

DATE 03/06/2008

Columbia County Building Permit

PERMIT

This Permit Must Be Prominently Posted on Premises During Construction

000026825

APPLICANT MICHEAL CADY PHONE 386.755.6237
 ADDRESS 300 SW SWEETBRIAR DRIVE LAKE CITY FL 32024
 OWNER MICHAEL & LINDA CADY PHONE 386.755.6237
 ADDRESS 300 SW SWEETBRIAR DRIVE LAKE CITY FL 32024
 CONTRACTOR MICHAEL CADY PHONE 386.755.6237

LOCATION OF PROPERTY 47-S TO BRENTWOOD, TL TO HAMLET, TL TO PRISM LOOP, TL TO SWEETBRIAR, TL GO THROUGH GATE, 1ST. ON R.

TYPE DEVELOPMENT GARAGE/UTILITY ESTIMATED COST OF CONSTRUCTION 166950.00

HEATED FLOOR AREA 1534.00 TOTAL AREA _____ HEIGHT 24.20 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 36-4S-16-03322-009 SUBDIVISION _____

LOT _____ BLOCK _____ PHASE _____ UNIT _____ TOTAL ACRES 2.22

Culvert Permit No. _____ Culvert Waiver _____ Contractor's License Number _____
 EXISTING 08-0167 BLK JTH N
 Driveway Connection _____ Septic Tank Number _____ LU & Zoning checked by _____ Approved for Issuance _____ New Resident _____

COMMENTS: NOC ON FILE. NO IMPACT FEES. ACCESSORY USE.

DETACHED GARAGE.
 Check # or Cash 657

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ Foundation _____ Monolithic _____
 date/app. by _____ date/app. by _____ date/app. by _____

Under slab rough-in plumbing _____ Slab _____ Sheathing/Nailing _____
 date/app. by _____ date/app. by _____ date/app. by _____

Framing _____ Rough-in plumbing above slab and below wood floor _____
 date/app. by _____ date/app. by _____

Electrical rough-in _____ Heat & Air Duct _____ Peri. beam (Lintel) _____
 date/app. by _____ date/app. by _____ date/app. by _____

Permanent power _____ C.O. Final _____ Culvert _____
 date/app. by _____ date/app. by _____ date/app. by _____

M/H tie downs, blocking, electricity and plumbing _____ Pool _____
 date/app. by _____ date/app. by _____

Reconnection _____ Pump pole _____ Utility Pole _____
 date/app. by _____ date/app. by _____ date/app. by _____

M/H Pole _____ Travel Trailer _____ Re-roof _____
 date/app. by _____ date/app. by _____ date/app. by _____

BUILDING PERMIT FEE \$ 835.00 CERTIFICATION FEE \$ 0.00 SURCHARGE FEE \$ 0.00

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$ _____

FLOOD DEVELOPMENT FEE \$ _____ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ _____ **TOTAL FEE** 910.00

INSPECTORS OFFICE _____ CLERKS OFFICE _____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

BOARD OF COUNTY COMMISSIONERS
OFFICE OF
BUILDING & ZONING
COLUMBIA COUNTY, FLORIDA

BUILDING PERMIT RECEIPT

RECEIPT NUMBER / PERMIT NUMBER 000028967 DATE 11/01/2010

APPLICANT MICHEAL CADY

OWNER MICHAEL & LINDA CADY

CONTRACTOR MICHAEL CADY

PARCEL ID NUMBER 36-4S-16-03322-009 NUMBER OF EXISTING DWELLINGS 1

TYPE OF DEVELOPMENT RENEW PERMIT 26825

COMMENTS: ACCESSAORY BUILDING

COMPLETION OF GARAGE PERMIT # 26825

FEES:

BUILDING PERMIT 0.00 CERTIFICATION FEE 0.00

ZONING FEE _____ SURCHARGE FEE 0.00

FLOOD ZONE FEE _____ FLOOD DEVELOPMENT PERMIT _____

MOBILE HOME PERMIT _____ RELOCATION PERMIT _____

TRAVEL TRAILER PERMIT _____ RECONNECTION PERMIT _____

UTILITY POLE PERMIT _____ WASTE ASSESSMENT FEE _____

FIRE FEE (5 ACRES OR LESS) _____ CULVERT PERMIT _____

FIRE FEE (MORE THAN 5 ACRES) _____ RENEW PERMIT 208.75

CHECK NUMBER 1063 **TOTAL FEES CHARGES** 208.75

MAKE CHECKS PAYABLE TO: BCC (Board of County Commissioners)

NOTE: A SEPARATE CHECK IS REQUIRED FOR THE CULVERT WAIVER PERMITS

135 NE HERNANDO AVE.
SUITE B-21
LAKE CITY, FL 32055
Phone: 386-758-1008
Fax: 386-758-2160



Columbia County Building Permit Application

28967

For Office Use Only Application # 0802-24 Date Received 3/20 By JW Permit # 26825
 Zoning Official BLK Date 03.03.08 Flood Zone X FEMA Map # NA Zoning A-3
 Land Use A-3 Elevation NA MFE NA River MA Plans Examiner DKH Date 3-21-08
 Comments DETACHED GARAGE No Impact Fees Accessory use
 NOC EH Deed or PA Site Plan State Road Info Parent Parcel # _____
 Dev Permit # _____ In Floodway Letter of Authorization from Contractor SHARINER WELL
 Unincorporated area Incorporated area Town of Fort White Town of Fort White Compliance letter

Septic Permit No. 08-0167 Fax 752-2282
 Name Authorized Person Signing Permit MICHAEL CADY Phone 752-2281
 Address 387 SW Kemp Ct Lake City FL 32024
 Owners Name MICHAEL CADY Phone 755-6237
 911 Address existing 300 SW Sweetbriar Drive, L.C. 32024
 Contractors Name SAME AS ABOVE Phon _____
 Address _____

Fee Simple Owner Name & Address _____
 Bonding Co. Name & Address _____
 Architect/Engineer Name & Address Will Myers Nick Geisler
 Mortgage Lenders Name & Address NA

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy
36-45-16-03322-009
 Property ID Number PO 3322-009 Estimated Cost of Construction 125K

Subdivision Name _____ Lot _____ Block _____ Unit _____ Phase _____
 Driving Directions 475, Lon Brentwood, Lon Hamlet, Lon Prism, Lon Sweetbriar, follow around to end on right

Number of Existing Dwellings on Property 1
 Construction of garage with existing area Total Acreage 2.22 Lot Size 2.22
 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 29'-2"
 Actual Distance of Structure from Property Lines - Front 210.94" Side 158'-4" Side 39'-8" Rear 33'-5"
 Number of Stories 2 Heated Floor Area 1534 Total Floor Area 1534 Roof Pitch 6-12
Living Area + 3,339 GARAGE BREEZWAY

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
 Spoke to Linda 3/4/08

Columbia County Building Permit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Michael Cody - owner builder

Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature _____ ee)

Contractor's License Number _____
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 2 day of Feb 2008
Personally known or Produced Identification _____

Linda R. Rode

State of Florida Notary Signature (For the Contractor)

SEAL:



Linda R. Rode
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru
Atlantic Bonding Co., Inc.



Cal-Tech Testing, Inc.

- Engineering
 - Geotechnical
 - Environmental
- Laboratories**

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456

4784 Rosselle St., Jacksonville, FL 32254 • Tel(904)381-8901 • Fax(904)381-8902

2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 08-00217

DATE TESTED: 4/11/08

DATE REPORTED: 4/18/08

PROJECT:	Cady Residence
CLIENT:	Mike Cady, PO Box 889, Lake City, FL 32056
GENERAL CONTRACTOR:	Mike Cady
EARTHWORK CONTRACTOR:	Mike Cady
INSPECTOR:	Shane Osteen
ASTM METHOD	SOIL USE
(D-2922) Nuclear	SUBGRADE/NATURAL SOIL
SPECIFIED REQUIREMENTS: 95%	

TEST NO.	TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft ³)	MOISTURE PERCENT	DRY DENSITY (lb/ft ³)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1	20' from East Wall 10' from North Wall	0-12"	106.3	5.2	101.0	1	106.2	95%
2	8' from East Wall 8' from South Wall	0-12"	106.9	5.6	101.2	1	106.2	95%
3	10' from West Wall 8' from North Wall	0-12"	106.7	5.7	100.9	1	106.2	95%
4	5' from West Wall 10' from South Wall	0-12"	106.2	5.0	101.1	1	106.2	95%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS				
PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft ³)	OPT. MOIST.	TYPE
1	Light Tan Sand	106.2	12.7	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By:

[Signature]
Date: 4/21/08
Licensed, Florida No: 57842

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test locations and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

NOTICE OF COMMENCEMENT

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number RO 3322-009

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

- 1. Description of property (legal description): 36-45-16 0100/0100
 a) Street (job) Address: 300 SW Sweet Briar Drive
- 2. General description of improvements: Garage
- 3. Owner Information
 a) Name and address: MICHAEL CADY / PO Box 889 / Lake City, FL 32056
 b) Name and address of fee simple titleholder (if other than owner) _____
 c) Interest in property: OWNER
- 4. Contractor Information
 a) Name and address: JJM Construction PO Box 1796 Lake City, FL 32056
 b) Telephone No.: (386) 752-1805 Fax No. (Opt.) _____
- 5. Surety Information
 a) Name and address: _____
 b) Amount of Bond: _____
 c) Telephone No.: _____
- 6. Lender NONE
 a) Name and address: _____
 b) Phone No.: _____
- 7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served
 a) Name and address: NA
 b) Telephone No.: _____ Fax No. (Opt.) _____
- 8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b).
 Florida Statutes:
 a) Name and address: NA
 b) Telephone No.: _____ Fax No. (Opt.) _____
- 9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

Inst:200812003401 Date:2/20/2008 Time:12:19 PM
29 DC, P. DeWitt Cason, Columbia County Page 1 of 1

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru
Atlantic Bonding Co., Inc.

10. Michael Cady
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager
Michael Cady
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 8 day of Feb, 20 08 by:
Linda Roder as notary (type of authority, e.g. officer, trustee, attorney
fact) for Mike Cady (name of party on behalf of whom instrument was executed).
Personally Known OR Produced Identification _____ Type _____

Notary Signature Linda Roder Notary Stamp or Seal: _____

---AND---

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Signature of Natural Person Signing (in line #10 above.): _____



Project Information for: L266887F

... Address: 300 Southwest Sweetbryer Drive
 ... Lake City, FL
 ... County: Columbia
 Truss Count: 16
 Design Program: MiTek 20/20 6.3
 Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: Wind:

Roof (psf): N/A Wind Standard: N/A Wind Exposure: N/A
 Floor (psf): 55.0 Wind Speed (mph): N/A

Note: See the individual truss drawings for special loading conditions.

Engineer of Record: Unknown at time of Seal Date

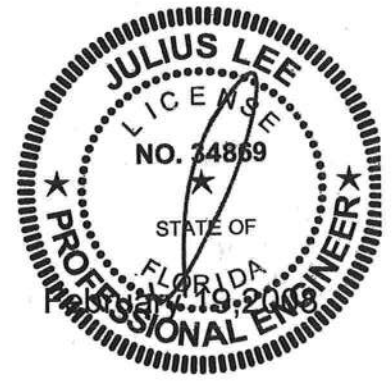
Address: Unknown at time of Seal Date

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.



No.	Drwg. #	Truss ID	Seal Date
1	J1936277	F01	2/19/08
2	J1936278	F02	2/19/08
3	J1936279	F03	2/19/08
4	J1936280	F04	2/19/08
5	J1936281	F05	2/19/08
6	J1936282	F06	2/19/08
7	J1936283	F07	2/19/08
8	J1936284	F08	2/19/08
9	J1936285	F09	2/19/08
10	J1936286	F10	2/19/08
11	J1936287	F11	2/19/08
12	J1936288	F12	2/19/08
13	J1936289	F13	2/19/08
14	J1936290	F14	2/19/08
15	J1936291	F15	2/19/08
16	J1936292	F16	2/19/08

Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936277
L266887F	F01	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:44 2008 Page 1

0-1-8

0-1-8

Scale = 1:41.2

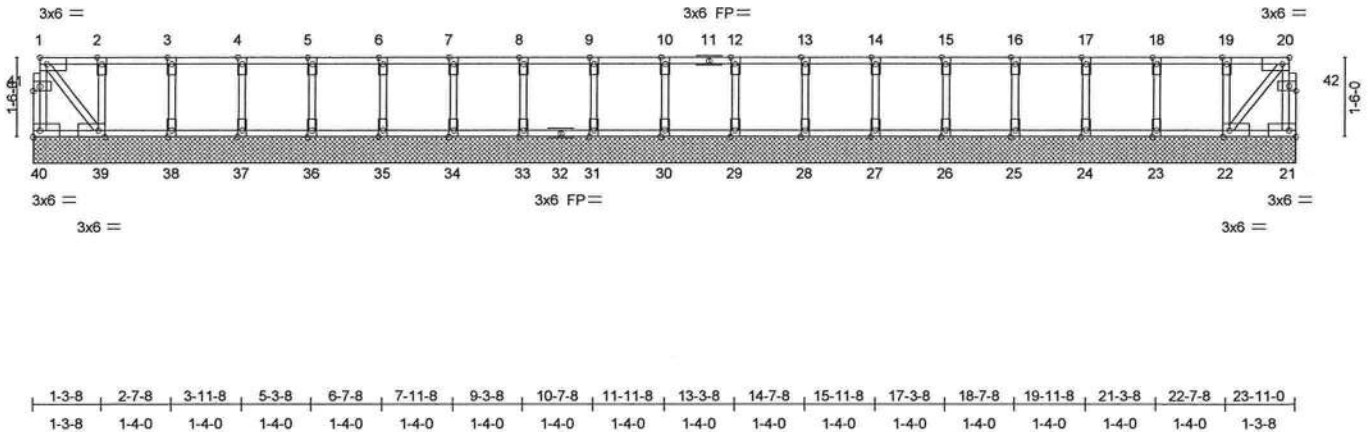


Plate Offsets (X,Y): [20:0-1-8,Edge], [22:0-1-8,Edge], [39:0-1-8,Edge], [41:0-1-8,0-1-0], [42:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plates Increase	1.00	TC 0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.01	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.02	Horz(TL)	-0.00	22	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
								Weight: 114 lb		

LUMBER	BRACING
TOP CHORD 4 X 2 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD 4 X 2 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
WEBS 4 X 2 SYP No.3	10-0-0 oc bracing: 39-40,21-22.
OTHERS 4 X 2 SYP No.3	

REACTIONS (lb/size) 40=34/23-11-0, 21=34/23-11-0, 30=117/23-11-0, 31=117/23-11-0, 33=117/23-11-0, 34=117/23-11-0, 35=117/23-11-0, 36=117/23-11-0, 37=117/23-11-0, 38=117/23-11-0, 39=122/23-11-0, 29=117/23-11-0, 28=117/23-11-0, 27=117/23-11-0, 26=117/23-11-0, 25=117/23-11-0, 24=117/23-11-0, 23=117/23-11-0, 22=122/23-11-0

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 40-41=-31/0, 1-41=-30/0, 21-42=-31/0, 20-42=-30/0, 1-2=0/1, 2-3=0/1, 3-4=0/1, 4-5=0/1, 5-6=0/1, 6-7=0/1, 7-8=0/1, 8-9=0/1, 9-10=0/1, 10-11=0/1, 11-12=0/1, 12-13=0/1, 13-14=0/1, 14-15=0/1, 15-16=0/1, 16-17=0/1, 17-18=0/1, 18-19=0/1, 19-20=0/1
BOT CHORD 39-40=0/1, 38-39=-1/0, 37-38=-1/0, 36-37=-1/0, 35-36=-1/0, 34-35=-1/0, 33-34=-1/0, 32-33=-1/0, 31-32=-1/0, 30-31=-1/0, 29-30=-1/0, 28-29=-1/0, 27-28=-1/0, 26-27=-1/0, 25-26=-1/0, 24-25=-1/0, 23-24=-1/0, 22-23=-1/0, 21-22=0/1
WEBS 10-30=-107/0, 9-31=-107/0, 8-33=-107/0, 7-34=-107/0, 6-35=-107/0, 5-36=-107/0, 4-37=-107/0, 3-38=-107/0, 2-39=-108/0, 12-29=-107/0, 13-28=-107/0, 14-27=-107/0, 15-26=-107/0, 16-25=-107/0, 17-24=-107/0, 18-23=-107/0, 19-22=-108/0, 1-39=-4/0, 20-22=-4/0

Julius Lee
Truss Design Engineer
Florida PE No. 34868
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

Continued on page 2

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MI TEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F01	GABLE	1	1	J1936277 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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JOINT STRESS INDEX

1 = 0.64, 2 = 0.47, 3 = 0.47, 4 = 0.47, 5 = 0.47, 6 = 0.47, 7 = 0.47, 8 = 0.47, 9 = 0.47, 10 = 0.47, 11 = 0.13, 12 = 0.47, 13 = 0.47, 14 = 0.47, 15 = 0.47, 16 = 0.47, 17 = 0.47, 18 = 0.47, 19 = 0.47, 20 = 0.64, 21 = 0.64, 22 = 0.64, 23 = 0.47, 24 = 0.47, 25 = 0.47, 26 = 0.47, 27 = 0.47, 28 = 0.47, 29 = 0.47, 30 = 0.47, 31 = 0.47, 32 = 0.13, 33 = 0.47, 34 = 0.47, 35 = 0.47, 36 = 0.47, 37 = 0.47, 38 = 0.47, 39 = 0.64, 40 = 0.64, 41 = 0.00, 42 = 0.00 and 42 = 0.47

NOTES

- 1) All plates are 2x4 MT20 unless otherwise indicated.
- 2) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 11 and 32.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34868
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F02	FLOOR	2	1	J1936278
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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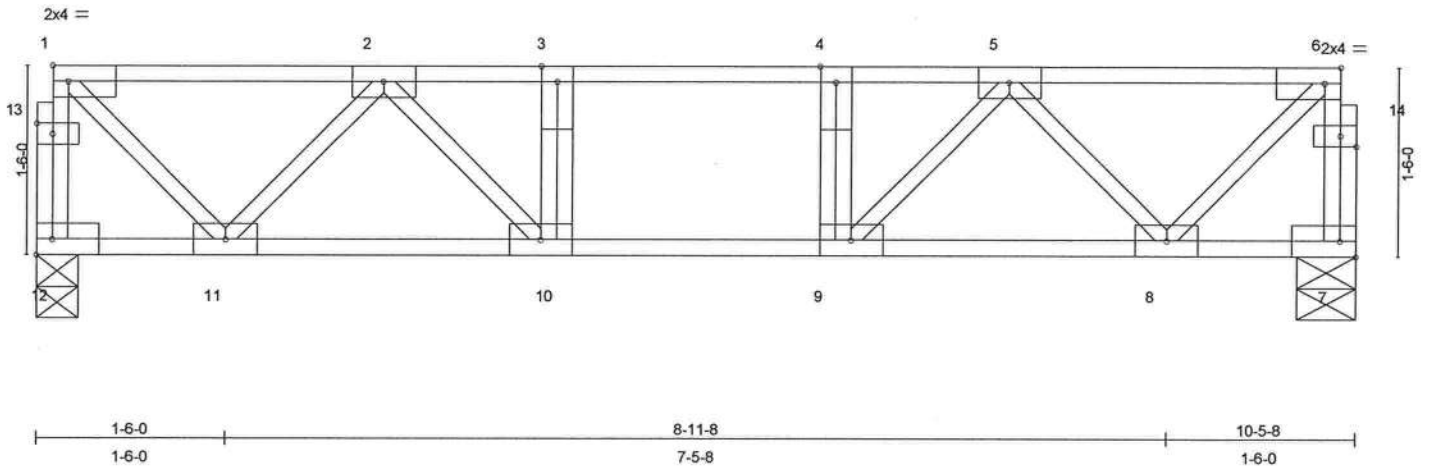
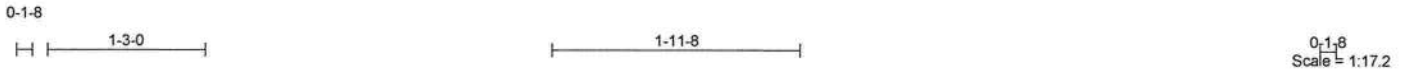


Plate Offsets (X,Y): [6:0-1-8,Edge], [13:0-1-8,0-1-0], [14:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.26	Vert(LL)	-0.04 8-9	>999	360	MT20	244/190
BCDL 5.0	Lumber Increase	1.00	BC 0.26	Vert(TL)	-0.05 10-11	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.20	Horz(TL)	0.01 7	n/a	n/a		
	Code	FBC2004/TPI2002	(Matrix)						Weight: 61 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 12=444/0-4-0, 7=444/0-5-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 12-13=-441/0, 1-13=-440/0, 7-14=-441/0, 6-14=-440/0, 1-2=-359/0, 2-3=-810/0, 3-4=-810/0, 4-5=-810/0, 5-6=-359/0
 BOT CHORD 11-12=0/20, 10-11=0/665, 9-10=0/810, 8-9=0/665, 7-8=0/20
 WEBS 6-8=0/491, 1-11=0/491, 5-8=-455/0, 2-11=-455/0, 5-9=0/310, 2-10=0/310, 3-10=-168/0, 4-9=-168/0

JOINT STRESS INDEX

1 = 0.57, 2 = 0.37, 3 = 0.07, 4 = 0.07, 5 = 0.37, 6 = 0.57, 7 = 0.35, 8 = 0.58, 9 = 0.37, 10 = 0.37, 11 = 0.58, 12 = 0.35, 13 = 0.00, 13 = 0.00, 14 = 0.00 and 14 = 0.00

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x6 MT20 unless otherwise indicated.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Julius Lee
 Truss Design Engineer
 Florida PE No. 24868
 1300 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Julius Lee
Truss Design Engineer
Florida P.E. No. 24868
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Boynton Beach, FL 33435

February 19, 2008

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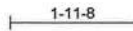
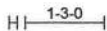


Job L266887F	Truss F03	Truss Type FLOOR	Qty 2	Ply 1	MIKE & LINDA CADY / FLOOR Job Reference (optional)	J1936279
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Builders FirstSource, Lake City, FL 32055

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0-1-8



0-1-8
Scale = 1:34.2

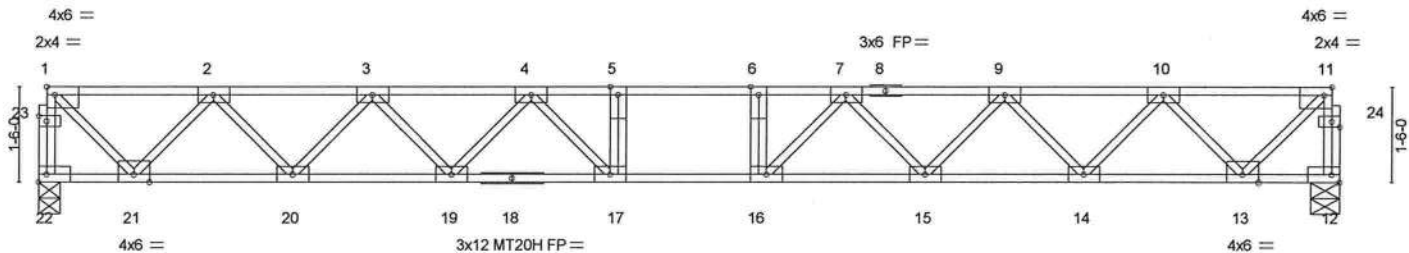


Plate Offsets (X,Y): [1:Edge,0-1-8], [11:0-1-8,Edge], [23:0-1-8,0-1-0], [24:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.55	Vert(LL)	-0.23 16-17	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.80	Vert(TL)	-0.35 16-17	>687	240	MT20H	187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.44	Horz(TL)	0.07 12	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						Weight: 112 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 22=884/0-4-0, 12=884/0-5-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 22-23=-880/0, 1-23=-879/0, 12-24=-880/0, 11-24=-879/0, 1-2=-800/0, 2-3=-2010/0, 3-4=-2796/0, 4-5=-3237/0, 5-6=-3237/0, 6-7=-3237/0, 7-8=-2796/0, 8-9=-2796/0, 9-10=-2010/0, 10-11=-800/0
 BOT CHORD 21-22=0/40, 20-21=0/1508, 19-20=0/2494, 18-19=0/3079, 17-18=0/3079, 16-17=0/3237, 15-16=0/3079, 14-15=0/2494, 13-14=0/1508, 12-13=0/40
 WEBS 11-13=0/1100, 1-21=0/1100, 10-13=-1053/0, 2-21=-1053/0, 10-14=0/746, 2-20=0/746, 9-14=-719/0, 3-20=-719/0, 9-15=0/449, 3-19=0/449, 7-15=-421/0, 4-19=-421/0, 7-16=-99/510, 4-17=-99/510, 5-17=-261/7, 6-16=-261/7

JOINT STRESS INDEX

1 = 0.71, 2 = 0.88, 3 = 0.56, 4 = 0.60, 5 = 0.32, 6 = 0.32, 7 = 0.60, 8 = 0.36, 9 = 0.56, 10 = 0.88, 11 = 0.71, 12 = 0.69, 13 = 0.74, 14 = 0.88, 15 = 0.56, 16 = 0.66, 17 = 0.66, 18 = 0.62, 19 = 0.56, 20 = 0.88, 21 = 0.74, 22 = 0.69, 23 = 0.00, 23 = 0.49, 24 = 0.00 and 24 = 0.49

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 18 and 8.

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 Truss Design Engineer
 Florida PE No. 21803
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936279
L266887F	F03	FLOOR	2	1		
						Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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NOTES

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 31888
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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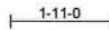


Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F04	FLOOR	18	1	J1936280
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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0-1-8



0-1-8

Scale = 1:41.4

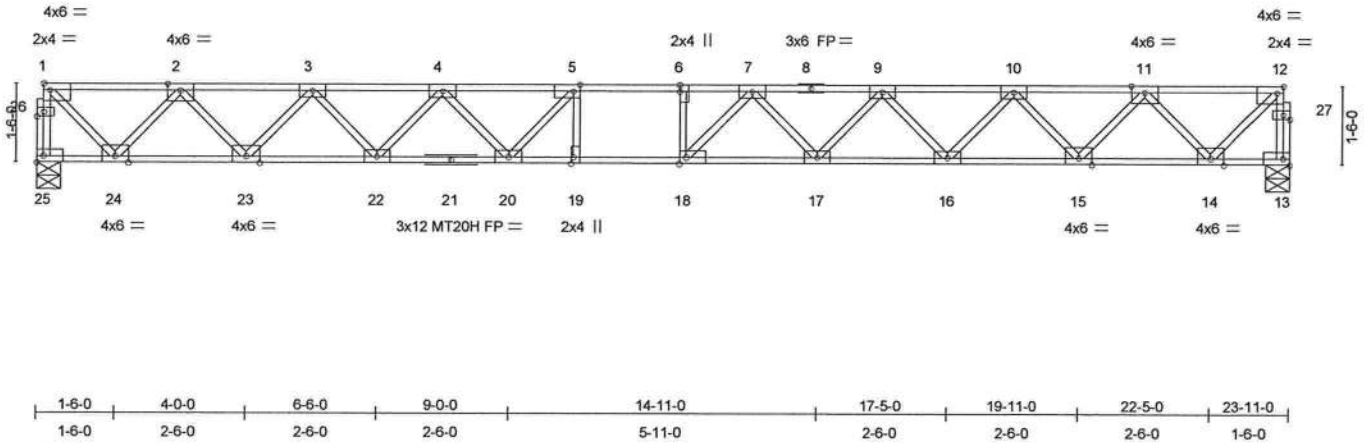


Plate Offsets (X,Y): [1:Edge,0-1-8], [5:0-1-8,Edge], [6:0-1-8,0-0-0], [12:0-1-8,Edge], [18:0-1-8,Edge], [19:0-1-8,Edge], [26:0-1-8,0-1-0], [27:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.59	Vert(LL)	-0.41	18	>689	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.92	Vert(TL)	-0.65	17-18	>439	MT20H	187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.53	Horz(TL)	0.10	13	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						Weight: 126 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2 *Except*
 13-21 4 X 2 SYP No.1D
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-11-12 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
 2-2-0 oc bracing: 19-20,18-19.

REACTIONS (lb/size) 25=1036/0-5-8, 13=1036/0-5-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 25-26=-1032/0, 1-26=-1031/0, 13-27=-1031/0, 12-27=-1030/0, 1-2=-952/0, 2-3=-2437/0, 3-4=-3502/0, 4-5=-4172/0, 5-6=-4441/0, 6-7=-4441/0, 7-8=-4176/0, 8-9=-4176/0, 9-10=-3501/0, 10-11=-2437/0, 11-12=-951/0
 BOT CHORD 24-25=0/47, 23-24=0/1798, 22-23=0/3057, 21-22=0/3926, 20-21=0/3926, 19-20=0/4441, 18-19=0/4441, 17-18=0/4390, 16-17=0/3932, 15-16=0/3055, 14-15=0/1799, 13-14=0/47
 WEBS 12-14=0/1310, 1-24=0/1310, 11-14=-1259/0, 2-24=-1259/0, 11-15=0/949, 2-23=0/949, 10-15=-920/0, 3-23=-923/0, 10-16=0/663, 3-22=0/662, 9-16=-641/0, 4-22=-630/0, 9-17=0/362, 4-20=0/488, 7-17=-385/0, 5-20=-637/38, 7-18=-263/461, 5-19=-131/220, 6-18=-212/59

Julius Lee
 Truss Design Engineer
 Florida PE No. 34869
 1100 Coastal Bay Blvd.
 Boynton Beach, FL 33436

JOINT STRESS INDEX

1 = 0.82, 2 = 0.62, 3 = 0.76, 4 = 0.56, 5 = 0.64, 6 = 0.47, 7 = 0.55, 8 = 0.53, 9 = 0.55, 10 = 0.76, 11 = 0.62, 12 = 0.82, 13 = 0.79, 14 = 0.85, 15 = 0.62, 16 = 0.76, 17 = 0.55, 18 = 0.64, 19 = 0.47, 20 = 0.56, 21 = 0.77, 22 = 0.76, 23 = 0.62, 24 = 0.85, 25 = 0.79, 26 = 0.00, 26 = 0.47, 27 = 0.00 and 27 = 0.47

Continued on page 2

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936280
L266887F	F04	FLOOR	18	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 21 and 8.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34883
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936281
L266887F	F05	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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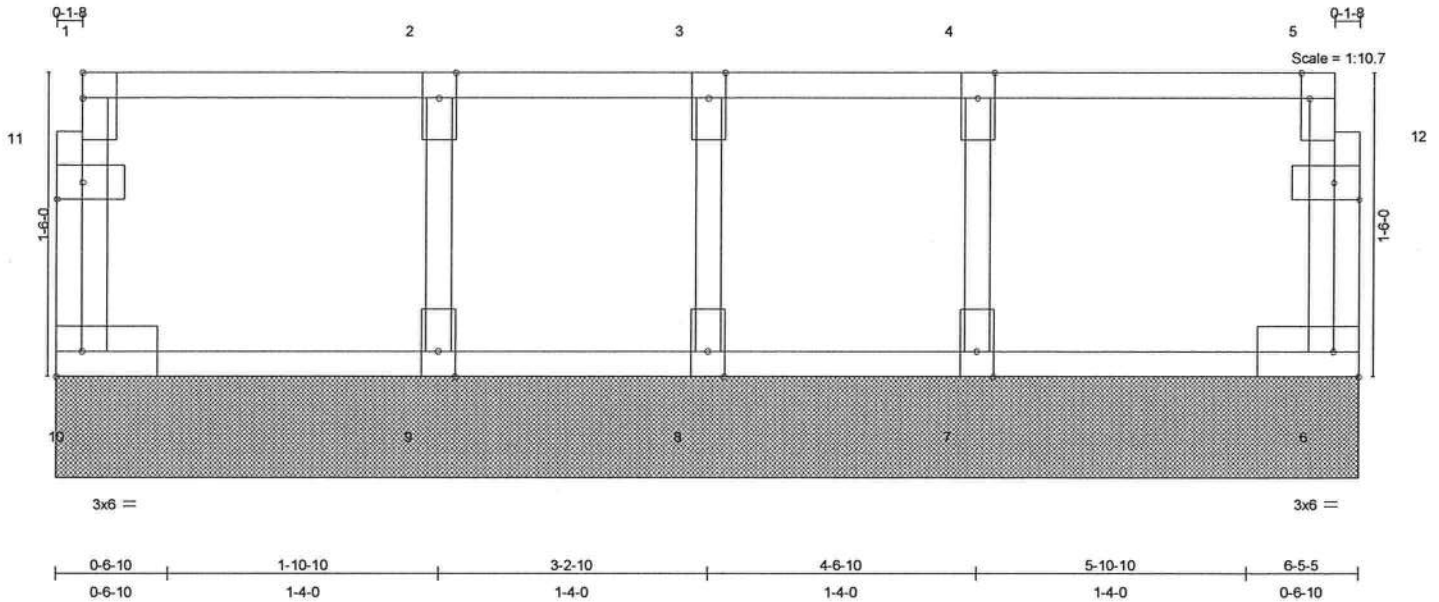


Plate Offsets (X,Y): [5:0-1-8,Edge], [11:0-1-8,0-1-0], [12:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 40.0	Plates Increase	1.00	TC 0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.02	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.03	Horz(TL)	0.00	6	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)							Weight: 31 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3
 OTHERS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=73/6-5-5, 6=73/6-5-5, 8=103/6-5-5, 9=143/6-5-5, 7=143/6-5-5

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 10-11=-65/0, 1-11=-65/0, 6-12=-65/0, 5-12=-65/0, 1-2=-15/0, 2-3=-15/0, 3-4=-15/0, 4-5=-15/0
 BOT CHORD 9-10=0/15, 8-9=0/15, 7-8=0/15, 6-7=0/15
 WEBS 3-8=-92/0, 2-9=-132/0, 4-7=-132/0

JOINT STRESS INDEX

1 = 0.22, 2 = 0.08, 3 = 0.06, 4 = 0.08, 5 = 0.22, 6 = 0.09, 7 = 0.08, 8 = 0.06, 9 = 0.08, 10 = 0.09, 11 = 0.00, 11 = 0.00, 12 = 0.00 and 12 = 0.00

NOTES

- 1) All plates are 2x4 MT20 unless otherwise indicated.
- 2) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 3) Gable studs spaced at 1-4-0 oc.
- 4) Non Standard bearing condition. Review required.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Lee
 Truss Design Engineer
 Florida PE No. 21883
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F05	GABLE	1	1	J1936281 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:48 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 24888
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job L266887F	Truss F06	Truss Type FLOOR	Qty 1	Ply 1	MIKE & LINDA CADY / FLOOR Job Reference (optional)	J1936282
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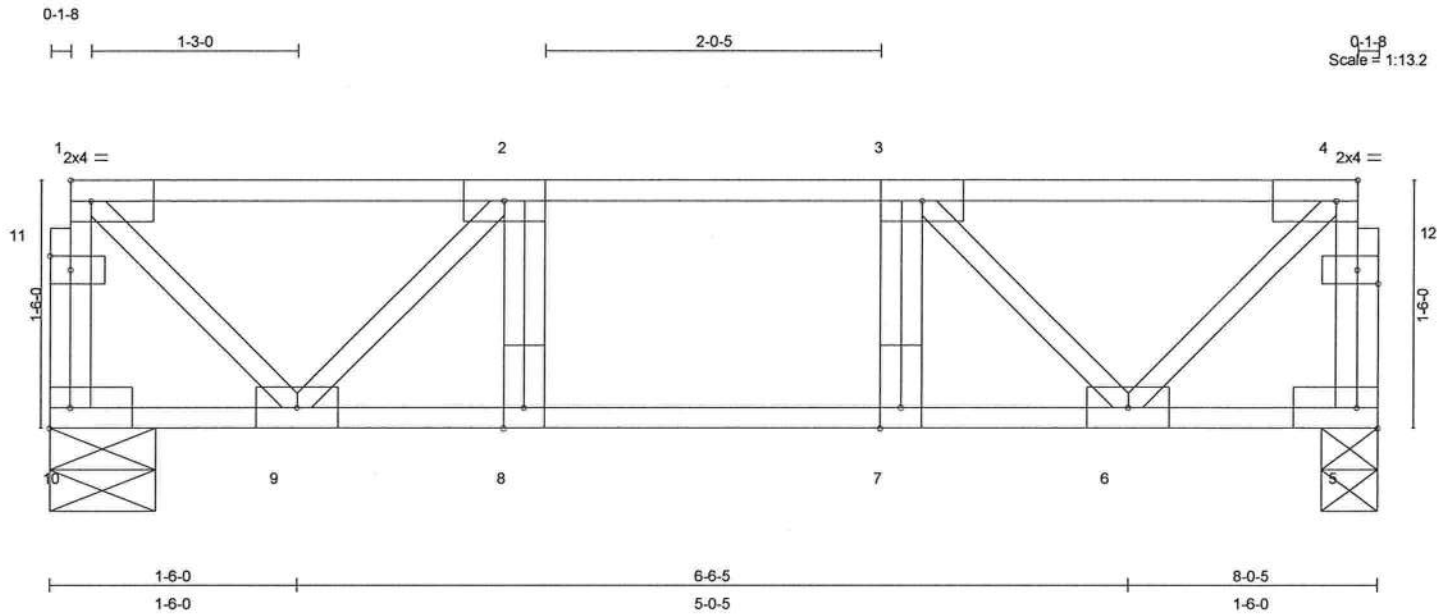


Plate Offsets (X,Y): [4:0-1-8,Edge], [11:0-1-8,0-1-0], [12:0-1-8,0-1-0]

LOADING (psf)	SPACING 1-7-3	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.27	Vert(LL) -0.03 7 >999 360	MT20	244/190
TCDL 10.0	Lumber Increase 1.00	BC 0.26	Vert(TL) -0.03 8 >999 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.13	Horz(TL) 0.00 5 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 48 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=337/0-7-12, 5=337/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 10-11=-332/0, 1-11=-332/0, 5-12=-332/0, 4-12=-332/0, 1-2=-247/0, 2-3=-479/0, 3-4=-247/0
 BOT CHORD 9-10=0/15, 8-9=0/479, 7-8=0/479, 6-7=0/479, 5-6=0/15
 WEBS 4-6=0/336, 1-9=0/336, 3-6=-328/0, 2-9=-328/0, 2-8=-46/76, 3-7=-46/76

JOINT STRESS INDEX

1 = 0.39, 2 = 0.19, 3 = 0.19, 4 = 0.39, 5 = 0.26, 6 = 0.40, 7 = 0.07, 8 = 0.07, 9 = 0.40, 10 = 0.26, 11 = 0.00, 11 = 0.00, 12 = 0.00 and 12 = 0.00

NOTES

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x6 MT20 unless otherwise indicated.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Julius Lee
 Truss Design Engineer
 Florida P.E. No. 34589
 1159 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936283
L266887F	F07	FLOOR	1	1	Job Reference (optional)	

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6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:49 2008 Page 1

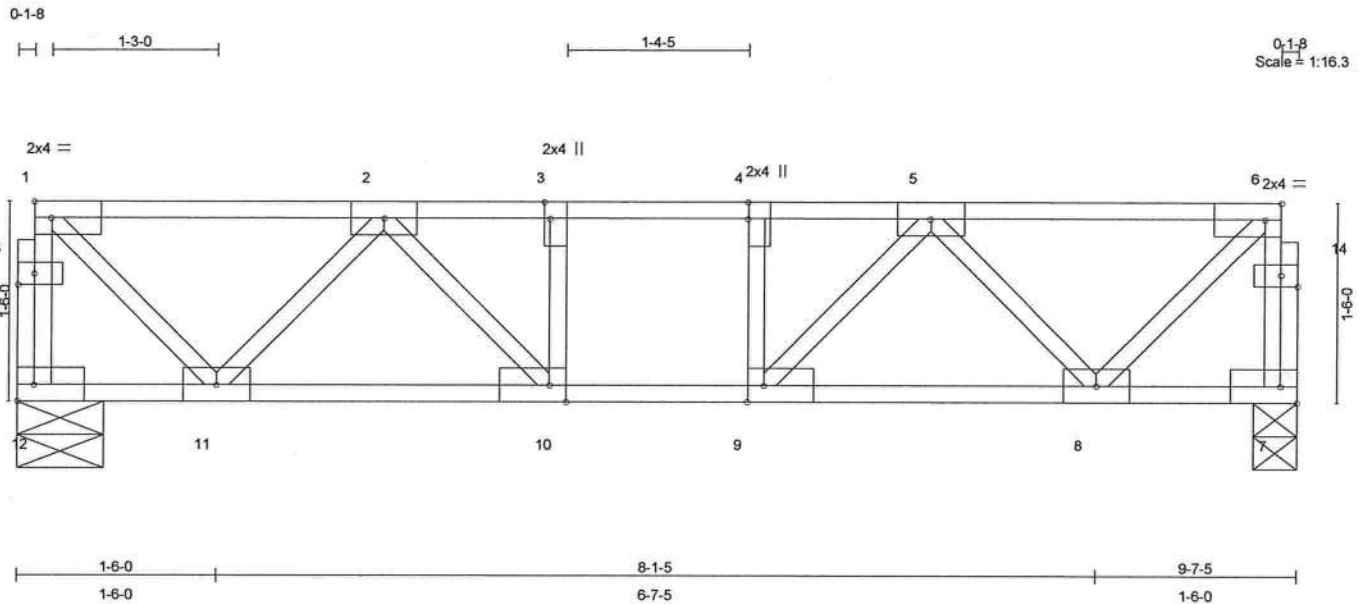


Plate Offsets (X,Y): [3:0-1-8,Edge], [4:0-1-8,Edge], [6:0-1-8,Edge], [9:0-1-8,Edge], [10:0-1-8,Edge], [13:0-1-8,0-1-0], [14:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.25	Vert(LL)	-0.02 10-11	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.19	Vert(TL)	-0.03 10-11	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.18	Horz(TL)	0.01 7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 55 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 12=407/0-7-12, 7=407/0-4-0

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 12-13=-403/0, 1-13=-403/0, 7-14=-403/0, 6-14=-403/0, 1-2=-322/0, 2-3=-687/0, 3-4=-687/0, 4-5=-687/0, 5-6=-322/0
BOT CHORD 11-12=0/18, 10-11=0/596, 9-10=0/687, 8-9=0/596, 7-8=0/18
WEBS 6-8=0/441, 1-11=0/441, 5-8=-406/0, 2-11=-406/0, 5-9=0/230, 2-10=0/230, 3-10=-122/0, 4-9=-122/0

JOINT STRESS INDEX

1 = 0.50, 2 = 0.26, 3 = 0.08, 4 = 0.08, 5 = 0.26, 6 = 0.50, 7 = 0.31, 8 = 0.51, 9 = 0.26, 10 = 0.26, 11 = 0.51, 12 = 0.31, 13 = 0.00, 13 = 0.00, 14 = 0.00 and 14 = 0.00

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x6 MT20 unless otherwise indicated.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Lee
Truss Design Engineer
Florida PE No. 21803
1199 Coastal Bay Blvd
Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

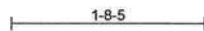
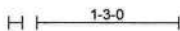


Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936284
L266887F	F08	FLOOR	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:50 2008 Page 1

0-1-8



0-1-8
Scale = 1:19.1

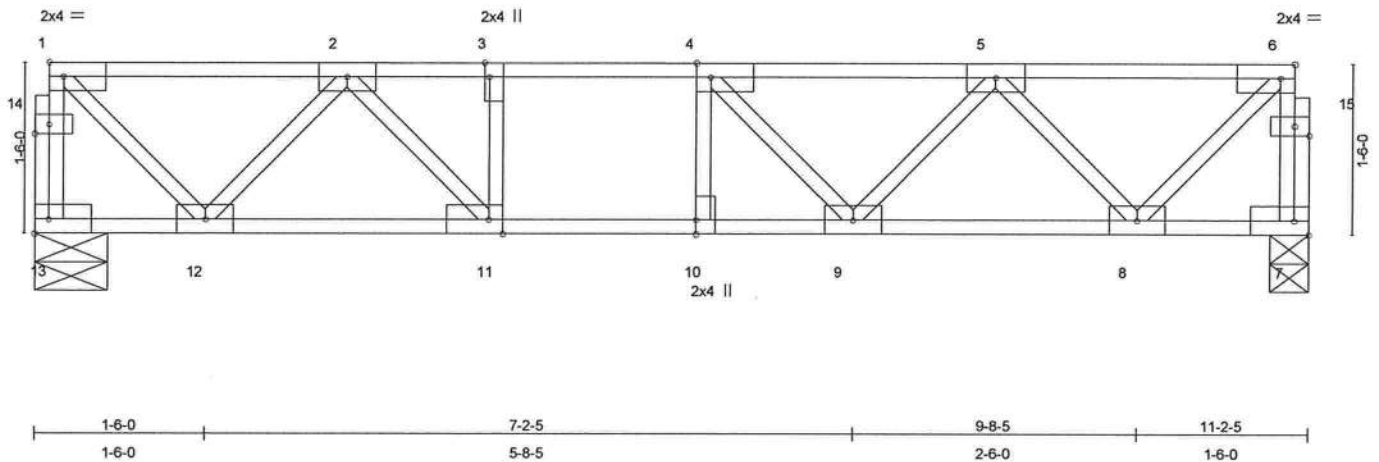


Plate Offsets (X,Y): [3:0-1-8,Edge], [4:0-1-8,Edge], [6:0-1-8,Edge], [10:0-1-8,Edge], [11:0-1-8,Edge], [14:0-1-8,0-1-0], [15:0-1-8,0-1-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase 1.00	TC 0.33	Vert(LL)	-0.05	9-10	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase 1.00	BC 0.45	Vert(TL)	-0.08	9-10	>999	240		
BCLL 0.0	Rep Stress Incr YES	WB 0.22	Horz(TL)	0.01	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 62 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 13=476/0-7-12, 7=476/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 13-14=-466/0, 1-14=-466/0, 7-15=-471/0, 6-15=-471/0, 1-2=-383/0, 2-3=-932/0, 3-4=-932/0, 4-5=-859/0, 5-6=-396/0
 BOT CHORD 12-13=0/21, 11-12=0/731, 10-11=0/932, 9-10=0/932, 8-9=0/741, 7-8=0/21
 WEBS 6-8=0/542, 1-12=0/524, 5-8=-514/0, 2-12=-517/0, 5-9=0/203, 2-11=0/379, 4-9=-199/0, 3-11=-175/0, 4-10=-100/30

JOINT STRESS INDEX

1 = 0.59, 2 = 0.43, 3 = 0.11, 4 = 0.11, 5 = 0.29, 6 = 0.61, 7 = 0.36, 8 = 0.62, 9 = 0.23, 10 = 0.06, 11 = 0.43, 12 = 0.60, 13 = 0.36, 14 = 0.00, 14 = 0.00, 15 = 0.00 and 15 = 0.00

