-18, 6-18 2 X 4 SYP No.3 WERS

REACTIONS (lb/size) 1=1200/0-8-0, 14=1819/0-8-0, 11=271/0-3-8

Max Horz 1=-452(load case 3)

Max Uplift1=-264(load case 5), 14=-318(load case 6), 11=-283(load case 4) Max Grav 1=1200(load case 1), 14=1819(load case 1), 11=293(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1607/337, 2-22=-1154/320, 3-22=-1013/321, 3-4=-959/359, 4-5=-756/327, 5-6=-1066/368, 6-7=-1234/310, TOP CHORD

7-8=-886/206, 8-9=-941/198, 9-23=-77/597, 10-23=-89/590, 10-11=-55/179, 11-12=0/37

1-21=-182/1117, 20-21=-182/1117, 19-20=0/783, 18-19=0/783, 17-18=-15/886, 16-17=-63/873, 15-16=-63/873, **BOT CHORD**

14-15=-529/194, 13-14=-155/88, 11-13=-155/88

2-21=0/313, 2-20=-451/310, 4-20=-133/438, 4-18=-216/174, 5-18=-152/400, 6-18=-291/176, 6-17=-45/164, 7-17=-19/127, **WEBS**

7-15=-740/180, 9-15=-257/1604, 9-14=-1550/314, 10-14=-434/159, 10-13=0/160

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

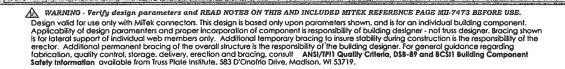
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

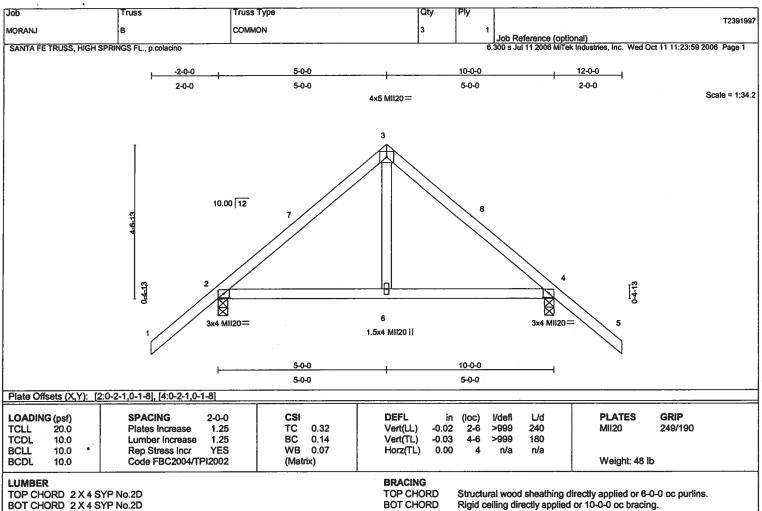
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 1, 318 lb uplift at joint 14 and 283 lb uplift at joint 11.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634







WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=517/0-3-8, 4=517/0-3-8

Max Horz 2=-155(load case 3)

Max Uplift2=-251(load case 5), 4=-251(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/77, 2-7=-396/78, 3-7=-302/95, 3-8=-302/95, 4-8=-396/77, 4-5=0/77

BOT CHORD 2-6=0/228, 4-6=0/228

WEBS 3-6=0/217

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT.7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding tobication, quality control, storage, delivery, erection and bracing, consult AMSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Truss Type Qtv Job Truss T2391998 GABLE MORANJ DET Job Reference (optional) 6.300 s Jul 11 2006 MITek industries, Inc. Wed Oct 11 11:24:02 2008 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL., p. colacino 42-0-0 40-0-0 -2-0-0 13-11-1 18-0-15 26-9-12 13-2-4 2-0-0 2-0-0 13-11-1 4-1-15 8-8-12 Scale = 1:84.1 6x6 MII20 // 6x6 MII20 > 10.00 123x6 MII20 / 11 12 13 10 9 15 16 3x6 MII20> 4.00 12 6 17 7x8 MII20 | 1819 20 3x4 MII20~ 21 3x4 MII20~ 23 26 5x8 MI120 II 39 37 36 35 33 32 31 30 29 28 27 43 42 41 40 38 44 3x5 MII20 = 5x6 MH20= 5x5 MII20= 5x5 MII20= 40-0-0 40-0-0 Plate Offsets (X,Y): [2:0-3-0,0-2-15], [11:0-2-4,Edge], [13:Edge,0-2-11], [18:0-3-0,0-1-5], [25:0-4-12,Edge], [25:0-3-8,Edge], [33:0-2-8,0-3-0], [37:0-2-8,0-3-0] GRIP DEFL 1 /d **PLATES** LOADING (psf) SPACING CSI in (loc) I/defl 2-0-0 249/190 MII20 TC 0.29 Vert(LL) -0.0226 n/r 120 TCLL 20.0 Plates Increase 1.25 ВС 0.07 Vert(TL) -0.0326 n/r 120 TCDL 10.0 Lumber Increase 1.25 YES WB 0.14 Horz(TL) 0.01 25 n/a n/a Rep Stress Incr **BCLL** 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 300 lb **BCDI** 10.0 **BRACING** LUMBER **TOP CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD 2 X 4 SYP No.2D Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.2D BOT CHORD 14-35, 13-36, 12-37, 11-38, 9-39 WEBS 1 Row at midpt 2 X 4 SYP No.3 **OTHERS** REACTIONS (lb/size) 25=301/40-0-0, 2=265/40-0-0, 35=156/40-0-0, 36=147/40-0-0, 37=165/40-0-0, 38=147/40-0-0, 39=156/40-0-0. 40=161/40-0-0, 41=160/40-0-0, 42=158/40-0-0, 43=171/40-0-0, 44=100/40-0-0, 34=161/40-0-0, 33=159/40-0-0, 32=166/40-0-0, 31=164/40-0-0, 30=158/40-0-0, 29=165/40-0-0, 28=140/40-0-0, 27=241/40-0-0 Max Horz 2=-408(load case 3) Max Uplift25=-227(load case 4), 2=-185(load case 3), 35=-54(load case 5), 37=-59(load case 4), 39=-55(load case 5), 40=-75(load case 6), 41=-92(load case 5), 42=-117(load case 5), 43=-124(load case 5), 44=-24(load case 6), 34=-75(load case 6), 33=-89(load case 5), 32=-66(load case 6), 31=-105(load case 4), 30=-76(load case 4), 29=-79(load case 4), 28=-90(load case 4), 27=-72(load case 4) Max Grav 25=301 (load case 1), 2=265 (load case 1), 35=156 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 38=147(load case 9), 39=156(load case 1), 40=161(load case 1), 41=160(load case 1), 42=158(load case 9). 43=171 (load case 1), 44=108 (load case 2), 34=161 (load case 1), 33=159 (load case 10), 32=168 (load case 1), 31=164(load case 1), 30=158(load case 10), 29=165(load case 1), 28=140(load case 10), 27=241(load case 1) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/75, 2-3=-237/296, 3-4=-199/273, 4-5=-189/281, 5-6=-140/287, 6-7=-72/276, 7-8=-59/268, 8-9=-60/262, 9-10=-57/276, 10-11=-5/284, 11-12=-16/246, 12-13=-16/246, 13-14=-58/284, 14-15=-60/225, 15-16=-59/192, 16-17=-60/207, TOP CHORD 17-18=-44/167, 18-19=-46/167, 19-20=-69/126, 20-21=-96/90, 21-22=-122/51, 22-23=-147/13, 23-24=-169/19, 24-25=-177/8 25-26=0/36 **BOT CHORD** 2-44=0/209, 43-44=0/208, 42-43=0/208, 41-42=0/208, 40-41=0/208, 39-40=0/208, 38-39=0/208, 37-38=0/207, 38-37=0/207, 35-36=0/207, 34-35=0/207, 33-34=0/207, 32-33=0/207, 31-32=0/207, 30-31=0/207, 29-30=0/207, 28-29=0/207, 27-28=0/207 25-27=0/207 14-35=-116/74, 13-36=-109/0, 12-37=-129/79, 11-38=-108/0, 9-39=-116/74, 8-40=-121/95, 7-41=-120/111, 6-42=-119/139, **WEBS** 5-43=-126/136, 3-44=-77/53, 15-34=-121/95, 16-33=-119/89, 17-32=-126/86, 19-31=-124/125, 20-30=-119/95, 21-29=-122/102, 22-28=-111/101, 23-27=-174/113 **NOTES** 1) Unbalanced roof live loads have been considered for this design. Guo-Jie Zhang, FL Lic #47744 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33. MiTek Industries, Inc. 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable 1801 Massaro Blvd Tampa FL 33619 End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002. FL Cert.#6634 4) Provide adequate drainage to prevent water ponding.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not frus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult AMS/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) All plates are 1.5x4 MII20 unless otherwise indicated.



	Job	Truss	Truss Type	Qty Pl	у		T2391998
i	MORANJ	DET	GABLE	2	1		12351360
				11.		Job Reference (optional)	

SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino

6.300 s Jul 11 2006 MiTek Industries, Inc. Wed Oct 11 11:24:02 2006 Page 2

NOTES

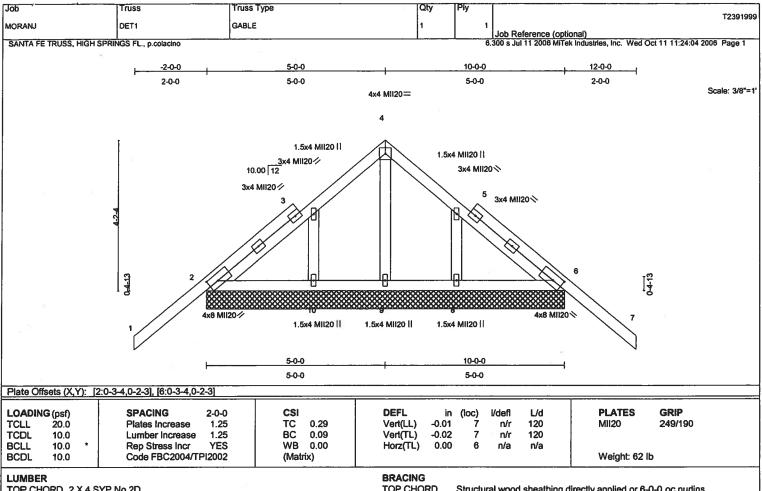
- 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 8) Gable requires continuous bottom chord bearing.

9) Gable studs spaced at 2-0-0 oc.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 25, 185 lb uplift at joint 2, 54 lb uplift at joint 35, 59 lb uplift at joint 37, 55 lb uplift at joint 39, 75 lb uplift at joint 40, 92 lb uplift at joint 41, 117 lb uplift at joint 42, 124 lb uplift at joint 43, 24 lb uplift at joint 44, 75 lb uplift at joint 34, 69 lb uplift at joint 33, 66 lb uplift at joint 32, 105 lb uplift at joint 31, 76 lb uplift at joint 30, 79 lb uplift at joint 29, 90 lb uplift at joint 28 and 72 lb uplift at joint 27.

LOAD CASE(S) Standard





TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 **OTHERS**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=434/10-0-0, 6=434/10-0-0, 9=13/10-0-0, 10=79/10-0-0, 8=79/10-0-0

Max Horz 2=-142(load case 3)

Max Uplift2=-275(load case 5), 6=-275(load case 6)

Max Grav 2=434(load case 1), 6=434(load case 1), 9=54(load case 2), 10=120(load case 2), 8=120(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/75, 2-3=-328/126, 3-4=-233/143, 4-5=-233/143, 5-6=-328/125, 6-7=0/75

2-10=-15/189, 9-10=-15/189, 8-9=-15/189, 6-8=-15/189 BOT CHORD

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 275 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Ty	ne .		QI	y [Ply				
MORANJ	F1	FLOOR	p -0		13		1			T2392000
	SH SPRINGS FL., p.colacino			-			Job R 6.300 s Ju	eference (opti il 11 2006 MiTe	onal) k Industries, Inc. Wed (Oct 11 11:24:06 2008 Page 1
0-3-8										
Н Н	1-3-0		0-10-8		2-0-0	1 0-9-8	-			0-3-8 Scale = 1:21.8
1.5x4 MII20 I			1.5x4 MII	20 II	1.3	5x4 MII20				1.5x4 MII20
1	2	3	4			5	6		7	8
200										
4x12 MII20		13	12			11	·	10		9 4x12 MII20=
	5-6-0			6-6-0	7-6-0			12	2-11-0	
	5-6-0			1-0-0	1-0-0	·			5-5-0	
Plate Offsets (X,Y):	[1:0-2-0,0-1-0], [8:0-2-0,	0-1-0]						 	1	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING Plates Increase Lumber Increas Rep Stress Incr Code FBC2004	YES	CSI TC 0.34 BC 0.52 WB 0.19 (Matrix)		DEFL Vert(LL) Vert(TL) Horz(TL)	in (loc) -0.03 12 -0.05 11-12 0.02 9	l/defl >999 >999 n/a	L/d 480 240 n/a	PLATES MII20 Weight: 72	GRIP 249/190
LUMBER TOP CHORD 2 X 4 BOT CHORD 2 X 4 WEBS 2 X 4		-			BRACING TOP CHOR	end ve	erticals.	_	firectly applied or 5	-1-15 oc purlins, except

REACTIONS (lb/size) 14=678/0-8-0, 9=678/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-14=-74/0, 8-9=-74/0, 1-2=-28/0, 2-3=-826/0, 3-4=-1189/0, 4-5=-1189/0, 5-6=-1189/0, 6-7=-826/0, 7-8=-29/0

13-14=0/552, 12-13=0/1088, 11-12=0/1189, 10-11=0/1090, 9-10=0/552

4-12=-249/0, 5-11=-264/0, 2-14=-782/0, 2-13=0/464, 3-13=-444/0, 3-12=0/376, 7-9=-781/0, 7-10=0/463, 6-10=-447/0, WEBS

6-11=0/390

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MII20 unless otherwise indicated.

