DATE 02/20/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000024155

APPLICANT AARON SIMQUE PHONE 755-0841

ADDRESS P.O. BOX 2183 LAKE CITY FL 32056

OWNER AARON SIMQUE PHONE 755-0841

ADDRESS 286 SW AUDREY WAY LAKE CITY FL 32024

CONTRACTOR AARON SIMQUE PHONE 755-0841

LOCATION OF PROPERTY 47S, TR ON CR 242, TR ON ARROWHEAD, TL ON CANNON CREEK, TR O
2ND LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 74250.00

HEATED FLOOR AREA 1485.00 TOTAL AREA 2131.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RR MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 24-4S-16-03103-010 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES

000000977 | Culvert Permit No.
Culvert Waiver
Contractor's License Number
Applicant/Owner/Contractor

CULVERT | Driveway Connection
06-0090-N | Septic Tank Number
BK | LU & Zoning checked by
JH | Approved for Issuance
New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD

Check # or Cash CASH

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by | Foundation date/app. by | Monolithic date/app. by

Under slab rough-in plumbing date/app. by | Slab date/app. by | Sheathing/Nailing date/app. by

Framing date/app. by | Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by | Heat & Air Duct date/app. by | Peri. beam (Lintel) date/app. by

Permanent power date/app. by | C.O. Final date/app. by | Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by | Pool date/app. by

Reconnection date/app. by | Pump pole date/app. by | Utility Pole date/app. by

M/H Pole date/app. by | Travel Trailer date/app. by | Re-roof date/app. by

BUILDING PERMIT FEE $ 375.00 CERTIFICATION FEE $ 10.65 SURCHARGE FEE $ 10.65

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 496.30

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Audrey Way

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0601-41 Date Received 1.31.06 By CP Permit # 27124155

Application Approved by - Zoning Official BLK Date 03.02.06 Plans Examiner OKJTH Date 2-15-06

Flood Zone X Development Permit N/A Zoning RR Land Use Plan Map Category RES. U.L.D.

Comments Noc and ~~Ent~~ missing

Applicants Name Melanie Roder Phone 752-2281

Address 387 SW Kemplet Lake City. FL 32024

Owners Name Aaron Simque homes Phone 755-0841

911 Address 286 SW Audrey Lane Lake City, FL 32024

Contractors Name Aaron Simque Homes Phone 755-0841

Address P.O.BOX 2183 Lake City, FL 32056

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address Will Myers / Nick Geisler

Mortgage Lenders Name & Address Columbia County Bank

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energy

Property ID Number 24-4S-16-03103-010 Estimated Cost of Construction 70,000

Subdivision Name NA Lot 11 Block ____ Unit ____ Phase ____

Driving Directions 47 S. Turn right on County rd 242, Turn right on Arrowhead rd. Turn left on Cannon Creek rd. Turn right on Audrey, 1st on left 2nd

Type of Construction SFD Number of Existing Dwellings on Property 0

Total Acreage 1.00 Lot Size _____ Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 50' Side 80.9 Side 80.9 Rear 121'

Total Building Height 18.5' Number of Stories 1 Heated Floor Area 1485 Roof Pitch 6-12

Porches 238 GARAGE 408 TOTAL 2131

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Linda R. Roder
Commission #DD303275
Expires: Mar. 24, 2008
Bonded Thru Atlantic Bonding Inc.

Sworn to (or affirmed) and subscribed before me this 8th day of Jan 20 06.

Personally known ____ or Produced Identification ____

Contractor Signature

Contractors License Number BB 2900 3130

Competency Card Number ____

NOTARY STAMP/SEAL

Linda Roder

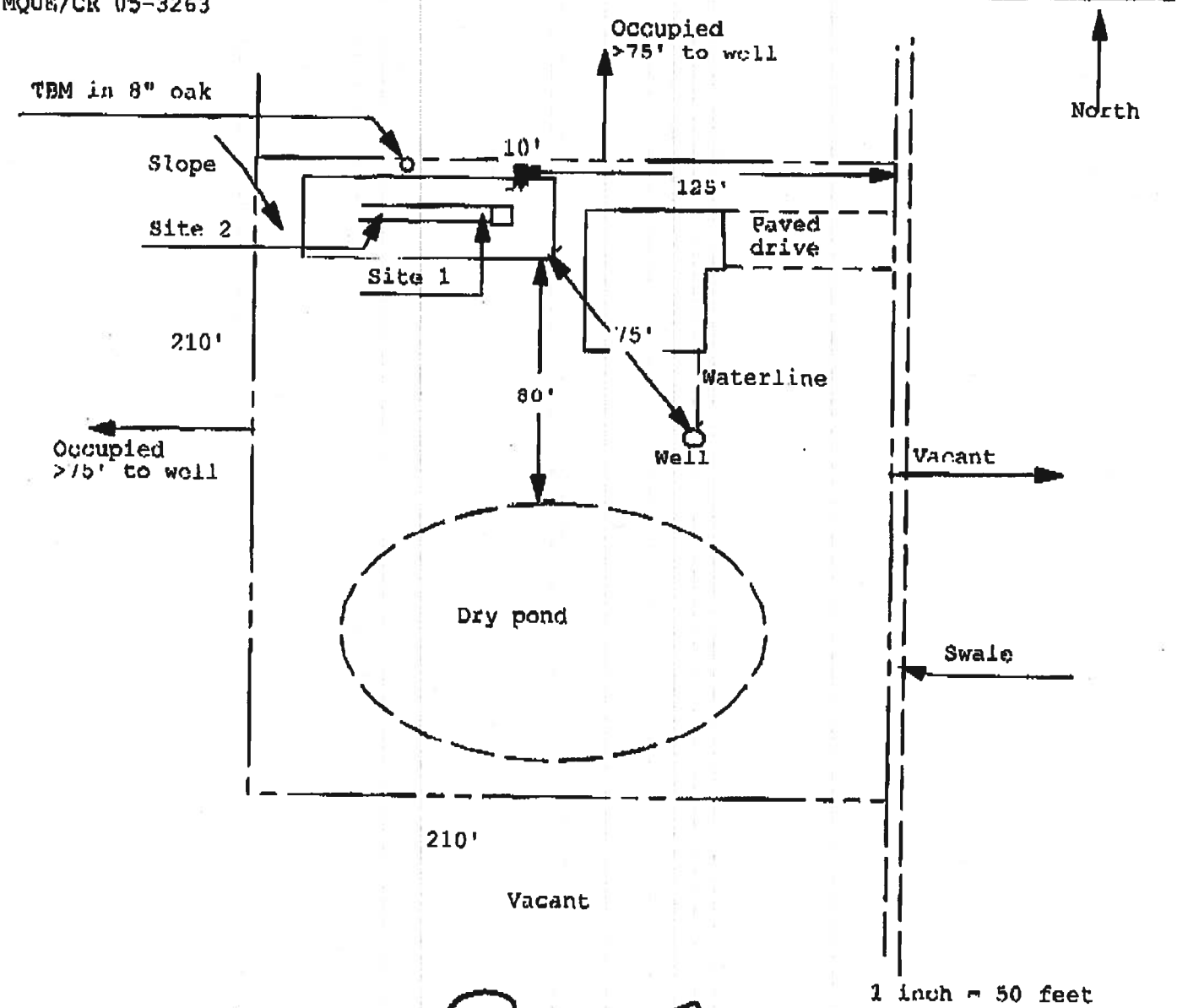
Notary Signature

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0090N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

SIMQUE/CR 05-3263

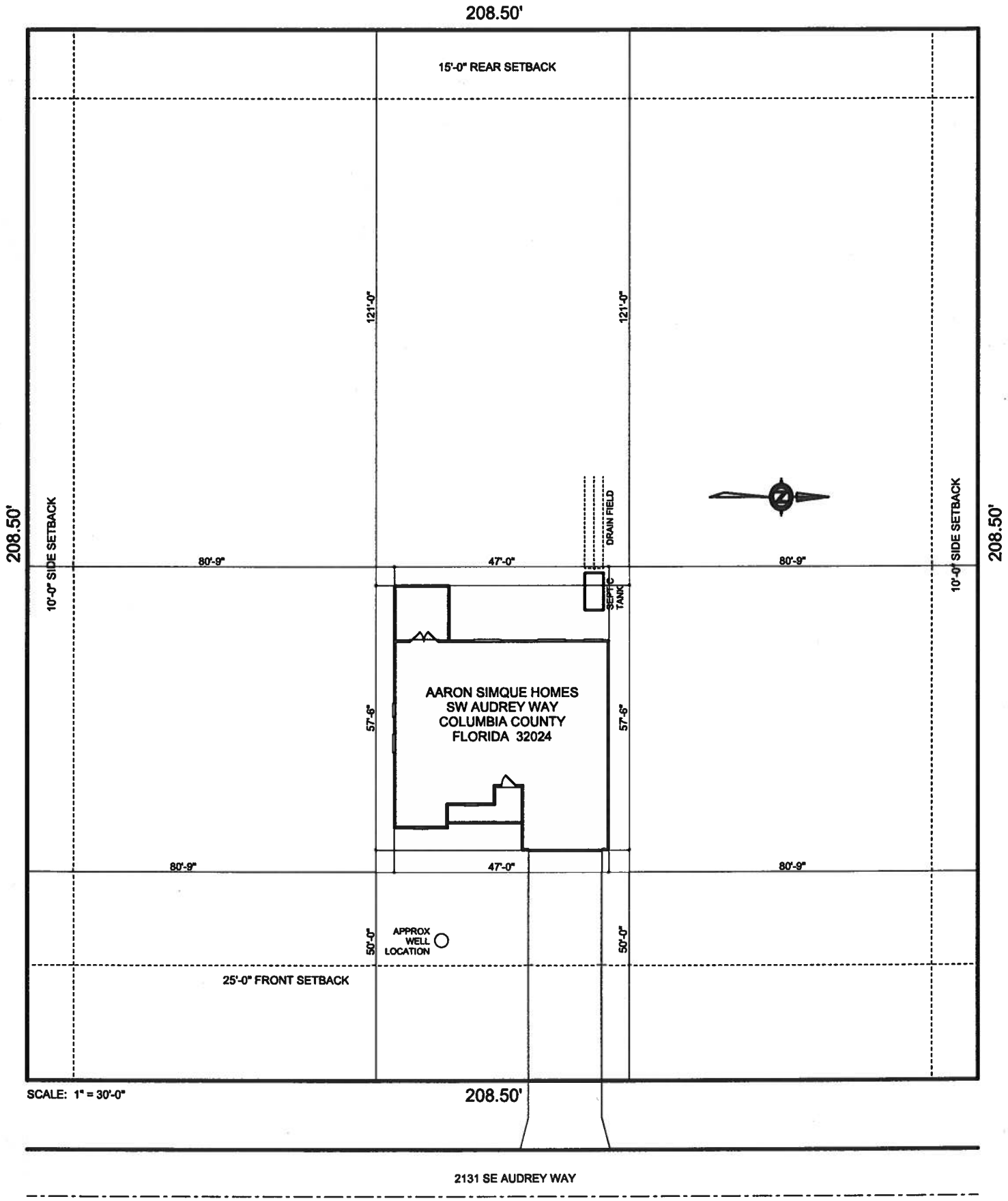


Site Plan Submitted By Paul Hoff Date 12/18/05

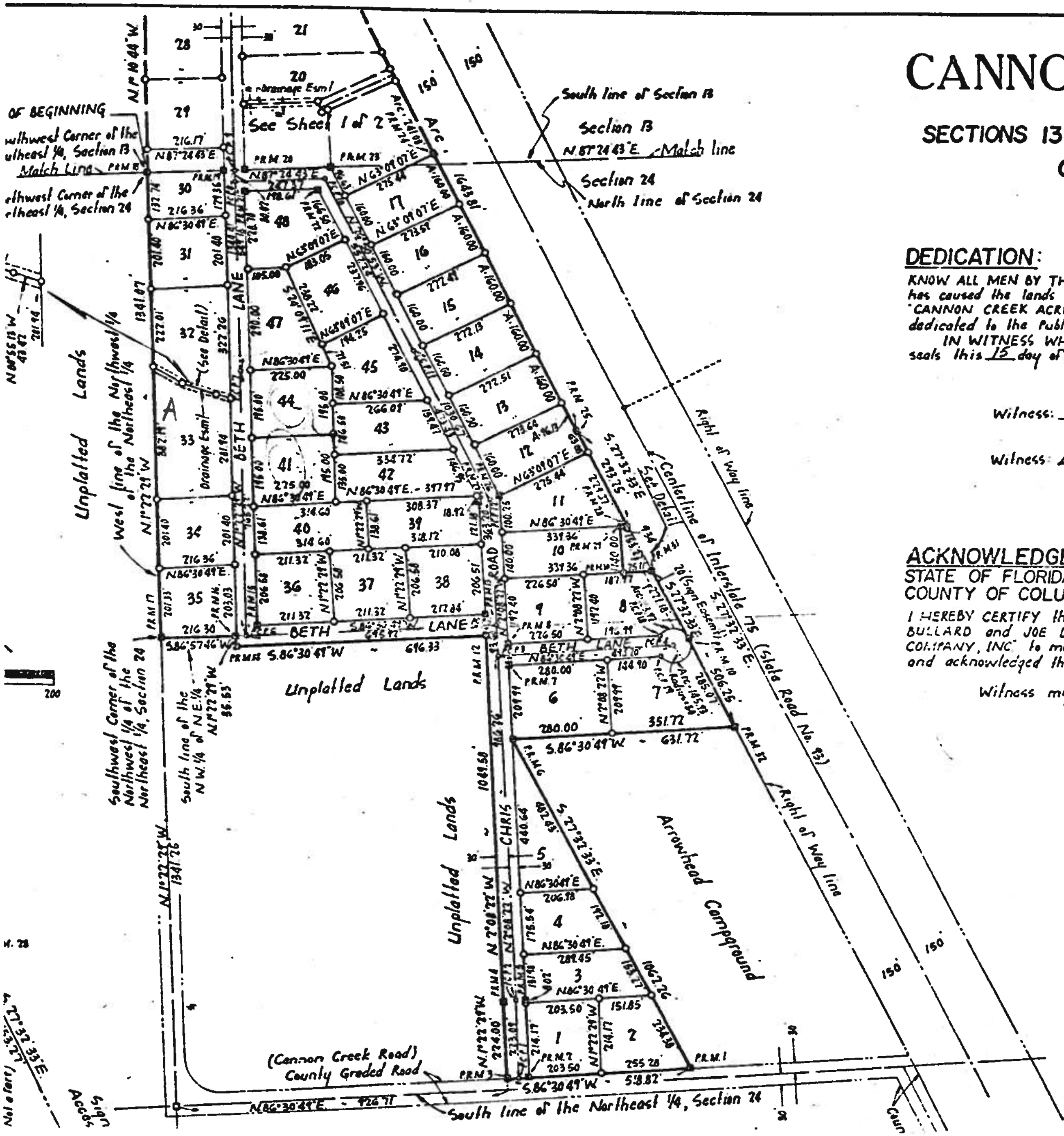
Plan Approved ✓ Not Approved ____ Date 2-2-06

By [signature] Columbia CPHU

Notes: _____


208.50'
15'-0" REAR SETBACK
121'-0"
121'-0"
208.50'
10'-0" SIDE SETBACK
DRAIN FIELD
SEPTIC TANK
10'-0" SIDE SETBACK
208.50'
80'-9"
47'-0"
80'-9"
AARON SIMQUE HOMES
SW AUDREY WAY
COLUMBIA COUNTY
FLORIDA 32024
57'-6"
57'-6"
80'-9"
47'-0"
80'-9"
50'-0"
APPROX WELL LOCATION
50'-0"
25'-0" FRONT SETBACK
SCALE: 1" = 30'-0"
208.50'
2131 SE AUDREY WAY

CANNON CREEK ACRES
IN
SECTIONS 13 & 24, TOWNSHIP 4 SOUTH, RANGE 16 EAST
COLUMBIA COUNTY, FLORIDA



DEDICATION:

KNOW ALL MEN BY THESE PRESENTS that "BULLARD DEVELOPMENT COMPANY, INC." a Florida Corporation has caused the lands herein described to be surveyed, laid out, subdivided, and platted to be know as "CANNON CREEK ACRES" and that the roads, streets and drainage easements as shown are hereby dedicated to the Public.

IN WITNESS WHEREOF the said owners have executed this dedication under their respective seals this 15 day of ______, A.D. 1977.

BULLARD DEVELOPMENT COMPANY, INC.

Witness: [signature] Signed: [signature] TOMMIE P. BULLARD, PRESIDENT

Witness: [signature] Signed: [signature] JOE D. BULLARD, SECRETARY

ACKNOWLEDGEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

I HEREBY CERTIFY that on this ____ day of ______, 1977 before me personally appeared TOMMIE P. BULLARD and JOE D. BULLARD, respectively President and Secretary of BULLARD DEVELOPMENT COMPANY, INC. to me known to be the individuals and officers described in the foregoing dedication and acknowledged that they executed the same for the purposes therein expressed.

Witness my hand and official Seal at ______ State of Florida, this ____ day of ______ 1977

Signed: ______
NOTARY PUBLIC, STATE OF FLORIDA
My Commission Expires ______

APPROVED BY BOARD OF COUNTY COMMISSIONERS COLUMBIA COUNTY, FLORIDA

Signed: [signature] Chairman

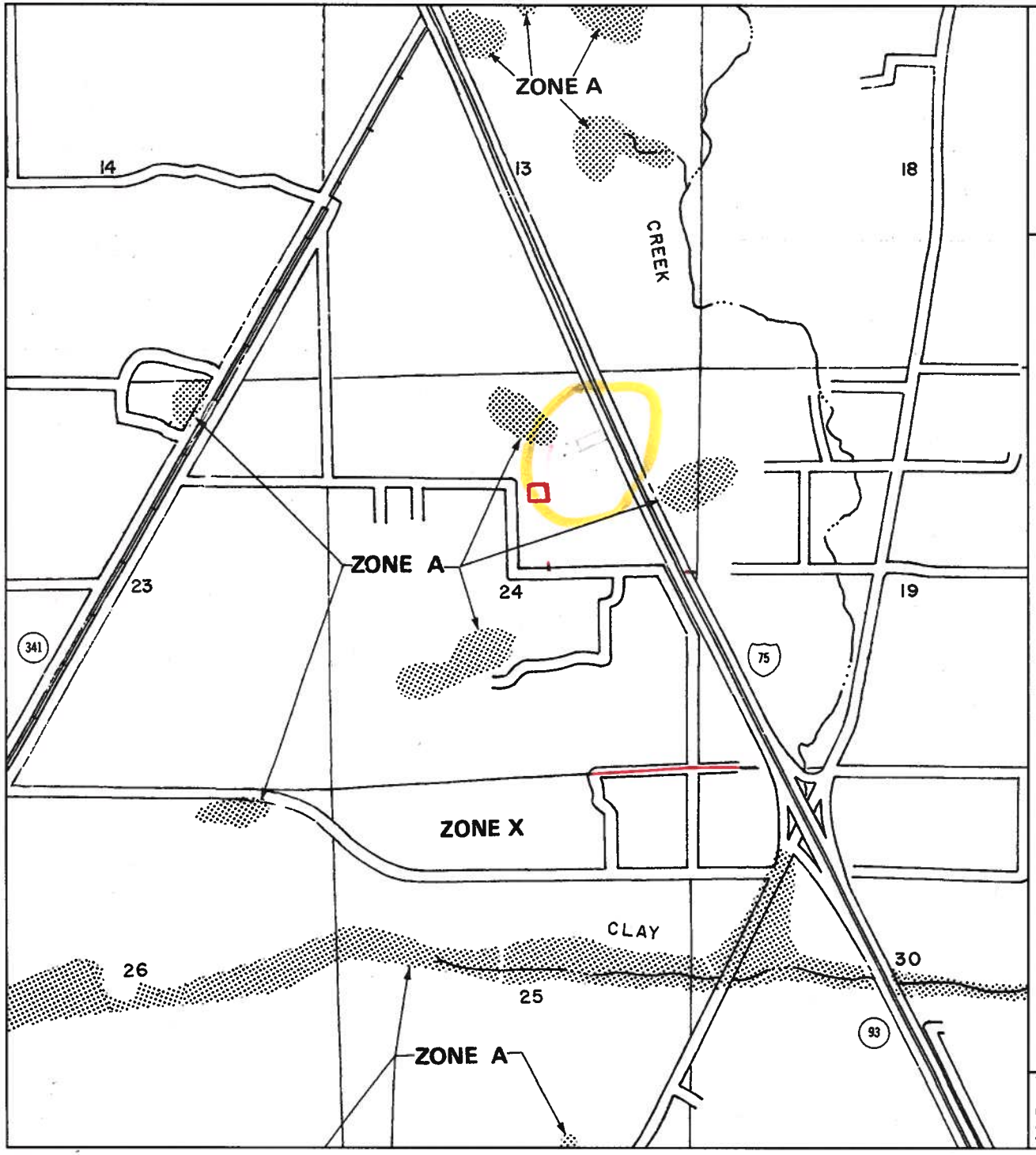
Attest: [signature] Clerk

Date: August 18 1977

CERTIFICATE OF CLERK

I HEREBY CERTIFY that the foregoing plat, having been duly approved by the Board of County Commissioners, of Columbia County, Florida, was accepted by me and filed for record this 22nd day of August A.D. 1977, in Plat Book 4, Pages 56-57 of the Public Records of Columbia County, Florida.

Signed [signature]



N

APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

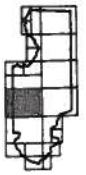
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 175 OF 290

PANEL LOCATION



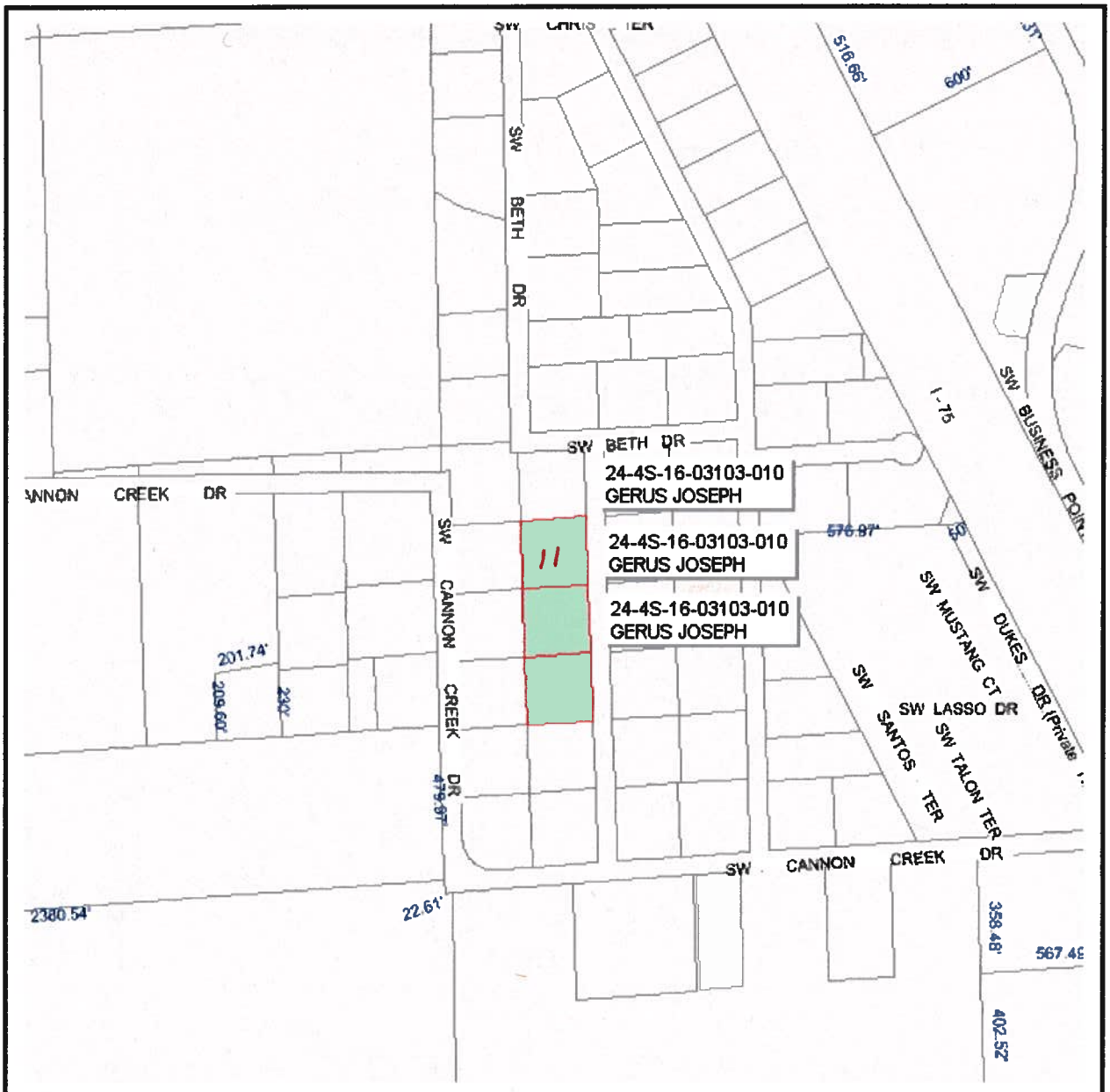
COMMUNITY-PANEL NUMBER
120070 0175 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 2/2/2008 (printed at scale and type A)



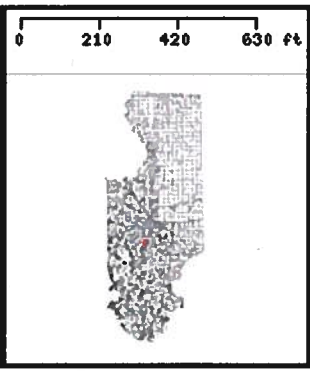
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 24-4S-16-03103-010 HX - SINGLE FAM (000100)

