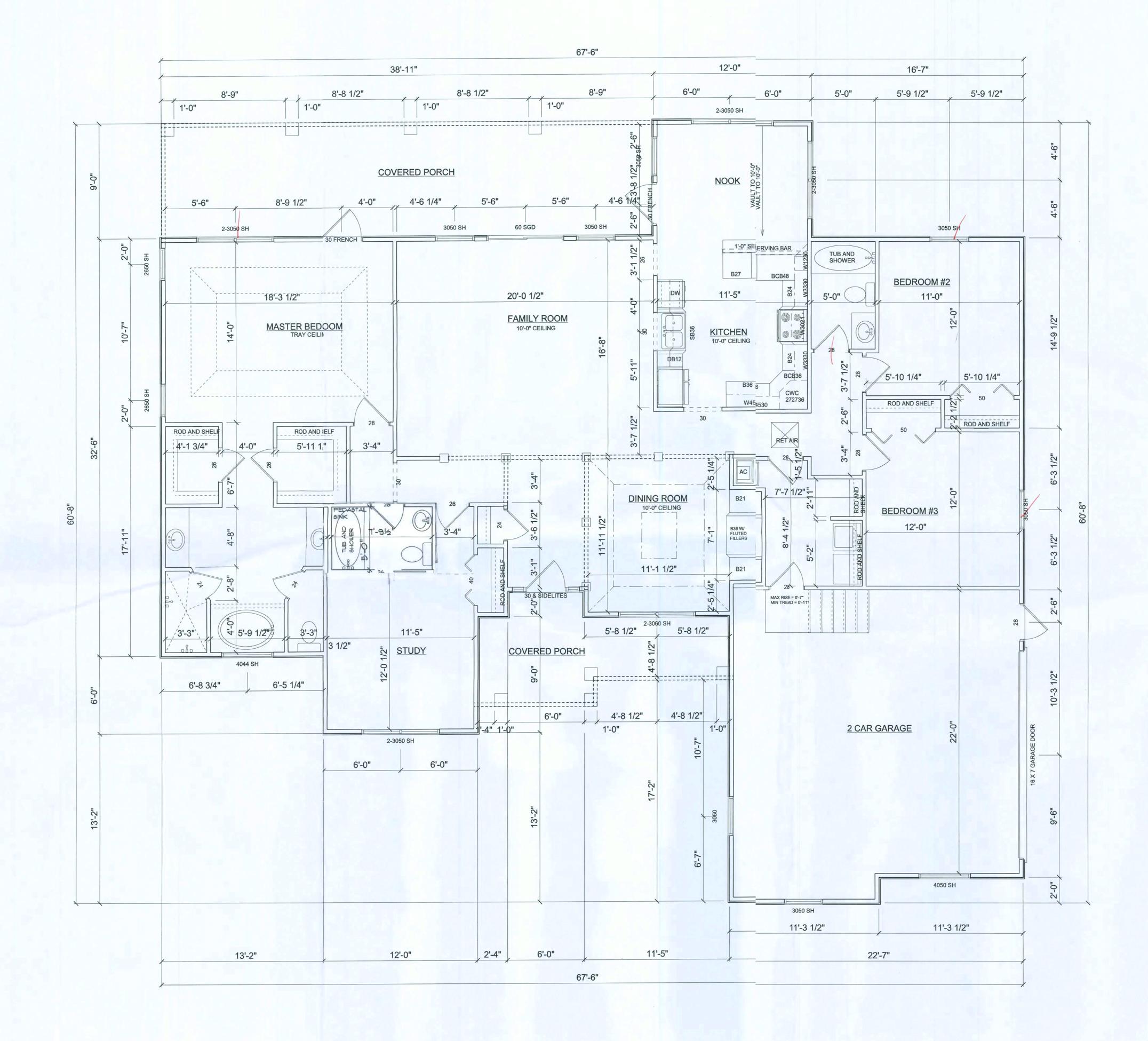


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