3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

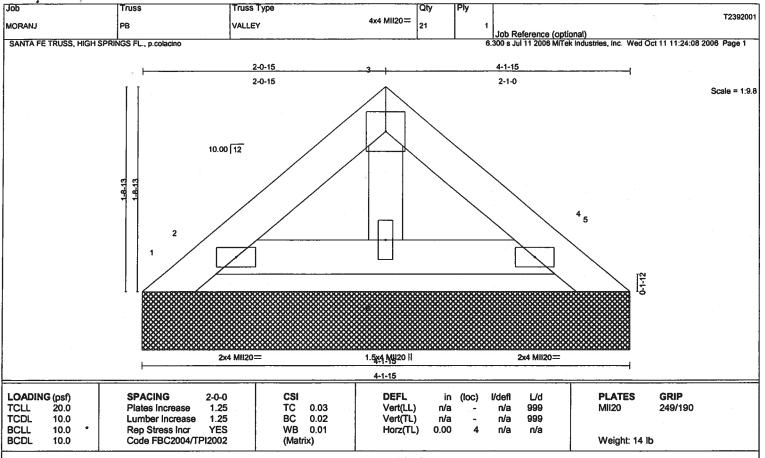
LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

October 11,2006

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LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-1-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-15/4-1-15, 5=-15/4-1-15, 2=122/4-1-15, 4=122/4-1-15, 6=87/4-1-15

Max Horz 1=54(load case 4)

Max Uplift1=-42(load case 3), 5=-15(load case 1), 2=-78(load case 5), 4=-78(load case 5)

Max Grav 1=56(load case 4), 5=27(load case 5), 2=122(load case 1), 4=122(load case 1), 6=89(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-76/73, 2-3=-48/27, 3-4=-48/27, 4-5=-15/29

BOT CHORD 2-6=-11/31, 4-6=-11/31

WEBS 3-6=-50/11

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS automatic zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Gable requires continuous bottom chord bearing.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 15 lb uplift at joint 5, 78 lb uplift at joint 2 and 78 lb uplift at joint 4.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4, 6.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in. SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#8634

October 11,2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incroporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, reaction and bracing, consult ANSI/P11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL	REQUIREME	NTS: Two (2) complete sets of plans containing the following:
Applicant	Plans Examine	r
	o o	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans. Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
b' /	D ,	 Site Plan including: a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
ts .	0	 d) Provide a full legal description of property. Wind-load Engineering Summary, calculations and any details required Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design
e/	0 0 0	professional. Elevations including: a) All sides b) Roof pitch c) Overhang dimensions and detail with attic ventilation

. *		
	0	d) Location, size and height above roof of chimneys.
	0	e) Location and size of skylights
	0	f) Building height
B	0	e) Number of stories
		Floor Plan including:
d /		a) Rooms labeled and dimensioned.
B/		b) Shear walls identified.
9		c) Show product approval specification as required by Fla. Statute 553.842 and
/		Fla. Administrative Code 9B-72 (see attach forms).
B/	<u> </u>	d) Show safety glazing of glass, where required by code.
8/	0	e) Identify egress windows in bedrooms, and size.
M	<u>_</u>	f) Fireplace (gas vented), (gas non-vented) or wood burning with
	U	hearth, (Please circle applicable type).
rd.	0	g) Stairs with dimensions (width, tread and riser) and details of guardrails and
_/	J	handrails.
	0	
	ь	h) Must show and identify accessibility requirements (accessible bathroom) Foundation Plan including:
57	O.	2) I continue of all lead bearing mall with required Continue to the state of the s
/	U	 a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
rol	0	
3,		b) All posts and/or column footing including size and reinforcing
3/		c) Any special support required by soil analysis such as piling
9/	0	d) Location of any vertical steel.
n /	-	Roof System;
BU	0	a) Truss package including:
		1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
		2. Roof assembly (FBC 106.1.1.2)Roofing system, materials,
/		manufacturer, fastening requirements and product evaluation with wind resistance rating)
	0	b) Conventional Framing Layout including:
Car C	u	1. Rafter size, species and spacing
		2. Attachment to wall and uplift
		3. Ridge beam sized and valley framing and support details
		4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials,
		manufacturer, fastening requirements and product evaluation with
7		wind resistance rating)
		Wall Sections including:
8	0	a) Masonry wali
	u	1. All materials making up wall
		2. Block size and mortar type with size and spacing of reinforcement
		3. Lintel, tie-beam sizes and reinforcement
		4. Gable ends with rake beams showing reinforcement or gable truss
		and wall bracing details
		5. All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation shall be
		designed by a Windload engineer using the engineered roof truss
		plans.
		6. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening

- requirements and product evaluation with resistance rating)
- 7. Fire resistant construction (if required)
- 8. Fireproofing requirements
 9. Shoe type of termite treatment (termiticide or alternative method)
- 10. Slab on grade
 - a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
- 11. Indicate where pressure treated wood will be placed
- 12. Provide insulation R value for the following:

1		
đ		b) Wood frame wall
		1. All materials making up wall
		2. Size and species of studs
		3. Sheathing size, type and nailing schedule
		4. Headers sized
		5. Gable end showing balloon framing detail or gable truss and wall
		ninge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with wind resistance rating)
		8. Fire resistant construction (if applicable)
		9. Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)11. Slab on grade
		a. Vapor retarder (6Mil. Polyethylene with joints lapped 6
		inches and sealed
		b. Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports 12. Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following:
		a. Attic space
1		b. Exterior wall cavity
		c. Crawl space (if applicable)
M	0	c) Metal frame wall and roof (designed, signed and sealed by Florida Prof
1		Engineer or Architect)
1		Floor Framing System:
Μ,	D	a) Floor truss package including layout and details, signed and sealed by Florida
KJ,	0	Registered Professional Engineer
	0	b) Floor joist size and spacing
3/	0	c) Girder size and spacing
B.		d) Attachment of joist to girder
N.		e) Wind load requirements where applicable
	- u	Plumbing Fixture layout Electrical layout including:
B		a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
₫.	ō	b) Ceiling fans
В.	D	c) Smoke detectors
e,	0	d) Service panel and sub-panel size and location(s)
8,		e) Meter location with type of service entrance (overhead or underground)
8		f) Appliances and HVAC equipment
P REE REE REE		g) Arc Fault Circuits (AFCI) in bedrooms
	<u></u>	h) Exhaust fans in bathroom
1		HVAC information
G/		a) Energy Calculations (dimensions shall match plans)
8/		b) Manual J sizing equipment or equivalent computation
B ′,		c) Gas System Type (LP or Natural) Location and BTU demand of equipment
E./		Disclosure Statement for Owner Builders
2 C C C C C C C C C C C C C C C C C C C	0	*** Notice Of Commencement Required Before Any Inspections Will Be Done
B	0	Private Potable Water

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

WINDOW & DOOR COMPANY

PHONE: (866) 337-6879 FAX: (740) 375-0642

CUSTOMER SERVICE

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Florida Approval	FL. 3	884.	381	
Miami-Dade NOA #_				
Mulled: Yes or NO	Mullion	Serie	3 :	

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R W Building Consultants, Inc.

Consulting and Engineering Services for the Building Industry
P.O. Box 230 Valrico, FL 33595 Phone 813.659.9197 Facsimile 813.754.9989

Florida Board of Professional Engineers Certificate of Authorization No. 9813

Product Evaluation Report

Report No.:

FL 3861.3 R1

Date:

July 27, 2005

Product Category:

Windows

Product sub-category:

Fixed

Product Name:

2300 Series - Model 2350 Extruded Vinyl Fixed Window

Manufacturer:

Silverline Building Products Corporation

One Silverline Drive

North Brunswick, NJ 08902

Phone - 732.435.1000 Facsimile - 732.247.6820

Scope:

This is a Product Evaluation report issued by R W Building Consultants, Inc. and Wendell W. Haney, P.E. (System ID # 1993) for Silverline Building Products Corporation based on Rule Chapter No. 9B-72.070, Method 1d of the State of Florida Product Approval, Department of Community Affairs-Florida Building Commission.

RW Building Consultants and Wendell W. Haney, P.E. do not have nor will acquire financial interest in the company manufacturing or distributing the product or in any other entity involved in the approval process of the product named herein.

This product has been evaluated for use in locations adhering to the Florida Building Code (2004 Edition) and where pressure requirements, as determined by Chapter 16 of The Florida Building Code, do not exceed the following design pressures:

Design Pressure Rating:

Maximum Design Pressure Rating (See Limitations for size restrictions)

Positive 40.0 PSF

Negative 50.0 PSF

See Drawing No.: FL 403 prepared by R W Building Consultants, Inc. and signed and sealed by Wendell W. Haney, P.E. (FL # 54158) for specific use parameters.

Wendell W Haney, P.P.

FL No. 54158 July 27, 2005

Limitations

- The 2300 Series Model 2350 Extruded Vinyl Fixed Window has been evaluated and meets the requirements for use within the State of Florida including the "High Velocity Hurricane Zone".
- 2. When used in the "High Velocity Hurricane Zone" this product is required to be covered with an external protection device that complies with Section 1626 of the Florida Building Code.
- 3. When used in areas requiring windborne debris protection outside of the "High Velocity Hurricane Zone" then this product is required to be covered with an external protection device that complies with Section 1609.1.4 of the Florida Building Code.
- 4. Size Limitations:

Configurations

0

MAX. Width

MAX, Height

Single

74.5"

75.5"

5. The Design Pressure Rating for the various size units are as follows:

Overali Fin	Overall Frame Dimensions	Overall Daylight Opening Dimensions	Glass Type	Design Pressure Testing	
Dimensions				Positive	Negative
74-1/2" x 75-1/2"	72" x 73"	68" x 69"	1/8" Tempered Air 1/8" Tempered	. +40.0	-50.0
74-1/2" x 75-1/2"	72" x 73"	68" x 69"	5/32" Annealed Air 5/32" Annealed	+30.0	-30.0
74-1/2" x 75-1/2"	72" x 73"	68" x 69"	1/8" Annealed Air 1/8" Annealed	+20.0	-20.0
54-1/2" x 64-1/2"	52" x 62"	48" x 58"	1/8" Annealed Air -1/8" Annealed	+35.0	-35.0

FL No. 54158 July 27, 2005

Supporting Documents

A Drawing

 Drawing No. FL 403 titled Series 2300 Model 2350 Extruded Vinyl Fixed Window prepared by R W Building Consultants, Inc. (Florida Board of Professional Engineers Certificate of Authorization No. 9813) signed and sealed by Wendell W. Haney, P.E.

B Tests Performed

- Testing per TAS 202-94 as performed by Architectural Testing, Inc. and reported in test report number 01-45598.01, dated October 17, 2003, and signed by Steven M. Urich, P.E.
- Plastics testing in accordance with the "High Velocity Hurricane Zone" substantiated by issuance of Miami-Dade Notice of Acceptance 02-0523.01, expiring August 15, 2007.

C Calculations

- 1. Frame anchoring per manufacturer's published installation instructions and as substantiated by test report number ATI 01-45598.01.
- Buck anchor analysis for loading conditions, prepared, signed and sealed by Wendell W. Haney, P.E.
- 3. Glass Load Resistance Report ASTM E1300-02 prepared by Wendell W. Haney.

D Other

Certificate of Participation issued by National Accreditation & Management Institute, Inc., certifying that Silverline Building Products, North Brunswick, NJ is manufacturing products within a quality assurance program that complies with ISO/IEC 17020 and Guide 53. ID # S-019-1.

Wendell W. Haney, R.E.

FL No. 54158 July 27, 2005

FL3861.3 RI

SilverLINE BUILDING PRODUCTS CORP.

1 SILVERLINE DRIVE ACHTH GRUNSWICK, NJ 08902 PH. 732435, 1000

2300 SERIES, MODEL 2350 EXTRUDED VINYL **FIXED WINDOW**

CENERAL MOTES

- 1. THIS FROBUCT HIS BEEN EXALIATED AND IS IN COMPLIANCE WITH THE FLORIDA EVICIDING CODE INCLUDING THE "MICH VICH VELOCITY HURRICANE ZONE".
- PRODUCT ANCHORS SHALL BE AS LISTED AND SPADED AS SHOWN ON DETAILS, ANCHOR EMBEDAINT TO RISE AKTERKI, SHALL BE BEYOND WALL DRESSING OR STUCCO.
- WHEN LISED IN THE "HIGH VELOCITY HURRICAME ZONE" THIS PRODUCT IS REQUIRED TO BE PROTECTED WITH AN IMPACT RESISTANT COVERNO THAT COMPLES WITH SECTION 1628 OF THE PLORICA BUILDING CODE.
- WHEN USED IN AFEAS QUISIDE OF THE THAY!" REQUIRING WAND—BORNE DEBRIS PROTECTION THAS PRODUCT IS REQUIRED TO BE PROTECTED WITH AN WIPACT RESISTANT CONFRING BUILDING COME.