COMM SW COR OF SW1/4 OF NE1/4, RUN E 240.02 FT, N 464.02 FT FOR POB, CONT N 628.5 FT, E

Name:	GERUS JOSEPH	LandVal	$14,280.00
Site:		BldgVal	$76,050.00
	& ASSUNTA	ApprVal	$92,535.00
Mail:	P O BOX 2347	JustVal	$92,535.00
	LAKE CITY, FL 32056	Assd	$81,435.00
Sales Info		Exmpt	$25,000.00
		Taxable	$56,435.00



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

INSTRUMENT WAS PREPARED BY:

TERRY McDAVID 05-1007
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Inst:2005031225 Date:12/19/2005 Time:09:25
Doc Stamp-Deed : 105.00
DC,P.DeWitt Cason,Columbia County B:1060 P:1357

Property Appraiser's
Identification Number Part of R03103-010

WARRANTY DEED

This Warranty Deed, made this 15th day of December, 2005, BETWEEN JOSEPH GERUS and ASSUNTA GERUS, husband and wife whose post office address is Post Office Box 2347, Lake City, FL 32056, of the County of Columbia, State of Florida, grantor*, and AARON SIMQUE HOMES, INC., A Florida Corporation, whose post office address is Post Office Box 2183, Lake City, FL 32056, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

TOWNSHIP 4 SOUTH - RANGE 16 EAST

SECTION 24: Commence at the Southwest corner of the SW 1/4 of the NE 1/4 and run N 86 deg. 29'46" E along the South line of SW 1/4 of NE 1/4 a distance of 240.02 feet; thence N 1 deg. 22' 55" W, 881.02 feet to the Point of Beginning; thence continue N 1 deg. 22'55" W, 211.50 feet; thence N 86 deg. 29'46" E, 205.00 feet; thence S 1 deg. 22'55" E, 208.50 feet; thence N 86 deg. 29' 46" E, 5.00 feet; thence S 1 deg. 22'55" E, 3.00 feet; thence S 86 deg. 29'46" W, 210.00 feet to the Point of Beginning. Columbia County, Florida. Also known as Lot No. 11, of the unrecorded subdivision of Cannon Creek Acres.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered
in our presence:

(Signature of First Witness)
Terry McDavid
(Typed Name of First Witness)

(Signature of Second Witness)
Crystal L. Brunner
(Typed Name of Second Witness)

Joseph Gerus (SEAL)
Grantor
JOSEPH GERUS
Printed Name

Assunta J Gerus (SEAL)
Grantor
ASSUNTA GERUS
Printed Name

STATE OF Florida
COUNTY OF Columbia

The foregoing instrument was acknowledged before me this 15th day of December, 2005, by JOSEPH GERUS and ASSUNTA GERUS, Husband and Wife who are personally known to me or who have produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

TERRY MCDAVID
MY COMMISSION EXPIRES
January 14, 2009
DD 0379005
NOTARY PUBLIC, STATE OF FLORIDA

Inst:200531225 Date:12/19/2005 Time:08:25
Doc Stamp-Deed : 105.00
DC,P.DeWitt Cason,Columbia County B:1068 P:1358

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

[redacted]

DONALD AND MARY HALL
OWNERS

[redacted]
LAKE CITY, FLORIDA
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

Aaron Simpson
S.W. Audrey Way

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. **ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH**
2. **ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -------110 MPH**
3. **NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION**

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	Site Plan including: a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	Wind-load Engineering Summary, calculations and any details required a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
☑	☐	Elevations including:
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- ☐ ☐ a) Rooms labeled and dimensioned
- ☐ ☐ b) Shear walls
- ☐ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☐ ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☐ ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☐ ☐ f) Must show and identify accessibility requirements (accessable bathroom)

Foundation Plan including:

- ☐ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☐ ☐ b) All posts and/or column footing including size and reinforcing
- ☐ ☐ c) Any special support required by soil analysis such as piling
- ☐ ☐ d) Location of any vertical steel

Roof System:

- ☐ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- ☐ ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- ☐ ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termiticide or alternative method)
  10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☑ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☑ ☐ b) Floor joist size and spacing

☑ ☐ c) Girder size and spacing

☑ ☐ d) Attachment of joist to girder

☑ ☐ e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☐ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

Notice Of Commencement

Private Potable Water

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

NICHOLAS
PAUL
GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Rd.
Lake City, FL 32055
(386) 755-9021

15 FEBRUARY 2006

JOHNNY KEARSE, BUILDING OFFICIAL
COLUMBIA COUNTY, BUILDING DEPT.
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA 32055

RE: ARLINGTON MODEL, SW AUDREY WAY
PERMIT Nr.: 0106-80

DEAR SIR:

PLEASE BE ADVISED ON THE FOLLOWING CHANGES TO THE CONSTRUCTION DOCUMENTS FOR THE ABOVE REFERENCED PROJECT:

IN LIEU OF THE TRUSS ANCHORS INDICATED IN THE CON DOCS, IT SHALL BE PERMISSIBLE TO ANCHOR THE TRUSSES TO THE WALL FRAMING WITH "SIMPSON" H2.5A ANCHORS EXCEPT AS NOTED HERE:

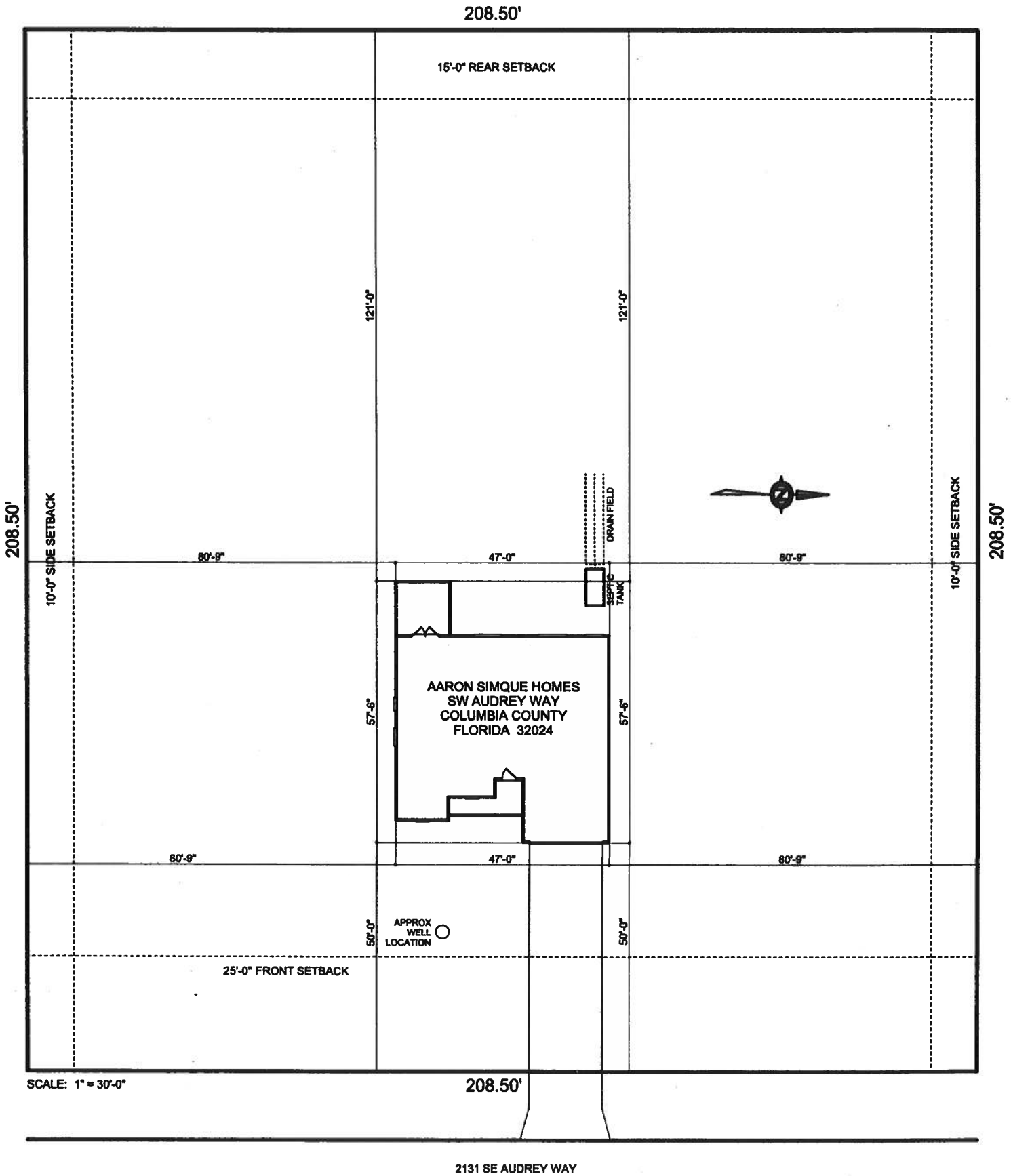
TRUSSES T01, T09 - T14 & T20, USE 2 "SIMPSON" H2.5A, MOUNTED DIAGONALLY OPPOSITE ACROSS TRUSS - 2 EACH END, EACH TRUSS.

TRUSS T08, USE "SIMPSON" H16 ANCHOR STRAP, EACH END OF TRUSS

TRUSS T03A & T05, USE "SIMPSON" LGT2 ANCHOR STRAP, EACH END.
NOTE: STUDS SUPPORTING GIRDERS SHALL BE SYP #2 OR BETTER.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005


208.50'
15'-0" REAR SETBACK
121'-0"
121'-0"
208.50'
10'-0" SIDE SETBACK
10'-0" SIDE SETBACK
208.50'
DRAIN FIELD
80'-9"
47'-0"
80'-9"
SEPTIC TANK
AARON SIMQUE HOMES
SW AUDREY WAY
COLUMBIA COUNTY
FLORIDA 32024
57'-6"
57'-6"
80'-9"
47'-0"
80'-9"
50'-0"
APPROX WELL LOCATION
50'-0"
25'-0" FRONT SETBACK
SCALE: 1" = 30'-0"
208.50'
2131 SE AUDREY WAY

Residential System Sizing Calculation

Summary

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

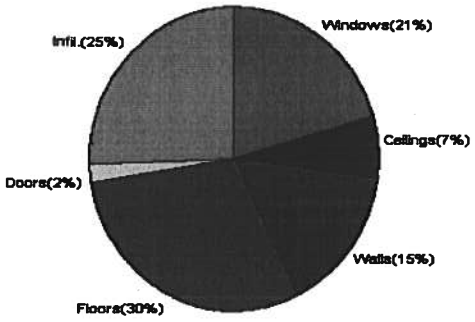
12/8/2005

Location for weather data: Gainesville - User customized: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (79F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	99	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	24	F
Total heating load calculation	**25497**	**Btuh**	**Total cooling load calculation**	**33801**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	117.7	30000	Sensible (SHR = 0.75)	81.7	22500
Heat Pump + Auxiliary(0.0kW)	117.7	30000	Latent	119.7	7500
			Total (Electric Heat Pump)	88.8	30000

WINTER CALCULATIONS

Winter Heating Load (for 1485 sqft)

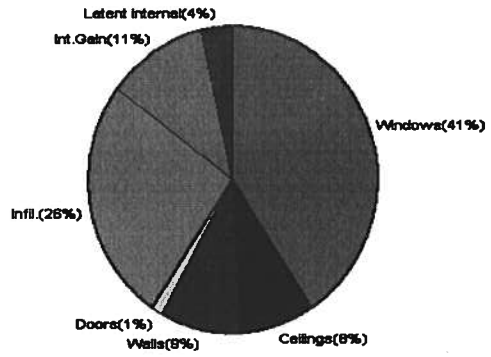
Load component			Load	
Window total	163	sqft	5257	Btuh
Wall total	1198	sqft	3935	Btuh
Door total	39	sqft	499	Btuh
Ceiling total	1485	sqft	1750	Btuh
Floor total	175	sqft	7641	Btuh
Infiltration	158	cfm	6416	Btuh
Duct loss			0	Btuh
Subtotal			**25497**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**25497**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1485 sqft)

Load component			Load	
Window total	163	sqft	13712	Btuh
Wall total	1198	sqft	3139	Btuh
Door total	39	sqft	472	Btuh
Ceiling total	1485	sqft	2790	Btuh
Floor total			0	Btuh
Infiltration	139	cfm	3642	Btuh
Internal gain			3780	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**27535**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			5065	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1200	Btuh
Total latent gain			**6265**	**Btuh**
TOTAL HEAT GAIN			**33801**	**Btuh**



For Florida residences only

EnergyGauge® System Sizing
PREPARED BY: _____
DATE: _____

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Winter Temperature Difference: 37.0 F

12/8/2005

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
2	2, Clear, Metal, 0.87	W	60.0	32.2	1931 Btuh
3	2, Clear, Metal, 0.87	W	4.0	32.2	129 Btuh
4	2, Clear, Metal, 0.87	E	13.3	32.2	428 Btuh
5	2, Clear, Metal, 0.87	E	15.0	32.2	483 Btuh
6	2, Clear, Metal, 0.87	E	15.0	32.2	483 Btuh
7	2, Clear, Metal, 0.87	S	4.0	32.2	129 Btuh
8	2, Clear, Metal, 0.87	S	12.0	32.2	386 Btuh
	Window Total		163(sqft)		5257 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1017	3.3	3339 Btuh
2	Frame - Wood - Adj(0.09)	13.0	181	3.3	596 Btuh
	Wall Total		1198		3935 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		19	12.9	240 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		39		499Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1485	1.2	1750 Btuh
	Ceiling Total		1485		1750Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	175.0 ft(p)	43.7	7641 Btuh
	Floor Total		175		7641 Btuh
			Zone Envelope Subtotal:		19081 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	11880	158.4	6416 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**25497 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

12/8/2005

WHOLE HOUSE TOTALS

	Subtotal Sensible	25497 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	25497 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Winter Temperature Difference: 37.0 F

12/8/2005

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
2	2, Clear, Metal, 0.87	W	60.0	32.2	1931 Btuh
3	2, Clear, Metal, 0.87	W	4.0	32.2	129 Btuh
4	2, Clear, Metal, 0.87	E	13.3	32.2	428 Btuh
5	2, Clear, Metal, 0.87	E	15.0	32.2	483 Btuh
6	2, Clear, Metal, 0.87	E	15.0	32.2	483 Btuh
7	2, Clear, Metal, 0.87	S	4.0	32.2	129 Btuh
8	2, Clear, Metal, 0.87	S	12.0	32.2	386 Btuh
	Window Total		163(sqft)		5257 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1017	3.3	3339 Btuh
2	Frame - Wood - Adj(0.09)	13.0	181	3.3	596 Btuh
	Wall Total		1198		3935 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		19	12.9	240 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		39		499Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1485	1.2	1750 Btuh
	Ceiling Total		1485		1750Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	175.0 ft(p)	43.7	7641 Btuh
	Floor Total		175		7641 Btuh
			Zone Envelope Subtotal:		19081 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	11880	158.4	6416 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**25497 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

12/8/2005

WHOLE HOUSE TOTALS

	Subtotal Sensible	25497 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	25497 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Summer Temperature Difference: 24.0 F 12/8/2005

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	W	13.5f	8ft.	40.0	40.0	0.0	35	86	1402 Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	60.0	0.0	60.0	35	86	5136 Btuh
3	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	4.0	0.0	4.0	35	86	342 Btuh
4	2, Clear, 0.87, None,N,N	E	9.5ft	8ft.	13.3	13.1	0.2	35	86	478 Btuh
5	2, Clear, 0.87, None,N,N	E	5.5ft	8ft.	15.0	4.7	10.3	35	86	1047 Btuh
6	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	15.0	0.0	15.0	35	86	1284 Btuh
7	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	4.0	4.0	0.0	35	40	140 Btuh
8	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	12.0	12.0	0.0	35	40	421 Btuh
	Excursion									3462 Btuh
	Window Total				163 (sqft)					13712 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1016.7	2.7	2752 Btuh
2	Frame - Wood - Adj	13.0/0.09	181.4	2.1	386 Btuh
	Wall Total		1198 (sqft)		3139 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	18.6	12.3	227 Btuh
2	Insulated - Exterior	20.0	12.3	245 Btuh
	Door Total	39 (sqft)		472 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1485.0	1.9	2790 Btuh
	Ceiling Total		1485 (sqft)		2790 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	175 (ft(p))	0.0	0 Btuh
	Floor Total		175.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		20114 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	11880	138.6	3642 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	2400	3780 Btuh
Duct load	Unsealed, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**27535 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

12/8/2005

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**27535 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**27535 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**27535 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5065 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6265 Btuh**
	TOTAL GAIN	**33801 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Summer Temperature Difference: 24.0 F 12/8/2005

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	W	13.5f	8ft.	40.0	40.0	0.0	35	86	1402 Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	60.0	0.0	60.0	35	86	5136 Btuh
3	2, Clear, 0.87, None,N,N	W	1.5ft	8ft.	4.0	0.0	4.0	35	86	342 Btuh
4	2, Clear, 0.87, None,N,N	E	9.5ft	8ft.	13.3	13.1	0.2	35	86	478 Btuh
5	2, Clear, 0.87, None,N,N	E	5.5ft	8ft.	15.0	4.7	10.3	35	86	1047 Btuh
6	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	15.0	0.0	15.0	35	86	1284 Btuh
7	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	4.0	4.0	0.0	35	40	140 Btuh
8	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	12.0	12.0	0.0	35	40	421 Btuh
	Excursion									3462 Btuh
	Window Total				163 (sqft)					13712 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1016.7	2.7	2752 Btuh
2	Frame - Wood - Adj	13.0/0.09	181.4	2.1	386 Btuh
	Wall Total		1198 (sqft)		3139 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	18.6	12.3	227 Btuh
2	Insulated - Exterior	20.0	12.3	245 Btuh
	Door Total	39 (sqft)		472 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1485.0	1.9	2790 Btuh
	Ceiling Total		1485 (sqft)		2790 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	175 (ft(p))	0.0	0 Btuh
	Floor Total		175.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		20114 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.70	11880	138.6	3642 Btuh

Internal gain	Occupants	Btuh/occupant		Appliance	Load
	6	X 230	+	2400	3780 Btuh

Duct load	Unsealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh
		Sensible Zone Load	**27535 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

Code Only
Professional Version
Climate: North

12/8/2005

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**27535 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**27535 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**27535 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	5065 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6265 Btuh**
	TOTAL GAIN	**33801 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Aaron Simque Homes
SW Audrey Way
, FL 32025-

Project Title:
The Arlington Model - Audrey Road Spec

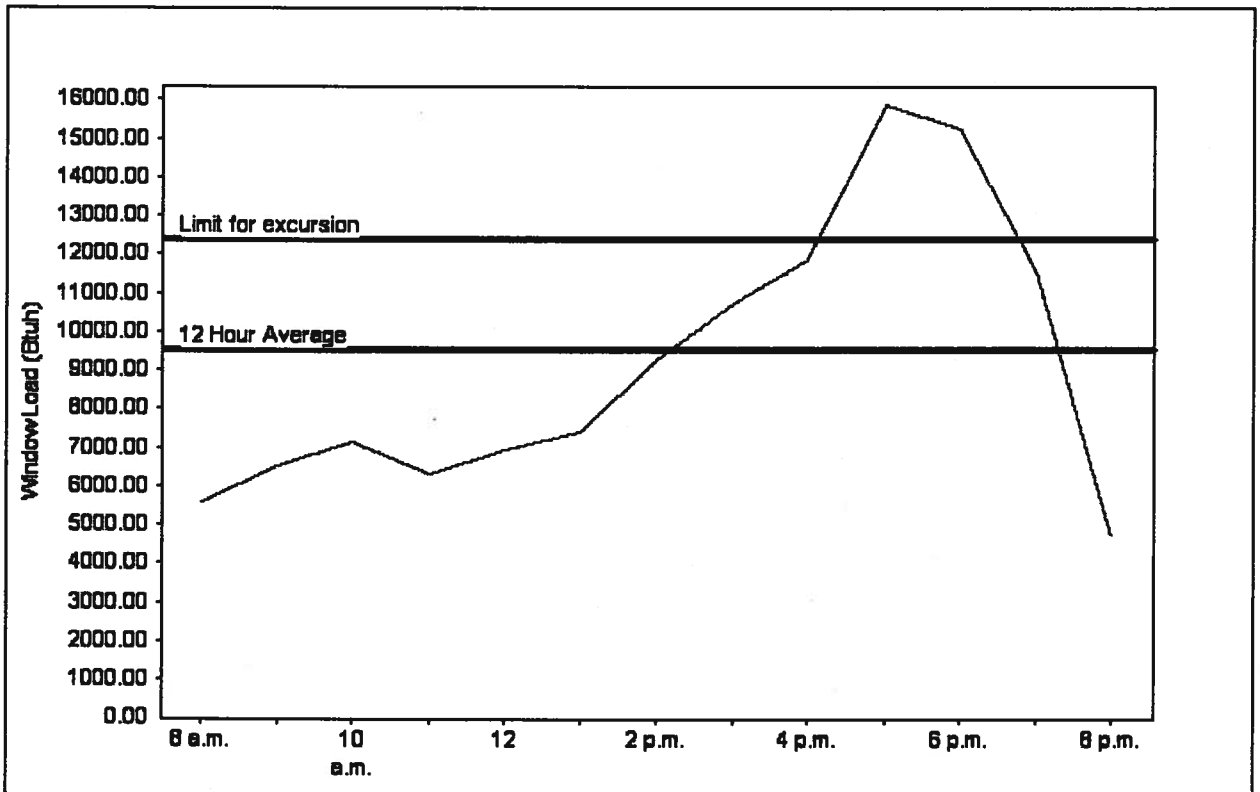
Code Only
Professional Version
Climate: North

12/8/2005

Weather data for: Gainesville - User customized

Summer design temperature	99 F	Average window load for July	9531 Btuh
Summer setpoint	75 F	Peak window load for July	15853 Btu
Summer temperature difference	24 F	Excusion limit(130% of Ave.)	12391 Btu
Latitude	29 North	Window excursion (July)	3462 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY: _____
DATE: _____

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

[redacted]

PHONE (904) 755-7824
FAX (904) 755-7822
[redacted]
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphrum tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphrum tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

ALLEN M. REEVES
CERTIFICATE
NO. 18354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves
1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reeves
1 APRIL 2002

REEVES
No. 18354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reeves
1 APRIL 2002

REEVES ☆ No. 19354 ☆ STATE OF FLORIDA ☆ PROFESSIONAL ENGINEER ☆

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN H. REEVES
CERTIFICATE NO. 19[illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen H. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Allen N. Reeves
1 APRIL 2002

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NO. 1837
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FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
	Optional Performance		
4.3	Water Resistance (ASTM E 547-00)		
	(with and without screen)		
	WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the meeting rail)		
	(Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.
	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the meeting rail)		
	(Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.18" max.
	@ 70.8 psf (negative)	0.05"	0.18" max.

Exceeds L/175 for deflection, but passes all other test requirements.

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES CERTIFICATE NO. 18034 STATE OF FLORIDA PROFESSIONAL ENGINEER

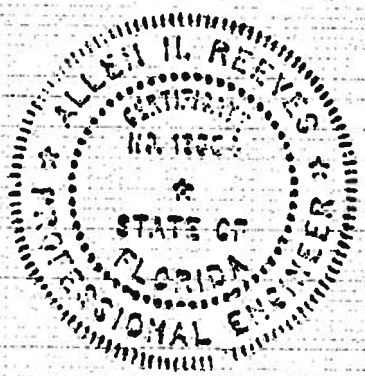
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:mlb
01-41134.01

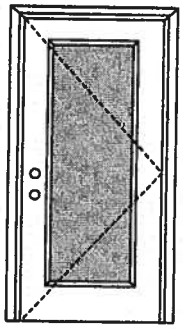


X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

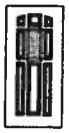
Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

Y
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite
Masonite International Corporation

TAMKO®
ROOFING PRODUCTS

FEB - 4 RECD

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

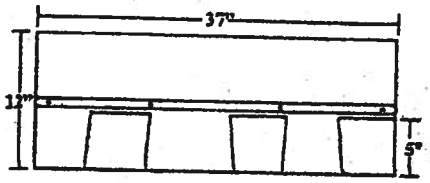
TAMKO Roofing Products, Inc.

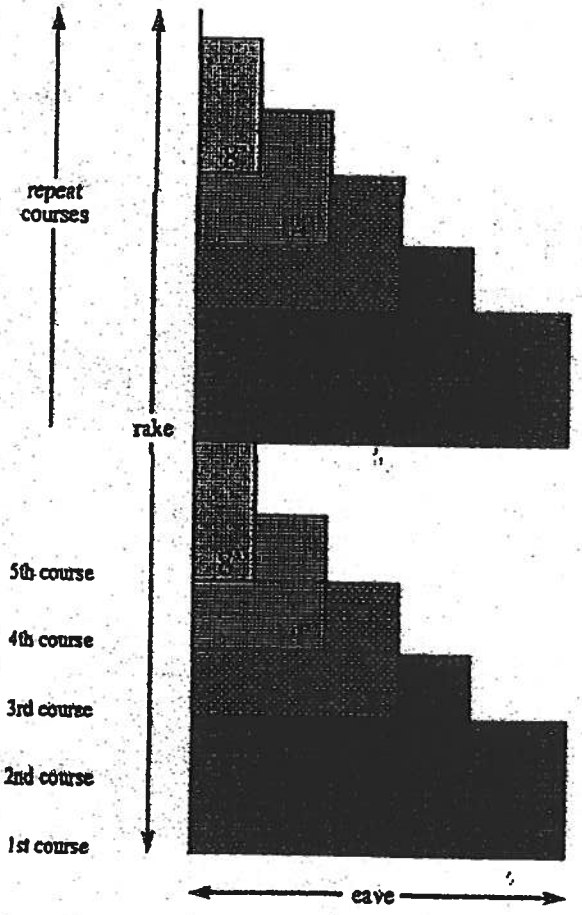
CORPORATE HEADQUARTERS
220 W. FOURTH STREET P.O. BOX 1404 JOPLIN, MO 64802-1404 800-641-4691 FAX 800-641-1925

TAMKO
ROOFING PRODUCTS

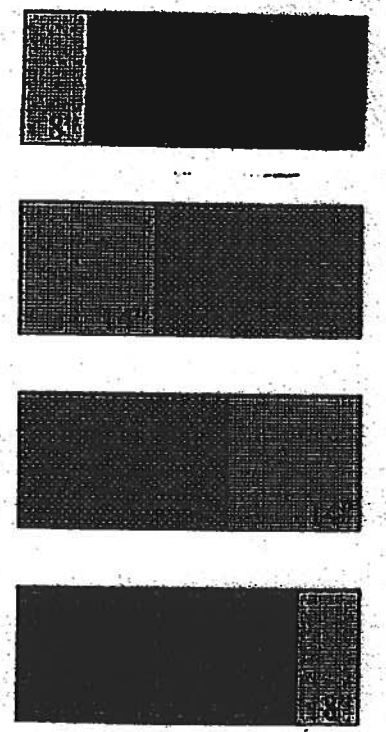
Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"





The 4 cuts in the first 10 courses:



In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.