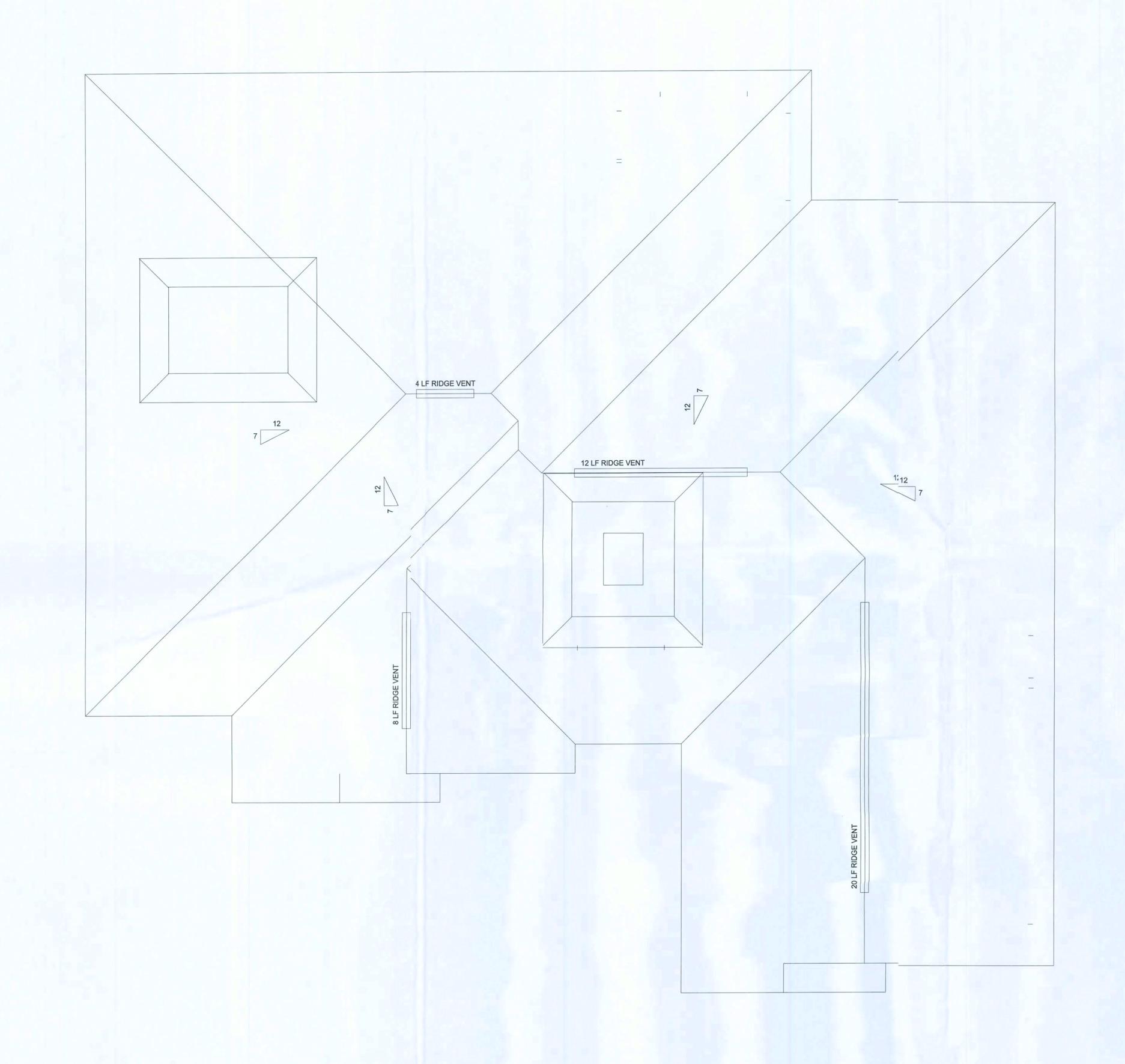
AREA SUMMARY