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x6 MT20 unless otherwise indicated.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Lee
 Truss Design Engineer
 Florida Reg. No. 31868
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

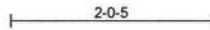


Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F09	FLOOR	1	1	J1936285
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:50 2008 Page 1

0-1-8



0,1,8
Scale = 1:21.9

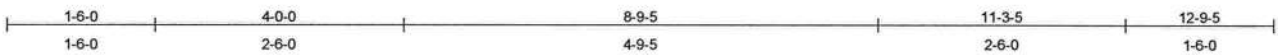
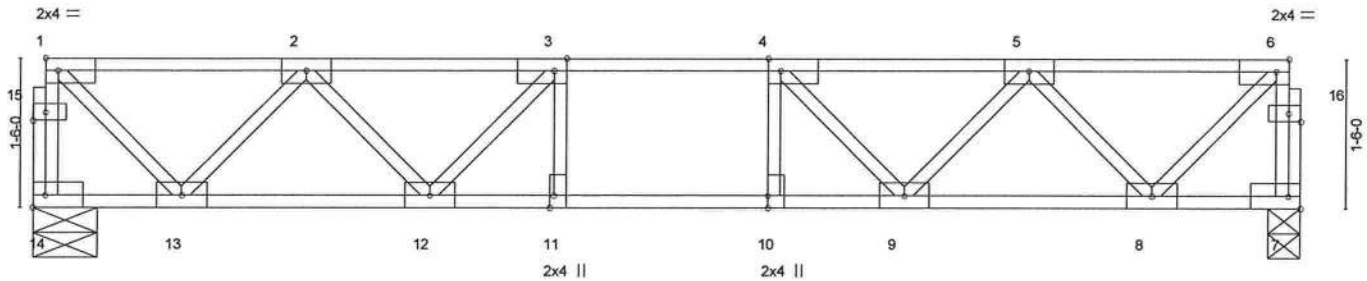


Plate Offsets (X,Y): [3:0-1-8,Edge], [4:0-1-8,Edge], [6:0-1-8,Edge], [10:0-1-8,0-0-0], [11:0-1-8,Edge], [15:0-1-8,0-1-0], [16:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.30	Vert(LL)	-0.06 11-12	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.44	Vert(TL)	-0.08 11-12	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.26	Horz(TL)	0.02 7	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						
								Weight: 69 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 14=546/0-7-12, 7=546/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 14-15=-542/0, 1-15=-542/0, 7-16=-542/0, 6-16=-542/0, 1-2=-464/0, 2-3=-1056/0, 3-4=-1239/0, 4-5=-1056/0, 5-6=-464/0
 BOT CHORD 13-14=0/25, 12-13=0/868, 11-12=0/1239, 10-11=0/1239, 9-10=0/1239, 8-9=0/868, 7-8=0/25
 WEBS 6-8=0/637, 1-13=0/637, 5-8=-599/0, 2-13=-599/0, 5-9=0/295, 2-12=0/295, 4-9=-349/0, 3-12=-349/0, 3-11=-78/104, 4-10=-78/104

JOINT STRESS INDEX

1 = 0.72, 2 = 0.34, 3 = 0.20, 4 = 0.20, 5 = 0.34, 6 = 0.72, 7 = 0.41, 8 = 0.73, 9 = 0.34, 10 = 0.13, 11 = 0.13, 12 = 0.34, 13 = 0.73, 14 = 0.41, 15 = 0.00, 15 = 0.00, 16 = 0.00 and 16 = 0.00

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x6 MT20 unless otherwise indicated.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Lee
 Truss Design Engineer
 Florida P.E. No. 34888
 1109 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

LOAD CASE(S) Standard

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Julius Lee
Truss Design Engineer
Florida P.E. No. 34888
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

February 19, 2008

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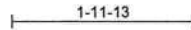
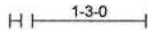


Job L266887F	Truss F10	Truss Type FLOOR	Qty 1	Ply 1	MIKE & LINDA CADY / FLOOR J1936286 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 19 10:16:33 2008 Page 1

0-1-8



0-1-8
Scale: 1/2"=1'

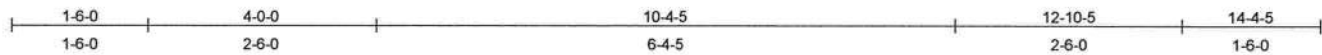
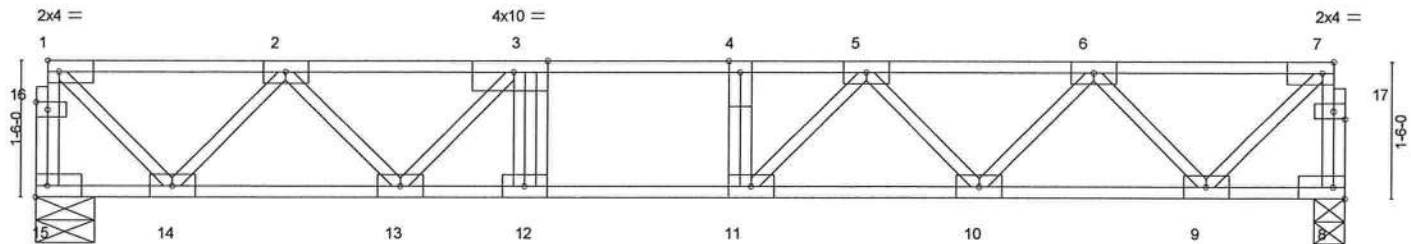


Plate Offsets (X,Y): [3:0-4-8,Edge], [7:0-1-8,Edge], [16:0-1-8,0-1-0], [17:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.42	Vert(LL)	-0.12 10-11	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.64	Vert(TL)	-0.17 10-11	>995	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.29	Horz(TL)	0.02 8	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						
								Weight: 82 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 15=615/0-7-12, 8=615/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 15-16=-613/0, 1-16=-613/0, 8-17=-610/0, 7-17=-609/0, 1-2=-533/0, 2-3=-1254/0,
 3-4=-1558/0, 4-5=-1558/0, 5-6=-1259/0, 6-7=-532/0
 BOT CHORD 14-15=0/28, 13-14=0/994, 12-13=0/1556, 11-12=0/1558, 10-11=0/1482, 9-10=0/1002,
 8-9=0/28
 WEBS 7-9=0/730, 1-14=0/732, 6-9=-699/0, 2-14=-686/0, 6-10=0/382, 2-13=0/391, 5-10=-332/0,
 3-13=-490/0, 5-11=-57/310, 3-12=-39/138, 4-11=-161/0

JOINT STRESS INDEX

1 = 0.85, 2 = 0.46, 3 = 0.16, 4 = 0.07, 5 = 0.37, 6 = 0.45, 7 = 0.85, 8 = 0.48, 9 = 0.86, 10 = 0.45, 11 = 0.37, 12 = 0.07, 13 = 0.46, 14 = 0.86, 15 = 0.48, 16 = 0.00, 16 = 0.00, 17 = 0.00 and 17 = 0.00

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x6 MT20 unless otherwise indicated.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails.
 Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Law
 Truss Design Engineer
 Florida PE No. 31888
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Julius Lutz
Truss Design Engineer
Florida Lic. No. 34868
1100 Coastal Bay Blvd
Boynton Beach, FL 33426

February 19, 2008

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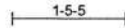
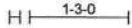


Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936287
L266887F	F11	FLOOR	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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0-1-8



0-1-8
Scale = 1:27.4

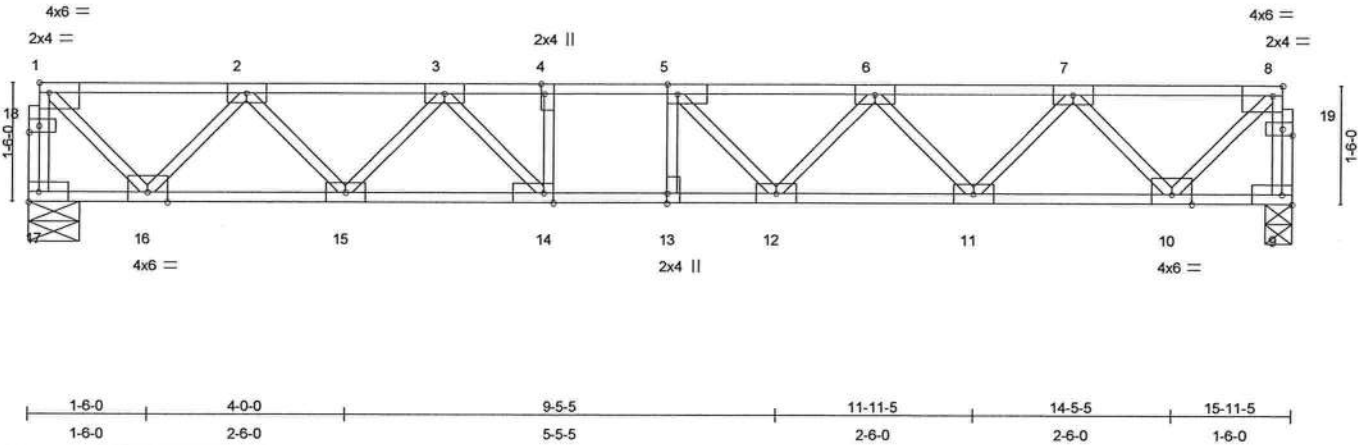


Plate Offsets (X,Y): [1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,Edge], [8:0-1-8,Edge], [13:0-1-8,Edge], [14:0-1-8,Edge], [18:0-1-8,0-1-0], [19:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.36	Vert(LL)	-0.10 12-13	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.67	Vert(TL)	-0.16 12-13	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.33	Horz(TL)	0.03 9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 87 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 17=685/0-7-12, 9=685/0-4-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 17-18=-683/0, 1-18=-682/0, 9-19=-682/0, 8-19=-681/0, 1-2=-605/0, 2-3=-1442/0, 3-4=-1947/0, 4-5=-1947/0, 5-6=-1875/0, 6-7=-1453/0, 7-8=-602/0
 BOT CHORD 16-17=0/31, 15-16=0/1133, 14-15=0/1747, 13-14=0/1947, 12-13=0/1947, 11-12=0/1762, 10-11=0/1129, 9-10=0/31
 WEBS 8-10=0/826, 1-16=0/831, 7-10=-784/0, 2-16=-784/0, 7-11=0/482, 2-15=0/460, 6-11=-459/0, 3-15=-453/0, 6-12=0/250, 3-14=0/437, 5-12=-273/78, 4-14=-192/0, 5-13=-135/66

JOINT STRESS INDEX

1 = 0.52, 2 = 0.53, 3 = 0.50, 4 = 0.12, 5 = 0.15, 6 = 0.29, 7 = 0.55, 8 = 0.52, 9 = 0.52, 10 = 0.54, 11 = 0.55, 12 = 0.29, 13 = 0.08, 14 = 0.50, 15 = 0.53, 16 = 0.54, 17 = 0.52, 18 = 0.00, 18 = 0.00, 19 = 0.00 and 19 = 0.00

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x6 MT20 unless otherwise indicated.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Lee
 Truss Design Engineer
 Florida PE No. 31883F
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F11	FLOOR	1	1	J1936287 Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:52 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 21903
1109 Coastal Bay Blvd
Boynton Beach, FL 33426

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job L266887F	Truss F12	Truss Type FLOOR	Qty 1	Ply 1	MIKE & LINDA CADY / FLOOR J1936288 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:52 2008 Page 1

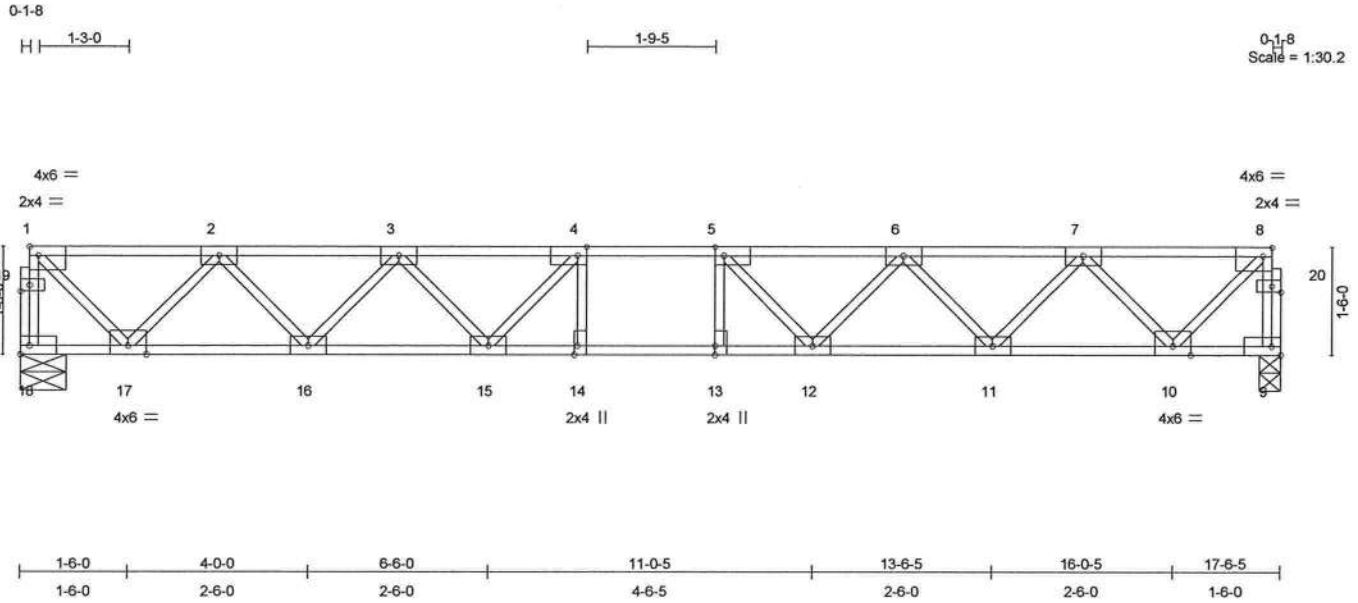


Plate Offsets (X,Y): [1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,Edge], [8:0-1-8,Edge], [13:0-1-8,Edge], [14:0-1-8,Edge], [19:0-1-8,0-1-0], [20:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.31	Vert(LL)	-0.12 13-14	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.71	Vert(TL)	-0.20 13-14	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.37	Horz(TL)	0.04 9	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						
								Weight: 94 lb	

LUMBER	BRACING
TOP CHORD 4 X 2 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 4 X 2 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 4 X 2 SYP No.3	

REACTIONS (lb/size) 18=755/0-7-12, 9=755/0-3-8

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 18-19=-751/0, 1-19=-750/0, 9-20=-751/0, 8-20=-750/0, 1-2=-671/0, 2-3=-1649/0, 3-4=-2198/0, 4-5=-2376/0, 5-6=-2198/0, 6-7=-1649/0, 7-8=-671/0
 BOT CHORD 17-18=0/34, 16-17=0/1263, 15-16=0/2015, 14-15=0/2376, 13-14=0/2376, 12-13=0/2376, 11-12=0/2015, 10-11=0/1263, 9-10=0/34
 WEBS 8-10=0/923, 1-17=0/923, 7-10=-880/0, 2-17=-880/0, 7-11=0/573, 2-16=0/573, 6-11=-545/0, 3-16=-545/0, 6-12=0/340, 3-15=0/340, 5-12=-422/8, 4-15=-422/8, 4-14=-124/147, 5-13=-124/147

JOINT STRESS INDEX
 1 = 0.58, 2 = 0.66, 3 = 0.55, 4 = 0.64, 5 = 0.64, 6 = 0.55, 7 = 0.66, 8 = 0.58, 9 = 0.64, 10 = 0.60, 11 = 0.66, 12 = 0.55, 13 = 0.47, 14 = 0.47, 15 = 0.55, 16 = 0.66, 17 = 0.60, 18 = 0.64, 19 = 0.00, 19 = 0.47, 20 = 0.00 and 20 = 0.47

- NOTES
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x6 MT20 unless otherwise indicated.
 - 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Lee
 Truss Design Engineer
 Florida PE No. 24886B
 1400 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F12	FLOOR	1	1	J1936288
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:53 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 24868
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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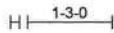


Job L266887F	Truss F13	Truss Type FLOOR	Qty 1	Ply 1	MIKE & LINDA CADY / FLOOR Job Reference (optional)	J1936289
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Feb 19 10:17:12 2008 Page 1

0-1-8



0-1-8
Scale: 3/8"=1'

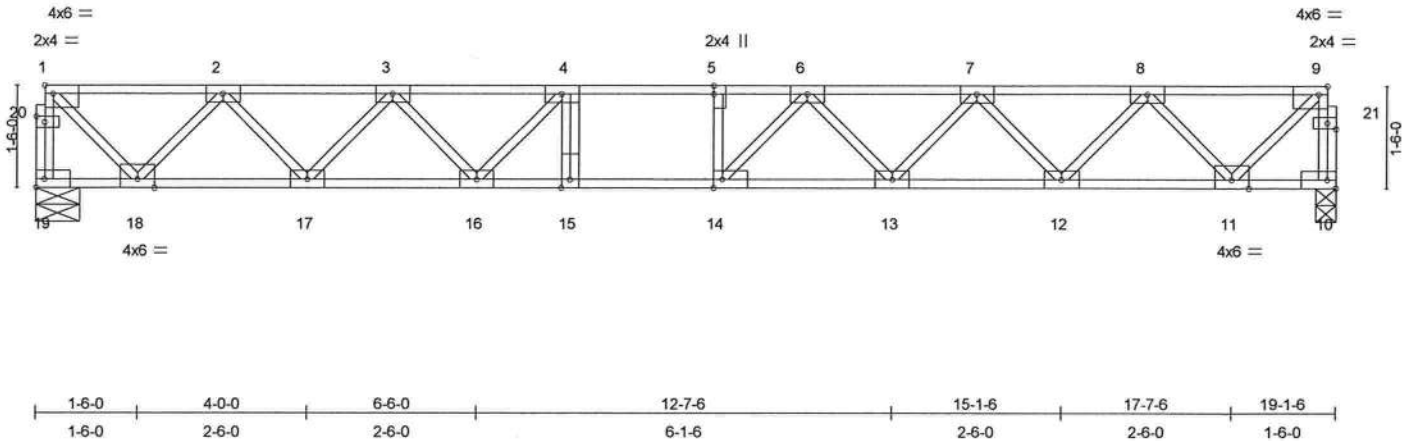


Plate Offsets (X,Y): [1:Edge,0-1-8], [5:0-1-8,0-0-0], [9:0-1-8,Edge], [14:0-1-8,Edge], [20:0-1-8,0-1-0], [21:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.50	Vert(LL)	-0.21 13-14	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.89	Vert(TL)	-0.32 13-14	>709	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.41	Horz(TL)	0.06 10	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						
								Weight: 103 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 19=824/0-7-12, 10=824/0-3-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 19-20=-820/0, 1-20=-819/0, 10-21=-821/0, 9-21=-820/0, 1-2=-741/0, 2-3=-1843/0, 3-4=-2523/0, 4-5=-2811/0, 5-6=-2811/0, 6-7=-2527/0, 7-8=-1842/0, 8-9=-741/0
 BOT CHORD 18-19=0/37, 17-18=0/1397, 16-17=0/2268, 15-16=0/2811, 14-15=0/2811, 13-14=0/2751, 12-13=0/2274, 11-12=0/1396, 10-11=0/37
 WEBS 9-11=0/1019, 1-18=0/1019, 8-11=-973/0, 2-18=-976/0, 8-12=0/664, 2-17=0/664, 7-12=-641/0, 3-17=-631/0, 7-13=0/376, 3-16=0/438, 6-13=-332/0, 4-16=-562/0, 6-14=-169/390, 4-15=-85/169, 5-14=-192/27

JOINT STRESS INDEX

1 = 0.66, 2 = 0.78, 3 = 0.56, 4 = 0.66, 5 = 0.49, 6 = 0.56, 7 = 0.56, 8 = 0.78, 9 = 0.66, 10 = 0.66, 11 = 0.68, 12 = 0.78, 13 = 0.56, 14 = 0.66, 15 = 0.32, 16 = 0.56, 17 = 0.78, 18 = 0.68, 19 = 0.66, 20 = 0.00, 20 = 0.49, 21 = 0.00 and 21 = 0.49

NOTES

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x6 MT20 unless otherwise indicated.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Julius Lee
 Truss Design Engineer
 Florida PE No. 3-1868
 15109 Coastal Bay Blvd
 Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MI TEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F14	FLOOR	1	1	J1936290
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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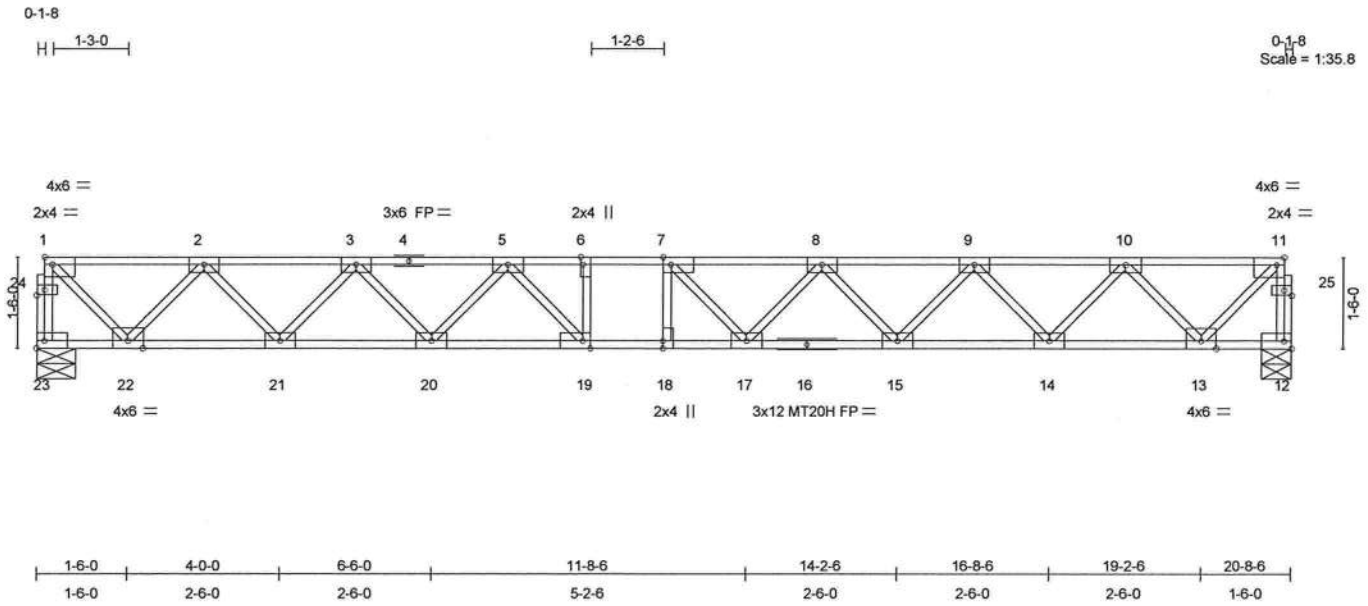


Plate Offsets (X,Y): [1:Edge,0-1-8], [6:0-1-8,Edge], [7:0-1-8,Edge], [11:0-1-8,Edge], [18:0-1-8,0-0-0], [19:0-1-8,Edge], [24:0-1-8,0-1-0], [25:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.41	Vert(LL)	-0.25 17-18	>987	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.90	Vert(TL)	-0.39 17-18	>634	240	MT20H	187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.45	Horz(TL)	0.07 12	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						Weight: 111 lb

LUMBER
 TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 23=894/0-7-12, 12=894/0-6-0

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 23-24=-890/0, 1-24=-889/0, 12-25=-890/0, 11-25=-889/0, 1-2=-810/0, 2-3=-2041/0, 3-4=-2839/0, 4-5=-2839/0, 5-6=-3320/0, 6-7=-3320/0, 7-8=-3257/0, 8-9=-2849/0, 9-10=-2038/0, 10-11=-811/0
 BOT CHORD 22-23=0/40, 21-22=0/1529, 20-21=0/2531, 19-20=0/3141, 18-19=0/3320, 17-18=0/3320, 16-17=0/3155, 15-16=0/3155, 14-15=0/2528, 13-14=0/1529, 12-13=0/40
 WEBS 11-13=0/1115, 1-22=0/1114, 10-13=-1069/0, 2-22=-1069/0, 10-14=0/756, 2-21=0/762, 9-14=-728/0, 3-21=-729/0, 9-15=0/477, 3-20=0/458, 8-15=-455/0, 5-20=-449/0, 8-17=-35/294, 5-19=-80/483, 7-17=-331/164, 6-19=-202/7, 7-18=-165/94

Julius Lee
 Truss Design Engineer
 Florida PE No. 34869
 1400 Crystal Bay Blvd.
 Boynton Beach, FL 33486

JOINT STRESS INDEX
 1 = 0.70, 2 = 0.87, 3 = 0.55, 4 = 0.36, 5 = 0.55, 6 = 0.47, 7 = 0.64, 8 = 0.55, 9 = 0.55, 10 = 0.87, 11 = 0.70, 12 = 0.68, 13 = 0.73, 14 = 0.87, 15 = 0.55, 16 = 0.62, 17 = 0.55, 18 = 0.47, 19 = 0.64, 20 = 0.55, 21 = 0.87, 22 = 0.73, 23 = 0.68, 24 = 0.00, 24 = 0.47, 25 = 0.00 and 25 = 0.47

Continued on page 2

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F14	FLOOR	1	1	J1936290 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 16 and 4.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 2-1868
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F15	FLOOR	1	1	J1936291
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:55 2008 Page 1

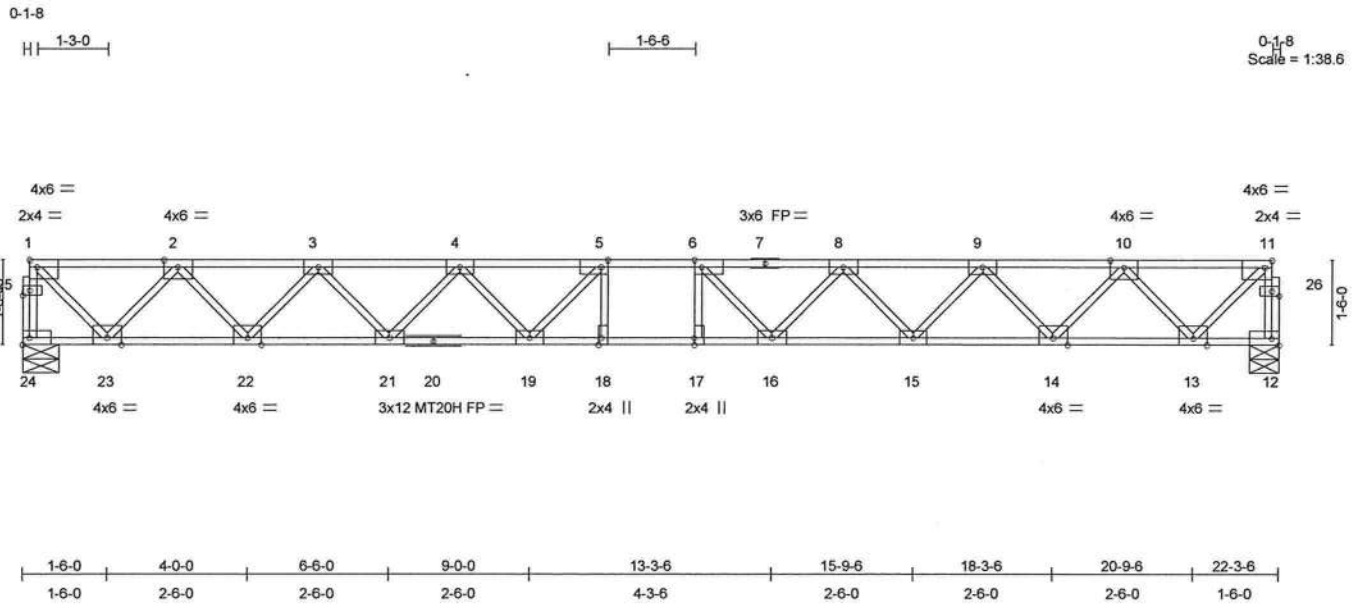


Plate Offsets (X,Y): [1:Edge,0-1-8], [5:0-1-8,Edge], [6:0-1-8,Edge], [11:0-1-8,Edge], [17:0-1-8,0-0-0], [18:0-1-8,Edge], [25:0-1-8,0-1-0], [26:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.42	Vert(LL)	-0.32 17-18	>839	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.99	Vert(TL)	-0.49 17-18	>536	240	MT20H	187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.49	Horz(TL)	0.09 12	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						Weight: 119 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
 2-2-0 oc bracing: 18-19,17-18,16-17.

REACTIONS (lb/size) 24=964/0-7-12, 12=964/0-6-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 24-25=-960/0, 1-25=-959/0, 12-26=-960/0, 11-26=-959/0, 1-2=-880/0, 2-3=-2234/0, 3-4=-3170/0, 4-5=-3706/0, 5-6=-3870/0, 6-7=-3706/0, 7-8=-3706/0, 8-9=-3170/0, 9-10=-2234/0, 10-11=-880/0
 BOT CHORD 23-24=0/44, 22-23=0/1662, 21-22=0/2788, 20-21=0/3535, 19-20=0/3535, 18-19=0/3870, 17-18=0/3870, 16-17=0/3870, 15-16=0/3535, 14-15=0/2788, 13-14=0/1662, 12-13=0/44
 WEBS 11-13=0/1211, 1-23=0/1211, 10-13=-1162/0, 2-23=-1162/0, 10-14=0/851, 2-22=0/851, 9-14=-823/0, 3-22=-823/0, 9-15=0/569, 3-21=0/569, 8-15=-542/0, 4-21=-542/0, 8-16=0/383, 4-19=0/383, 6-16=-481/107, 5-19=-481/107, 5-18=-163/183, 6-17=-163/183

Julius Lee
 Truss Design Engineer
 Florida PE No. 23859
 1400 Coastal Bay Blvd
 Boynton Beach, FL 33435

JOINT STRESS INDEX

1 = 0.76, 2 = 0.55, 3 = 0.65, 4 = 0.55, 5 = 0.64, 6 = 0.64, 7 = 0.47, 8 = 0.55, 9 = 0.65, 10 = 0.55, 11 = 0.76, 12 = 0.73, 13 = 0.79, 14 = 0.55, 15 = 0.65, 16 = 0.55, 17 = 0.47, 18 = 0.47, 19 = 0.55, 20 = 0.69, 21 = 0.65, 22 = 0.55, 23 = 0.79, 24 = 0.73, 25 = 0.00, 25 = 0.47, 26 = 0.00 and 26 = 0.47

Continued on page 2

February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F15	FLOOR	1	1	J1936291
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:55 2008 Page 2

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 20 and 7.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34865
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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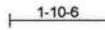


Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR	J1936292
L266887F	F16	FLOOR	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:56 2008 Page 1

0-1-8



0-1-8
Scale = 1:41.3

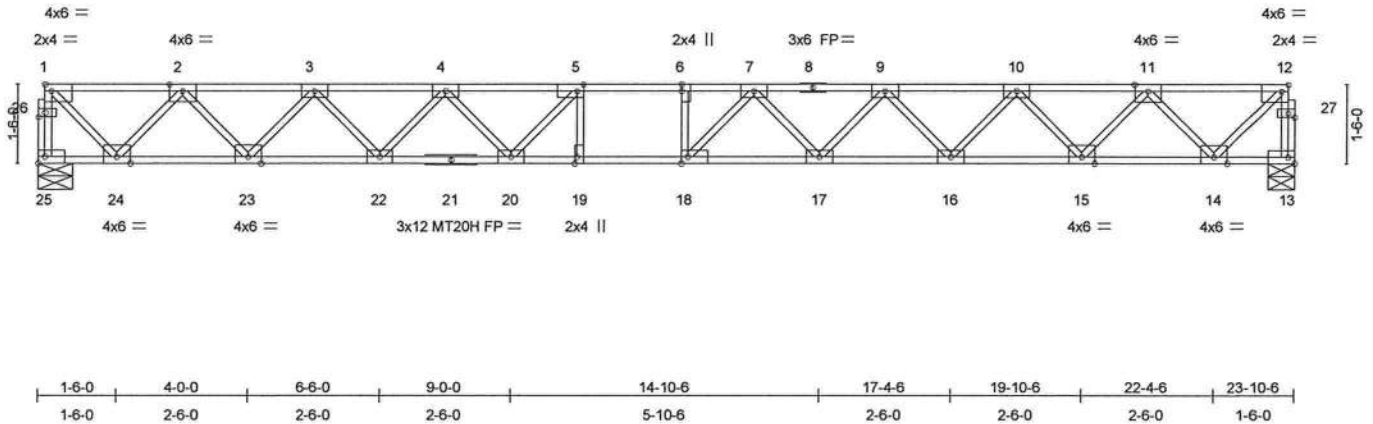


Plate Offsets (X,Y): [1:Edge,0-1-8], [5:0-1-8,Edge], [6:0-1-8,0-0-0], [12:0-1-8,Edge], [18:0-1-8,Edge], [19:0-1-8,Edge], [26:0-1-8,0-1-0], [27:0-1-8,0-1-0]

LOADING (psf)	SPACING	1-7-3	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.58	Vert(LL)	-0.41 18	>695	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.91	Vert(TL)	-0.64 17-18	>443	240	MT20H	187/143
BCLL 0.0	Rep Stress Incr	YES	WB 0.52	Horz(TL)	0.10 13	n/a	n/a		
BCDL 5.0	Code	FBC2004/TPI2002	(Matrix)						Weight: 126 lb

LUMBER

TOP CHORD 4 X 2 SYP No.2
 BOT CHORD 4 X 2 SYP No.2 *Except*
 13-21 4 X 2 SYP No.1D
 WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or
 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
 bracing.

REACTIONS (lb/size) 25=1033/0-7-12, 13=1033/0-6-0

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 25-26=-1029/0, 1-26=-1028/0, 13-27=-1029/0, 12-27=-1028/0, 1-2=-949/0,
 2-3=-2430/0, 3-4=-3492/0, 4-5=-4157/0, 5-6=-4422/0, 6-7=-4422/0, 7-8=-4161/0,
 8-9=-4161/0, 9-10=-3490/0, 10-11=-2430/0, 11-12=-949/0
 BOT CHORD 24-25=0/47, 23-24=0/1794, 22-23=0/3048, 21-22=0/3913, 20-21=0/3913,
 19-20=0/4422, 18-19=0/4422, 17-18=0/4373, 16-17=0/3920, 15-16=0/3047,
 14-15=0/1794, 13-14=0/47
 WEBS 12-14=0/1307, 1-24=0/1307, 11-14=-1256/0, 2-24=-1256/0, 11-15=0/946,
 2-23=0/946, 10-15=-916/0, 3-23=-919/0, 10-16=0/659, 3-22=0/659, 9-16=-638/0,
 4-22=-627/0, 9-17=0/359, 4-20=0/483, 7-17=-382/0, 5-20=-628/39, 7-18=-263/454,
 5-19=-131/217, 6-18=-208/60

Julius Lee
 Truss Design Engineer
 Florida PE No. 34869
 1400 Coastal Bay Blvd
 Boynton Beach, FL 33435

JOINT STRESS INDEX

1 = 0.82, 2 = 0.62, 3 = 0.76, 4 = 0.55, 5 = 0.64, 6 = 0.47, 7 = 0.55, 8 = 0.52, 9 = 0.55, 10 = 0.76, 11 = 0.62, 12 = 0.82, 13 =
 0.79, 14 = 0.85, 15 = 0.62, 16 = 0.76, 17 = 0.55, 18 = 0.64, 19 = 0.47, 20 = 0.55, 21 = 0.77, 22 = 0.76, 23 = 0.62, 24 = 0.85,
 25 = 0.79, 26 = 0.00, 26 = 0.47, 27 = 0.00 and 27 = 0.47

February 19,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY / FLOOR
L266887F	F16	FLOOR	1	1	J1936292 Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:56 2008 Page 2

NOTES

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 21 and 8.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

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February 19,2008

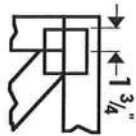
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

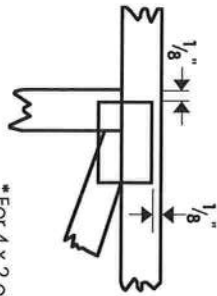


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

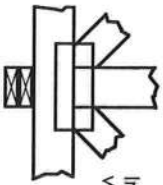
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



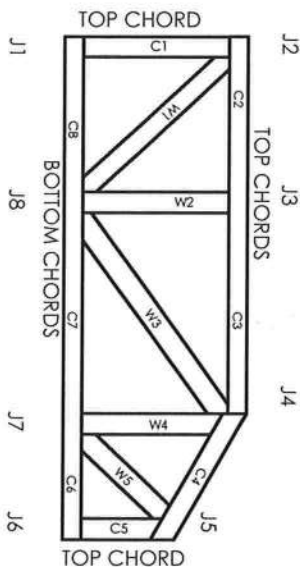
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System

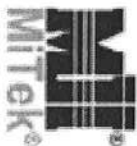


JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561



MITek Engineering Reference Sheet: MIT-7473

General Safety Notes

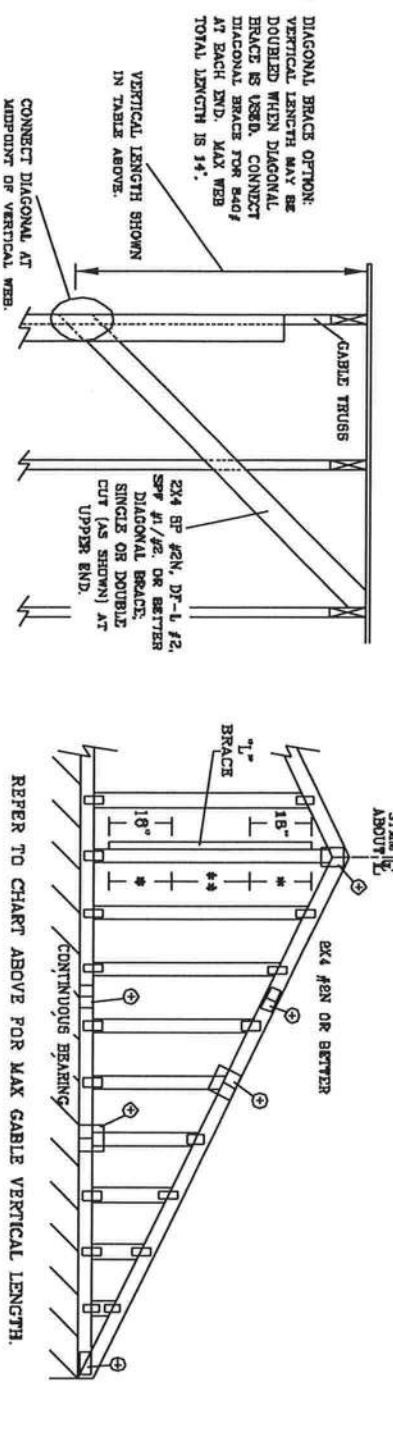
Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		2x4 CABLE VERTICAL SPACING		BRACE		NO BRACES		GROUP A		GROUP B		GROUP A		GROUP B		GROUP A		GROUP B		
SPECIES	GRADE	SPF	HF	SP	DFL	SPF	HF	SP	DFL	SPF	HF	SP	DFL	SPF	HF	SP	DFL	SPF	HF	
																				GROUP A
12" O.C.	16" O.C.	24" O.C.																		



REFER TO CHART ABOVE FOR MAX CABLE VERTICAL LENGTH.

ATTACH EACH "L" BRACE WITH 10d NAILS.
 * FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
 "L" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1x4 OR 2x3
GREATER THAN 4' 0" BUT LESS THAN 11' 8"	2x4
GREATER THAN 11' 8"	2.5x4

+ REFER TO COMMON TRUSS DESIGN FOR MAX. SPLICE AND HEEL PLATES.

BRACING GROUP SPECIES AND GRADES:

GROUP A:		GROUP B:	
SPRUCE-PINE-TR	HEM-PIR	SPRUCE-PINE-TR	HEM-PIR
#1 / #2	#1	#1 / #2	#1
STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD

DOUGLAS FIR-LARCH

#3	#3
STUD	STUD
STANDARD	STANDARD

SOUTHERN PINE

#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-PIR	DOUGLAS FIR-LARCH
#1 & BTR	#1
#1	#1

CABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEFLECTION CENTERLINE IS L/240.
 PROVIDE UPLIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BEARING (6 PSF VC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

WARNING TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2021 I-60 (BUILDING CONSTRUCTION SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 283 DORCHESTER DR., SUITE 200, MADISON, VA 22795 AND VTEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENCLERFEST LN, MADISON, VA 22719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THE FACTURING. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PARALLEL ATTACHED RIGID CEILING.

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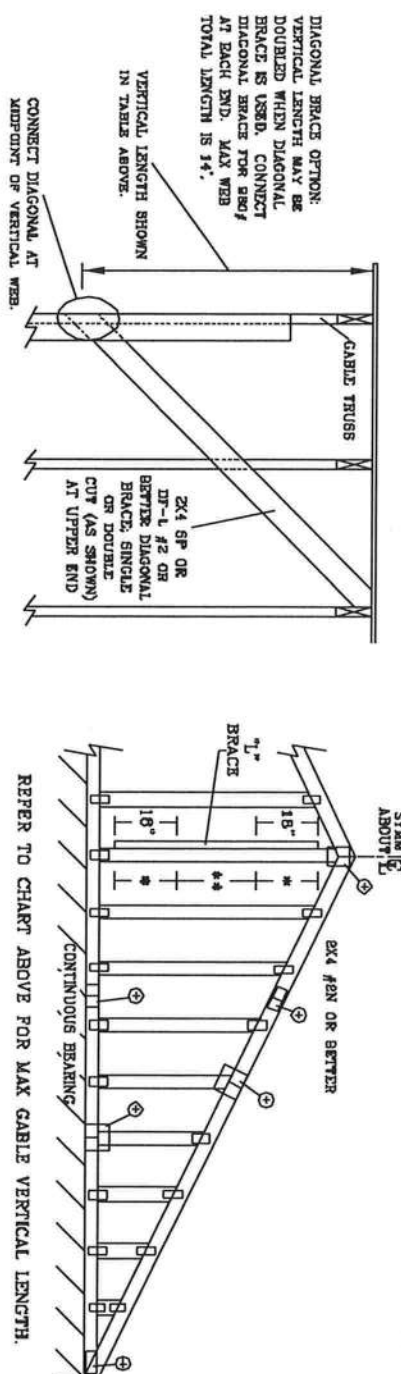
Ref. 34889
 STATE OF FLORIDA

MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCE 7-02-CAB13015
 DATE 11/26/03
 DRWG MTKX STD CABLE 15 E HT
 -ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

GABLE VERTICAL SPACING	2x4 BRACE SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 1" BRACE *		(1) 2x4 1" BRACE *		(2) 2x4 1" BRACE *		(1) 2x6 1" BRACE *		(2) 2x6 1" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B		
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
	HF	STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	6' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
		#1	3' 8"	5' 6"	5' 11"	8' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 6"	5' 6"	5' 11"	8' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
	DFL	#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	6' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"
		#1 / #2	3' 9"	6' 4"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	8' 11"	11' 2"	14' 0"	14' 0"
	SPF	#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 3"	8' 3"	11' 1"	11' 1"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 3"	8' 3"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 3"	8' 3"	11' 1"	11' 1"	14' 0"	14' 0"
SP	#1	4' 0"	6' 4"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	8' 11"	11' 2"	14' 0"	14' 0"	
	#2	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	6' 3"	6' 3"	9' 9"	9' 9"	13' 3"	13' 3"	
	#3	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	6' 3"	6' 3"	9' 9"	9' 9"	13' 3"	13' 3"	
DFL	STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	6' 3"	6' 3"	9' 9"	9' 9"	13' 3"	13' 3"	
	#1 / #2	4' 0"	6' 11"	6' 11"	8' 3"	8' 3"	8' 3"	8' 3"	12' 11"	12' 11"	14' 0"	14' 0"	
	#3	4' 0"	6' 11"	6' 11"	8' 3"	8' 3"	8' 3"	8' 3"	12' 11"	12' 11"	14' 0"	14' 0"	
SP	STANDARD	3' 11"	6' 3"	6' 3"	6' 3"	6' 3"	6' 3"	6' 3"	9' 9"	9' 9"	13' 3"	13' 3"	
	#1	4' 5"	6' 11"	6' 11"	7' 6"	8' 3"	8' 10"	9' 10"	10' 7"	10' 7"	14' 0"	14' 0"	
	#2	4' 4"	6' 11"	6' 11"	7' 6"	8' 3"	8' 10"	9' 10"	10' 7"	10' 7"	14' 0"	14' 0"	
DFL	STANDARD	4' 2"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	6' 4"	10' 4"	10' 4"	14' 0"	14' 0"	
	#1	4' 5"	6' 11"	6' 11"	7' 6"	8' 3"	8' 10"	9' 10"	10' 7"	10' 7"	14' 0"	14' 0"	
	#2	4' 4"	6' 11"	6' 11"	7' 6"	8' 3"	8' 10"	9' 10"	10' 7"	10' 7"	14' 0"	14' 0"	



MAX GABLE VERTICAL LENGTH

DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR BRUF AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL BRACE OPTION:
VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:	SPF-#1 / #2	STUD	DFL-#1	STUD
GROUP B:	SPF-#3	STUD	DFL-#3	STUD

GROUP A: SOUTHERN PINE #1 & #2

GROUP B: SOUTHERN PINE #1 & #2

CABLE TRUSS DETAIL NOTES:
LIVE LOAD DEFLECTION CRITERIA IS L/240.
PROVIDE UP/LIFT CONNECTIONS FOR 180 PSF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLINES WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH 1" BRACE WITH 10d NAILS.
* FOR (1) 1" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) 1" BRACES: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
1" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1x4 OR 2x3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2x4
GREATER THAN 11' 6"	2.5x4

+ REFER TO COMMON TRUSS DESIGN FOR MAX. SPLICE AND HEEL PLATES.

MANUFACTURERS TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICE GUIDELINES FOR TRUSS CONSTRUCTION. CONSULT WITH TRUSS MANUFACTURER FOR TRUSS CONSTRUCTION. TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR TRUSS CONSTRUCTION. TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR TRUSS CONSTRUCTION. TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR TRUSS CONSTRUCTION.

