

SHINGLE- MIN. 15/32", 32/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED w/ 0.131x21 RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

TILE - MIN. 15/32" 32/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2\frac{1}{2}" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

METAL - MIN. 1/2", 24/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" OWOTHIN 4'-0" OF ROOF EDGE).

FLEXIBLE FINISH—MIN. 7/16", 24/16, APA RATED OSB **OR** PLYWOOD SHEATHING, FASTENED W/ 8D @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS INCLUDE: WOOD, CEMENT, OR VINYL SIDING, HARDI PANEL & BRICK. ALL OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.

STUCCO FINISH,—MIN. 7/16", 24/16, APA RATED OSB **OR** PLYWOOD SHEATHING, FASTENED W/ 8D 🚳 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 7/16 STRUCTURAL 1 GRADE SHEATHING OR 15/32 OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI530.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2000 psi PER ASTM C1019, GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF $^3\!\!$ A PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND

<u> CONCRETE MASONRY UNITS (CMU)</u>

CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEMWALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90E, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE 'S' MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH — #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REINF. SHALL BE INCREASED AS NOTED ON 1/S1.0. UNLESS NOTED OTHERWISE. LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

CONCRETE SPECIFICATIONS: CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS CONCRETE AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:

FOOTING AND FOUNDATIONS: FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR) UNLESS OTHERWISE NOTED ON DRAWINGS. MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF

REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. SAWCUTS: FOR

CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12'x12' GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

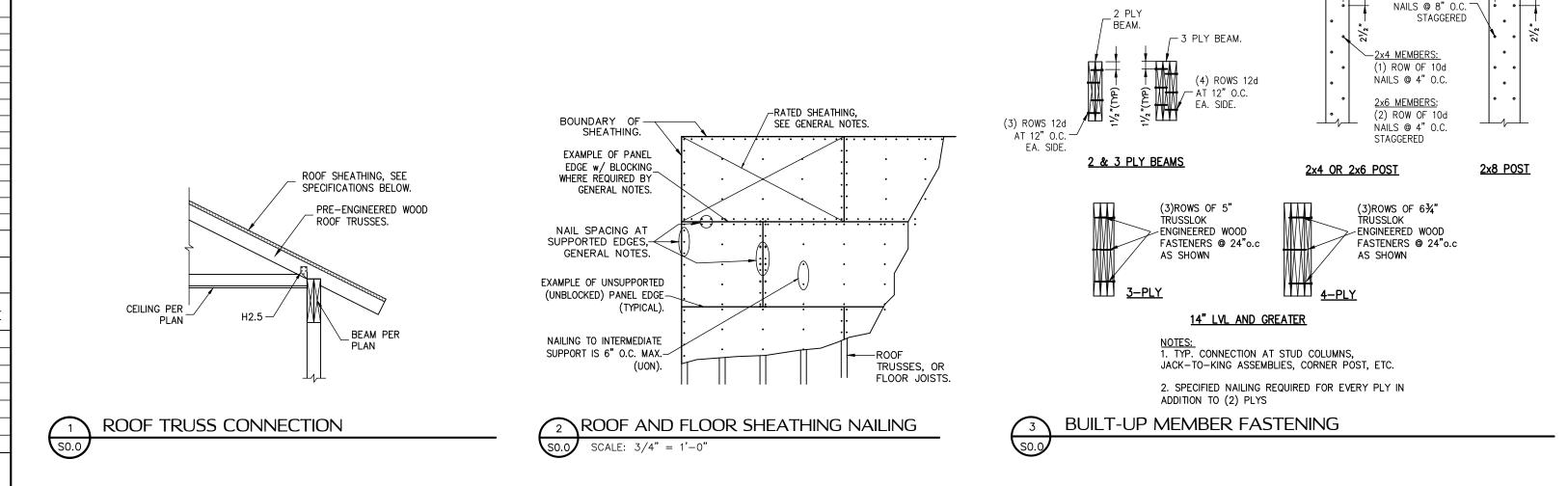
ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE—TREATED. IF, ACQ OR NON—DOT BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF ACZA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

