



NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
October 24, 2019

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS
SCALE: 1/4" = 1'-0"

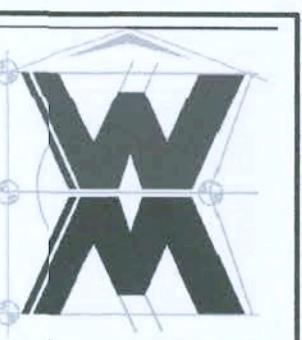
TYPICAL WALL SECTION

SCALE: 1" = 1'-0"

LOT 45 ROLLING MEADOWS
Property Address: 259 SW Buttercup Dr., Lake City, Florida 32025 (Parcel: 15-4S-16-03023-545)

GIBB LIA R CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

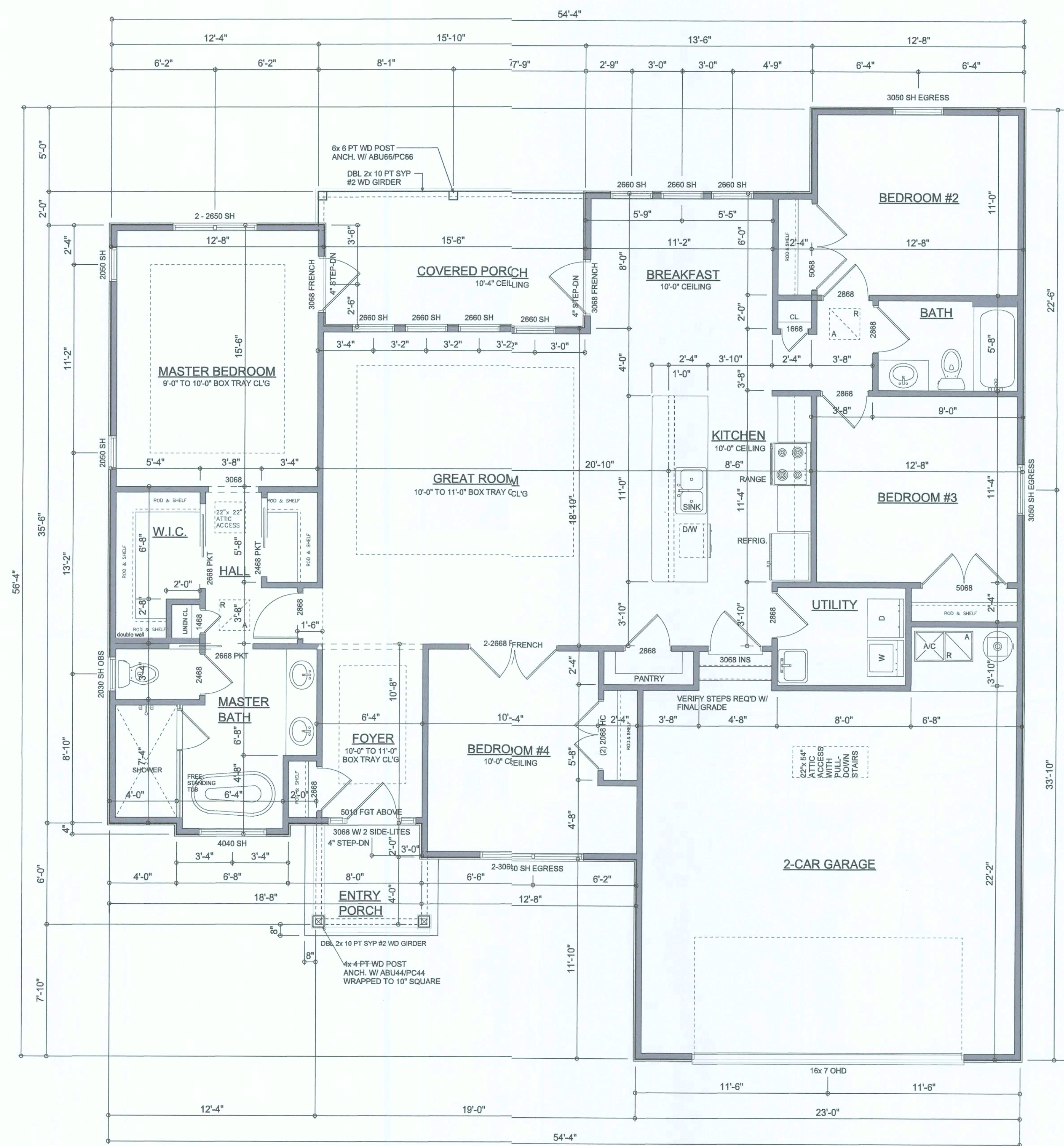
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ASSOCIATES, INC.
6 SW COMMERCE DR, STE 130
LAKE CITY, FL 32025
(386) 758-8406
will@willmyers.net



JOB NUMBER
20191023

SHEET NUMBER
A.1
OF 3 SHEETS





FLOOR PLAN

SCALE: 1/4" = 1'-0"
ALL CEILINGS SHALL BE 9'-0" UNLESS OTHERWISE NOTED

AREA SUMMARY

LIVING AREA	1,771	S.F.
GARAGE AREA	524	S.F.
ENTRY PORCH AREA	50	S.F.
COVERED PORCH AREA	125	S.F.
TOTAL AREA	2,470	S.F.

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W.C. Myers

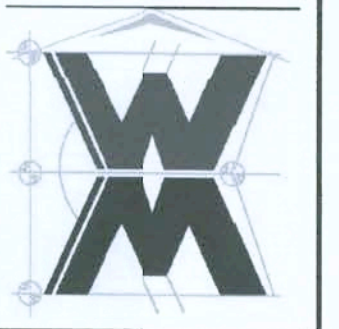
REVISIONS
November 04, 2019



DIMENSIONED FLOOR PLAN
SCALE: 1/4" = 1'-0"

MODEL 1763 (LEFT-HAND) FOR:
LOT 45 ROLLING MEADOWS
Property Address: 260 SW Bultmore Dr., Lake City, Florida 32025 (Parcel: 15-10-000020-047)
GIBALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

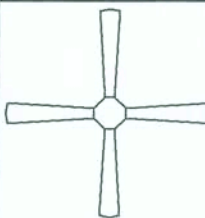













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JOB NUMBER
20191023

SHEET NUMBER

A.2
OF 3 SHEETS

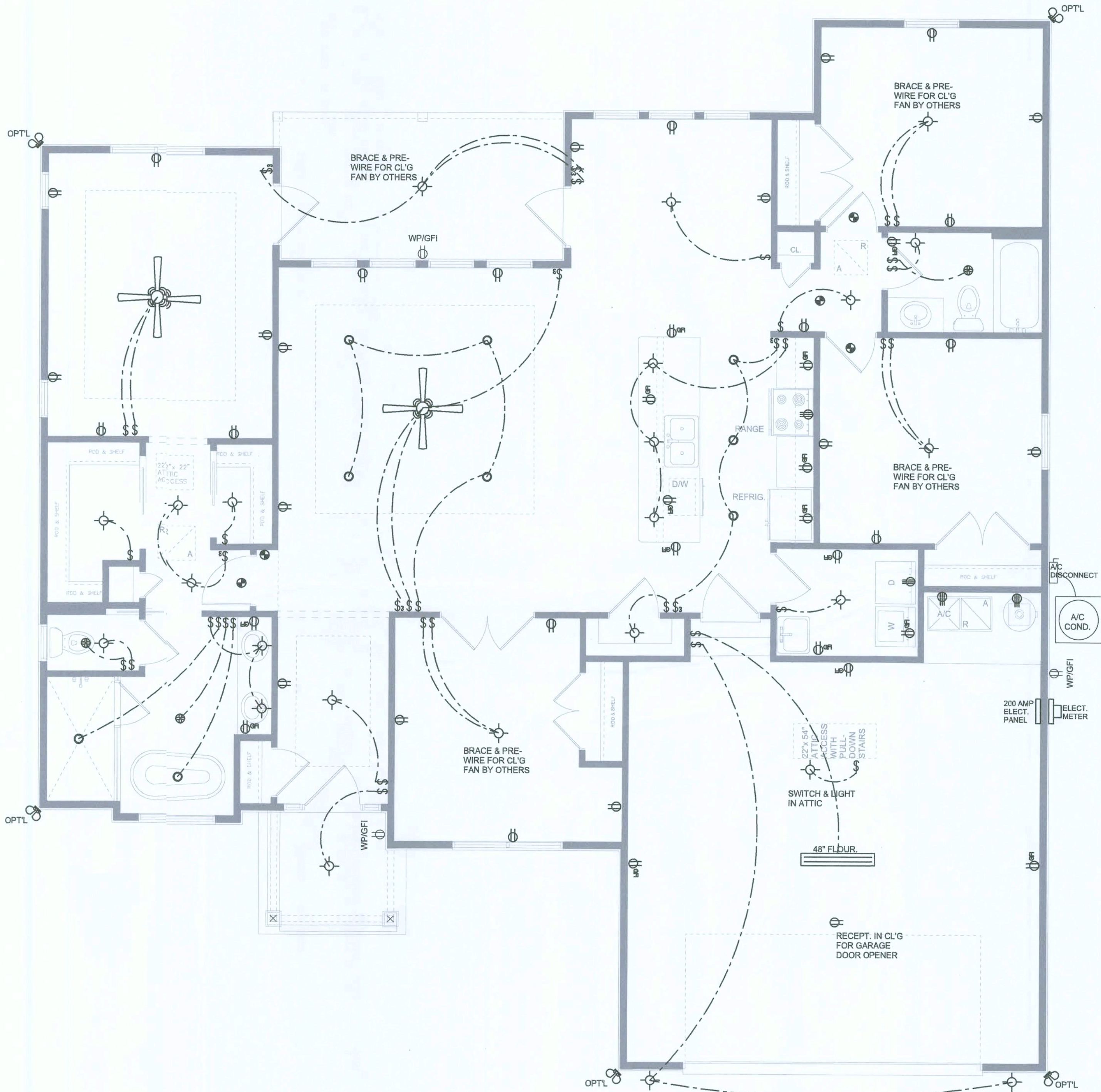
ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
	GFI DUPLEX OUTLET (PER NEC 406.8)
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
<div>48" FLOOR</div>	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:
ALL INTERIOR RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER
NEC 406.11