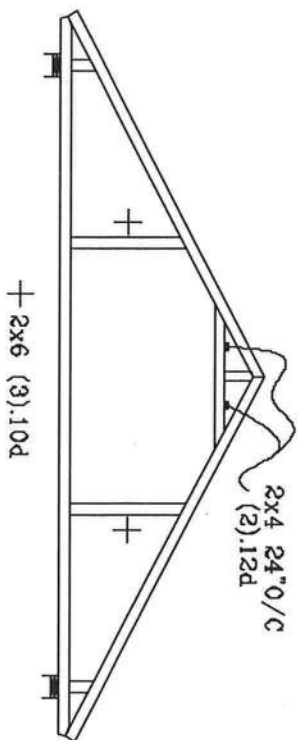
JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-8161

No. 34866
STATE OF FLORIDA

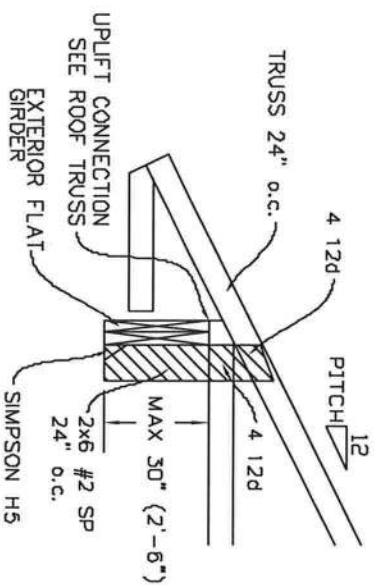
MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF ASCE 7-02-CAB130390
DATE 11/26/03
DWG WAVE STD GABLE 50' E 1/2
-ENG

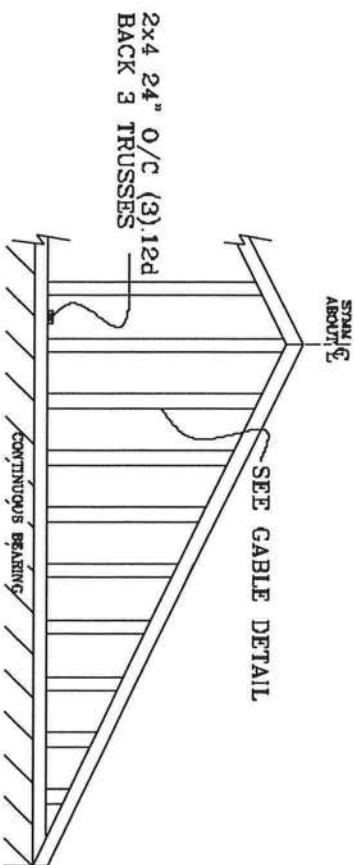
TYPICAL ATTIC TRUSS BRACING



TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

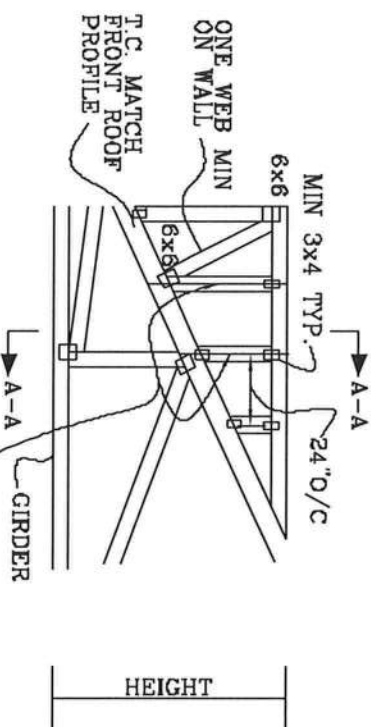


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR BOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



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DELRAT BRANCH, FL 33444-2161

No. 34869
STATE OF FLORIDA

TOP CHORD 2x4 #2 OR BETTER
 BOT CHORD 2x4 #2 OR BETTER
 WEBS 2x4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.
 SPACE PIGGYBACK VERTICALS AT 4' OC MAX.
 TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OR SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

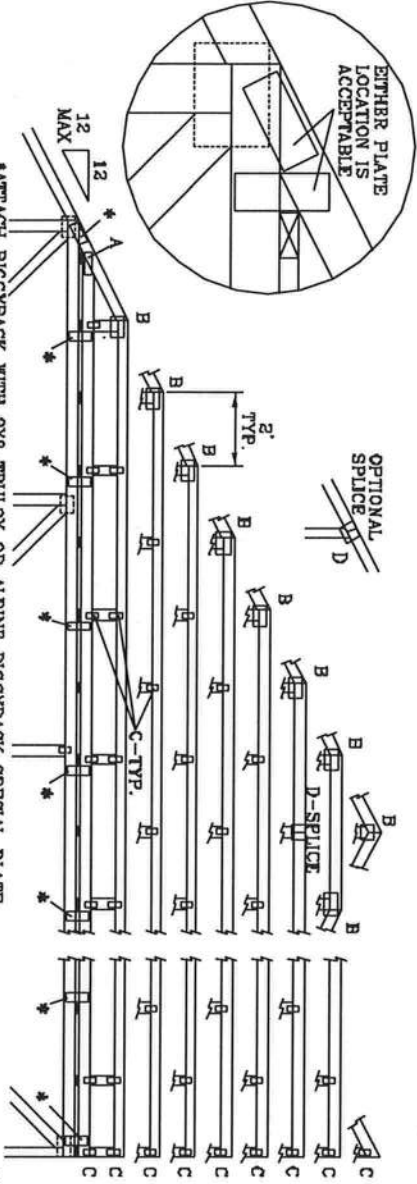
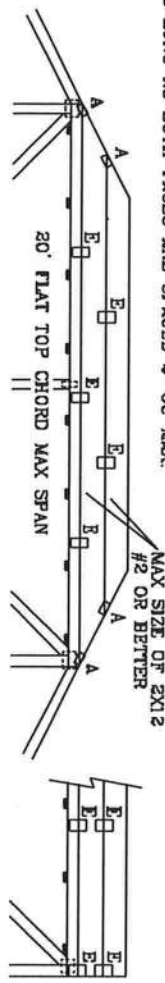
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, PBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

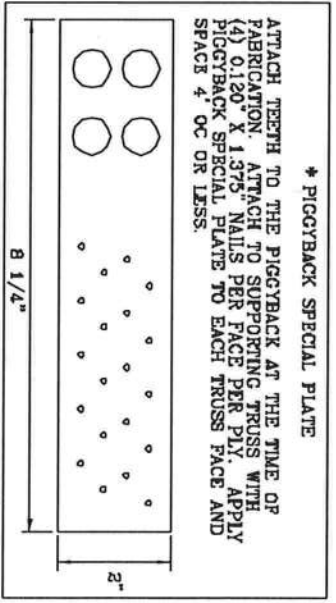


*ATTACH PIGGYBACK WITH 3X8 TRUSS OR ALPINE PIGGYBACK SPECIAL PLATE.

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

JOINT TYPE	SPANS UP TO		
	30'	34'	38'
A	2X4	2.5X4	2.5X4
B	4X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4
D	5X4	5X5	5X5
E	4X8 OR 3X8 TRUSS AT 4' OC, ROTATED VERTICALLY		

WEB LENGTH	WEB BRACING REQUIRED
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4' OC.
10' TO 14'	2x4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



ENGINEERS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST-1-00 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 260 BOWEN RD. DR., SUITE 200, MOUNTAIN, VA. 23179 AND VTRG COUNCIL THESE FUNCTIONS MUST BE PERFORMED BY QUALIFIED PERSONNEL. ADVISE THE DESIGNER OF ANY STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED 0120 CELLING.

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No. 34886
 STATE OF FLORIDA

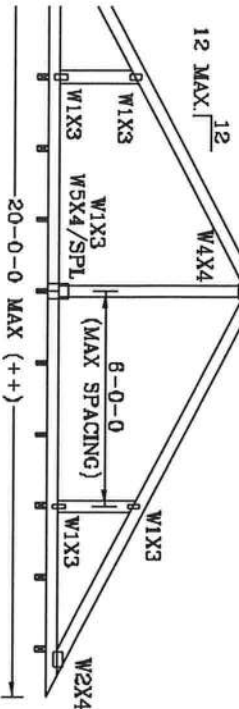
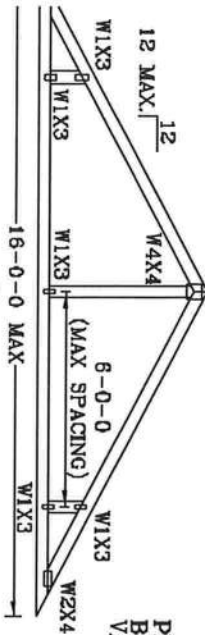
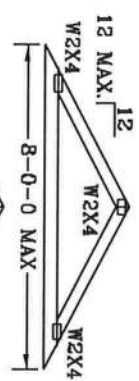
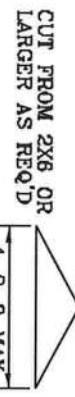
MAX LOADING	REF
55 PSF AT	PIGGYBACK
1.33 DUR. FAC.	DATE 09/12/07
50 PSF AT	DRG/MITEK STD PIGGY
1.25 DUR. FAC.	-ENG JL
47 PSF AT	
1.15 DUR. FAC.	
SPACING 24.0"	

THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

VALLEY TRUSS DETAIL

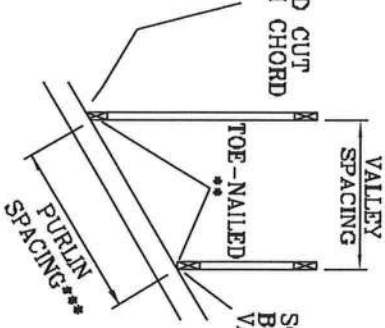
TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
 BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
 WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
 ** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
 (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
 FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
 ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
 BUILDING. EXP. C. RESIDENTIAL, WIND TC DL-6 PSF.

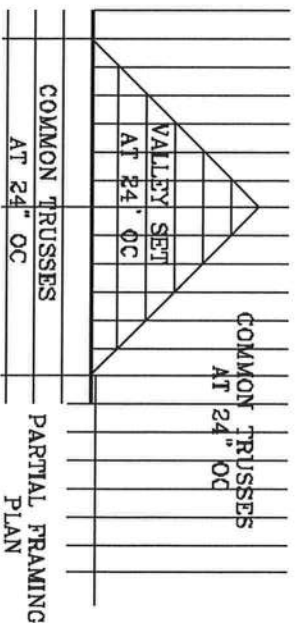
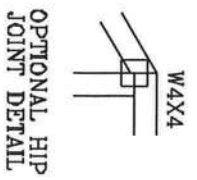
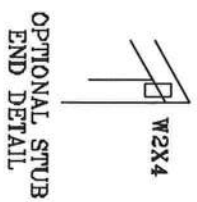


SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
 ** LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
 BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SQUARE CUT BOTTOM CHORD VALLEY



THIS DRAWING REPLACES DRAWING A105

REWARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICE BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS AND ROOFING INSTITUTE, 1000 N. 17TH ST., SUITE 200, FORT WORTH, TX 76102. THE TRUSS AND ROOFING INSTITUTE IS AN AFFILIATE OF THE AMERICAN WOOD PRESERVATION SOCIETY. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BRIDG CEILING.

JULIUS LEE'S
 CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
 DELRAY BEACH, FL 33444-6161

No. 34868
 STATE OF FLORIDA

TC LL	20	20	PSF	RFP	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG VAL/TRUSS	1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DUR/FAC.	1.25	1.25			
SPACING	24"				

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

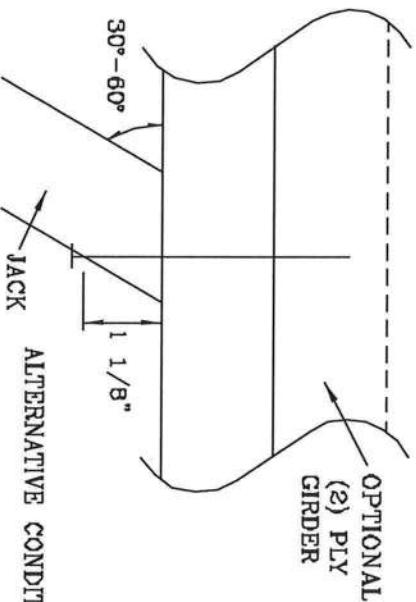
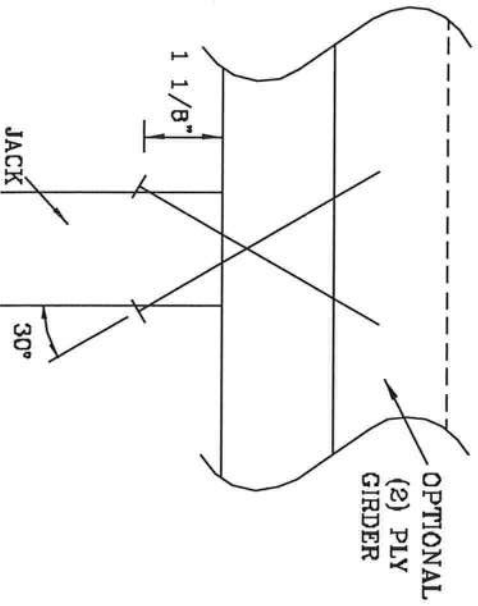
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	298#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICE GUIDELINES FOR TRUSS BRACING. CONTACT THE TRUSS MANUFACTURER FOR BRACING REQUIREMENTS. TRUSS MANUFACTURER: TRUSS COMPANY OF AMERICA, 6500 ENTERPRISE LN, MARIETTA, VA 20279 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEF'S
CONS. ENGINEERS P.A.

1405 SW 4th AVENUE
DELMAR BEACH, FL 33441-2161

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		

No. 34889

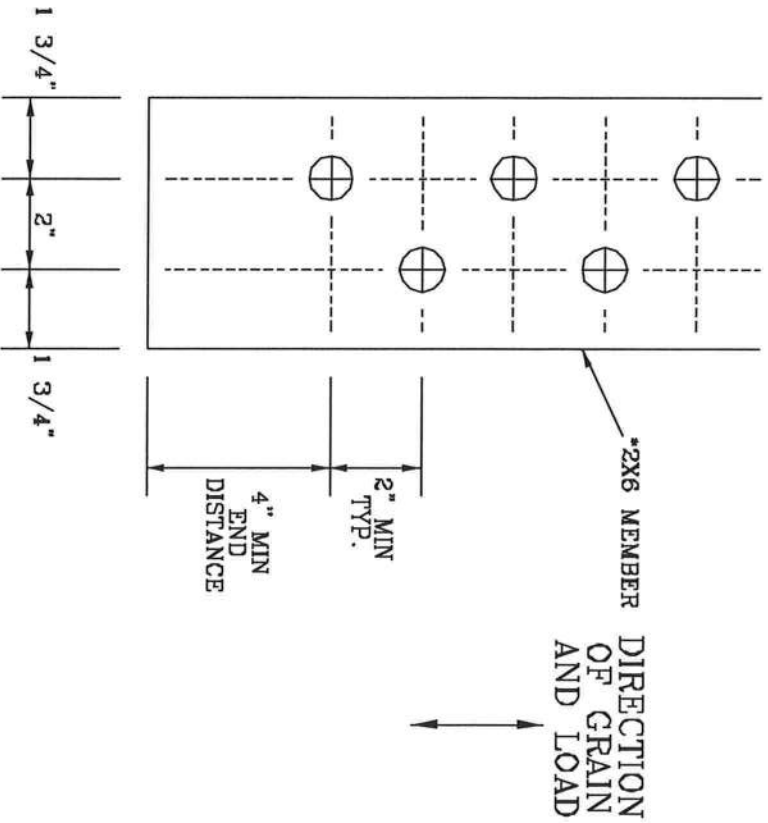
STATE OF FLORIDA

DUR. FAC. 1.00
SPACING

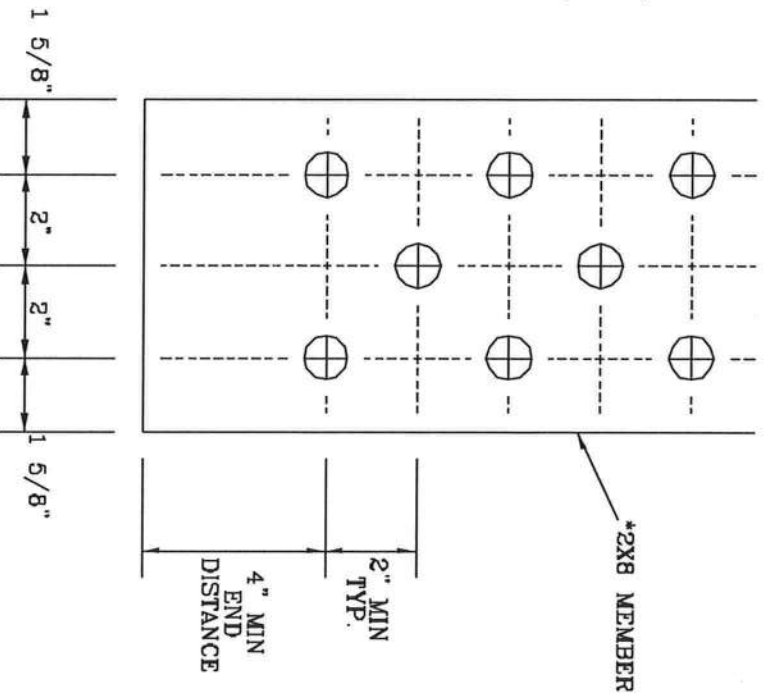
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
 BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
 WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016

UNLESS OTHERWISE SPECIFIED, TRUSSES REQUIRE CUTTING EDGE CASE. IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO PART 1-03 (BUILDING COMPONENT SAFETY INFORMATION, SPILLING AND LEAKAGE) AND PART 1-04 (WOOD JOINTS AND CONNECTIONS) OF THE ALPINE DESIGN. REFER TO PART 1-03 (BUILDING COMPONENT SAFETY INFORMATION, SPILLING AND LEAKAGE) AND PART 1-04 (WOOD JOINTS AND CONNECTIONS) OF THE ALPINE DESIGN. UNLESS OTHERWISE INDICATED, THE DESIGN SHALL HAVE PROTECTIVE TREATMENT APPLIED TO ALL EXPOSED SURFACES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1435 SW 4TH AVENUE
 DEERBERRY BLDG., FL 33444-2161

No. 34869
 STATE OF FLORIDA

TRC LL	PSF	REF	BOLT	SPACING
BC DL	PSF	DATE	11/26/03	
BC LL	PSF	DRWG	CNBOLTSPI103	
TOT. LD.	PSF	-ENG	JL	
DUR. FAC.				
SPACING				

TRULOX CONNECTION DETAIL

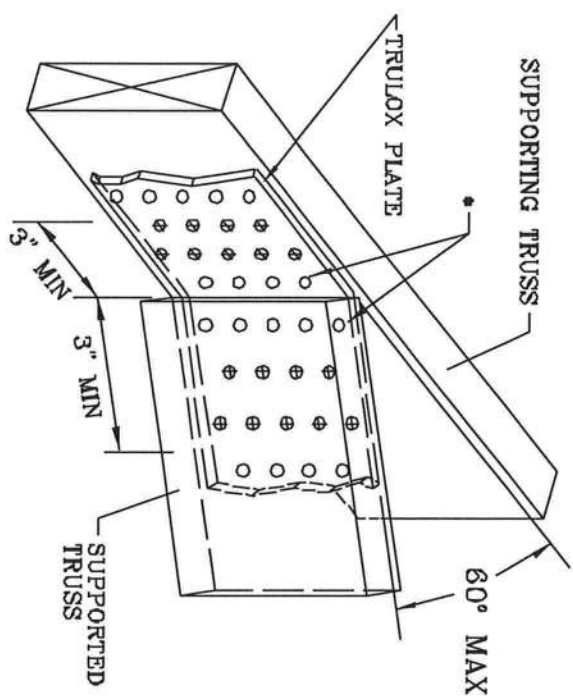
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

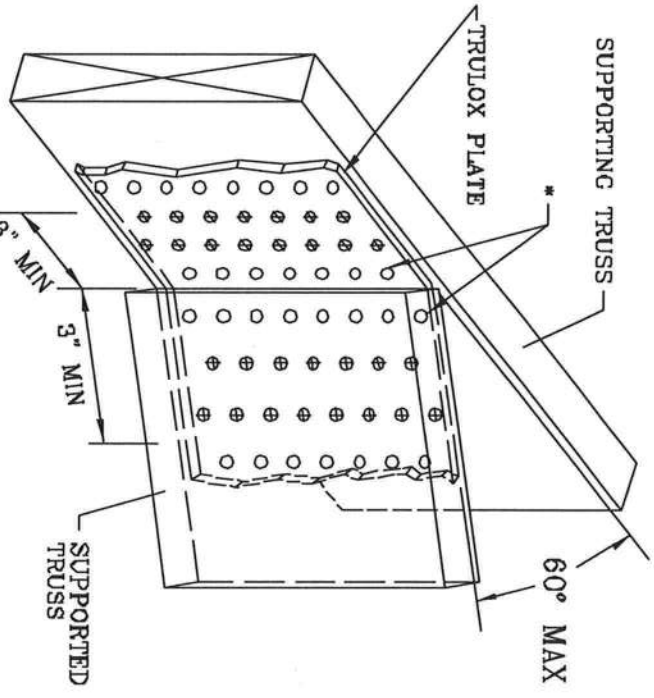
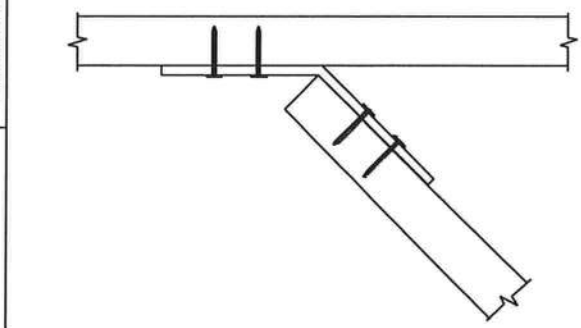
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350 #
5X6	15	990 #



MINIMUM 5X6 TRULOX PLATE

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE TRUSS MANUFACTURER'S INSTRUCTIONS FOR THE PROPER USE OF TRUSS CHORDS. PLATE INSTITUTE, 260 DUNNWOOD DR., SUITE 200, MANASSAS, VA 20108. TRUSS CHORDS OF AMERICA, 6300 ENTERPRISE LN, WATSON, VI 20759 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

THIS DRAWING REPLACES DRAWINGS 1.158.989 1.158.989/R
1.154.944 1.152.217 1.152.017 1.159.154 & 1.151.524

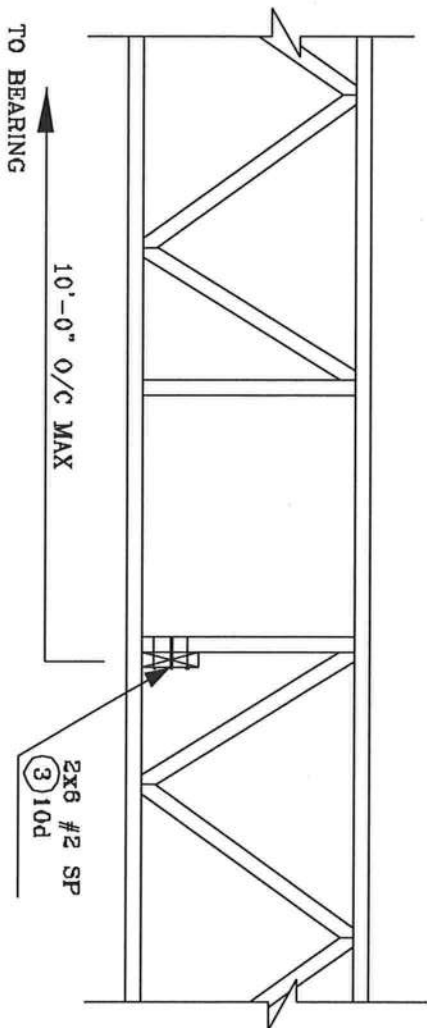
JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SR 4th AVENUE
DELRAY BEACH, FL 33444-2101

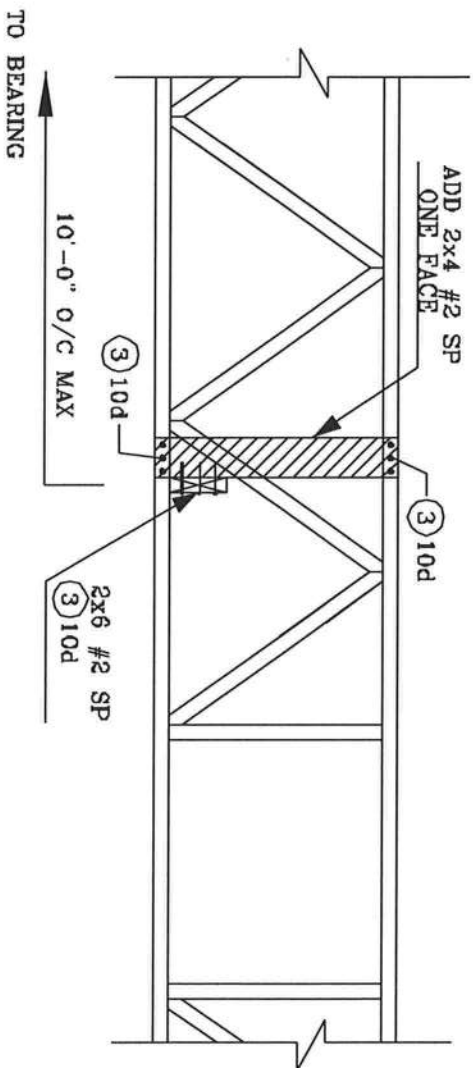
REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

No: 34869
STATE OF FLORIDA

**STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS**

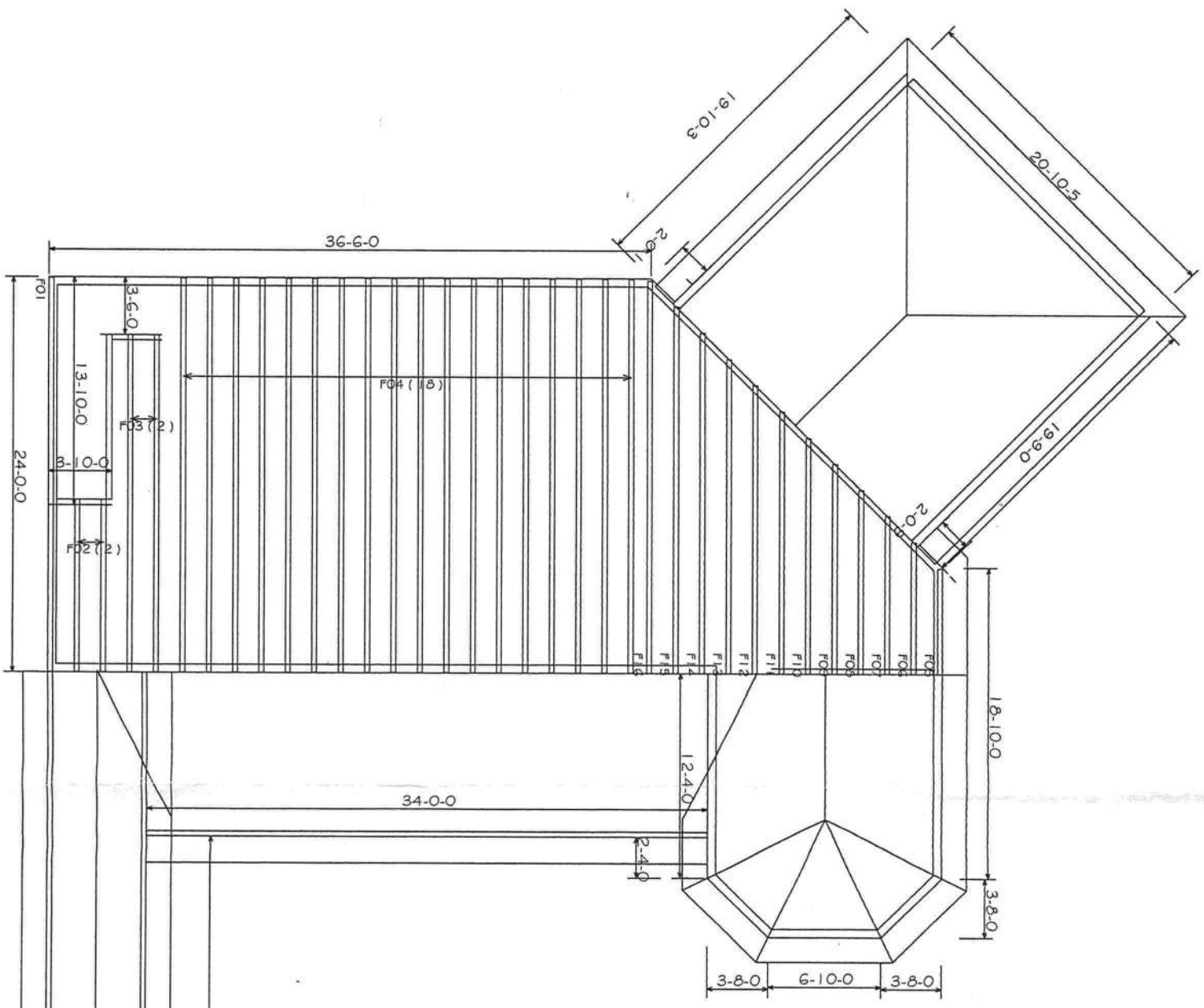


**ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP**



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4TH AVENUE
DEERFLAT BRANCH, FL. 32444-2761

No: 34869
STATE OF FLORIDA



TRUSS INFORMATION:
 FLOOR DEPTH: 18"
 FLOOR SPACING: 19.2" O/C
 HANGER LIST:
 NONE
 BEAM LIST:
 BY OTHERS
 NOTE: STAIR WALLS ARE USED FOR BEARING

BEARING HEIGHT SCHEDULE
 10' 0"

NOTES:
 1) REFER TO THE ARCHITECTURAL DRAWINGS FOR THE LOCATION OF THE TRUSSES AND BEAMS.
 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE CORRECTLY BRACED OR REFER TO DETAIL FOR ATTACHMENT BRACKETS.
 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BALDER.
 4) ALL TRUSSES ARE DESIGNED FOR 2' O.C. MAXIMUM SPACING UNLESS OTHERWISE NOTED.
 5) ALL WALLS SHOWN ON PLUMBING DRAWINGS ARE TO BE CONFORMANT WITH THE 10' BEAM O.C.
 6) 5/16" TRUSSES MUST BE INSTALLED WITH THE TOP BEAM O.C.
 7) ALL 800# TRUSS HANGERS TO BE SHIMMED WITH 1/2" THICK SHIMS.
 8) ALL TRUSSES TO BE INSTALLED WITH THE TOP BEAM O.C.
 9) BEARING HEIGHTS TO BE VERIFIED BY BALDER.

SHOP DRAWING APPROVAL
 THIS DRAWING IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND JOISTS. ALL TRUSSES AND JOISTS, OR OTHER TRUSS COMPONENTS, SHALL BE MANUFACTURED AND ASSEMBLED IN ACCORDANCE WITH THE DRAWING. THE FABRICATOR SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS TO BE MET PRIOR TO INSTALLATION. RETURN DRAWINGS TO 100.

Builders FirstSource
 Bunnell
 PHONE: 904-437-3349 FAX: 904-437-3984
 Jacksonville
 PHONE: 904-772-8100 FAX: 904-772-7873
 Lake City
 PHONE: 386-795-6844 FAX: 386-795-7973
 Sanford
 PHONE: 407-322-0094 FAX: 407-322-9933

WORK ADDRESS:
 LAKE CITY, FL

DATE: 1/31/08
 DRAWN BY: JP
 CHECKED BY: NIS
 SCALE: 1/2" = 1'-0"



Project Information for: L266887

... Address: 300 Southwest Sweetbryer Drive
 ... Lake City, FL
 ... County: Columbia
 Truss Count: 33
 Design Program: MiTek 20/20 6.3
 Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: Wind:

Roof (psf): 50.0 Wind Standard: ASCE 7-02 Wind Exposure: B

Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Engineer of Record: Unknown at time of Seal Date

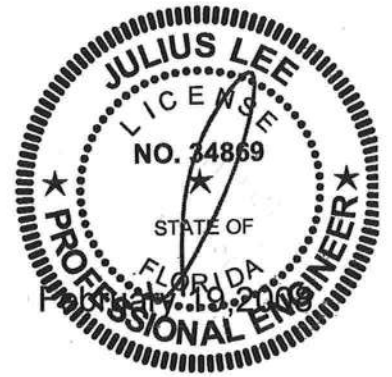
Address: Unknown at time of Seal Date

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.



No.	Drwg. #	Truss ID	Seal Date	No.	Drwg. #	Truss ID	Seal Date
1	J1936244	CJ1	2/19/08	29	J1936272	T11	2/19/08
2	J1936245	CJ3	2/19/08	30	J1936273	T12	2/19/08
3	J1936246	CJ5	2/19/08	31	J1936274	T12G	2/19/08
4	J1936247	EJ2	2/19/08	32	J1936275	T13	2/19/08
5	J1936248	EJ3	2/19/08	33	J1936276	T14	2/19/08
6	J1936249	EJ7	2/19/08				
7	J1936250	HJ2	2/19/08				
8	J1936251	HJ9	2/19/08				
9	J1936252	T01	2/19/08				
10	J1936253	T01A	2/19/08				
11	J1936254	T01G	2/19/08				
12	J1936255	T02	2/19/08				
13	J1936256	T03	2/19/08				
14	J1936257	T04	2/19/08				
15	J1936258	T04A	2/19/08				
16	J1936259	T04B	2/19/08				
17	J1936260	T04C	2/19/08				
18	J1936261	T04D	2/19/08				
19	J1936262	T04E	2/19/08				
20	J1936263	T04F	2/19/08				
21	J1936264	T04G	2/19/08				
22	J1936265	T04H	2/19/08				
23	J1936266	T05	2/19/08				
24	J1936267	T06	2/19/08				
25	J1936268	T07	2/19/08				
26	J1936269	T08	2/19/08				
27	J1936270	T09	2/19/08				
28	J1936271	T10	2/19/08				

Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936244
L266887	CJ1	JACK	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:08 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
 Truss Design Engineer
 Florida PE No. 34869
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 20

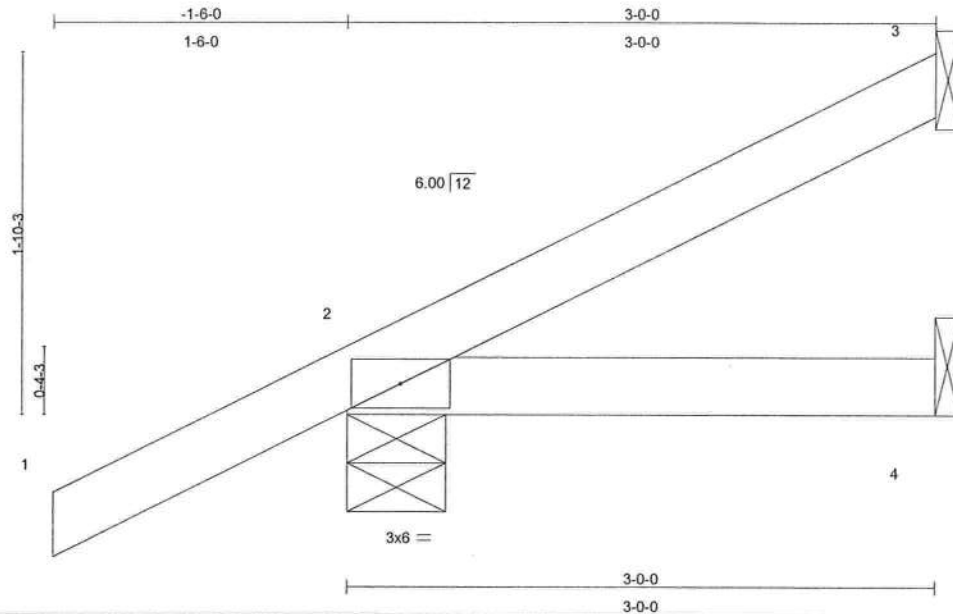
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936245
L266887	CJ3	JACK	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:09 2008 Page 1



Scale = 1:11.4

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.20	Vert(LL)	-0.00 2-4	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	-0.00 2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00 3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=54/Mechanical, 2=270/0-6-0, 4=13/Mechanical
 Max Horz 2=115(load case 6)
 Max Uplift 3=-33(load case 7), 2=-160(load case 6)
 Max Grav 3=54(load case 1), 2=270(load case 1), 4=40(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-66/17
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 160 lb uplift at joint 2.
- Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida PE No. 34888
 1400 Coastal Bay Blvd.
 Boynton Beach, FL 33435

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936245
L266887	CJ3	JACK	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:09 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

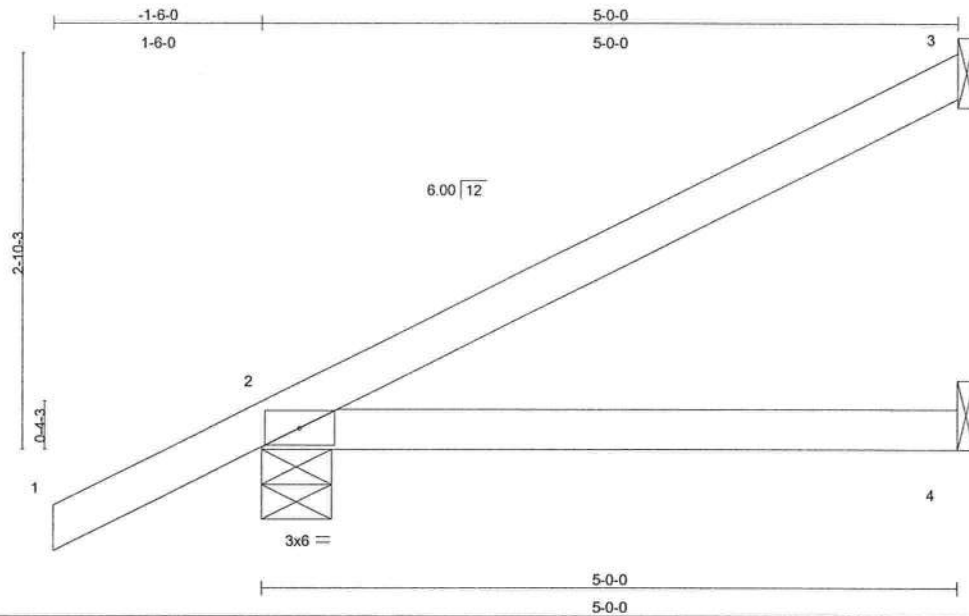
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936246
L266887	CJ5	JACK	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:09 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.26	Vert(LL)	-0.03 2-4	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.16	Vert(TL)	-0.04 2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00 3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=141/Mechanical, 2=333/0-6-0, 4=23/Mechanical
Max Horz 2=162(load case 6)
Max Uplift 3=-97(load case 6), 2=-164(load case 6)
Max Grav 3=141(load case 1), 2=333(load case 1), 4=70(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-95/51
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 3 and 164 lb uplift at joint 2.
- Continued on page 2

Julius Lee
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Florida PE No. 34869
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February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY
L266887	CJ5	JACK	4	1	J1936246 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:09 2008 Page 2

LOAD CASE(S) Standard

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Florida PE No. 34859
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

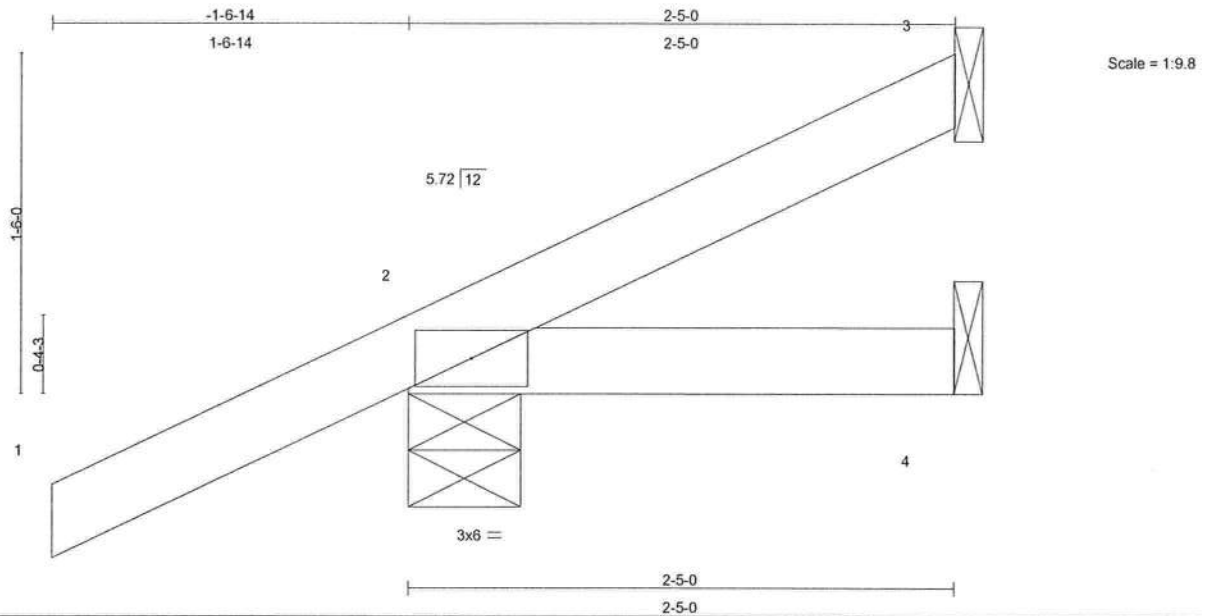
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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936247
L266887	EJ2	MONO TRUSS	2	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.00 2-4	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.03	Vert(TL)	-0.00 2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00 3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=18/Mechanical, 2=267/0-6-0, 4=11/Mechanical
Max Horz 2=99(load case 6)
Max Uplift 3=-19(load case 7), 2=-173(load case 6)
Max Grav 3=18(load case 1), 2=267(load case 1), 4=32(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-62/4
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.12

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 3 and 173 lb uplift at joint 2.
- Continued on page 2

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Florida PE No. 34889
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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY
L266887	EJ2	MONO TRUSS	2	1	J1936247 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:10 2008 Page 2

LOAD CASE(S) Standard

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1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

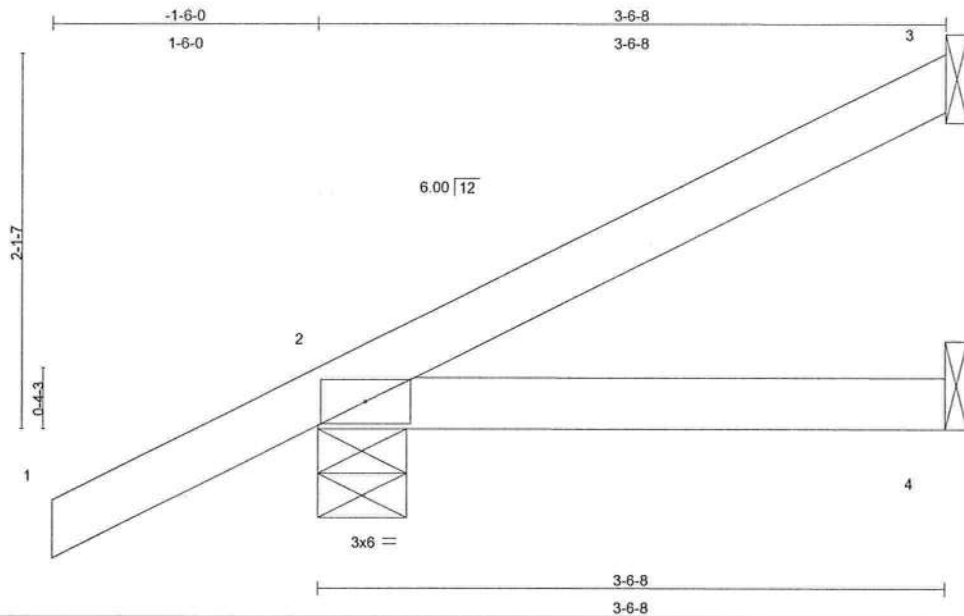
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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936248
L266887	EJ3	MONO TRUSS	3	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.20	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.07	Vert(TL)	-0.01	2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 14 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-6-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=80/Mechanical, 2=285/0-6-0, 4=16/Mechanical
 Max Horz 2=128(load case 6)
 Max Uplift 3=-52(load case 6), 2=-159(load case 6)
 Max Grav 3=80(load case 1), 2=285(load case 1), 4=48(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-71/28
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 3 and 159 lb uplift at joint 2.
- Continued on page 2

Julius Lee
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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY
L266887	EJ3	MONO TRUSS	3	1	J1936248 Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:11 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 3-3885
1459 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

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Job L266887	Truss EJ7	Truss Type MONO TRUSS	Qty 5	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936249
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6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:11 2008 Page 1

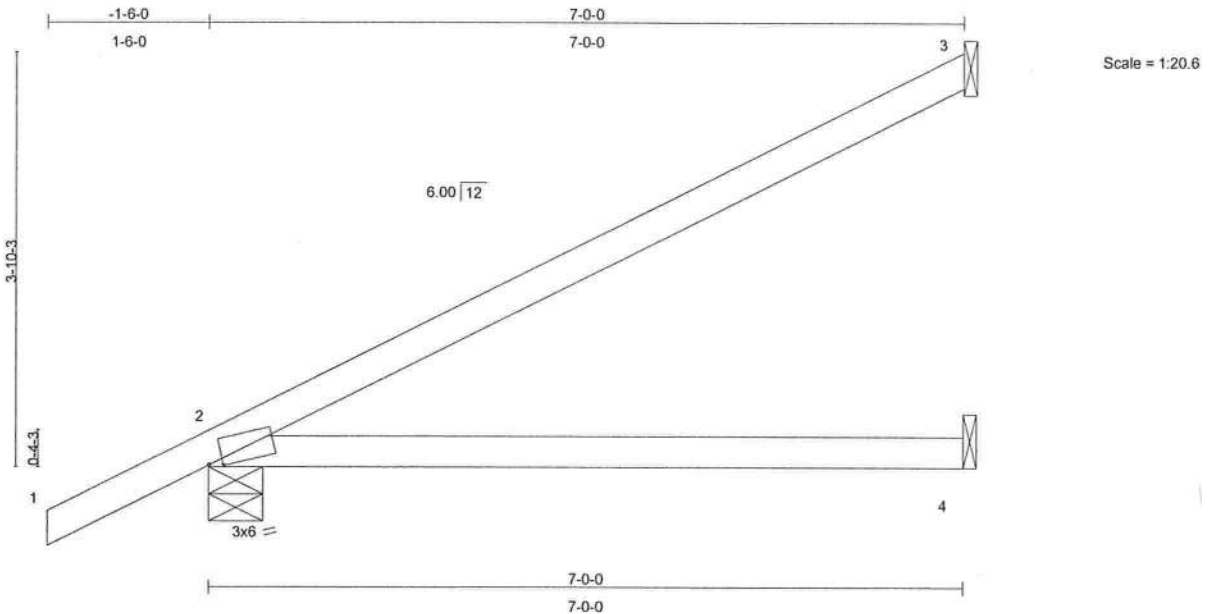


Plate Offsets (X,Y): [2:0-1-9,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.52	Vert(LL)	0.10 2-4	>814	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	-0.19 2-4	>430	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00 3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 25 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS (lb/size) 3=199/Mechanical, 2=406/0-6-0, 4=52/Mechanical
Max Horz 2=149(load case 6)
Max Uplift 3=-87(load case 6), 2=-115(load case 6)
Max Grav 3=199(load case 1), 2=406(load case 1), 4=98(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-125/71
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.84

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 115 lb uplift at joint 2.