TAMKO®
ROOFING PRODUCTS

Application Instructions for

- Glass-Seal
- Glass-Seal AR
- Elite Glass-Seal®
- Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.
THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.
IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.
FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

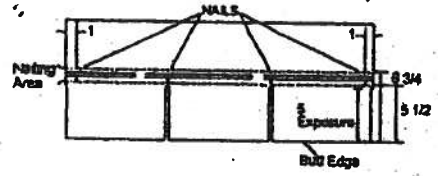
3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

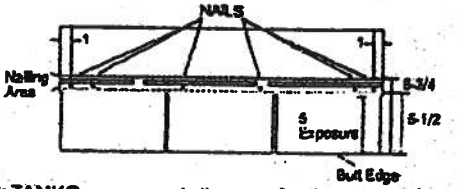
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below).



2) Mansard or High Wind Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fastener per shingle. (See Mansard fastening pattern illustrated below.)



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

TAMKO ROOFING PRODUCTS

- Glass-Seal
- Glass-Seal AR
- Elite Glass-Seal®
- Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a $.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Mansard Fastening Pattern.

5. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

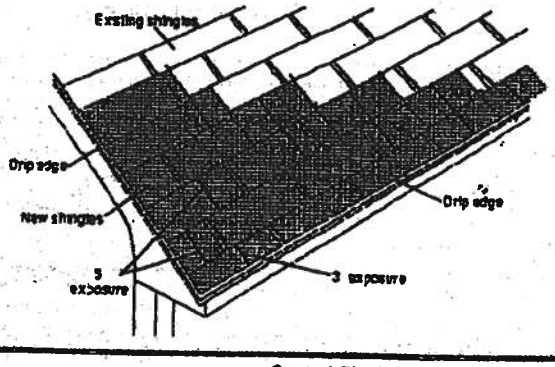
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nesting procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Succeeding Courses: According to the off-set application method you choose to use, remove the appropriate length from the rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.



6. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 50 lb. roll roofing in the valley. Nail the felt only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 6 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

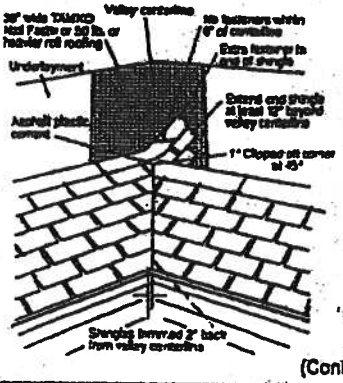
Note: For a neater installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

- ***CAUTION:*** *Adhesive must be applied in smooth, thin, even layers.*

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

(CONTINUED from Pg. 3)

TAMKO ROOFING PRODUCTS

• Glass-Seal • Elite Glass-Seal®
• Glass-Seal AR • Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

FOR ALTERNATE VALLEY APPLICATION METHODS, PLEASE CONTACT TAMKO'S TECHNICAL SERVICES DEPARTMENT.

10. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener 5-1/2 in. back from the exposed end and 1 in. up from the edge. Do not nail directly into the sealant.

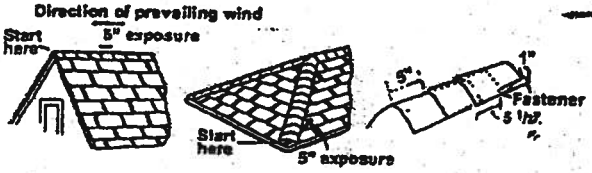
TAMKO recommends the use of TAMKO Hip & Ridge shingle products. Where matching colors are available, it is acceptable to use TAMKO's Glass-Seal or Elite Glass-Seal shingles cut down to 12 in. pieces.

NOTE: AR type shingle products should be used as Hip & Ridge on Glass-Seal AR and Elite Glass-Seal AR shingles.

Fasteners should be 1/4 in. longer than the one used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES IN COOL WEATHER.

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.



THIS PRODUCT IS COVERED BY A LIMITED WARRANTY. THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IMPORTANT - READ CAREFULLY BEFORE OPENING BUNDLE

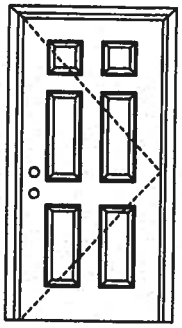
In this paragraph "You" and Your" refer to the installer of the shingles and the owner of the building on which these shingles will be installed. This is a legally binding agreement between You and TAMKO Roofing Products, Inc. ("TAMKO"). By opening this bundle You agree: (a) to install the shingles strictly in accordance with the instructions printed on this wrapper; or (b) that shingles which are not installed strictly in accordance with the instructions printed on this wrapper are sold "AS IS" and are not covered by the limited warranty that is also printed on this wrapper, or any other warranty, including, but not limited to (except where prohibited by law) implied warranties of MERCHANTABILITY and FITNESS FOR USE.

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

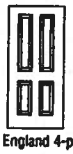
APPROVED DOOR STYLES:


Flush


Arch Top 3-panel


3-panel


6-panel


New England 4-panel


Eyebrow 4-panel


8-panel


9-panel


15-panel


5-panel


5-panel with scroll


Eyebrow 5-panel


Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors
Exclusively from Masonite®
Masonite International Corporation

Y
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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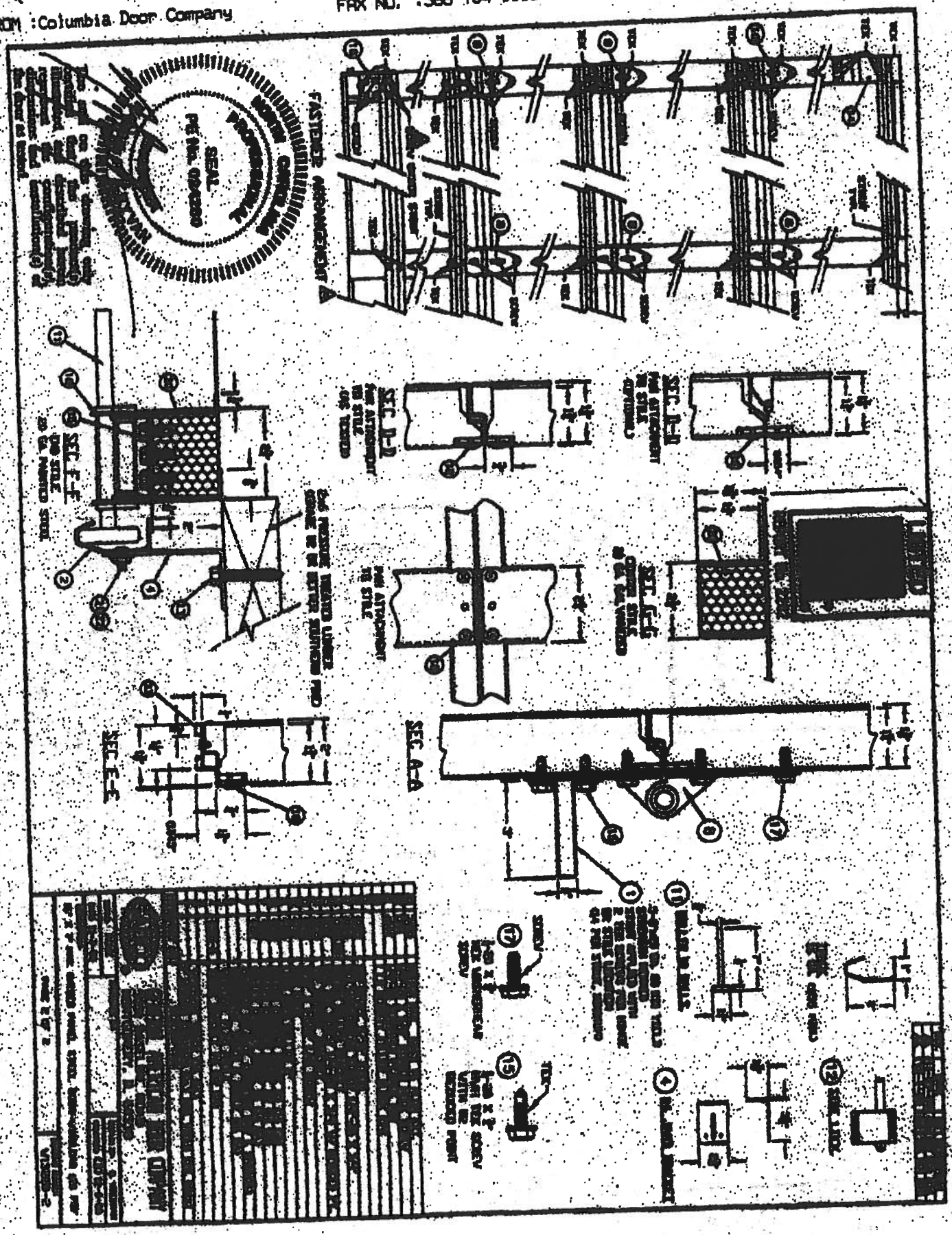


FORM 600A-2004 EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**The Arlington Model - Audrey Road Spec**	Builder:	**Aaron Simque Homes**
Address:	**SW Audrey Way**	Permitting Office:	Columbia
City, State:	**, FL 32025-**	Permit Number:	24155
Owner:	**Aaron Simque Homes**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 11.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	No		
6. Conditioned floor area (ft²)	1485 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAULT)	7a. (Dble Default) 163.3 ft²	a. Electric Heat Pump	Cap: 30.0 kBtu/hr
b. SHGC:			HSPF: 6.80
(or Clear or Tint DEFAULT)	7b. (Clear) 163.3 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 175.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Frame, Wood, Exterior	R=13.0, 1016.7 ft²		EF: 0.90
b. Frame, Wood, Adjacent	R=13.0, 181.4 ft²	b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits	
a. Under Attic	R=30.0, 1485.0 ft²	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 35.0 ft		
b. N/A			

Glass/Floor Area: 0.11	Total as-built points: 23075 Total base points: 23274	PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Will Myers

DATE: 12·08·05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: 12-8-05

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Audrey Way, , FL, 32025-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1485.0	20.04	5356.7	Double, Clear	W	13.5	8.0	40.0	38.52	0.43	657.8	
				Double, Clear	W	1.5	8.0	60.0	38.52	0.96	2214.6	
				Double, Clear	W	1.5	8.0	4.0	38.52	0.96	147.6	
				Double, Clear	E	9.5	8.0	13.3	42.06	0.47	263.5	
				Double, Clear	E	5.5	8.0	15.0	42.06	0.62	391.2	
				Double, Clear	E	1.5	8.0	15.0	42.06	0.96	604.2	
				Double, Clear	S	1.5	8.0	4.0	35.87	0.92	132.5	
				Double, Clear	S	1.5	8.0	12.0	35.87	0.92	397.4	
				As-Built Total:				**163.3**			**4808.7**	
WALL TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM	=	Points	
Adjacent	181.4	0.70	127.0	Frame, Wood, Exterior			13.0	1016.7	1.50		1525.1	
Exterior	1016.7	1.70	1728.4	Frame, Wood, Adjacent			13.0	181.4	0.60		108.8	
Base Total:	**1198.1**		**1855.4**	**As-Built Total:**				**1198.1**			**1633.9**	
DOOR TYPES	Area X	BSPM	= Points	Type				Area X	SPM	=	Points	
Adjacent	18.6	1.60	29.7	Exterior Insulated				20.0	4.10		82.0	
Exterior	20.0	4.10	82.0	Adjacent Insulated				18.6	1.60		29.7	
Base Total:	**38.6**		**111.7**	**As-Built Total:**				**38.6**			**111.7**	
CEILING TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM X SCM	=	Points	
Under Attic	1485.0	1.73	2569.1	Under Attic			30.0	1485.0	1.73 X 1.00		2569.1	
Base Total:	**1485.0**		**2569.1**	**As-Built Total:**				**1485.0**			**2569.1**	
FLOOR TYPES	Area X	BSPM	= Points	Type			R-Value	Area X	SPM	=	Points	
Slab	175.0(p)	-37.0	-6475.0	Slab-On-Grade Edge Insulation			0.0	175.0(p	-41.20		-7210.0	
Raised	0.0	0.00	0.0									
Base Total:			**-6475.0**	**As-Built Total:**				**175.0**			**-7210.0**	
INFILTRATION	Area X	BSPM	= Points					Area X	SPM	=	Points	
	1485.0	10.21	15161.8					1485.0	10.21		15161.8	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Audrey Way, , FL, 32025-**	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 18579.7			**Summer As-Built Points:**					**17075.2**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 30000 btuh ,SEER/EFF(11.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
			17075	1.00	(1.09 x 1.147 x 1.00)	0.310	1.000	6623.7
18579.7	**0.4266**	**7926.1**	**17075.2**	**1.00**	**1.250**	**0.310**	**1.000**	**6623.7**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Audrey Way, , FL, 32025-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1485.0	12.74	3405.4	Double, Clear	W	13.5	8.0	40.0	20.73	1.21	1006.9	
				Double, Clear	W	1.5	8.0	60.0	20.73	1.01	1257.5	
				Double, Clear	W	1.5	8.0	4.0	20.73	1.01	83.8	
				Double, Clear	E	9.5	8.0	13.3	18.79	1.34	334.2	
				Double, Clear	E	5.5	8.0	15.0	18.79	1.19	335.3	
				Double, Clear	E	1.5	8.0	15.0	18.79	1.02	287.5	
				Double, Clear	S	1.5	8.0	4.0	13.30	1.04	55.4	
				Double, Clear	S	1.5	8.0	12.0	13.30	1.04	166.1	
				As-Built Total:				**163.3**			**3526.7**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	181.4	3.60	653.0	Frame, Wood, Exterior			13.0	1016.7	3.40		3456.8	
Exterior	1016.7	3.70	3761.8	Frame, Wood, Adjacent			13.0	181.4	3.30		598.6	
Base Total:	**1198.1**		**4414.8**	**As-Built Total:**				**1198.1**			**4055.4**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	18.6	8.00	148.5	Exterior Insulated				20.0	8.40		168.0	
Exterior	20.0	8.40	168.0	Adjacent Insulated				18.6	8.00		148.5	
Base Total:	**38.6**		**316.5**	**As-Built Total:**				**38.6**			**316.5**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1485.0	2.05	3044.3	Under Attic			30.0	1485.0	2.05 X	1.00	3044.3	
Base Total:	**1485.0**		**3044.3**	**As-Built Total:**				**1485.0**			**3044.3**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	175.0(p)	8.9	1557.5	Slab-On-Grade Edge Insulation			0.0	175.0(p	18.80		3290.0	
Raised	0.0	0.00	0.0									
Base Total:			**1557.5**	**As-Built Total:**				**175.0**			**3290.0**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1485.0	-0.59	-876.1					1485.0	-0.59		-876.1	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Audrey Way, , FL, 32025-**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**11862.3**	**Winter As-Built Points:**					**13356.7**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 30000 btuh ,EFF(6.8) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
			13356.7	1.000	(1.069 x 1.169 x 1.00)	0.501	1.000	8370.2
11862.3	**0.6274**	**7442.4**	**13356.7**	**1.00**	**1.250**	**0.501**	**1.000**	**8370.2**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Audrey Way, , FL, 32025-** PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X Multiplier	= Total		Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3	2635.00	7905.0		50.0	0.90	3	1.00	2693.56	1.00	8080.7
				As-Built Total:						**8080.7**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
7926	**7442**	**7905**	**23274**	**6624**	**8370**	**8081**	**23075**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Audrey Way, , FL, 32025-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.3

The higher the score, the more efficient the home.

Aaron Simque Homes, SW Audrey Way, , FL, 32025-

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	No	
6.	Conditioned floor area (ft²)	1485 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description Area (Dble Default) 163.3 ft²	
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 163.3 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 175.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1016.7 ft²	
b.	Frame, Wood, Adjacent	R=13.0, 181.4 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1485.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 35.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 30.0 kBtu/hr	
		SEER: 11.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 30.0 kBtu/hr	
		HSPF: 6.80	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 50.0 gallons	
		EF: 0.90	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000977

DATE 02/20/2006 PARCEL ID # 24-4S-16-03103-010

APPLICANT AARON SIMQUE PHONE 755-0841

ADDRESS P.O. BOX 2183 LAKE CITY FL 32056

OWNER AARON SIMQUE PHONE 755-0841

ADDRESS 286 SW AUDREY WAY LAKE CITY FL 32024

CONTRACTOR AARON SIMQUE PHONE 755-0841

LOCATION OF PROPERTY 47S, TR ON CR 242, TR ON ARROWHEAD, TL ON CANNON CREEK, TR ON AUDREY, 2ND LOT ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT

SIGNATURE

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY, FLORIDA

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By [signature] Deputy Clerk
Date Dec 19, 2005

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

Inst:2005031227 Date:12/19/2005 Time:09:25
MK DC,P.DeWitt Cason,Columbia County B:1068 P:1361

THIS INSTRUMENT PREPARED BY
& RETURN TO:
Columbia County Bank
Linda Evans
173 NW Hillsboro Street
Lake City, FL 32055
REC: $

24155

NOTICE OF COMMENCEMENT

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement:

1. Description of Property: Audrey Way, Lake City FL
Township 4 South, Range 16 East Section 24
Commence at the SW corner of the SW ¼ of the NE ¼ and run N86 deg. 29' 46" E along the South line of SW ¼ of NE ¼ a distance of 204.02 feet; thence N 1 deg.22' 55" W 881.02 feet to the Point of Beginning; thence continue N 1 deg. 22' 55" W 211.50 feet; thence N 86 deg. 29' 46" E 5.00 feet; thence S 1 deg 22' 55" E 3.00 feet; thence S 86 deg. 29" 46" W 210.00 feet to the point of Beginning. Columbia County, FL Also known as Lot No. 11 of the unrecorded subdivision of Cannon Creek Acres.

2. General Description of Improvements: Approx. 1,504 square foot single family residence

3. Owner Information: Aaron Simque Homes, Inc.
P.O. Box 2183
Lake City, FL 32056

Owner's Interest in Property: Fee Simple

4. Contractor: Aaron Simque Homes, Inc.
P.O.Box 2183
Lake City, FL 32056

5. Lender: Columbia County Bank
173 NW Hillsboro Street
Lake City, FL 32055

6. Additional persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:

7. Expiration date of Notice of Commencement is one (1) year from the date of recording.

Aaron Simque Homes, Inc.
[signature]
Aaron Simque, President

STATE OF FLORIDA}
COUNTY OF Columbia}

The foregoing instrument was acknowledged before me this 15th day of December, 2005 by Aaron Simque as President of Aaron Simque Homes, Inc.

NOTARY PUBLIC
[signature: Janice Elaine Gonzalez]

JANICE ELAINE GONZALEZ
NOTARY PUBLIC, STATE OF FLORIDA
MY COMM. EXPIRES MARCH 29, 2006
NO. DD 101097

Name:
State of Florida at Large (SEAL)
Personally Known: ✓ JG
Produced Identification:
Type:
My Commission Expires:

(NOC)

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 24-4S-16-03103-010 **Building permit No.** 000024155

Use Classification SFD,UTILITY **Fire:** 67.00

Permit Holder AARON SIMQUE **Waste:** 201.00

Owner of Building AARON SIMQUE **Total:** 268.00

Location: 286 SW AUDREY WAY, LAKE CITY, FL

Date: 10/10/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L146588		
Builder:	AARON SIMQUE	Date:	1/25/2006
Lot:	LOT 11 CANNON CREEK ES	Start Number:	**1651**
Subdivision:	N\A		
County or City:	COLUMBIA		
Truss Page Count:	35		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

SIMQUE, AARON D. RB29003130
Address: 320 SW AINSLEY GLN.
LAKE CITY, FL 32025 Designer: **119**

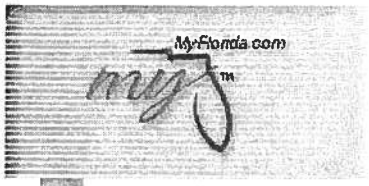
Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

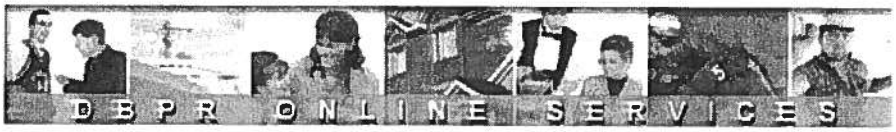
Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0125061651	1/25/2006				
2	CJ3	0125061652	1/25/2006				
3	CJ5	0125061653	1/25/2006				
4	EJ5	0125061654	1/25/2006				
5	EJ7	0125061655	1/25/2006				
6	EJ7A	0125061656	1/25/2006				
7	EJ7B	0125061657	1/25/2006				
8	EJ7C	0125061658	1/25/2006				
9	EJ7D	0125061659	1/25/2006				
10	HJ7	0125061660	1/25/2006				
11	HJ9	0125061661	1/25/2006				
12	T01	0125061662	1/25/2006				
13	T02	0125061663	1/25/2006				
14	T03A	0125061664	1/25/2006				
15	T03G	0125061665	1/25/2006				
16	T04	0125061666	1/25/2006				
17	T04A	0125061667	1/25/2006				
18	T04B	0125061668	1/25/2006				
19	T04G	0125061669	1/25/2006				
20	T05	0125061670	1/25/2006				
21	T06	0125061671	1/25/2006				
22	T07	0125061672	1/25/2006				
23	T08	0125061673	1/25/2006				
24	T09	0125061674	1/25/2006				
25	T10	0125061675	1/25/2006				
26	T11	0125061676	1/25/2006				
27	T12	0125061677	1/25/2006				
28	T13	0125061678	1/25/2006				
29	T14	0125061679	1/25/2006				
30	T15	0125061680	1/25/2006				
31	T16	0125061681	1/25/2006				
32	T17	0125061682	1/25/2006				
33	T18	0125061683	1/25/2006				
34	T19	0125061684	1/25/2006				
35	T20	0125061685	1/25/2006				

JAN 25 2006





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Licensee Details

Licensee Information

Name:	**SIMQUE, AARON DAVID (Primary Name)**
	AARON SIMQUE HOMES INC (DBA Name)
Main Address:	**320 SW AINSLEY GLN.** **LAKE CITY, Florida 32024**

License Information

License Type:	**Registered Building Contractor**
Rank:	**Reg Building**
License Number:	**RB29003130**
Status:	**Current, Active**
Licensure Date:	**10/23/2002**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	02/20/2004

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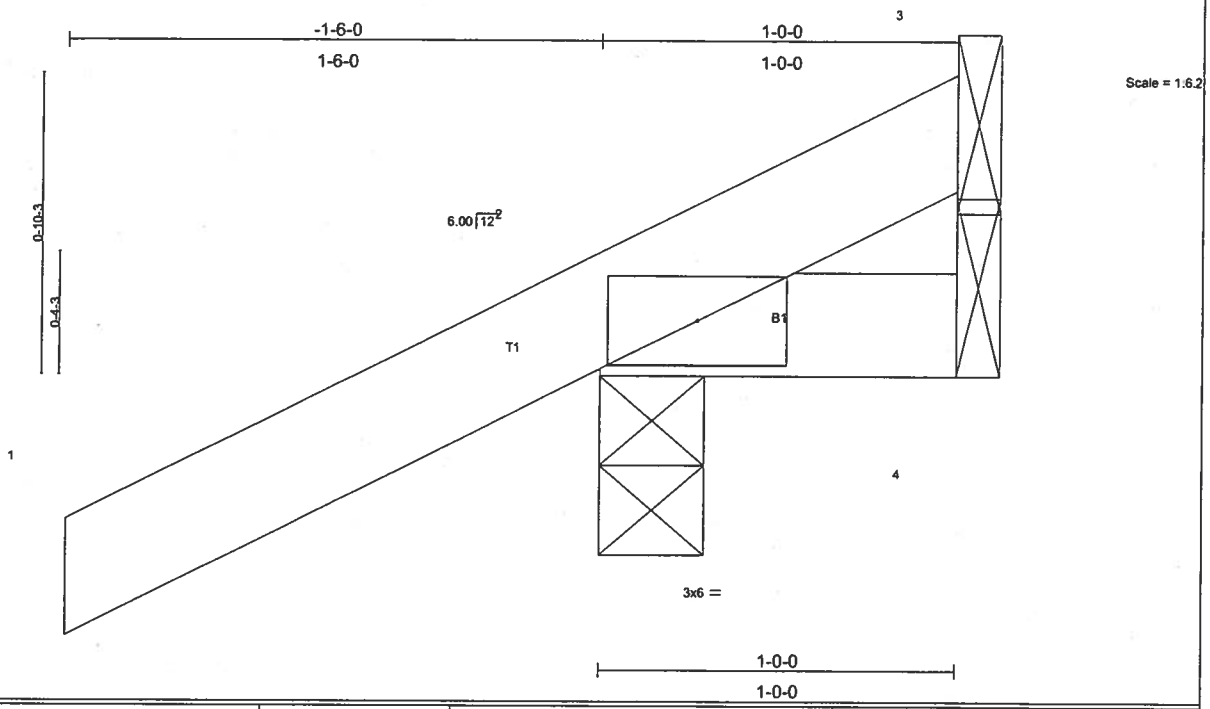
JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061651

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	CJ1	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=70(load case 5)
Max Uplift2=-192(load case 5), 4=-9(load case 3), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=61(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/34
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 192 lb uplift at joint 2, 9 lb uplift at joint 4 and 40 lb uplift at joint 3.