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR
AND SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE NFPA70 2011 NATIONAL
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
November 04, 2019

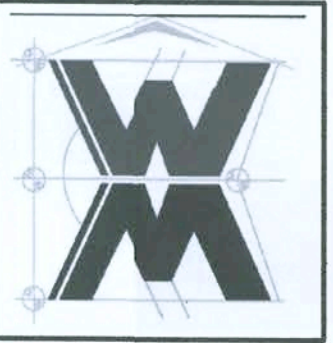
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

MODEL 1763 (LEFT-HAND) FOR:
LOT 45 ROLLING MEADOWS
Property Address: 259 SW Buttercup Dr., Lake City, Florida 32025 (Parcel: 15-4S-16-03023-545)

GIBALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

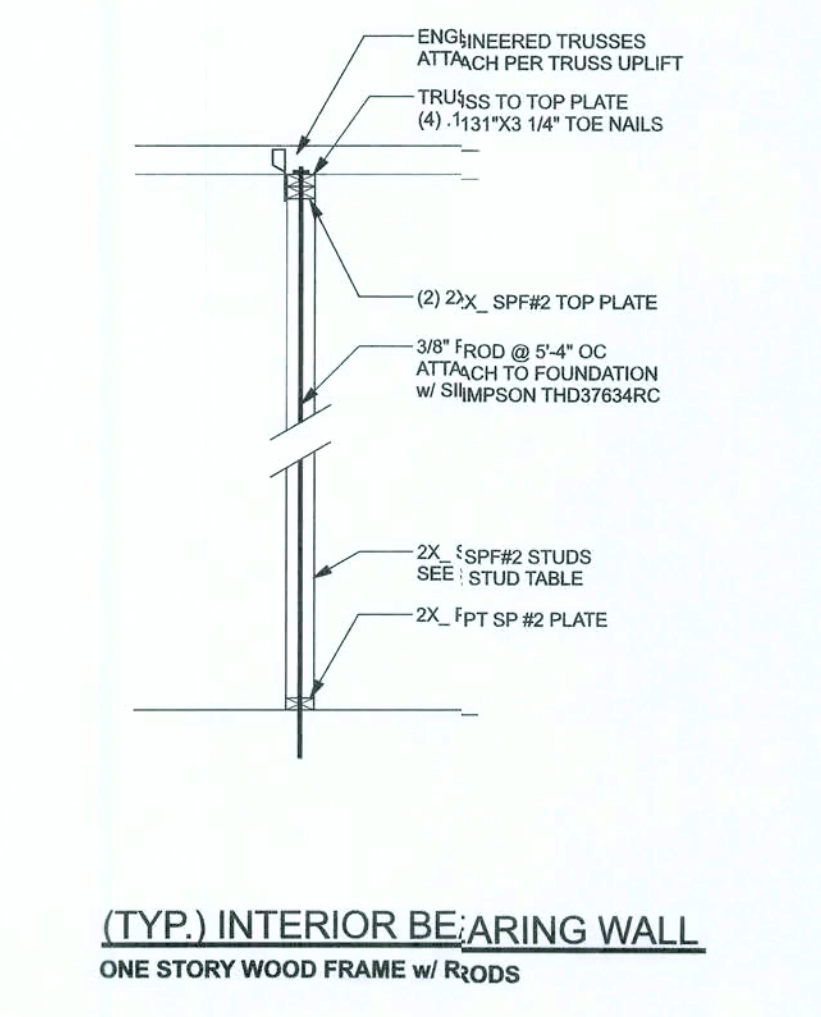
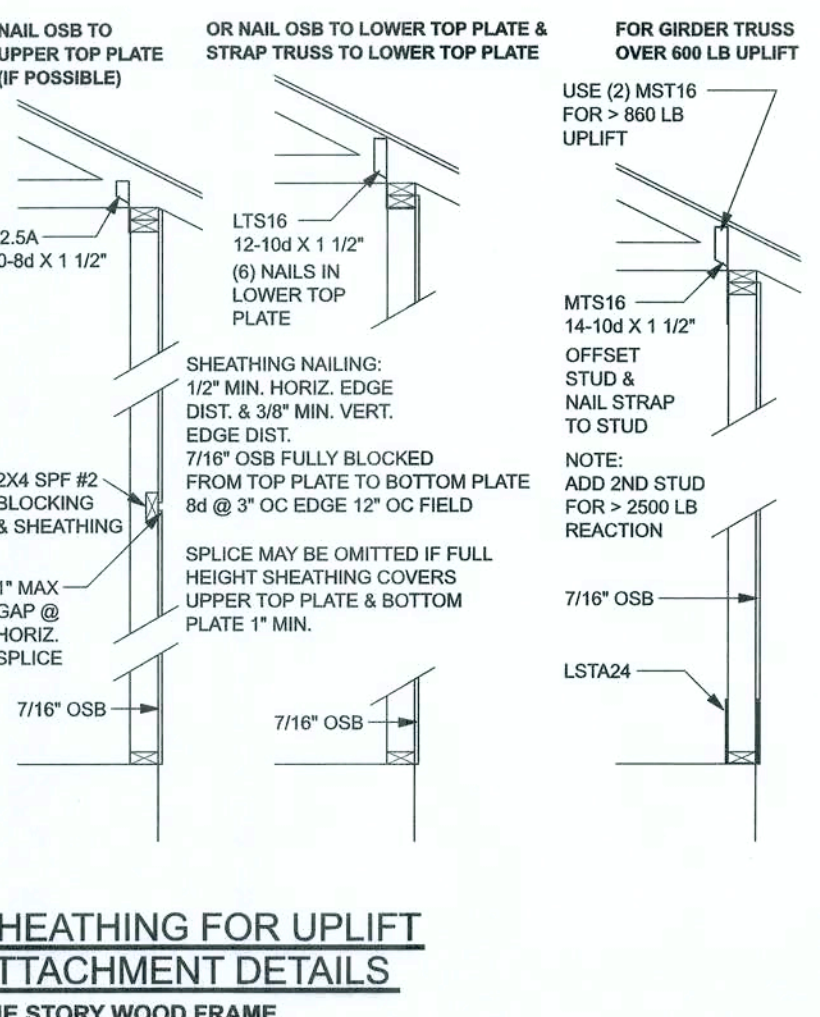
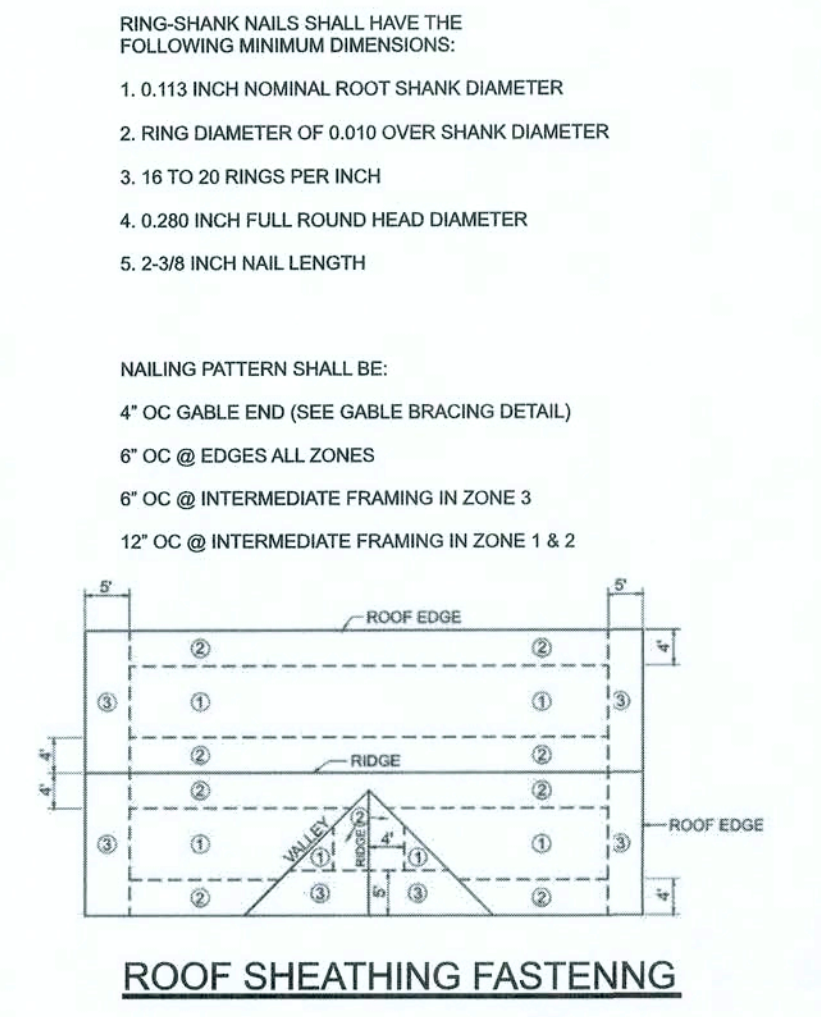
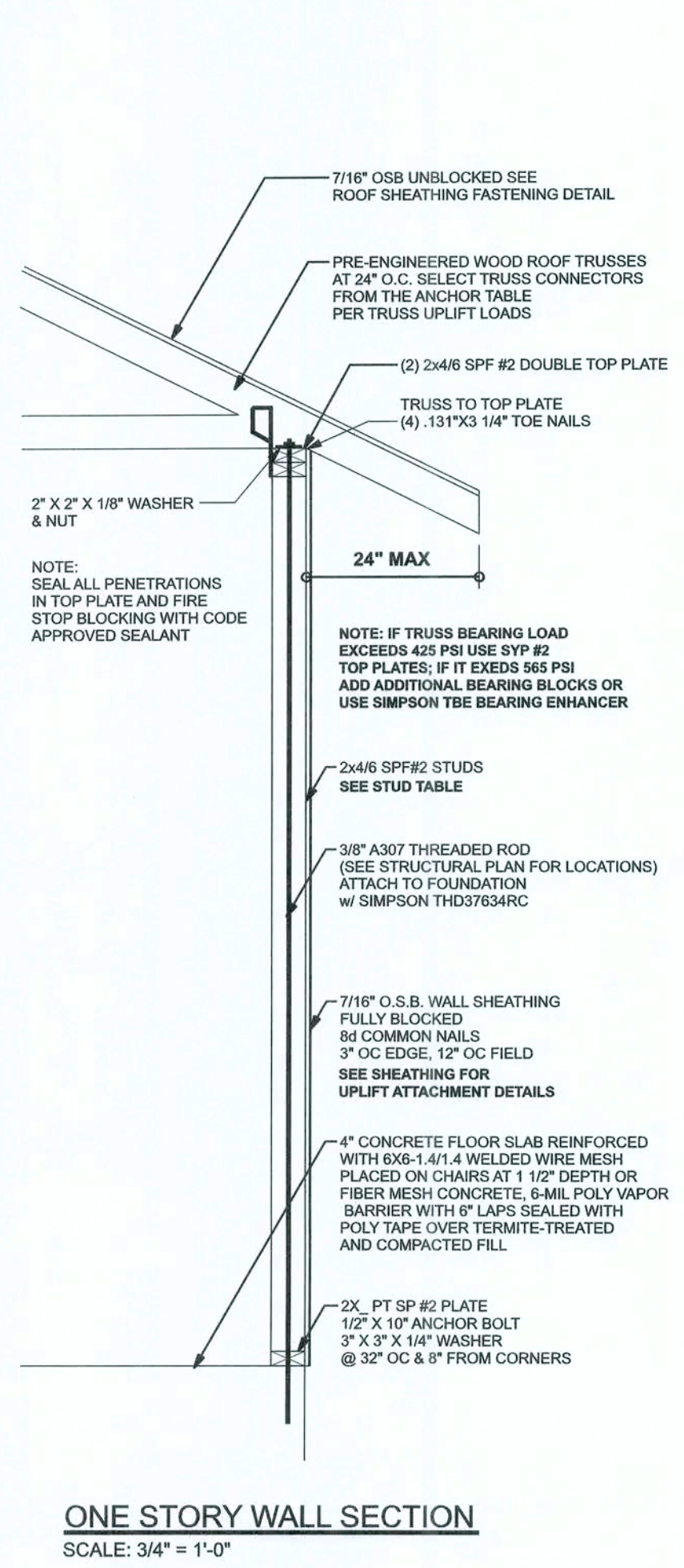
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JOB NUMBER
20191023