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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936249
L266887	EJ7	MONO TRUSS	5	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:11 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
 Truss Design Engineer
 Florida PE No. 3-1889
 1300 Coastal Bay Blvd
 Boynton Beach, FL 33436

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

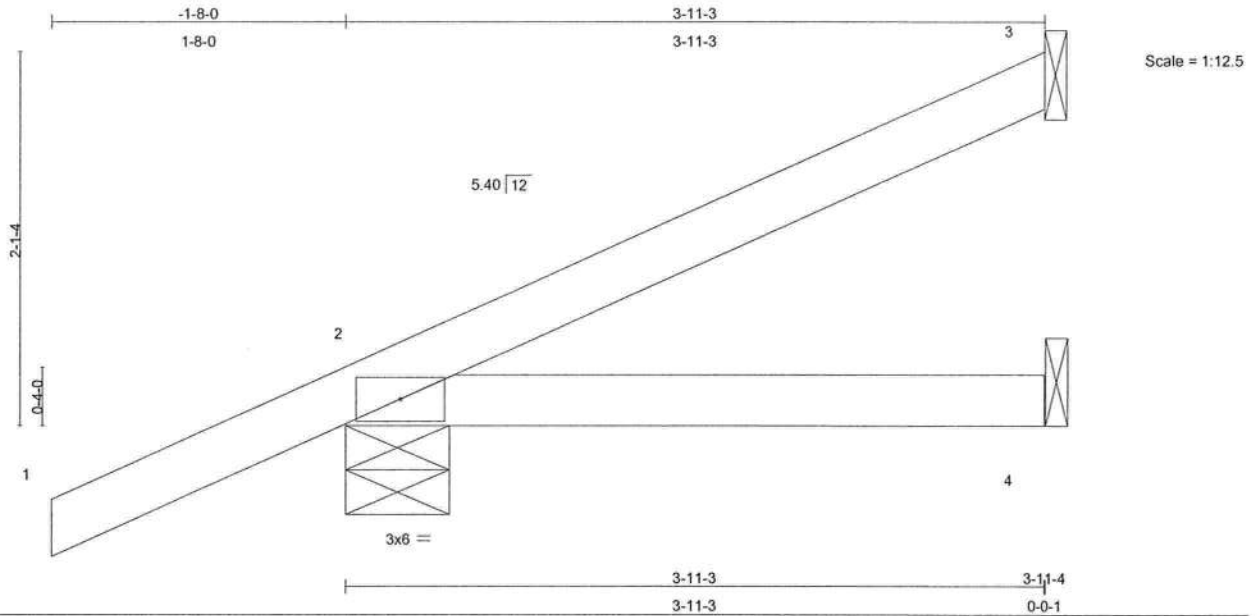
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Job L266887	Truss HJ2	Truss Type JACK	Qty 2	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936250
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6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:12 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase	1.25	TC 0.29	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
								Weight: 15 lb		

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=48/Mechanical, 2=228/0-7-0, 4=12/Mechanical
Max Horz 2=82(load case 5)
Max Uplift 3=-18(load case 5), 2=-144(load case 5)
Max Grav 3=48(load case 1), 2=228(load case 1), 4=48(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-38/11
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.10

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 - 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 3 and 144 lb uplift at joint 2.
 - 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- Continued on page 2

Julius Lee
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Florida PE No. 34888
1100 Coastal Bay Blvd
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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936250
L266887	HJ2	JACK	2	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:12 2008 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-70

Trapezoidal Loads (plf)

Vert: 2=-5(F=32, B=32)-to-3=-69(F=1, B=1), 2=0(F=5, B=5)-to-4=-10(F=0, B=0)

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Boynton Beach, FL 33435

February 19, 2008

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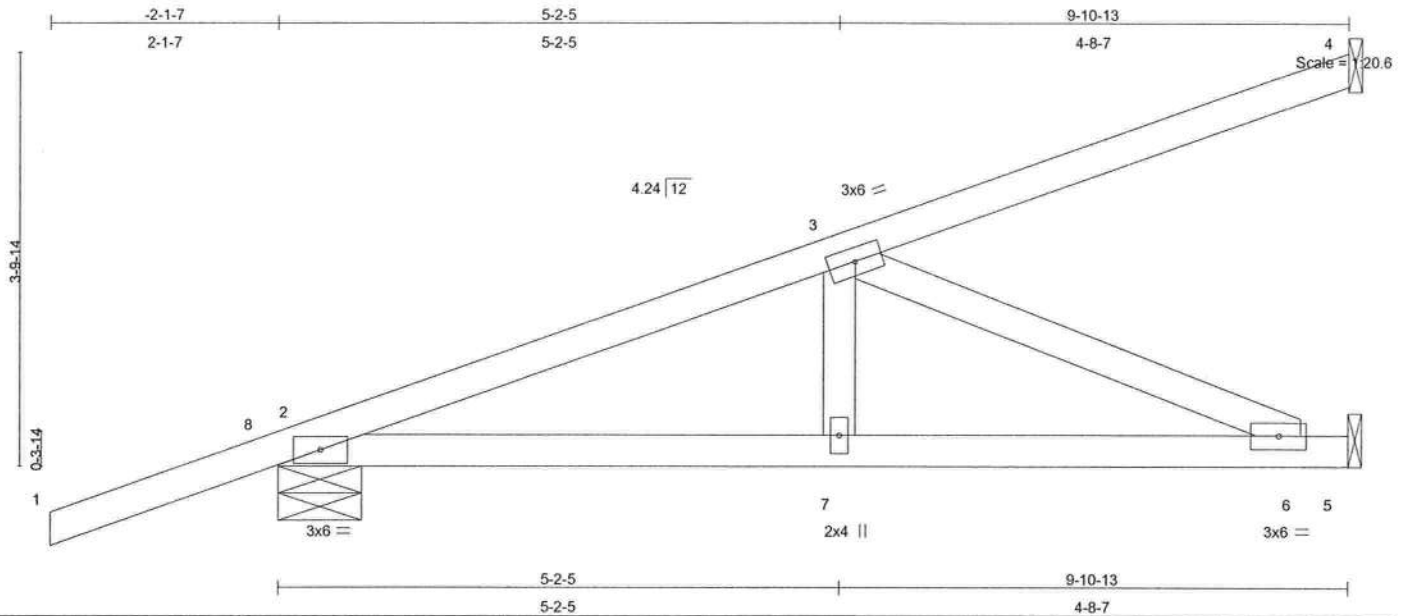
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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936251
L266887	HJ9	MONO TRUSS	2	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:12 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.57	Vert(LL)	0.04	6-7	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.38	Vert(TL)	-0.09	6-7	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.31	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 4=296/Mechanical, 2=506/0-9-4, 5=320/Mechanical
 Max Horz 2=250(load case 3)
 Max Uplift 4=-199(load case 3), 2=-221(load case 3), 5=-105(load case 3)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-8=0/42, 2-8=0/42, 2-3=-757/135, 3-4=-106/73
 BOT CHORD 2-7=-312/685, 6-7=-312/685, 5-6=0/0
 WEBS 3-7=0/195, 3-6=-747/340

JOINT STRESS INDEX

2 = 0.64, 3 = 0.20, 6 = 0.20 and 7 = 0.14

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 - *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 4, 221 lb uplift at joint 2 and 105 lb uplift at joint 5.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida PE No. 34868
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33436

February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936251
L266887	HJ9	MONO TRUSS	2	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:12 2008 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-8=-70

Trapezoidal Loads (plf)

Vert: 8=0(F=35, B=35)-to-4=-173(F=-52, B=-52), 2=-1(F=5, B=5)-to-5=-25(F=-7, B=-7)

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Boynton Beach, FL 33435

February 19, 2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936252
L266887	T01	SCISSOR	6	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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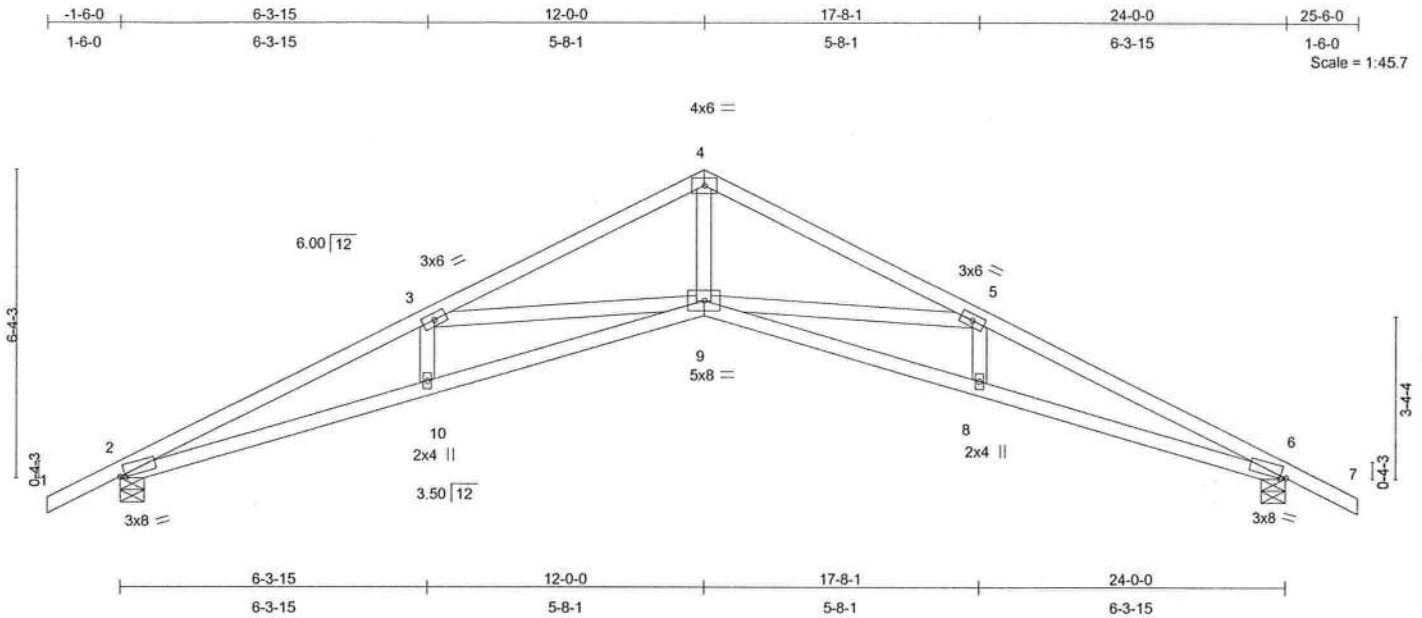


Plate Offsets (X,Y): [2:0-1-6,0-0-7], [6:0-1-6,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.43	Vert(LL)	0.26	9-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.65	Vert(TL)	-0.61	9-10	>459	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.54	Horz(TL)	0.49	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 106 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-13 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-2-14 oc bracing.

REACTIONS

(lb/size) 2=1063/0-6-0, 6=1063/0-6-0
 Max Horz 2=96(load case 6)
 Max Uplift 2=-240(load case 6), 6=-240(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/42, 2-3=-3319/1271, 3-4=-2442/864, 4-5=-2442/864, 5-6=-3319/1271, 6-7=0/42
 BOT CHORD 2-10=-1016/2959, 9-10=-1019/2963, 8-9=-1019/2963, 6-8=-1016/2959
 WEBS 3-10=0/173, 3-9=-808/498, 4-9=-518/1693, 5-9=-808/498, 5-8=0/173

JOINT STRESS INDEX

2 = 0.78, 3 = 0.39, 4 = 0.79, 5 = 0.39, 6 = 0.78, 8 = 0.33, 9 = 0.96 and 10 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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 Florida PE No. 34888
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February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936252
L266887	T01	SCISSOR	6	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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NOTES

- 5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2 and 240 lb uplift at joint 6.

LOAD CASE(S) Standard

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Job L266887	Truss T01A	Truss Type SCISSORS	Qty 2	Ply 3	MIKE & LINDA CADY Job Reference (optional)	J1936253
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Warning: This truss has not been designed to support any additional load from conventional framing.

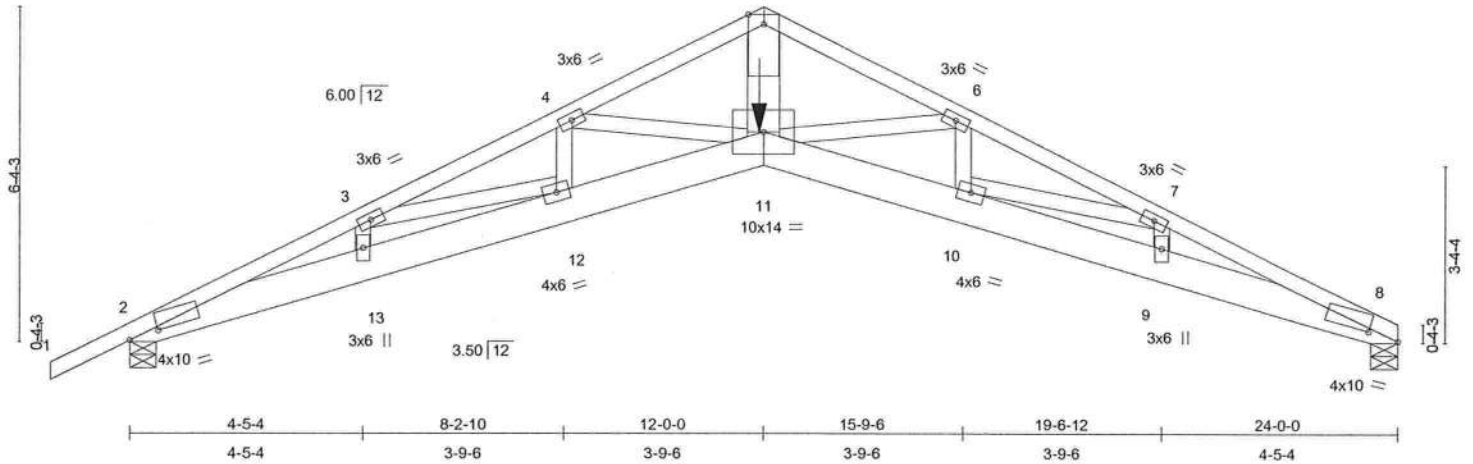


Plate Offsets (X,Y): [2:0-6-13,0-0-6], [8:0-6-13,0-0-6]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.59	Vert(LL)	-0.36	11	>781	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.51	Vert(TL)	-0.90	11	>312	240	MT20H	187/143
BCLL 10.0	Rep Stress Incr	NO	WB 0.43	Horz(TL)	0.61	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 460 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 8 SYP 2400F 2.0E
 WEBS 2 X 4 SYP No.2 *Except*
 4-11 2 X 4 SYP No.3, 5-11 2 X 8 SYP No.1D
 6-11 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-11-9 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=4132/0-6-0, 8=4000/0-6-0
 Max Horz 2=111(load case 5)
 Max Uplift 2=-920(load case 5), 8=-842(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-15149/3161, 3-4=-18339/3866, 4-5=-17704/3717, 5-6=-17704/3727,
 6-7=-18361/3796, 7-8=-15229/3159
 BOT CHORD 2-13=-2905/13722, 12-13=-2991/14108, 11-12=-3557/17002, 10-11=-3429/17023,
 9-10=-2915/14183, 8-9=-2836/13804
 WEBS 3-13=-1343/331, 3-12=-611/3045, 4-12=-5/165, 4-11=-608/198, 5-11=-3249/15498,
 6-11=-627/214, 6-10=-15/170, 7-10=-627/3021, 7-9=-1320/305

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JOINT STRESS INDEX

2 = 0.88, 3 = 0.49, 4 = 0.41, 5 = 1.00, 6 = 0.41, 7 = 0.49, 8 = 0.88, 9 = 0.16, 10 = 0.38, 11 = 0.89, 12 = 0.38 and 13 = 0.16

NOTES

- Distribute loads equally between all plies. Additional screws (+) are required to distribute the load equally among all plies.
- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 4 rows at 0-4-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.

February 19,2008

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Job L266887	Truss T01A	Truss Type SCISSORS	Qty 2	Ply 3	MIKE & LINDA CADY Job Reference (optional)	J1936253
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NOTES

- 3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 4) Unbalanced roof live loads have been considered for this design.
- 5) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 5.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 920 lb uplift at joint 2 and 842 lb uplift at joint 8.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-70, 5-8=-70, 2-11=-10, 8-11=-10
Concentrated Loads (lb)
Vert: 11=-6130(F)

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February 19,2008

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Job L266887	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936254
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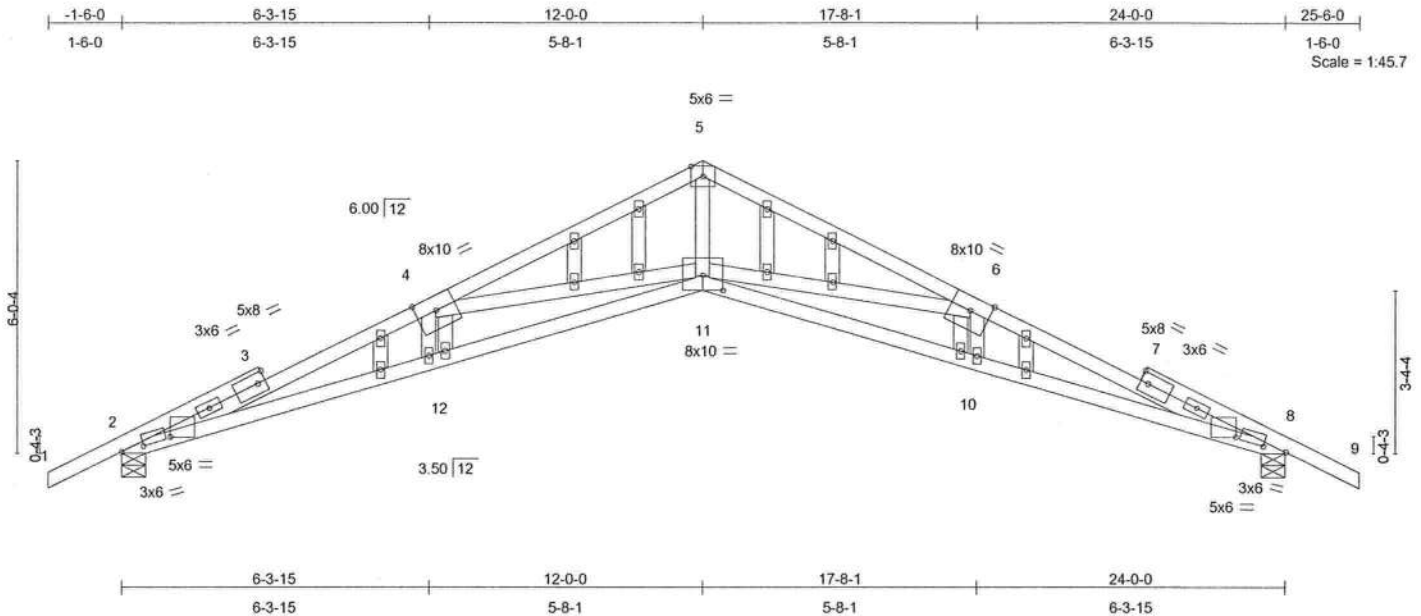


Plate Offsets (X,Y): [2:0-5-11,0-0-3], [2:1-0-2,0-3-11], [8:0-5-11,0-0-3], [8:1-0-2,0-3-11], [11:0-5-0,0-3-10]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.83	Vert(LL)	0.41 11-12	>688	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.78	Vert(TL)	-0.91 11-12	>308	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.80	Horz(TL)	0.69 8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 124 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D *Except*
 1-3 2 X 4 SYP No.2, 7-9 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.1D
 WEBS 2 X 4 SYP No.3
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-3-7 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing.

REACTIONS (lb/size)

2=1198/0-6-0, 8=1198/0-6-0
 Max Horz 2=106(load case 6)
 Max Uplift 2=-453(load case 6), 8=-453(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/48, 2-3=-4572/1784, 3-4=-4479/1791, 4-5=-3129/1127, 5-6=-3129/1127,
 6-7=-4479/1791, 7-8=-4572/1784, 8-9=0/48
 BOT CHORD 2-12=-1543/4195, 11-12=-1548/4205, 10-11=-1548/4205, 8-10=-1543/4195
 WEBS 4-12=0/153, 4-11=-1353/748, 5-11=-726/2233, 6-11=-1353/748, 6-10=0/153

JOINT STRESS INDEX

2 = 0.83, 2 = 0.95, 3 = 0.00, 3 = 0.50, 3 = 0.82, 4 = 0.30, 4 = 0.00, 5 = 0.76, 6 = 0.30, 6 = 0.00, 7 = 0.00, 7 = 0.82, 7 = 0.50, 8 = 0.83, 8 = 0.95, 10 = 0.33, 11 = 0.74, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33 and 26 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

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Continued on page 2

February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936254
L266887	T01G	GABLE	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 2 and 453 lb uplift at joint 8.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-80(F=-10), 5-9=-80(F=-10), 2-11=-10, 8-11=-10

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February 19,2008

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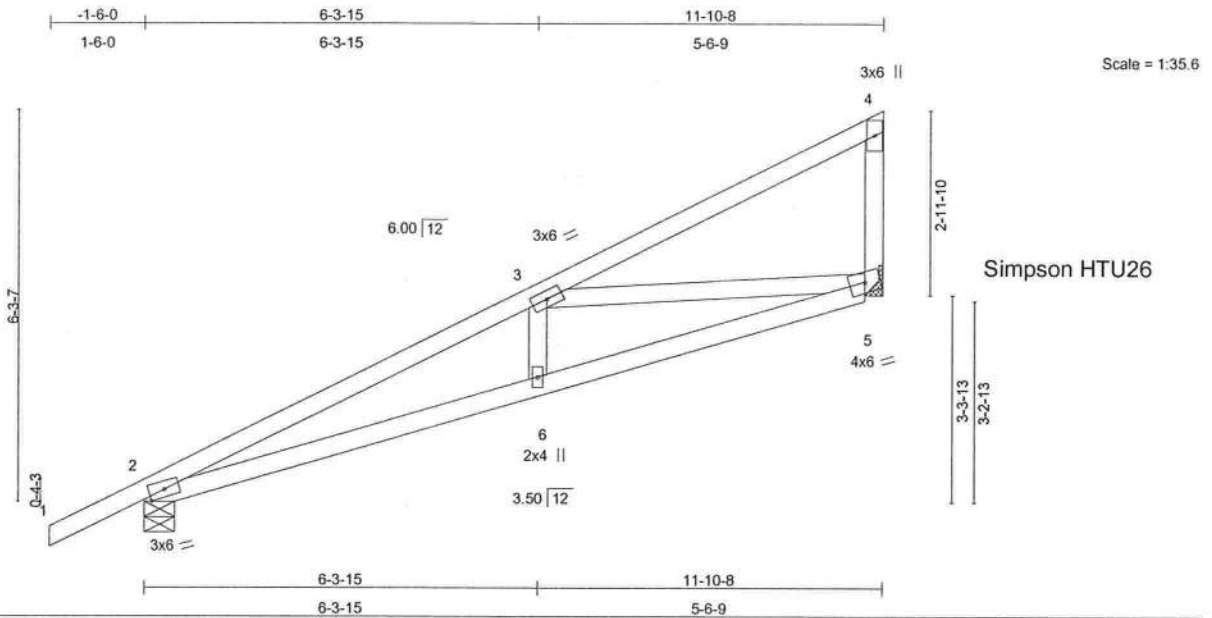
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Job L266887	Truss T02	Truss Type MONO SCISSOR	Qty 12	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936255
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase	1.25	TC 0.44	Vert(LL)	0.07	2-6	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.30	Vert(TL)	-0.12	2-6	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.48	Horz(TL)	0.02	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 54 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-9 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 7-3-14 oc bracing.

REACTIONS (lb/size) 2=591/0-6-0, 5=450/Mechanical
 Max Horz 2=224(load case 6)
 Max Uplift 2=-132(load case 6), 5=-145(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/42, 2-3=-1211/528, 3-4=-161/9, 4-5=-143/132
 BOT CHORD 2-6=-735/1031, 5-6=-738/1032
 WEBS 3-6=0/186, 3-5=-919/656

JOINT STRESS INDEX

2 = 0.41, 3 = 0.25, 4 = 0.35, 5 = 0.58 and 6 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

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Continued on page 2

February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936255
L266887	T02	MONO SCISSOR	12	1	Job Reference (optional)	

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 2 and 145 lb uplift at joint 5.

LOAD CASE(S) Standard

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February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

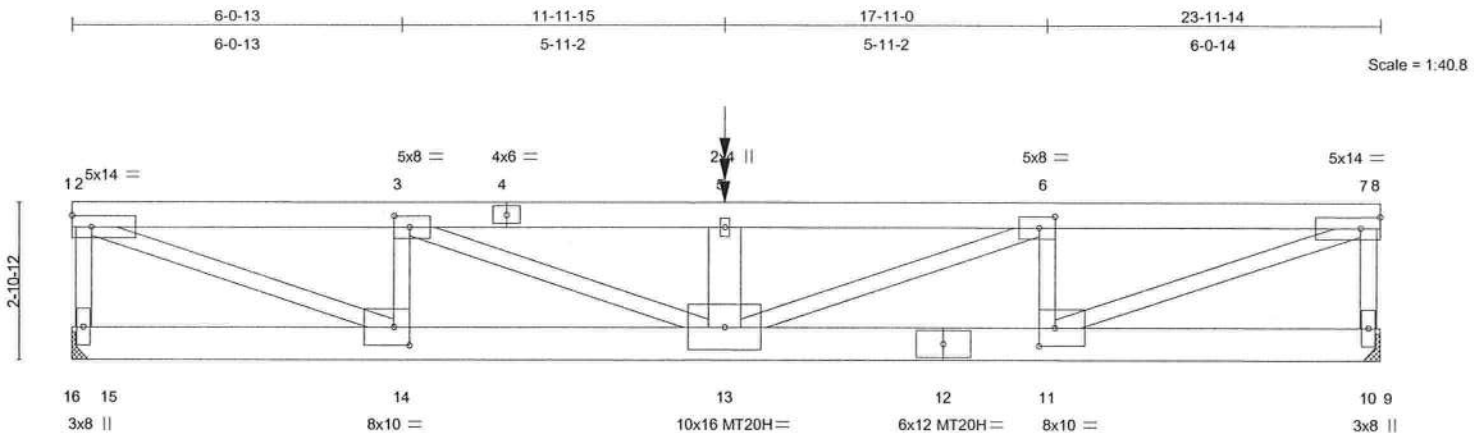
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Job L266887	Truss T03	Truss Type SPECIAL	Qty 1	Ply 2	MIKE & LINDA CADY Job Reference (optional)	J1936256
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Simpson HGUS28-2

Simpson HGUS28-2

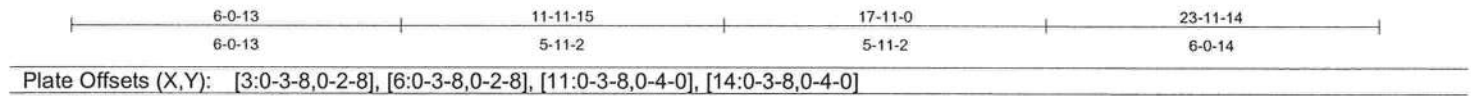


Plate Offsets (X,Y): [3:0-3-8,0-2-8], [6:0-3-8,0-2-8], [11:0-3-8,0-4-0], [14:0-3-8,0-4-0]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25		TC 0.79	Vert(LL)	-0.27	13	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase 1.25		BC 0.78	Vert(TL)	-0.68	13	>417	240	MT20H	187/143
BCLL 10.0	* Rep Stress Incr NO		WB 0.87	Horz(TL)	-0.06	15	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 362 lb

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 8 SYP No.1D
 WEBS 2 X 4 SYP No.3 *Except*
 7-10 2 X 4 SYP No.2, 2-15 2 X 4 SYP No.2
 7-11 2 X 4 SYP No.1D, 5-13 2 X 8 SYP No.1D
 2-14 2 X 4 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-15 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 10=6130/Mechanical, 15=6130/Mechanical
 Max Uplift 10=-1080(load case 4), 15=-1080(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 7-10=-5323/993, 1-2=0/0, 2-3=-11743/2080, 3-4=-16773/3032, 4-5=-16773/3032,
 5-6=-16773/3032, 6-7=-11743/2080, 7-8=0/0, 2-15=-5323/993
 BOT CHORD 15-16=0/0, 14-15=-109/586, 13-14=-2080/11743, 12-13=-2080/11743, 11-12=-2080/11743,
 10-11=-109/586, 9-10=0/0
 WEBS 7-11=-2125/12029, 6-11=-3064/674, 6-13=-1025/5415, 5-13=-2933/694, 3-13=-1025/5415,
 3-14=-3064/674, 2-14=-2125/12029

JOINT STRESS INDEX

2 = 0.83, 3 = 0.61, 4 = 0.90, 5 = 0.86, 6 = 0.61, 7 = 0.83, 10 = 0.83, 11 = 0.97, 12 = 0.79, 13 = 0.69, 14 = 0.97 and 15 = 0.83

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-4-0 oc.

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February 19,2008

Continued on page 2

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Job L266887	Truss T03	Truss Type SPECIAL	Qty 1	Ply 2	MIKE & LINDA CADY Job Reference (optional)	J1936256
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Builders FirstSource, Lake City, FL 32055

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NOTES

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BC DL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 4) Provide adequate drainage to prevent water ponding.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1080 lb uplift at joint 10 and 1080 lb uplift at joint 15.
- 9) Girder carries tie-in span(s): 11-10-8 from 0-0-0 to 23-11-14

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 - Uniform Loads (plf)
 - Vert: 9-16=-232(F=-222)
 - Concentrated Loads (lb)
 - Vert: 5=-2520(F)
 - Trapezoidal Loads (plf)
 - Vert: 1=-240(F=-210)-to-2=-236(F=-206), 2=-276(F=-206)-to-5=-70, 5=-70-to-7=-276(F=-206), 7=-236(F=-206)-to-8=-240(F=-210)

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February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

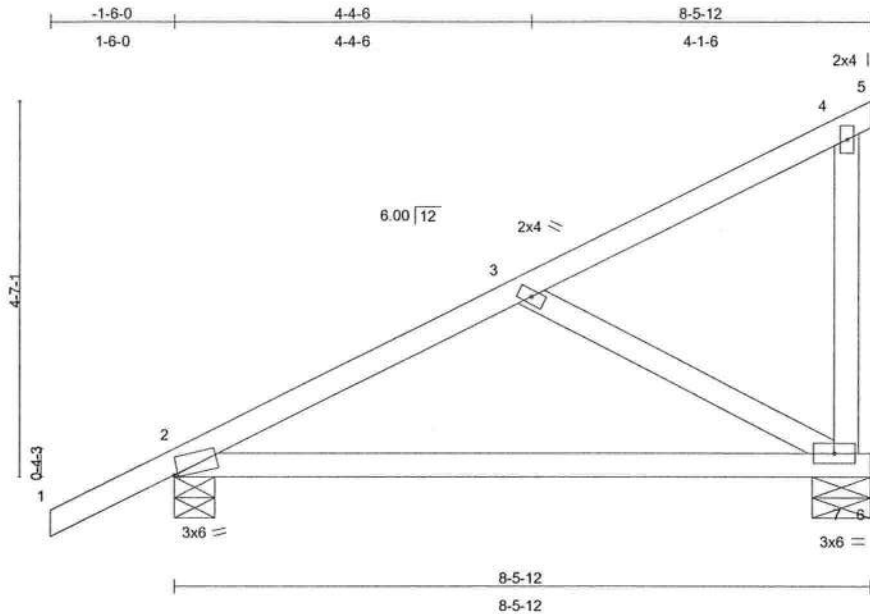
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Job L266887	Truss T04	Truss Type MONO TRUSS	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936257
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Builders FirstSource, Lake City, FL 32055

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Scale = 1:27.1

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.44	Vert(LL)	-0.08	2-7	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.19	Vert(TL)	-0.14	2-7	>661	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.11	Horz(TL)	0.00	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 42 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 7=316/0-8-7, 2=453/0-6-0
 Max Horz 2=172(load case 6)
 Max Uplift 7=-100(load case 6), 2=-117(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-402/103, 3-4=-94/30, 4-5=-4/0, 4-7=-125/121
 BOT CHORD 2-7=-269/300, 6-7=0/0
 WEBS 3-7=-313/299

JOINT STRESS INDEX

2 = 0.89, 3 = 0.16, 4 = 0.56 and 7 = 0.54

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 7 and 117 lb uplift at joint 2.

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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936257
L266887	T04	MONO TRUSS	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:18 2008 Page 2

LOAD CASE(S) Standard

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Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

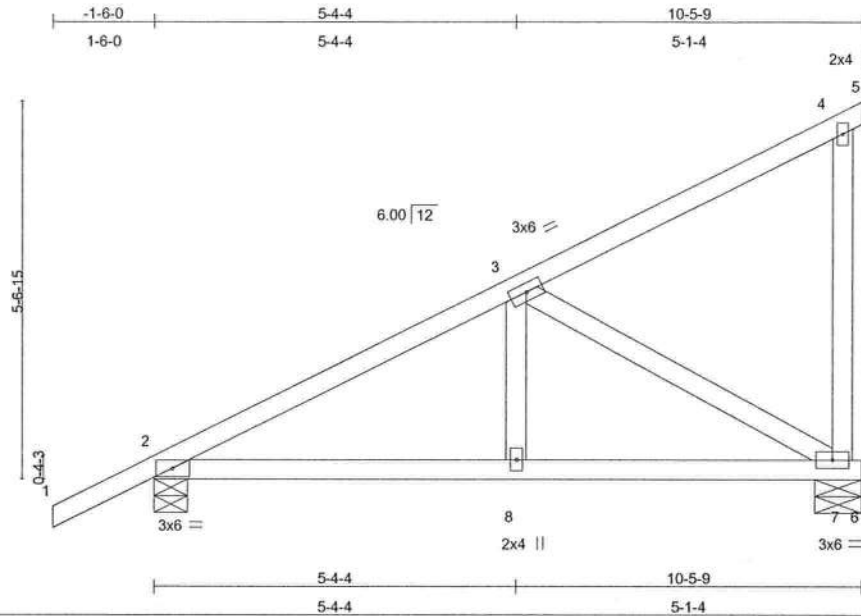
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Job L266887	Truss T04A	Truss Type MONO TRUSS	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936258
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Scale = 1:32.9

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase	1.25	TC 0.25	Vert(LL)	-0.01	2-8	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.15	Vert(TL)	-0.04	2-8	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.24	Horz(TL)	0.01	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 54 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 7=398/0-8-7, 2=530/0-6-0
 Max Horz 2=204(load case 6)
 Max Uplift 7=-128(load case 6), 2=-125(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-543/98, 3-4=-105/46, 4-5=-4/0, 4-7=-148/129
 BOT CHORD 2-8=-300/409, 7-8=-300/409, 6-7=0/0
 WEBS 3-8=0/167, 3-7=-449/328

JOINT STRESS INDEX

2 = 0.29, 3 = 0.16, 4 = 0.68, 7 = 0.17 and 8 = 0.12

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 7 and 125 lb uplift at joint 2.

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LOAD CASE(S) Standard

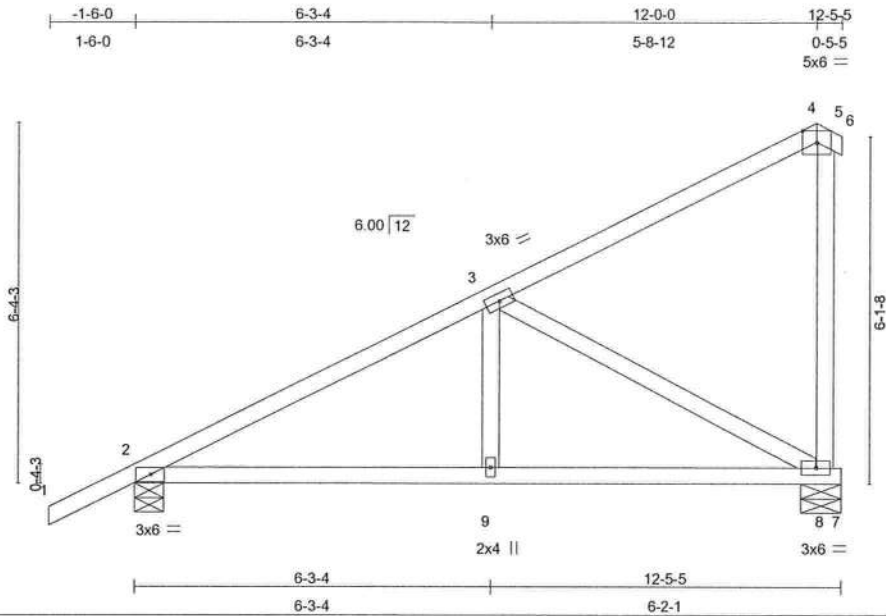
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Job L266887	Truss T04B	Truss Type COMMON	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936259
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6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:19 2008 Page 1



Scale = 1:39.1

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.38	Vert(LL)	-0.03	2-9	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.21	Vert(TL)	-0.07	2-9	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.43	Horz(TL)	0.01	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 64 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=607/0-6-0, 8=479/0-8-7
 Max Horz 2=226(load case 6)
 Max Uplift 2=-138(load case 6), 8=-144(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/46, 2-3=-674/134, 3-4=-129/50, 4-5=-97/82, 5-6=0/4, 5-8=-179/146
 BOT CHORD 2-9=-357/516, 8-9=-357/516, 7-8=0/0
 WEBS 3-9=0/203, 3-8=-560/386

JOINT STRESS INDEX

2 = 0.41, 3 = 0.18, 4 = 0.52, 5 = 0.00, 8 = 0.33 and 9 = 0.14

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) -1-6-0 to 12-5-5 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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Continued on page 2

February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936259
L266887	T04B	COMMON	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055

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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2 and 144 lb uplift at joint 8.

LOAD CASE(S) Standard

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February 19, 2008

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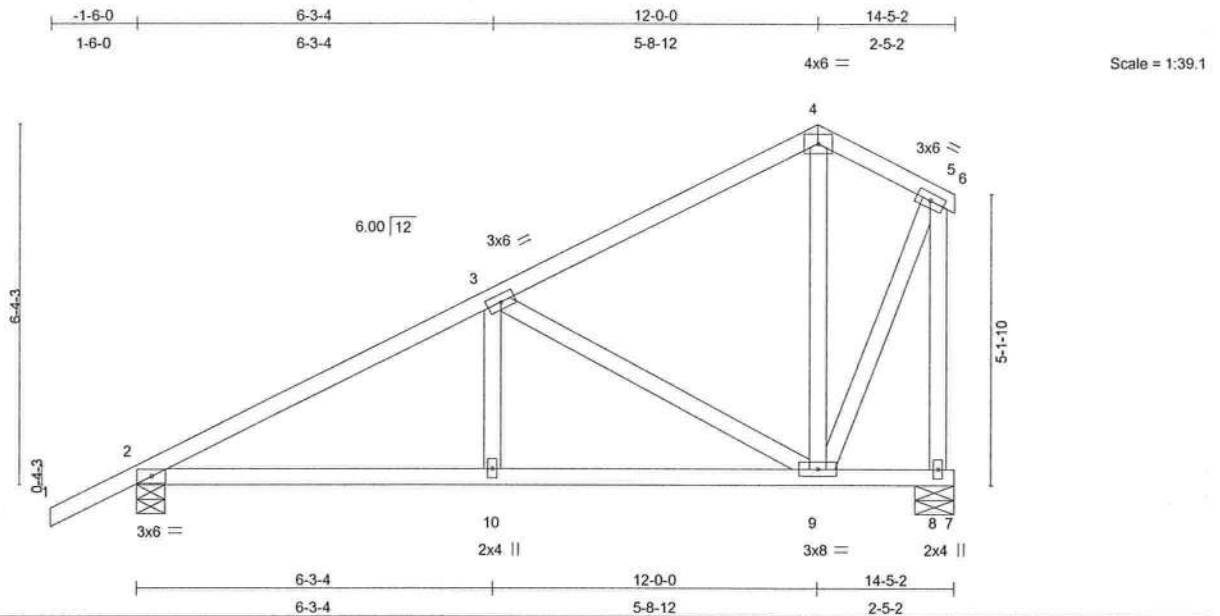
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Job L266887	Truss T04C	Truss Type COMMON	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936260
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.29	Vert(LL)	-0.03	2-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.23	Vert(TL)	-0.07	2-10	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.41	Horz(TL)	0.01	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(lb/size) 2=685/0-6-0, 8=559/0-8-7
 Max Horz 2=206(load case 6)
 Max Uplift 2=-167(load case 6), 8=-127(load case 6)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/46, 2-3=-839/272, 3-4=-306/114, 4-5=-209/131, 5-6=0/4, 5-8=-547/307
 BOT CHORD 2-10=-413/660, 9-10=-413/660, 8-9=-1/3, 7-8=0/0
 WEBS 3-10=0/199, 3-9=-542/350, 4-9=-136/112, 5-9=-271/472

JOINT STRESS INDEX

2 = 0.48, 3 = 0.17, 4 = 0.46, 5 = 0.39, 8 = 0.19, 9 = 0.55 and 10 = 0.14

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 2 and 127 lb uplift at joint 8.

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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936260
L266887	T04C	COMMON	1	1	Job Reference (optional)	

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6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:20 2008 Page 2

LOAD CASE(S) Standard

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February 19, 2008

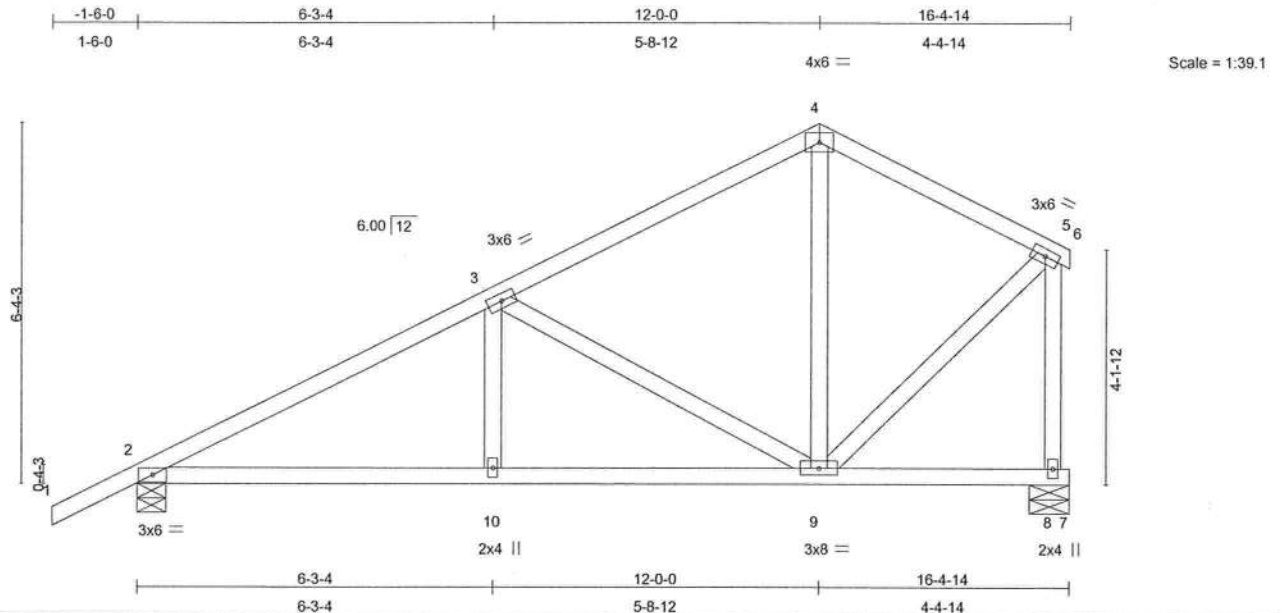
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Job L266887	Truss T04D	Truss Type COMMON	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936261
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.03	2-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.24	Vert(TL)	-0.08	2-10	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.40	Horz(TL)	0.02	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 90 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 9-5-5 oc bracing.