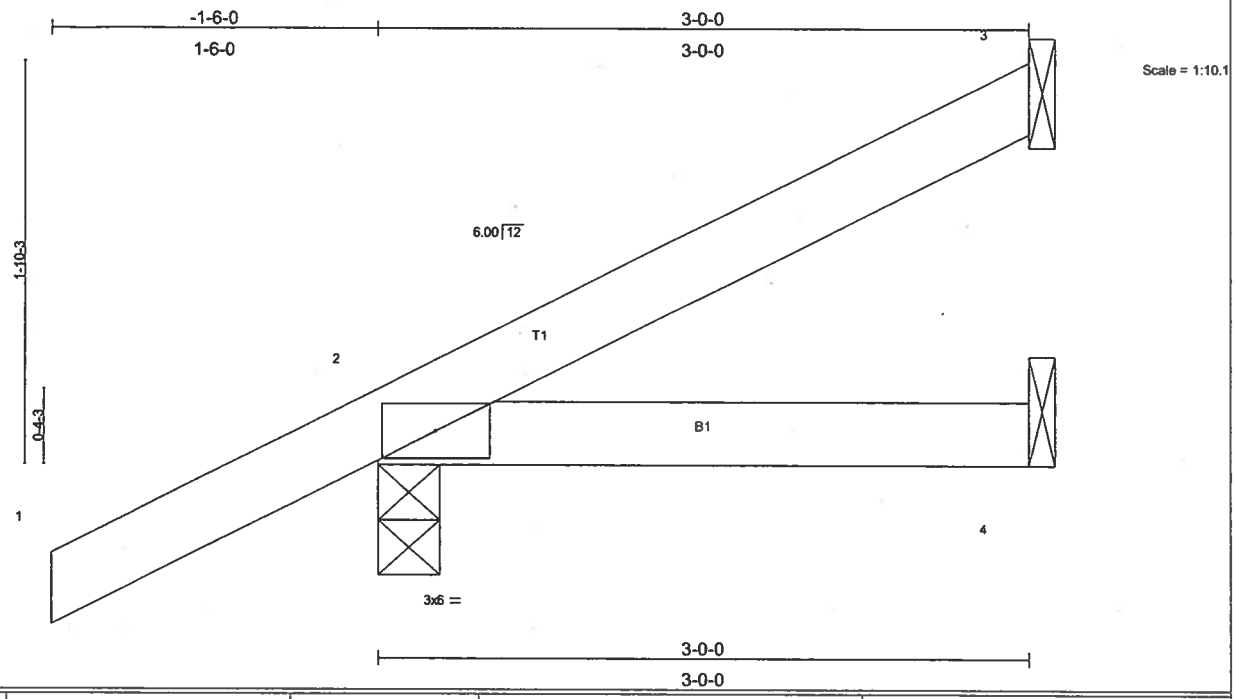
LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061652

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	CJ3	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-38(load case 5), 2=-186(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-48/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3, 186 lb uplift at joint 2 and 27 lb uplift at joint 4.

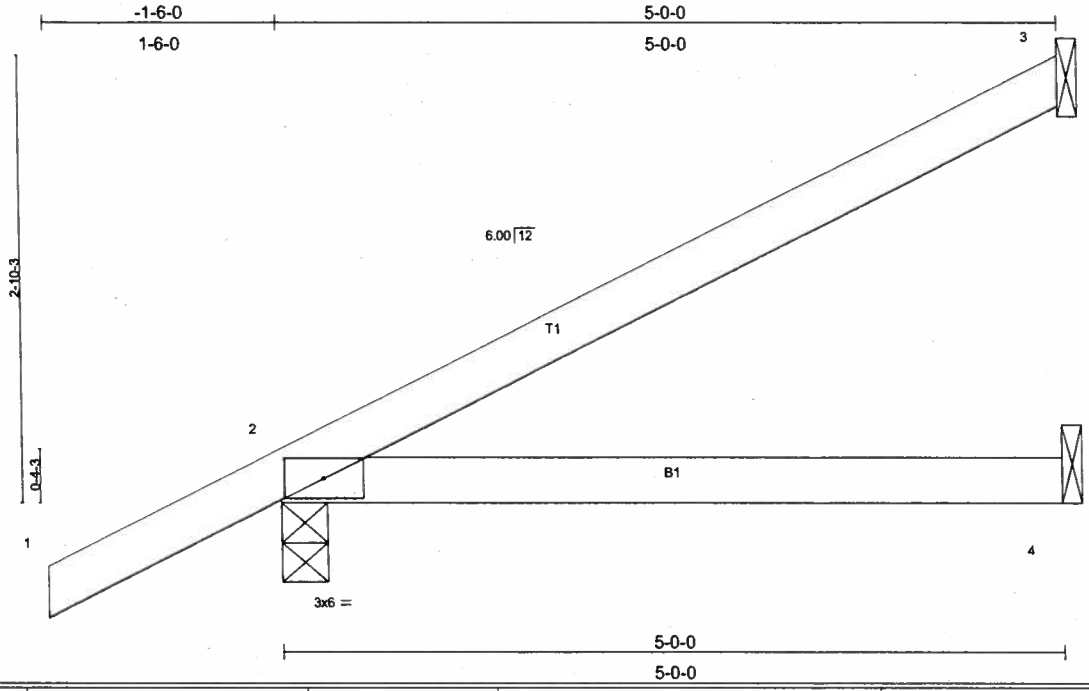
LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061653

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	CJ5	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:45 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-157(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 157 lb uplift at joint 2.

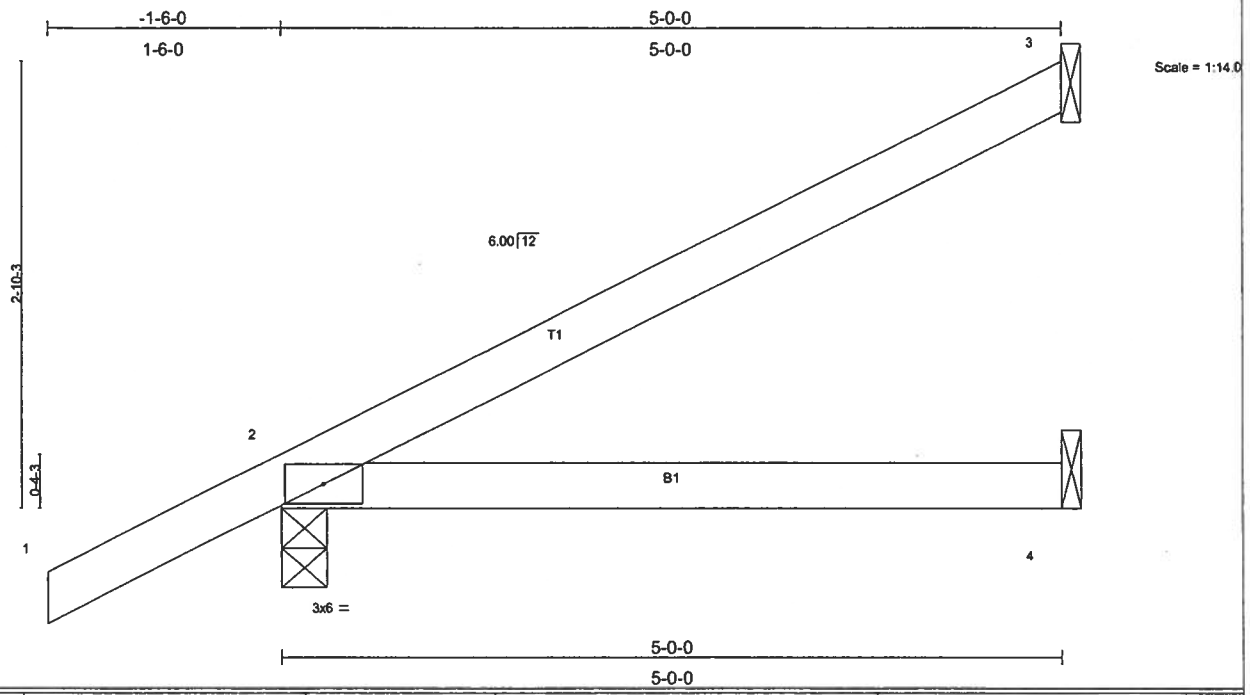
LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061654

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	EJ5	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:45 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-218(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3, 218 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061655

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	EJ7	MONO TRUSS	23	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:46 2006 Page 1

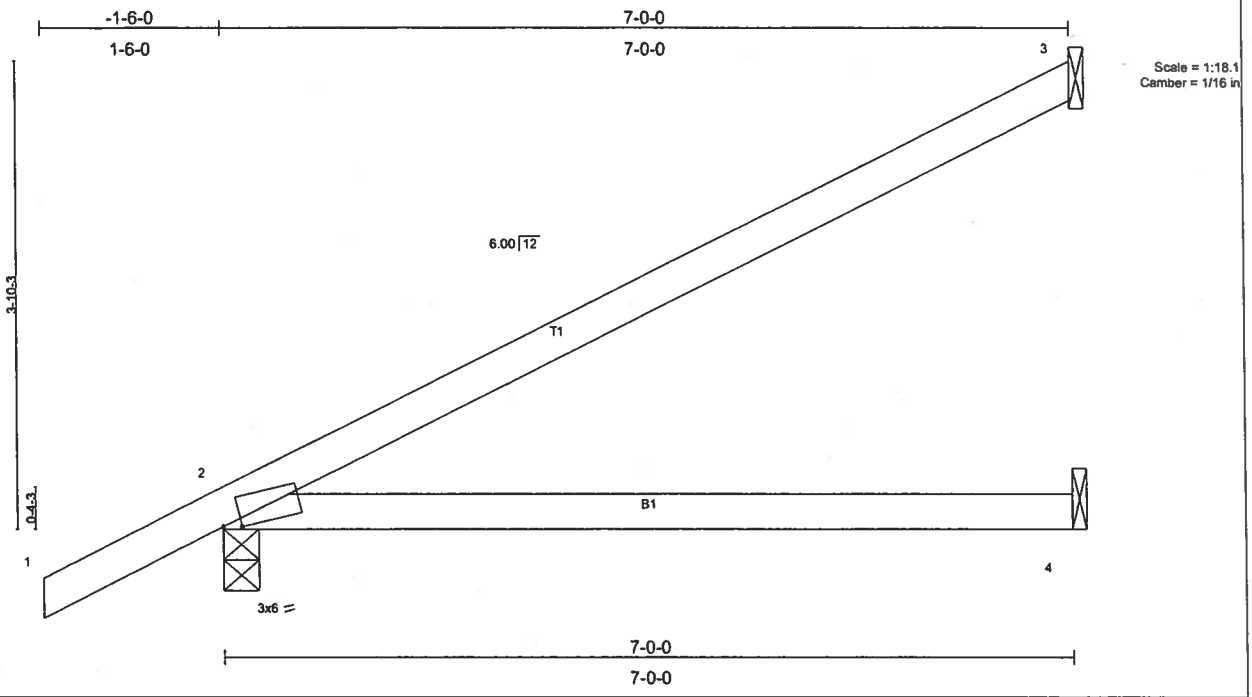


Plate Offsets (X,Y): [2:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=208(load case 5)
Max Uplift3=-139(load case 5), 2=-172(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-122/59
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 3 and 172 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877 BYRON K. ANDERSON PE 60987

Dwg.#0125061656

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	EJ7A	COMMON	2	1	Job Reference (optional)

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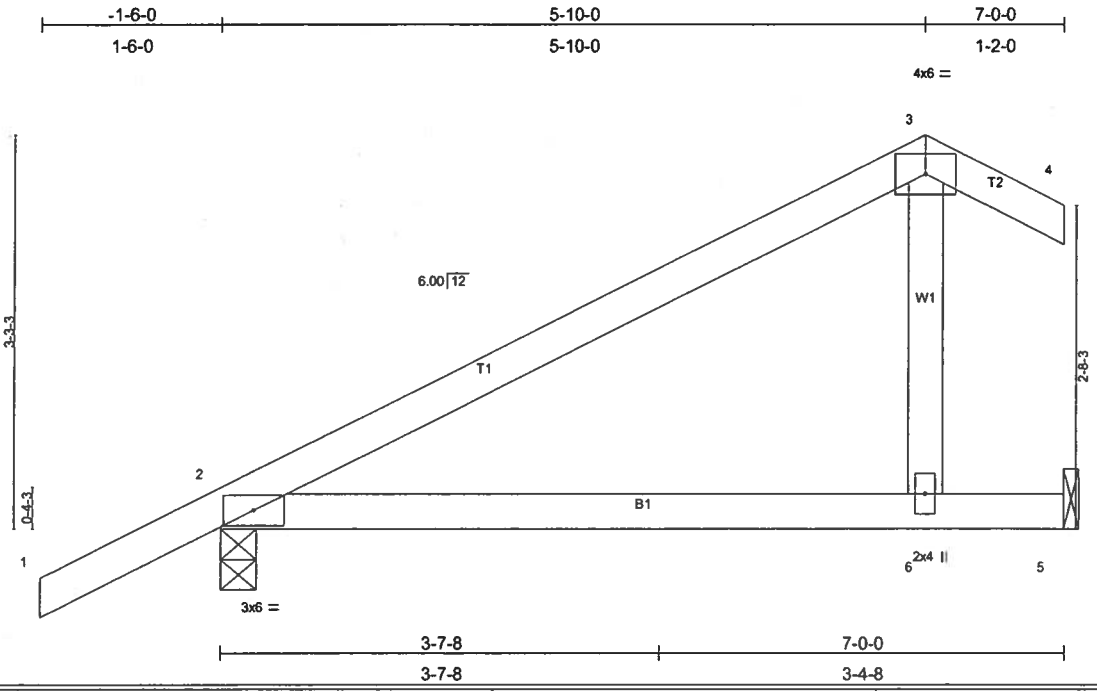


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.20	2-6	>410	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.31	2-6	>260	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=385/0-3-8, 5=278/Mechanical
Max Horz 2=153(load case 5)
Max Uplift2=-196(load case 5), 5=-119(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-59/58, 3-4=0/28
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=-220/256

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 196 lb uplift at joint 2 and 119 lb uplift at joint 5.

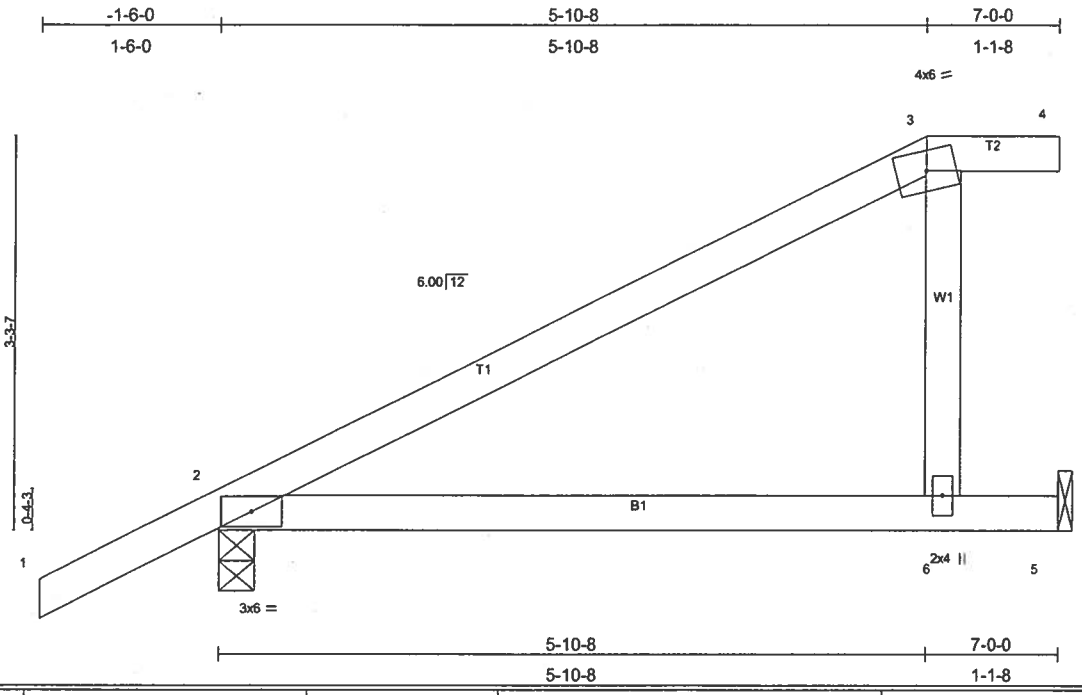
LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061657

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	EJ7B	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:47 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.18	2-6	>464	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.29	2-6	>284	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=385/0-3-8, 5=278/Mechanical
Max Horz 2=184(load case 5)
Max Uplift2=-184(load case 5), 5=-131(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-97/46, 3-4=-0/0
BOT CHORD 2-6=-12/10, 5-6=0/0
WEBS 3-6=-212/244

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 2 and 131 lb uplift at joint 5.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061658

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	EJ7C	COMMON	2	1	Job Reference (optional)

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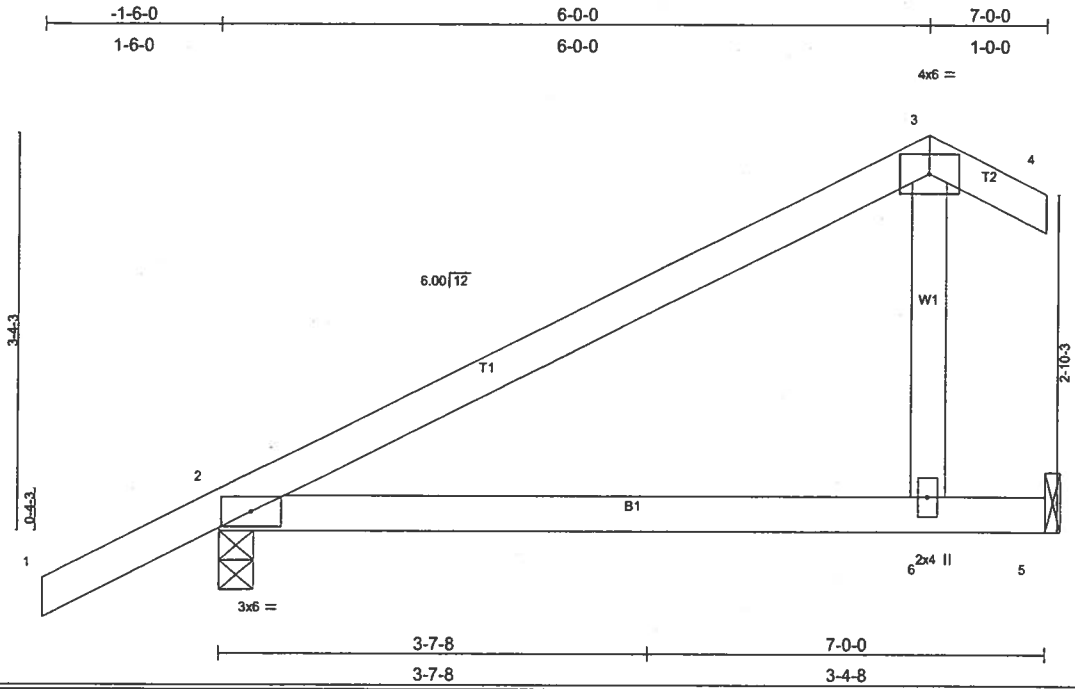

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.18	2-6	>451	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.30	2-6	>276	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=385/0-3-8, 5=278/Mechanical
Max Horz 2=161(load case 5)
Max Uplift2=-193(load case 5), 5=-122(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-67/58, 3-4=0/24
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=-211/247

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2 and 122 lb uplift at joint 5.

LOAD CASE(S) Standard

Dwg.#0125061659

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	EJ7D	MONO HIP	1	1	Job Reference (optional)

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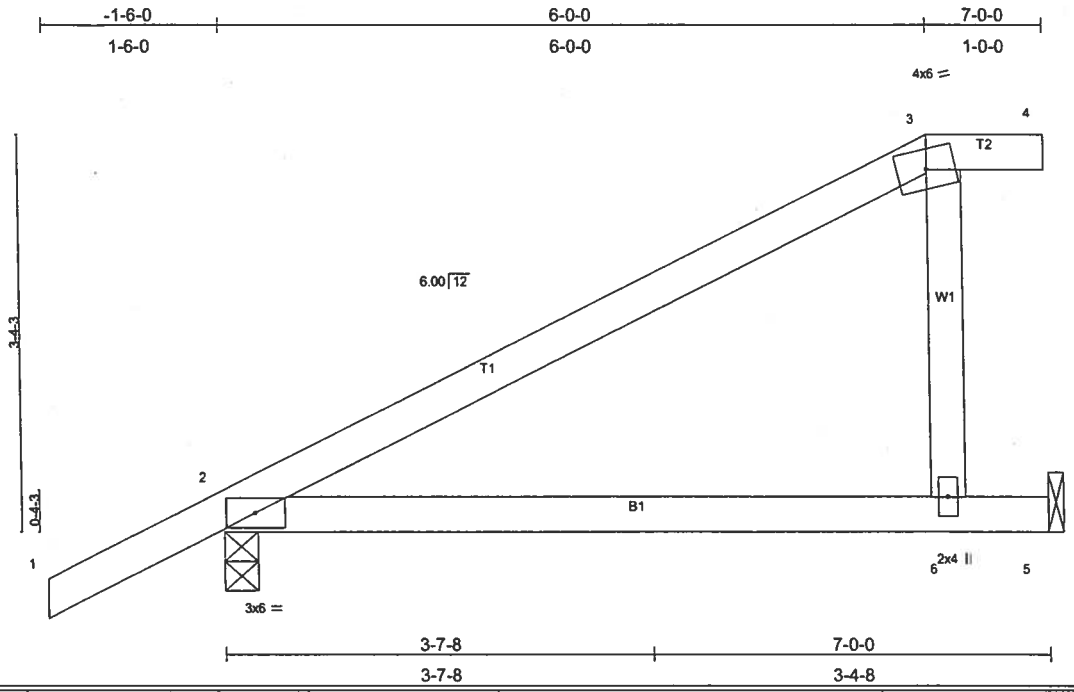

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.17	2-6	>486	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.27	2-6	>296	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=385/0-3-8, 5=278/Mechanical
Max Horz 2=187(load case 5)
Max Uplift2=-182(load case 5), 5=-132(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-98/47, 3-4=-0/0
BOT CHORD 2-6=-11/10, 5-6=0/0
WEBS 3-6=-206/239

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 182 lb uplift at joint 2 and 132 lb uplift at joint 5.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877 BYRON K. ANDERSON PE 60087

Dwg.#0125061660

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	HJ7	MONO TRUSS	2	1	Job Reference (optional)

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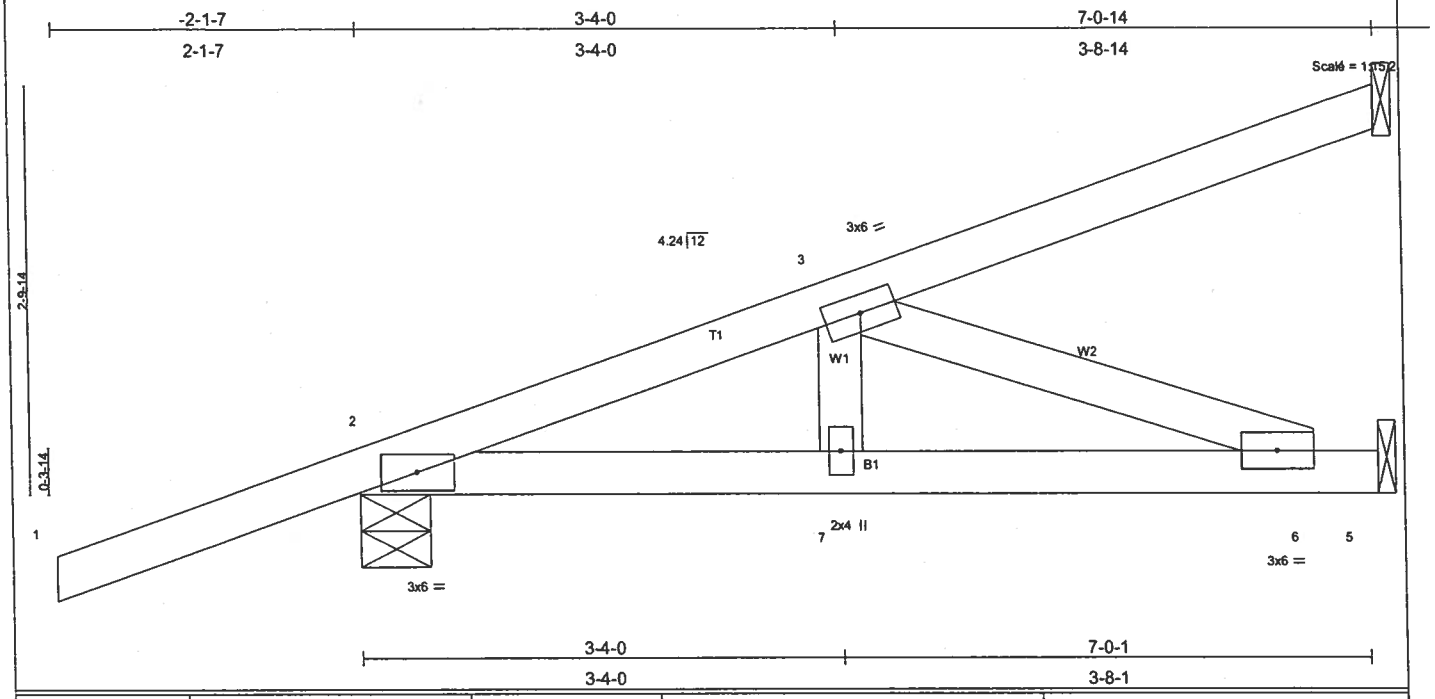