SHEET NUMBER
A.3
OF 3 SHEETS

Wm C. Myers



CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	-	-	
415	290	H3	4-8x1 1/2"	4-8x1 1/2"	
575	495	H2.5A	5-8x1 1/2"	5-8x1 1/2"	
1340	1015	H10A	9-10x1 1/2"	9-10x1 1/2"	
720	620	LTS12-20	6-10x1 1/2"	6-10x1 1/2"	
1000	860	MTS12-30	7-10x1 1/2"	7-10x1 1/2"	
1450	1245	HTS20-30	12-10x1 1/2"	12-10x1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	8-10x1	8-10x1	
1640	1455	MSTA24	9-10x1	9-10x1	
1030	1030	CS20	7-10x1	7-10x1	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
585	535	SP1	6-10x1	4-10x1	
1065	605	SP2	6-10x1	6-10x1	
771	771	LSTA24	10-10x1	wrap under or over plate	
1235	1235	LSTA24	14-10x1	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	18-16x2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Stud	Anchor	
2200	ABU44	12-16x1	5/8"x12" Drill & Epoxy		
2300	ABU66	12-16x1	5/8"x12" Drill & Epoxy		
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200	ABU44	12-16x1	5/8"x7" Drill & Epoxy		
2300	ABU66	12-16x1	5/8"x7" Drill & Epoxy		

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:			
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)			
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT		
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT		
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT		
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT		

GRADE & SPECIES TABLE			
		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBOR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f'_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.A.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2" INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO LEFT OR RIGHT OF WALL OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPICES 40" DB (25" FOR 8 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-88, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: 3-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