REACTIONS

(lb/size) 2=764/0-6-0, 8=640/0-8-7
 Max Horz 2=186(load case 6)
 Max Uplift 2=-189(load case 6), 8=-116(load case 6)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/46, 2-3=-1004/374, 3-4=-479/231, 4-5=-436/238, 5-6=0/4, 5-8=-614/318
 BOT CHORD 2-10=-445/806, 9-10=-445/806, 8-9=-10/20, 7-8=0/0
 WEBS 3-10=0/196, 3-9=-537/323, 4-9=-23/107, 5-9=-212/456

JOINT STRESS INDEX

2 = 0.53, 3 = 0.39, 4 = 0.50, 5 = 0.44, 8 = 0.33, 9 = 0.56 and 10 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 2 and 116 lb uplift at joint 8.
- Continued on page 2

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 Florida PE No. 34888
 1496 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job L266887	Truss T04D	Truss Type COMMON	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936261
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Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:21 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

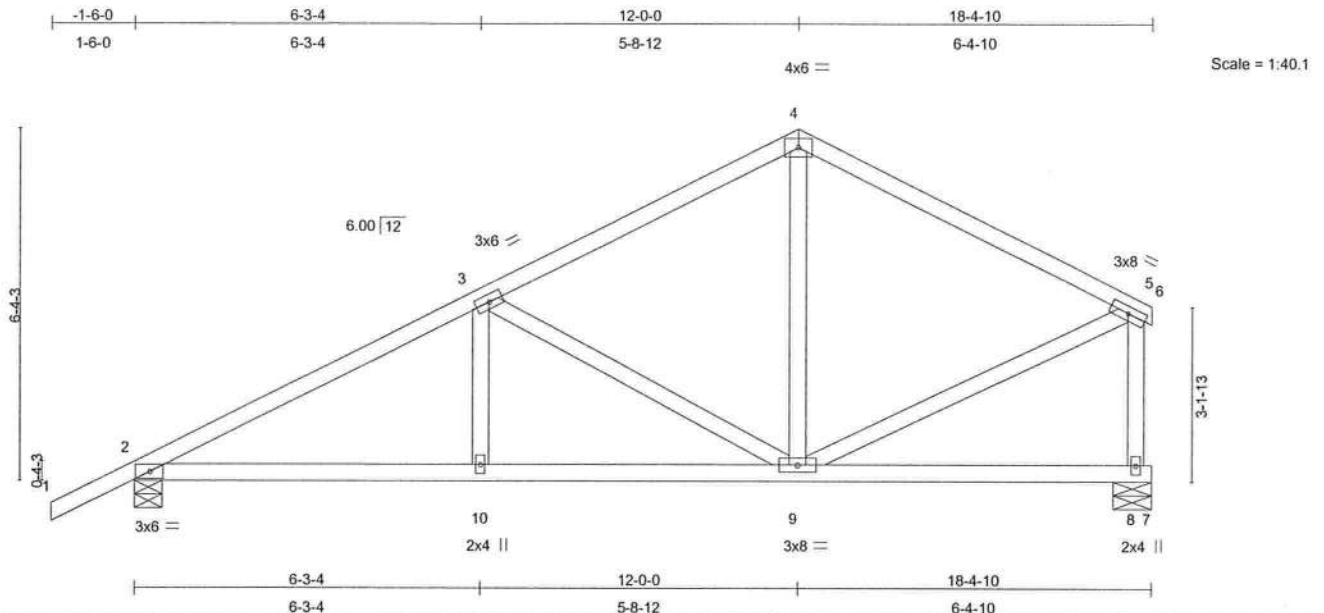
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Job L266887	Truss T04E	Truss Type COMMON	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936262
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.64	Vert(LL)	-0.03 2-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.27	Vert(TL)	-0.09 2-10	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.39	Horz(TL)	0.02 8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 97 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-2 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 9-2-0 oc bracing.

REACTIONS

(lb/size) 2=842/0-6-0, 8=720/0-8-7
 Max Horz 2=166(load case 6)
 Max Uplift 2=-207(load case 6), 8=-110(load case 6)

FORCES

(lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/46, 2-3=-1168/465, 3-4=-655/333, 4-5=-658/326, 5-6=0/4, 5-8=-680/340
 BOT CHORD 2-10=-471/949, 9-10=-471/949, 8-9=-43/76, 7-8=0/0
 WEBS 3-10=0/190, 3-9=-527/302, 4-9=-22/203, 5-9=-183/487

JOINT STRESS INDEX

2 = 0.59, 3 = 0.39, 4 = 0.68, 5 = 0.83, 8 = 0.77, 9 = 0.56 and 10 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 2 and 110 lb uplift at joint 8.
- Continued on page 2

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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936262
L266887	T04E	COMMON	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:21 2008 Page 2

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 34889
1409 Coastal Bay Blvd
Boynton Beach, FL 33436

February 19, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job L266887	Truss T04F	Truss Type SPECIAL	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936263
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Builders FirstSource, Lake City, FL 32055

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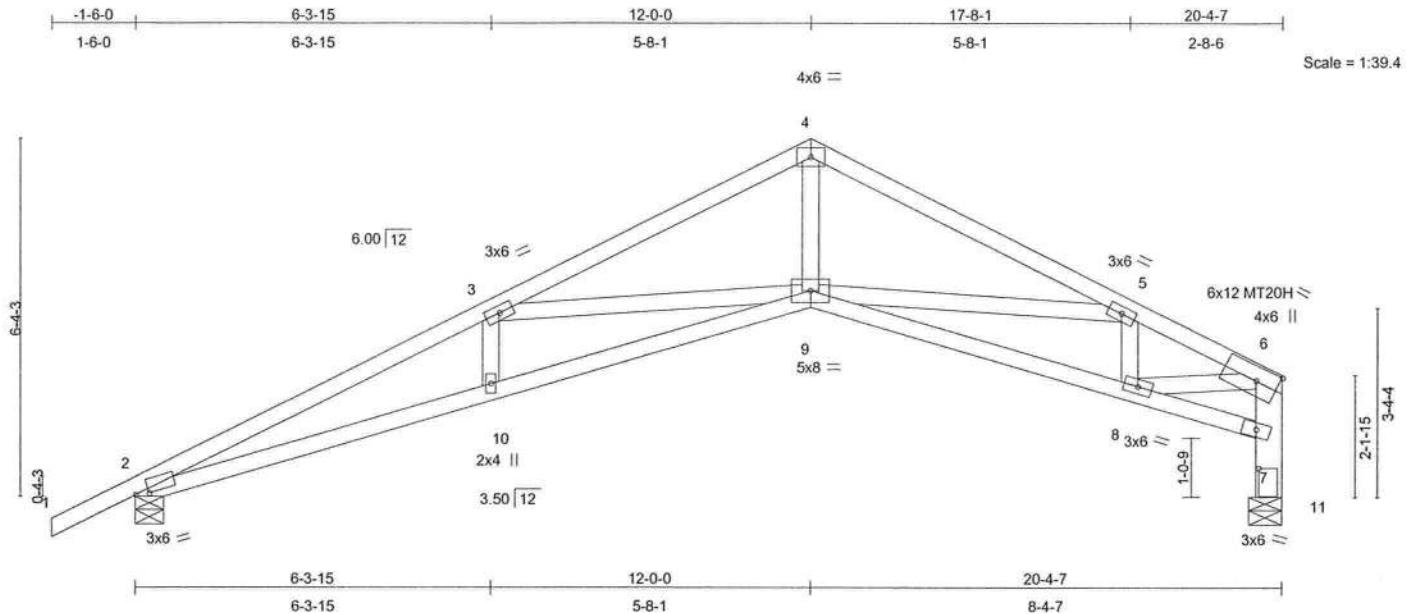


Plate Offsets (X,Y): [2:0-2-14,0-0-7], [4:5-6-3,7-11-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.36	Vert(LL)	0.17 9-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.55	Vert(TL)	-0.34 9-10	>699	240	MT20H	187/143
BCLL 10.0	* Rep Stress Incr	YES	WB 0.49	Horz(TL)	0.27 11	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 99 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 6-11 2 X 6 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-5-13 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-3 oc bracing.

REACTIONS (lb/size) 2=923/0-6-0, 11=790/0-6-15
 Max Horz 2=146(load case 6)
 Max Uplift 2=-221(load case 6), 11=-127(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/42, 2-3=-2697/1182, 3-4=-1760/770, 4-5=-1757/767, 5-6=-1377/631,
 7-11=-790/362, 6-7=-776/357
 BOT CHORD 2-10=-1096/2390, 9-10=-1098/2392, 8-9=-573/1273, 7-8=-21/54
 WEBS 3-10=0/177, 3-9=-850/503, 4-9=-425/1078, 5-9=-54/371, 5-8=-444/254,
 6-8=-525/1166

JOINT STRESS INDEX

2 = 0.80, 3 = 0.39, 4 = 0.64, 4 = 0.00, 5 = 0.39, 6 = 0.20, 7 = 0.15, 8 = 0.59, 9 = 0.77 and 10 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida P.E. No. 31888
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19, 2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936263
L266887	T04F	SPECIAL	1	1		
						Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:22 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2 and 127 lb uplift at joint 11.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34885
1155 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job L266887	Truss T04G	Truss Type GABLE	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936264
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Builders FirstSource, Lake City, FL 32055

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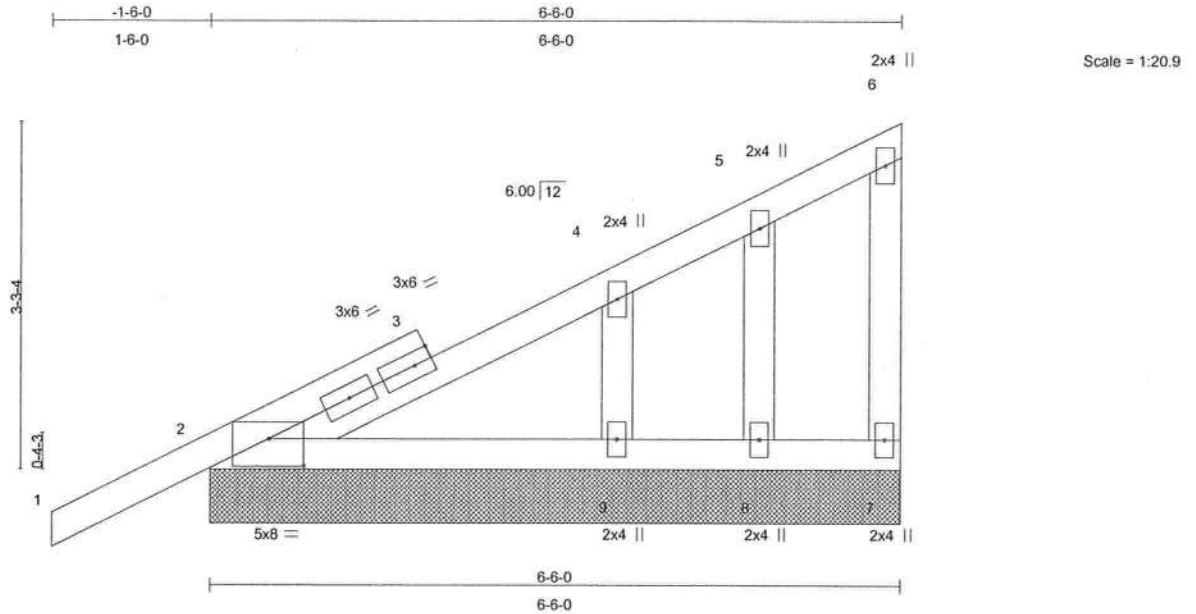


Plate Offsets (X,Y): [2:0-4-0,0-3-1]

LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	2-0-0	TC 0.20	Vert(LL) -0.00		1	n/r	120	MT20	244/190
TCDL 15.0	Lumber Increase 1.25		BC 0.09	Vert(TL) -0.00		1	n/r	90		
BCLL 10.0	* Rep Stress Incr NO		WB 0.05	Horz(TL) 0.00			n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 35 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=295/6-6-0, 7=54/6-6-0, 8=51/6-6-0, 9=291/6-6-0

Max Horz 2=179(load case 6)

Max Uplift 2=-134(load case 6), 7=-25(load case 6), 8=-50(load case 6), 9=-109(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-153/0, 3-4=-151/66, 4-5=-54/0, 5-6=-21/17, 6-7=-47/45

BOT CHORD 2-9=0/0, 8-9=0/0, 7-8=0/0

WEBS 5-8=-53/70, 4-9=-249/216

JOINT STRESS INDEX

2 = 0.61, 3 = 0.00, 3 = 0.24, 3 = 0.24, 4 = 0.11, 5 = 0.04, 6 = 0.02, 7 = 0.03, 8 = 0.04 and 9 = 0.12

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

Julius Lee
 Truss Design Engineer
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 Boynton Beach, FL 33435

February 19, 2008

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Job L266887	Truss T04G	Truss Type GABLE	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936264
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Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:23 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 1-4-0 oc.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 2, 25 lb uplift at joint 7, 50 lb uplift at joint 8 and 109 lb uplift at joint 9.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-7=-10, 1-6=-80(F=-10)

Julius Lee
Truss Design Engineer
Florida P.E. No. 31968
1100 Coastal Bay Blvd
Boynton Beach, FL 33495

February 19, 2008

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Job L266887	Truss T04H	Truss Type SPECIAL	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936265
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Builders FirstSource, Lake City, FL 32055

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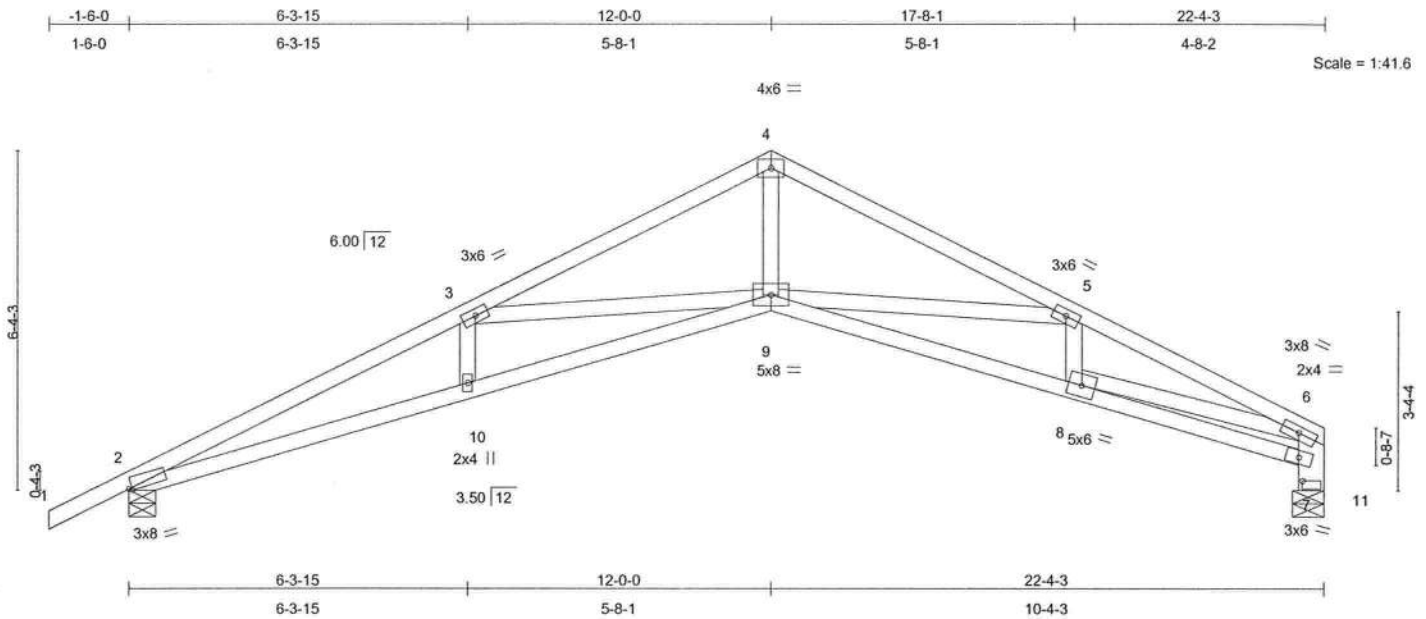


Plate Offsets (X,Y): [2:0-0-12,Edge], [4:9-11-8,5-10-1]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.29	Vert(LL)	0.20 9-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	-0.42 9-10	>629	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.69	Horz(TL)	0.31 11	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 107 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D
 BOT CHORD 2 X 4 SYP No.1D
 WEBS 2 X 4 SYP No.3 *Except*
 6-11 2 X 6 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(lb/size) 2=1002/0-6-0, 11=870/0-6-15
 Max Horz 2=126(load case 6)
 Max Uplift 2=-232(load case 6), 11=-146(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/42, 2-3=-3047/1293, 3-4=-2149/893, 4-5=-2147/891, 5-6=-2413/1063
 BOT CHORD 2-10=-1145/2710, 9-10=-1148/2714, 8-9=-941/2219, 7-8=-13/21
 WEBS 3-10=0/174, 3-9=-823/491, 4-9=-538/1425, 5-9=-415/292, 7-11=-870/395,
 6-7=-841/398, 5-8=-239/161, 6-8=-909/2148

JOINT STRESS INDEX

2 = 0.77, 3 = 0.39, 4 = 0.67, 4 = 0.00, 5 = 0.39, 6 = 0.83, 7 = 0.15, 8 = 0.62, 9 = 0.88 and 10 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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February 19,2008

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Job L266887	Truss T04H	Truss Type SPECIAL	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936265
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Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:24 2008 Page 2

NOTES

- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 2 and 146 lb uplift at joint 11.

LOAD CASE(S) Standard

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Truss Design Engineer
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Boynton Beach, FL 33435

February 19, 2008

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Job L266887	Truss T05	Truss Type HIP	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936266
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Builders FirstSource, Lake City, FL 32055

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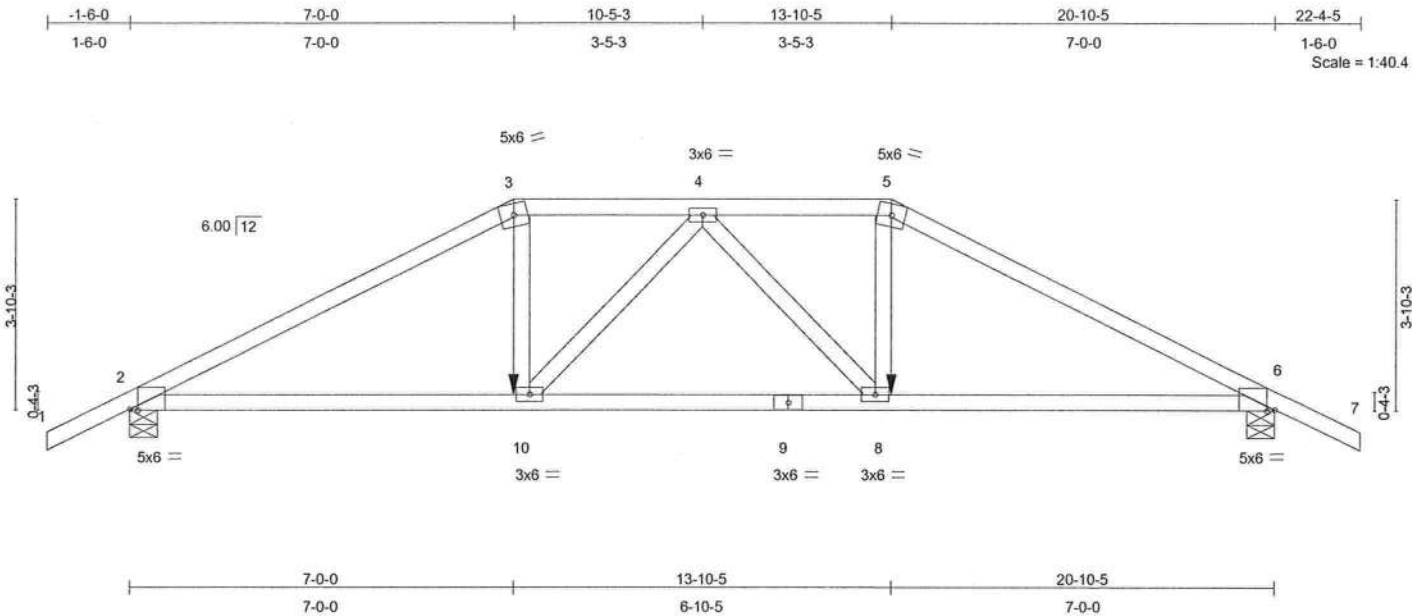


Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL)	-0.09	8-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.66	Vert(TL)	-0.28	8-10	>887	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.29	Horz(TL)	0.10	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 93 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-1-5 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-3-0 oc bracing.

REACTIONS

(lb/size) 2=1757/0-6-0, 6=1757/0-6-0
 Max Horz 2=-68(load case 6)
 Max Uplift 2=-403(load case 5), 6=-403(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-3101/611, 3-4=-2687/577, 4-5=-2687/577, 5-6=-3101/611, 6-7=0/46
 BOT CHORD 2-10=-514/2652, 9-10=-586/2867, 8-9=-586/2867, 6-8=-478/2652
 WEBS 3-10=-162/908, 4-10=-387/180, 4-8=-387/180, 5-8=-162/908

JOINT STRESS INDEX

2 = 0.76, 3 = 0.82, 4 = 0.36, 5 = 0.82, 6 = 0.76, 8 = 0.58, 9 = 0.97 and 10 = 0.58

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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February 19,2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936266
L266887	T05	HIP	1	1		
						Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 403 lb uplift at joint 2 and 403 lb uplift at joint 6.
- 7) Girder carries hip end with 7-0-0 end setback.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-70, 3-5=-149(F=-79), 5-7=-70, 2-10=-10, 8-10=-21(F=-11), 6-8=-10

Concentrated Loads (lb)

Vert: 10=-513(F) 8=-513(F)

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February 19, 2008

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Job L266887	Truss T06	Truss Type HIP	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936267
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Builders FirstSource, Lake City, FL 32055

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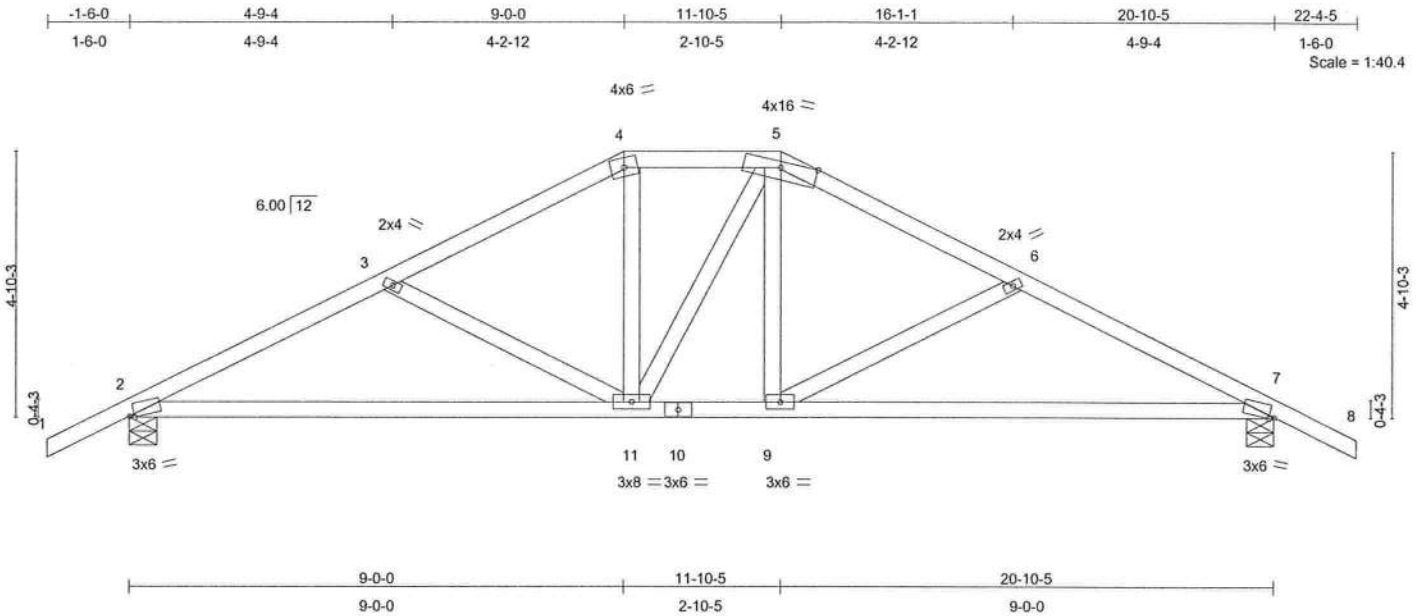


Plate Offsets (X,Y): [2:0-1-1,0-0-7], [7:0-1-1,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.14	7-9	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.42	Vert(TL)	-0.27	7-9	>909	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.15	Horz(TL)	0.04	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 105 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-11 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-10-8 oc bracing.

REACTIONS (lb/size) 2=937/0-6-0, 7=937/0-6-0
 Max Horz 2=80(load case 6)
 Max Uplift 2=-212(load case 6), 7=-212(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/46, 2-3=-1395/610, 3-4=-1071/491, 4-5=-899/489, 5-6=-1070/491, 6-7=-1395/610, 7-8=0/46
 BOT CHORD 2-11=-405/1170, 10-11=-200/898, 9-10=-200/898, 7-9=-405/1170
 WEBS 3-11=-316/233, 4-11=-70/249, 5-11=-101/104, 5-9=-70/250, 6-9=-317/233

JOINT STRESS INDEX
 2 = 0.87, 3 = 0.33, 4 = 0.36, 5 = 0.54, 6 = 0.33, 7 = 0.88, 9 = 0.34, 10 = 0.52 and 11 = 0.59

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936267
L266887	T06	HIP	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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NOTES

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 212 lb uplift at joint 2 and 212 lb uplift at joint 7.

LOAD CASE(S) Standard

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February 19, 2008

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Job L266887	Truss T07	Truss Type COMMON	Qty 5	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936268
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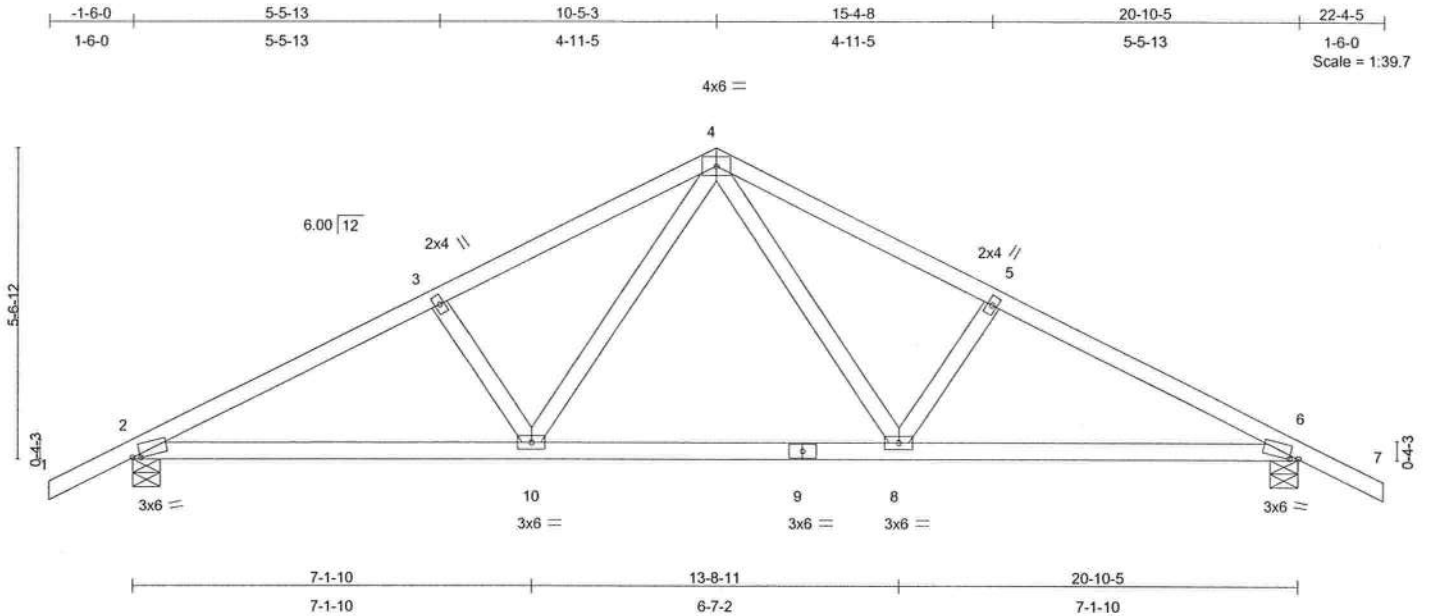


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0 1.25	TC 0.30	Vert(LL) 0.16	8-10	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase 1.25	BC 0.67	Vert(TL) -0.33	8-10	>744	240		
BCLL 10.0	* Rep Stress Incr NO	WB 0.20	Horz(TL) 0.05	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						
							Weight: 98 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-3-4 oc bracing.

REACTIONS

(lb/size) 2=1135/0-6-0, 6=1135/0-6-0
 Max Horz 2=88(load case 6)
 Max Uplift 2=-263(load case 6), 6=-263(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-1842/804, 3-4=-1650/804, 4-5=-1650/804, 5-6=-1842/804, 6-7=0/46
 BOT CHORD 2-10=-568/1550, 9-10=-298/1080, 8-9=-298/1080, 6-8=-568/1550
 WEBS 3-10=-285/219, 4-10=-288/634, 4-8=-288/634, 5-8=-285/219

JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.52, 5 = 0.33, 6 = 0.78, 8 = 0.47, 9 = 0.62 and 10 = 0.47

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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February 19,2008

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936268
L266887	T07	COMMON	5	1	Job Reference (optional)	

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 263 lb uplift at joint 2 and 263 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
- Uniform Loads (plf)
- Vert: 1-4=-70, 4-7=-70, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

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February 19, 2008

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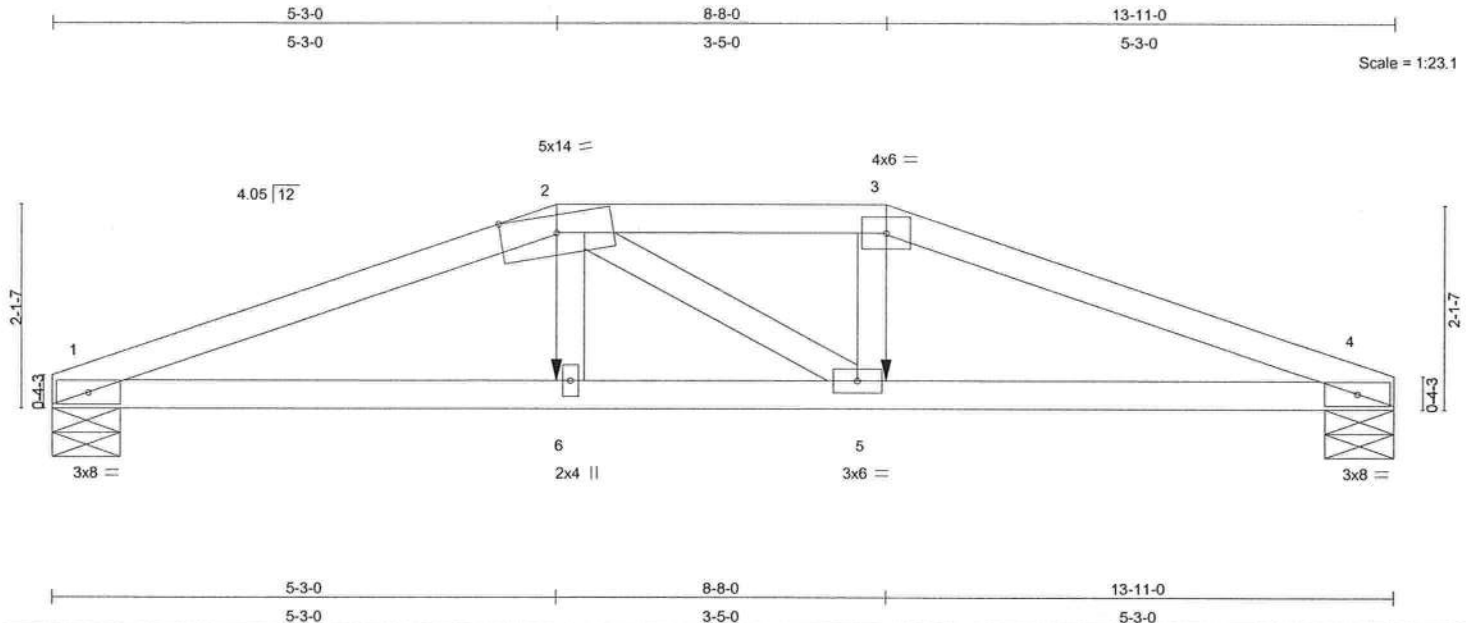
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Job L266887	Truss T08	Truss Type HIP	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936269
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.06 6	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.67	Vert(TL)	-0.17 1-6	>955	240		
BCLL 10.0	Rep Stress Incr	NO	WB 0.13	Horz(TL)	0.05 4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 52 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-6-13 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-6-15 oc bracing.

REACTIONS (lb/size) 1=1130/0-8-8, 4=1130/0-8-8
 Max Horz 1=-22(load case 6)
 Max Uplift 1=-222(load case 3), 4=-222(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2525/467, 2-3=-2310/467, 3-4=-2527/467
 BOT CHORD 1-6=-422/2280, 5-6=-425/2308, 4-5=-408/2281
 WEBS 2-6=-31/362, 2-5=-123/127, 3-5=-45/403

JOINT STRESS INDEX

1 = 0.75, 2 = 0.66, 3 = 0.84, 4 = 0.75, 5 = 0.26 and 6 = 0.26

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 1 and 222 lb uplift at joint 4.
- Girder carries hip end with 5-3-0 end setback.

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February 19,2008

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936269
L266887	T08	HIP	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, Fl 32055

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NOTES

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-115(F=-44), 2-3=-114(F=-44), 3-4=-114(F=-44), 1-4=-17(F=-6)

Concentrated Loads (lb)

Vert: 6=-263(F) 5=-263(F)

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February 19,2008

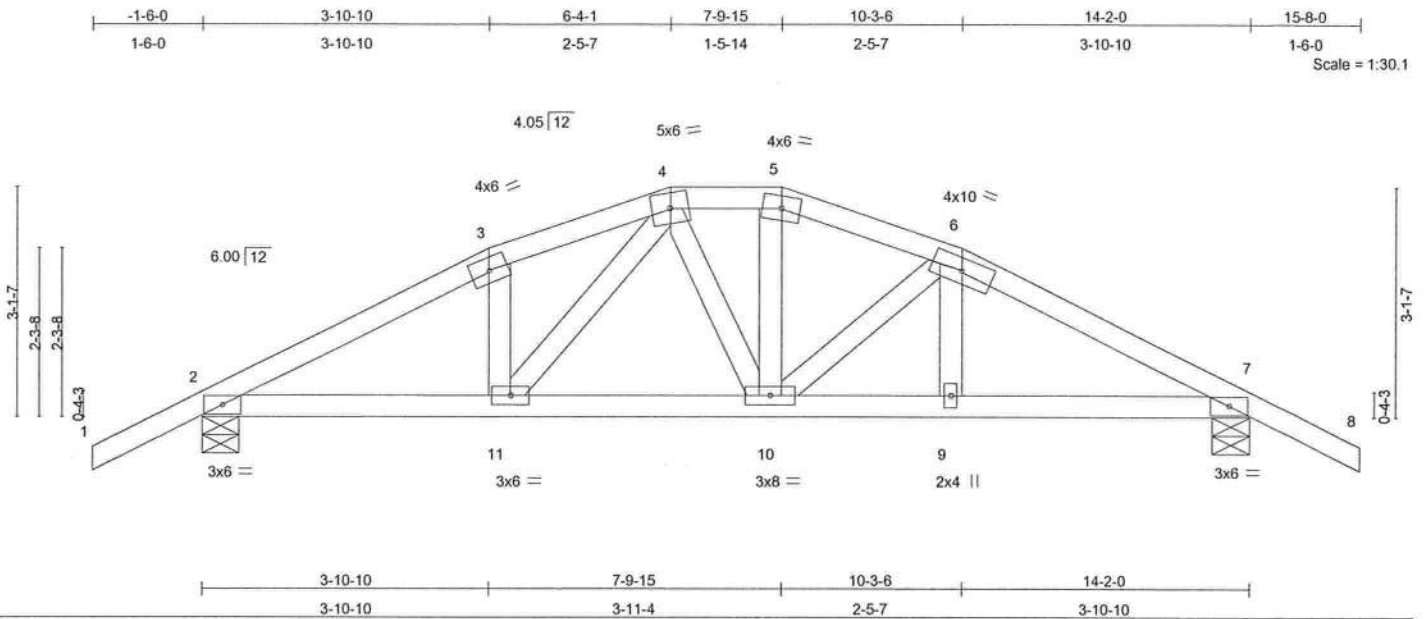
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Job L266887	Truss T09	Truss Type SPECIAL	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936270
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.20	in (loc) l/defl L/d	MT20	244/190
TCDL 15.0	Plates Increase 1.25	BC 0.15	Vert(LL) -0.02 10 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.05	Vert(TL) -0.04 10-11 >999 240		
BCDL 5.0	* Rep Stress Incr YES	(Matrix)	Horz(TL) 0.02 7 n/a n/a		
	Code FBC2004/TPI2002			Weight: 72 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=669/0-6-0, 7=669/0-6-0
 Max Horz 2=-60(load case 7)
 Max Uplift 2=-171(load case 6), 7=-171(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-866/363, 3-4=-767/401, 4-5=-628/356, 5-6=-690/357, 6-7=-863/364, 7-8=0/46
 BOT CHORD 2-11=-192/701, 10-11=-146/620, 9-10=-191/700, 7-9=-194/699
 WEBS 3-11=-85/90, 4-11=-62/146, 4-10=-45/79, 5-10=-36/110, 6-10=-121/59, 6-9=0/89

JOINT STRESS INDEX

2 = 0.41, 3 = 0.25, 4 = 0.09, 5 = 0.13, 6 = 0.19, 7 = 0.41, 9 = 0.06, 10 = 0.09 and 11 = 0.10

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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February 19, 2008

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Job L266887	Truss T09	Truss Type SPECIAL	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936270
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Builders FirstSource, Lake City, FL 32055

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NOTES

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 2 and 171 lb uplift at joint 7.

LOAD CASE(S) Standard

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February 19, 2008

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Job L266887	Truss T10	Truss Type COMMON	Qty 5	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936271
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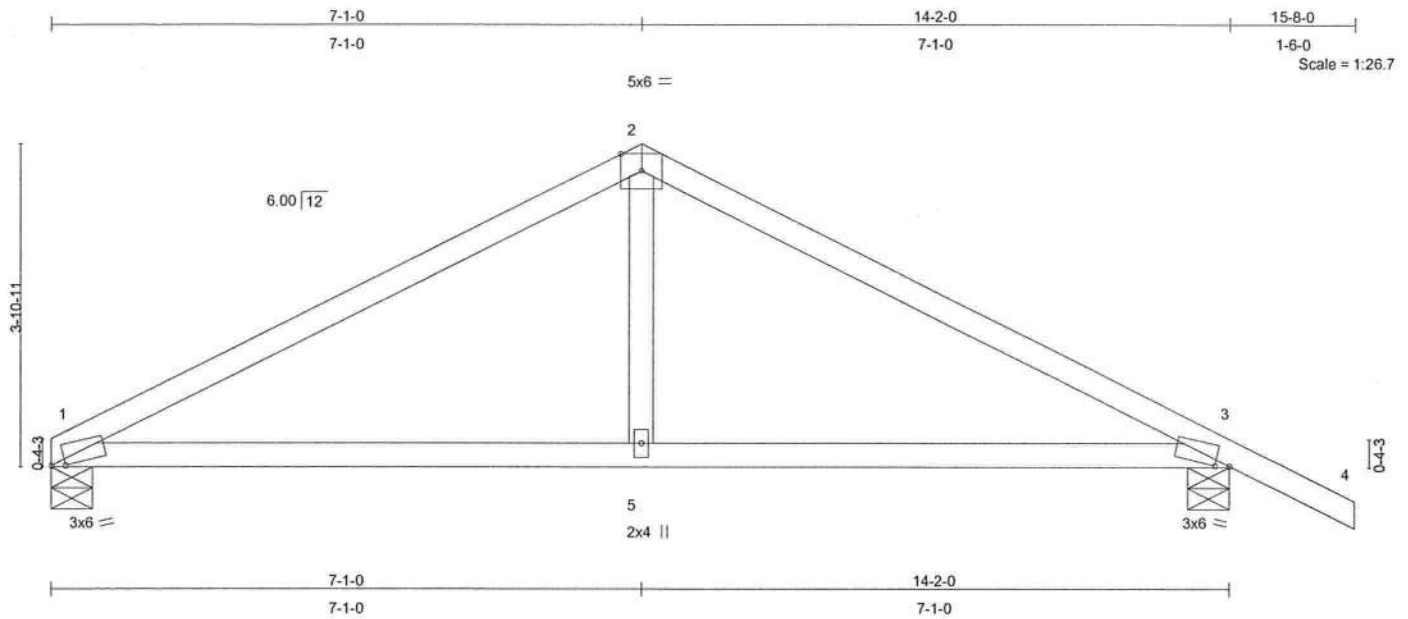


Plate Offsets (X,Y): [1:0-2-1,0-0-7], [3:0-2-1,0-0-7]

LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	1.25	TC 0.44	Vert(LL) 0.07	1-5	>999	360		MT20	244/190
TCDL 15.0	Lumber Increase 1.25		BC 0.39	Vert(TL) -0.13	1-5	>999	240			
BCLL 10.0	* Rep Stress Incr YES		WB 0.08	Horz(TL) 0.02	3	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=539/0-6-0, 3=677/0-6-0
 Max Horz 1=-78(load case 7)
 Max Uplift 1=-94(load case 6), 3=-175(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-803/364, 2-3=-807/368, 3-4=0/46
 BOT CHORD 1-5=-164/625, 3-5=-164/625
 WEBS 2-5=0/251

JOINT STRESS INDEX

1 = 0.86, 2 = 0.72, 3 = 0.86 and 5 = 0.18

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
 Truss Design Engineer
 Florida PE No. 33888
 1406 Coastal Bay Blvd.
 Boynton Beach, FL 33436

February 19, 2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job L266887	Truss T10	Truss Type COMMON	Qty 5	Ply 1	MIKE & LINDA CADY J1936271 Job Reference (optional)
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NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 1 and 175 lb uplift at joint 3.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34889
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

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Job L266887	Truss T11	Truss Type MONO TRUSS	Qty 16	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936272
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Builders FirstSource, Lake City, FL 32055

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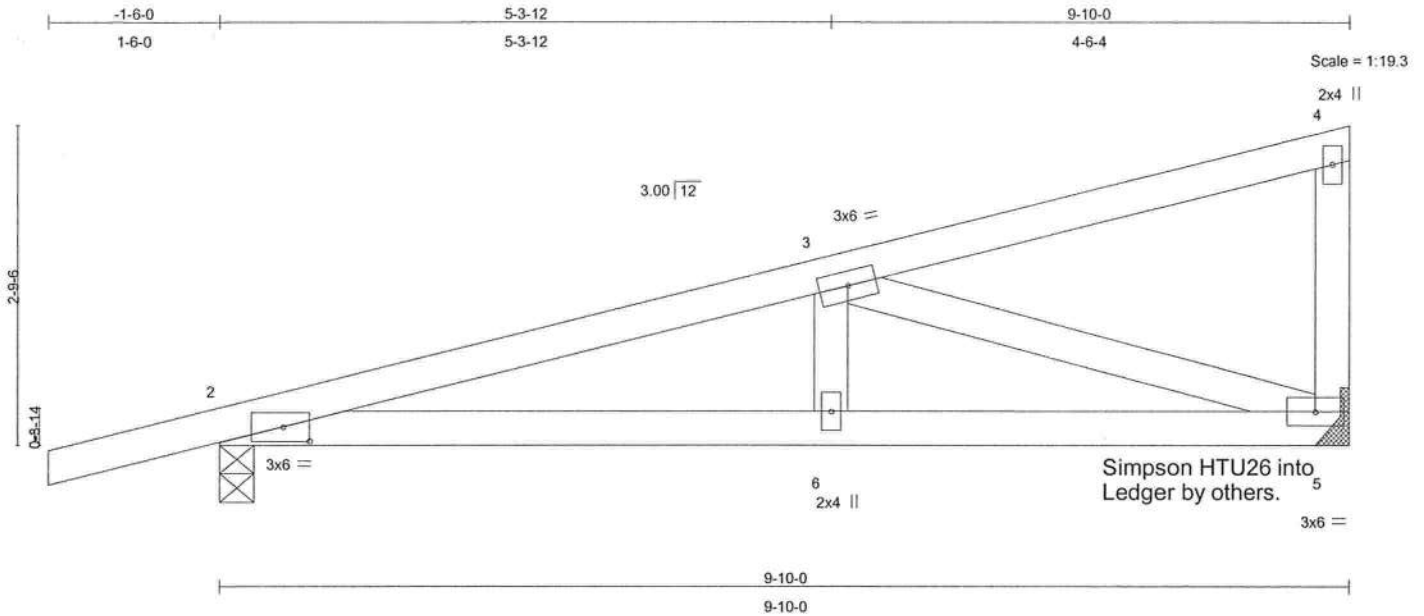


Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	2-0-0	TC 0.27	Vert(LL) 0.09	2-6	>999	360		MT20	244/190
TCDL 15.0	Lumber Increase 1.25		BC 0.21	Vert(TL) -0.06	2-6	>999	240			
BCLL 10.0	* Rep Stress Incr YES		WB 0.27	Horz(TL) -0.02	5	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-12 oc bracing.