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.02	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.04	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.09	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 31 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=133/Mechanical, 2=324/0-5-11, 5=184/Mechanical
Max Horz 2=148(load case 2)
Max Uplift4=-112(load case 2), 2=-279(load case 2), 5=-105(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-369/157, 3-4=-51/33
BOT CHORD 2-7=-232/331, 6-7=-232/331, 5-6=0/0
WEBS 3-7=-75/85, 3-6=-353/247

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 4, 279 lb uplift at joint 2 and 105 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-5=-53(F=-12, B=-12)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 66877, BYRON K. ANDERSON PE 60087

Dwg.#0125061661

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	HJ9	MONO TRUSS	1	1	Job Reference (optional)

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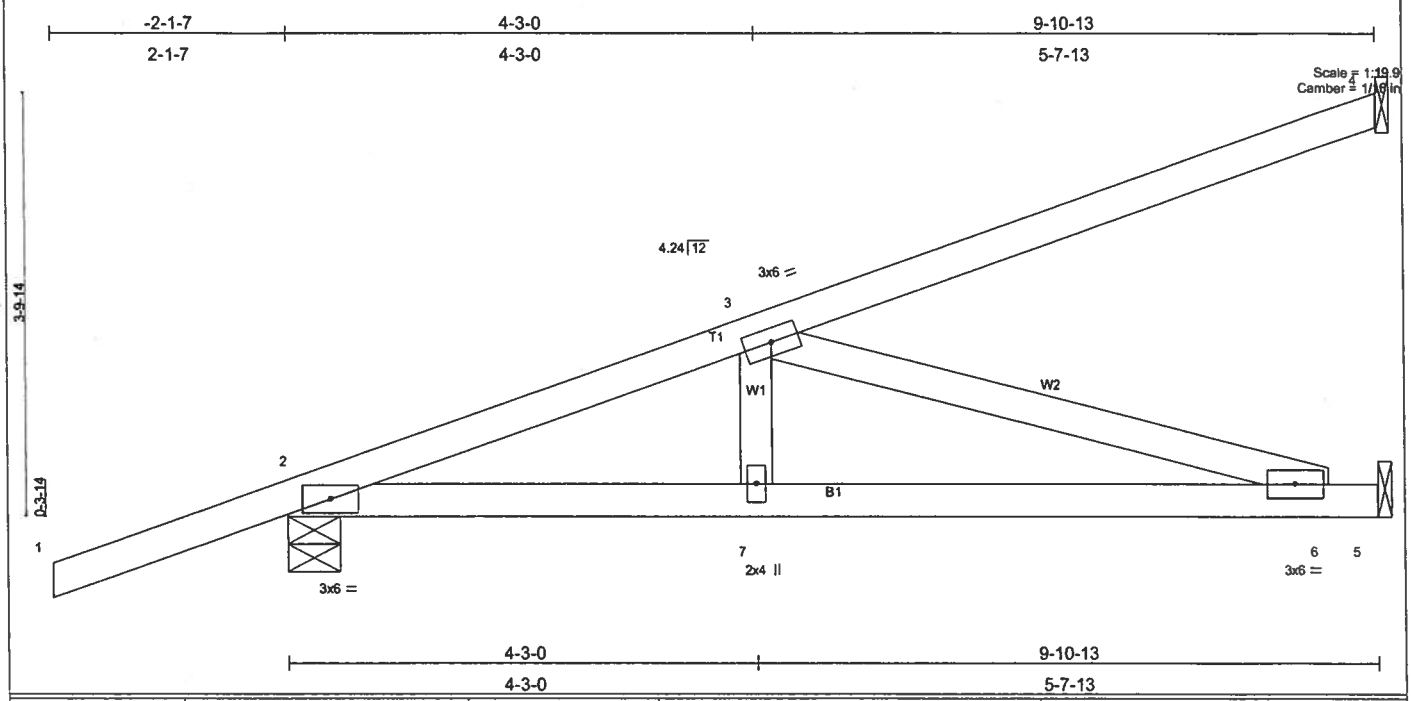

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.18	6-7	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.50	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-9 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=386/Mechanical
Max Horz 2=253(load case 2)
Max Uplift4=-230(load case 2), 2=-229(load case 2), 5=-77(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-934/180, 3-4=-104/65
BOT CHORD 2-7=-370/869, 6-7=-370/869, 5-6=0/0
WEBS 3-7=0/208, 3-6=-906/385

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 229 lb uplift at joint 2 and 77 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

Dwg.#0125061662

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T01	HIP	1	1	Job Reference (optional)

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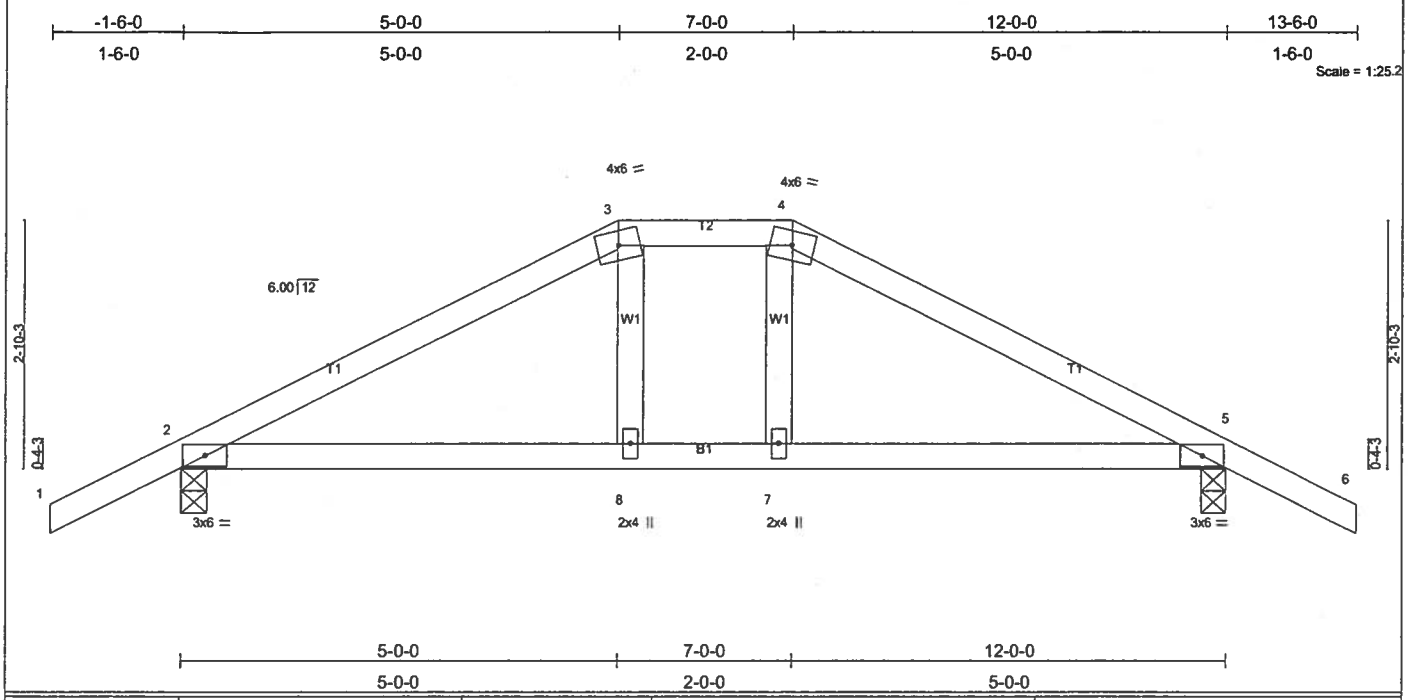

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.05	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.08	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.13	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-11 oc bracing.

REACTIONS (lb/size) 2=880/0-3-8, 5=880/0-3-8
Max Horz 2=64(load case 4)
Max Uplift2=-554(load case 4), 5=-554(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1340/714, 3-4=-1155/679, 4-5=-1340/713, 5-6=0/35
BOT CHORD 2-8=-569/1134, 7-8=-579/1155, 5-7=-567/1134
WEBS 3-8=-212/415, 4-7=-212/415

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 554 lb uplift at joint 2 and 554 lb uplift at joint 5.
5) Girder carries hip end with 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 7-0-0, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-30, 7-8=-50(F=-20), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-245(F) 7=-245(F)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:

Dwg.#0125061663

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T02	COMMON	3	1	Job Reference (optional)

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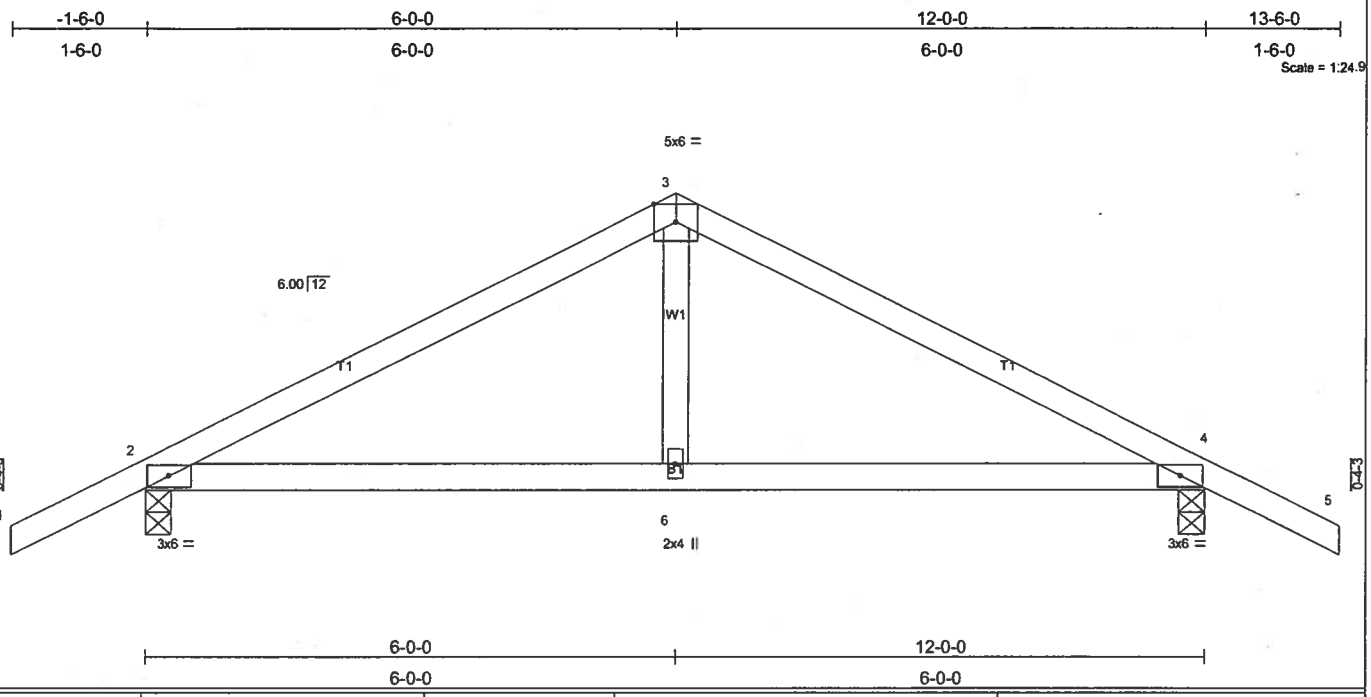

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	0.09	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.07	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-13 oc bracing.

REACTIONS (lb/size) 2=581/0-3-8, 4=581/0-3-8
Max Horz 2=70(load case 5)
Max Uplift2=-404(load case 5), 4=-404(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-684/765, 3-4=-684/765, 4-5=0/35
BOT CHORD 2-6=-536/551, 4-6=-536/551
WEBS 3-6=-371/217

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 4.

LOAD CASE(S) Standard

Dwg.#0125061664

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T03A	COMMON	1	2	Job Reference (optional)

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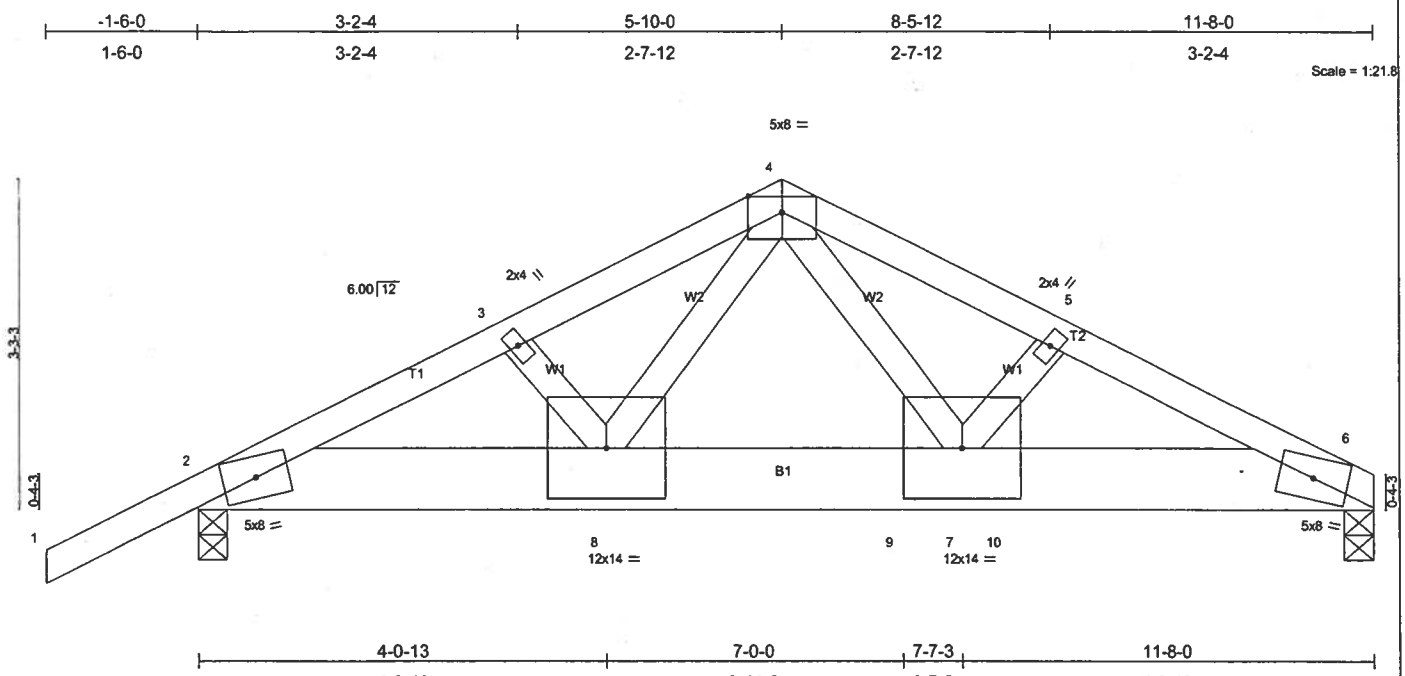

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.08	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.12	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=4987/0-3-8, 2=2439/0-3-8
Max Horz 2=95(load case 4)
Max Uplift6=-1858(load case 5), 2=-961(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-4979/1807, 3-4=-4899/1811, 4-5=-7567/2838, 5-6=-7647/2844
BOT CHORD 2-8=-1611/4425, 8-9=-1398/3919, 7-9=-1398/3919, 7-10=-2517/6840, 6-10=-2517/6840
WEBS 3-8=-84/101, 4-8=-292/867, 4-7=-2018/5356, 5-7=-122/119

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1858 lb uplift at joint 6 and 961 lb uplift at joint 2.
6) Girder carries tie-in span(s): 39-8-0 from 8-0-0 to 11-8-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3618 lb down and 1366 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-10=-30, 6-10=-815(F=-785)
Concentrated Loads (lb)
Vert: 9=-3618(F)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:

Dwg.#0125061665

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T03G	COMMON	1	1	Job Reference (optional)

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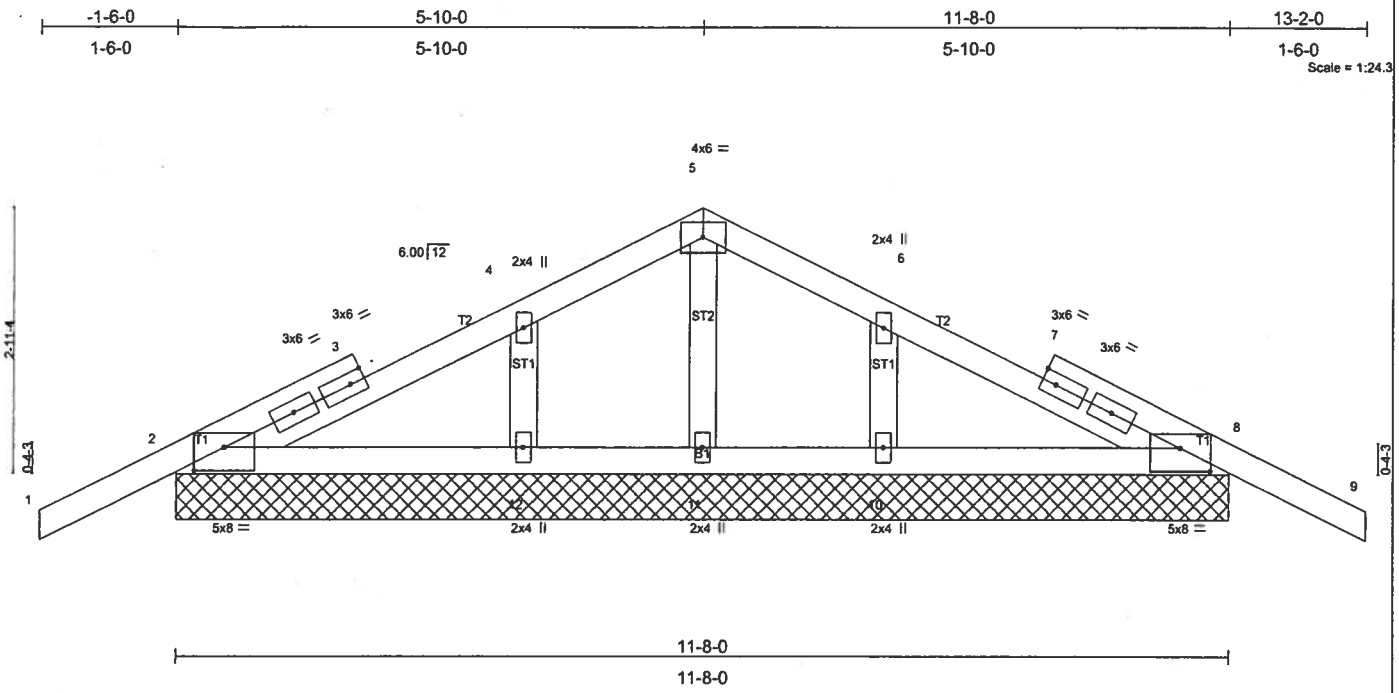


Plate Offsets (X,Y): [2:0-4-0,0-3-1], [8:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.00	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 54 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=325/11-8-0, 8=325/11-8-0, 11=188/11-8-0, 12=394/11-8-0, 10=394/11-8-0
Max Horz 2=65(load case 5)
Max Uplift2=-185(load case 5), 8=-195(load case 6), 11=-28(load case 5), 12=-155(load case 5), 10=-157(load case 6)
Max Grav 2=330(load case 9), 8=330(load case 10), 11=188(load case 1), 12=396(load case 9), 10=396(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5/56, 2-3=-43/66, 3-4=-48/141, 4-5=0/93, 5-6=0/93, 6-7=-24/141, 7-8=-20/66, 8-9=-5/56
BOT CHORD 2-12=-62/122, 11-12=-62/122, 10-11=-62/122, 8-10=-62/122
WEBS 5-11=-168/37, 4-12=-284/226, 6-10=-284/226

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 2, 195 lb uplift at joint 8, 28 lb uplift at joint 11, 155 lb uplift at joint 12 and 157 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-87(F=-33), 5-9=-87(F=-33), 2-8=-30

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:

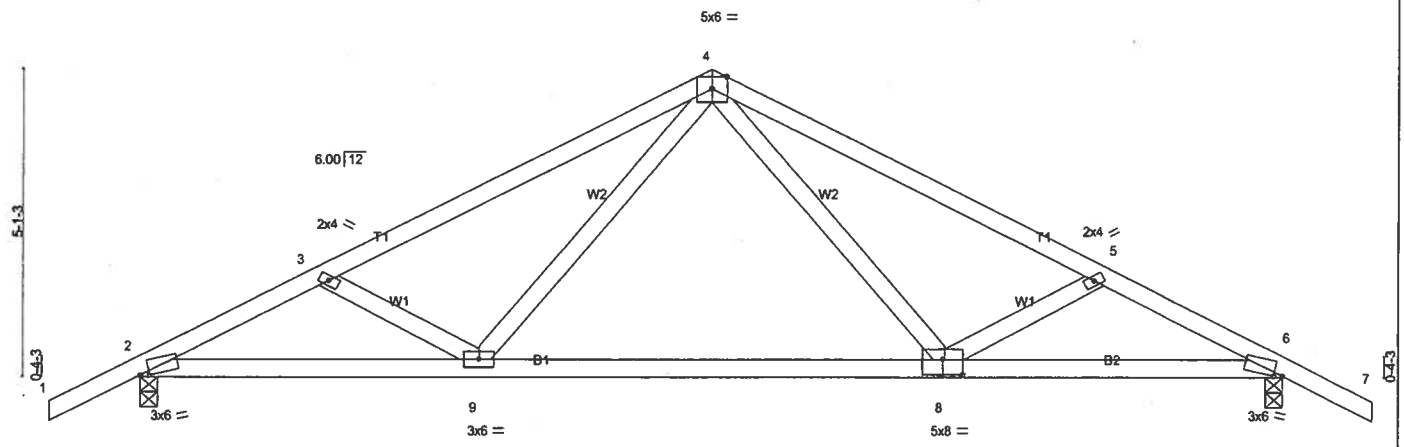
Dwg.#0125061666

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T04	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:53 2006 Page 1

-1-6-0 | 3-1-11 | 9-6-0 | 15-10-5 | 19-0-0 | 20-6-0
1-6-0 | 3-1-11 | 6-4-5 | 6-4-5 | 3-1-11 | 1-6-0

Scale = 1:36.5
Camber = 3/16 in



5-7-10 | 13-4-6 | 19-0-0
5-7-10 | 7-8-12 | 5-7-10

Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.28	8-9	>793	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.46	8-9	>487	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.23	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-10 oc bracing.

REACTIONS (lb/size) 2=1068/0-3-8, 6=1068/0-3-8
Max Horz 2=-95(load case 6)
Max Uplift2=-424(load case 5), 6=-424(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1886/822, 3-4=-1714/722, 4-5=-1714/722, 5-6=-1886/822, 6-7=0/35
BOT CHORD 2-9=-622/1630, 8-9=-292/1038, 6-8=-622/1630
WEBS 3-9=-223/229, 4-9=-210/712, 4-8=-210/712, 5-8=-223/229

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 424 lb uplift at joint 2 and 424 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-9=-30, 8-9=-80(F=-50), 6-8=-30

Dwg.#0125061667

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T04A	COMMON	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industnes, Inc. Mon Jan 23 11:51:54 2006 Page 1

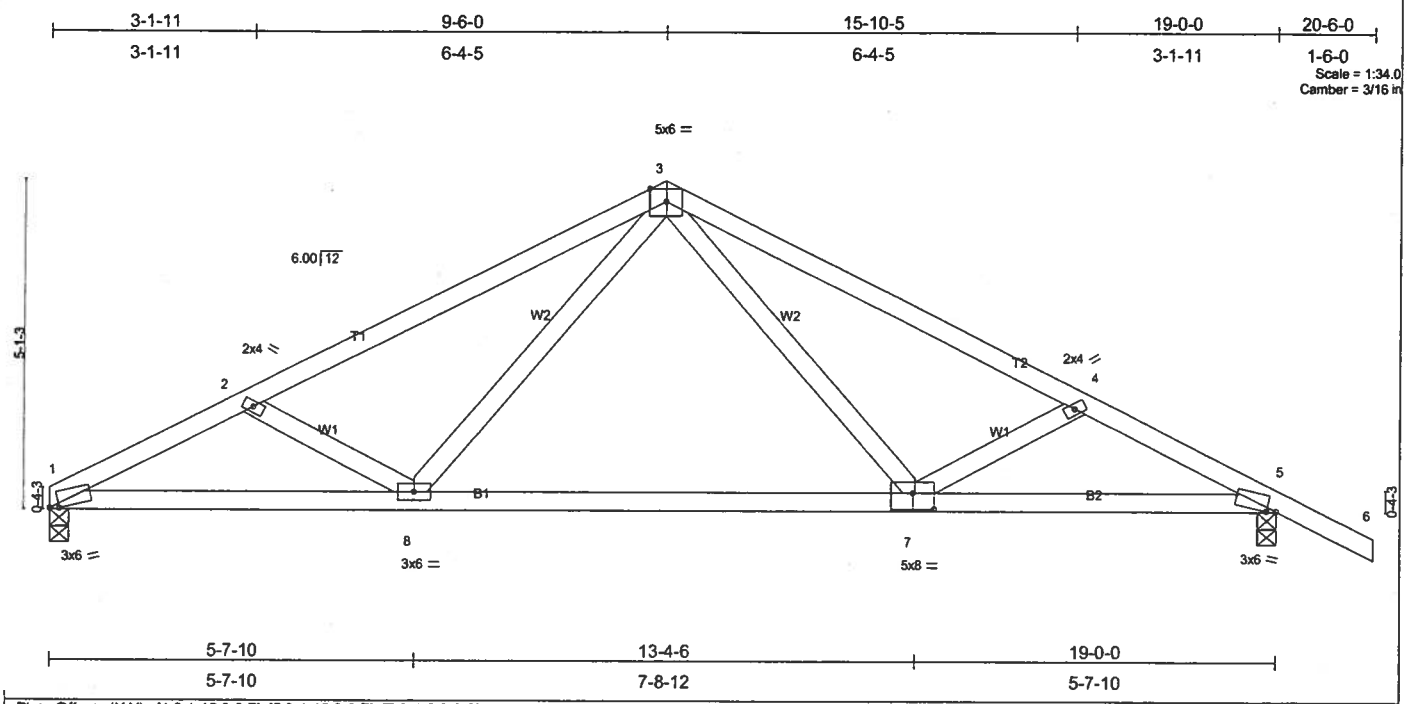


Plate Offsets (X,Y): [1:0-1-12,0-0-7], [5:0-1-12,0-0-7], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.28	7-8	>797	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.46	7-8	>489	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-6-8 oc bracing.