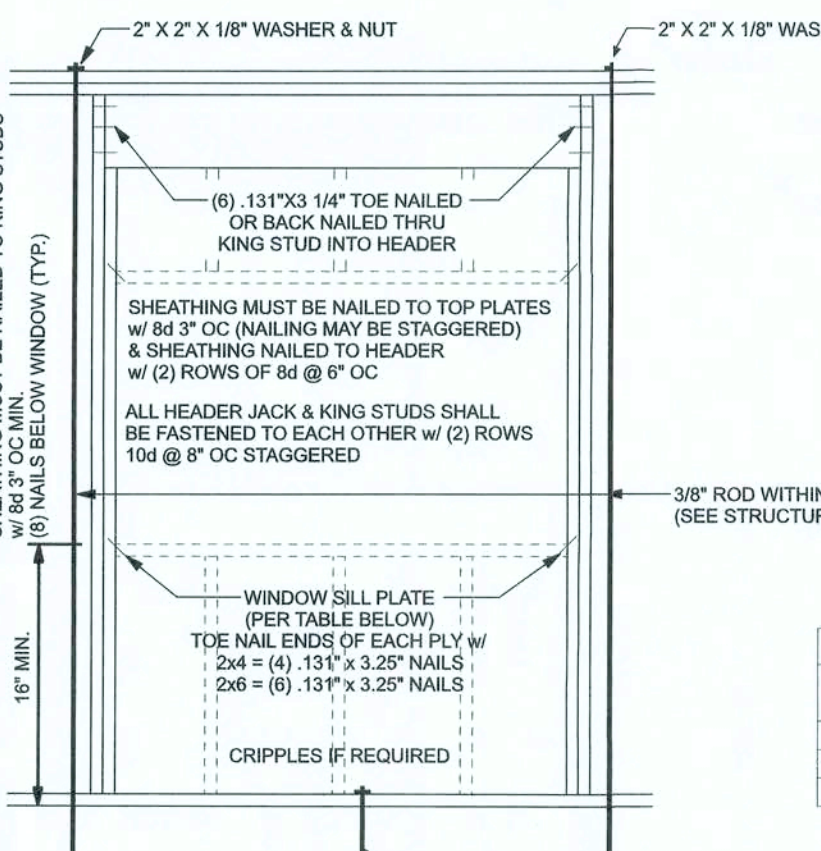
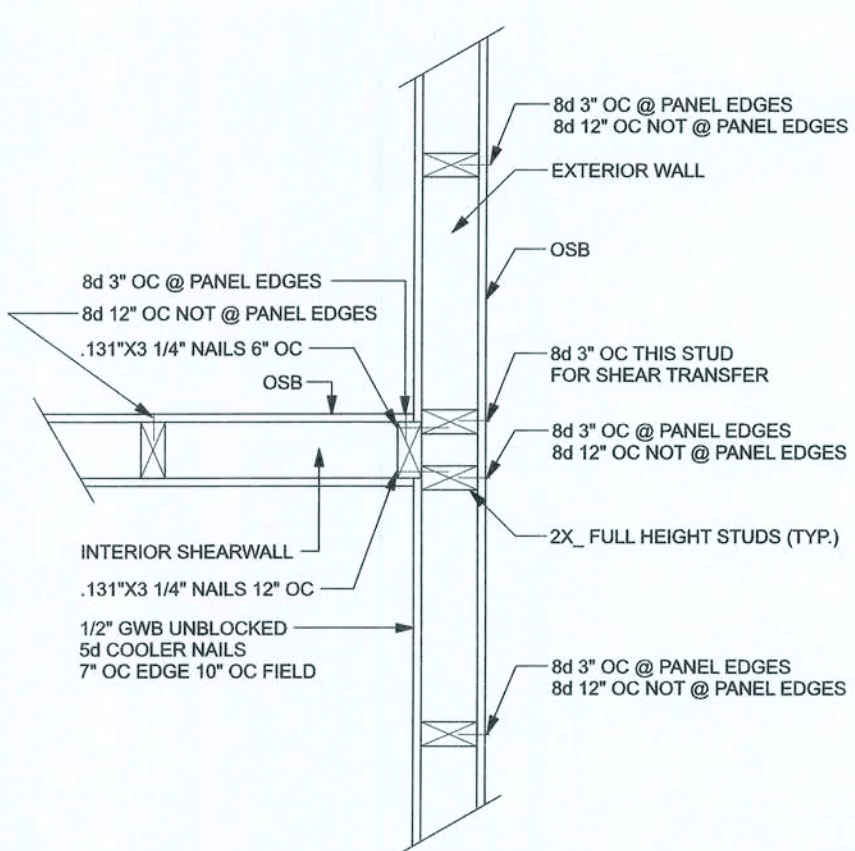
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBOR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBOR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESISTANT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

(TYP.) GABLE BRACING DETAIL WOOD FRAME

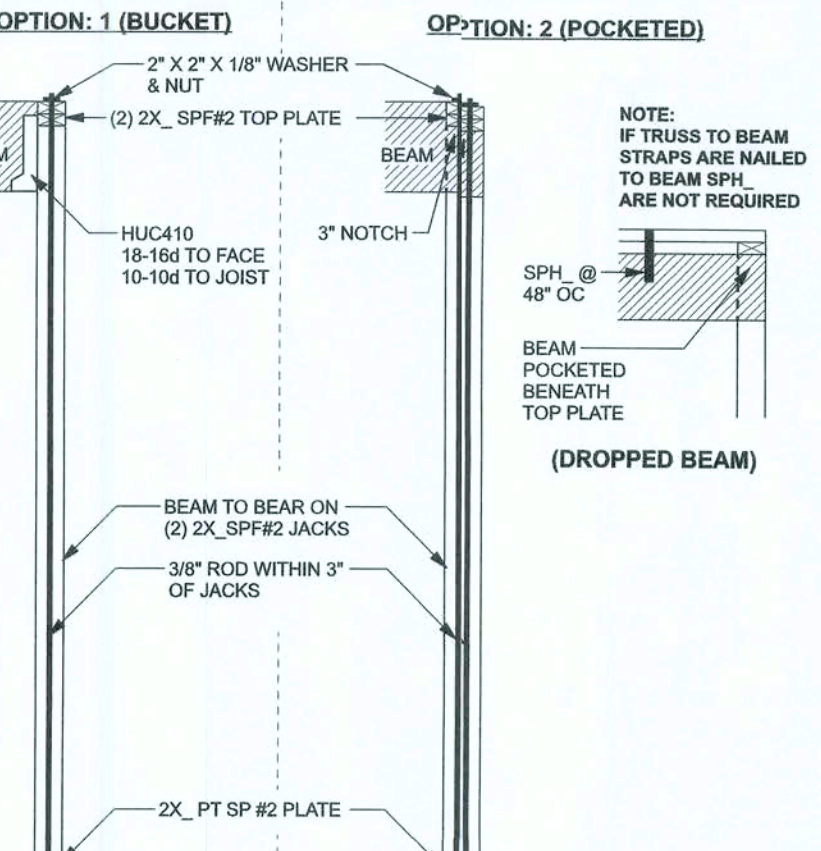
(TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME

(TYP.) PORCH POST ONE STORY WOOD



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT			
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE 3.23B	
	(1) 2x4	(2) 2x4	(1) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"
			11'-3"