CONTRACT CONTRACT CONTRACTOR
THE COMPANY CLACING OF INT. B. COMPANYENTS
4 BILL OF MATERIALS, CLAZING DETAIL & COMPONENTS
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2 VERTICAL & MONZONITAL CROSS SECTIONS 3 BLICK & NUNCOW ANCHORING 4 BILL OF MATERIALS, CLAZING DETAIL & COMPONENTS
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2 VERTICAL & POINTAL CROSS SECTIONS 3 BLICK & RENCOW ANGROPHING 4 BILL OF MATERIALS, GLAZING DETAIL & COMPONENTS
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1 TYPICAL BLEWIDDAS, DESIGN PRESSURES & GENERAL AUSTES 2 VERTICAL & HORIZONITA, CASOS SECTIONS 3 BLICK & HORIZONIA, CAROSS SECTIONS 4 BILL & HATERMAS, GLAZING DETAI, & COMPONENTS
1 TYPICAL BLEMTONS, DESIGN PRESSURES & CÉNERAL MOTES 2 VERTICAL & MORZONTAL CROSS SECTIONS 3 BLICK & NUMBER MORZONTAL CROSS SECTIONS 4 BILL OF MATERIALS, CLAZING DETAIL & COMPONENTS
1 TYPICAL BLEWIDGE OF DESIGN PRESSURES & CENERAL MOTES 2 VERTICAL & HORZONTAL CROSS SECTIONS 3 BLICK & RENCOW ANCHORING 4 BILL OF MATERIALS, GLAZING DETAIL & COMPONENTS
SHEET # DESCRIPTION 1 Trylical Elemidius, design pressures # general notes 2 Vertical # horizontal notes 3 Bluck # necoral and pressure of Bluck # general notes 4 Bluck # Necoral notes Gluck Gluck
4
4

74.5" KAX. WOTH	72" MAK, OVERALL FRAME WIDTH	68° мл.		"	7
			MAN, HEIGHT N. OVERALL E. HEIGHT F. MEIGHT HEIGHT	NA "L7 NA "L7 MART 69	

BRESSIBES & CENTEN HOLES
LIMING STERNION' BERICH
EVILUED HAVE ENTEL BRANDON
2500 SENES HODEL 5320
2500 SENES HODEL 5320
BRODONEL
BRODONEL

CVERNIL	OVERALL	CACRACL	CLASS	DESIGN PRESSUR	RESSURE
DUMENSION	DIMENSION	DINENSION	INPE	POSITIVE	NEGATIVE
74.5° r 75.5°	72" x 73"	.69 x 69	1/6" TEMPERED AIR 1/8" TEMPERED	+40.0 PSF	-50.0 PSC
74.5° x 75.5°	72" x 75"	58° x 69°	5/32" ANNEALED 5/32" ANNEALED	+30.0 PSF	-30.0 PSF
74.5° × 75.5°	72" x 73"	.89 x 68"	1/8" ANNEALED AIR ANNEALED	+20.0 PSF	-20.0 PSF
54.5" x 64.5"	22. # 62.	48" × 58"	1/8" ANVEALED AMP 1/8" ANVEALED	+35.0 PSF	-35.6 PSF

DOTE 12/29/D¢

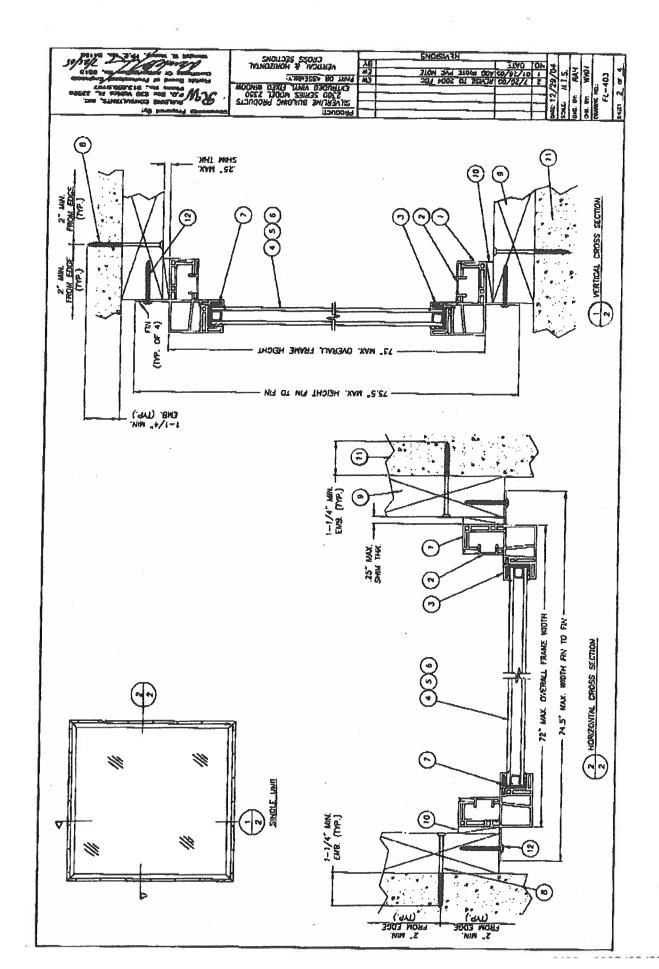
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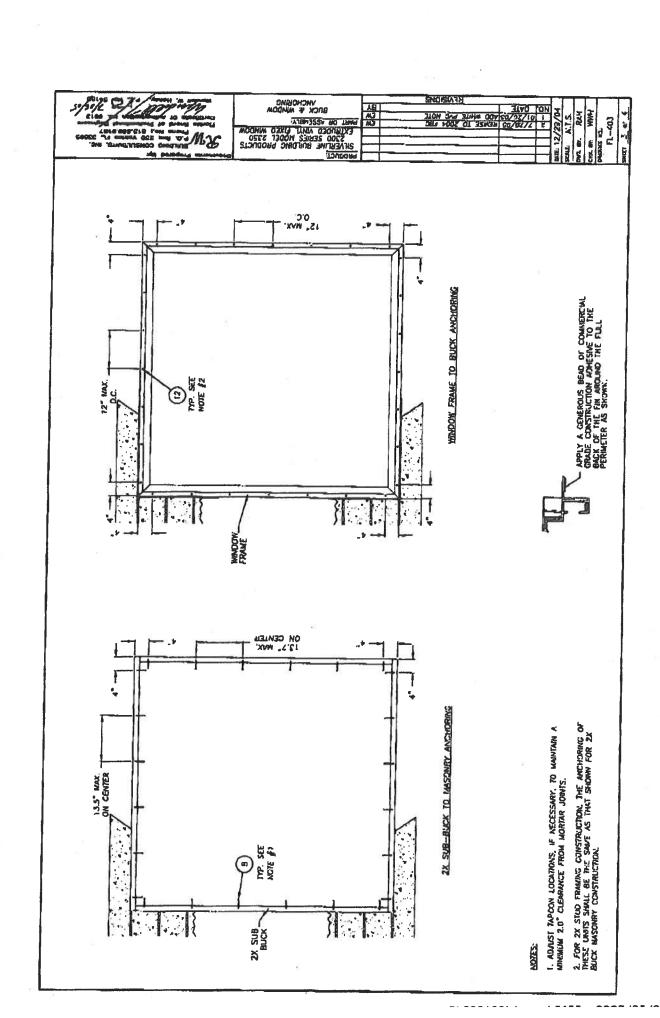
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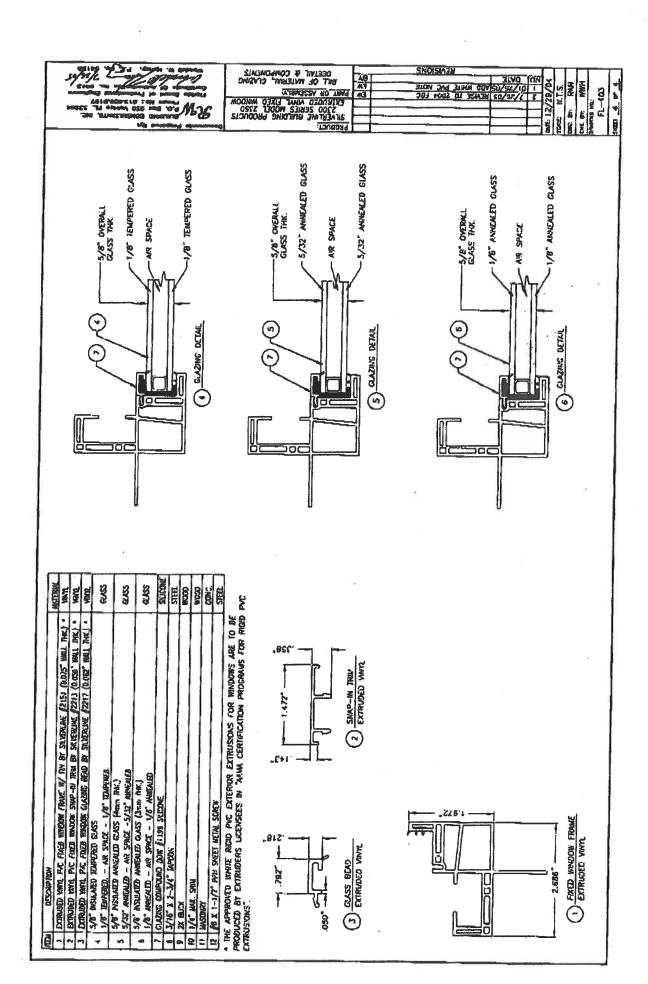
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FL-693







WINDOW & DOOR COMPANY

(866) 337-6879 FAX: (740) 375-0642

CUSTOMER SERVICE

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	Pages:	11	
Sont For Review Please	• Comment	□ Please Raply	Please Recycle
Florida Bui	lding (Codes	3 2 2
Series/Model #_2110	Fin/F	lange	
Florida Approval # [ello3.	3	
Miami-Dade NOA #			
Mulled: Yes or NO Mullio	n Serie	s:	

RWBC

R W Building Consultants, Inc.

Consulting and Engineering Services for the Building Industry

P.O. Box 230 Valrico, FL 33595 Phone 813.659.9197 Facsimile 813.754.9989

Plorida Board of Professional Engineers Certificate of Authorization No. 9813

Product Evaluation Report

Report No.:

FL 6163.3

Date:

February 21, 2006

Product Category:

Windows

Product sub-category:

Single Hung

Product Name:

Model Series 2100

Extruded White Single & Twin Extruded PVC Single Hung Windows

Manufacturer:

Silverline Building Products Corporation

One Silverline Drive North Brunswick, NJ 08902

Phone - 732.435.1000 Facsimile - 732.247.6820

Scope

This is a Product Evaluation report issued by R W Building Consultants, Inc. and Wendell W. Haney, P.E. (System ID # 1993) for Silverline Building Products based on Rule Chapter No. 9B-72.070, Method 1d of the State of Florida Product Approval, Department of Community Affairs-Florida Building Commission.

RW Building Consultants and Wendell W. Haney, P.E. do not have nor will acquire financial interest in the company manufacturing or distributing the product or in any other entity involved in the approval process of the product named herein.

This product has been evaluated for use in locations adhering to the Florida Building Code (2004 Edition) and where pressure requirements, as determined by Chapter 16 of The Florida Building Code, do not exceed the following design pressures:

Design Pressure Rating:

Maximum Design Pressure Rating (See Limitations for size restrictions)

Positive 50.0 PSF

Negative 50.0 PSF

See Drawing No.: S-2375 prepared by R W Building Consultants, Inc. and signed and sealed by Wendell W. Haney, P.E. (FL # 54158) for specific use parameters.

endell W. Haney, P.E. PL No. 54158

February 21, 2006

FL 6163.3.doc FF 907

Sheet 1 of 3

Limitations

- The Model Series 2100 Extruded White Single & Twin Extruded PVC Single Hung Windows has been evaluated and meets the requirements for use within the State of Florida including the "High Velocity Hurricane Zone".
- When used in areas outside of the "HVHZ" requiring wind-borne debris protection this product is required to be protected with an impact resistant covering that complies with Section 1609.1.4 of the Florida Building Code.
- 3. Size Limitations:

Configurations	MAX. Width	MAX. Height
Single with Flange	53.0"	63.0"
Single with Fin	54.5°	64.5"
Double with Flange	106.0"	62.0"
Double with Fin	107.5"	64.5"

4. See Drawing # S-2375 for Design Pressure Ratings.

Wendell W Haney, P.E.

FL No. 54158 February 21, 2006

Supporting Documents

A Drawing

 Drawing No. S-2375 titled Model Series 2100 Extruded White Single & Twin Extruded PVC Single Hung Windows, prepared by R W Building Consultants, Inc. (Florida Board of Professional Engineers Certificate of Authorization No. 9813) signed and sealed by Wendell W. Haney, P.E.

B Tests Performed

- Testing per TAS 202-94 as performed by Architectural Testing, Inc. and reported in test report number 01-44453.01, dated June 5, 2003, signed and sealed by Joseph A. Reed, P.E.
- 2. Plastics testing in accordance with the "High Velocity Hurricane Zone" substantiated by Issuance of Miami-Dade Notice of Acceptance 03-1110.03, expiring August 15, 2007.