REACTIONS (lb/size) 1=975/0-3-8, 5=1072/0-3-8
Max Horz 1=-113(load case 6)
Max Uplift1=-327(load case 5), 5=-426(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1919/878, 2-3=-1737/761, 3-4=-1721/734, 4-5=-1892/834, 5-6=0/35
BOT CHORD 1-8=-684/1666, 7-8=-305/1045, 5-7=-633/1635
WEBS 2-8=-241/260, 3-8=-245/733, 3-7=-207/710, 4-7=-222/229

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 327 lb uplift at joint 1 and 426 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-8=-30, 7-8=-80(F=-50), 5-7=-30

Dwg.#0125061668

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T04B	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:54 2006 Page 1

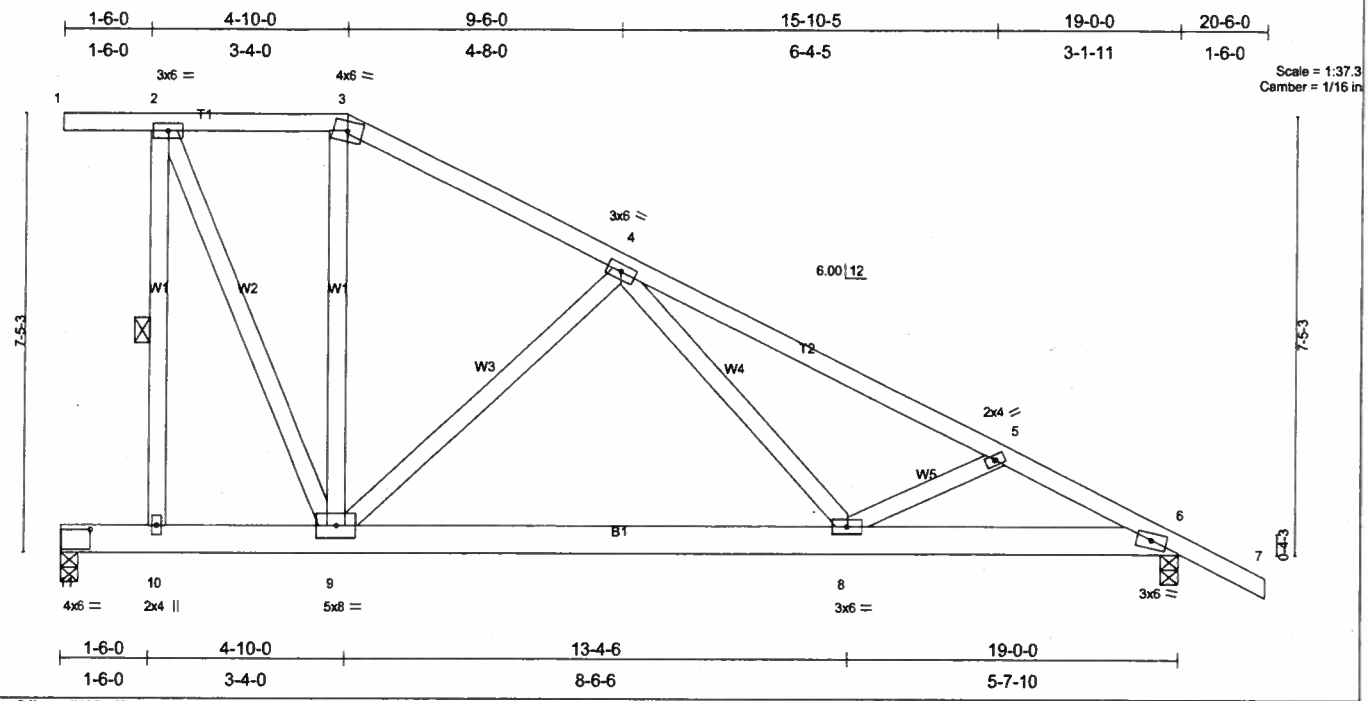


Plate Offsets (X,Y): [8:12-10-3,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.11	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.17	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 131 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-4-14 oc bracing.
WEBS 1 Row at midpt 2-10

REACTIONS (lb/size) 11=790/0-3-8, 6=879/0-3-8
Max Horz 11=-377(load case 6)
Max Uplift11=-288(load case 6), 6=-324(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/0, 2-3=-427/214, 3-4=-546/196, 4-5=-1255/363, 5-6=-1493/523, 6-7=0/39
BOT CHORD 10-11=0/424, 9-10=0/424, 8-9=-48/786, 6-8=-378/1325
WEBS 2-9=-540/1079, 3-9=0/60, 4-8=-75/424, 4-9=-503/359, 5-8=-307/322, 2-10=-1174/645

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 288 lb uplift at joint 11 and 324 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60087

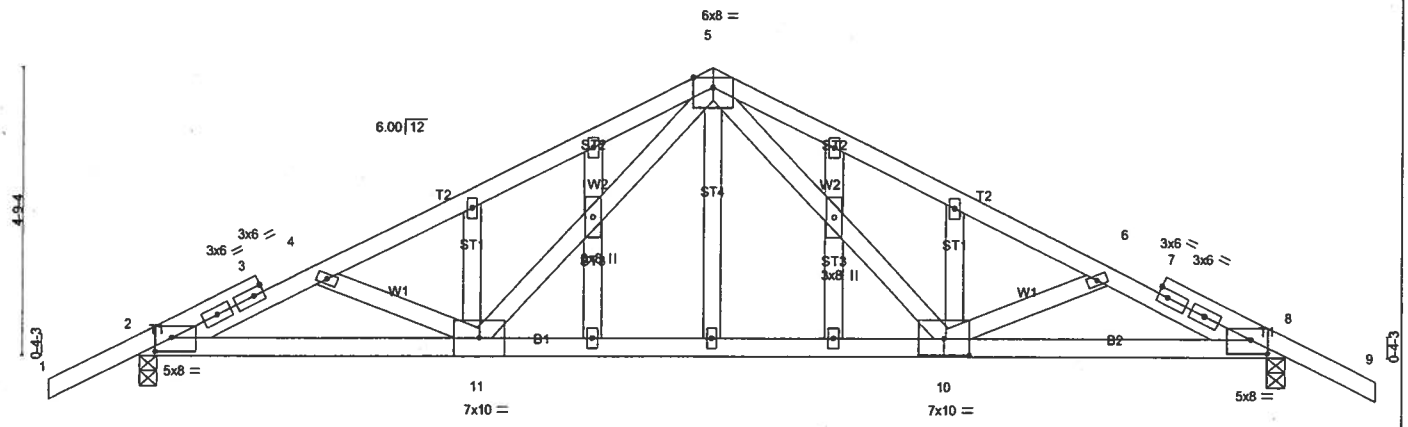
Dwg.#0125061669

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T04G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:55 2006 Page 1

-1-6-0 | 3-1-11 | 9-6-0 | 15-10-5 | 19-0-0 | 20-6-0
1-6-0 | 3-1-11 | 6-4-5 | 6-4-5 | 3-1-11 | 1-6-0

Scale = 1:36.5
Camber = 1/16 in



5-7-10 | 13-4-6 | 19-0-0
5-7-10 | 7-8-12 | 5-7-10

Plate Offsets (X,Y): [2:0-3-4,0-2-12], [8:0-3-4,0-2-12], [10:0-5-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.14	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.23	10-11	>957	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.18	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 116 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-6-10 oc bracing.

REACTIONS (lb/size) 2=985/0-3-8, 8=985/0-3-8
Max Horz 2=-90(load case 6)
Max Uplift2=-394(load case 5), 8=-394(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-1956/848, 3-4=-1885/848, 4-5=-1546/618, 5-6=-1546/618, 6-7=-1885/848, 7-8=-1956/848, 8-9=0/42
BOT CHORD 2-11=-699/1782, 10-11=-259/938, 8-10=-699/1782
WEBS 4-11=-542/379, 5-11=-143/571, 5-10=-143/571, 6-10=-542/379

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 2 and 394 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-64(F=-10), 5-9=-64(F=-10), 2-8=-30

Dwg.#0125061670

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T05	HIP	1	2	Job Reference (optional)

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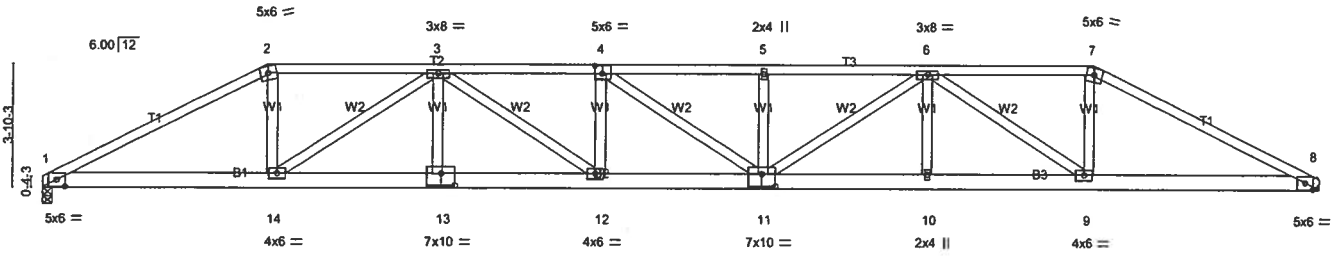
3-9-4	7-0-0	12-2-10	17-3-9	22-4-7	27-5-6	32-8-0	35-10-12	39-8-0
3-9-4	3-2-12	5-2-10	5-0-14	5-0-14	5-0-14	5-2-10	3-2-12	3-9-4

Scale = 1:68.3
Camber = 5/16 in



3-9-4	7-0-0	12-2-10	17-3-9	22-4-7	27-5-6	32-8-0	35-10-12	39-8-0
3-9-4	3-2-12	5-2-10	5-0-14	5-0-14	5-0-14	5-2-10	3-2-12	3-9-4

Plate Offsets (X,Y): [1:0-3-0,0-2-9], [4:0-2-12,0-3-0], [8:0-3-0,0-2-9], [11:0-5-0,0-4-8], [13:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.48	11-12	>975	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.78	11-12	>610	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.15	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 449 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-9-11 oc bracing.

REACTIONS (lb/size) 1=3618/0-3-8, 8=3618/Mechanical
Max Horz 1=50(load case 3)
Max Uplift1=-1410(load case 3), 8=-1409(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6857/2799, 2-3=-6181/2582, 3-4=-10129/4363, 4-5=-10107/4350, 5-6=-10113/4355, 6-7=-6194/2588, 7-8=-6869/2804
BOT CHORD 1-14=-2495/6074, 13-14=-3725/8763, 12-13=-3722/8771, 11-12=-4319/10135, 10-11=-3687/8781, 9-10=-3687/8781, 8-9=-2451/6087
WEBS 2-14=-951/2552, 3-14=-3229/1508, 3-13=0/253, 3-12=-749/1673, 4-12=-596/478, 4-11=-80/43, 5-11=-583/474, 6-11=-729/1638, 6-10=0/276, 6-9=-3235/1507, 7-9=-950/2550

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1410 lb uplift at joint 1 and 1409 lb uplift at joint 8.
8) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 7-0-0; 7-0-0 from 32-8-0 to 39-8-0
9) Girder carries hip end with 7-0-0 end setback.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-7=-118(F=-64), 7-8=-54, 1-14=-130(F=-100), 9-14=-65(F=-35), 8-9=-130(F=-100)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061671

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T06	HIP	1	1	Job Reference (optional)

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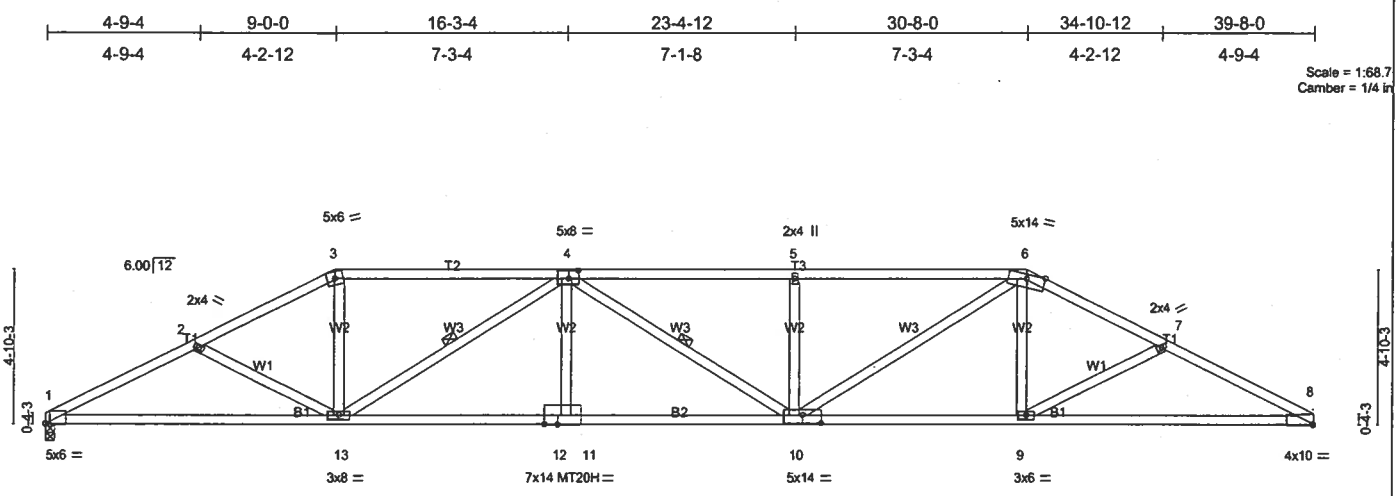

Plate Offsets (X,Y): [1:0-1-11,Edge], [4:0-3-12,0-3-0], [8:0-0-0,0-0-4], [10:0-7-0,0-3-0], [12:0-4-15,0-0-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.41	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.67	10-11	>708	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 197 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-13, 4-10

REACTIONS (lb/size) 1=1657/0-3-8, 8=1657/Mechanical
Max Horz 1=-66(load case 3)
Max Uplift1=-456(load case 5), 8=-457(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3144/1333, 2-3=-2926/1222, 3-4=-2609/1158, 4-5=-3545/1534, 5-6=-3545/1534, 6-7=-2944/1230, 7-8=-3183/1351
BOT CHORD 1-13=-1113/2758, 12-13=-1303/3546, 11-12=-1303/3546, 10-11=-1304/3543, 9-10=-925/2606, 8-9=-1133/2804
WEBS 2-13=-205/221, 3-13=-296/954, 4-13=-1194/476, 4-11=0/202, 4-10=-69/73, 5-10=-394/278, 6-10=-478/1195, 6-9=-64/378, 7-9=-239/236

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 456 lb uplift at joint 1 and 457 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061672

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T07	HIP	1	1	Job Reference (optional)

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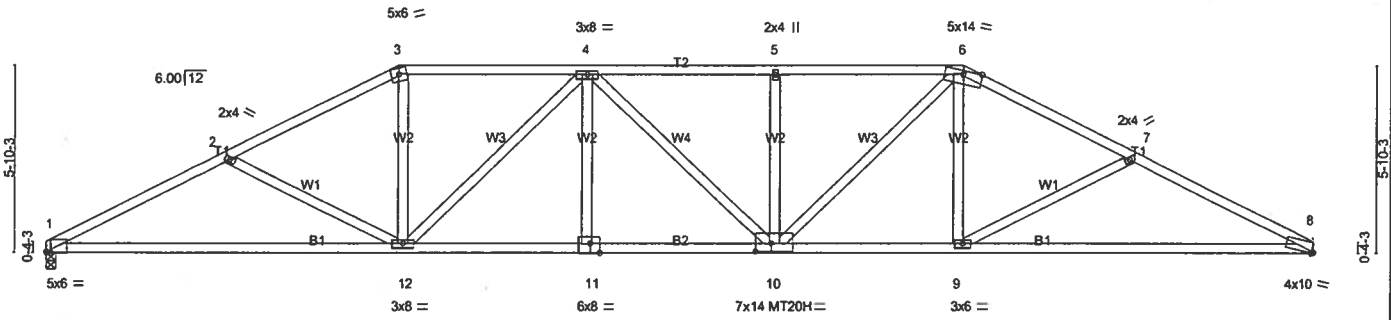
5-9-4	11-0-0	16-10-11	22-9-5	28-8-0	33-10-12	39-8-0
5-9-4	5-2-12	5-10-11	5-10-11	5-10-11	5-2-12	5-9-4

Scale = 1:68.7
Camber = 5/16 in



11-0-0	16-10-11	22-9-5	28-8-0	39-8-0
11-0-0	5-10-11	5-10-11	5-10-11	11-0-0

Plate Offsets (X,Y): [1:0-1-11,Edge], [8:0-0-13,Edge], [10:0-5-14,0-3-0], [11:0-3-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.48	8-9	>982	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.81	8-9	>584	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.18	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 205 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 1=1657/0-3-8, 8=1657/Mechanical
Max Horz 1=-81(load case 3)
Max Uplift1=-475(load case 5), 8=-476(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3091/1343, 2-3=-2770/1177, 3-4=-2439/1119, 4-5=-2914/1321, 5-6=-2920/1325, 6-7=-2783/1182, 7-8=-3122/1356
BOT CHORD 1-12=-1112/2718, 11-12=-1038/2907, 10-11=-1037/2910, 9-10=-836/2438, 8-9=-1127/2754
WEBS 2-12=-345/318, 3-12=-291/913, 4-12=-756/330, 4-11=0/135, 4-10=-78/87, 5-10=-330/229, 6-10=-334/764, 6-9=-102/475, 7-9=-371/330

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 1 and 476 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987

Dwg.#0125061673

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:58 2006 Page 1

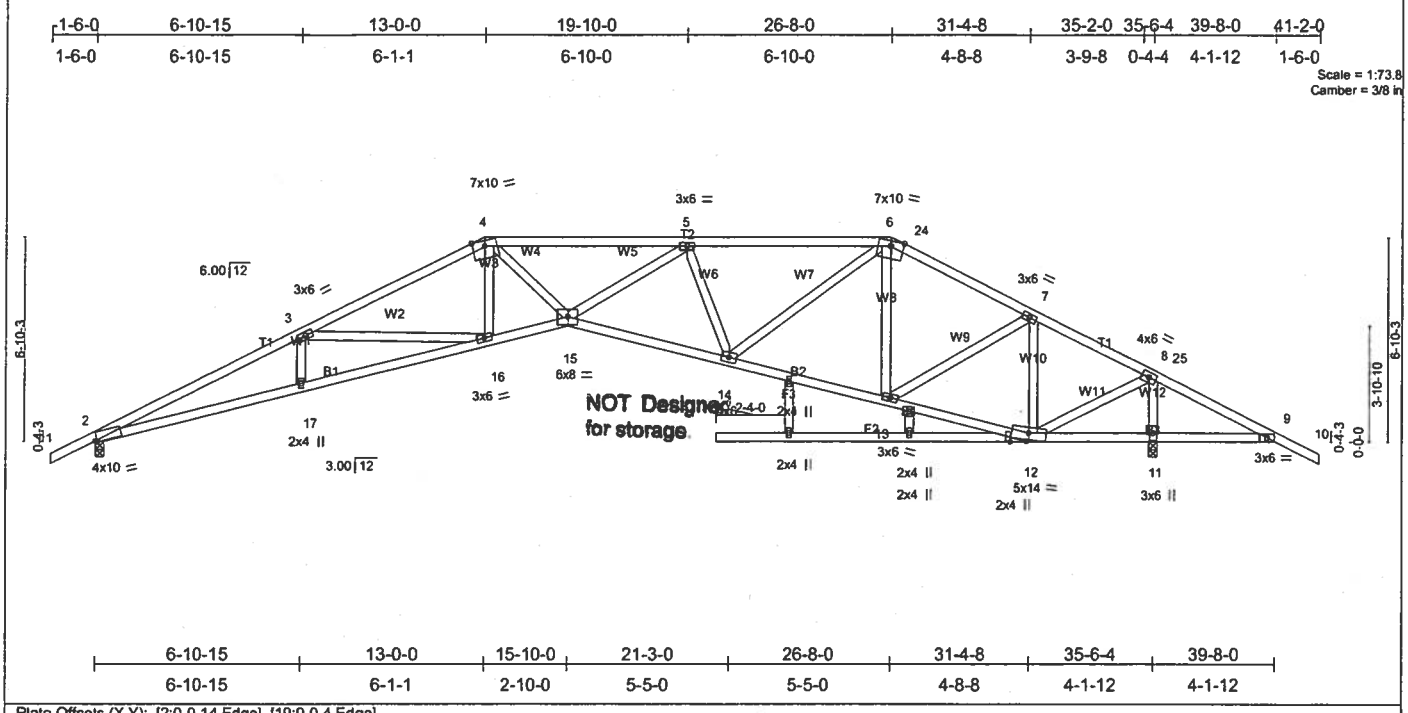


Plate Offsets (X,Y): [2:0-0-14,Edge], [19:0-0-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.60	14-15	>710	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.96	14-15	>443	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.59	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 223 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B3 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W11 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-0-6 oc bracing.
JOINTS 1 Brace at Jt(s): 13

REACTIONS (lb/size) 2=1629/0-3-8, 11=2711/0-3-8
Max Horz 2=-119(load case 6)
Max Uplift2=-590(load case 5), 11=-1216(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-5357/2093, 3-4=-4385/1697, 4-5=-5111/1963, 5-6=-3428/1464, 6-24=-2304/1081, 7-24=-2566/1192, 7-25=-1909/889, 8-25=-1967/933, 8-9=-558/521, 9-10=0/35
BOT CHORD 2-17=-1754/4848, 16-17=-1756/4845, 15-16=-1234/3995, 14-15=-1280/3978, 13-14=-696/2235, 12-13=-536/1630, 11-12=-404/592, 9-11=-404/592
WEBS 3-17=0/199, 3-16=-827/506, 4-16=-138/413, 4-15=-512/1694, 5-15=-401/1517, 5-14=-1264/498, 6-14=-499/1591, 6-13=-335/168, 7-13=-198/690, 7-12=-1301/757, 8-12=-1272/2257, 8-11=-2560/1606

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 590 lb uplift at joint 2 and 1216 lb uplift at joint 11.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 110 lb down and 101 lb up at 35-2-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 6-24=-54, 10-25=-54, 2-15=-30, 12-15=-30, 9-12=-30
Concentrated Loads (lb)
Vert: 25=-110(F)
Trapezoidal Loads (plf)
Vert: 24=-119(F=-65)-to-25=-174(F=-120)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987

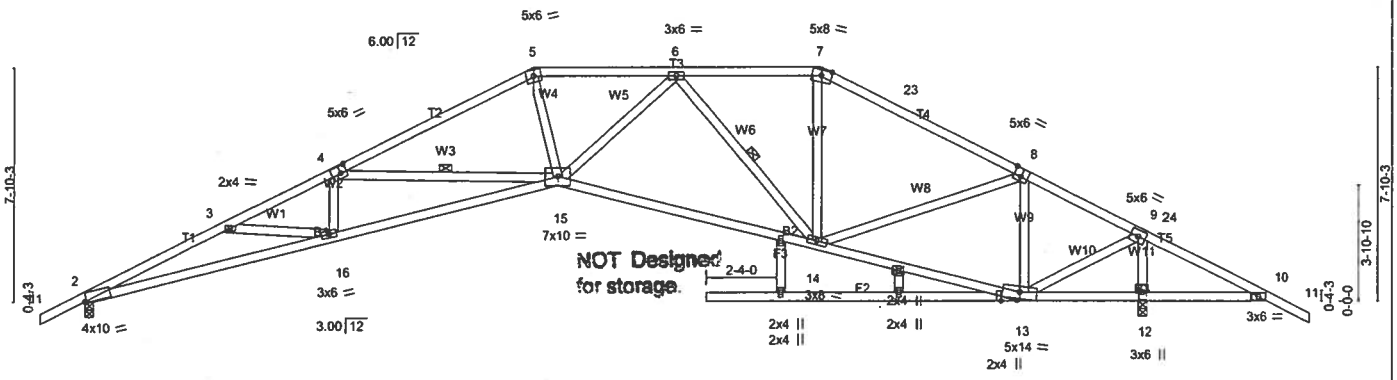
Dwg.#0125061674

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:51:59 2006 Page 1

1-6-0	7-10-15	15-0-0	19-10-0	24-8-0	31-4-8	35-2-0	39-8-0	41-2-0
1-6-0	7-10-15	7-1-1	4-10-0	4-10-0	6-8-8	3-9-8	4-6-0	1-6-0