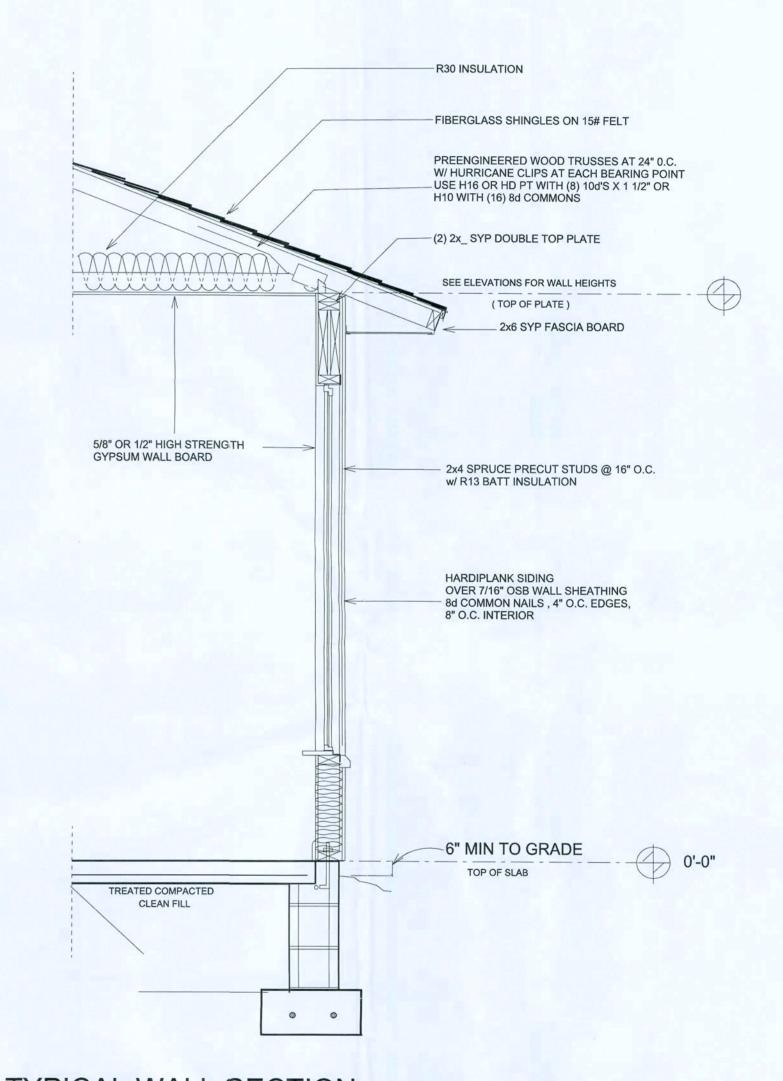
LIVING AREA - 2180.5 SF GARAGE - 532.6 SF PORCHES - 479.7 SF TOTAL AREA - 3192.8 SF