REACTIONS (lb/size)

5=372/Mechanical, 2=507/0-3-8
 Max Horz 2=96(load case 4)
 Max Uplift 5=-208(load case 4), 2=-270(load case 4)

Recommended hanger connection based on manufacturer tested capacities and nail calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Hanger connection to be reviewed and approved by the Architect/Engineer of Record.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/24, 2-3=-804/959, 3-4=-85/33, 4-5=-129/100
 BOT CHORD 2-6=-1031/735, 5-6=-1031/735
 WEBS 3-5=-724/1017, 3-6=-275/153

JOINT STRESS INDEX

2 = 0.31, 3 = 0.42, 4 = 0.75, 5 = 0.54 and 6 = 0.11

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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February 19, 2008

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936272
L266887	T11	MONO TRUSS	16	1	Job Reference (optional)	

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NOTES

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 208 lb uplift at joint 5 and 270 lb uplift at joint 2.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 34868
1400 Coastal Bay Blvd
Boynton Beach, FL 33495

February 19, 2008

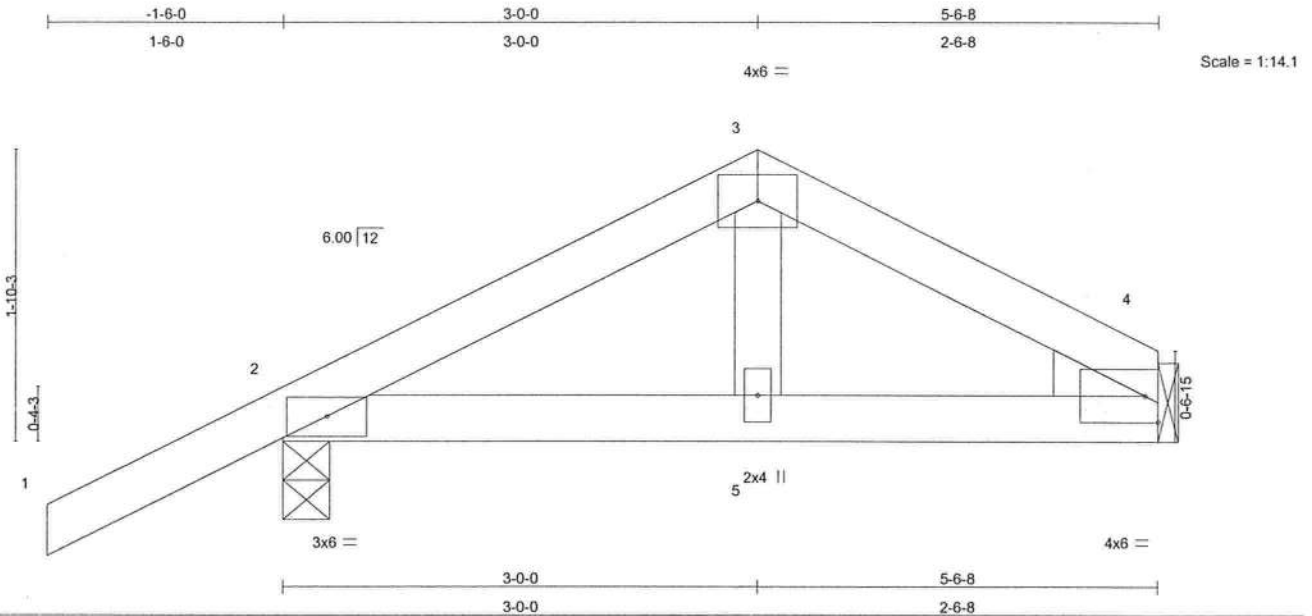
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Job L266887	Truss T12	Truss Type COMMON	Qty 20	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936273
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.18	Vert(LL)	0.01	2-5	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.24	Vert(TL)	-0.01	2-5	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.03	Horz(TL)	-0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 WEDGE
 Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=346/0-3-8, 4=196/Mechanical
 Max Horz 2=68(load case 6)
 Max Uplift 2=-250(load case 6), 4=-132(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-242/319, 3-4=-222/310
 BOT CHORD 2-5=-207/157, 4-5=-207/157
 WEBS 3-5=-182/96

JOINT STRESS INDEX

2 = 0.17, 3 = 0.09, 4 = 0.14, 4 = 0.00 and 5 = 0.07

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

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 Truss Design Engineer
 Florida PE No. 34859
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936273
L266887	T12	COMMON	20	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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NOTES

- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 250 lb uplift at joint 2 and 132 lb uplift at joint 4.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 34885
1400 Coastal Bay Blvd
Boynton Beach, FL 33436

February 19, 2008

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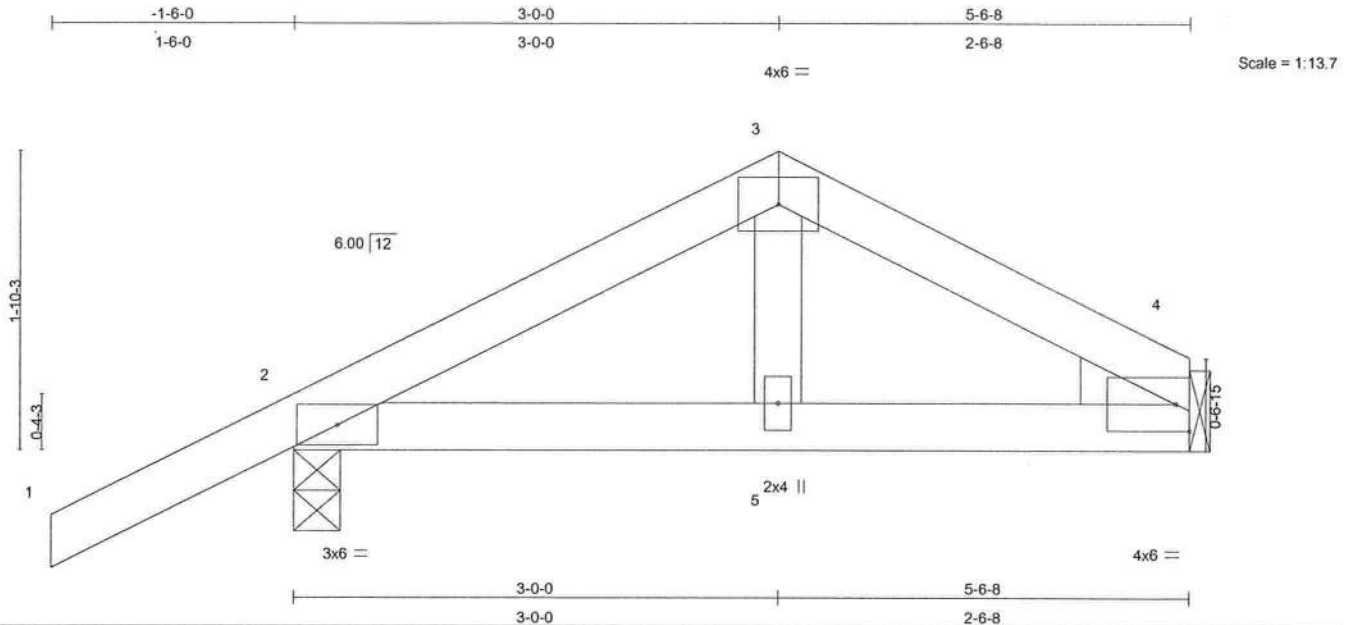
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Job L266887	Truss T12G	Truss Type GABLE	Qty 1	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936274
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Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.24	Vert(LL)	0.01	2-5	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.30	Vert(TL)	-0.01	2-5	>999	240		
BCLL 10.0	Rep Stress Incr	NO	WB 0.03	Horz(TL)	-0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 WEDGE
 Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=392/0-3-8, 4=220/Mechanical
 Max Horz 2=68(load case 6)
 Max Uplift 2=-282(load case 6), 4=-149(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-4/52, 2-3=-272/353, 3-4=-249/340
 BOT CHORD 2-5=-234/176, 4-5=-234/176
 WEBS 3-5=-181/96

JOINT STRESS INDEX
 2 = 0.20, 3 = 0.10, 4 = 0.16, 4 = 0.00 and 5 = 0.07

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

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February 19,2008

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Job	Truss	Truss Type	Qty	Ply	MIKE & LINDA CADY	J1936274
L266887	T12G	GABLE	1	1		
						Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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NOTES

- 5) Gable studs spaced at 2-0-0 oc.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2 and 149 lb uplift at joint 4.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) Gable truss supports 1' 0" max. rake gable overhang.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-4=-10, 1-3=-80(F=-10), 3-4=-80(F=-10)

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February 19,2008

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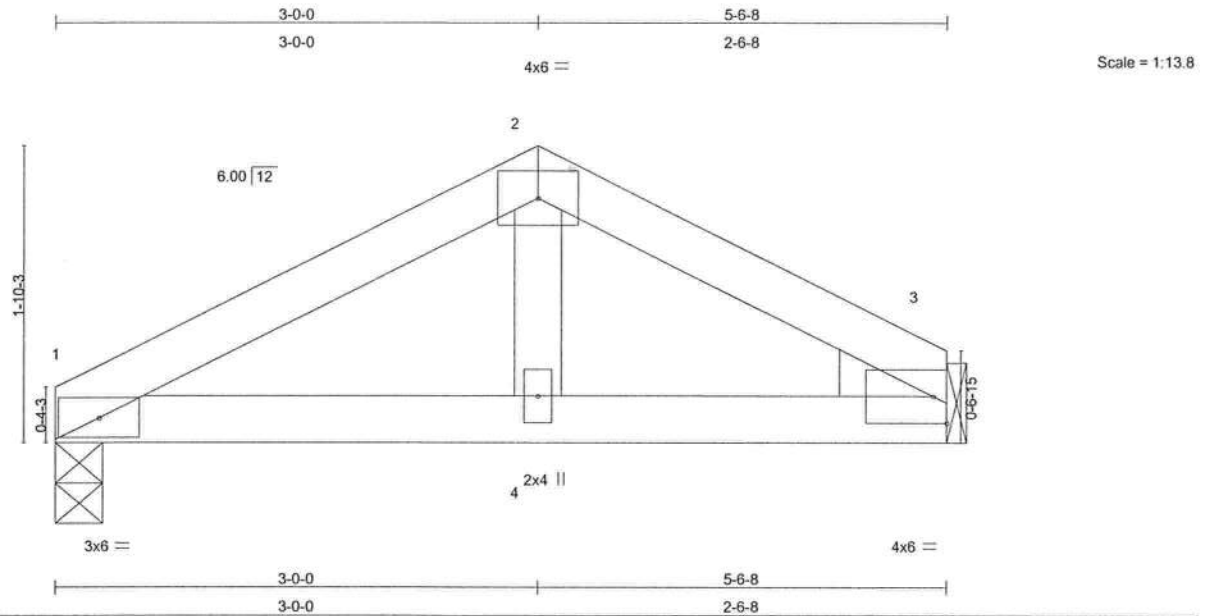
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Job L266887	Truss T13	Truss Type COMMON	Qty 2	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936275
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase	1.25	TC 0.18	Vert(LL)	0.01	1-4	>999	360	MT20	244/190
TCDL 15.0	Lumber Increase	1.25	BC 0.25	Vert(TL)	-0.01	1-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.03	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 21 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 WEDGE
 Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=213/0-3-8, 3=213/Mechanical
 Max Horz 1=-22(load case 4)
 Max Uplift 1=-140(load case 6), 3=-138(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-249/339, 2-3=-255/351
 BOT CHORD 1-4=-242/186, 3-4=-242/186
 WEBS 2-4=-181/96

JOINT STRESS INDEX

1 = 0.12, 2 = 0.10, 3 = 0.15, 3 = 0.00 and 4 = 0.07

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

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 Boynton Beach, FL 33435

February 19,2008

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Job L266887	Truss T13	Truss Type COMMON	Qty 2	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936275
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Builders FirstSource, Lake City, FL 32055

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NOTES

- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 1 and 138 lb uplift at joint 3.

LOAD CASE(S) Standard

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Truss Design Engineer
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Boynton Beach, FL 33435

February 19, 2008

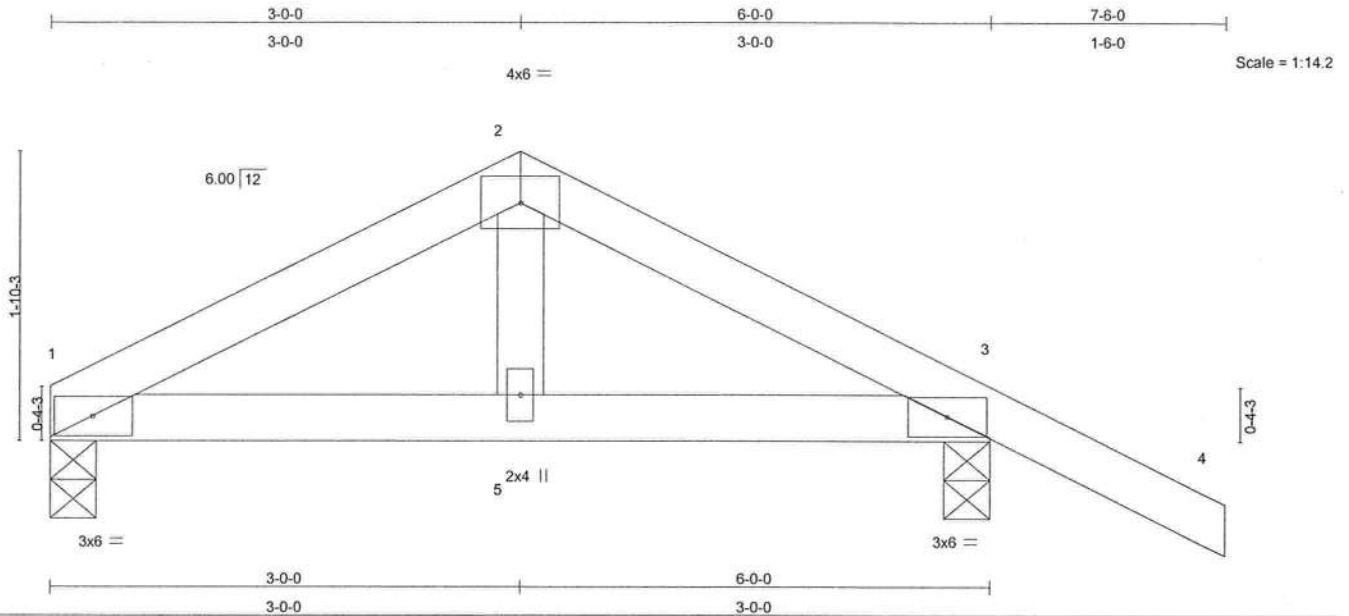
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Job L266887	Truss T14	Truss Type COMMON	Qty 2	Ply 1	MIKE & LINDA CADY Job Reference (optional)	J1936276
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LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	1.25	TC 0.18	Vert(LL) 0.01	3-5	>999	360		MT20	244/190
TCDL 15.0	Lumber Increase 1.25		BC 0.07	Vert(TL) -0.01	3-5	>999	240			
BCLL 10.0	* Rep Stress Incr YES		WB 0.03	Horz(TL) 0.00	3	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 24 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=212/0-3-8, 3=360/0-3-8
 Max Horz 1=-68(load case 7)
 Max Uplift 1=-144(load case 6), 3=-258(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-246/345, 2-3=-271/364, 3-4=0/46
 BOT CHORD 1-5=-189/183, 3-5=-189/183
 WEBS 2-5=-194/103

JOINT STRESS INDEX

1 = 0.18, 2 = 0.10, 3 = 0.18 and 5 = 0.07

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

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February 19,2008

Continued on page 2

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Job L266887	Truss T14	Truss Type COMMON	Qty 2	Ply 1	MIKE & LINDA CADY Job Reference (optional) J1936276
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Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Feb 15 13:17:32 2008 Page 2

NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 1 and 258 lb uplift at joint 3.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

February 19, 2008

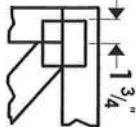
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

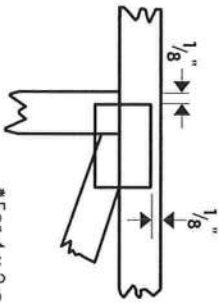


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

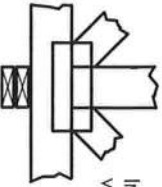
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



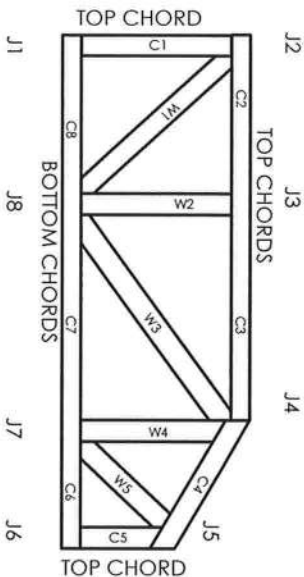
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561



Mitek Engineering Reference Sheet: MIT-7473



General Safety Notes

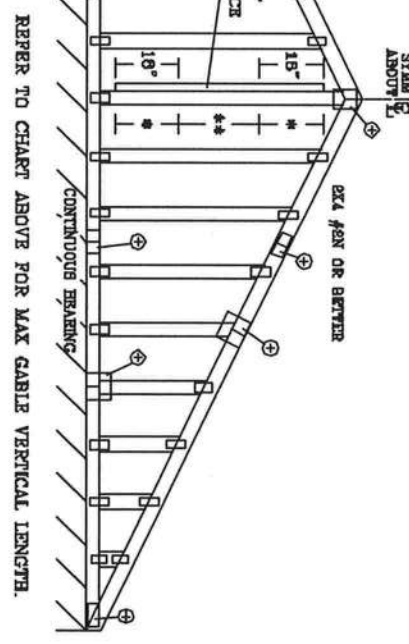
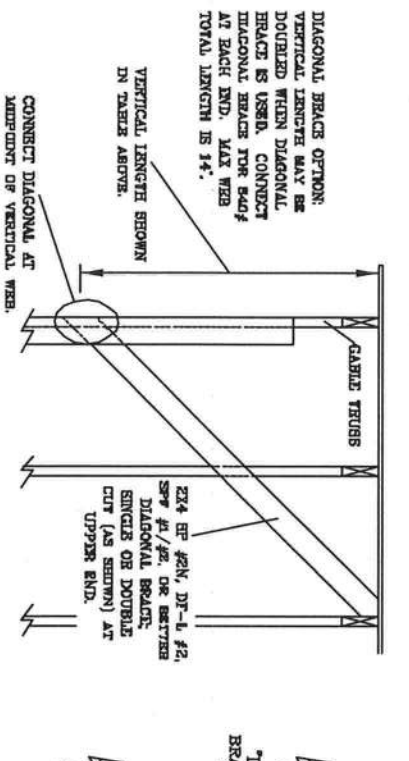
Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and warps at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

CABLE SPACING	BRACE NO.	GROUP A						GROUP B					
		12" O.C.	16" O.C.	24" O.C.	12" O.C.	16" O.C.	24" O.C.	12" O.C.	16" O.C.	24" O.C.	12" O.C.	16" O.C.	24" O.C.
24" GABLE VERTICAL SPECIES	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
	HF	STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	9' 8"	9' 8"	11' 8"	11' 8"
		#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 14"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	6' 10"	6' 3"	6' 11"	7' 6"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
	SP	#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 3"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	9' 10"	9' 10"	12' 0"	12' 0"
	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
HF	STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	8' 2"	8' 2"	10' 7"	10' 7"	14' 0"	14' 0"	
	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
	#2	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	13' 5"	14' 0"	14' 0"	
SP	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	13' 5"	14' 0"	14' 0"	
	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	13' 5"	14' 0"	14' 0"	
	STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	8' 4"	8' 4"	10' 10"	10' 10"	14' 0"	14' 0"	
DFL	#1 / #2	4' 3"	7' 4"	7' 4"	8' 9"	8' 9"	10' 6"	10' 6"	13' 8"	13' 8"	14' 0"	14' 0"	
	#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"	
	STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"	
SPF	STANDARD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"	
	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"	
	#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"	
SP	#3	4' 4"	7' 2"	7' 2"	8' 9"	8' 9"	10' 6"	10' 6"	13' 8"	14' 0"	14' 0"	14' 0"	
	STUD	4' 4"	7' 1"	7' 1"	8' 9"	8' 9"	10' 6"	10' 6"	13' 8"	14' 0"	14' 0"	14' 0"	
	STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 5"	12' 6"	12' 6"	14' 0"	14' 0"	



BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-FIR	MEM-FIR	SOUTHERN PINE	DOUGLAS FIR-LARCH
#1 / #2	#1	#1	#1
STANDARD	STUD	STUD	STUD
#3	#3	#3	#3
STUD	STANDARD	STANDARD	STANDARD

CABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEPRESSION CRITERIA IS L/240.
 PROVIDE VERTICAL CONNECTIONS FOR 135 PSF OVER CONTINUOUS BEARING (6 PSF VC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0" OUTLIMBERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
 ATTACH EACH T-BRACE WITH 10d NAILS.
 * FOR (1) T-BRACE, BRACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) T-BRACES, BRACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 T-BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES			
VERTICAL LENGTH	NO BRACE	1X4 OR 2X4	2X4
LESS THAN 4' 0"	1X4 OR 2X4	2X4	2X4
GREATER THAN 4' 0", BUT	2X4	2X4	2X4
LESS THAN 11' 6"	2X4	2X4	2X4
GREATER THAN 11' 6"	2X4	2X4	2X4

+ REFER TO COLUMN THRU DESIGN FOR PEAK SPURSE AND BEEL PLATES.

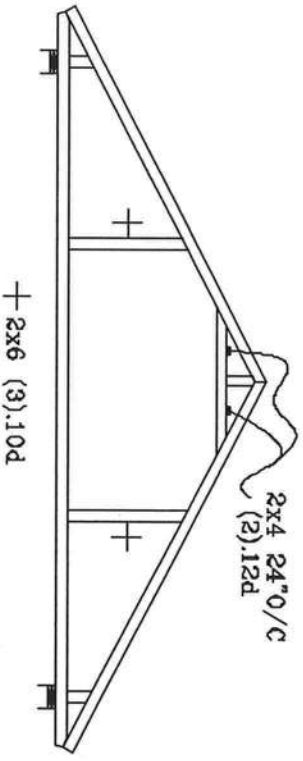
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS 1-83 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY PCI TRUSS PLATE INSTITUTE, 593 JONES RD, SUITE 200, MARIETTA, VA 25775) AND VITA (VITA TRUSS CONSULT OF AMERICA, 6300 ENTWISLER LN, MARIETTA, VA 25775) FOR SAFETY PRACTICES FROM TO PERFORM THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1456 SW 4th AVENUE
 DELRAY BEACH, FL 33444-2161

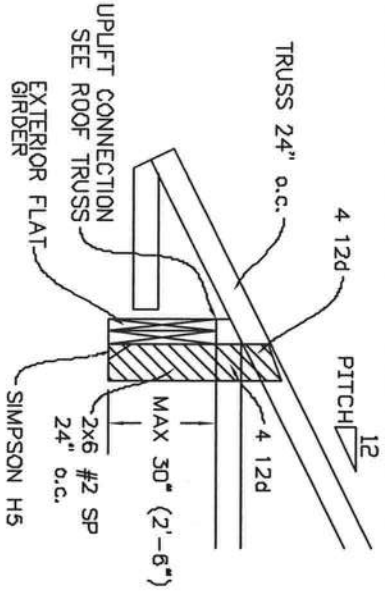
No. 34889
 STATE OF FLORIDA

REF	ASCE7-02-CAB13015
DATE	11/26/03
DRWG	MTRK STD CABLE 16 D ET
ENG	
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

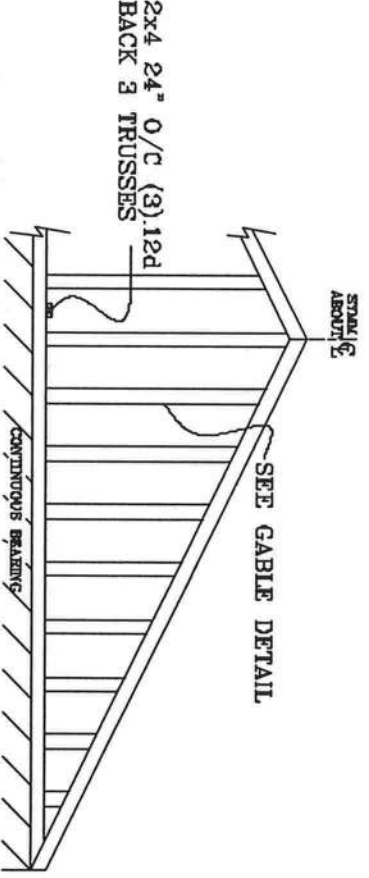
TYPICAL ATTIC TRUSS BRACING



TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

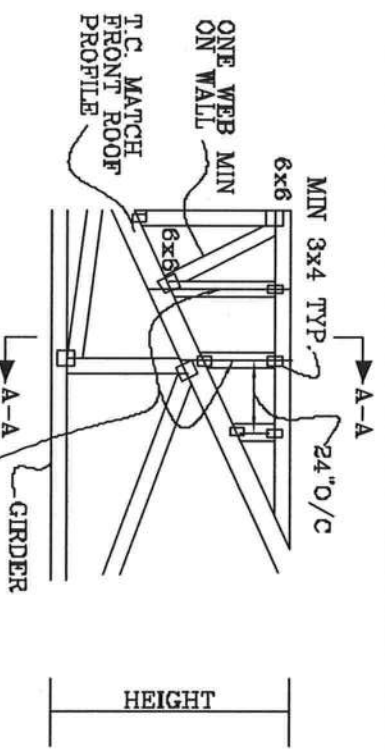


GABLE END TRUSS DETAIL

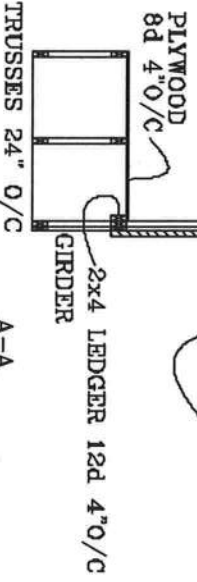


MINIMUM BC BRACING ON GABLE TRUSS OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR BOB

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



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OWEN BRANCH, FL 33444-2091

No. 34869
STATE OF FLORIDA

TOP CHORD 2x4 #2 OR BETTER
 BOT CHORD 2x4 #2 OR BETTER
 WEBS 2x4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.
 SPACE PIGGYBACK VERTICALS AT 4' OC MAX.
 TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

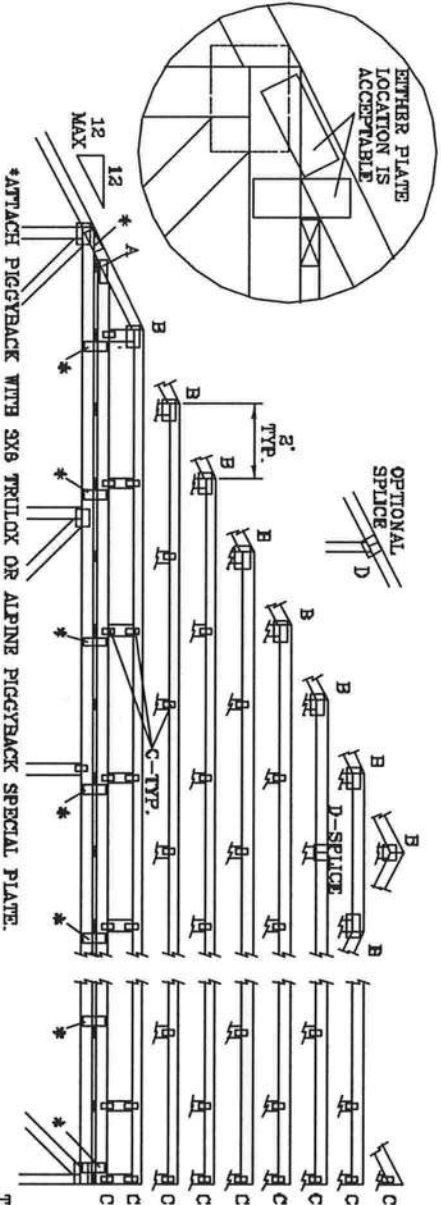
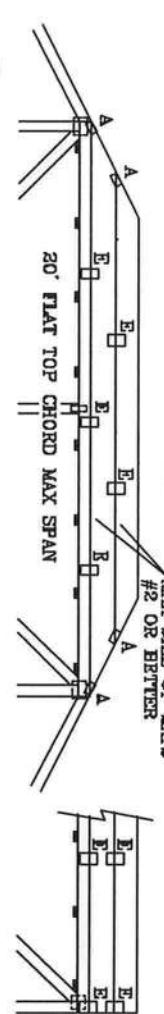
PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:
 110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
 CAT I, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF
 110 MPH WIND, 30' MEAN HGT, ENG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
 WIND TC DL=6 PSF, WIND BC DL=6 PSF

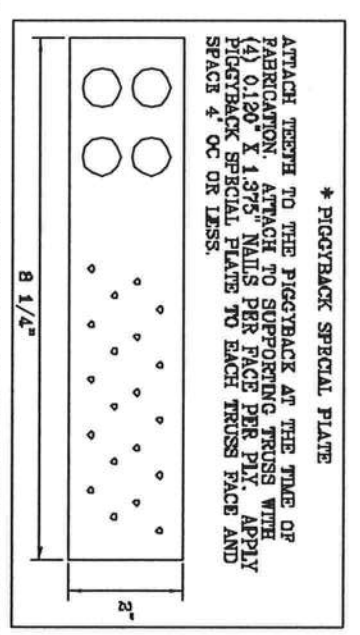
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



JOINT TYPE	SPANS UP TO		
	30'	36'	62'
A	2x4	2.5x4	3x5
B	4x6	6x6	6x8
C	1.5x3	1.5x4	1.5x4
D	6x4	6x5	6x5
E	4x8 OR 3x6 TRUSS AT 4' OC, HOT ROLLED VERTICALLY		

ATTACH TRUSS PLATES WITH (6) 0.120" X 1.375" NAILS OR EQUAL PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE SAME GRADE, SPECIES AS WEB MEMBER OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE SAME GRADE, SPECIES AS WEB MEMBER OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.



ENGINEER: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICE GUIDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS AND LUMBER ASSOCIATION, INC., 500 W. WASHINGTON, ST. LOUIS, MO 63102. THESE TRUSSES ARE DESIGNED FOR USE IN RESIDENTIAL AND COMMERCIAL APPLICATIONS. THESE TRUSSES ARE NOT TO BE USED IN ANY OTHER APPLICATIONS WITHOUT THE EXPRESS WRITTEN PERMISSION OF THE ENGINEER. UNLESS OTHERWISE INDICATED, ALL TRUSS MEMBERS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBBON CEILING.

JULIUS LEE'S
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 1450 SW 4th AVENUE
 DEERBET BEACH, FL 33444-2161

MAX LOADING	REF
55 PSF AT	PIGGYBACK
1.33 DUR. FAC.	DATE 09/12/07
50 PSF AT	DRWG/ITERK STD PIGGY
1.25 DUR. FAC.	-ENG JL
47 PSF AT	
1.15 DUR. FAC.	
SPACING 24.0"	

No: 34868
 STATE OF FLORIDA

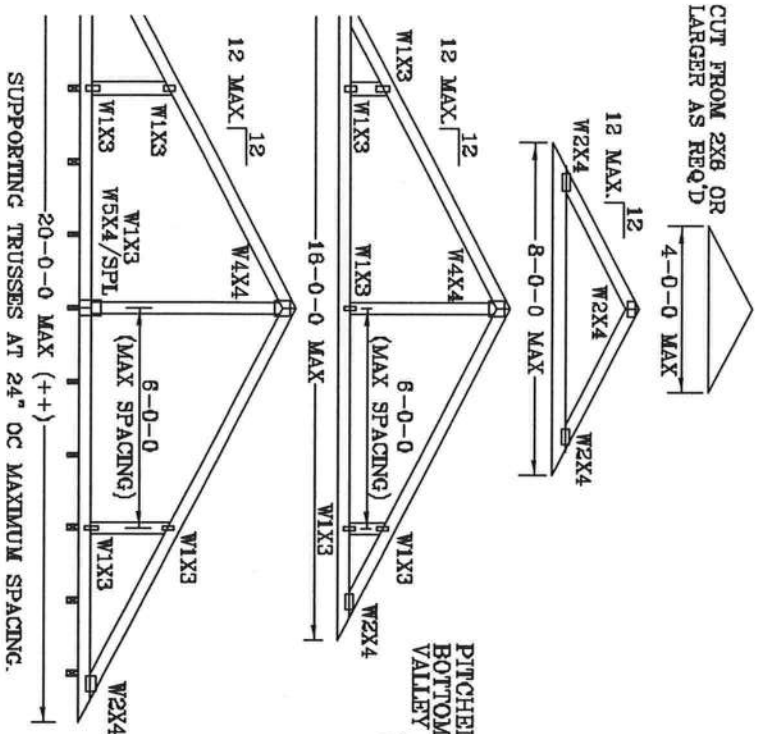
THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
 BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
 WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
 (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
 FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
 ASCE 7-02 130 MPH WIND, 15' MEAN HEIGHT, ENCLOSED
 BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.



UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "I"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

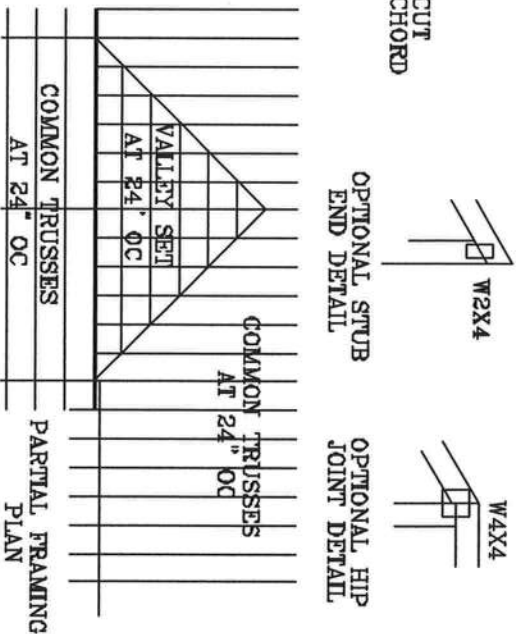
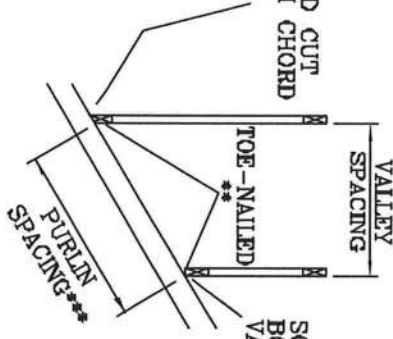
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

OR PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



THIS DRAWING REPLACES DRAWING A105

REWORKING TRUSSES, REMOVE CUTSIDE GAGE IN FABRICATING, HANDLING, SHIPPING, REINSTALLING AND BRACING. REFER TRUSS AND GAGE TO THE MANUFACTURER'S INSTRUCTIONS. THESE TRUSSES ARE DESIGNED FOR USE IN THE UNITED STATES OF AMERICA, 62ND DISTRICT, WASHINGTON, D.C. 20579 AND VITACORP, TRUSS DIVISION, 1000 W. WASHINGTON, WASHINGTON, D.C. 20004. THESE TRUSSES ARE DESIGNED TO PERFORM THE FUNCTIONS UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBBON CEILING.

JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1655 SW 4th AVENUE
 DEBALT BEACH, FL 33442-2801

No. 34889
 STATE OF FLORIDA

TC LL	20	PSF	REF	VALLEY DETAIL
TC DL	7	PSF	DATE	11/26/03
BC DL	5	PSF	DRWG	VALTRUSS1103
BC LL	0	PSF	-ENG	JL
TOT. LD.	32	PSF		
DUR.FAC.	125			
SPACING	24"			

TOE-NAIL DETAIL

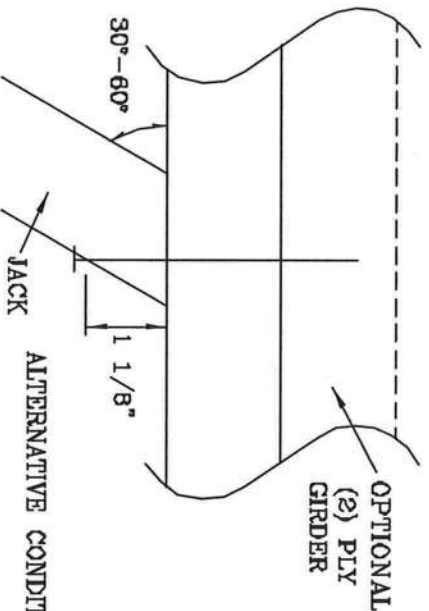
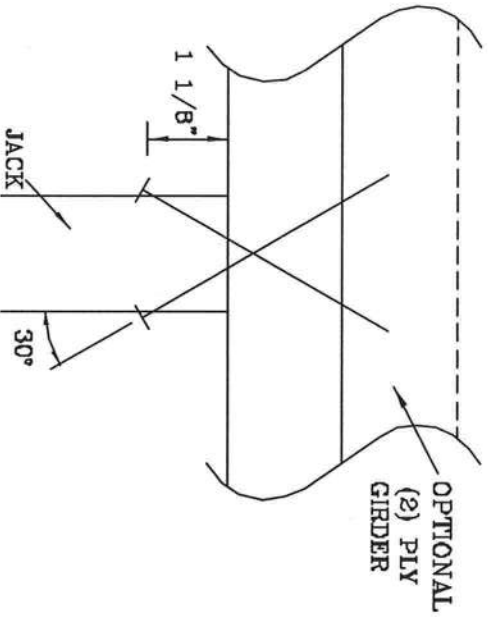
TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AP&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-43 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS OF AMERICA, 6500 ENTERPRISE LN, MADISON, VA 20719 AND VICA (AIA) TRUSS COUNCIL TRUSS FOUNDATION. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1400 ST 4TH AVENUE
DELAWARE BEACH, FL 33444-2181

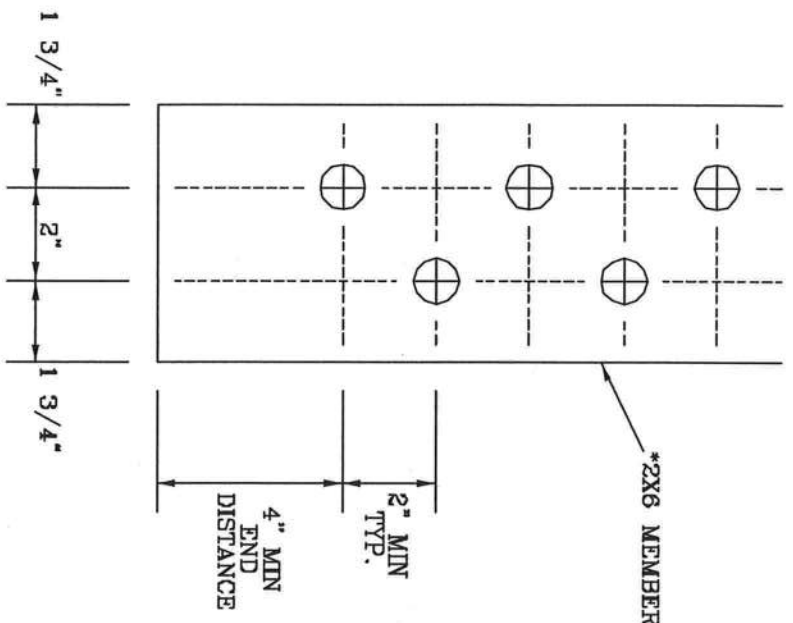
No. 34889
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

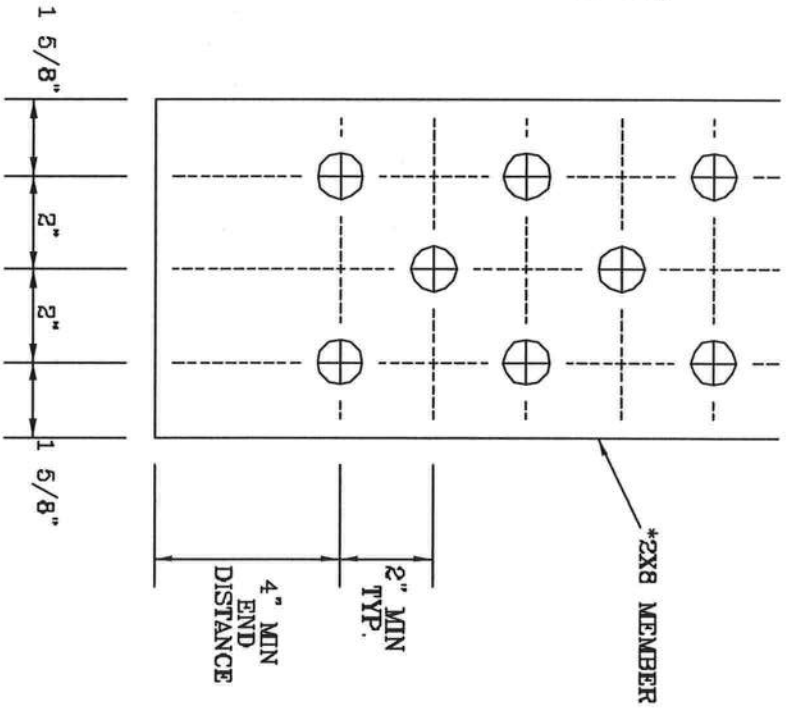
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
 BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
 WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 385 DOWNEY DR., SUITE 200, WOODSTOCK, VA, 20179 AND WTCA CODED TRUSS CONSTRUCTION. FOR ADDITIONAL INFORMATION, CONTACT THE TRUSS MANUFACTURER. ALL TRUSS MANUFACTURERS SHALL HAVE A PERMIT ATTACHED TO THEIR STRUCTURAL PANELS AND DETAILINGS SHALL HAVE A PERMIT ATTACHED TO THEIR DETAILINGS.

JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1400 W. 4TH AVENUE
 DEERBURY BEACH, FL 33444-2611

No: 34689
 STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

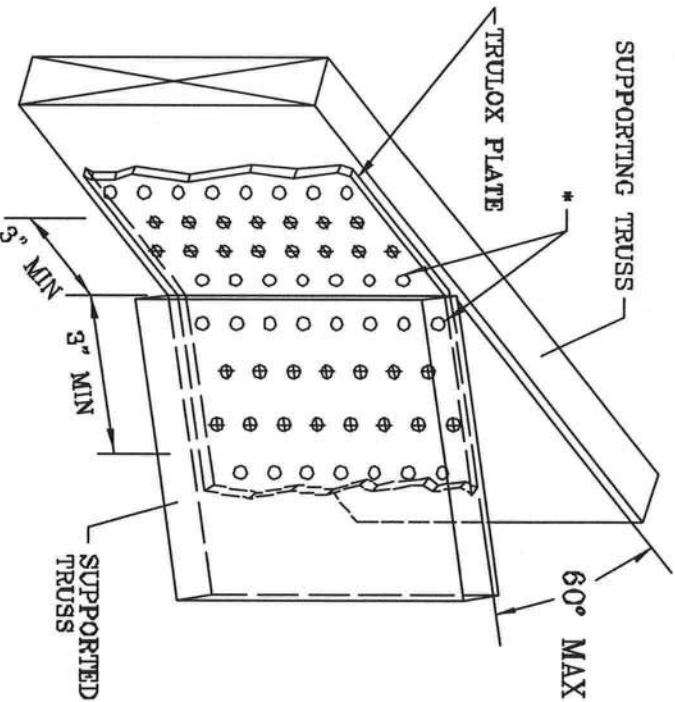
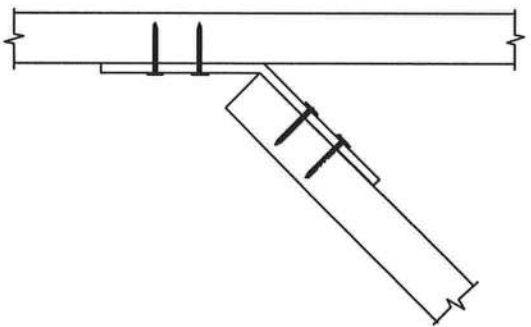
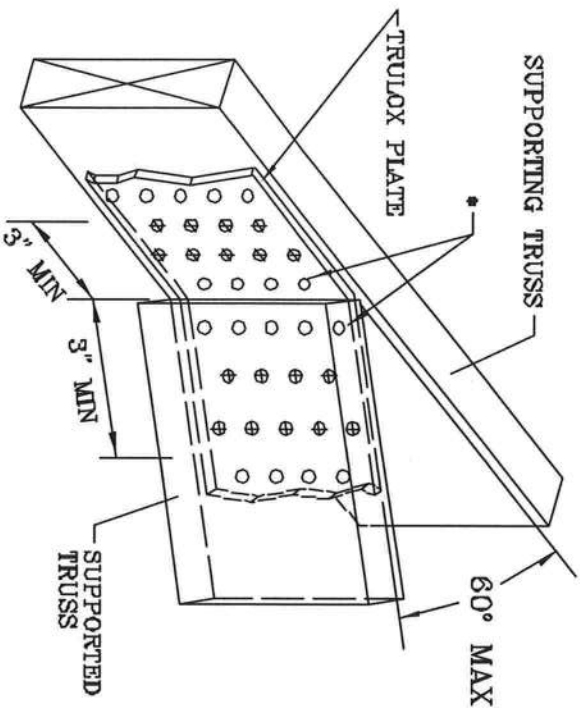
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	16	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R
1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCAT 1-100 (INCLUDING COMPONENT SAFETY INSTRUCTIONS, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 380 JENNIFER DR., SUITE 800, MADISON, VA 22729) AND VITA (VEDD) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, VA 22729 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELAFT BEACH, FL 33444-3201

REF TRULOX

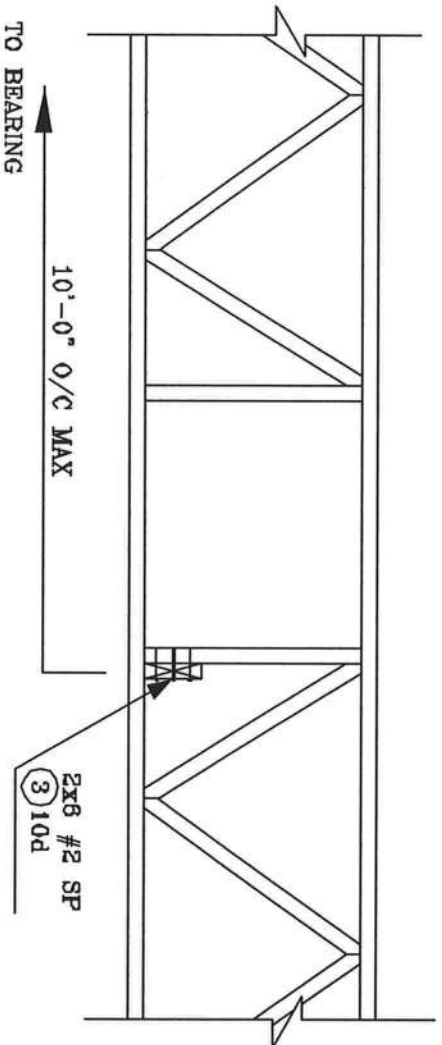
DATE 11/26/03

DRWG CNTRULOX1103

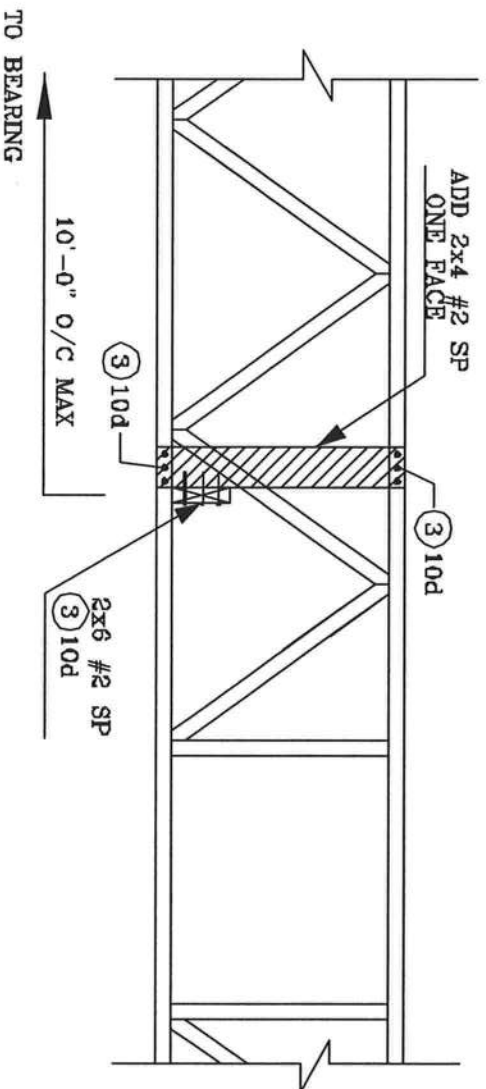
-ENG JL

No: 34889
STATE OF FLORIDA

**STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS**



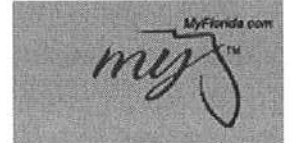
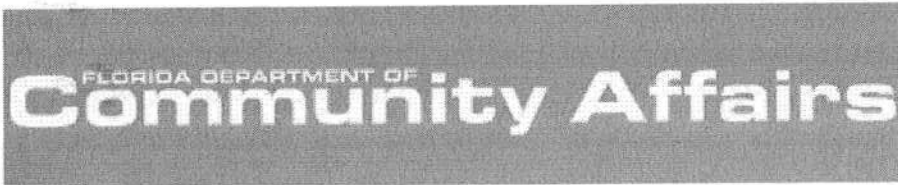
**ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP**



JULIUS LEF'S
CONS. ENGINEERS P.A.
1426 SW 4th AVENUE
DIERHAT BEACH, FL 33444-2161

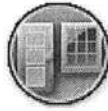
No: 34959
STATE OF FLORIDA

roofing



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Product Approval
USER: Public User

[Product Approval Menu](#) > [Product or Application Search](#) > [Application List](#) > **Application Detail**

- ▶ COMMUNITY PLANNING
- ▶ HOUSING & COMMUNITY DEVELOPMENT
- ▶ EMERGENCY MANAGEMENT
- ▶ OFFICE OF THE SECRETARY

FL # FL7849
 Application Type New
 Code Version 2004
 Application Status Approved
 Comments
 Archived

Product Manufacturer MonierLifetile
 Address/Phone/Email 200 Story Road
 Lake Wales, FL 33853
 (863) 676-9405
 szigich@monierlifetile.com

Authorized Signature Steve Zigich
 szigich@monierlifetile.com

Technical Representative Reese Moody
 Address/Phone/Email 200 Story Road
 Lake Wales, FL 33853
 rmoody@monierlifetile.com

Quality Assurance Representative Steve Zigich
 Address/Phone/Email 200 Story Road
 Lake Wales, FL 33853
 szigich@monierlifetile.com

Category Roofing
 Subcategory Roofing Tiles

Compliance Method Evaluation Report from a Product Eval

Evaluation Entity ICC Evaluation Service, Inc.
 Quality Assurance Entity R I Ogawa & Associates, Inc.
 Quality Assurance Contract Expiration Date
 Validated By Gary W. Walker
 Validation Checklist - Hardcopy Re

Certificate of Independence [FL7849_R0_COI_ICC Independence S](#)

Referenced Standard and Year (of Standard) **Standard**
 ASTM C 1492

Equivalence of Product Standards
 Certified By

Sections from the Code

Product Approval Method Method 1 Option C

Date Submitted 11/16/2006
 Date Validated 11/16/2006
 Date Pending FBC Approval 11/17/2006
 Date Approved 12/06/2006

Summary of Products		
FL #	Model, Number or Name	Description
7849.1	Saxony	Flat Concrete Roof tile
Limits of Use Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other: Class A roof covering		Installation Instructions FL7849_R0_II_FRSA-TRI Instal Verified By: Gary W. Walker 4C Created by Independent Third I Evaluation Reports FL7849_R0_AE_ESR-1647.pdf

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards

2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436
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Product Approval Accepts:



FLORIDA DEPARTMENT OF Community Affairs



- ▶ COMMUNITY PLANNING
- ▶ HOUSING & COMMUNITY DEVELOPMENT
- ▶ EMERGENCY MANAGEMENT
- ▶ OFFICE OF THE SECRETARY

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Product Approval
USER: Public User

[Product Approval Menu](#) > [Product or Application Search](#) > [Application List](#) > **Application Detail**

FL #	FL5108
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	<input type="checkbox"/>

Product Manufacturer
Address/Phone/Email

MI Windows and Doors
650 W Market St
Gratz, PA 17030
(717) 365-3300 ext 2101
surich@miwd.com

Authorized Signature

Steven Urlich
surich@miwd.com

Technical Representative
Address/Phone/Email

Quality Assurance Representative
Address/Phone/Email

Window



(Validator / Operations Administrator)



AAMA CERTIFICATION PROGRAM

AUTHORIZATION FOR PRODUCT CERTIFICATION

MI Windows & Doors, Inc.
P.O. Box 370
Gratz, PA 17030-0370

Attn: Bill Emley

The product described below is hereby approved for listing in the next issue of the AAMA Certified Products Directory. The approval is based on successful completion of tests, and the reporting to the Administrator of the results of tests, accompanied by related drawings, by an AAMA Accredited Laboratory.