Scale = 1:73.8
Camber = 5/16 in



8-3-15	15-10-0	24-8-0	31-4-8	35-6-4	39-8-0
8-3-15	7-6-1	8-10-0	6-8-8	4-1-12	4-1-12

Plate Offsets (X,Y): [2:0-0-14,Edge], [4:0-2-8,0-3-0], [8:0-3-0,0-3-4], [18:0-0-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.52	15-16	>816	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.83	15-16	>509	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.50	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 226 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except* B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except* W10 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-10 oc bracing.
WEBS 1 Row at midpt 4-15, 6-14

REACTIONS (lb/size) 2=1629/0-3-8, 12=2711/0-3-8
Max Horz 2=-133(load case 6)
Max Uplift2=-585(load case 5), 12=-1065(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-5220/2018, 3-4=-5086/1904, 4-5=-3993/1398, 5-6=-3818/1403, 6-7=-2222/953, 7-23=-2406/961, 8-23=-2663/1003, 8-24=-1934/686, 9-24=-1991/706, 9-10=-578/538, 10-11=0/35
BOT CHORD 2-16=-1700/4734, 15-16=-1542/4703, 14-15=-778/3093, 13-14=-410/1698, 12-13=-424/616, 10-12=-424/616
WEBS 4-16=0/245, 4-15=-1044/608, 5-15=-354/1427, 6-15=-295/1151, 6-14=-1231/381, 7-14=-119/691, 8-14=-146/620, 8-13=-1316/657, 9-13=-1126/2321, 9-12=-2567/1337, 3-16=-40/154

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 585 lb uplift at joint 2 and 1065 lb uplift at joint 12.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 110 lb down and 48 lb up at 35-2-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-7=-54, 7-23=-54, 11-24=-54, 2-15=-30, 13-15=-30, 10-13=-30
Concentrated Loads (lb)
Vert: 24=-110(F)
Trapezoidal Loads (plf)
Vert: 23=-119(F=-65)-to-24=-174(F=-120)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877 BYRON K. ANDERSON PE 60987

Dwg.#0125061675

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:52:00 2006 Page 1

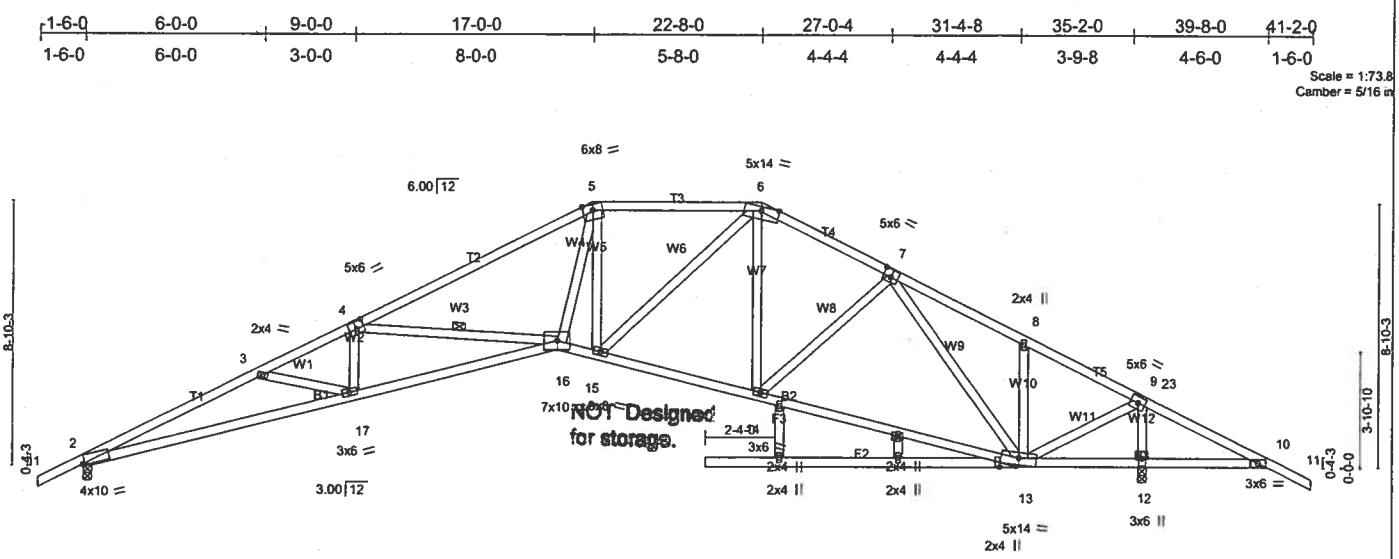


Plate Offsets (X,Y): [2:0-0-14,Edge], [4:0-3-0,0-3-0], [7:0-3-0,0-3-0], [13:0-0-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	-0.53	16-17	>807	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.84	16-17	>502	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.49	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-6 oc bracing. Except:
1 Row at midpt 13-14
WEBS 1 Row at midpt 4-16

REACTIONS (lb/size) 2=1631/0-3-8, 12=2718/0-3-8
Max Horz 2=-147(load case 6)
Max Uplift2=-598(load case 5), 12=-1083(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-5245/2033, 3-4=-5045/1924, 4-5=-3830/1365, 5-6=-2863/1176, 6-7=-2447/999, 7-8=-1948/783, 8-23=-1910/657, 9-23=-1968/677, 9-10=-564/539, 10-11=0/35
BOT CHORD 2-17=-1706/4767, 16-17=-1540/4646, 15-16=-707/2976, 14-15=-462/2218, 13-14=-570/2196, 12-13=-420/600, 10-12=-420/600
WEBS 3-17=-111/163, 4-17=-37/350, 4-16=-1162/677, 5-16=-571/2070, 5-15=-777/275, 6-15=-334/997, 6-14=-121/264, 7-14=-33/198, 7-13=-852/343, 8-13=-677/421, 9-13=-1072/2292, 9-12=-2618/1343

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 598 lb uplift at joint 2 and 1083 lb uplift at joint 12.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 110 lb down and 48 lb up at 35-2-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-6=-54, 6-7=-54, 11-23=-54, 2-16=-30, 13-16=-30, 10-13=-30
Concentrated Loads (lb)
Vert: 23=-110(F)
Trapezoidal Loads (plf)
Vert: 7=-118(F=-64)-to-23=-174(F=-120)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987

Dwg.#0125061676

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T11	SPECIAL	1	1	Job Reference (optional)

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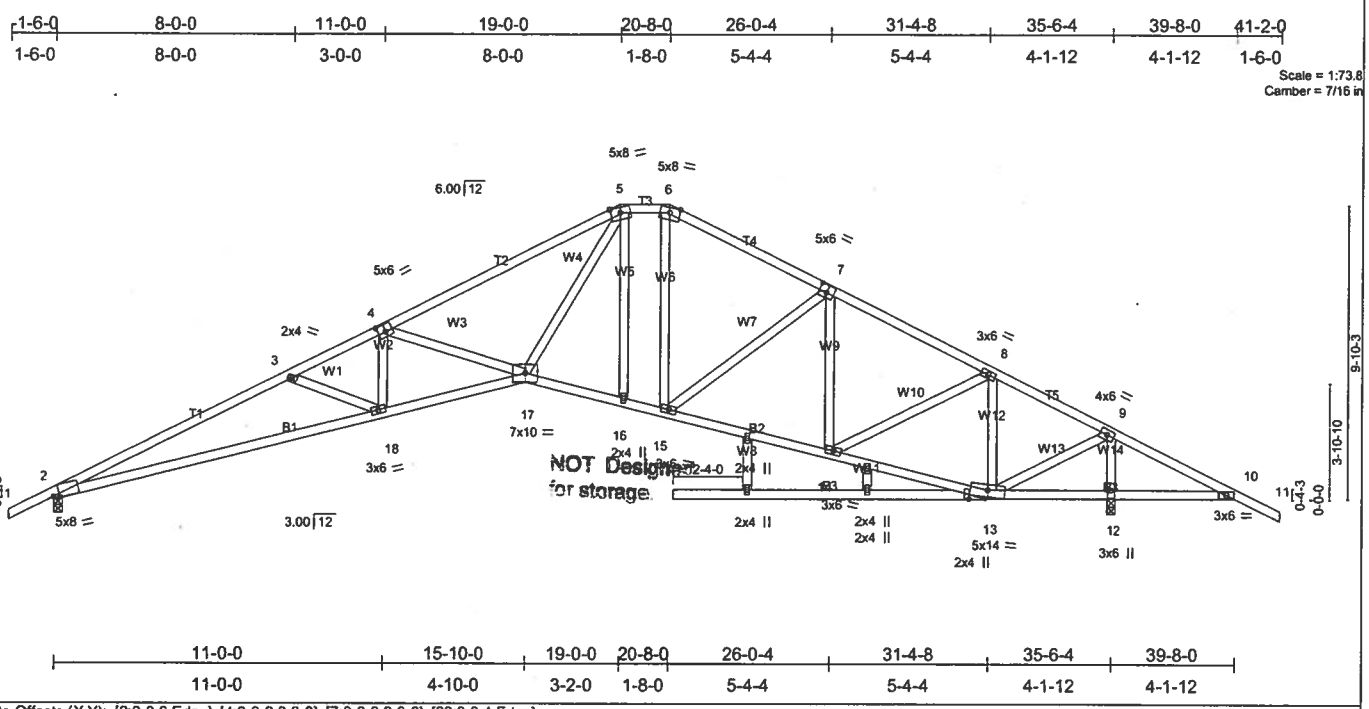


Plate Offsets (X,Y): [2:0-2-6,Edge], [4:0-3-0,0-3-0], [7:0-3-0,0-3-0], [20:0-0-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	-0.67	2-18	>638	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-1.10	2-18	>388	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.49	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 5-2-14 oc bracing.
JOINTS 1 Brace at Jt(s): 14

REACTIONS (lb/size) 2=1545/0-3-8, 12=1945/0-3-8
Max Horz 2=-161(load case 6)
Max Uplift2=-576(load case 5), 12=-804(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4780/1856, 3-4=-4437/1683, 4-5=-3457/1261, 5-6=-1873/862, 6-7=-2129/891, 7-8=-2005/794, 8-9=-1285/429, 9-10=-587/562, 10-11=0/35
BOT CHORD 2-18=-1526/4357, 17-18=-1254/4067, 16-17=-397/1985, 15-16=-313/1802, 14-15=-389/1826, 13-14=-230/1121, 12-13=-452/625, 10-12=-452/625
WEBS 3-18=-273/281, 4-18=-117/458, 4-17=-1003/571, 5-17=-708/2273, 5-16=-677/325, 6-15=-275/593, 7-15=-79/281, 7-14=-415/234, 8-14=-245/734, 8-13=-910/476, 9-13=-884/1751, 9-12=-1802/1025

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 576 lb uplift at joint 2 and 804 lb uplift at joint 12.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987

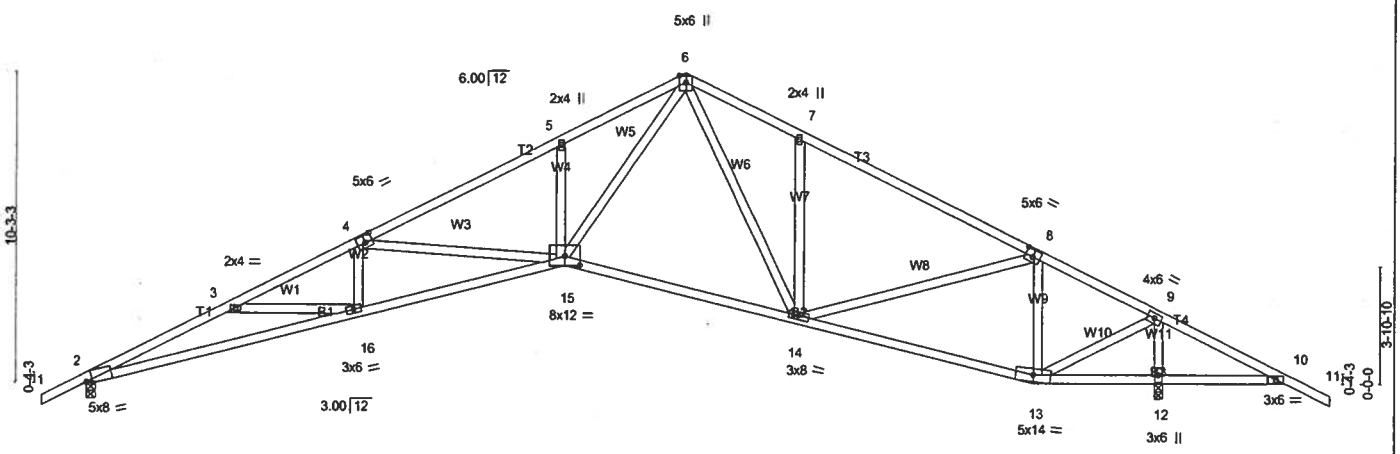
Dwg.#0125061677

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:52:01 2006 Page 1

1-6-0	4-10-15	9-0-0	15-10-0	19-10-0	23-7-4	31-4-8	35-6-4	39-8-0	41-2-0
1-6-0	4-10-15	4-1-1	6-10-0	4-0-0	3-9-4	7-9-4	4-1-12	4-1-12	1-6-0

Scale = 1:72.8
Camber = 5/16 in



9-0-0	15-10-0	23-7-4	31-4-8	35-6-4	39-8-0
9-0-0	6-10-0	7-9-4	7-9-4	4-1-12	4-1-12

Plate Offsets (X,Y): [2:0-2-6,Edge], [4:0-2-12,0-3-0], [8:0-3-0,0-3-0], [15:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.50	15-16	>845	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.80	15-16	>528	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.48	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1545/0-3-8, 12=1945/0-3-8
Max Horz 2=-167(load case 6)
Max Uplift2=-579(load case 5), 12=-809(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4883/1956, 3-4=-4670/1778, 4-5=-3575/1291, 5-6=-3543/1443, 6-7=-2143/1036, 7-8=-2171/852, 8-9=-1309/445, 9-10=-595/579, 10-11=0/35
BOT CHORD 2-16=-1645/4434, 15-16=-1413/4307, 14-15=-343/1815, 13-14=-253/1176, 12-13=-471/637, 10-12=-471/637
WEBS 3-16=-151/225, 4-16=-10/286, 4-15=-1048/598, 5-15=-314/328, 6-15=-902/2505, 6-14=-324/431, 7-14=-372/380, 8-14=-214/773, 8-13=-923/525, 9-13=-932/1809, 9-12=-1810/1018

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 579 lb uplift at joint 2 and 809 lb uplift at joint 12.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877 RYRON K. ANDERSON PE 60987

Dwg.#0125061678

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T13	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:52:02 2006 Page 1

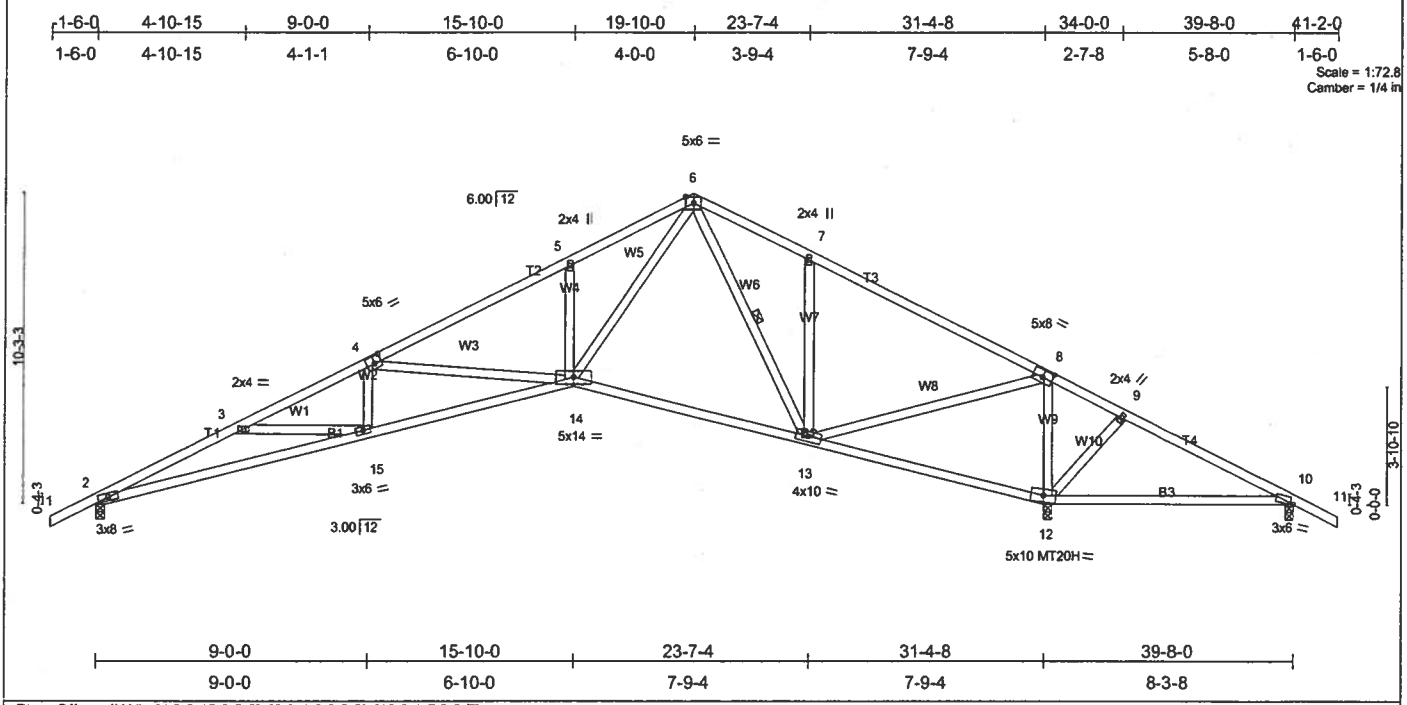


Plate Offsets (X,Y): [4:0-2-12,0-3-0], [8:0-4-0,0-3-0], [10:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	-0.35	2-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	0.19	10-12	>521	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.28	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-4-2 oc bracing.
WEBS 1 Row at midpt 6-13

REACTIONS (lb/size) 2=1158/0-3-8, 12=2825/0-3-8, 10=-498/0-3-8
Max Horz 2=-167(load case 6)
Max Uplift2=-454(load case 5), 12=-959(load case 5), 10=-615(load case 9)
Max Grav 2=1158(load case 1), 12=2825(load case 1), 10=116(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-3377/1421, 3-4=-3071/1203, 4-5=-1915/702, 5-6=-1888/855, 6-7=-504/440, 7-8=-527/255, 8-9=-656/1968, 9-10=-617/1843, 10-11=0/35
BOT CHORD 2-15=-1158/3070, 14-15=-904/2819, 13-14=0/701, 12-13=-1871/867, 10-12=-1566/660
WEBS 3-15=-251/269, 4-15=-21/314, 4-14=-1082/609, 5-14=-324/332, 6-14=-647/1772, 6-13=-756/192, 7-13=-376/382, 8-13=-701/2235, 8-12=-1959/841, 9-12=-215/201

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 2, 959 lb uplift at joint 12 and 615 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061679

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T14	MONO HIP	1	1	Job Reference (optional)

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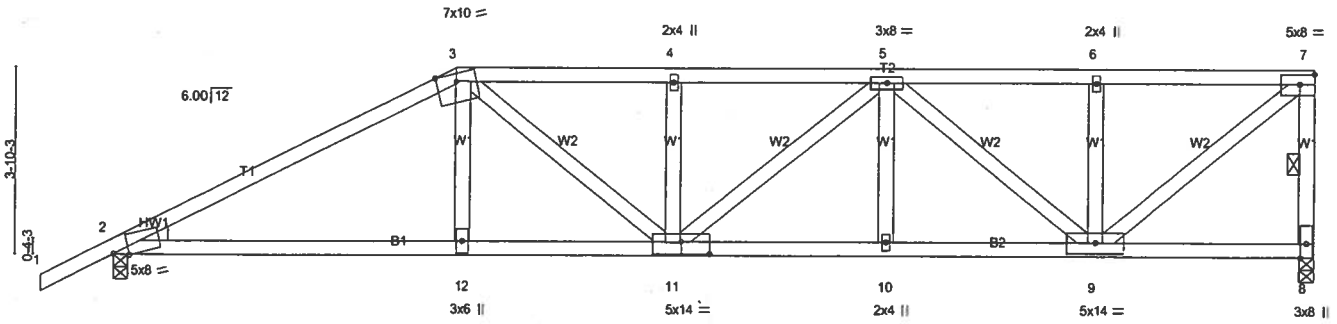
-1-6-0	3-9-4	7-0-0	11-5-14	15-10-0	20-2-2	24-8-0
1-6-0	3-9-4	3-2-12	4-5-14	4-4-2	4-4-2	4-5-14

Scale = 1:45.2
Camber = 1/8 in



3-9-4	7-0-0	11-5-14	15-10-0	20-2-2	24-8-0
3-9-4	3-2-12	4-5-14	4-4-2	4-4-2	4-5-14

Plate Offsets (X,Y): [2:0-3-13,Edge], [11:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.23	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.37	10-11	>797	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 133 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-10-12 oc bracing.
WEBS 1 Row at midpt 7-8

REACTIONS (lb/size) 8=2284/0-3-8, 2=2119/0-3-8
Max Horz 2=210(load case 4)
Max Uplift8=-1027(load case 3), 2=-902(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3948/1658, 3-4=-4068/1825, 4-5=-4068/1825, 5-6=-2376/1067, 6-7=-2376/1067, 7-8=-2133/1039
BOT CHORD 2-12=-1516/3440, 11-12=-1525/3474, 10-11=-1664/3711, 9-10=-1664/3711, 8-9=-34/74
WEBS 3-12=-237/817, 3-11=-447/757, 4-11=-492/448, 5-11=-220/461, 5-10=0/288, 5-9=-1725/771, 6-9=-520/437, 7-9=-1336/2976

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1027 lb uplift at joint 8 and 902 lb uplift at joint 2.
4) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-30, 8-12=-65(F=-35)
Concentrated Loads (lb)
Vert: 12=-539(F)

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877. BYRON K. ANDERSON PE 60987

Dwg.#0125061680

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T15	MONO HIP	1	1	Job Reference (optional)

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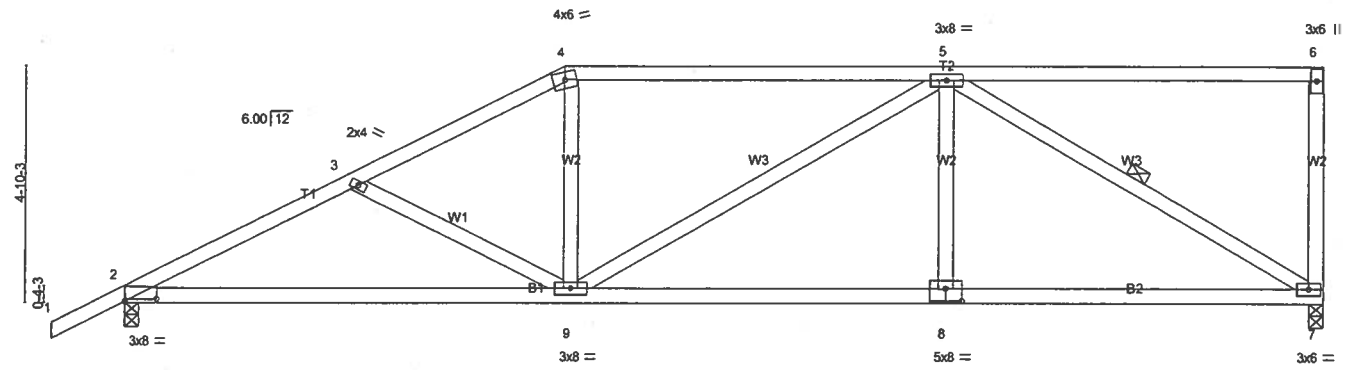
-1-6-0 | 4-9-4 | 9-0-0 | 16-10-0 | 24-8-0
1-6-0 | 4-9-4 | 4-2-12 | 7-10-0 | 7-10-0

Scale = 1:45.2
Camber = 1/8 in



9-0-0 | 16-10-0 | 24-8-0
9-0-0 | 7-10-0 | 7-10-0

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.17	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.30	2-9	>990	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 129 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-15 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=1021/0-3-8, 2=1116/0-3-8
Max Horz 2=256(load case 5)
Max Uplift7=-369(load case 4), 2=-394(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1767/705, 3-4=-1536/602, 4-5=-1347/596, 5-6=-70/26, 6-7=-192/135
BOT CHORD 2-9=-780/1535, 8-9=-540/1272, 7-8=-540/1272
WEBS 3-9=-224/207, 4-9=-10/326, 5-9=-87/87, 5-8=0/221, 5-7=-1398/598

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 369 lb uplift at joint 7 and 394 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061681

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T16	MONO HIP	1	1	Job Reference (optional)