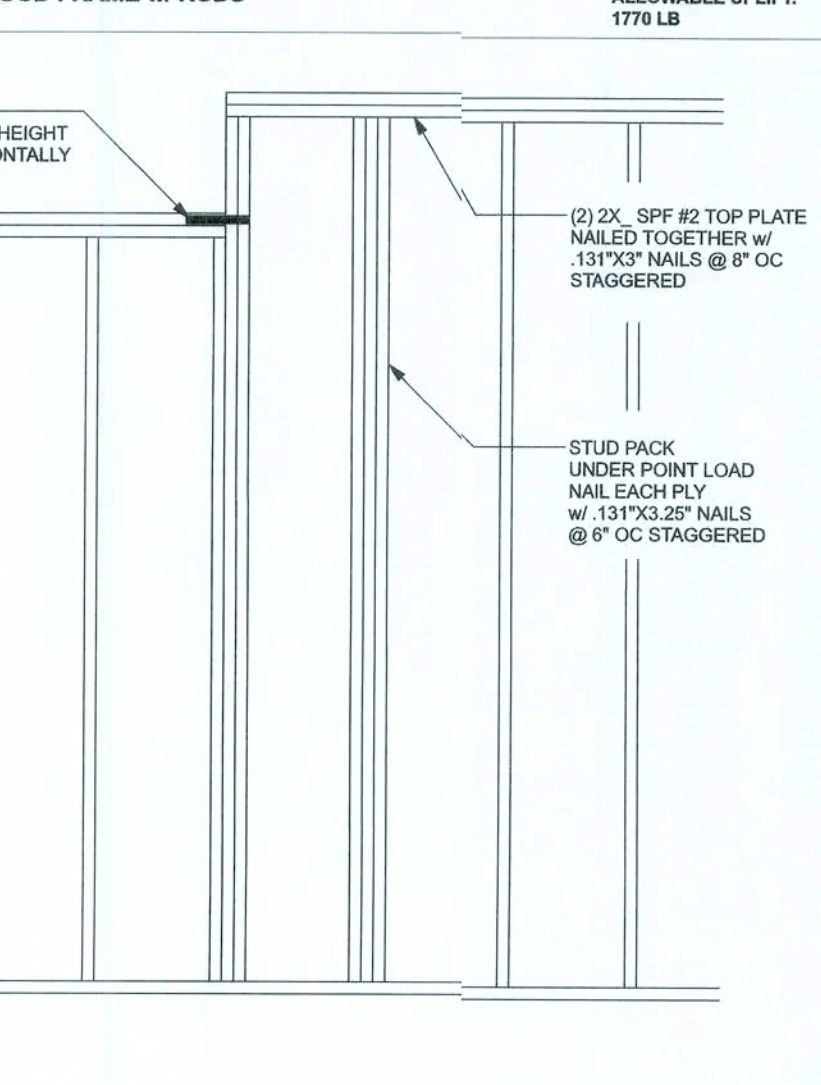
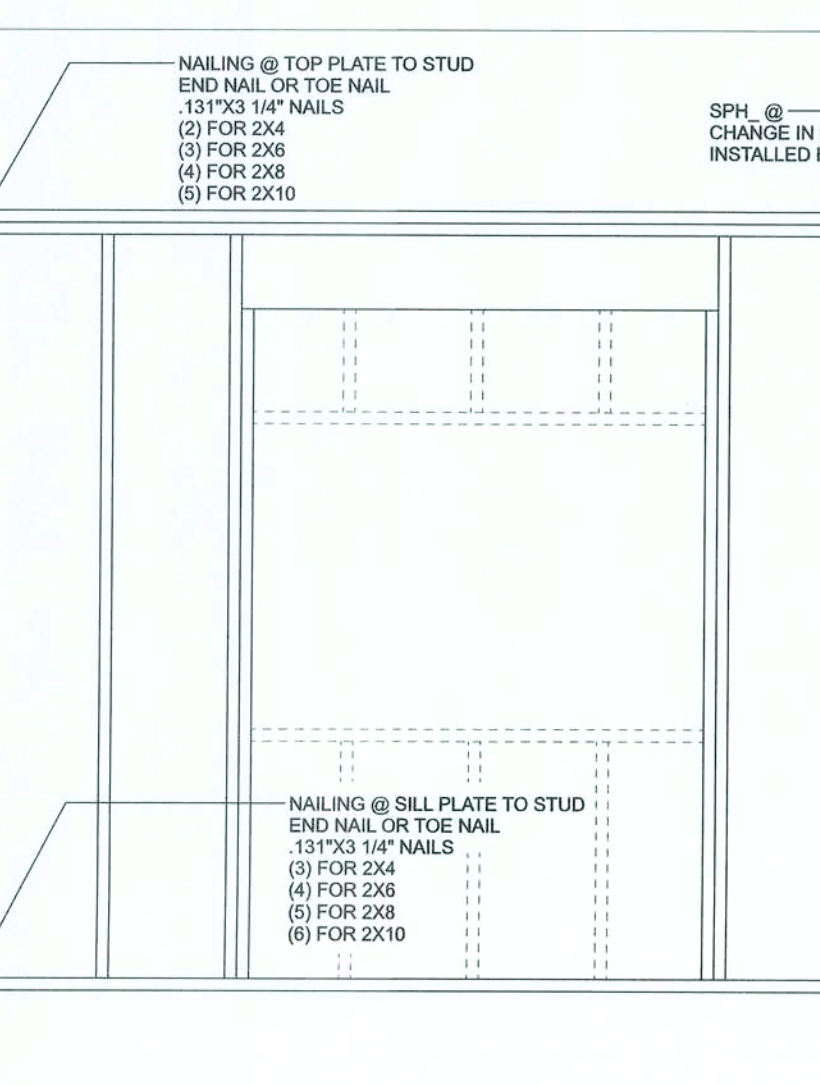
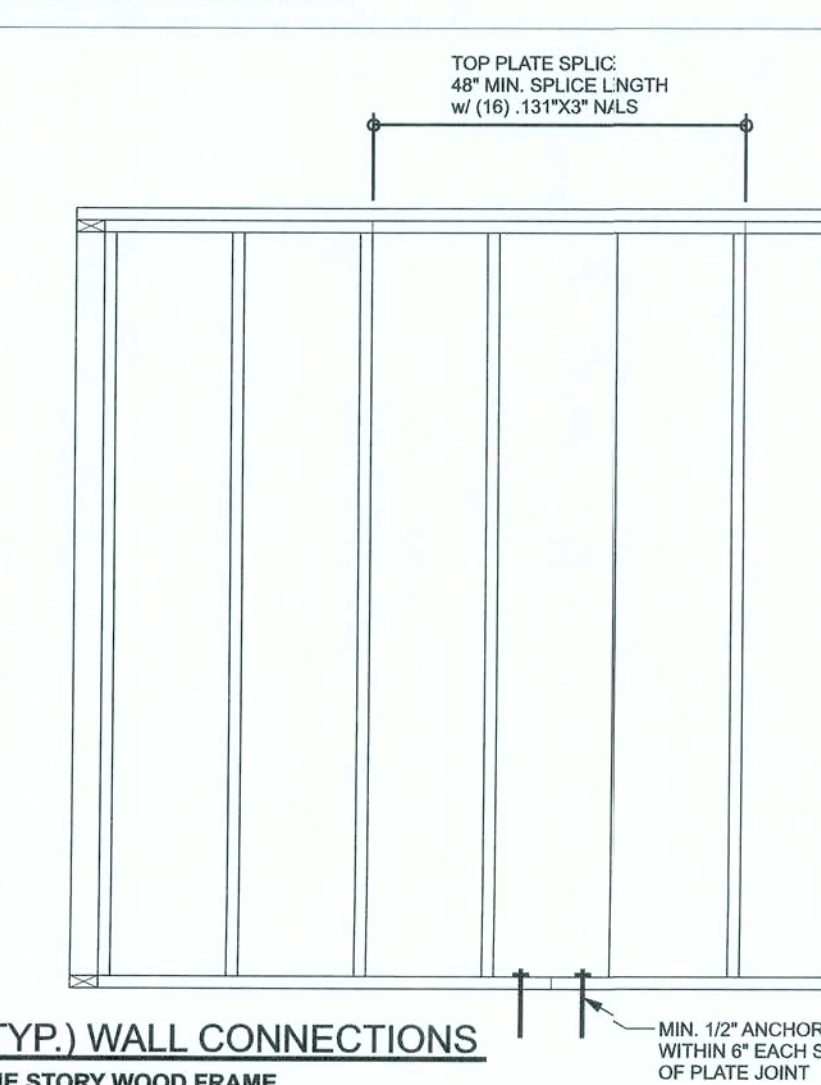
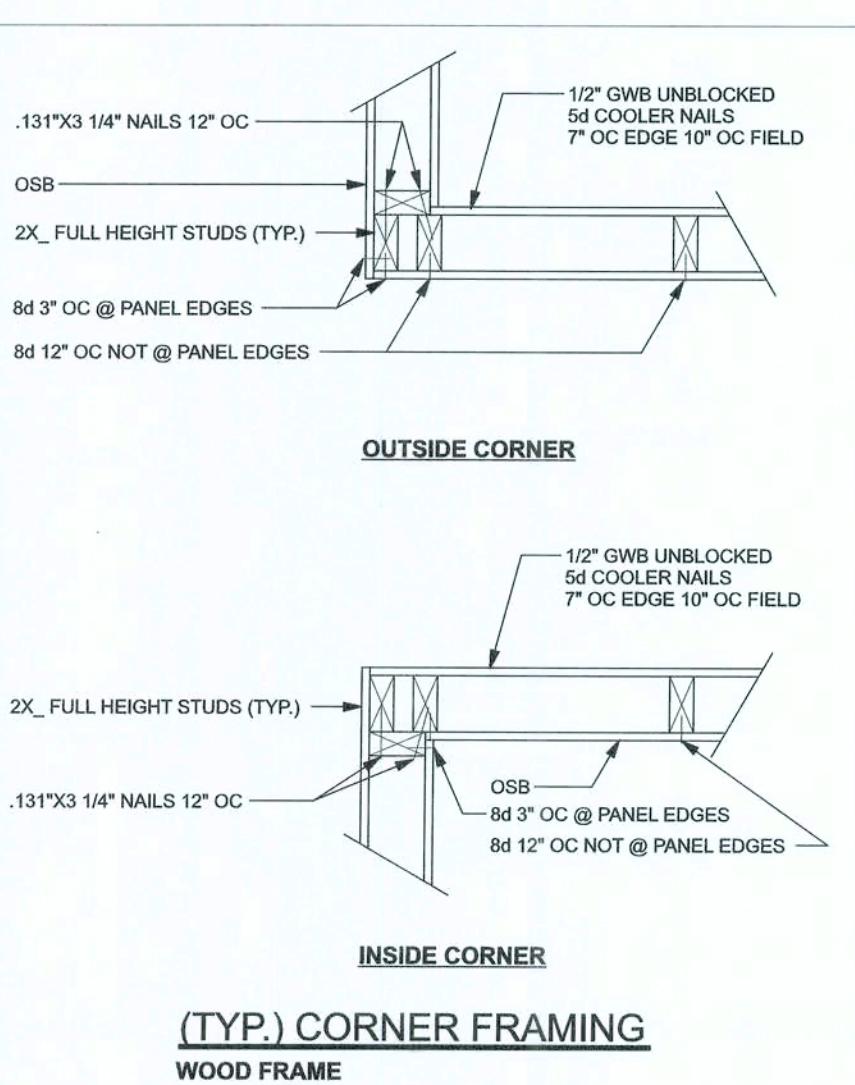
FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