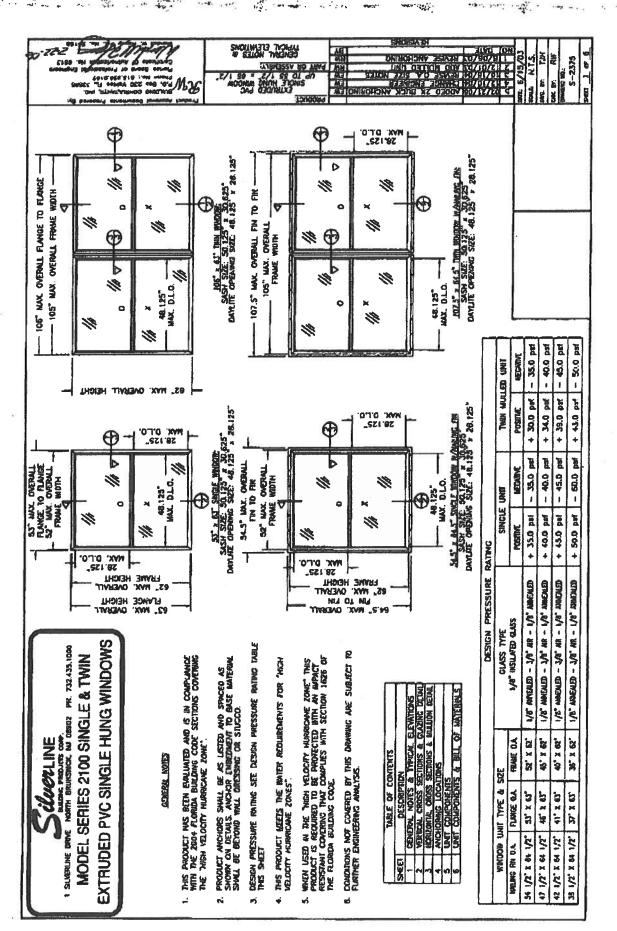
C Calculations

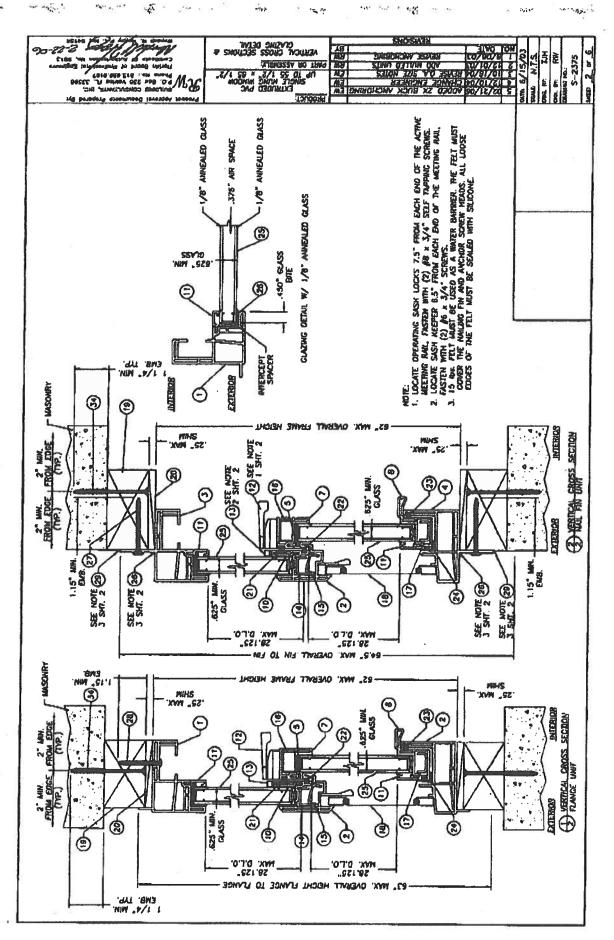
- 1. Product anchoring is in accordance with manufacturer's published recommendations as substantiated by tested specimens reported in test report number 01-44453.01.
- 2. Buck anchor analysis for loading conditions, prepared, signed and sealed by Wendell W. Haney, P.E.
- 3. Glass Load Resistance Report ASTM E1300-02 prepared by Wendell W. Haney, P.E.

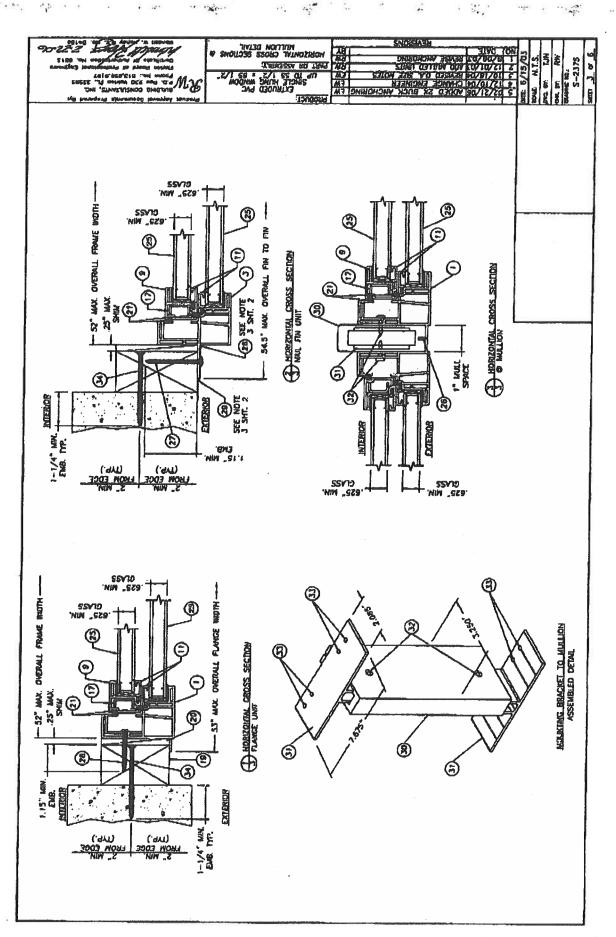
D Other

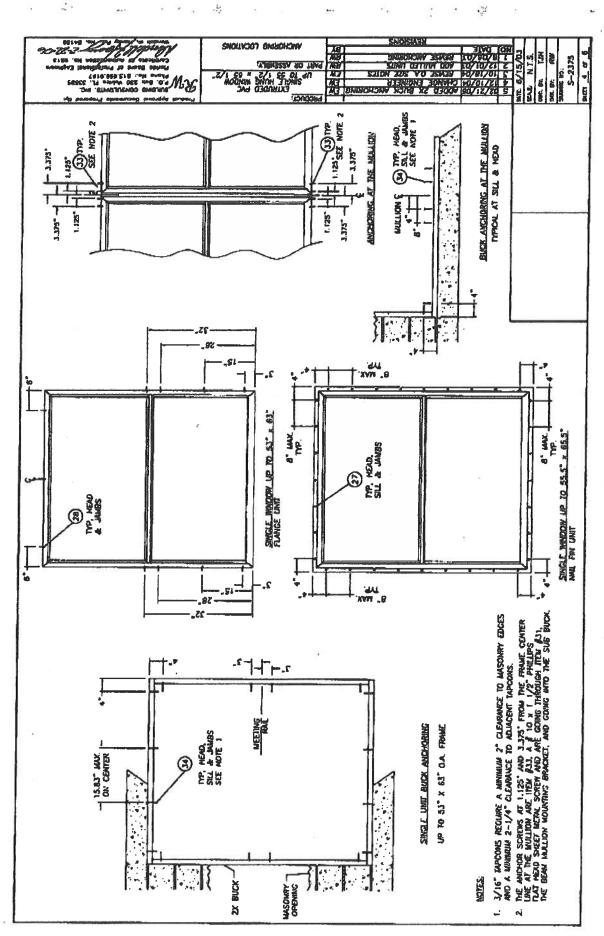
1. Certificate of Participation issued by National Accreditation & Management Institute, Inc., certifying that Silverline Building Products Corporation is manufacturing products within a quality assurance program.

Wendell W. Hanky, P.E. FL No. 54158 February 21, 2006

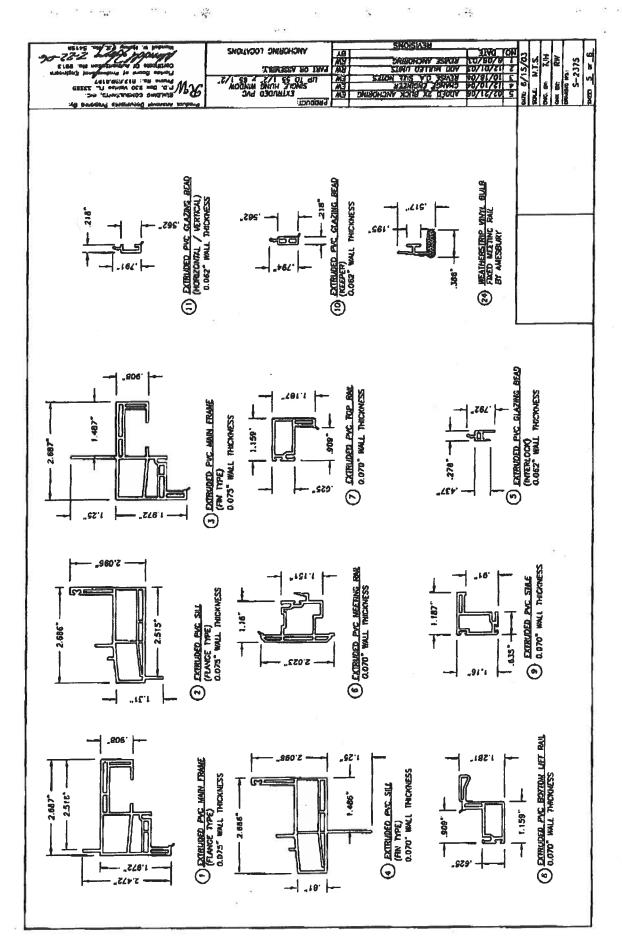




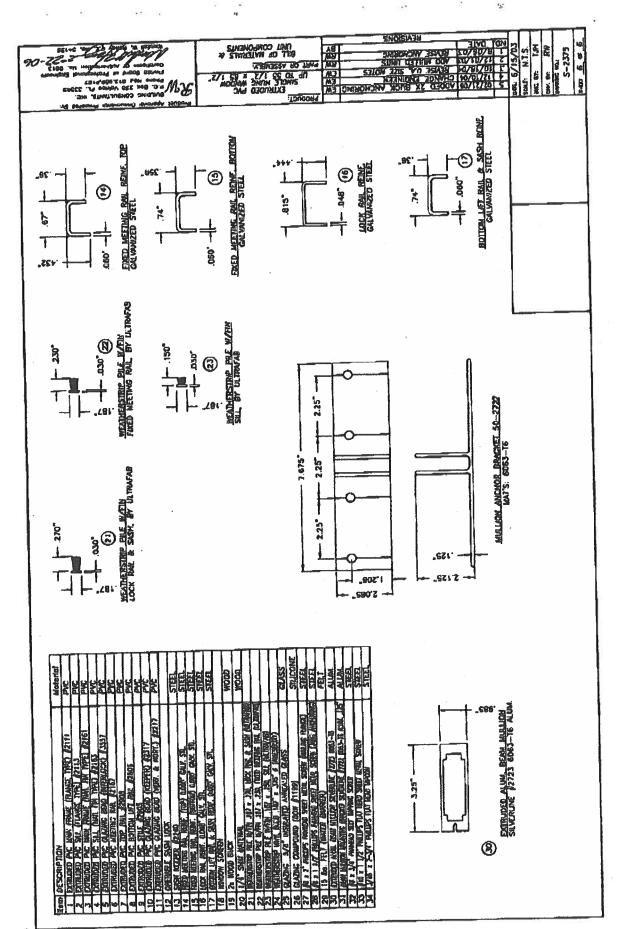




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WINDOW & DOOR COMPANY

CUSTOMER SERVICE

Bonnie Fax: 380 454 4080.
rexandria Date: 10/18/06
Pages:
Please Reply Please Reply Please
Florida Building Codes
Series/Model # 7500 Fin/Flange_
Florida Approval #_ A SAOVO P. [. [.000.]
Miami-Dade NOA #
Mulled: Yes or NO Mullion Series:

R_W_BC

R W Building Consultants, Inc.

Consulting and Engineering Services for the Building Industry
P.O. Box 230 Valrico, FL 33595 Phone 813.659.9197 Facsimile 813.754.9989

Florida Board of Professional Engineers Certificate of Authorization No. 9813

Product Evaluation Report

Report No.:

FL 6000.1

Date:

December 21, 2005

Product Category:

Windows

Product sub-category:

Casement

Product Name:

7500 Series - Model 7500

Extruded Vinyl Casement Window

Single & Twin Units

Manufacturer:

Silverline Building Products Corporation

One Silverline Drive

North Brunswick, NJ 08902

Phone - 732.435.1000 Facsimile - 732.247.6820

Scope:

This is a Product Evaluation report issued by R W Building Consultants, Inc. and Wendell W. Haney, P.E. (System ID # 1993) for Silverline Building Products based on Rule Chapter No. 9B-72.070, Method 1d of the State of Florida Product Approval, Department of Community Affairs-Florida Building Commission.

RW Building Consultants and Wendell W. Haney, P.E. do not have nor will acquire financial interest in the company manufacturing or distributing the product or in any other entity involved in the approval process of the product named herein.

This product has been evaluated for use in locations adhering to the Florida Building Code (2004 Edition) and where pressure requirements, as determined by Chapter 16 of The Florida Building Code, do not exceed the following design pressures:

Design Pressure Rating:

Maximum Design Pressure Rating (See Limitations for size restrictions)

Positive 50.0 PSF

Negative 50.0 PSF

See Drawing No.: FL 707 prepared by R W Building Consultants, Inc. and signed and sealed by Wendell W. Haney, P.E. (FL # 54158) for specific use parameters.

Wendell W. Hancy P.F. FL No. 54158

December 21, 2005

Sheet 1 of 3

FL 6000.1 Eval PF 877

Limitations

- 1. Series 7500 Model 7500 Extruded Vinyl Casement Window, Single & Twin Units has been evaluated and meets the requirements for use within the State of Florida excluding the "High Velocity Hurricane Zone".
- 2. When used in areas requiring wind-borne debris protection this product is required to be protected with an impact resistant covering that complies with Section 1609.1.4 of the Florida Building Code.
- 3. Size Limitations:

Configurations		MAX. Width	MAX. Height
Single	X	32.0"	66.0"
Twin	XX	60.0"	72. 0"

4. The Design Pressure Rating (PSF) for the various size units are as follows:

5.