- The listing below will be added to the next published AAMA Certified Products Directory.

SPECIFICATION	RECORD OF PRODUCT TESTED				LABEL ORDER NO.
AAMA/NWMDA 101/I.S. 2-97 H-R55"-36x52	CODE NO.	SERIES MODEL & PRODUCT DESCRIPTION	MAXIMUM SIZE TESTED		
COMPANY AND PLANT LOCATION					
MI Windows & Doors, Inc. (Didsmar, FL) MI Windows & Doors, Inc. (Smyrna, TN)	MTL-8 MTL-9	185/3185 SH (Fin) (AL)(O/D)(DG) (ASTM)	<u>FRAME</u> 3'0" x 5'2"	<u>SASH</u> 2'10" x 2'7"	By Request

- This Certification will expire May 14, 2008 and requires validation until then by continued listing in the current AAMA Certified Products Directory.
- Product Tested and Reported by: Architectural Testing, Inc.
Report No.: 01-50360.02
Date of Report: June 14, 2004

**NOTE: PLEASE REVIEW,
AND ADVISE ALI IMMEDIATELY
IF DATA, AS SHOWN, NEEDS
CORRECTION.**

Date: August 1, 2005

cc: AAMA
JGS/df
ACP-04 (Rev. 5/03)

Validated for Certification:



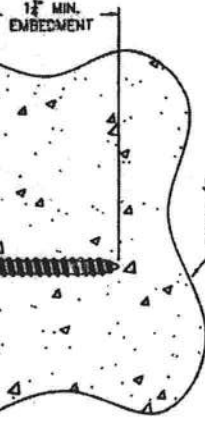
Associated Laboratories, Inc.

Authorized for Certification:

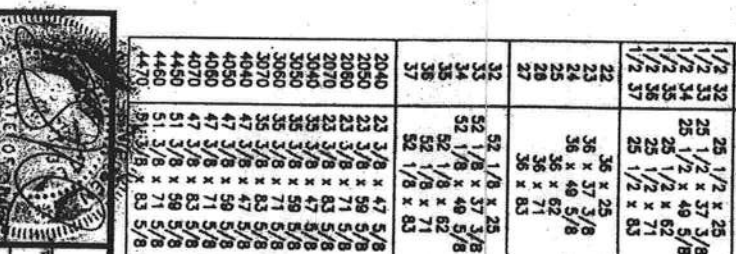
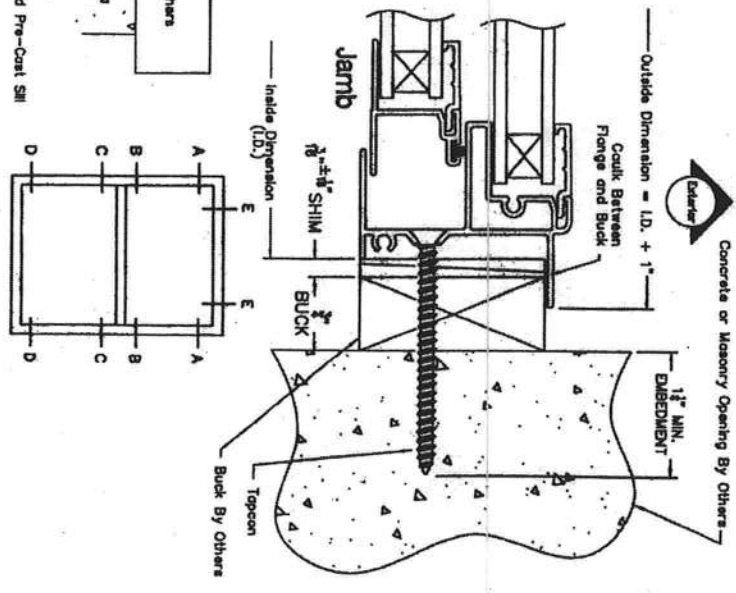
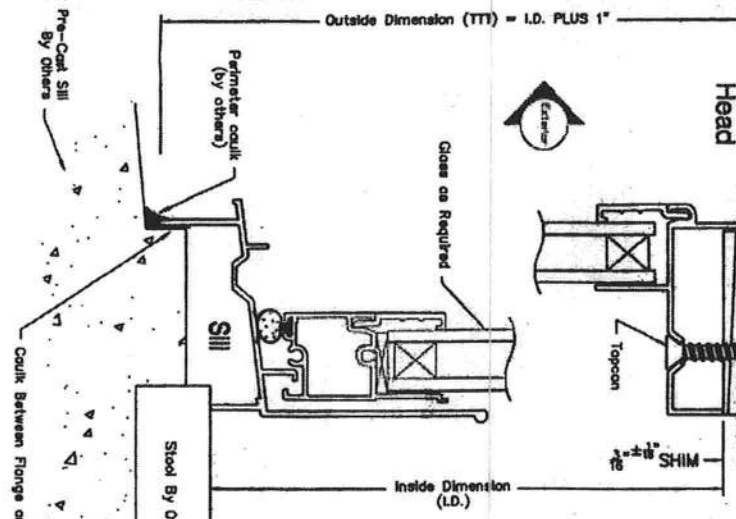


American Architectural Manufacturers Association

Concrete header (shown) or steel lintel By Others



- "ONE BY" (3/4) BUCKS (SHOWN)**
1. Before installation, caulk back of flange, or face of buck.
 2. 3/16" dia. masonry Tapcon must be of a length to have 1 1/4" embedment into masonry or concrete.
 3. Shim as required with load bearing shims at each installation anchor as shown.
 4. All factory applied holes not designated for Tapcon anchor should be filled with #10 screws of sufficient length to provide min. 5/8" embedment into wood buck.
 5. Letter designations on the Tapcon location chart indicate where anchors are to be installed using the elevation as a key.
 6. If exact window size is not given, use anchor quantity for next larger window in chart.
 7. For continuous head and sill twine & triples, use the same fastener schedule for each unit in the main frame except ignore the intermediate jamb.



"TWO BY" (1 1/2) BUCKS

"TWO BY" bucks are engineered and fastened to the masonry opening BY OTHERS.

Follow the same instructions and fastener requirements for "one by" bucks except use #10 screws of sufficient length for 1 1/4" minimum embedment into buck.

* TAPCON LOCATION CHART

CODE SIZE	WINDOW ID SIZE	FASTENER LOCATIONS			
		UP TO DRS	DRS. 1 TO DRS6	DRS6.1 TO DRS8.3	DRS8.3 TO DRS8.5
12	18 1/8 x 25	A D & E	A D & E	A D & E	A D & E
13	18 1/8 x 37 3/8	A D & E	A D & E	A D & E	A D & E
14	18 1/8 x 49 5/8	A D & E	A D & E	A D & E	A D & E
15	18 1/8 x 62	A D & E	A D & E	A D & E	A D & E
16	18 1/8 x 71	A D & E	A D & E	A D & E	A D & E
17	18 1/8 x 83	A D & E	A D & E	A D & E	A D & E
17 1/2	25 1/2 x 25	A D & E	A D & E	A D & E	A D & E
17 1/2	25 1/2 x 37 3/8	A D & E	A D & E	A D & E	A D & E
17 1/2	25 1/2 x 49 5/8	A D & E	A D & E	A D & E	A D & E
17 1/2	25 1/2 x 62	A D & E	A D & E	A D & E	A D & E
17 1/2	25 1/2 x 71	A D & E	A D & E	A D & E	A D & E
17 1/2	25 1/2 x 83	A D & E	A D & E	A D & E	A D & E
22	36 x 25	A D & E	A D & E	A D & E	A D & E
23	36 x 37 3/8	A D & E	A D & E	A D & E	A D & E
24	36 x 49 5/8	A D & E	A D & E	A D & E	A D & E
25	36 x 62	A D & E	A D & E	A D & E	A D & E
26	36 x 71	A D & E	A D & E	A D & E	A D & E
27	36 x 83	A D & E	A D & E	A D & E	A D & E
32	52 1/8 x 25	A D & E	A D & E	A D & E	A D & E
33	52 1/8 x 37 3/8	A D & E	A D & E	A D & E	A D & E
34	52 1/8 x 49 5/8	A D & E	A D & E	A D & E	A D & E
35	52 1/8 x 62	A D & E	A D & E	A D & E	A D & E
36	52 1/8 x 71	A D & E	A D & E	A D & E	A D & E
37	52 1/8 x 83	A D & E	A D & E	A D & E	A D & E
2040	23 3/8 x 47 5/8	A D & E	A D & E	A D & E	A D & E
2050	23 3/8 x 59 5/8	A D & E	A D & E	A D & E	A D & E
2060	23 3/8 x 71 5/8	A D & E	A D & E	A D & E	A D & E
2070	23 3/8 x 83 5/8	A D & E	A D & E	A D & E	A D & E
3040	35 3/8 x 47 5/8	A D & E	A D & E	A D & E	A D & E
3050	35 3/8 x 59 5/8	A D & E	A D & E	A D & E	A D & E
3060	35 3/8 x 71 5/8	A D & E	A D & E	A D & E	A D & E
3070	35 3/8 x 83 5/8	A D & E	A D & E	A D & E	A D & E
4040	47 3/8 x 47 5/8	A D & E	A D & E	A D & E	A D & E
4050	47 3/8 x 59 5/8	A D & E	A D & E	A D & E	A D & E
4070	47 3/8 x 71 5/8	A D & E	A D & E	A D & E	A D & E
4450	51 3/8 x 83 5/8	A D & E	A D & E	A D & E	A D & E
4460	51 3/8 x 95 5/8	A D & E	A D & E	A D & E	A D & E
4470	51 3/8 x 107 5/8	A D & E	A D & E	A D & E	A D & E

* "TAPCON" TYPE HARDENED MASONRY SCREWS INCLUDE TAPCON, RAWL, & SURPSON

A	REVISION	DATE	BY
1	REVISION	7/24/04	BT



MI HOME PRODUCTS
GRATZ, PA

185/5185 SINGLE HUNG FLANGE FRAME
INSTALLATION DETAILS & FASTENER SCHEDULE

DATE: 06/15/04
REV: N.T.S.
REV: A

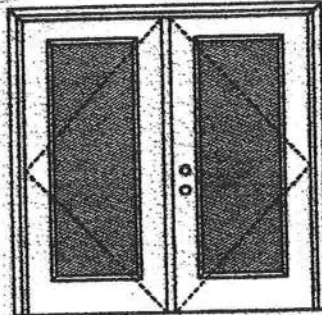
1 OF 1

XX
Glazed Outswing Unit

COP-WL-JH4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Minimum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series



822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 RL, 23 RL, 24 RL Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson
EntrySystems

March 29, 2002
Our continuing program of product improvement meets specifications, design and product detail subject to change without notice.

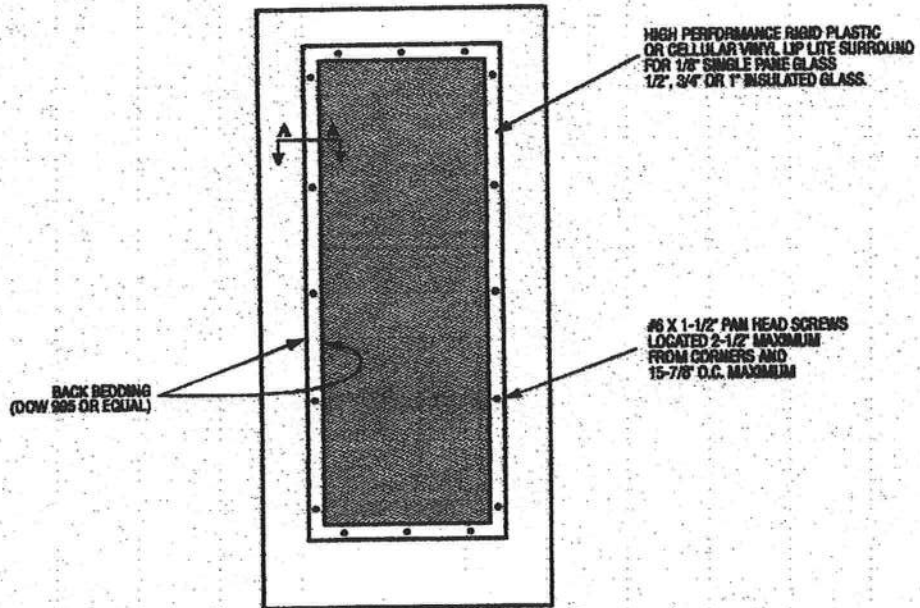


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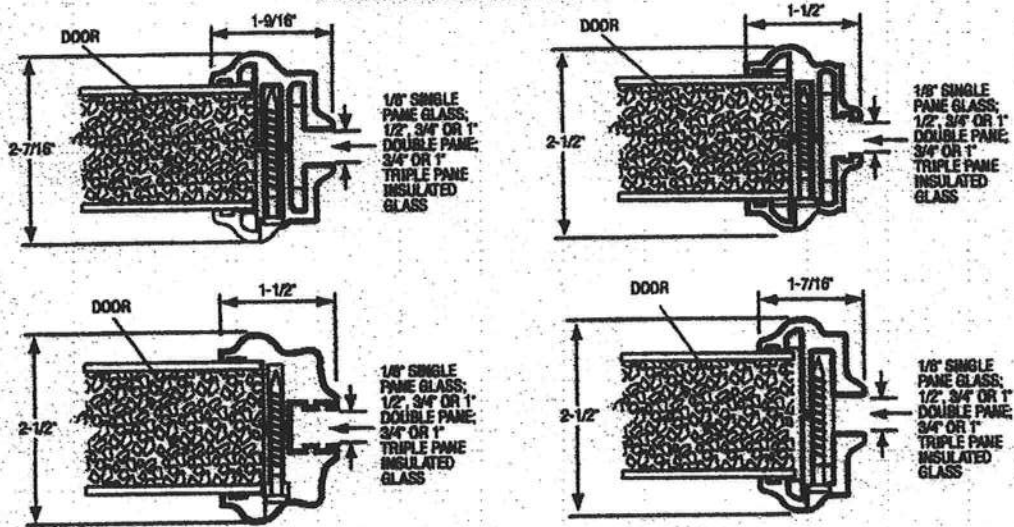
Masonite
Masonite International Corporation

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A TYPICAL RIGID PLASTIC LIP LITE SURROUND



March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



Exclusively from
Masonite
Masonite International Corporation

XX
Glazed Outswing Unit

COP-WL-JH4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:



404 Series

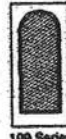


416 Series

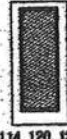


450 Series

FULL GLASS:



109 Series



114, 120, 122 Series



152 Series



149 Series



500 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1864-5, 6, 7, 8; NCTL 210-2178-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533

Johnson
EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



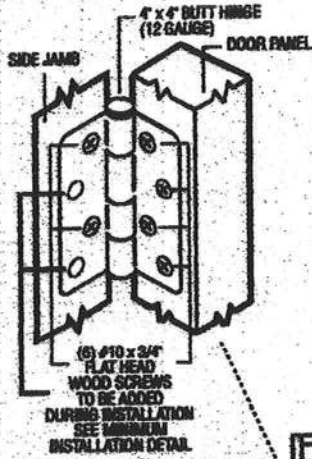
Exclusively from
Masonite
Masonite International Corporation

XX
Unit

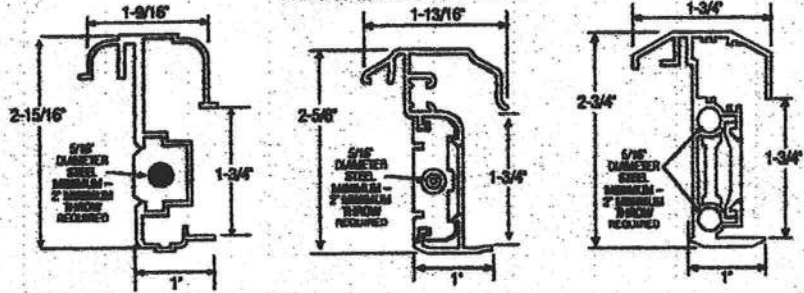
MAD-WL-MAD012-02

**OUTSWING UNITS WITH
DOUBLE DOOR**

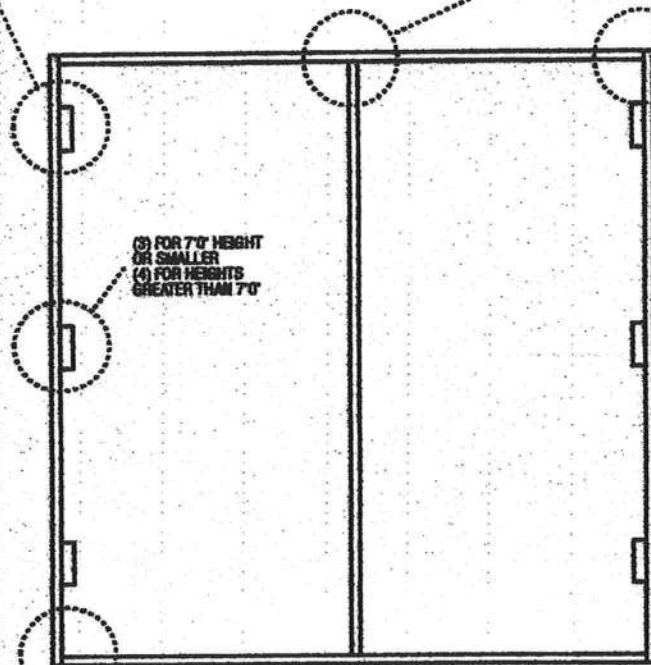
TYPICAL HINGE ATTACHMENT



TYPICAL ASTRAGAL PROFILES



ALUMINUM EXTRUDED ASTRAGAL (0.06\"/>



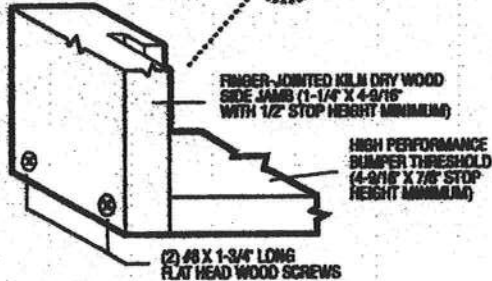
**TYPICAL HEADER &
SIDE JAMB ATTACHMENT**

FINGER-JOINTED KILN DRY WOOD
FRAME HEADER (1-1/4\"/>



FINGER-JOINTED
KILN DRY WOOD
SIDE JAMB
(1-1/4\"/>

**TYPICAL THRESHOLD &
SIDE JAMB ATTACHMENT**



March 29, 2002
Our continuing program of product improvement meets specifications,
design and product detail subject to change without notice.

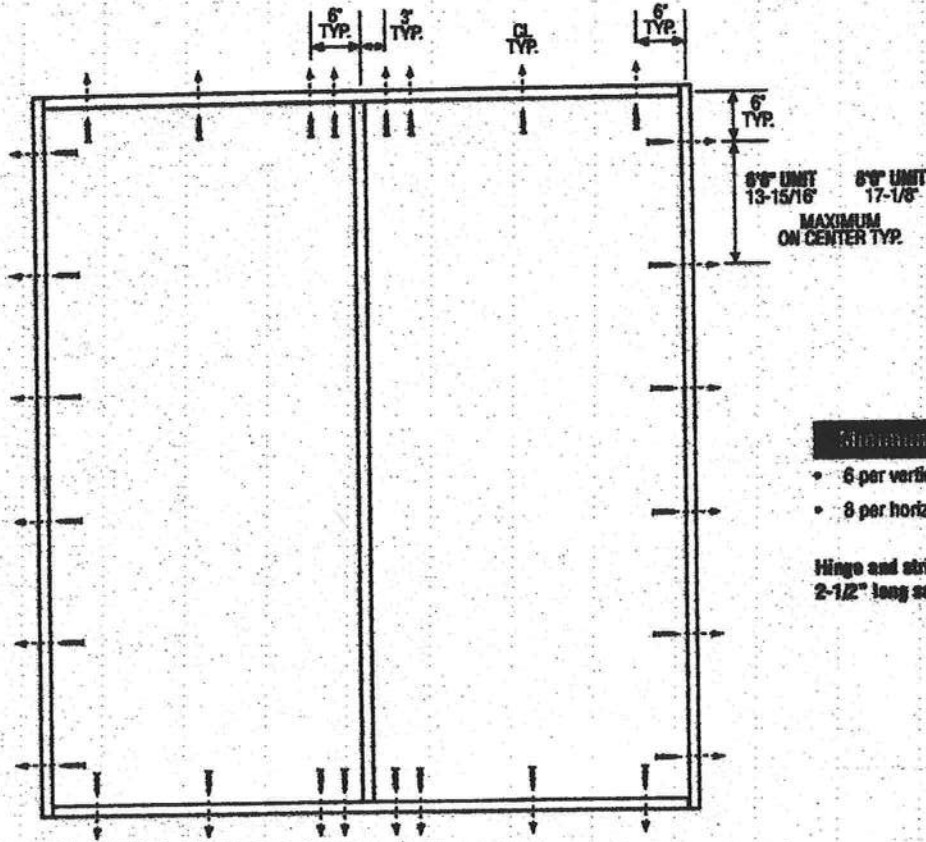


Exclusively from
Masonite
Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

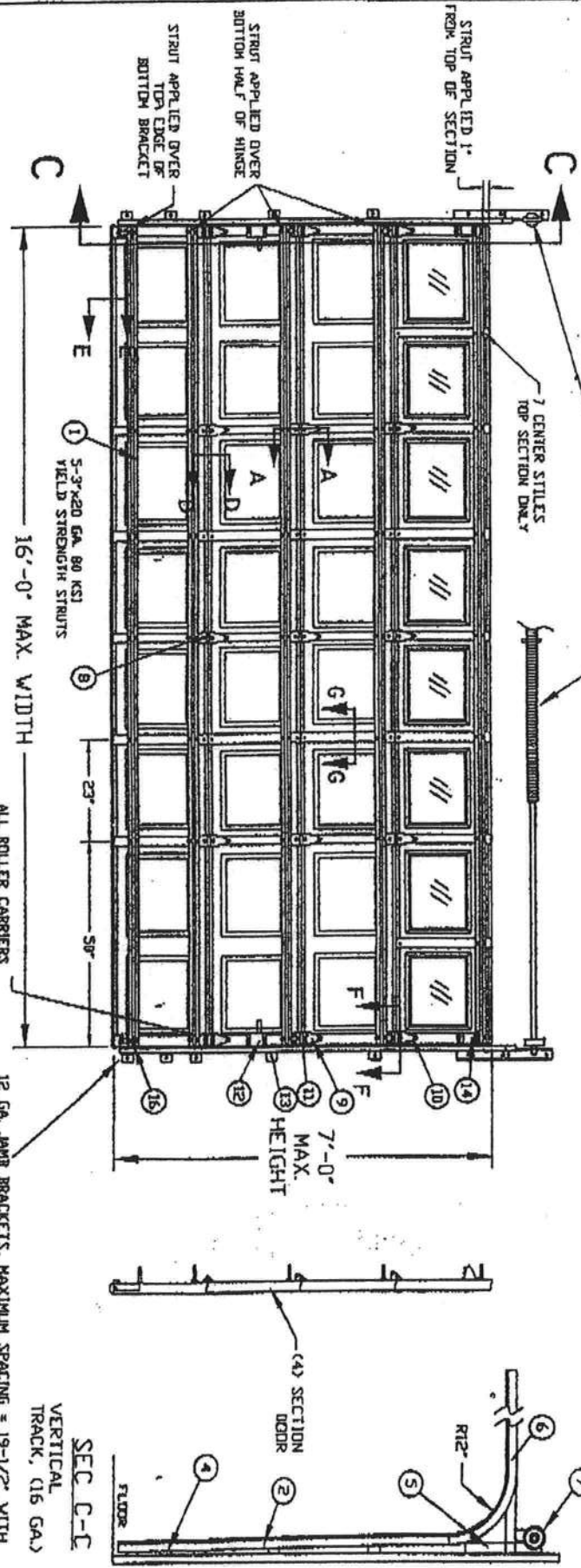
Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



- NOTES:**
1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-530
 2. MAXIMUM SECTION HEIGHT = 21'
 3. SECTION HEIGHTS OF 21'0" AND 19'6" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS BEAR HEIGHTS.
 4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION. GAS TESTED WITH 1/8" NSB GLASS OR EQUIVALENT OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
 5. MAXIMUM LENGTH OF ROLLER STEEL IS 5'-0" AS TESTED.
 6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOW.
 7. STRUTS SECURED AT ALL LOCATIONS WITH TEE SCREWS.
 8. QUANTITY OF SIDE LOCKS CAN BE O.L. OR 2 AS TESTED.
 9. DROP IN TYPE OF INSULATION IS OPTIONAL.



TEST REPORTS ON FILE VIDEO 10/19/00 002933

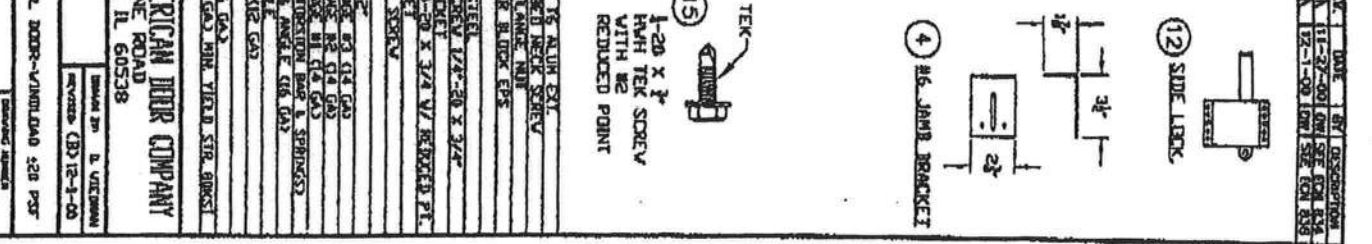
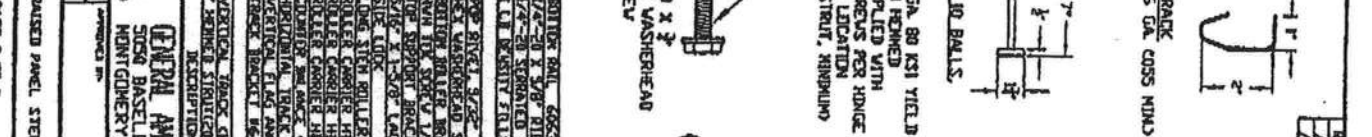
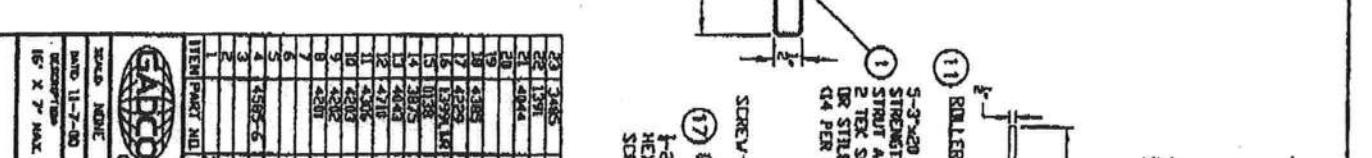
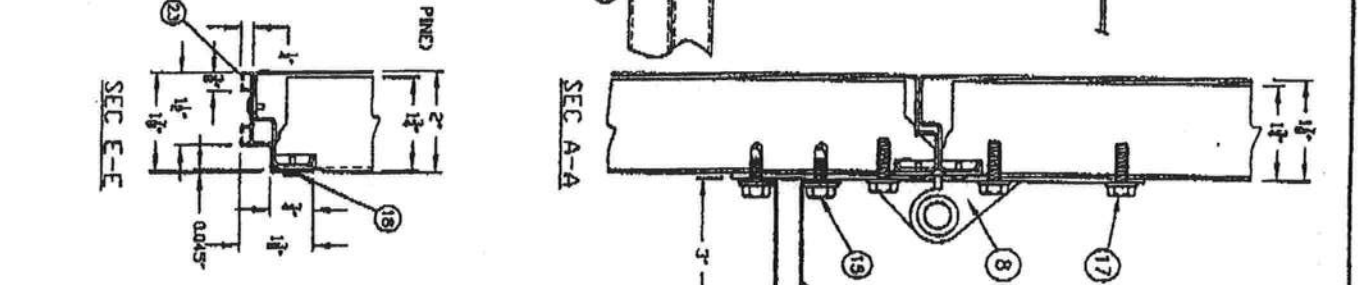
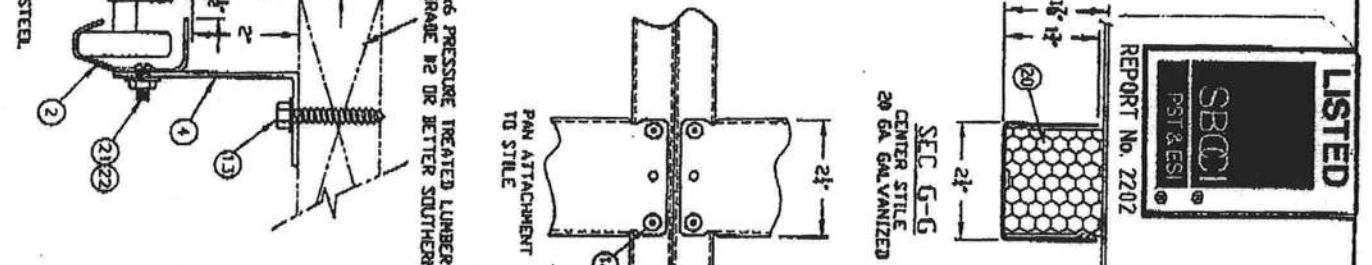
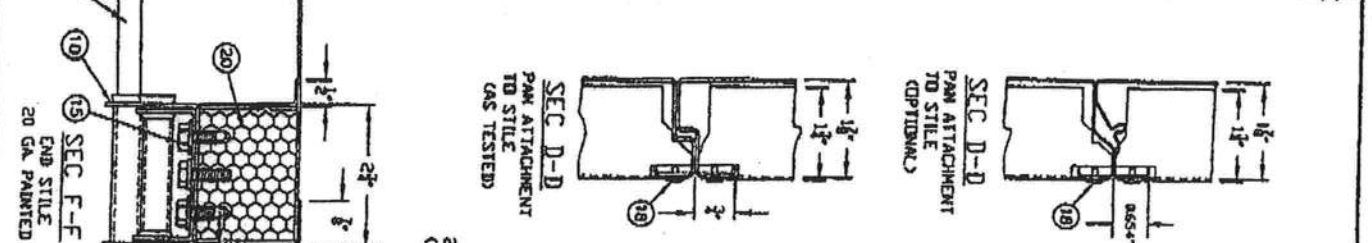
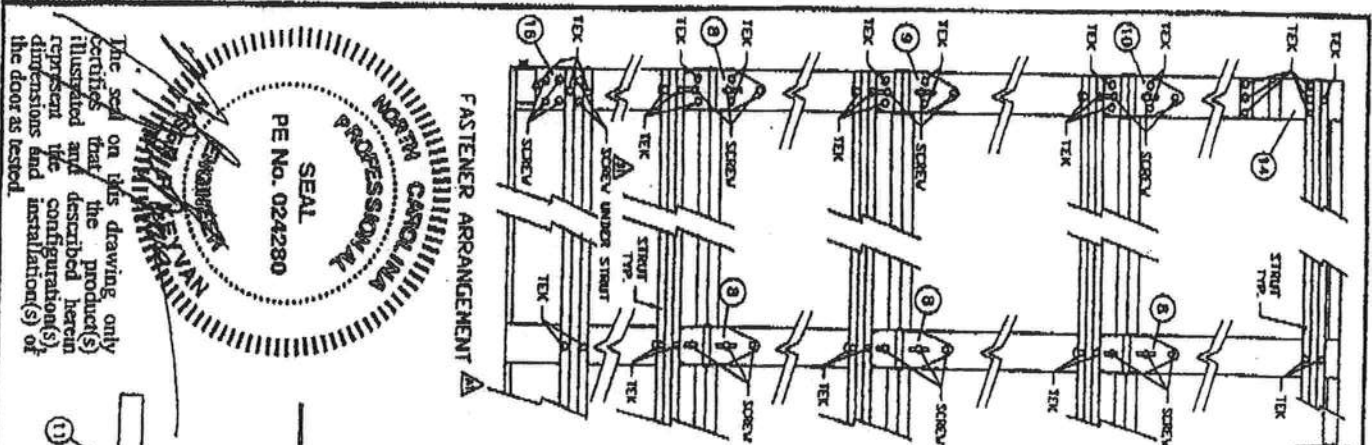
DESIGN LOAD +200 PSF & -200 PSF
TEST LOAD +300 PSF & -300 PSF

GADCO DOORS
SERIES 7400, EXTERIOR STEEL = .017 MIN GAS TESTED
SERIES 7825, EXTERIOR STEEL = .019" MIN A
SERIES 7824, EXTERIOR STEEL = .024" MIN A
(TESTED WITH WINDOWS)

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MIDMONTGOMERY, IL 60538

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 AST	VERTICAL TRACK
16'	7'	23"	3"	5
			VERT. TRACK	2 IN.

FORM 105
PART: 18-20-10
DRAWN BY: R. VICKHAM
REVIEWED: (A) 11-10-03
PAGE 1 OF 2
DRAWING NUMBER: V13220-1



NO.	QTY.	DESCRIPTION
1	1	20 GA. PAINTED STEEL
2	1	2x6 PRESSURE TREATED LUMBER GRADE #2 OR BETTER SOUTHERN PINE
3	1	2x6 PRESSURE TREATED LUMBER GRADE #2 OR BETTER SOUTHERN PINE
4	1	#6 JAMB BRACKET
5	1	5-3/8" DIA. 80 KSI YIELD STRENGTH HEADED STRUT APPLIED WITH 2 TEK SCREWS PER HINGE OR STILE LOCATION (14 PER STRUT, MINIMUM)
6	1	1-20 x 3/4 HEX WASHERHEAD SCREW
7	1	1-20 x 3/4 HEX TEK SCREW WITH #2 REDUCED POINT
8	1	1/4" X 3/8" RIBBED ROLLER BRACKET
9	1	1/4" X 3/8" RIBBED ROLLER BRACKET
10	1	1/4" X 3/8" RIBBED ROLLER BRACKET
11	1	1/4" X 3/8" RIBBED ROLLER BRACKET
12	1	1/4" X 3/8" RIBBED ROLLER BRACKET
13	1	1/4" X 3/8" RIBBED ROLLER BRACKET
14	1	1/4" X 3/8" RIBBED ROLLER BRACKET
15	1	1/4" X 3/8" RIBBED ROLLER BRACKET
16	1	1/4" X 3/8" RIBBED ROLLER BRACKET
17	1	1/4" X 3/8" RIBBED ROLLER BRACKET
18	1	1/4" X 3/8" RIBBED ROLLER BRACKET
19	1	1/4" X 3/8" RIBBED ROLLER BRACKET
20	1	1/4" X 3/8" RIBBED ROLLER BRACKET
21	1	1/4" X 3/8" RIBBED ROLLER BRACKET
22	1	1/4" X 3/8" RIBBED ROLLER BRACKET
23	1	1/4" X 3/8" RIBBED ROLLER BRACKET
24	1	1/4" X 3/8" RIBBED ROLLER BRACKET
25	1	1/4" X 3/8" RIBBED ROLLER BRACKET
26	1	1/4" X 3/8" RIBBED ROLLER BRACKET
27	1	1/4" X 3/8" RIBBED ROLLER BRACKET
28	1	1/4" X 3/8" RIBBED ROLLER BRACKET
29	1	1/4" X 3/8" RIBBED ROLLER BRACKET
30	1	1/4" X 3/8" RIBBED ROLLER BRACKET
31	1	1/4" X 3/8" RIBBED ROLLER BRACKET
32	1	1/4" X 3/8" RIBBED ROLLER BRACKET
33	1	1/4" X 3/8" RIBBED ROLLER BRACKET
34	1	1/4" X 3/8" RIBBED ROLLER BRACKET
35	1	1/4" X 3/8" RIBBED ROLLER BRACKET
36	1	1/4" X 3/8" RIBBED ROLLER BRACKET
37	1	1/4" X 3/8" RIBBED ROLLER BRACKET
38	1	1/4" X 3/8" RIBBED ROLLER BRACKET
39	1	1/4" X 3/8" RIBBED ROLLER BRACKET
40	1	1/4" X 3/8" RIBBED ROLLER BRACKET
41	1	1/4" X 3/8" RIBBED ROLLER BRACKET
42	1	1/4" X 3/8" RIBBED ROLLER BRACKET
43	1	1/4" X 3/8" RIBBED ROLLER BRACKET
44	1	1/4" X 3/8" RIBBED ROLLER BRACKET
45	1	1/4" X 3/8" RIBBED ROLLER BRACKET
46	1	1/4" X 3/8" RIBBED ROLLER BRACKET
47	1	1/4" X 3/8" RIBBED ROLLER BRACKET
48	1	1/4" X 3/8" RIBBED ROLLER BRACKET
49	1	1/4" X 3/8" RIBBED ROLLER BRACKET
50	1	1/4" X 3/8" RIBBED ROLLER BRACKET

SEAL
 PE No. 024280
 NORTH CAROLINA PROFESSIONAL SEAL
 THE SEAL ON THIS DRAWING ONLY CERTIFIES THAT THE PRODUCT(S) ILLUSTRATED AND DESCRIBED HEREIN CONFORMS TO THE CONFIGURATION(S) OF THE DOOR AS TESTED.