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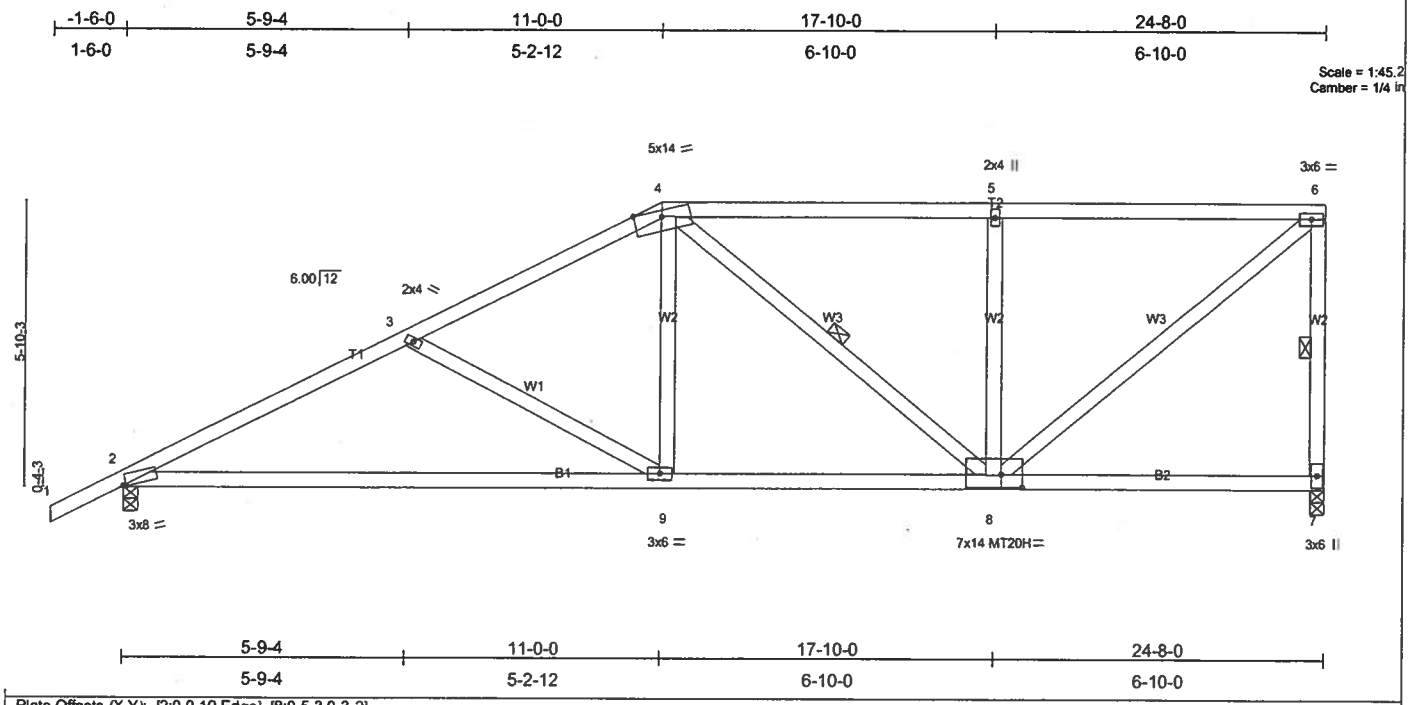

5-9-4	11-0-0	17-10-0	24-8-0
5-9-4	5-2-12	6-10-0	6-10-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-5-3,0-3-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.36	2-9	>823	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.61	2-9	>480	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 135 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-8 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8

REACTIONS (lb/size) 7=1021/0-3-8, 2=1116/0-3-8
Max Horz 2=302(load case 5)
Max Uplift7=-360(load case 4), 2=-401(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1698/682, 3-4=-1374/532, 4-5=-945/418, 5-6=-945/418, 6-7=-915/451
BOT CHORD 2-9=-800/1477, 8-9=-538/1182, 7-8=-15/42
WEBS 3-9=-349/301, 4-9=-88/469, 4-8=-306/154, 5-8=-392/283, 6-8=-524/1174

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 360 lb uplift at joint 7 and 401 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061682

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T17	MONO HIP	1	1	Job Reference (optional)

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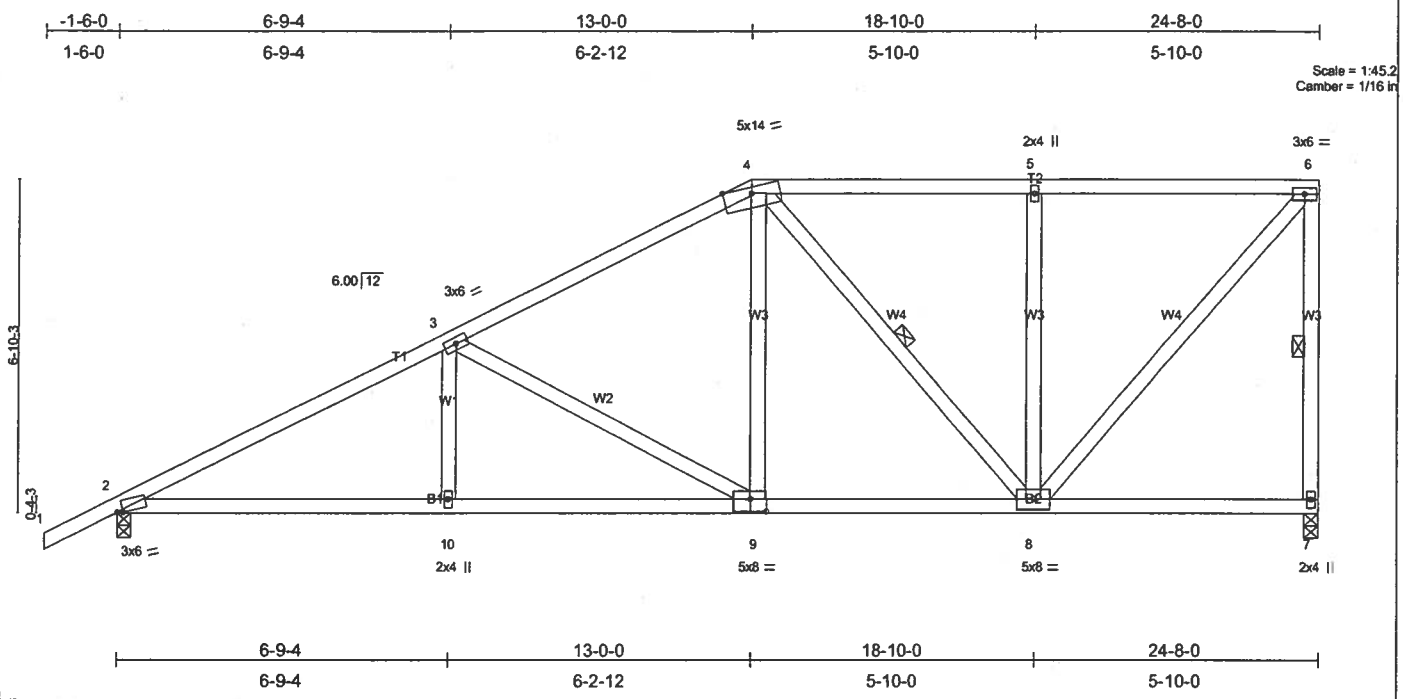

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.10	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.17	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-4 oc bracing.
WEBS 1 Row at midpt 6-7, 4-8

REACTIONS (lb/size) 7=1021/0-3-8, 2=1116/0-3-8
Max Horz 2=348(load case 5)
Max Uplift7=-350(load case 4), 2=-404(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1787/628, 3-4=-1167/462, 4-5=-715/324, 5-6=-714/324, 6-7=-938/463
BOT CHORD 2-10=-794/1522, 9-10=-794/1522, 8-9=-482/983, 7-8=-9/23
WEBS 3-10=0/222, 3-9=-624/358, 4-9=-135/470, 4-8=-404/237, 5-8=-324/243, 6-8=-481/1056

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 350 lb uplift at joint 7 and 404 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061683

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T18	MONO HIP	1	1	Job Reference (optional)

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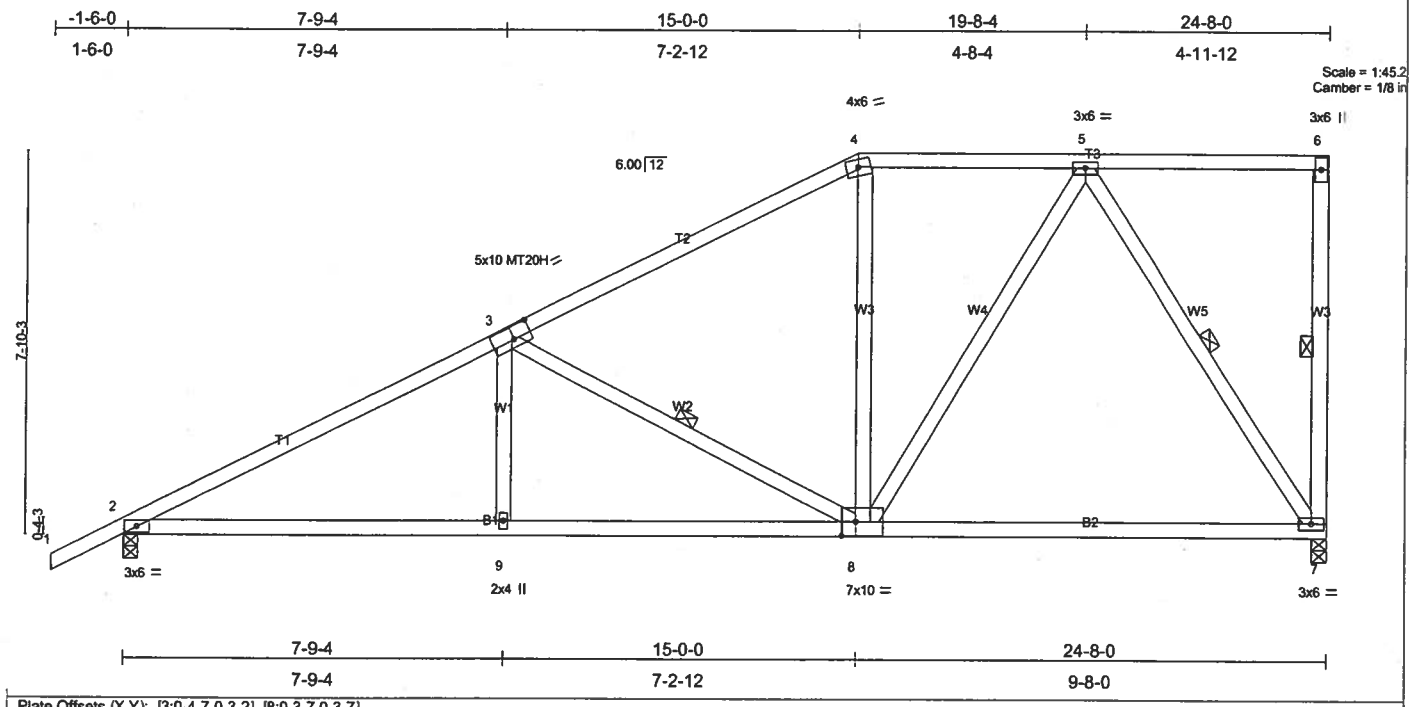

Plate Offsets (X,Y): [3:0-4-7,0-3-2], [8:0-3-7,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.19	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.33	7-8	>883	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 143 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-11 oc bracing.
WEBS 1 Row at midpt 6-7, 3-8, 5-7

REACTIONS (lb/size) 7=1021/0-3-8, 2=1116/0-3-8
Max Horz 2=394(load case 5)
Max Uplift7=-338(load case 4), 2=-403(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1732/587, 3-4=-1026/379, 4-5=-832/413, 5-6=-32/1, 6-7=-132/87
BOT CHORD 2-9=-798/1467, 8-9=-798/1467, 7-8=-264/513
WEBS 3-9=0/238, 3-8=-722/438, 4-8=0/94, 5-8=-283/605, 5-7=-892/486

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 7 and 403 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Dwg.#0125061684

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T19	HIP	1	1	Job Reference (optional)

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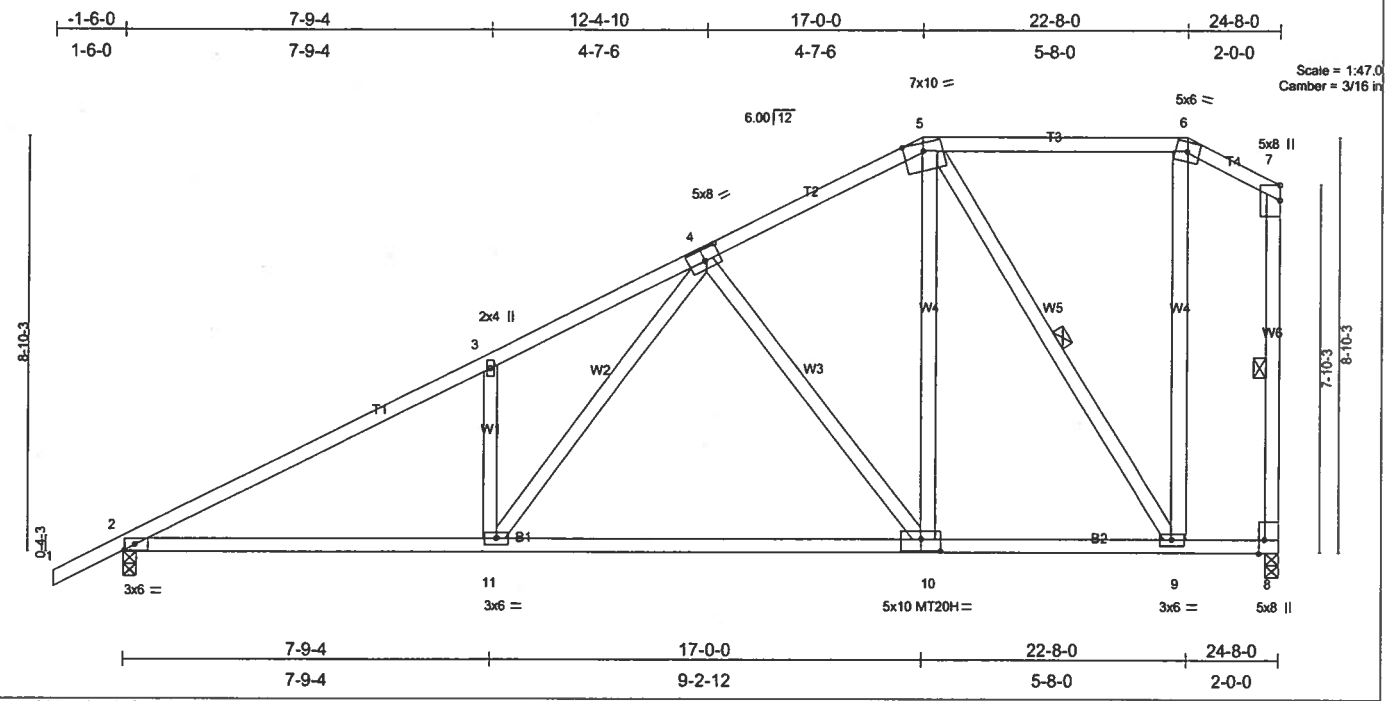


Plate Offsets (X,Y): [2:0-2-12,0-1-8], [4:0-4-0,0-3-0], [7:0-3-15,0-0-0], [8:0-3-8,Edge], [10:0-5-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.27	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.43	9-10	>674	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-14 oc bracing.
WEBS 1 Row at midpt 5-9, 7-8

REACTIONS (lb/size) 2=1116/0-3-8, 8=1021/0-3-8
Max Horz 2=410(load case 5)
Max Uplift2=-407(load case 5), 8=-353(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1738/571, 3-4=-1717/756, 4-5=-781/346, 5-6=-142/114, 6-7=-364/186, 7-8=-538/270
BOT CHORD 2-11=-782/1468, 10-11=-543/982, 9-10=-328/681, 8-9=-64/136
WEBS 3-11=-370/369, 4-11=-403/816, 4-10=-518/362, 5-10=-280/753, 5-9=-992/481, 6-9=-129/392

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 407 lb uplift at joint 2 and 353 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

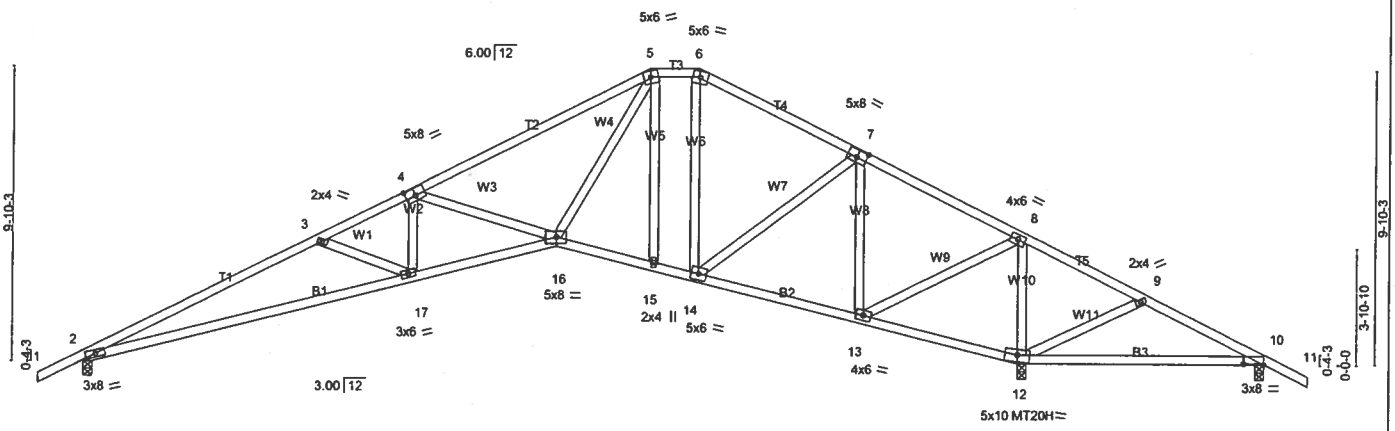
Dwg.#0125061685

Job	Truss	Truss Type	Qty	Ply	SIMQUE CONST.-LOT 11 CANNON CREEK ES
L146588	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jan 23 11:52:06 2006 Page 1

1-6-0	8-0-0	11-0-0	19-0-0	20-8-0	26-0-4	31-4-8	35-6-4	39-8-0	41-2-0
1-6-0	8-0-0	3-0-0	8-0-0	1-8-0	5-4-4	5-4-4	4-1-12	4-1-12	1-6-0

Scale = 1:73.8
Camber = 7/16 in



11-0-0	15-10-0	19-0-0	20-8-0	26-0-4	31-4-8	39-8-0
11-0-0	4-10-0	3-2-0	1-8-0	5-4-4	5-4-4	8-3-8

Plate Offsets (X,Y): [4:0-4-0,0-3-0], [7:0-4-0,0-3-0], [10:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	1.00	Vert(LL)	-0.61	2-17	>616	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-1.01	2-17	>372	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.34	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS (lb/size) 2=1159/0-3-8, 12=2823/0-3-8, 10=-496/0-3-8
Max Horz 2=-162(load case 6)
Max Uplift2=-457(load case 5), 12=-926(load case 5), 10=-616(load case 9)
Max Grav 2=1159(load case 1), 12=2823(load case 1), 10=97(load case 5)

* Uplift in gravity load case

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-3184/1305, 3-4=-2810/1120, 4-5=-1799/688, 5-6=-730/469, 6-7=-862/454, 7-8=-157/238, 8-9=-657/2000, 9-10=-486/1697, 10-11=0/35
BOT CHORD 2-17=-1023/2899, 16-17=-765/2565, 15-16=-37/818, 14-15=0/608, 13-14=-201/266, 12-13=-1897/861, 10-12=-1441/516
WEBS 3-17=-312/300, 4-17=-127/480, 4-16=-1026/578, 5-16=-481/1571, 5-15=-750/353, 6-14=-69/39, 7-14=-277/1014, 7-13=-1264/535, 8-13=-619/1931, 8-12=-1976/812, 9-12=-331/305

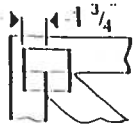
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 2, 926 lb uplift at joint 12 and 616 lb uplift at joint 10.

LOAD CASE(S) Standard

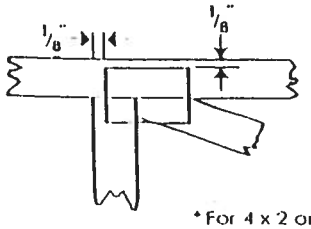
JANUARY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987

Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

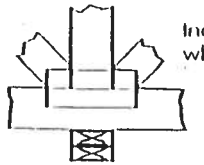
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

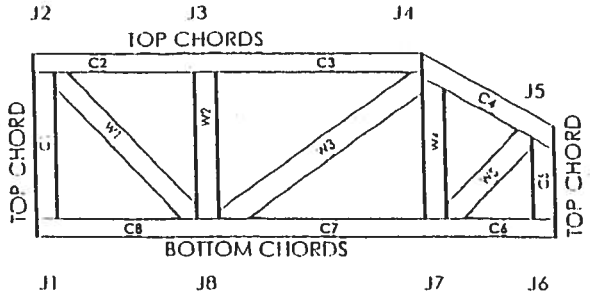
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

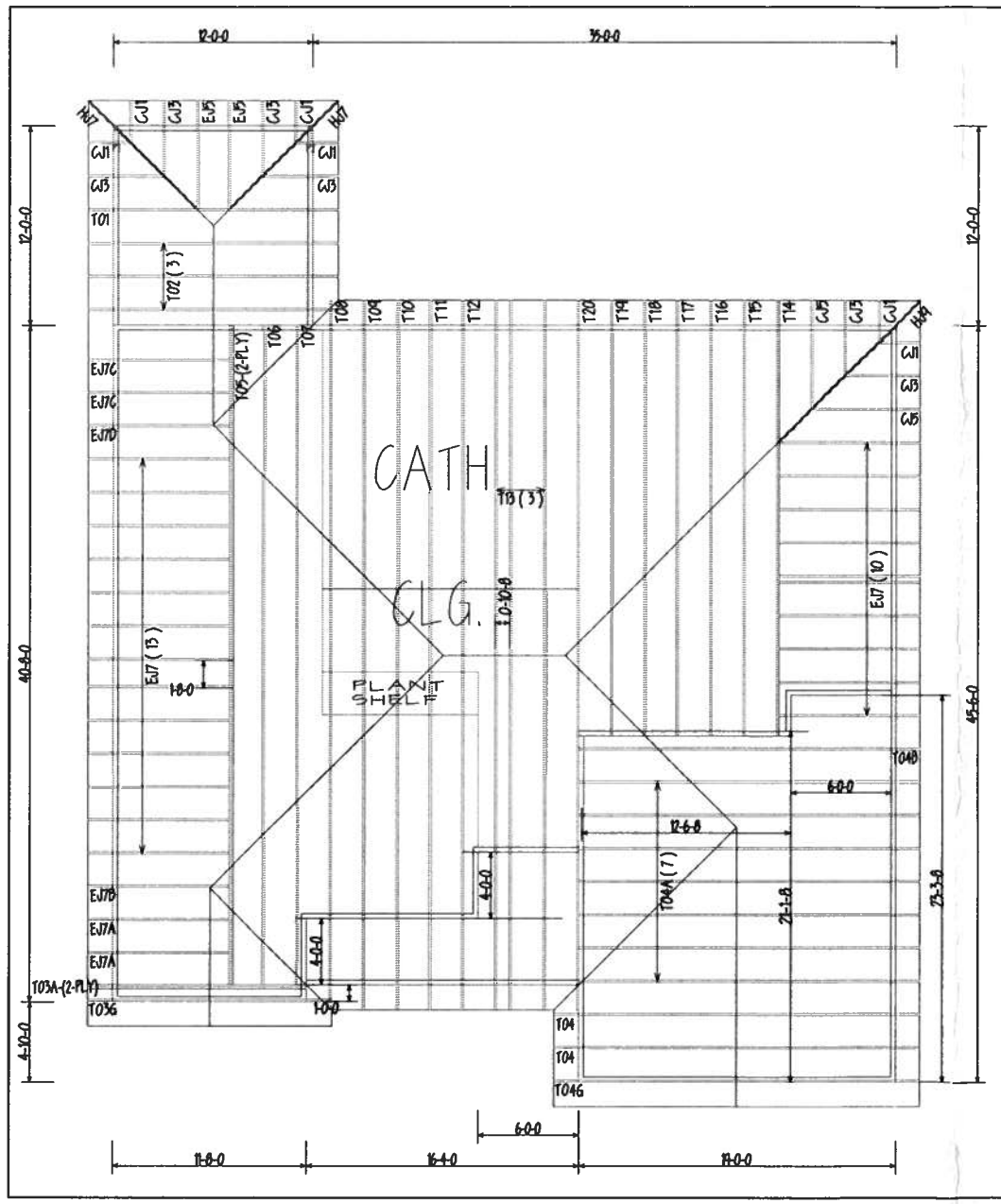
BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

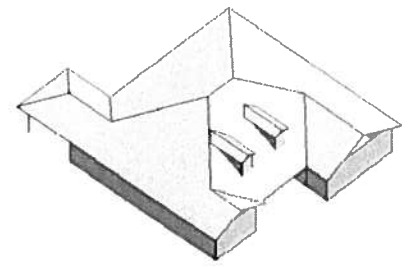
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.





6/12 PITCH

1'6" OH

BEARING HEIGHT SCHEDULE

PLATE1

NOTES:

1) REFER TO WTPI (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL YOO5 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) ST42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON H2526 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED
8) DRAWING ADDENDUM (IDN) TO BE FURNISHED BY BUILDER

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource
Bunnell
PHONE: 904-437-3344 FAX: 904-437-3494
Jacksonville
PHONE: 904-772-6100 FAX: 904-772-7973
Lake City
PHONE: 904-755-6844 FAX: 904-755-7973
Sanford
PHONE: 407-322-0054 FAX: 407-322-9993

BUILDER:	AARON SIMQUE
LEGAL ADDRESS:	LOT 11 CANNON CREEK EST.
CITY:	ARLINGTON
SCALE:	NTS
DATE:	1-23-06
DRAWN BY:	AVM
JOB #:	L146588