Overall Frame Size	Design Pressure Rating	
	Positive	Negative
32.0" x 66.0"	+40.0 PSF	-40.0 PSF
30.0" x 66.0"	+45.0 PSF	-45.0 PSF
28.0" x 48.0"	+50.0 PSF	-50.0 PSF

Design Pressure Ratings Twins (PSF)

Overall Frame Size	Design Pre	n Pressure Rating	
The state of the	Positive	Negative	
60.0° x 72.0°	+40.0 PSF	-40.0 PSF	
60.0" x 58.0"	+45.0 PSF	-45.0 PSF	
56.0" x 48.0"	+50.0 PSF	-50.0 PSF	

Wendell W. Haney P.E

FL No. 54158 December 21, 2005

Supporting Documents

A. Drawing

Drawing No. FL 707 titled Series 7500 - Model 7500 Extruded Vinyl Casement Window, 1. Single & Twin Units prepared by R W Building Consultants, Inc. (Florida Board of Professional Engineers Certificate of Authorization No. 9813) signed and scaled by Wendell W. Haney, P.E.

В Tests Performed

- Testing per 101 LS.2 97 as performed by Architectural Testing Inc. and reported in test report number 01-37756.04, dated August 20, 2001, signed by David G. Kranz
- Testing per 101 I.S.2 97 as performed by Architectural Testing Inc. and reported in test 2. report number 01-37756.05, dated August 20, 2001, signed by David G. Kranz
- Plastics testing (Extrusion) in accordance with the "High Velocity Hurricane Zone" 3. substantiated by Issuance of Miami-Dade Notice of Acceptance 03-1110.03, expiring August 15, 2007.

C

- Anchor Analysis for loading conditions, prepared, signed and sealed by Wendell W. Haney, P.E.
- Buck anchor analysis for loading conditions, prepared, signed and sealed by Wendell W. 2. Haney, P.E.
- Glass Load Resistance Report ASTM E1300-02 prepared by Wendell W. Haney, P.E. 3.

D Other

Certificate of Participation issued by National Accreditation & Management Institute, Inc., certifying that Silverline Building Products Corporation is manufacturing products within a quality assurance program.

> Vendell W. Haney P. FL No. 54158 December 21, 2005

FL 6000.1 Eval PF 877

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- COD)

SilverLINE

BULDING PRODUCTS CORP.

1 SILVERLINE DSTVE NORTH BRITINSWICK, NJ 08602 PH. 722,435,1000

EXTRUDED VINYL CASEMENT WINDOW SERIES 7500 - MODEL 7500 SINGLE AND TWIN UNITS

GENERAL MOTES

THIS PRODUCT HAS BEEN EVALUATED AND IS IN COMPLIANCE WITH THE FLORIDA BUILDING CODE EXCLUDING THE "MICH VELOCITY HURRICAME ZONE". ۲.

SERVING WELLIAM STATES W SCHOOLS ON THE STATES ON THE STATES OF THE STAT

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0'7'U 'XW# _CZC'99

MAX, FRAME HEIGHT

0'7'0

64.25" MAX, VENT HEIGHT

24.87" MAX. D.L.O.

28.75 MAX.

26.37 Max.

32.00 MAK. FRAME WIDTH NENT WORM

- 60.00" MAX. FRAME WIDTH --

72.00" MAX FRANE HEIGHT

- PRODUCT ANCHORS SKALL BE AS LISTED AND SPACED AS SHOWN, ON DETARS, ANCHOR ENREDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSANG OR STUDGO. N
- WHEN USED IN AREAS REDUIRING WHID—BORNE DEBRIS
 PROTECTION THIS PRODUCT IS REQUIRED TO BE PROTECTED WITH
 AN IMPACT RESISTANT CONFERING THAT COMPLES WITH SECTION
 1609.1.4 OF THE FLORIDA BUILDING CODE.
 - FOR 2X STUD FRAMBIC CONSTRUCTION, ANCHORING OF THESE UNITS SHALL BE THE SAME AS THAT SHOWN FOR 2X BUCK MASONRY CONSTRUCTION.
- CONDITIONS NOT CONFRED BY THIS DRAWING ARE SUBJECT TO FLATHER ENGINEERING ANALYSIS. vi

DESIGN PRESSURE RATINGS SINGLES (PSF)

SURE RATING	NECABOL	40.0 PSF	-45.0 PSF	-56.0 PSF
DESIGN PRES	POSITINE	+40.0 PSF	+45.0 PSf	+50.0 PSF
CWERALL	FRAME SIZE	32.0" x 66.0"	30.0" = 66.0"	28.0" * 45.0"

DESIGN PRESSURE RATINGS TWINS (PSF)

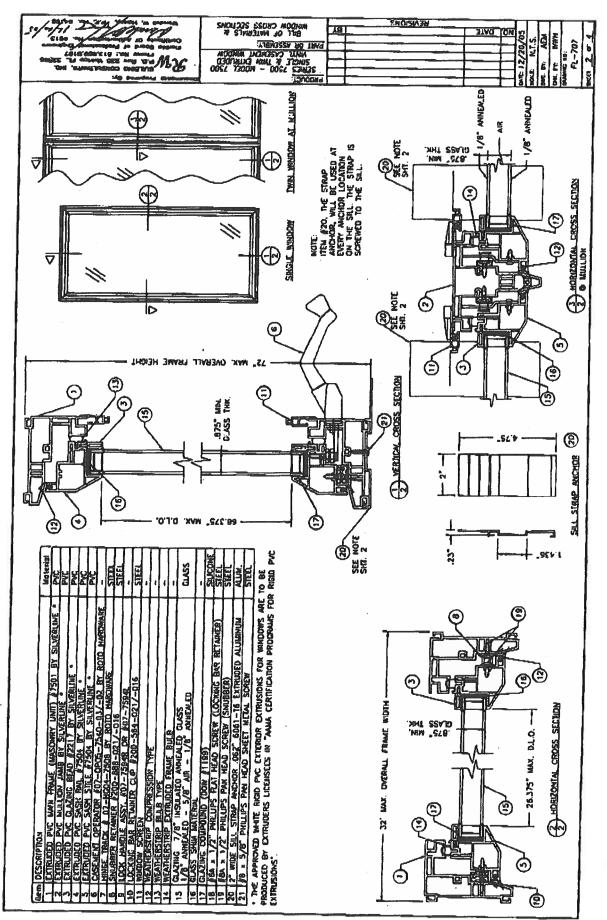
SURE PATING	NEGATIVE	-40.0 PSF	-45.0 PSF	-50.0 PSF
DESIGN PRESSURE	POSITIVE	+40.0 PSF	+45.0 PSF	+50.0 PSF
OVERMEL	FRANE SIZE	60.0° = 72.0°	60.0" x 58.0"	56.0 × 48.0*

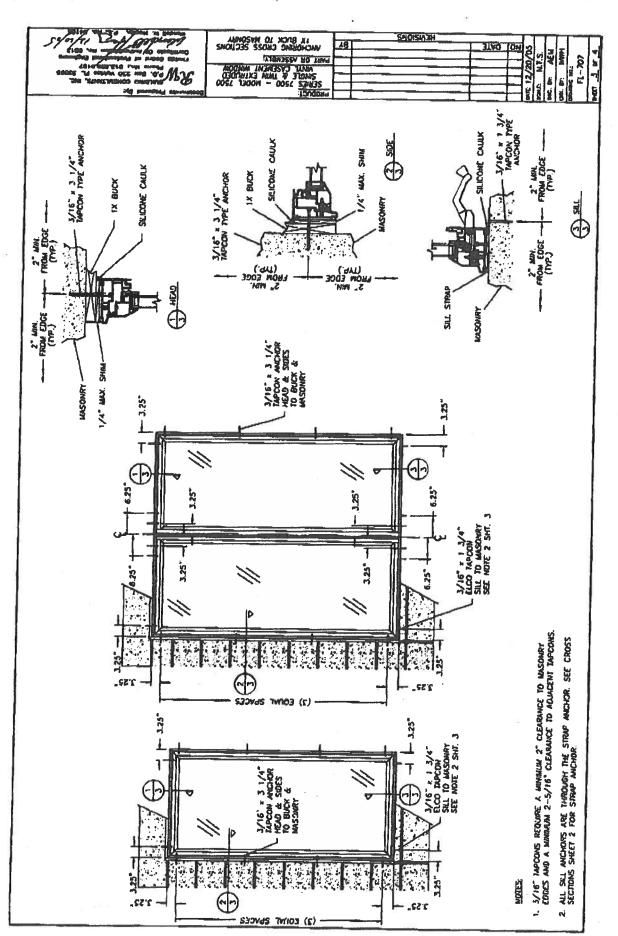
FL-707

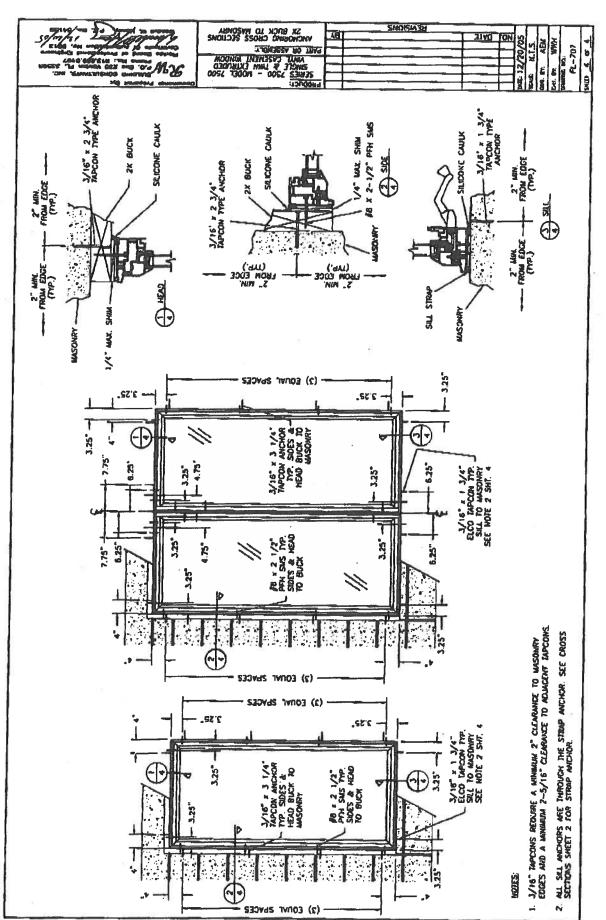
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DESCRIPTION	PRICAL ELEVATIONS, DESIGN PRESSURES & GOVERAL NOTES	ANCHORNE CROSS SECTIONS IN BUCK TO MASONEY	MYCHORANG CAROSS SECTIONS 2X BUCK TO MASDINGY
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TABLE OF CONTENTS	DESCRIPTION	PYPICAL ELLUNDONS, DESIGN PRESSURES & GENERAL NOTES REL OF HITEMALS & INDONE CROSS SECIENTS ANGINERAL CROSS SECTIONS IX BLOX TO MISOMET ANGINE CROSS SECTIONS 2X BUCK TO MISOMET
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	SHEET !	- ~ 7
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THORSESHI OF: 9T 9007/81/01

Residential System Sizing Calculation

Summary Project Title:

Moran Residence 1501 SW CR 778 , FL 32038Project Title: Jeffrey & Bonnie Moran

Code Only Professional Version Climate: North

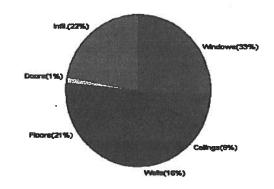
9/14/2006

				0/ 17/2000	
Location for weather data: Gaines	sville - Def	aults: Latiti	ude(29) Altitude(152 ft.) Temp Range	(M)	
Humidity data: Interior RH (50%) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)	10	
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	43209	Btuh	Total cooling load calculation	39604	Btuh
Submitted heating capacity	% of calc		Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	113.4	49000	Sensible (SHR = 0.75)	118.6	36750
Heat Pump + Auxiliary(0.0kW)	113.4	49000	Latent	142.1	12250
			Total (Electric Heat Pump)	123.7	49000

WINTER CALCULATIONS

Winter Heating Load (for 2176 sqft)

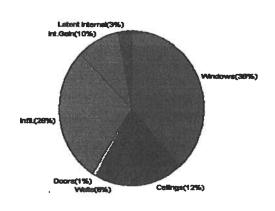
Load component			Load	
Window total	305	sqft	14346	Btuh
Wall total	1415	sqft	6737	Btuh
Door total	40	fipa	518	Btuh
Ceiling total	2800	sqft	3299	Btuh
Floor total	204	sqft	8907	Btuh
Infiltration	232	cfm	9402	Btuh
Duct loss		-	0	Btuh
Subtotal			43209	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS		l	43209	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2176 sqft)

Load component			Load	
Window total	305	sqft	15243	Btuh
Wall total	1415	sqft	3150	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	2800	sqft	× 4637	Btuh
Floor total			0	Btuh
Infiltration	203	cſm	3780	Btuh
Internal gain			3780	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			30982	Btuh
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			7422	Btuh
Latent gain(ventilation)	0	Btuh		
Latent gain(internal/occup	1200	Btuh		
Total latent gain			8622	Btuh
TOTAL HEAT GAIN			39604	Btuh



Control by

For Florida residences only

EnergyGauge® FLRCPB v4.1

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Moran Residence 1501 SW CR 778 , FL 32038Project Title: Jeffrey & Bonnie Moran