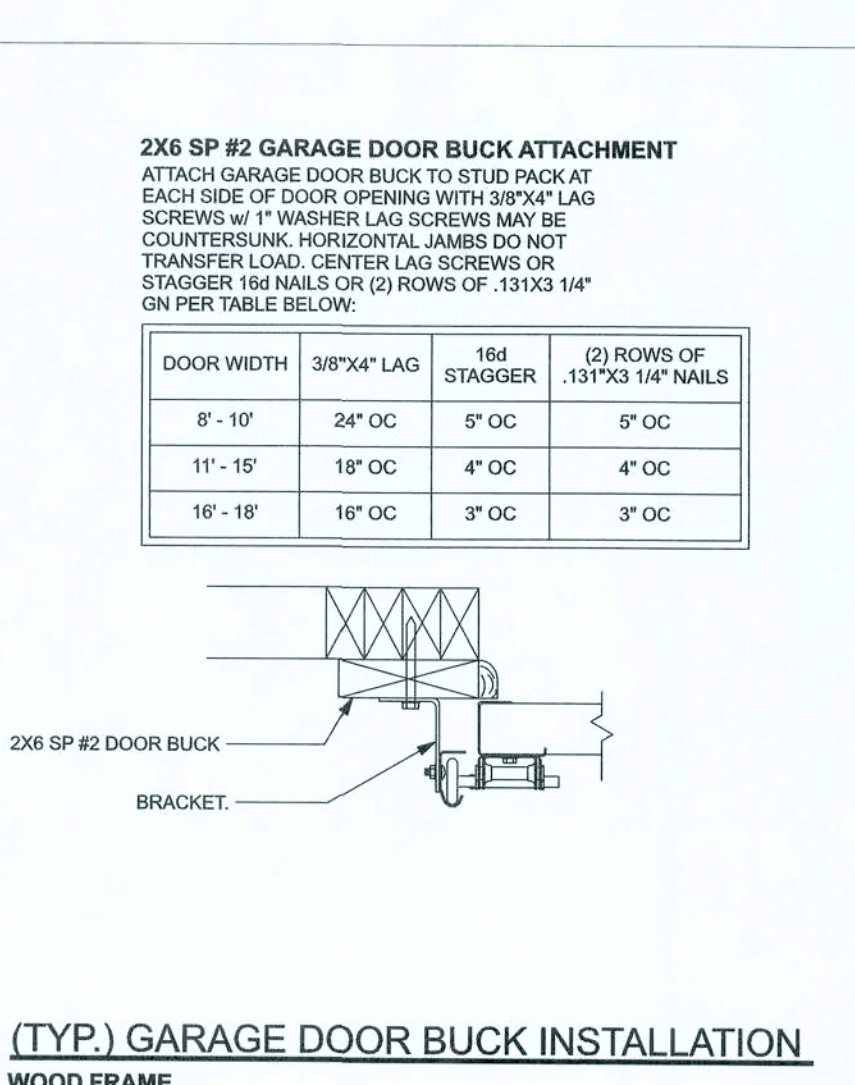
(TYP.) INTERSECTING WALL FRAMING WOOD FRAME

TYPICAL HEADER STRAPING DETAIL ONE STORY WOOD FRAME

(TYP.) BEAM TO WALL WOOD FRAME w/ RODS



2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT			
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 18d NAILS OR (2) ROWS OF 131X3 1/4" ON PER TABLE BELOW:			
DOOR WIDTH	3/8"x4" LAG	18d STAGGER	(2) ROWS OF 131X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION WOOD FRAME

DESIGN CRITERIA & LOADS:		
BUILDING CODE	6TH EDITION	FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10	
WINDLOADS		
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I	
RISK CATEGORY	II	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
ROOF ANGLE	7-45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
C&C DESIGN PRESSURES		
FLOOR LOADING		
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
ROOF LOADING		
PLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY	1500 PSF	
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE	

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) (Vuln)		
EFFECTIVE WIND AREA (FT2)	ZONE 1 INTERIOR	ZONE 5 END 4 FROM ALL OUTSIDE CORNER
0 - 20	+22.6	-26.2
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)		
8x7 GARAGE DOOR	+22.8	-25.5
16x7 GARAGE DOOR	+21.7	-24.1

Gibraltar Contracting, LLC

1763 Model - Lot 45 Rolling Meadows

PROJECT ADDRESS: Lot 45 Rolling Meadows, Columbia, FL

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

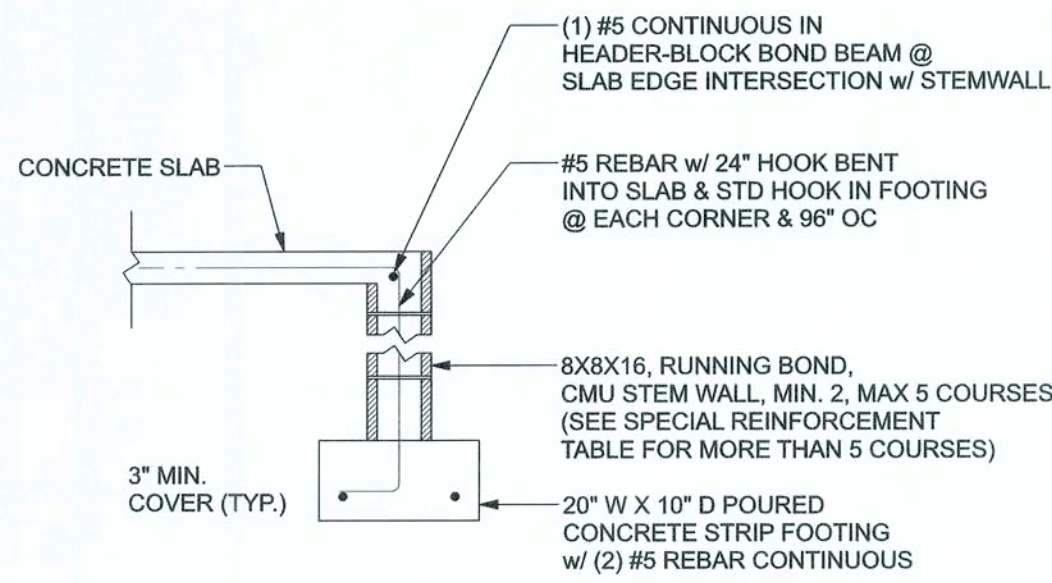
MARK DISOSWAY P.E. 63815

Wednesday, November 6, 2019

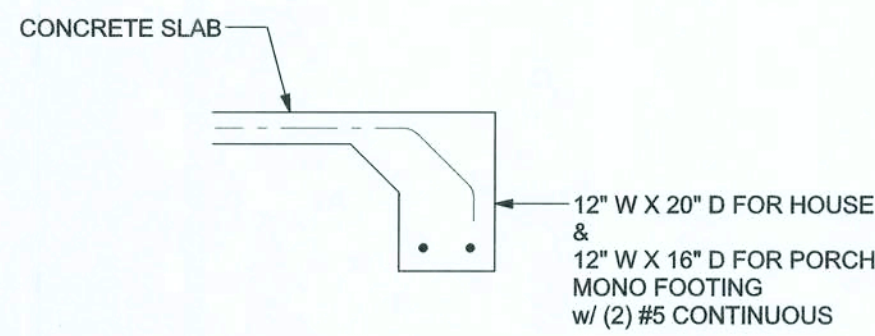
Mark Disosway P.E.
163 SW Niddtown Place
Suite 103
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386.754.5419
disoswaydesign@gmail.com

JOB NUMBER: 191233

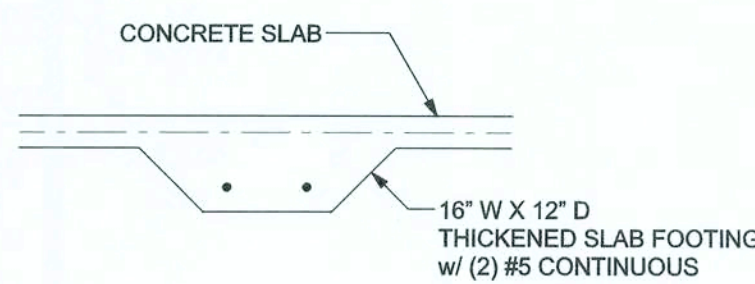
S-1
OF 3 SHEETS



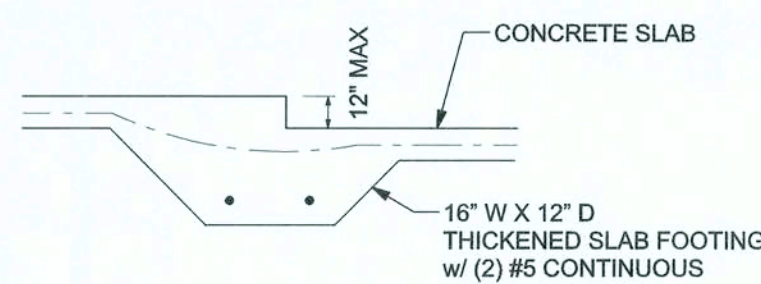
F1 S-2 OPTIONAL STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