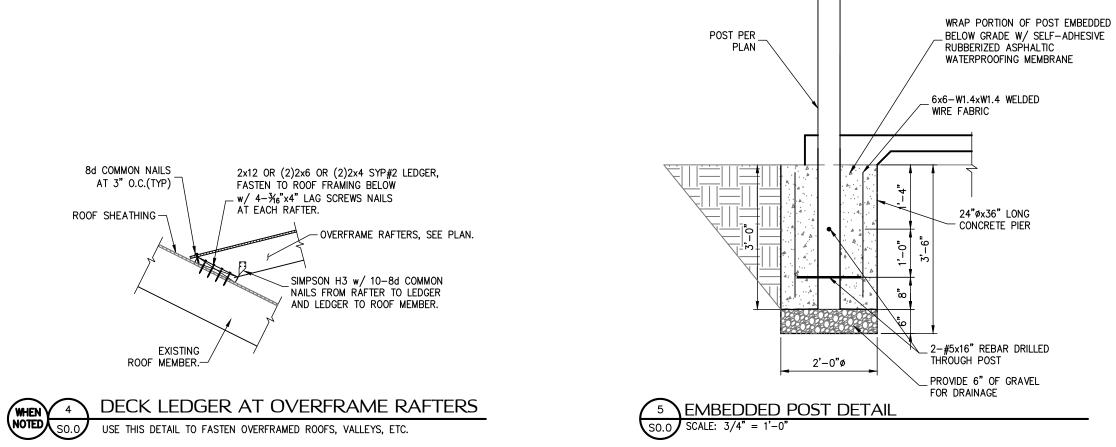
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER 'COMMENTARY' AND RECOMMENDATION FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91." AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

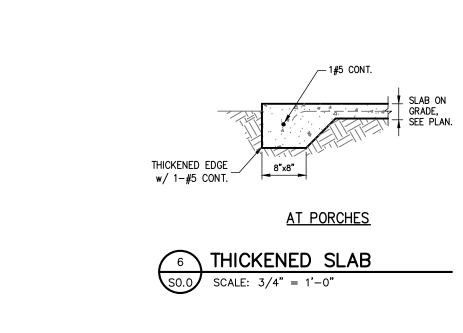
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL." AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOFS SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