Code Only
Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

9/14/2006

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	1, Clear, Metal, 1.27	W	40.0	47.0	1880 Btuh
2	1, Clear, Metal, 1.27	w	9.0	47.0	423 Btuh
3	1, Clear, Metal, 1.27	W	16.0	47.0	752 Btuh
4	1, Clear, Metal, 1.27	N	16.0	47.0	752 Btuh
5	1, Clear, Metal, 1.27	Ň	30.0	47.0	1410 Btuh
⁴⁰ 6	1, Clear, Metal, 1.27	Ē	80.0	47.0	3759 Btuh
7	1, Clear, Metal, 1.27	N	15.0	47.0	705 Btuh
8	1, Clear, Metal, 1.27	Ē	56.0	47.0	2631 Btuh
9	1, Clear, Metal, 1.27	S	13.3	47.0	625 Btuh
10	1, Clear, Metal, 1.27	S	30.0	47.0	1410 Btuh
	Window Total		305(sqft)	*****	14346 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Concrete Blk, - Ext(0.13)	5.0	1415	4.8	6737 Btuh
	Wall Total		1415		6737 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Exterior	51	20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1 .	Vented Attic/D/Shin)	30.0	2800	1.2	3299 Btuh
	Ceiling Total		2800		3299Btuh
Floors	Туре	R-Value	Size X	HTM≔	Load
1	Slab On Grade	0	204.0 ft(p)	43.7	8907 Btuh
	Floor Total		204		8907 Btuh
			Zone Envelope	Subtotal:	33807 Btuh
Infiltration	Туре	ACH X	Zone Volume	CFM=	
	Natural	0.80	17408	232.1	9402 Btuh
Ductioad	Proposed leak free, R6.0, \$	(DLM of 0.00)	0 Btuh		
Zone #1		ototal	43209 Btuh		

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title: Cod

Moran Residence 1501 SW CR 778 , FL 32038Jeffrey & Bonnie Moran

Code Only **Professional Version** Climate: North

PHOLE HOUSE TOTALS		BOOCIANO
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	43209 Btuh 0 Btuh 43209 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - Manual) Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details Project Title: Code C

Moran Residence 1501 SW CR 778 , FL 32038Jeffrey & Bonnie Moran

Professional Version Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

9/14/2006

Gempenent Lo	ads for Zone #1: Main		3 (2) 2		Marian Caranta (Caranta) September 1988 (Caranta)						
Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load						
1	1, Clear, Metal, 1.27	W	40.0	47.0	1880 Btuh						
. 2	1, Clear, Metal, 1.27	W	9.0	47.0	423 Btuh						
3	1, Clear, Metal, 1.27	W	16.0	47.0	752 Btuh						
4	1, Clear, Metal, 1.27	N	16.0	47.0	752 Btuh						
5	1, Clear, Metal, 1.27	N	30.0	47.0	1410 Btuh						
6	1, Clear, Metal, 1.27	E	80.0	47.0	3759 Btuh						
7	1, Clear, Metal, 1.27	N	15.0	47.0	705 Btuh						
8	1, Clear, Metal, 1.27	E	56.0	47.0	2631 Btuh						
9	1, Clear, Metal, 1:27	S	13.3	47.0	625 Btuh						
10	1, Clear, Metal, 1.27	S	30.0	47.0	- 1410 Btuh						
	Window Total	- 24	305(sqft)		14346 Btuh						
Walls	Туре	R-Value	Area X	HTM=	Load						
1	Concrete Blk, - Ext(0.13)	5.0	1415	4.8	6737 Btuh						
	Wall Total		1415		6737 Btuh						
Doors	Туре		Area X	HTM=	Load						
· 1	Insulated - Exterior		20	12.9	259 Btuh						
2	Insulated - Exterior		20	12.9	259 Btuh						
	Door Total		40		518Btuh						
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load						
1	Vented Attic/D/Shin)	30.0	2800	1.2	3299 Btuh						
	Ceiling Total		2800		3299Btuh						
Floors	Туре	R-Value	Size X	HTM≕	Load						
1	Slab On Grade	0	204.0 ft(p)	43.7	8907 Btuh						
	Floor Total		204		8907 Btuh						
			Zone Envelope	Subtotal:	33807 Btuh						
Infiltration	Type Natural	ACH X 0.80	Zone Volume 17408	CFM= 232.1	9402 Btuh						
Ductioad	Proposed leak free, R6.0,	(DLM of 0.00)	9402 Btuh								
Zone #1		Sen	sible Zone Sut	Sensible Zone Subtotal 43209 Btuh							

Manual J Winter Calculations

Residential Load - Component Details (continued)
Project Title: Cod

Moran Residence 1501 SW CR 778 , FL 32038Project Title: Jeffrey & Bonnie Moran Code Only Professional Version Climate: North

MAGLE HOUSE TOTALS		9/14/2006
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	43209 Btuh 0 Btuh 43209 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Moran Residence 1501 SW CR 778 , FL 32038Project Title: Jeffrey & Bonnie Moran

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

9/14/2006

Component Loads for Whole House

	Type* Overhang Window Area(sqft) HTM					ITM	Load				
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded 1	Unshaded	Shaded	Unshaded		
1	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	40.0	0.0	40.0	37	94	3762	Btuh
2	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	9.0	0.0	9.0	37	94	846	Btuh
3	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	16.0	0.0	16.0	37	94	1505	Btuh
4	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	16.0	0.0	16.0	37	37	599	Btuh
5	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	30.0	0.0	30.0	37	37	1124	Btuh
6	1, Clear, 1.27, None,N,N	E	9.5ft	8ft.	80.0	78.6	1.4	37	94	3074	Bluh
7	1, Clear, 1.27, None,N,N	N	19.5f	8fL	15.0	0.0	15.0	37	37	562 2149	Btuh Btuh
8	1, Clear, 1.27, None,N,N	E	9.5ft	8ft.	56.0	55.1	0.9	37 37	94 43	2149 498	Btuh
9	1, Clear, 1.27, None,N,N	S	19.5f	8ft.	13.3 30.0	13.3 30.0	0.0 0.0	37	43	498 1124	
10	1, Clear, 1.27, None,N,N	5	1.5ft	8ft.			0.0	3/	43		
	Window Total		<u></u>		305 (15243	Btun
Walls	Туре		R-Va	lue/U	-Value	Area			HTM	Load	
· 1	Concrete Blk, - Ext		- 10 - 10	5.0/	0.13	141			2.2	3150	Btuh
W.	Wali Total					1415 (sqft)				3150	Btuh
Doors	Туре					Area			HTM	Load	
1	Insulated - Exterior					20	.0		9.8	196	Btuh
2	Insulated - Exterior					20.0			9.8	196	Btuh
	Door Total					4	0 (sqft)		į	392	Btuh
Ceilings	Type/Color/Surface	-	R-Va	lue		Area(sqft)			HTM	Load	
1	Vented Attic/DarkShingle		12	30.0		2800.0			1.7	4637	Btuh
	Ceiling Total					2800 (sqft)				4637	Btuh
Floors	Туре		R-Va	alue		Size			HTM	Load	
1 1	Slab On Grade			0.0		204 (ft(p))			0.0	0	Btuh
•	Floor Total					204.0 (sqft)				0	Btuh
	110011041			5		204.	o (oq.t/				
	2.					. Z e	one Env	elope S	ubtotal:	23422	Btuh
Infiltration	Туре		A	CH		Volum			CFM=	Load	
	SensibleNatural			0.70		174			203.1	3780	Btuh
Internal			Occup	ants		Btuh/oc	cupant		Appliance	Load	
gain				6		X 23	0 +		2400	3780	Btuh
Duct load	Proposed leak free, R6.	0, Sup	ply(At	tic), R	Return(/	Attic)		DGM	= 0.00	0.0	Btuh
							Sensil	ole Zon	e Load	30982	Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Moran Residence 1501 SW CR 778 , FL 32038-

Project Title: Jeffrey & Bonnie Moran **Code Only Professional Version** Climate: North

9/14/2006

E HOUSE TOTALS

	Sensible Envelope Load All Zones	30982	Btuh
	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	30982	Btuh
2	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	, Total sensible gain	30982	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	7422	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
·	Latent other gain	0	Btuh
<u></u>	Latent total gain	8622	Btuh
84	TOTAL GAIN	39604	Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value) (BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Moran Residence 1501 SW CR 778 , FL 32038Jeffrey & Bonnie Moran

Professional Version

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

9/14/2006

Component Loads for Zone #1: Main

	Type* Over			hang	Wind	dow Area	(sqft)	ŀ	πм	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Omt	Len	Hgt	Gross	Shaded 1	Unshaded	Shaded	Unshaded		
1	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	40.0	0.0	40.0	37	94	3762	Btuh
2	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	9.0	0.0	9.0	37	94	846	Btuh
3	1, Clear, 1.27, None,N,N	W	1.5ft	8ft.	16.0	0.0	16.0	37	94	1505	Btuh
4	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	16.0	0.0	16.0	37	37	599	Btuh
5	1, Clear, 1.27, None,N,N	N	1.5ft	8ft.	30.0	0.0	30.0	37	37	1124	
6	1, Clear, 1.27, None,N,N	E	9.5ft	8ft.	80.0	78.6	1.4	37	94	3074	Btuh
7	1, Clear, 1.27, None,N,N	N	19.5f	8ft.	15.0	0.0	15.0	37	37	562	Btuh
8	1, Clear, 1.27, None,N,N	E	9.5ft	8ft.	56.0	55.1	0.9	37	94	2149 498	Btuh Btuh
9 10	1, Clear, 1.27, None,N,N	S	19.5f 1.5ft	8ft.	13.3 30.0	13.3 30.0	0.0 0.0	37 37	43 43	1124	
10	1, Clear, 1.27, None,N,N	3	1.5π	8ft.			0.0	3/	43	1	
	Window Total				305 (4.1550.4	15243	Btun
Walls	Туре		R-Va		I-Value	Area			HTM	Load	
1	Concrete Blk, - Ext		6	5.0/	0.13	141			2.2	3150	
	Wall Total					141	5 (sqft)			3150	Btuh
Doors	Туре					Area			HTM	Load	
1	Insulated - Exterior					20	.0		9.8	196	Btuh
2	Insulated - Exterior					20	.0		9.8	196	Btuh
	Door Total					4	0 (sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area(sqft)			HTM	Load	
1	Vented Attic/DarkShingle			30.0		2800.0			1.7	4637	Btuh
	Ceiling Total					2800 (sqft)				4637	Btuh
Floors	Туре		R-Va	alue		Size			HTM	Load	
1	Slab On Grade			0.0		20	14 (ft(p))		0.0	O	Btuh
	Floor Total						0 (sqft)			0	Btuh
						. Z 0	one Env	elope S	ubtotal:	23422	Btuh
Infiltration	Туре		A	CH		Volum	e(cuft)		CFM=	Load	
	SensibleNatural			0.70		174			203.1	3780	Btuh
internal			Occup	pants	11	Btuh/oc	cupant		Appliance	Load	600
gain				6		X 23	0 +		2400	3780	
Duct load	Proposed leak free, R6.	0, Sup	ply(At	tic), F	Return(/	Attic)		DGM	= 0.00	0.0	Btuh
	Sensible Zone Load 30982 Bt							Btuh			

Manual J Summer Calculations

Residential Load - Component Details (continued)

Moran Residence 1501 SW CR 778 , FL 32038-

Project Title: Jeffrey & Bonnie Moran Code Only **Professional Version** Climate: North

9/14/2006

HOUSE TOTALS

	•	· · · · · · · · · · · · · · · · · · ·	
	Sensible Envelope Load All Zones	30982	Btuh
6	Sensible Duct Load	0	Btuh
	Total Sensible Zone Loads	30982	Btuh
1	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	30982	Btuh
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	7422	Btuh
	Latent ventilation gain	0	Btuh
8	Latent duct gain	0	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	8622	Btuh
	TOTAL GAIN	39604	Btuh

*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))

(Exsh - Exterior shading device: none(N) or numerical value)

(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



For Florida residences only

Residential Window Diversity

MidSummer

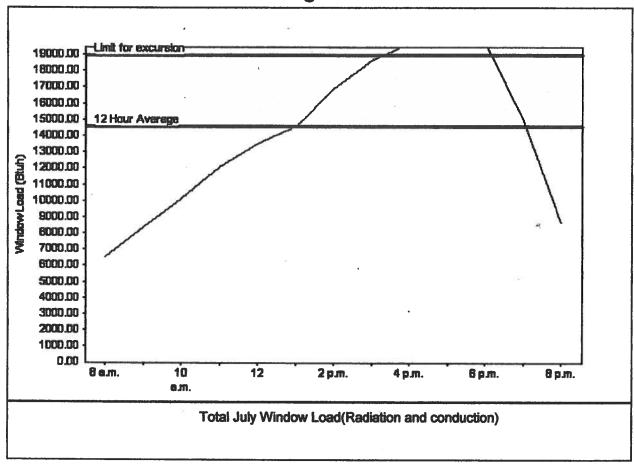
Moran Residence 1501 SW CR 778 , FL 32038Project Title: Jeffrey & Bonnie Moran

Code Only Professional Version Climate: North

9/14/2006

Weather data for: Gainesville - Defi	aults		in the second se	
Summer design temperature	92	F	Average window load for July	14576 Btu
Summer setpoint	75	F	Peak window load for July	19937 Btu
Summer temperature difference	17	F	Excusion limit(130% of Ave.)	18949 Btu
Latitude	29	North	Window excursion (July)	988 Btuh

WINDOW Average and Peak Loads



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY:
DATE:





RE: MORANJ -

TRENCO

818 Soundside Road Edenton, NC 27932 Telephone 252/482-7000

Fax 252/482-7115

Site Information:

Project Customer: JEFF MORAN Project Name: MORANJ

Lot/Block:

Subdivision:

Address: 1501 SW CR 778

City: HIGH SPRINGS

State: FLORIDA

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

State:

Address:

City:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special

Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.3

Wind Code: ASCE 7-02 Wind Speed: 110 mph

Design Method: Components/Cladding ASCE 7-02

Roof Load: 40.0 psf Floor Load: 55.0 psf

This package includes 7 individual, dated Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet

conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date
1	E3606418	MORANJ	Α	10/31/06
2	E3606419	MORANJ	A1	10/31/06
3	E3606420	MORANJ	В	10/31/06
4	E3606421	MORANJ	DET	10/31/06
5	E3606422	MORANJ		10/31/06
6	E3606423	MORANJ		10/31/06
7	E3606424	MORANJ		10/31/06

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.