A-3

OF 4 SHEETS

AR0007005





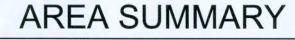
TYPICAL WALL SECTION

2 X 4 STUD WALL W/ SIDING

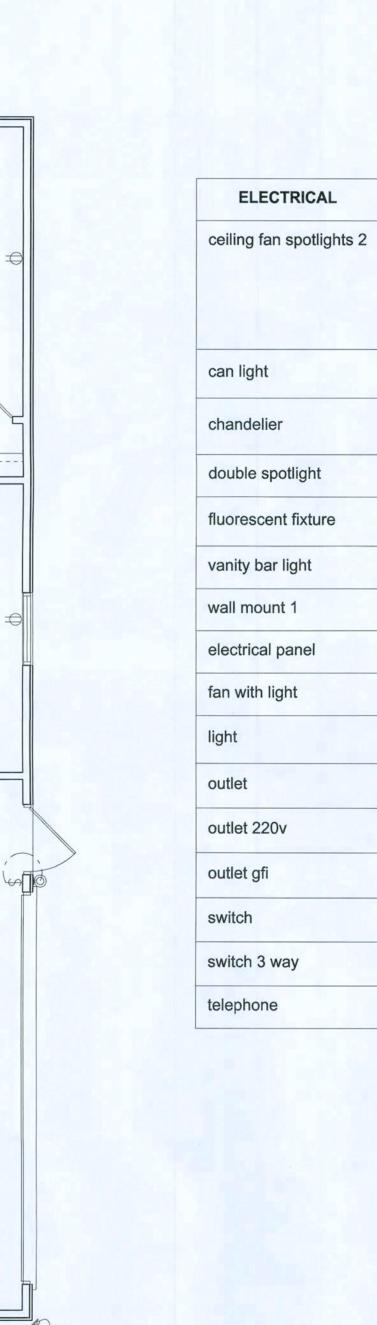
AREA SUMMARY

LIVING AREA - 2180.5 SF
GARAGE - 532.6 SF
PORCHES - 479.7 SF
TOTAL AREA - 3192.8 SF

AR0007005



LIVING AREA - 2180.5 SF GARAGE - 532.6 SF PORCHES - 479.7 SF TOTAL AREA - 3192.8 SF



WIRE ONLY FOR FLOOD LIGHTS COUNT

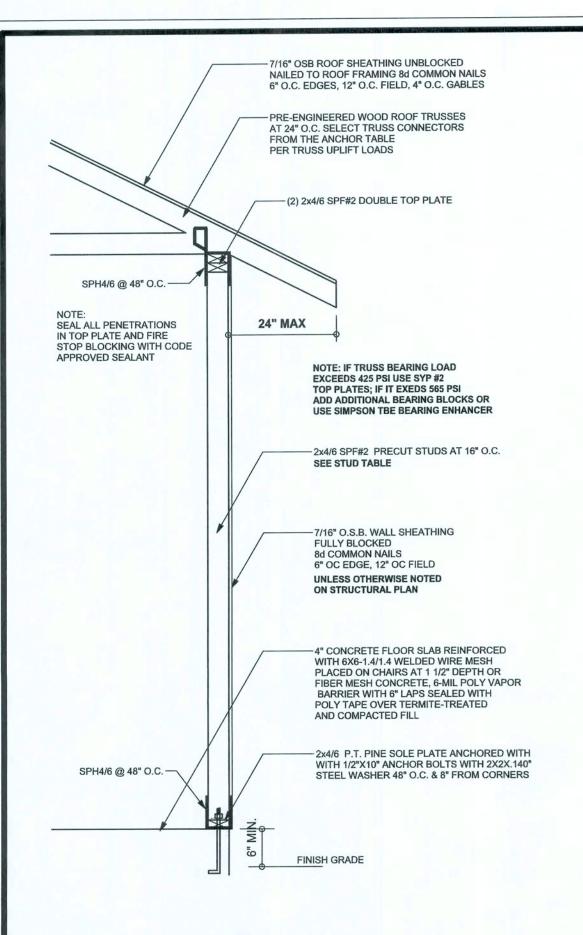
SYMBOL

TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

ELECTRICAL PLAN NOTES ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFIC CIRCUITS WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS. CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED. 00000 INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE. ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND 1--1 NEAR ALL BEDROOMS. TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION. 24 ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. 19 RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS. CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS

WIRE ONLY FOR FAN

WIRE ONLY FOR FLOOD LIGHTS



ONE STORY WALL SECTION

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTH RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

ANCHOR BOLT

TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

SEE FOOTING DETAILS

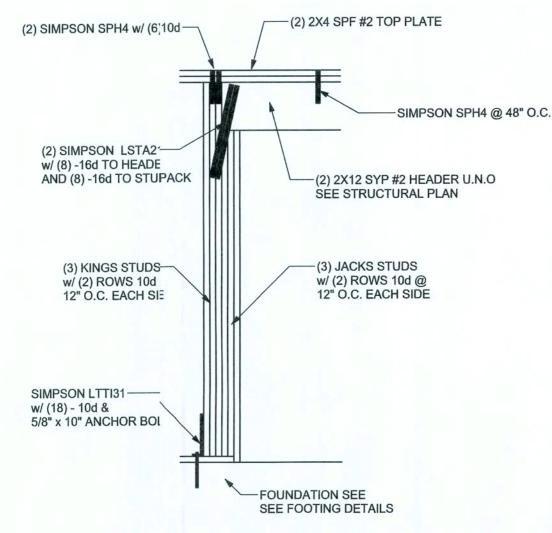
SIMPSON H2.5A U.N.O. -

SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/(8)-16d TO HEADER

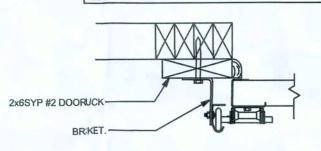
AND (8) -16d TO POST



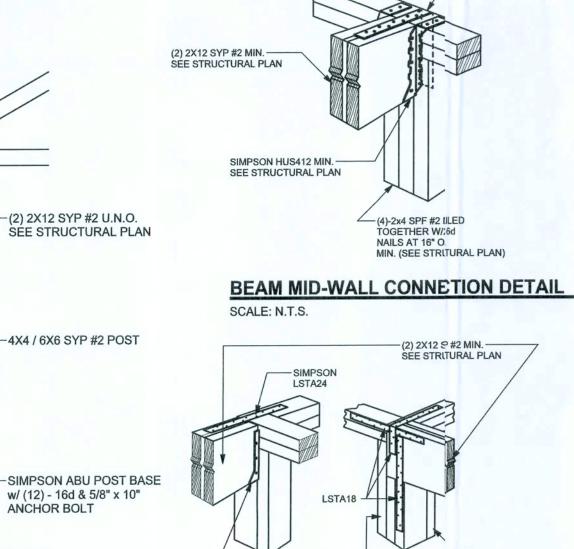
TYPICAL GARGE DOOR HEADER STRAPING DETAIL