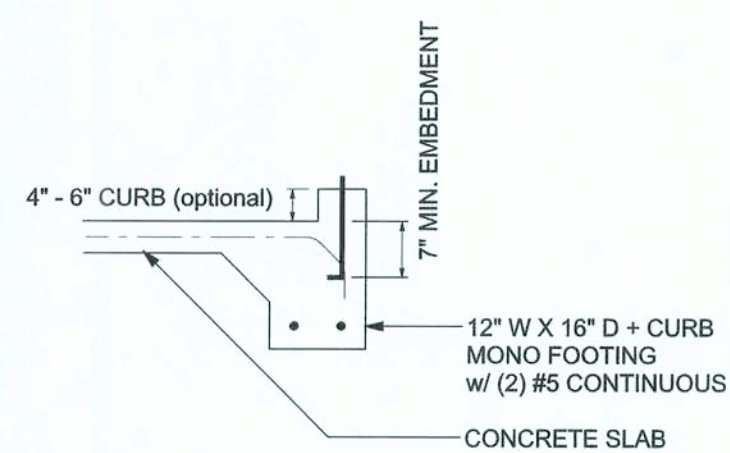
F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



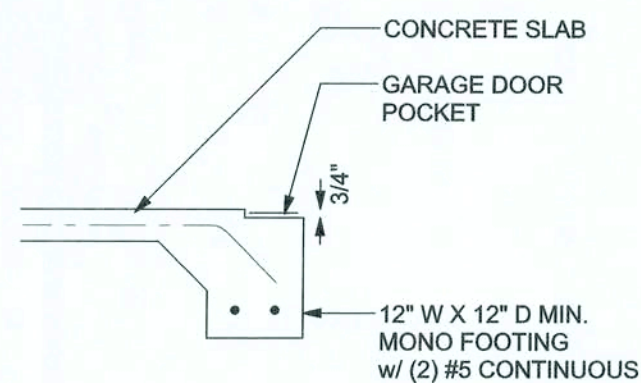
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"

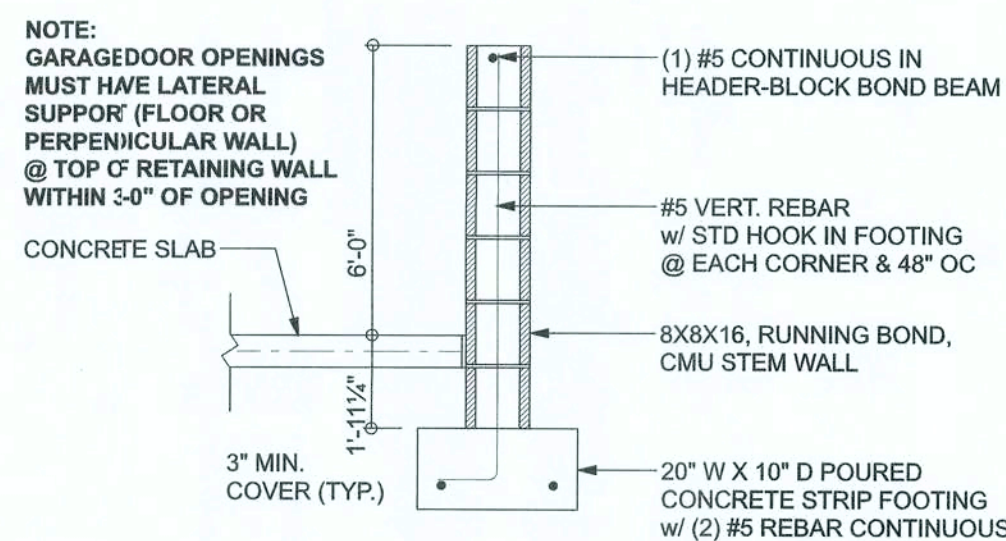
FULL STEM WALL TABLE:
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Diagonal ladder reinforcement at 16" OC vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

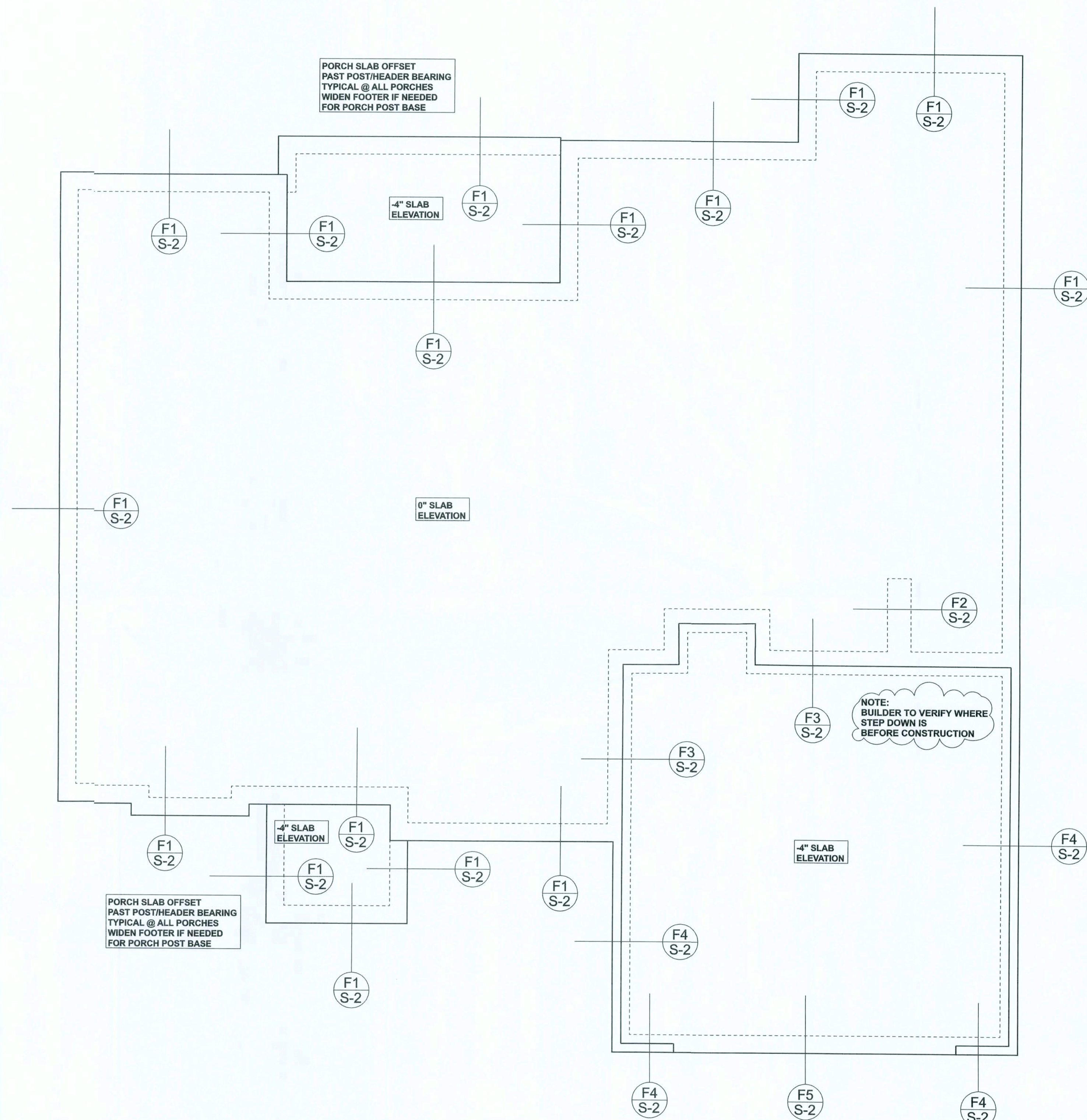
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
4A Compressive strength	8" block bearing walls Fm = 1500 psi
1 Mortar	ASTM C 270, Type N, UNO
2 Grout	ASTM C 476, admixtures require approval
3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 16"x16" running bond and 12"x12" or 16"x16" column block
3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2014-RES. SECTION R403.1.4