Marvin A. Strzyzewski Truss Engineering Comp 818 Soundside Rd. Edenton, NC 27932 FL Cert.#7239

October 31,2006

Strzyzewski, Marvin

Job Truss Truss Type Qty F3606418 MORANI 14 SPECIAL Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:19 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL -2-0-0 13-11-1 18-0-15 22-5-6 26-9-12 31-8-0 35-5-2 40-0-0 42-0-0 2-0-0 7-1-7 4-1-15 4-10-4 6-9-9 3-9-2 4-6-14 2-0-0 6x6 Mil20// Scale = 1:103.2 5x5 MII20× 10.00 12 3x6 MII20 // 5 6 3x5 MII20 √ 3x5 MII20 // 4.00 12 5x5 MII20> 8 3x6 MII20≈ 3 3x4 Mil20> 9 3x5 MII20 > 10 24 4-8-11 12 13 3x5 MII20= 3x5 MII20= 22 21 20 19 18 17 16 15 14 1.5x4 MII20 || 3x6 MII20= 3x4 MII20 = 3x5 MII20 = 3x4 MII20 = 3x4 MII20= 3x6 MII20= 1.5x4 MII20 || 3x10 MII20= 18-0-15 22-5-6 26-9-12 31-8-0 35-5-2 40-0-0 7-1-7 6-9-9 4-10-4 3-9-2 4-6-14 Plate Offsets (X,Y): [2:0-5-3,0-0-10], [5:0-2-12,0-1-8] LOADING (psf) **SPACING** 2-0-0 CSI **DEFL PLATES** GRIP in (loc) I/defl L/d TCLL 20.0 Plates Increase 1.25 TC 0.35 Vert(LL) -0.05 21-22 >999 240 MII20 249/190 TCDL 10.0 Lumber Increase 1.25 BC 0.35 Vert(TL) -0.16 21-22 >999 180 **BCLL** 0.0 Rep Stress Incr YES WB 0.51 0.04 Horz(TL) n/a n/a **BCDL** 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 270 lb

LUMBER

TOP CHORD 2 X 4 SYP No. 2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 WFBS

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-11 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. WFRS 3-21, 5-19, 7-19 1 Row at midot

fabrication. This determination is not the responsibility of the component/truss designer.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss

REACTIONS (ib/size) 2=1346/0-8-0, 15=1812/0-8-0, 12=272/0-3-8

Max Horz 2=-423(load case 3)

Max Uplift2=-399(load case 5), 15=-315(load case 6), 12=-284(load case 4) Max Grav 2=1346(load case 1), 15=1812(load case 1), 12=294(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/79, 2-3=-1579/296, 3-23=-1144/305, 4-23=-1026/306, 4-5=-1008/345, 5-6=-750/319, 6-7=-1059/357, **TOP CHORD**

7-8=-1228/301, 8-9=-883/201, 9-10=-937/193, 10-24=-71/592, 11-24=-83/586, 11-12=-49/175, 12-13=0/37

2-22=-142/1088, 21-22=-142/1088, 20-21=0/777, 19-20=0/777, 18-19=-8/881, 17-18=-58/870, 16-17=-58/870, BOT CHORD

15-16=-525/192, 14-15=-151/82, 12-14=-151/82

3-22=0/306, 3-21=-422/269, 5-21=-117/435, 5-19=-216/174, 6-19=-148/397, 7-19=-291/176, 7-18=-45/164,

8-18=-19/127, 8-16=-735/174, 10-16=-245/1595, 10-15=-1543/304, 11-15=-434/159, 11-14=0/160

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 2, 315 lb uplift at joint 15 and 284 lb uplift at joint 12.

LOAD CASE(S) Standard

October 31,2006

Job Truss Truss Type Qty F3606419 MORANJ 5 A1 SPECIAL Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:20 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL 7-1-7 13-11-1 18-0-15 22-5-6 26-9-12 31-8-0 35-5-2 40-0-0 42-0-0 7-1-7 4-1-15 4-4-6 4-10-4 3-9-2 4-6-14 2-0-0 6x6 MII20 // Scale = 1:101.0 10.00 18x6 MII20/ 5x5 MII20 >> 5 3x5 MII20 ◇ 3x5 MII20// 4.00 <u>12</u> 5x5 Mil20 ≈ 22 3x6 MII20 ≈ 2 3x4 MII20 > 8 3x5 MII20~ 9 4-8-11 10 12 4x6 MII20= 3x5 MII20= 21 20 19 13 1.5x4 MII20 || 3x6 MII20= 3x4 MII20= 3x5 MII20= 3x4 MII20= 3x4 MH20= 3x6 MII20= 1 5x4 MII20 II 3x10 MII20= 18-0-15 13-11-1 26-9-12 31-8-0 35-5-2 40-0-0 7-1-7 6-9-9 4-1-15 4-10-4 4-4-6 3-9-2 4-6-14 Plate Offsets (X,Y): [1:0-6-3,0-0-6], [4:0-2-12,0-1-8] LOADING (psf) **SPACING** 2-0-0 CSI **DEFL PLATES** GRIP in (loc) l/defl L/d TCLL 20.0 Plates Increase TC 1 25 0.34 Vert(LL) 0.10 1-21 >999 240 MII20 249/190 10.0 BC TCDL Lumber Increase 1.25 0.39 Vert(TL) -0.17 1-21 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.51 0.04 Horz(TL) 14 n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 266 lb LUMBER **BRACING** TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 4-9-2 oc purtins. **BOT CHORD** BOT CHORD 2 X 4 SYP No.2D Rigid ceiling directly applied or 6-0-0 oc bracing. **WEBS** 2 X 4 SYP No.3 WFRS 2-20, 4-18, 6-18 1 Row at midpt

> This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 1=1200/0-8-0, 14=1819/0-8-0, 11=271/0-3-8

Max Horz 1=-452(load case 3)

Max Uplift1=-264(load case 5), 14=-318(load case 6), 11=-283(load case 4)

Max Grav 1=1200(load case 1), 14=1819(load case 1), 11=293(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-1607/337, 2-22=-1154/320, 3-22=-1013/321, 3-4=-959/359, 4-5=-756/327, 5-6=-1066/368, 6-7=-1234/310,

7-8=-886/206, 8-9=-941/198, 9-23=-77/597, 10-23=-89/590, 10-11=-55/179, 11-12=0/37

BOT CHORD

1-21=-182/1117, 20-21=-182/1117, 19-20=0/783, 18-19=0/783, 17-18=-15/886, 16-17=-63/873, 15-16=-63/873,

14-15=-529/194, 13-14=-155/88, 11-13=-155/88

WEBS

2-21=0/313, 2-20=-451/310, 4-20=-133/438, 4-18=-216/174, 5-18=-152/400, 6-18=-291/176, 6-17=-45/164,

7-17=-19/127, 7-15=-740/180, 9-15=-257/1604, 9-14=-1550/314, 10-14=-434/159, 10-13=0/160

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 1, 318 lb uplift at joint 14 and 283 lb uplift at joint 11.

LOAD CASE(S) Standard

October 31,2006

818 Soundside Road Edenton, NC 27932

Job Qty Truss Truss Type F3606420 MORANJ В COMMON Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:21 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL



Scale = 1:36:1

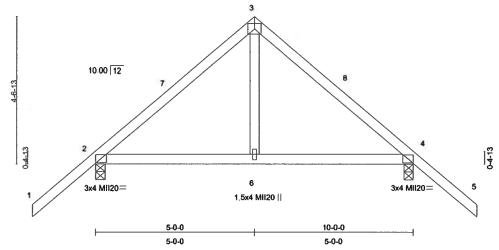


Plate Offsets (X,Y): [2:0-2-1,0-1-8], [4:0-2-1,0-1-8]										
LOADIN	IG (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES GRIP		
TCLL	20.0	Plates Increase 1.25	TC 0.32	Vert(LL)	-0.02 2-6	>989	240	MII20 249/190		
TCDL	10.0	Lumber Increase 1.25	BC 0.14	Vert(TL)	-0.03 4-6	>999	180			
BCLL	0.0	Rep Stress Incr YES	WB 0.07	Horz(TL)	0.00 4	n/a	n/a			
BCDL	10.0	Code FBC2004/TPI2002	(Matrix)					Weight: 48 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss

fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 2=517/0-3-8, 4=517/0-3-8

Max Horz 2=-155(load case 3)

Max Uplift2=-251(load case 5), 4=-251(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/77, 2-7=-396/78, 3-7=-302/95, 3-8=-302/95, 4-8=-396/77, 4-5=0/77

BOT CHORD 2-6=0/228, 4-6=0/228

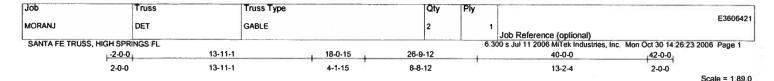
WEBS 3-6=0/217

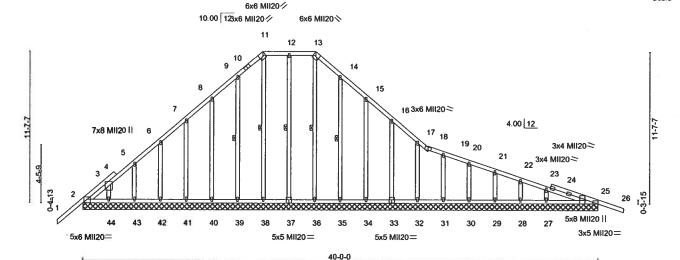
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 2 and 251 lb uplift at joint 4.

LOAD CASE(S) Standard

October 31,2006

Edenton NC 27932





OADING (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	1/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC.	0.29	Vert(LL)	-0.02	26	n/r	120	MII20	249/190
TCDL 10.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.03	26	n/r	120		
BCLL 0.0	Rep Stress Incr	YES	l wa	0.14	Horz(TL)	0.01	25	n/a	n/a		
BCDL 10.0	Code FBC2004/TF	12002	(Matr	ix)	,					Weight: 300 lb)

LUMBER TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **OTHERS**

BRACING TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 14-35, 13-36, 12-37, 11-38, 9-39

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss

fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 25=301/40-0-0, 2=265/40-0-0, 35=156/40-0-0, 36=147/40-0-0, 37=165/40-0-0, 38=147/40-0-0, 39=156/40-0-0, 40=161/40-0-0, 41=160/40-0-0, 42=158/40-0-0, 43=171/40-0-0, 44=100/40-0-0, 34=161/40-0-0, 33=159/40-0-0, 32=166/40-0-0, 31=164/40-0-0, 30=158/40-0-0, 29=165/40-0-0, 28=140/40-0-0, 27=241/40-0-0

Max Horz 2=-408(load case 3)

Max Uplift25=-227(load case 4), 2=-185(load case 3), 35=-54(load case 5), 37=-59(load case 4), 39=-55(load case 5), 40=-75(load case 6), 41=-92(load case 5), 42=-117(load case 5), 43=-124(load case 5), 44=-24(load case 6), 34=-75(load case 6), 33=-69(load case 5), 32=-66(load case 6), 31=-105(load case 4), 30=-76(load case 4),

29=-79(load case 4), 28=-90(load case 4), 27=-72(load case 4)

Max Grav 25=301 (load case 1), 2=265 (load case 1), 35=156 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 36=149 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 36=149 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 36=149 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 37=169 (load case 10), 36=149 (load case 10), 36=149 (load case 10), 37=169 (load case 10), 36=149 (load cas 38=147(load case 9), 39=156(load case 1), 40=161(load case 1), 41=160(load case 1), 42=158(load case 9), 43=171(load case 1), 44=108(load case 2), 34=161(load case 1), 33=159(load case 10), 32=166(load case 1), 31=164(load case 1), 30=158(load case 10), 29=165(load case 1), 28=140(load case 10), 27=241(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/75, 2-3=-237/296, 3-4=-199/273, 4-5=-189/281, 5-6=-140/287, 6-7=-72/276, 7-8=-59/268, 8-9=-60/262, 9-10=-57/276, 10-11=-5/284, 11-12=-16/246, 12-13=-16/246, 13-14=-58/284, 14-15=-60/225, 15-16=-59/192, 16-17=-60/207, 17-18=-44/167, 18-19=-46/167, 19-20=-69/126, 20-21=-96/90, 21-22=-122/51, 22-23=-147/13,

23-24=-169/19, 24-25=-177/8, 25-26=0/36

BOT CHORD 2-44=0/209, 43-44=0/208, 42-43=0/208, 41-42=0/208, 40-41=0/208, 39-40=0/208, 38-39=0/208, 37-38=0/207, 36-37=0/207, 35-36=0/207, 34-35=0/207, 33-34=0/207, 32-33=0/207, 31-32=0/207, 30-31=0/207, 29-30=0/207,

28-29=0/207, 27-28=0/207, 25-27=0/207

WEBS 14-35=-116/74, 13-36=-109/0, 12-37=-129/79, 11-38=-108/0, 9-39=-116/74, 8-40=-121/95, 7-41=-120/111,

6-42=-119/139, 5-43=-126/136, 3-44=-77/53, 15-34=-121/95, 16-33=-119/89, 17-32=-126/86, 19-31=-124/125,

20-30=-119/95, 21-29=-122/102, 22-28=-111/101, 23-27=-174/113

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

Continued on payer drainage to prevent water ponding.