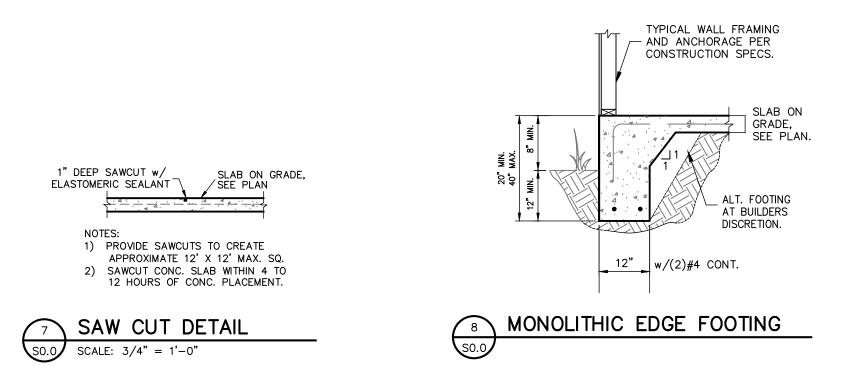
WOOD FASTENING SCHEDULE			BRICK NOTES / LINTEL SCHD			PLAN LEGEND AND ABBREVIATIONS	
			BRICK NOTES / EINTEL SCIED				
MEMBERS	CONNECTION TYPE	FASTENER	LINTEL DIMENSION	MIN. BRG.	MAX. SPAN	INTERIOR LOAD BEARING WALL	BUILT-UP POST IN THE WALL
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.	L3½x3½x½	4"	6'-0"	GABLE X-BRACE, SEE DETAIL 10/S0.1	
TOP PLATE, LAPS/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS	L4x3 ¹ / ₂ "x ¹ / ₄	6"	8'-0"	Short A shinery set be the largest	(2)2×8-1/2 HEADER SIZE, JACK AND
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAILS	L5x3 ¹ / ₂ "x ¹ / ₄	6"	10'-0"	DESIGNATES SHEARWALL. THE HIDDEN LINE DESIGNATES SIDE OF WALL THE	(2)2X3-1/2 KING STUD QUANTITY.
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"	L6x3 ¹ / ₂ "x ¹ / ₄	6"	12'-0"	SHEARWALL SHEATHING TO BE APPLIED.	
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS	L7x3 ¹ / ₂ "x ¹ / ₄	6"	16'-0"	SW 3"/6" 8d @ 3/6 DESIGNATES 8d COMMONS @ 3" O.C. EDGE & 6" O.C. "IN THE FIELD"	
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS			10 -0		
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS	1. STEEL LINTELS TO BE MINIMAL 36" LINTE MUST HAVE CORROSION RESISTANT COATING		BRICK VENEER WEATHER BARRIER	ADJ — ADJACENT	LG - Long
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS	OF EPOXY BASED PAINT.			BM — BEAM	MANUF - Manufacture
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS	2. LINTEL MORE THAN 8'-0". SHOULD BE			BOT — BOTTOM BRG — BEARING	MONO — Monolithic OC — On Center
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS	LATERALLY SUPPORTED NOT TO EXCEED 6 FT. O.C. w/ $2-\frac{1}{4} \times 3$ WD. SCREWS INTO			CMU - CONCRETE MASONRY UNIT	OSB — Oriented Strand Board
ROOF RAFTER TO 2x_ RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS	HEADER PROVIDE A 1/2" VERTICAL SLOTTED	\ \ <u>\</u>	A	DBL — DOUBLE DIA — DIAMETER	PERP - Perpendicular PRE ENG - Pre Engineered
CONT. HEADER, TWO PIECES	FACE NAIL	16d@ 16" O.C. @ EDGE	HOLE FOR SCREW.	ATTAC	LINTEL ATTACHMENT	EA — EACH EE — EACH END F	PSF — Pounds per Square Foot PSI — Pounds per Square Inch
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS	3. BRICK VENEER ATTACHMENT: HORIZONTA		SEE NOTE 2	EOR — ENGINEER OF RECORD	PT — PRESSURE TREATED
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS	TIES @ 24" O.C., VERT. TIES @ 12" O.C (FOR 110mph WIND-ZONE VERT. TIES @ 16"		- 	EQ — EQUAL EXT — EXTERIOR	QT — Quick Tie REINF — Reinforce
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"	O.C.). AT ALL OPENINGS SPACE TIES WITHIN		☐ FLASHING	FBC — FLORIDA BUILDING CODE	SF — Square Foot
NAIL SPECIFICATIONS 3"x0.131"ø = GUN NAILS 2"x0.113"ø = RINK SHANK			12" OF OPENINGS. PROVIDE 3/16" Ø WEEP HOLES @ 33" O.C. IMMEDIATELY ABOVE FLASHING.	SEE PLAN	BRICK LINTEL, SEE SCHEDULE	FDN — FOUNDATION FT — FOOT FTG — FOOTING	SPF —Spruce Pine Fur SYP — Southern Yellow Pine THRU — Through
$2"x0.113" \phi = 6d$ $3"x0.148" \phi = 10d$ $1\frac{1}{2}"x0.148" \phi = 10dx1\frac{1}{2}"$	$2\frac{1}{2}$ "x0.131"\delta = 8d $3\frac{1}{2}$ "x0.162"\delta = 16d $1\frac{1}{2}$ "x0.131"\delta = 8dx1\delta'"		TEXOTINO.		ECTION VIEW BRICK LINTEL	HDR — HEADER HORIZ — HORIZONTAL LBS — POUNDS	TYP — Typical UON — Unless Otherwise Noted VERT — Vertical WWF — Welded Wire Eabric