F4 S-2 OPTIONAL STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



Gibraltar Contracting, LLC

1763 Model - Lot 45 Rolling Meadows

PROJECT ADDRESS:
Lot 45 Rolling Meadows
Columbia, FL

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

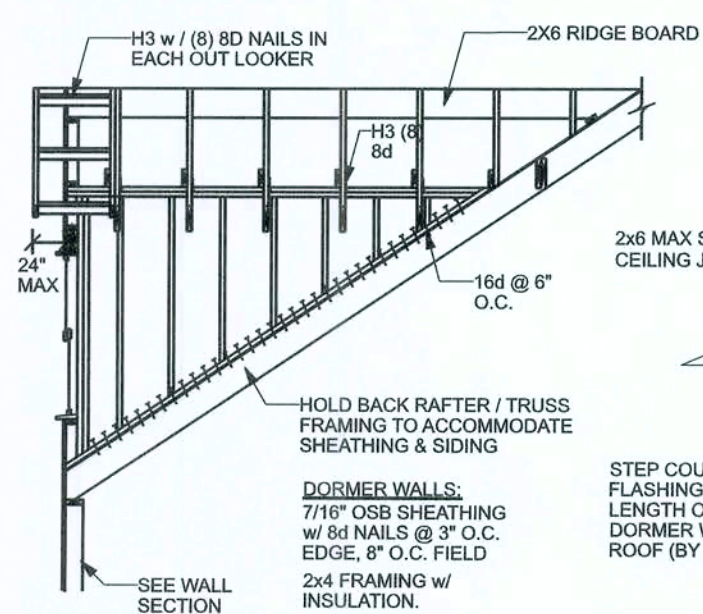
LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY, P.E.
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Lake City, Florida 32025
386.754.4419
disoswaydesign@gmail.com

Wednesday, November 6, 2019

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disoswaydesign@gmail.com

JOB NUMBER:
191233
S-2
OF 3 SHEETS



DORMER ROOF:
ROOFING (BY OWNER)

15lb. FELT OVER 7/16" OSB

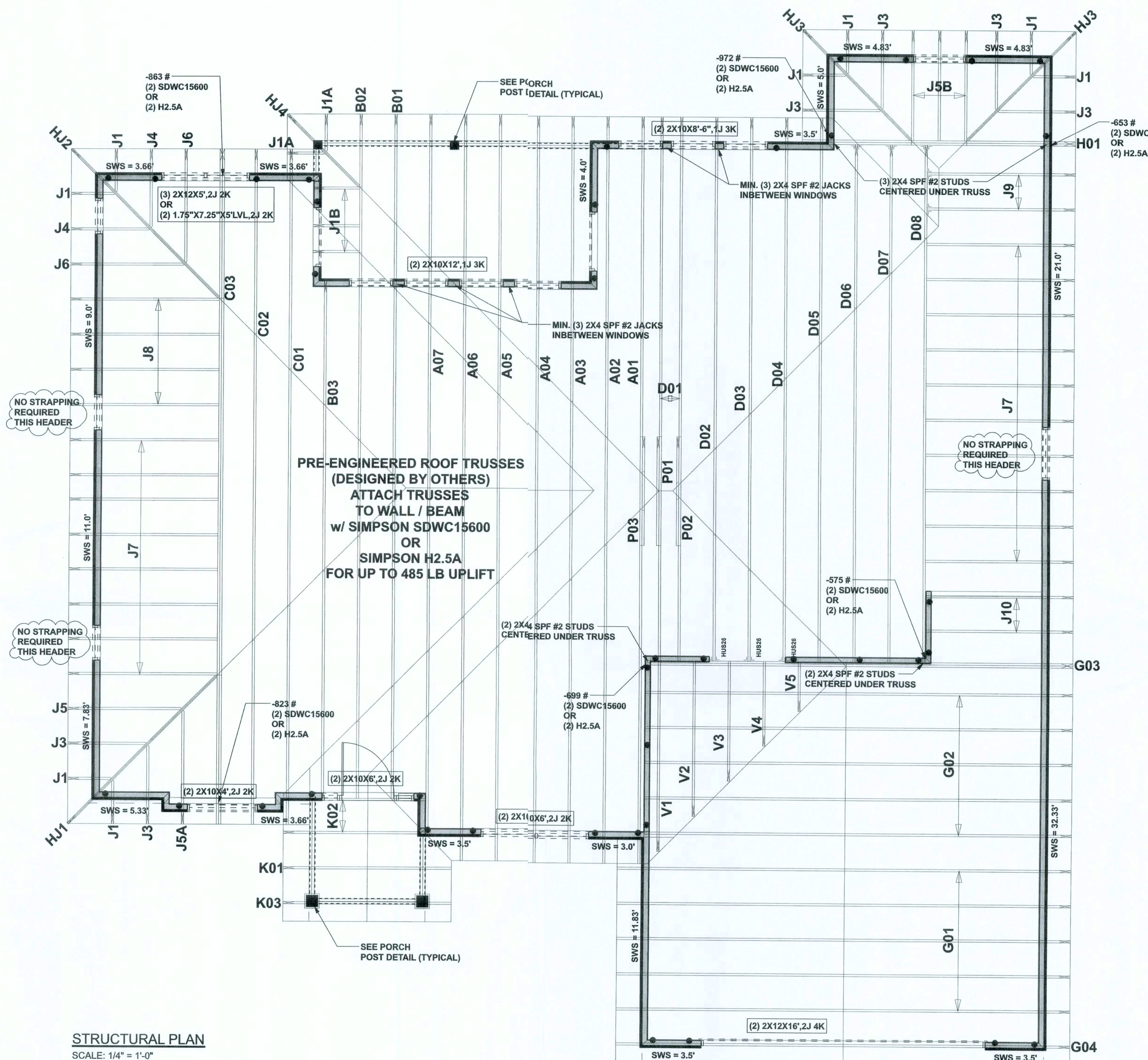
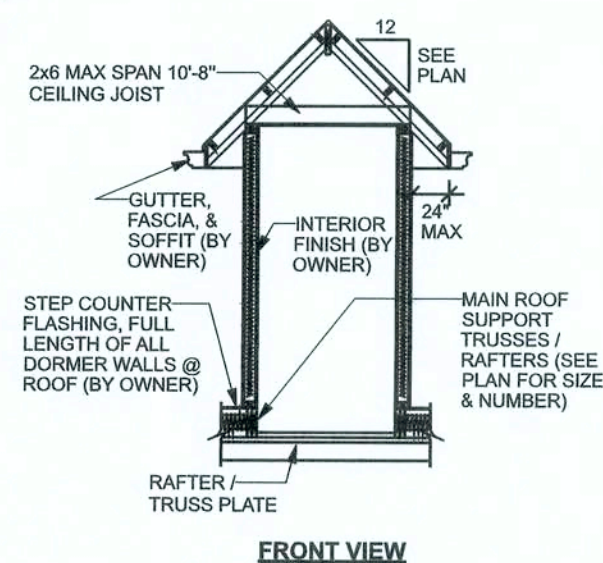
TRUSSES / RAFTERS (SEE
ROOF FRAMING PLAN FOR
SIZE & SPACING) TO RIDGE
BOARD (FULL DEPTH OF
RAFTER CUT).

CEILING JOISTS

CEILING FINISH (BY OWNER)

DORMER ANCHORING DETAIL (ON ROOF)
SCALE: N.T.S.

SCALE: N.T.S.



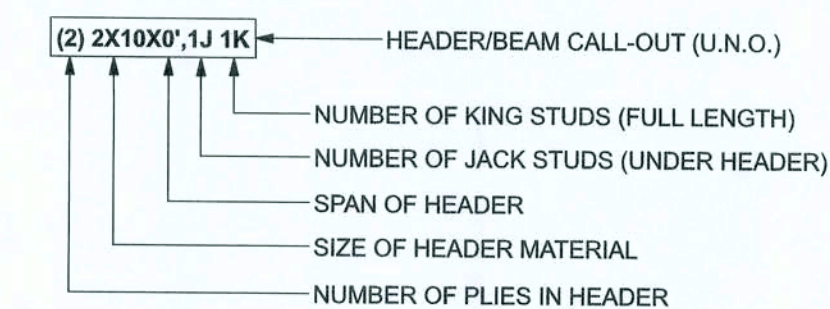
STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

SCALE: 1/4" = 1'-0"

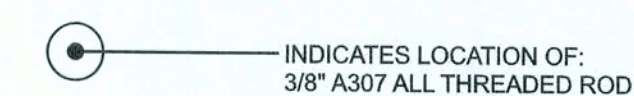
STRUCTURAL PLAN NOTES

- | | |
|------|--|
| SN-1 | ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.) |
| SN-2 | ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.) |
| SN-3 | USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD |
| SN-4 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS |
| SN-5 | PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC511-03, BC511-01, BC511-02, & BC511-03. BC511-01, BC511-02, & BC511-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE |

HEADER LEGEND



THREADED ROD LEGEND



ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	26174 LBF	14190 LBF
REQUIRED	15031 LBF	12593 LBF

Gibraltar Contracting, LLC

1763 Model - Lot 45 Rolling Meadows

PROJECT ADDRESS:
Lot 45 Rolling Meadows
Columbia, FL

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY PH.D. 2016

MALE DISHWASHER
No 5395
STATE OF
FLORIDA
PROFESSIONAL
N

Wednesday, November 1, 2019

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JOB NUMBER:
191233

S-3
OF 3 SHEETS

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO.
JOB #19-3670