October 31,2006



Edenton, NC 27932

Job	Tn	uss	Truss Type	Qty	Ply	
MORANJ	DE	-	GABLE		١.	E3606421
MURANI	UE	·'	GABLE	2	'	Job Reference (optional)
SANTA F	E TRUSS, HIGH SPRING	S FL.				00 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:23 2006 Page 2

SANTA FE TRUSS, HIGH SPRINGS FL

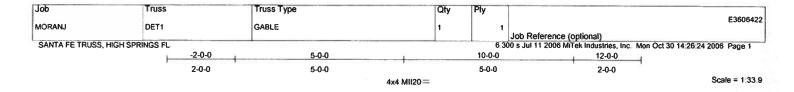
NOTES

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 1.5x4 MII20 unless otherwise indicated.
- 7) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 25, 185 lb uplift at joint 2, 54 lb uplift at joint 35, 59 lb uplift at joint 37, 55 lb uplift at joint 39, 75 lb uplift at joint 40, 92 lb uplift at joint 41, 117 lb uplift at joint 42, 124 lb uplift at joint 43, 24 lb uplift at joint 44, 75 lb uplift at joint 34, 69 lb uplift at joint 33, 66 lb uplift at joint 32, 105 lb uplift at joint 31, 76 lb uplift at joint 30, 79 lb uplift at joint 29, 90 lb uplift at joint 28 and 72 lb uplift at joint 27.

LOAD CASE(S) Standard

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fruss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flobrication, quality control, storage, delivery, erection and bracing, consult AMS/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrlo Drive, Madison, Wi 53719.





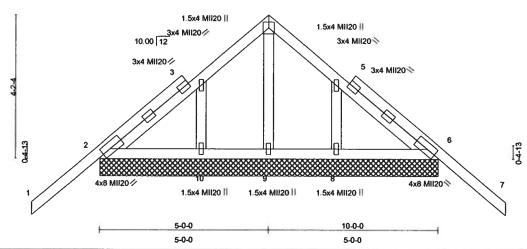


Plate Offsets (X,Y):	[2:0-3-4,0-2-3], [6:0-3-4,0-2-3]							
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc) l/defl	L/d	PLATES GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.29	Vert(LL)	-0.01	7 n/r	120	MII20 249/190	!
TCDL 10.0	Lumber Increase 1.25	BC 0.09	Vert(TL)	-0.02	7 n/r	120		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	0.00	6 n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)					Weight: 62 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D OTHERS 2 X 4 SYP No.3 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 2=434/10-0-0, 6=434/10-0-0, 9=13/10-0-0, 10=79/10-0-0, 8=79/10-0-0

Max Horz 2=-142(load case 3)

Max Uplift2=-275(load case 5), 6=-275(load case 6)

Max Grav 2=434(load case 1), 6=434(load case 1), 9=54(load case 2), 10=120(load case 2), 8=120(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/75, 2-3=-328/126, 3-4=-233/143, 4-5=-233/143, 5-6=-328/125, 6-7=0/75

BOT CHORD 2-10=-15/189, 9-10=-15/189, 8-9=-15/189, 6-8=-15/189

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 2 and 275 lb uplift at joint 6.

LOAD CASE(S) Standard

October 31,2006

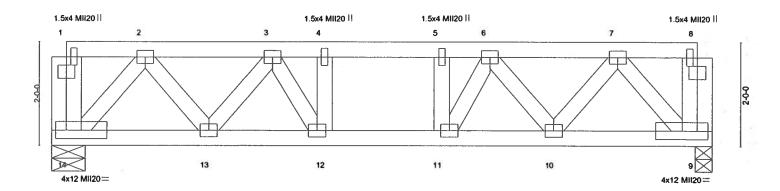


Qty Job Truss Truss Type Ply MORANJ 13 FLOOR Job Reference (optional) 6 300 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:24 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL

1-3-0

0-10-8 2-0-0 0-9-8

0-3-8 Grale = 1:22.5



	5-6-0	6-6-0	7-6-0		12-11-0		
	5-6-0	1-0-0	1-0-0	Je.	5-5-0		
Plate Offsets (X,Y):	[1:0-2-0,0-1-0], [8:0-2-0,0-1-0]						
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc) I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.34	Vert(LL)	-0.03 12 >999	480	MII20	249/190
TCDL 10.0	Lumber Increase 1.00	BC 0.52	Vert(TL)	-0.05 11-12 >999	360		
BCLL 0.0	Rep Stress Incr YES	WB 0.19	Horz(TL)	0.02 9 n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)				Weight: 72 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.3 BOT CHORD 2 X 4 SYP No.3 **WEBS** 2 X 4 SYP No.3 **BRACING**

TOP CHORD

Structural wood sheathing directly applied or 5-1-15 oc purlins, except

end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

REACTIONS (lb/size) 14=678/0-8-0, 9=678/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-14=-74/0, 8-9=-74/0, 1-2=-28/0, 2-3=-826/0, 3-4=-1189/0, 4-5=-1189/0, 5-6=-1189/0, 6-7=-826/0, 7-8=-29/0 13-14=0/552, 12-13=0/1088, 11-12=0/1189, 10-11=0/1090, 9-10=0/552

BOT CHORD

WEBS 4-12=-249/0, 5-11=-264/0, 2-14=-782/0, 2-13=0/464, 3-13=-444/0, 3-12=0/376, 7-9=-781/0, 7-10=0/463, 6-10=-447/0,

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MII20 unless otherwise indicated.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

October 31,2006

818 Soundside Road

Job Truss Truss Type Qty E3606424 MORANI PR VALLEY 21 Job Reference (optional) 6.300 s Jul 11 2006 MiTek Industries, Inc. Mon Oct 30 14:26:25 2006 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL 2-0-15 2-0-15 2-1-0 4x4 MII20= Scale = 1:10.3 10.00 12 2 2x4 Mil20= 1.5x4 MH20 | 2x4 MII20= 4-1-15 LOADING (psf) **SPACING** 2-0-0 CSI DEFL L/d **PLATES GRIP** in l/defl (loc) 20.0 **TCLL** Plates Increase 1.25 TC 0.03 Vert(LL) 999 MII20 249/190 n/a n/a TCDL 10.0 Lumber Increase 1.25 BC 0.02 999 Vert(TL) n/a n/a **BCLL** 0.0 Rep Stress Incr WB 0.01 0.00 YES Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 14 lb LUMBER BRACING

TOP CHORD

BOT CHORD

This truss design is based upon the building code shown. This code has been specified by the project engineer/architect, or building designer. The applicability of this code in any particular jurisdiction should be confirmed with the building official prior to truss fabrication. This determination is not the responsibility of the component/truss designer.

Structural wood sheathing directly applied or 4-1-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-15/4-1-15, 5=-15/4-1-15, 2=122/4-1-15, 4=122/4-1-15, 6=87/4-1-15

Max Horz 1=54(load case 4)

Max Uplift1=-42(load case 3), 5=-15(load case 1), 2=-78(load case 5), 4=-78(load case 5)

Max Grav 1=56(load case 4), 5=27(load case 5), 2=122(load case 1), 4=122(load case 1), 6=89(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-76/73, 2-3=-48/27, 3-4=-48/27, 4-5=-15/29

BOT CHORD 2-6=-11/31, 4-6=-11/31

WEBS 3-6=-50/11

TOP CHORD 2 X 4 SYP No.2D

BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3

NOTES

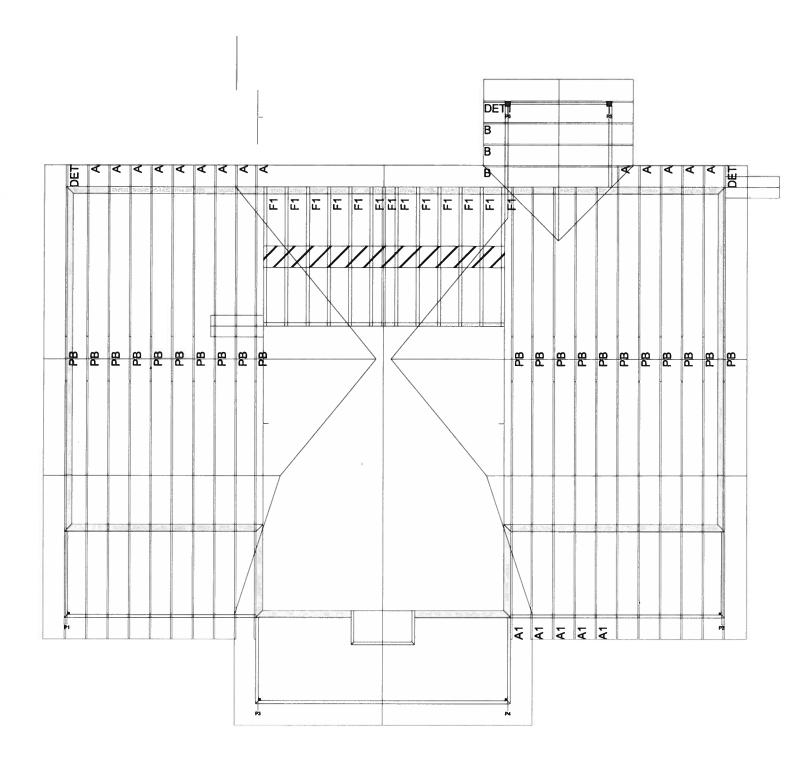
OTHERS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=5.opsf; BCDL=5.opsf; Category II; Exp B; enclosed; MWFRS automatic zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Gable requires continuous bottom chord bearing.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 15 lb uplift at joint 5, 78 lb uplift at joint 2 and 78 lb uplift at joint 4.
- 7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4, 6.
- 8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

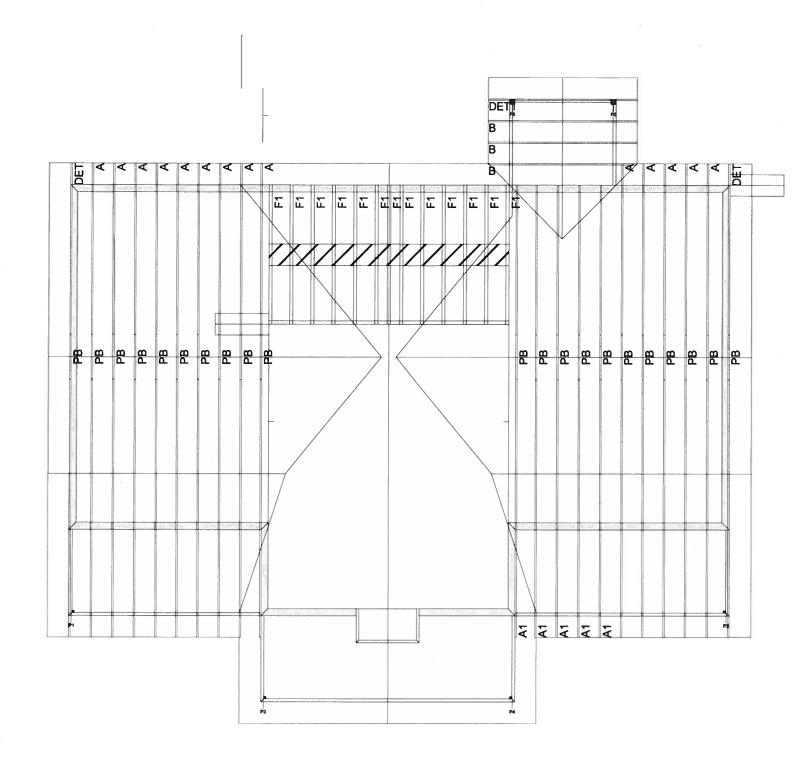
LOAD CASE(S) Standard

October 31,2006

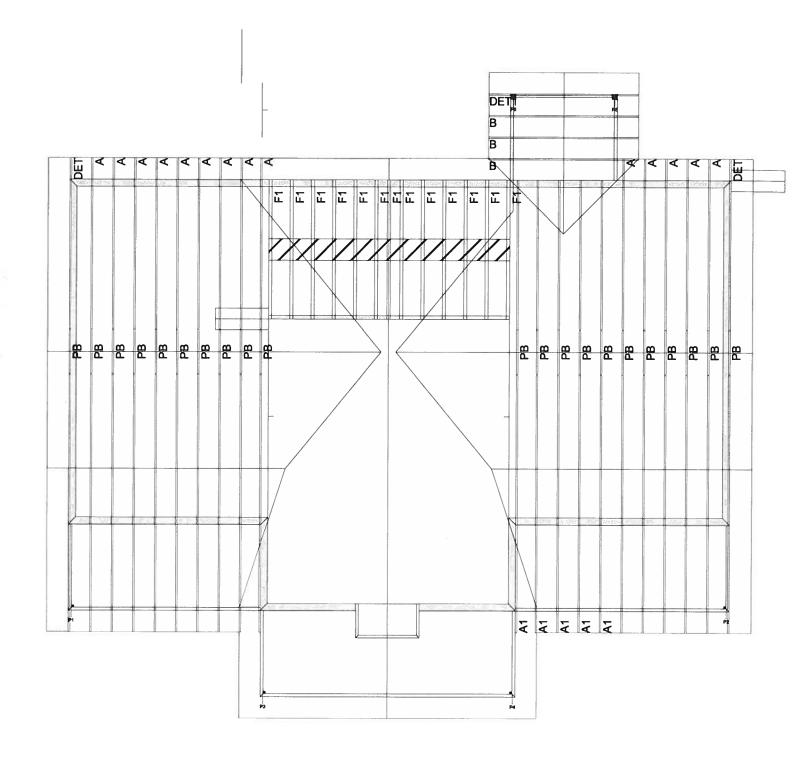




Santa Fe Truss	CUSTOMER	JEŁ
410 SW POE SPRINGS RD.	DATE	
HIGH SPRINGS, FL 32655	MODEL	MORA
FX#(386)454-1055 PH#(386)454-7711	DESIGNER	L.



Santa Fe Truss	CUSTOMER	JEŁ
410 SW POE SPRINGS RD.	DATE	
HIGH SPRINGS, FL 32655	MODEL M	ORAI
FX#(386)454-1055 PH#(386)454-7711	DESIGNER	L.



i	Santa I	e Truss	CUSTOMER	JEE
	410 SW POE	SPRINGS RD.	DATE	
	HIGH SPRIN	GS, FL 32655	MODEL	MORAI
	FX#(386)454-1055	PH#(386)454-7711	DESIGNER	L.