(3) ROW OF 10d





DESIGN **GENERAL**

SHEET 1 OF 5

No. 71461 STATE OF SIONAL 02.28.24

Christopher J Sabourin FL PE#71461

SABO STRUCTURAL ENGINEERING CA#32529 235 9TH AVE N JAX BEACH, FL 32250

> CHRIS@SABOENG.COM PLAN NAME LANE RESIDENCE SSE No. 24-0103

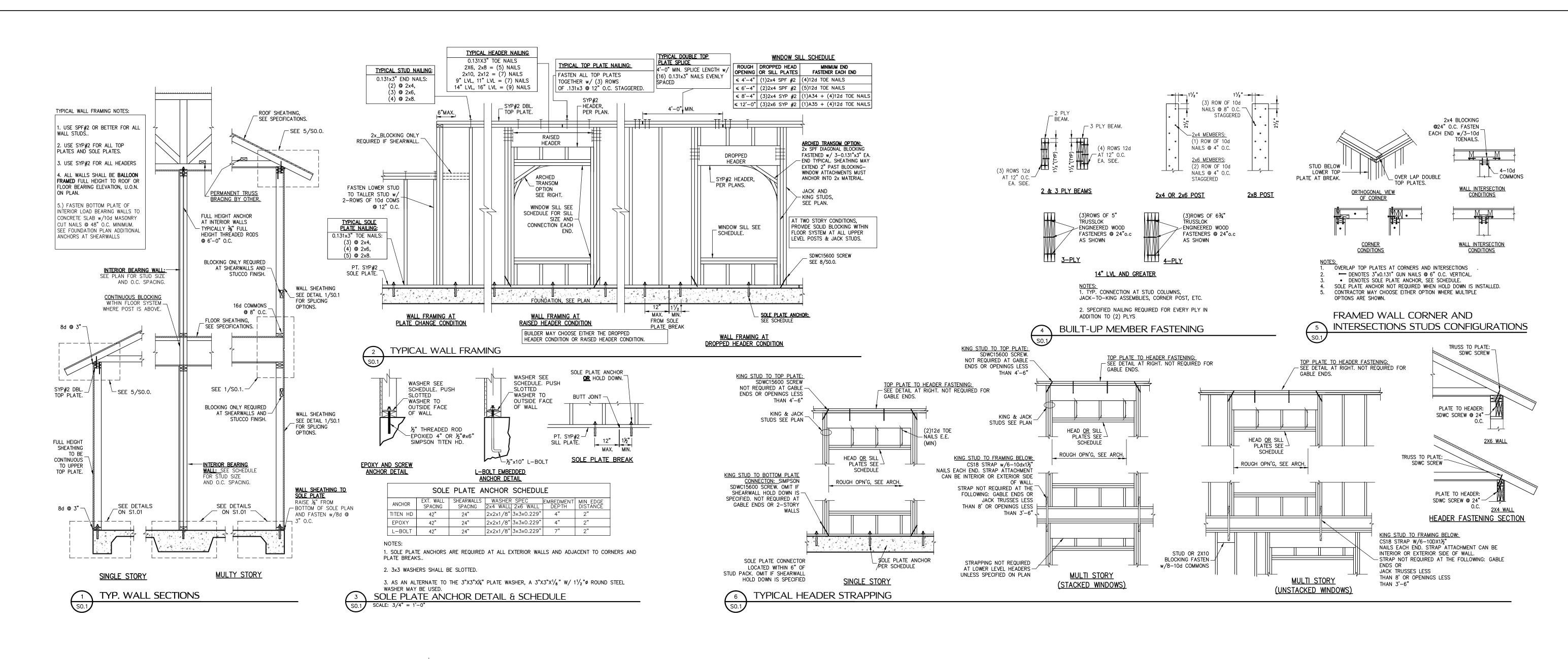
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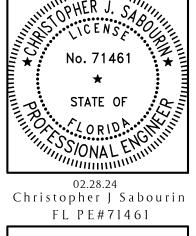
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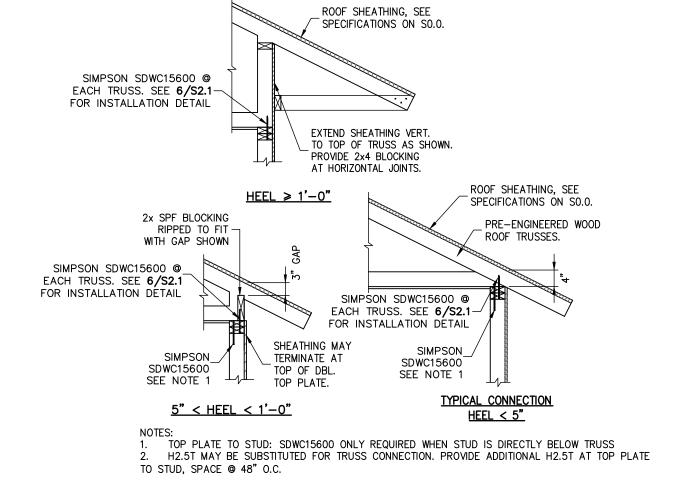
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DESIGN CRITERIA AND **GENERAL NOTES**