26 SYP #2 GARAGE DOOR BUCK ATTACHMENT TACH GARAGE DOOR BUCK TO STUD PACK AT ICH SIDE OF DOOR OPENING WITH 3/8"x4" LAG REWS w/ 1" WASHER LAG SCREWS MAY BE **OUNTERSUNK, HORIZONTAL JAMBS DO NOT** 'ANSFER LOAD, CENTER LAG SCREWS OR AGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGIDOOR BUCK INSTALLATION DETAIL



BEAM W/4-16d

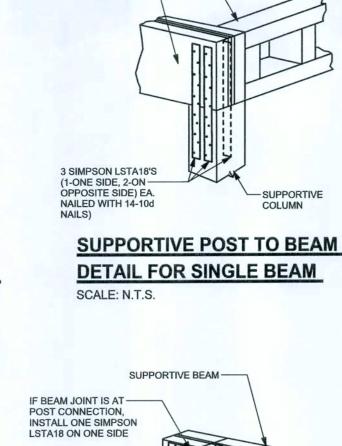
BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTIONDETAIL

-SIMPSON HUS412 MIN

SCALE: N.T.S.

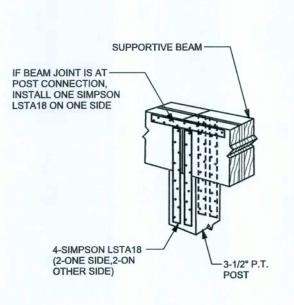
SEE STRUCTURAL PLAN



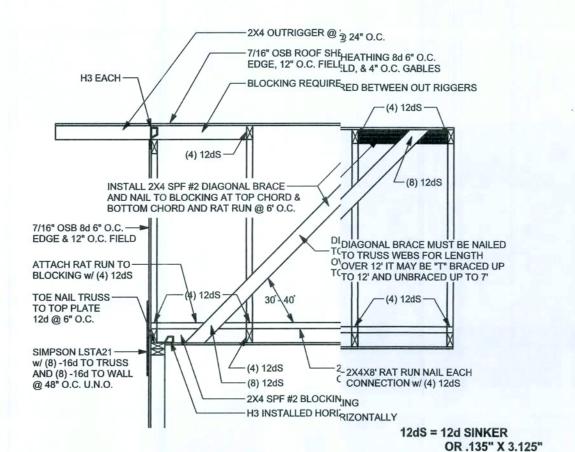
SUPPORTIVE

BEAMS

2X4 LADDER BEAM



SUPPORTIVE CENTER POST TO BEAM DETAIL

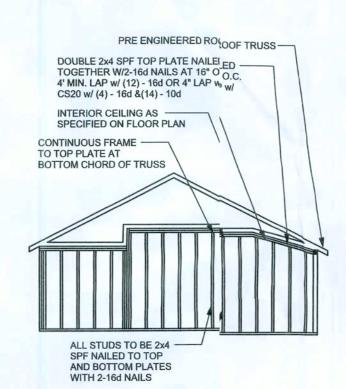


SPACE RAT RUN & DIAGONAL BRAACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 1 110 MPH, EXP. C, ENCLOSED GABLE BRACING DETTAIL

OR .131 X 3.25"

GRADE & SPECIES: TABLE

		Fb (p _(psi)	E (10 ⁶ psi)
2x8	SYP #2	120200	1.6
2x10	SYP #2	105)50	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	240,100	1.8
LSL	TIMBERSTRAND	170,00	1.7
LVL	MICROLAM	160300	1.9
PSL	PARALAM	290300	2.0



CONTINUOUS FRAAME TO CEILING DIAPHRAGM DETAIL

-SPH4/6 6 ALL OPENINGS (U.N.O.)-

CRIRIPPLES IF REQUIRED

(4) .13'31 x 3 1/4" GUN NAILS - TOE E NAILED THRU SILL -

INTO O JACK STUD U.N.O.