> SHEET SHEET 2 OF 5



ROOF TRUSS CONNECTION

MIN 2 PLY STUD IN LIEU OF 2 STUDS ITS ACCEPTABLE TO SISTER A 36" LONG

STUD FASTEN w/12-0.131x3" GUN

SIMPSON DTT2Z

½" THREADED ROD

-EPOXIED 4" MIN OR

½x6" TITEN HD

DTT2Z

HOLD DOWN ATTACHMENT DETAIL

/-w/8- ¼"x1½" SDS SCREWS

MIN 2 PLY

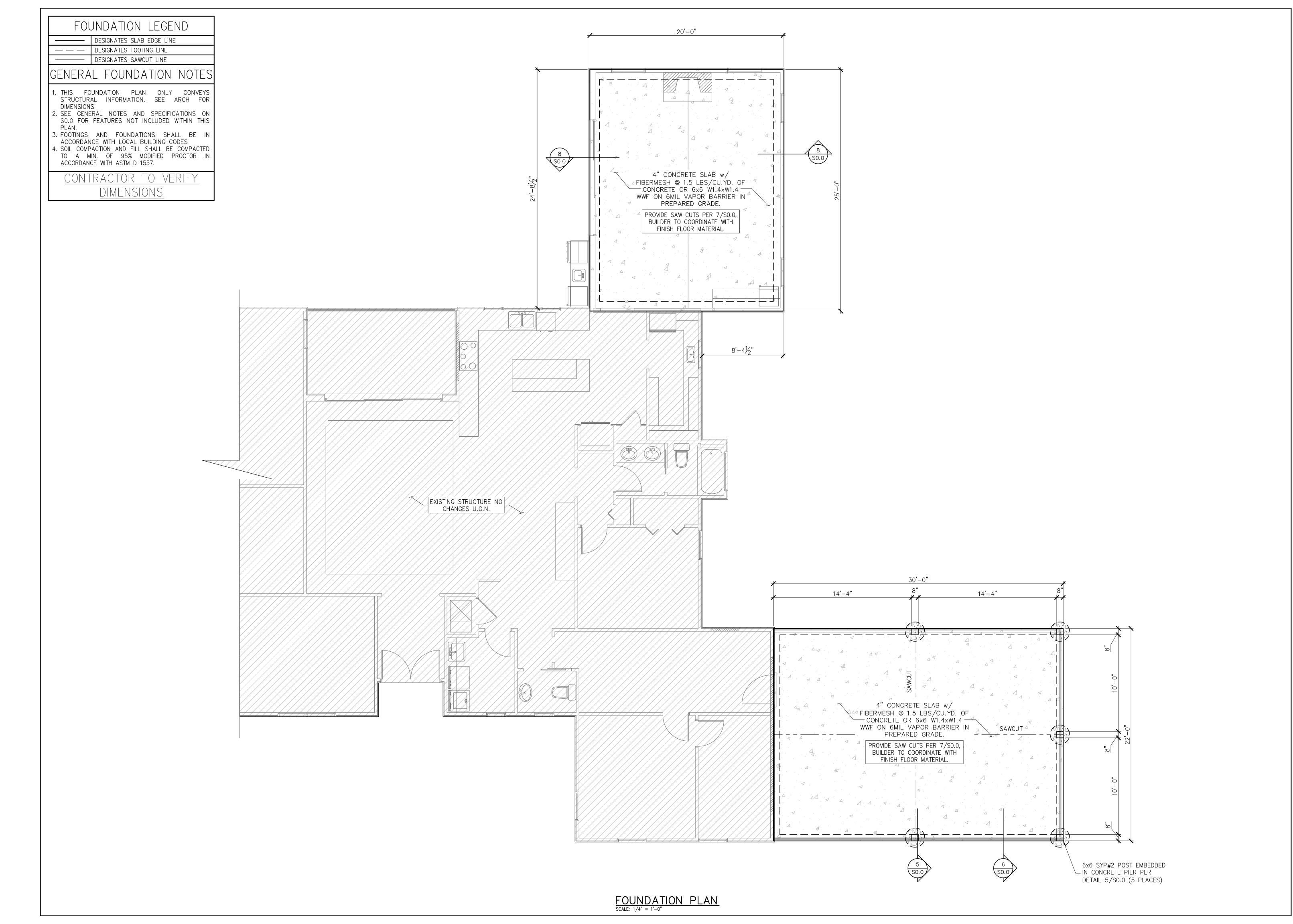
SIMPSON HTT5 OR

— HDU5. FASTEN

26-10d COMONS

ROD EPOXIED

HTT5 or HDU5