TYPIC CAL STRAPPING (U.N.O.) (SEE S STRUCTURAL PLAN)

-SPH4/6 6 ALL OPENINGS (U.N.O.)-

(1) 2X6 SPF _F #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF>_F #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MP|PH, 10'-0" WALL HIGHT U.N.O.)

SPH4/6 @ 48" O.C. (U.N.O.)

IF TRUSS TO WALL STRAPS ARIRE NAILED TO THE HEADER THE SPH4/6 @ 2 48" O.C.

ARE NOT REQUIRED

(6) .131 x 3 1/4" GUN NAILS --

INTO KING STUD

TOE NAILED THRU HEADER

GENERAL NOTES:

CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

BUILDER'S RESPONSIBILITY

THE WIND LOAD ENGINEER IMMEDIATELY.

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

(6) .131 x 3 1/4" GUN NAILS

INTO KING STUD

TOE NAILED THRU HEADER

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND

PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR

ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER

AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.

FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD

PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN

CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO

ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH

WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT

OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO, SUPPLIER MAY SUPPLY AN

APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004

SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION

R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN

TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS

THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE

COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS

MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO

REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF

BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF

TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES.

RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE

SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERA

BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED,

STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT

> **MASONRY NOTES:** MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approva		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet me ties not completely embedded in mortar grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.		

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

72.	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24		100 1116		
< 2050	< 1785	LGT2	14 -16d	14 -16d		
			, 1. Tou	14-100		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d			
< 2310	< 2310	LTTI31			1/2" AB	
< 2775	< 2570		18-10d, 1 1/2"		1/2" AB	
< 4175		HD2A	2-5/8" BOLTS		5/8" AB	
	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d			
< 3335	< 3335	HPAHD22	16-16d			
< 2200	< 2200	ABU44	12-16d		1/2" AB	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB	

Zone Effective Wind Area (ft2)

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH 2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

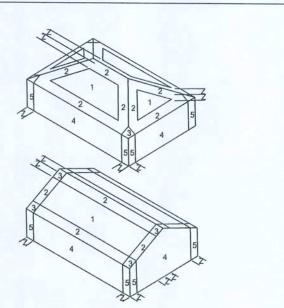
BUILDING CATEGORY = II

DESIGN DATA

5.) ROOF ANGLE = 10-45 DEGREES 6.) MEAN ROOF HEIGHT = <30 FT

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

		1	0		100
	1	19.9	-21.8	18.1	-18.1
	2	19.9	-25.5	18.1	-21.8
	2 O'hg		-40.6		-40.6
11	3	19.9	-25.5	18.1	-21.8
	3 O'hg		-68.3		-42.4
	4	21.8	-23.6	18.5	-20.4
	5	21.8	-29.1	18.5	-22.6
	Doors	& Wind		21.8	-29.1
	(Zone				
	8x7 Gar	age D	oor	19.5	-22.9
	16x7 Ga	rage [Door	18.5	-21.0
2					
R DWELLING ROOMS)					
POOLIO					

DESIGN	LOADS
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
1901	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)

REVISIONS

SOFPIAN

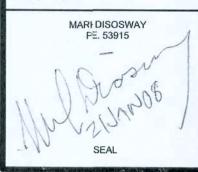
VINDLOAD ENGINEER: Mark Disosway PE No.53915, POB368, Lake City, FL 32056, 386-754-549 Stated dimensions spercede scaled

mensions. Refer a questions to Mark Disosway, P.E for resolution

Do not proceed withut clarification OPYRIGHTS ANDPROPERTY RIGHTS Mark Disosway, P.E hereby expressly reserv ts common law coprights and property right in ese instruments o service. This document not to be reproduce, altered or copied in any orm or manner without first the express written nission and conent of Mark Disosway.

mined this plan, and that the applicable portions of the plan, elating to wind engineeri comply with section R301.2.1, florida building e residential 200, to the best of my

IMITATION: This design is valid for one ouilding, at specified location.



Stanley Crawford Construction

J.L. Diccs Residence

AIDRESS: Price Creek Rd.

ColumbiaCounty, Florida

Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (336) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: Januay 21, 2008 DRAWN BY: CHECKED BY: avid Disosway

FINALS DATE: 21 / Jan / 08 JOB NUMBER: 801181

DRAWING NUMBER

OF (SHEETS