02.28.24 Christopher J Sabourin FL PE#71461

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STRUCTURAL ENGINEERING FOR 830 SW MANDIBA DR LAKE CITY, FL 32025

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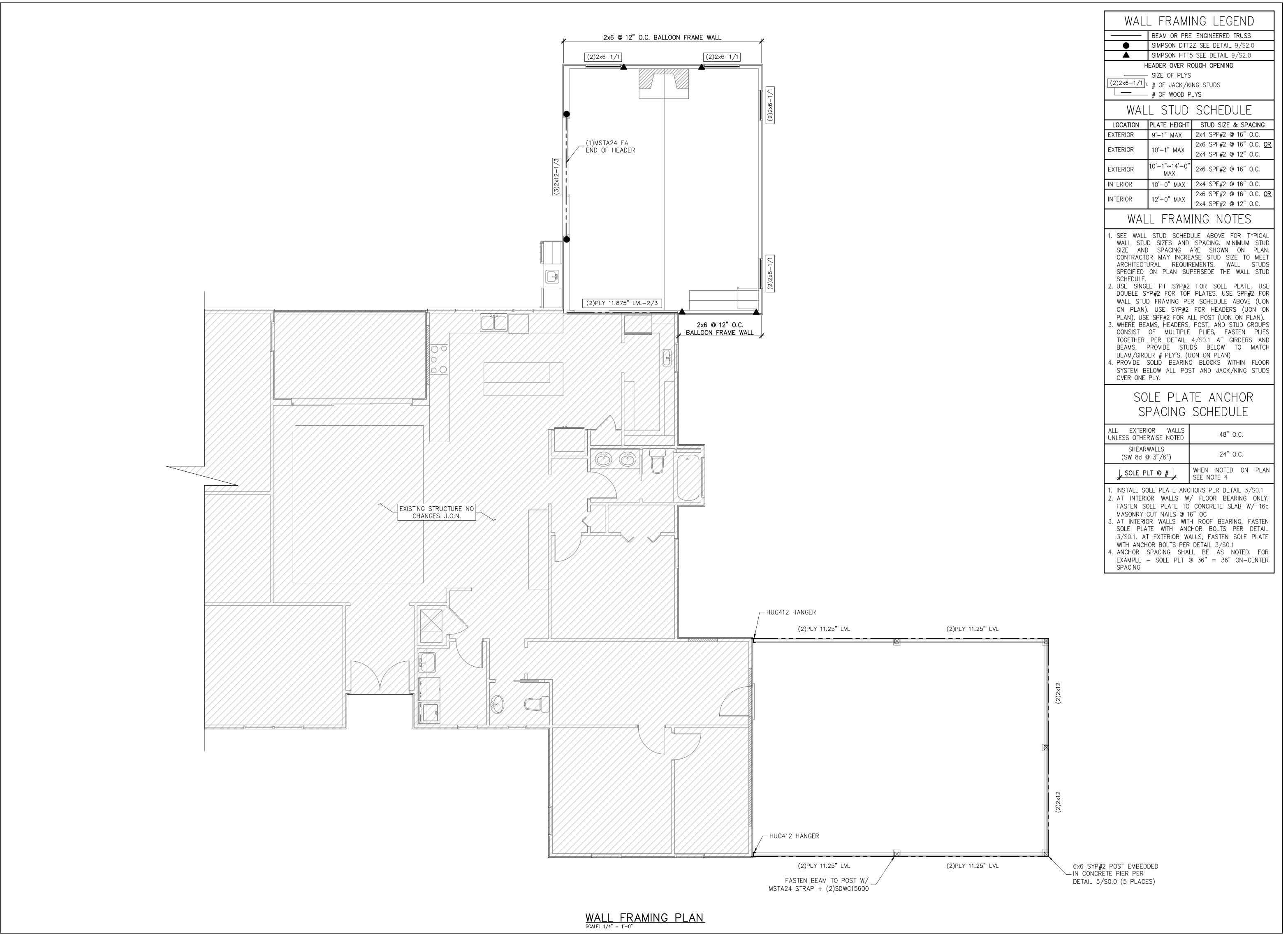
SCALING

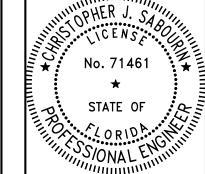
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CARPORT FOUNDATION, FRAMING AND ROOF PLAN

SHEET

SHEET 3 OF 5





02.28.24 Christopher J Sabourin FL PE#71461

SABO STRUCTURAL CA#32529

904-712-5750

SSE No. 24-0103

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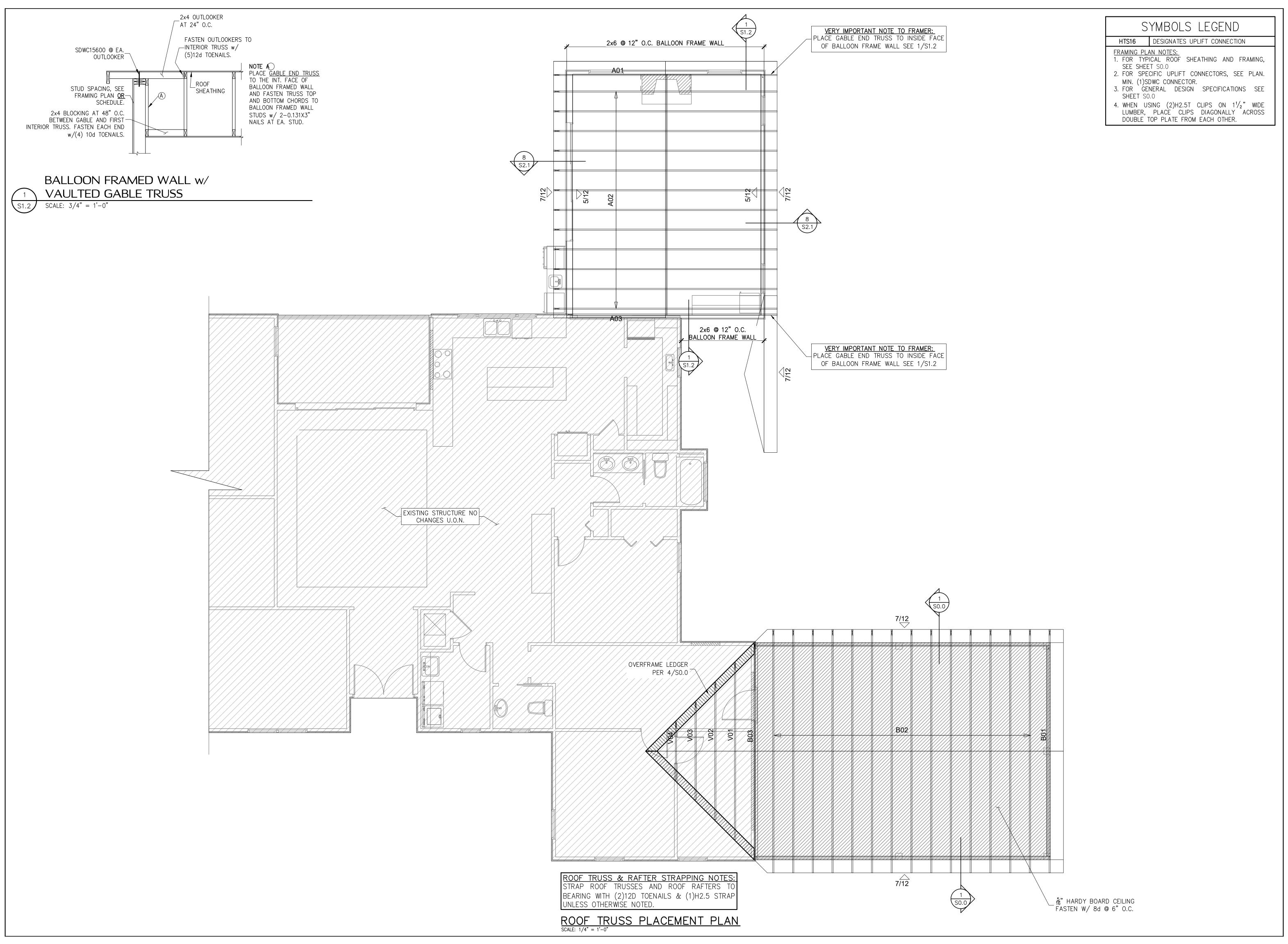
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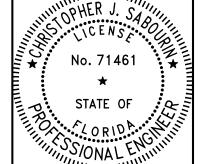
FIELD ALTERATION MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINA CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER SABOURIN MAY RESULT IN ADDITION ENGINEERING OR INSPECTION FEE SCALING

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> WALL **FRAMING** PLAN

SHEET 4 OF 5





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STRUCTURAL ENGINEERING FOR 830 SW MANDIBA DR LAKE CITY, FL 32025

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SCALING

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ROOF TRUSS PLACEMENT PLAN

SHEET

SHEET 5 OF 5