LISTED
 SBC01
 PST & ESI
 REPORT No. 2202

GENERAL AMERICAN DOOR COMPANY
 5000 BASSEL LINE ROAD
 MONTGOMERY, IL 60538

DATE: 11-27-00
 BY: [Signature]
 CHECKED: [Signature]
 APPROVED: [Signature]

16" X 7" MAX. GAUDED PANEL STEEL DOOR-VINYL/SLD 228 PST

REVISIONS: V132820-2

PAGE 2 OF 2

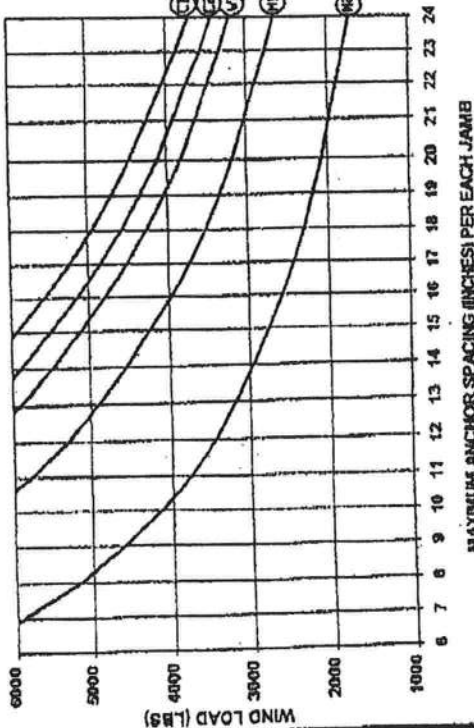
2X6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2X6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

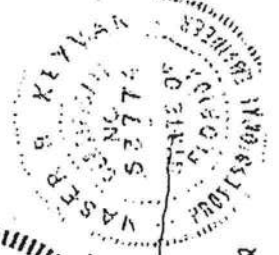
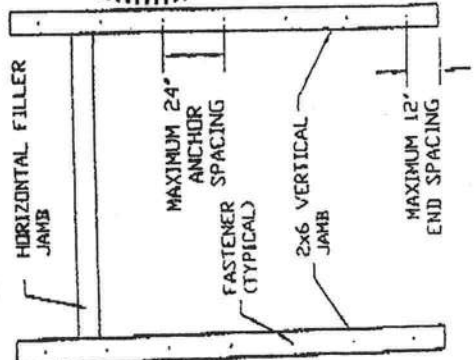
- 1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.
- 2) ALL DOOR OPENING STRUCTURE AND STANDERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10, CURRENT EDITION.
- 3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.
- 4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE (#2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.
- 5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2500 PSI GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
- 6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.
- 7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"
- 8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.
- 9) WASHERS ARE REQUIRED ON ALL FASTENERS.
- 10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.
- 11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

WIND LOAD VS ANCHOR SPACING



DESIGN (LBS) X GARAGE DOOR AREA (WIDTH-FT X HEIGHT-FT) = WIND LOAD (LBS)
LOAD / FT²

EXAMPLE
30 LBS / FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS
 (M1) USE 22" SPACING
 (M2) USE 21" SPACING
 (M3) USE 19" SPACING
 SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



GENERAL AMERICAN DOOR COMPANY 5050 BASELINE ROAD MONTGOMERY, IL 60538	
APPROVED BY:	REVIEW BY: DV
DATE: 8-30-99	REVISED:
JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS	
PROJECT NUMBER:	ISSUE NUMBER:
	PA0560

Residential System Sizing Calculation

Summary

Cady Residence
Lake City, FL 32024-

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

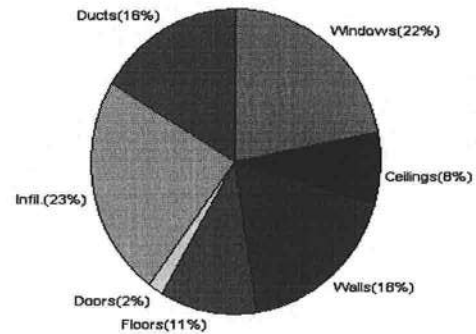
1/22/2008

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)			
Winter design temperature	33 F	Summer design temperature	92 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	37 F	Summer temperature difference	17 F
Total heating load calculation	26101 Btuh	Total cooling load calculation	42527 Btuh
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	118.8 31000	Sensible (SHR = 0.75)	74.3 23250
Heat Pump + Auxiliary(0.0kW)	118.8 31000	Latent	69.0 7750
		Total (Electric Heat Pump)	72.9 31000

WINTER CALCULATIONS

Winter Heating Load (for 1534 sqft)

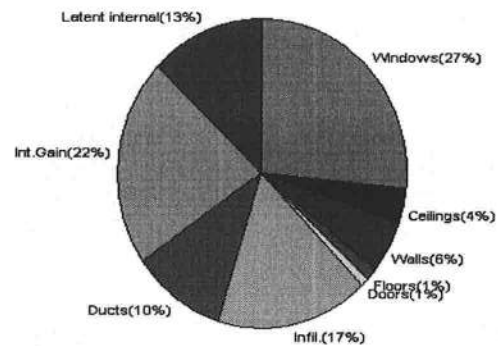
Load component		Load	
Window total	178 sqft	5730	Btuh
Wall total	1580 sqft	4641	Btuh
Door total	36 sqft	466	Btuh
Ceiling total	1742 sqft	2053	Btuh
Floor total	See detail report	2889	Btuh
Infiltration	149 cfm	6030	Btuh
Duct loss		4293	Btuh
Subtotal		26101	Btuh
Ventilation	0 cfm	0	Btuh
TOTAL HEAT LOSS		26101	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1534 sqft)

Load component		Load	
Window total	178 sqft	11383	Btuh
Wall total	1580 sqft	2371	Btuh
Door total	36 sqft	353	Btuh
Ceiling total	1742 sqft	1498	Btuh
Floor total		491	Btuh
Infiltration	130 cfm	2424	Btuh
Internal gain		9410	Btuh
Duct gain		3365	Btuh
Sens. Ventilation	0 cfm	0	Btuh
Total sensible gain		31295	Btuh
Latent gain(ducts)		1072	Btuh
Latent gain(infiltration)		4760	Btuh
Latent gain(ventilation)		0	Btuh
Latent gain(internal/occupants/other)		5400	Btuh
Total latent gain		11232	Btuh
TOTAL HEAT GAIN		42527	Btuh



Version 8
For Florida residences only

EnergyGauge® System Sizing

PREPARED BY: [Signature]

DATE: 1-22-08

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Cady Residence

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/22/2008

Component Loads for Whole House						
Window	Panes/SHGC/Frame/U	Orientation	Area(sqft)	X	HTM=	Load
1	2, Clear, Metal, 0.87	NE	15.0		32.2	483 Btuh
2	2, Clear, Metal, 0.87	E	20.0		32.2	644 Btuh
3	2, Clear, Metal, 0.87	SE	15.0		32.2	483 Btuh
4	2, Clear, Metal, 0.87	S	20.0		32.2	644 Btuh
5	2, Clear, Metal, 0.87	E	48.0		32.2	1545 Btuh
6	2, Clear, Metal, 0.87	S	24.0		32.2	773 Btuh
7	2, Clear, Metal, 0.87	W	36.0		32.2	1159 Btuh
	Window Total		178(sqft)			5730 Btuh
Walls	Type	R-Value	Area	X	HTM=	Load
1	Frame - Wood - Ext(0.08)	19.0	546		2.9	1561 Btuh
2	Frame - Wood - Adj(0.09)	13.0	290		3.3	952 Btuh
3	Frame - Wood - Ext(0.08)	19.0	744		2.9	2127 Btuh
	Wall Total		1580			4641 Btuh
Doors	Type		Area	X	HTM=	Load
1	Insulated - Adjacent		18		12.9	233 Btuh
2	Insulated - Adjacent		18		12.9	233 Btuh
	Door Total		36			466Btuh
Ceilings	Type/Color/Surface	R-Value	Area	X	HTM=	Load
1	Vented Attic/L/Tile	30.0	1350		1.2	1591 Btuh
2	Vented Attic/L/Tile	30.0	392		1.2	462 Btuh
	Ceiling Total		1742			2053Btuh
Floors	Type	R-Value	Size	X	HTM=	Load
1	Slab On Grade	5	84.0 ft(p)		16.4	1374 Btuh
2	Raised Wood - Adj	30	1142.0 sqft		1.3	1515 Btuh
	Floor Total		1226			2889 Btuh
			Envelope Subtotal:			15778 Btuh
Infiltration	Type	ACH X	Volume(cuft)	walls(sqft)	CFM=	
	Natural	0.80	11164	1580	148.9	6030 Btuh
Ductload					(DLM of 0.197)	4293 Btuh
All Zones			Sensible Subtotal All Zones			26101 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Cady Residence
Lake City, FL 32024-

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

1/22/2008

WHOLE HOUSE TOTALS

	Subtotal Sensible	26101 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	26101 Btuh

EQUIPMENT

1. Electric Heat Pump	#	31000 Btuh
-----------------------	---	------------

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8
For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Cady Residence
Lake City, FL 32024-

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/22/2008

Component Loads for Zone #1: Main

Window	Panels/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NE	15.0	32.2	483 Btuh
2	2, Clear, Metal, 0.87	E	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	SE	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
			Window Total	70(sqft)	2253 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.08)	19.0	546	2.9	1561 Btuh
2	Frame - Wood - Adj(0.09)	13.0	290	3.3	952 Btuh
			Wall Total		2513 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		18	12.9	233 Btuh
			Door Total		233Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
2	Vented Attic/L/Tile	30.0	392	1.2	462 Btuh
			Ceiling Total		462Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	84.0 ft(p)	16.4	1374 Btuh
2	Raised Wood - Adj	30	1142.0 sqft	1.3	1515 Btuh
			Floor Total		2889 Btuh
Zone Envelope Subtotal:					8351 Btuh
Infiltration	Type	ACH X Volume(cuft) walls(sqft)	CFM=		
	Natural	0.80 4312 836	78.8	3190 Btuh	
Ductload	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic) (DLM of 0.197)				2272 Btuh
Zone #1	Sensible Zone Subtotal				13813 Btuh

Component Loads for Zone #2: 2nd Floor

Window	Panels/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
5	2, Clear, Metal, 0.87	E	48.0	32.2	1545 Btuh
6	2, Clear, Metal, 0.87	S	24.0	32.2	773 Btuh
7	2, Clear, Metal, 0.87	W	36.0	32.2	1159 Btuh
			Window Total	108(sqft)	3477 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
3	Frame - Wood - Ext(0.08)	19.0	744	2.9	2127 Btuh
			Wall Total		2127 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Cady Residence

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

1/22/2008

Doors 2	Type Insulated - Adjacent Door Total	Area X 18 18	HTM= 12.9	Load 233 Btuh 233Btuh
Ceilings 1	Type/Color/Surface Vented Attic/L/Tile Ceiling Total	R-Value 30.0	Area X 1350 1350	HTM= 1.2 Load 1591 Btuh 1591Btuh
Zone Envelope Subtotal:				7428 Btuh
Infiltration	Type Natural	ACH X Volume(cuft) walls(sqft)	CFM= 70.1	2839 Btuh
Ductload	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic) (DLM of 0.197)			2021 Btuh
Zone #2	Sensible Zone Subtotal			12288 Btuh

WHOLE HOUSE TOTALS

	Subtotal Sensible	26101 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	26101 Btuh

EQUIPMENT

1. Electric Heat Pump	#	31000 Btuh
-----------------------	---	------------

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



Version 8
For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Cady Residence

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

1/22/2008

Component Loads for Whole House											
Window	Type*		Overhang		Window Area(sqft)			HTM		Load	
	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	NE	1.5ft	11ft.	15.0	0.0	15.0	29	60	901	Btuh
2	2, Clear, 0.87, None,N,N	E	1.5ft	11ft.	20.0	0.0	20.0	29	80	1590	Btuh
3	2, Clear, 0.87, None,N,N	SE	1.5ft	11ft.	15.0	0.0	15.0	29	63	938	Btuh
4	2, Clear, 0.87, None,N,N	S	8.5ft	10ft.	20.0	20.0	0.0	29	34	579	Btuh
5	2, Clear, 0.87, None,N,N	E	1.5ft	6ft.	48.0	0.0	48.0	29	80	3817	Btuh
6	2, Clear, 0.87, None,N,N	S	1.5ft	6ft.	24.0	24.0	0.0	29	34	695	Btuh
7	2, Clear, 0.87, None,N,N	W	1.5ft	6ft.	36.0	0.0	36.0	29	80	2863	Btuh
Window Total					178 (sqft)					11383 Btuh	
Walls	Type	R-Value/U-Value		Area(sqft)		HTM		Load			
1	Frame - Wood - Ext	19.0/0.08		546.0		1.5		819 Btuh			
2	Frame - Wood - Adj	13.0/0.09		290.0		1.5		438 Btuh			
3	Frame - Wood - Ext	19.0/0.08		744.0		1.5		1115 Btuh			
Wall Total					1580 (sqft)					2371 Btuh	
Doors	Type	R-Value/U-Value		Area (sqft)		HTM		Load			
1	Insulated - Adjacent			18.0		9.8		176 Btuh			
2	Insulated - Adjacent			18.0		9.8		176 Btuh			
Door Total					36 (sqft)					353 Btuh	
Ceilings	Type/Color/Surface	R-Value		Area(sqft)		HTM		Load			
1	Vented Attic/Light/Tile	30.0		1350.0		0.9		1161 Btuh			
2	Vented Attic/Light/Tile	30.0		392.0		0.9		337 Btuh			
Ceiling Total					1742 (sqft)					1498 Btuh	
Floors	Type	R-Value		Size		HTM		Load			
1	Slab On Grade	5.0		84 (ft(p))		0.0		0 Btuh			
2	Raised Wood - Adj	30.0		1142 (sqft)		0.4		491 Btuh			
Floor Total					1226.0 (sqft)					491 Btuh	
Envelope Subtotal:										16096 Btuh	
Infiltration	Type	ACH	Volume(cuft) wall area(sqft)		CFM=		Load				
	SensibleNatural	0.70	11164	1580	148.9		2424 Btuh				
Internal gain	Occupants	Btuh/occupant		Appliance		Load					
	27	X	230	+	3200		9410 Btuh				
Sensible Envelope Load:										27930 Btuh	
(DGM of 0.120)										3365 Btuh	
Sensible Load All Zones										31295 Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Cady Residence

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

1/22/2008

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	27930 Btuh
	Sensible Duct Load	3365 Btuh
	Total Sensible Zone Loads	31295 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	31295 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	4760 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1072 Btuh
	Latent occupant gain (27 people @ 200 Btuh per person)	5400 Btuh
	Latent other gain	0 Btuh
	Latent total gain	11232 Btuh
	TOTAL GAIN	42527 Btuh

EQUIPMENT

1. Central Unit	#	31000 Btuh
-----------------	---	------------

*Key: Window types (Pn - Number of panes of glass)
 (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
 (U - Window U-Factor or 'DEF' for default)
 (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
 (ExSh - Exterior shading device: none(N) or numerical value)
 (BS - Insect screen: none(N), Full(F) or Half(H))
 (Ornt - compass orientation)



Version 8
For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Cady Residence

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

1/22/2008

Component Loads for Zone #1: Main											
Window	Type*			Overhang		Window Area(sqft)			HTM		Load
	Pn/SHGC/U/InSh/ExSh/IS	Ornt		Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
1	2, Clear, 0.87, None,N,N	NE		1.5ft	11ft.	15.0	0.0	15.0	29	60	901 Btuh
2	2, Clear, 0.87, None,N,N	E		1.5ft	11ft.	20.0	0.0	20.0	29	80	1590 Btuh
3	2, Clear, 0.87, None,N,N	SE		1.5ft	11ft.	15.0	0.0	15.0	29	63	938 Btuh
4	2, Clear, 0.87, None,N,N	S		8.5ft	10ft.	20.0	20.0	0.0	29	34	579 Btuh
Window Total						70 (sqft)					4008 Btuh
Walls	Type	R-Value/U-Value		Area(sqft)			HTM		Load		
1	Frame - Wood - Ext	19.0/0.08		546.0			1.5		819 Btuh		
2	Frame - Wood - Adj	13.0/0.09		290.0			1.5		438 Btuh		
Wall Total				836 (sqft)					1256 Btuh		
Doors	Type	Area (sqft)			HTM		Load				
1	Insulated - Adjacent	18.0			9.8		176 Btuh				
Door Total		18 (sqft)					176 Btuh				
Ceilings	Type/Color/Surface	R-Value		Area(sqft)			HTM		Load		
2	Vented Attic/Light/Tile	30.0		392.0			0.9		337 Btuh		
Ceiling Total				392 (sqft)					337 Btuh		
Floors	Type	R-Value		Size			HTM		Load		
1	Slab On Grade	5.0		84 (ft(p))			0.0		0 Btuh		
2	Raised Wood - Adj	30.0		1142 (sqft)			0.4		491 Btuh		
Floor Total				1226.0 (sqft)					491 Btuh		
Zone Envelope Subtotal:										6269 Btuh	
Infiltration	Type	ACH		Volume(cuft) wall area(sqft)		CFM=		Load			
	SensibleNatural	0.70		4312 836		68.9		1283 Btuh			
Internal gain	Occupants	Btuh/occupant			Appliance		Load				
	2	X 230 +			800		1260 Btuh				
Sensible Envelope Load:										8812 Btuh	
Duct load	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic) (DGM of 0.120)								1062 Btuh		
Sensible Zone Load										9873 Btuh	

Component Loads for Zone #2: 2nd Floor											
Window	Type*			Overhang		Window Area(sqft)			HTM		Load
	Pn/SHGC/U/InSh/ExSh/IS	Ornt		Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded	
5	2, Clear, 0.87, None,N,N	E		1.5ft	6ft.	48.0	0.0	48.0	29	80	3817 Btuh
6	2, Clear, 0.87, None,N,N	S		1.5ft	6ft.	24.0	24.0	0.0	29	34	695 Btuh
7	2, Clear, 0.87, None,N,N	W		1.5ft	6ft.	36.0	0.0	36.0	29	80	2863 Btuh
Window Total						108 (sqft)					7374 Btuh
Walls	Type	R-Value/U-Value		Area(sqft)			HTM		Load		
3	Frame - Wood - Ext	19.0/0.08		744.0			1.5		1115 Btuh		
Wall Total				744 (sqft)					1115 Btuh		
Doors	Type	Area (sqft)			HTM		Load				
2	Insulated - Adjacent	18.0			9.8		176 Btuh				
Door Total		18 (sqft)					176 Btuh				

Manual J Summer Calculations

Residential Load - Component Details (continued)

Cady Residence

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

1/22/2008

Ceilings 1	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
	Vented Attic/Light/Tile	30.0	1350.0	0.9	1161 Btuh
	Ceiling Total		1350 (sqft)		1161 Btuh
	Zone Envelope Subtotal:				9827 Btuh
Infiltration	Type	ACH	Volume(cuft)	wall area(sqft)	CFM=
	SensibleNatural	0.70	6852	744	61.3
Internal gain		Occupants	Btuh/occupant		Appliance
		25	X	230	+
				2400	Load 8150 Btuh
	Sensible Envelope Load:				19118 Btuh
Duct load	Average sealed, Supply(R6.0-Attic), Return(R6.0-Attic)			(DGM of 0.120)	2303 Btuh
	Sensible Zone Load				21422 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Cady Residence

Project Title:
Mike & Linda Cady

Code Only
Professional Version
Climate: North

Lake City, FL 32024-

1/22/2008

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	27930 Btuh
	Sensible Duct Load	3365 Btuh
	Total Sensible Zone Loads	31295 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	31295 Btuh
	Latent infiltration gain (for 54 gr. humidity difference)	4760 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1072 Btuh
	Latent occupant gain (27 people @ 200 Btuh per person)	5400 Btuh
	Latent other gain	0 Btuh
	Latent total gain	11232 Btuh
	TOTAL GAIN	42527 Btuh

EQUIPMENT

1. Central Unit	#	31000 Btuh
-----------------	---	------------

*Key: Window types (Pn - Number of panes of glass)
 (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
 (U - Window U-Factor or 'DEF' for default)
 (InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
 (ExSh - Exterior shading device: none(N) or numerical value)
 (BS - Insect screen: none(N), Full(F) or Half(H))
 (Ornt - compass orientation)



Version 8
For Florida residences only

Residential Window Diversity

MidSummer

Cady Residence
Lake City, FL 32024-

Project Title:
Mike & Linda Cady

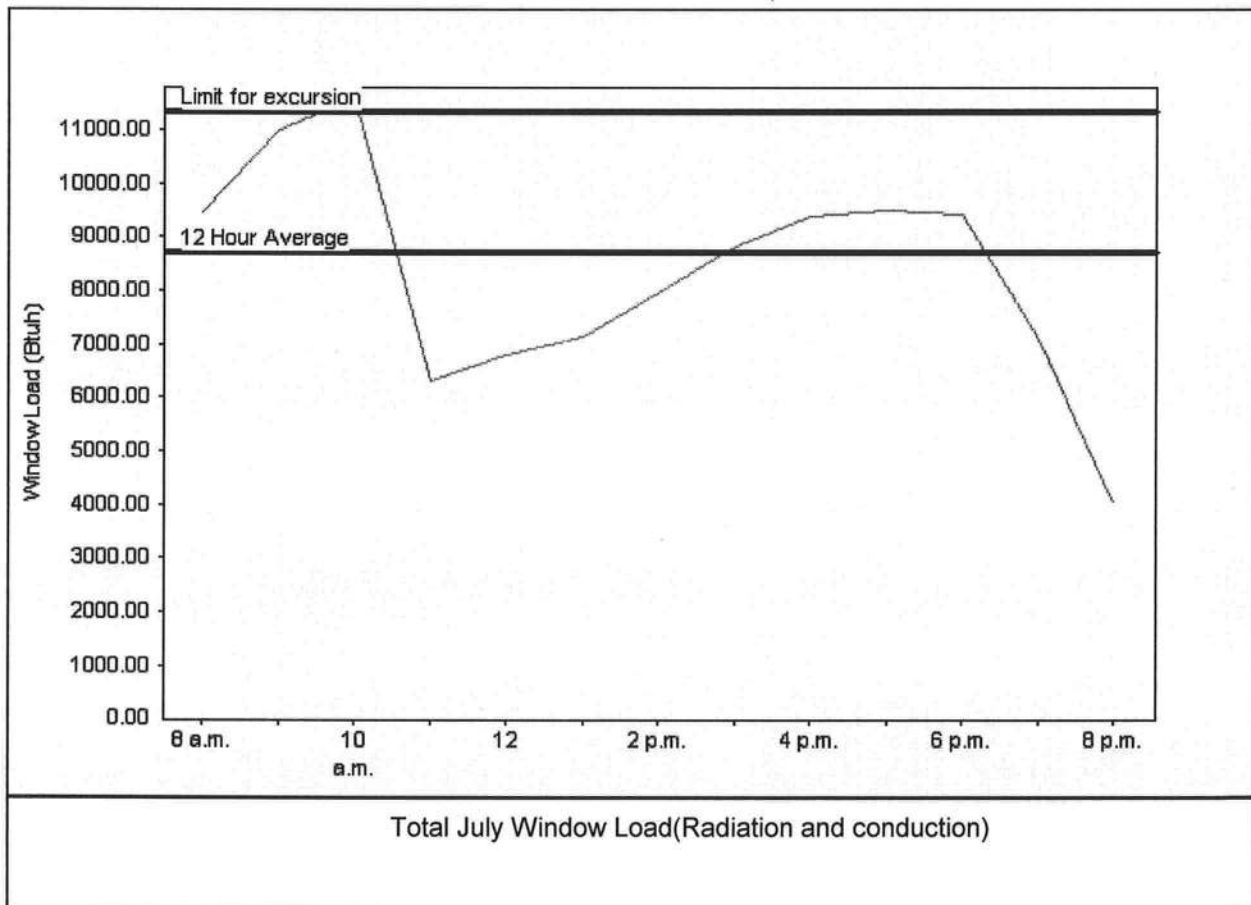
Code Only
Professional Version
Climate: North

1/22/2008

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	8712 Btuh
Summer setpoint	75 F	Peak window load for July	11609 Btu
Summer temperature difference	17 F	Excursion limit(130% of Ave.)	11325 Btu
Latitude	29 North	Window excursion (July)	284 Btuh

WINDOW Average and Peak Loads



Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A zoned system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only

PREPARED BY: _____

DATE: _____



Return to (enclose self-addressed stamped envelope)

Name: **Tri-County Title Services
of Lake City, Inc.**
Address: **229 N. Hernando Street
Lake City, Florida 32055**

This instrument prepared by
FURNISH TO THE ISSUANCE OF TITLE INSURANCE
Tri-County Title Services
Address: **of Lake City, Inc.
229 N. Hernando Street
Lake City, Florida 32055**

Property Appraisers Parcel Identification (Folio) Number(s):
36-48-16-0332-009 & 01-58-16-0335-005
Grantee(s) S.S. # (a): **MICHAEL L. CADY 503-54-9709
LINDA M. CADY 503-54-9709**

WARRANTY DEED
INDIVID. TO INDIVID.

RAMCO FORM 01

Complains Paper & Printing Co., Inc. 1987

91-12644

1991 SEP 12 2 43

R. Scippio

SPACE ABOVE THIS LINE FOR PROCESSING DATA

SPACE ABOVE THIS LINE FOR RECORDING DATA

This Warranty Deed Made the **4th** day of **September** A.D. 1991 by

ROBERT A. BERMAN
hereinafter called the grantor, to
MICHAEL L. CADY and his wife, **LINDA M. CADY**
whose post office address is **P.O. Box 1477, Lake City, Florida, 32056**

hereinafter called the grantees:

(Whoever used herein the terms "grantor" and "grantee" include all the parties to this instrument; and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth: That the grantor, for and in consideration of the sum of \$ **10.00** and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee all that certain land situate in **Columbia County, State of Florida**, viz:

SEE SCHEDULE "A" ATTACHED HERETO AND BY THIS REFERENCE MADE A PART HEREOF.

THE PROPERTY DESCRIBED IN SCHEDULE "A" HEREOF IS NOT THE HOMESTEAD OF THE GRANTOR HEREIN, WHO IN FACT RESIDES AT 1709 St. Andrews Road, Hollywood Florida 33021.

PK 0750 PG 0743

Together, with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantees that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land, and hereby warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 19 90.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in the presence of:

Robert A. Berman
Signature

MARTIN BERMAN
Printed Signature

Angela Tortorella
Signature

ANGELA TORTORELLA
Printed Signature

Signature

Printed Signature

Signature

Printed Signature

STATE OF FLORIDA
COUNTY OF **FLORIDA**

I HEREBY CERTIFY that on this day, before me, an officer duly authorized in the State aforesaid and in the County aforesaid to take acknowledgments, personally appeared

ROBERT A. BERMAN

to me known to be the person described in and who executed the foregoing instrument and he acknowledged before me that he executed the same.
WITNESS my hand and official seal in the County and State last aforesaid this **4th** day of **September**, A.D. 1991.

Robert A. Berman
Signature

ROBERT A. BERMAN
Printed Signature

1709 St. Andrews Road, Hollywood, Florida 33021
Post Office Address

Signature

Printed Signature

Post Office Address

DOCUMENTARY STAMP 210.00
INTANGIBLE TAX - 0
P. DEWITT CASON, CLERK OF
COURT, COLUMBIA COUNTY
R. Scippio

Linda Baker
Notary Signature

LINDA BAKER
Printed Notary Signature

My Commission Expires:

NOTARY PUBLIC STATE OF FLORIDA
MY COMMISSION EXP. MAR. 16, 1994
RECORDED THRU GENERAL INS. UND.

SEAL

TOGETHER WITH an easement for ingress and egress described as follows: BEGIN at the Southeast corner of Lot 6, Unit 4, SOUTHWOOD ACRES, a subdivision according to a plat thereof recorded in Plat Book 4, Page 7 of the Public Records of Columbia County, Florida and run thence S 0°08'06" E, 45 feet, thence N 89°20'07" E, 94.55 feet, thence S 0°08'06" E, 249.61 feet, thence N 89°20'07" E, 123.41 feet to the West line of the above described lands, thence N 0°08'06" W along said West line, 30.00 feet to the Northwest corner of said described lands, thence S 89°20'07" W, 93.41 feet, thence N 0°08'06" W, 264.61 feet to the South line of said Unit 4, SOUTHWOOD ACRES, thence S 89°20'07" W along said South line, 124.55 feet to the POINT OF BEGINNING.

PARCEL NO. 4

Commence at the Southeast corner of Section 36, Township 4 South, Range 16 East, Columbia County, Florida and run thence N 0°14'34" E along the East line of said Section 36, 37.54 feet, thence S 89°20'07" W, 220.15 feet to the POINT OF BEGINNING, thence continue S 89°20'07" W, 219.43 feet, thence S 0°08'06" E, 398.19 feet, thence N 88°51'13" E, 219.46 feet, thence N 0°08'06" W, 396.34 feet to the POINT OF BEGINNING. Said lands being a part of the SE¼ of SE¼ of Section 36 and the NE¼ of NE¼ of Section 1, Township 5 South, Range 16 East. The North 30 feet of said lands being subject to an easement for ingress and egress.

TOGETHER WITH an easement for ingress and egress described as follows: BEGIN at the Southeast corner of Lot 6, Unit 4, SOUTHWOOD ACRES, a subdivision according to plat thereof recorded in Plat Book 4, Page 7 of the Public Records of Columbia County, Florida and run thence S 0°08'06" E, 45 feet, thence N 89°20'07" E, 94.55 feet, thence S 0°08'06" E, 219.61 feet to the North line of the above described lands, thence N 89°20'07" E along said North line, 30.00 feet, thence N 0°08'06" W, 264.61 feet to the South line of said Unit 4, SOUTHWOOD ACRES, thence S 89°20'07" W along said South line, 124.55 feet to the POINT OF BEGINNING.

PARCEL NO. 5

Commence at the Southeast corner of Section 36, Township 4 South, Range 16 East, Columbia County, Florida and run thence N 0°14'34" E along the East line of said Section 36, 37.54 feet, thence S 89°20'07" W, 439.58 feet to the POINT OF BEGINNING, thence continue S 89°20'07" W, 218.42 feet, thence S 0°08'06" E, 400.02 feet, thence N 88°51'13" E, 218.44 feet, thence N 0°08'06" W, 398.19 feet to the POINT OF BEGINNING. Said lands being a part of the SE¼ of SE¼ of Section 36 and the NE¼ of NE¼ of Section 1, Township 5 South, Range 16 East.

TOGETHER WITH an easement for ingress and egress described as follows: BEGIN at the Southeast corner of Lot 6, Unit 4, SOUTHWOOD ACRES, a subdivision according to plat thereof recorded in Plat Book 4, Page 7 of the Public Records of Columbia County, Florida and run thence S 0°08'06" E, 45 feet, thence N 89°20'07" E, 94.55 feet, thence S 0°08'06" E, 219.61 feet, thence S 89°20'07" W, 96.02 feet to the Northeast corner of the above described lands, thence S 0°08'06" E along the East line of said above described lands, 30.00 feet, thence N 89°20'07" E, 126.02 feet, thence N 0°08'06" W, 294.61 feet to the South line of said Unit 4, SOUTHWOOD ACRES, thence S 89°20'27" W along said South line, 124.55 feet to the POINT OF BEGINNING.

EX 0750 PG 0744

OFFICIAL RECORDS

SCHEDULE "A"

PARCEL NO. 1

Commence at the Southeast corner of Section 36, Township 4 South, Range 16 East, Columbia County, Florida and run thence N 0°14'34" E along the East line of said Section 36, 37.54 feet, thence S 89°20'07" W, 328.56 feet to the POINT OF BEGINNING; thence continue S 89°20'07" W, 329.44 feet, thence N 0°08'06" W, 264.61 feet to the South line of Unit 4 of SOUTHWOOD ACRES, a subdivision according to plat thereof recorded in Plat Book 4, Page 7 of the Public Records of Columbia County, Florida, thence N 89°20'07" E along said South line, 329.44 feet, thence S 0°08'06" E, 264.61 feet to POINT OF BEGINNING. Said lands being subject to an easement for ingress and egress across a portion of the North 45 feet as recorded in Official Records Book 430, Page 671. Also the East 15 feet of said lands being subject to an easement for ingress and egress.

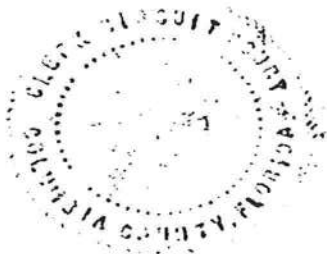
PARCEL NO. 2

Commence at the Southeast corner of Section 36, Township 4 South, Range 16 East, Columbia County, Florida and run thence N 0°14'34" E along the East line of said Section 36, 37.54 feet to the POINT OF BEGINNING, thence continue N 0°14'34" E along said East line, 264.63 feet to the Southeast corner of Lot 9, Unit 4, SOUTHWOOD ACRES, a subdivision according to plat thereof recorded in Plat Book 4, Page 7 of the Public Records of Columbia County, Florida, thence S 89°20'07" W along the South line of said Unit 4, SOUTHWOOD ACRES, 330.31 feet, thence S 0°08'06" E, 264.61 feet, thence N 89°20'07" E, 328.56 feet to the POINT OF BEGINNING. The North 45 feet of said lands being subject to an easement for ingress and egress as recorded in Official Records Book 430, Page 671. Also the West 15 feet of said lands being subject to an easement for ingress and egress.

TOGETHER WITH an easement for ingress and egress described as follows: BEGIN at the southeast corner of Lot 6, Unit 4, SOUTHWOOD ACRES, a subdivision according to plat thereof recorded in Plat Book 4, Page 7 of the Public Records of Columbia County, Florida and run thence S 0°08'06" E, 45 feet, thence N 89°20'07" E, 109.55 feet to the West line of the above described lands, thence N 0°08'06" W along said West line, 45 feet to the Northwest corner of said described lands, thence S 89°20'07" W along the South line of said Unit 4 of SOUTHWOOD ACRES, 109.55 feet to the POINT OF BEGINNING.

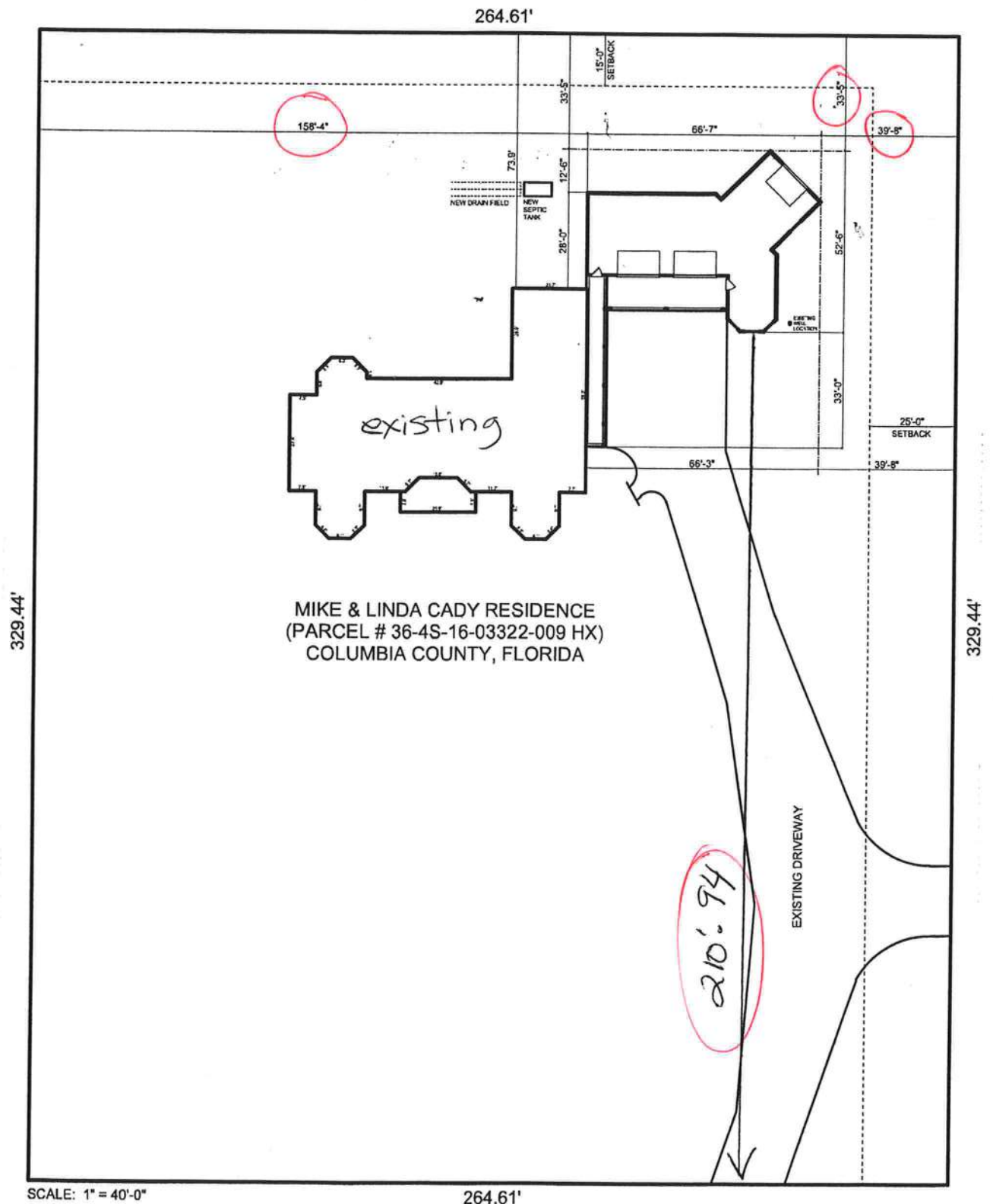
PARCEL NO. 3

BEGIN at the Southeast corner of Section 36, Township 4 South, Range 16 East, Columbia County, Florida and run thence N 0°14'34" E along the East line of said Section 36, 37.54 feet, thence S 89°20'07" W, 220.15 feet, thence S 0°08'06" E 396.34 feet, thence N 88°51'13" E, 221.14 feet to the East line of Section 1, Township 5 South, Range 16 East, thence N 0°19'47" W along said East line, 356.93 feet to the POINT OF BEGINNING. Said lands being a part of the SE $\frac{1}{4}$ of SE $\frac{1}{4}$ of Section 36, and the NE $\frac{1}{4}$ of NE $\frac{1}{4}$ of Section 1.



PK 0750 PG 0745

OFFICIAL RECORDS



MIKE & LINDA CADY RESIDENCE
(PARCEL # 36-4S-16-03322-009 HX)
COLUMBIA COUNTY, FLORIDA

210'-94

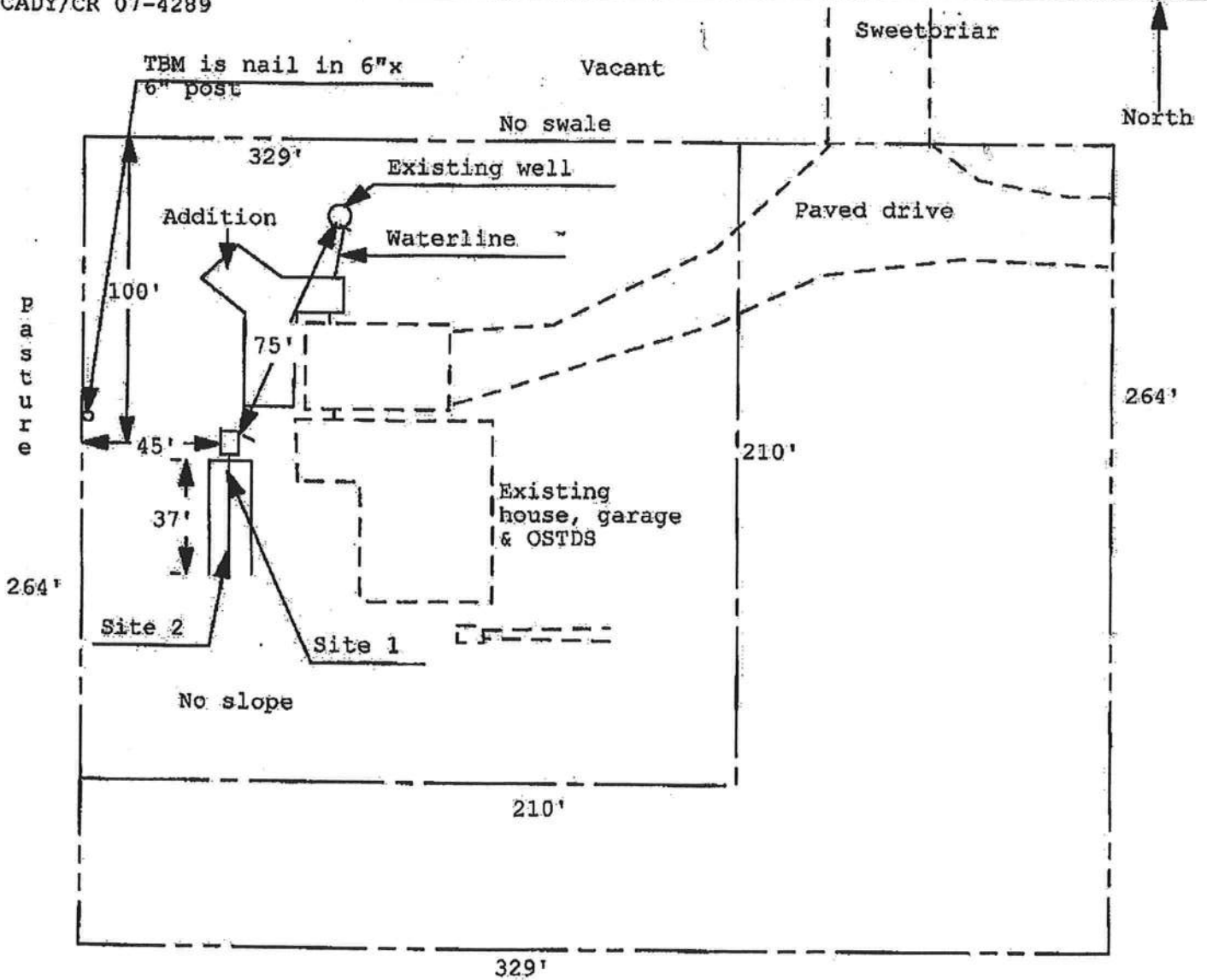
SCALE: 1" = 40'-0"

264.61'

**Application for Onsite Sewage Disposal System
Construction Permit. Part II Site Plan**
Permit Application Number: 08-0167

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

CADY/CR 07-4289



1 inch = 50 feet

Site Plan Submitted By Paul Lopez Date 2/7/08
 Plan Approved Not Approved Date 2-12-08
 By Ma A. Jar Columbia CPHU

Notes: _____

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: Mike & Linda Cady Address: City, State: Lake City, FL 32024- Owner: Cady Residence Climate Zone: North	Builder: Permitting Office: <i>Columbin</i> Permit Number: <i>26825</i> Jurisdiction Number: <i>221000</i>
--	---

<table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 30%;">1. New construction or existing</td> <td style="width: 20%; text-align: center;">New</td> <td style="width: 50%; text-align: right;">___</td> </tr> <tr> <td>2. Single family or multi-family</td> <td style="text-align: center;">Single family</td> <td style="text-align: right;">___</td> </tr> <tr> <td>3. Number of units, if multi-family</td> <td style="text-align: center;">1</td> <td style="text-align: right;">___</td> </tr> <tr> <td>4. Number of Bedrooms</td> <td style="text-align: center;">1</td> <td style="text-align: right;">___</td> </tr> <tr> <td>5. Is this a worst case?</td> <td style="text-align: center;">No</td> <td style="text-align: right;">___</td> </tr> <tr> <td>6. Conditioned floor area (ft²)</td> <td style="text-align: center;">1534 ft²</td> <td style="text-align: right;">___</td> </tr> <tr> <td>7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default)</td> <td></td> <td style="text-align: right;">___</td> </tr> <tr> <td style="padding-left: 20px;">a. U-factor:</td> <td style="text-align: center;">Description Area</td> <td></td> </tr> <tr> <td style="padding-left: 40px;">(or Single or Double DEFAULT)</td> <td style="text-align: center;">7a. (Dble Default) 178.0 ft²</td> <td style="text-align: right;">___</td> </tr> <tr> <td style="padding-left: 20px;">b. SHGC:</td> <td></td> <td></td> </tr> <tr> <td style="padding-left: 40px;">(or Clear or Tint DEFAULT)</td> <td style="text-align: center;">7b. (Clear) 178.0 ft²</td> <td style="text-align: right;">___</td> </tr> <tr> <td>8. Floor types</td> <td></td> <td></td> </tr> <tr> <td style="padding-left: 20px;">a. Slab-On-Grade Edge Insulation</td> <td style="text-align: center;">R=5.0, 84.0(p) ft</td> <td style="text-align: right;">___</td> </tr> <tr> <td style="padding-left: 20px;">b. 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Glass/Floor Area: 0.12	Total as-built points: 15965 Total base points: 17371	PASS
------------------------	--	------

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: *[Signature]*


DATE: 1-22-08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: *[Signature]*

DATE: 2-5-08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCPB v4.5.2)

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FL, 32024-	PERMIT #:
----------------------------------	-----------

BASE	AS-BUILT
GLASS TYPES	
.18 X Conditioned X BSPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X SPM X SOF = Points
.18 1534.0 18.59 5133.0	1.Double, Clear NE 1.5 11.0 15.0 29.56 0.99 437.0 2.Double, Clear E 1.5 11.0 20.0 42.06 0.99 828.0 3.Double, Clear SE 1.5 11.0 15.0 42.75 0.99 632.0 4.Double, Clear S 8.5 10.0 20.0 35.87 0.54 388.0 5.Double, Clear E 1.5 6.0 48.0 42.06 0.91 1842.0 6.Double, Clear S 1.5 6.0 24.0 35.87 0.86 736.0 7.Double, Clear W 1.5 6.0 36.0 38.52 0.91 1266.0
	As-Built Total: 178.0 6129.0
WALL TYPES Area X BSPM = Points	Type R-Value Area X SPM = Points
Adjacent 290.0 0.70 203.0 Exterior 1290.0 1.70 2193.0	1. Frame, Wood, Exterior 19.0 546.0 0.90 491.4 2. Frame, Wood, Adjacent 13.0 290.0 0.60 174.0 3. Frame, Wood, Exterior 19.0 744.0 0.90 669.6
Base Total: 1580.0 2396.0	As-Built Total: 1580.0 1335.0
DOOR TYPES Area X BSPM = Points	Type Area X SPM = Points
Adjacent 36.0 2.40 86.4 Exterior 0.0 0.00 0.0	1.Adjacent Insulated 18.0 1.60 28.8 2.Adjacent Insulated 18.0 1.60 28.8
Base Total: 36.0 86.4	As-Built Total: 36.0 57.6
CEILING TYPES Area X BSPM = Points	Type R-Value Area X SPM X SCM = Points
Under Attic 1534.0 1.73 2653.8	1. Under Attic 30.0 1350.0 1.73 X 1.00 2335.5 2. Under Attic 30.0 392.0 1.73 X 1.00 678.2
Base Total: 1534.0 2653.8	As-Built Total: 1742.0 3013.7
FLOOR TYPES Area X BSPM = Points	Type R-Value Area X SPM = Points
Slab 84.0(p) -37.0 -3108.0 Raised 1142.0 -3.99 -4556.6	1. Slab-On-Grade Edge Insulation 5.0 84.0(p) -36.20 -3040.8 2. Raised Wood, Adjacent 30.0 1142.0 0.40 456.8
Base Total: -7664.6	As-Built Total: 1226.0 -2584.0
INFILTRATION Area X BSPM = Points	Area X SPM = Points
1534.0 10.21 15662.1	1534.0 10.21 15662.1

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FL, 32024-	PERMIT #:
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BASE	AS-BUILT
Summer Base Points: 18266.8	Summer As-Built Points: 23613.4
Total Summer X System = Cooling Points Multiplier Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
18266.8 0.3250 5936.7	<small>(sys 1: Central Unit 31000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)</small> 23613 1.00 (1.09 x 1.147 x 0.91) 0.260 0.950 6635.7 23613.4 1.00 1.138 0.260 0.950 6635.7

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FL, 32024-

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt			Area X WPM X WOF = Points			
.18	1534.0	20.17	5569.0	1.Double, Clear	NE	1.5	11.0	15.0	23.57	1.00	353.0
				2.Double, Clear	E	1.5	11.0	20.0	18.79	1.01	379.0
				3.Double, Clear	SE	1.5	11.0	15.0	14.71	1.02	225.0
				4.Double, Clear	S	8.5	10.0	20.0	13.30	2.49	661.0
				5.Double, Clear	E	1.5	6.0	48.0	18.79	1.04	934.0
				6.Double, Clear	S	1.5	6.0	24.0	13.30	1.12	356.0
				7.Double, Clear	W	1.5	6.0	36.0	20.73	1.02	763.0
				As-Built Total:			178.0			3671.0	
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	290.0	3.60	1044.0	1. Frame, Wood, Exterior	19.0		546.0		2.20		1201.2
Exterior	1290.0	3.70	4773.0	2. Frame, Wood, Adjacent	13.0		290.0		3.30		957.0
				3. Frame, Wood, Exterior	19.0		744.0		2.20		1636.8
Base Total:	1580.0		5817.0	As-Built Total:			1580.0		3795.0		
DOOR TYPES Area X BWPM = Points				Type	Area X WPM = Points						
Adjacent	36.0	11.50	414.0	1.Adjacent Insulated			18.0		8.00		144.0
Exterior	0.0	0.00	0.0	2.Adjacent Insulated			18.0		8.00		144.0
Base Total:	36.0		414.0	As-Built Total:			36.0		288.0		
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1534.0	2.05	3144.7	1. Under Attic	30.0		1350.0		2.05 X 1.00		2767.5
				2. Under Attic	30.0		392.0		2.05 X 1.00		803.6
Base Total:	1534.0		3144.7	As-Built Total:			1742.0		3571.1		
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	84.0(p)	8.9	747.6	1. Slab-On-Grade Edge Insulation	5.0		84.0(p)		7.60		638.4
Raised	1142.0	0.96	1096.3	2. Raised Wood, Adjacent	30.0		1142.0		2.20		2512.4
Base Total:			1843.9	As-Built Total:			1226.0		3150.8		
INFILTRATION Area X BWPM = Points				Area X WPM = Points							
	1534.0	-0.59	-905.1			1534.0		-0.59		-905.1	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FL, 32024-	PERMIT #:
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BASE			AS-BUILT						
Winter Base Points: 15883.6			Winter As-Built Points: 13570.8						
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
15883.6	0.5540	8799.5	(sys 1: Electric Heat Pump 31000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Int(AH),R6.0 13570.8	1.000	(1.069 x 1.169 x 0.93)	0.443	0.950	6635.4	
15883.6	0.5540	8799.5	13570.8	1.00	1.162	0.443	0.950	6635.4	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FL, 32024-

PERMIT #:

BASE				AS-BUILT								
WATER HEATING												
Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit	=	Total Multiplier
1		2635.00		2635.0	50.0	0.90	1		1.00	2693.56	1.00	2693.6
As-Built Total:											2693.6	

CODE COMPLIANCE STATUS													
BASE					AS-BUILT								
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
5937		8799		2635		17371	6636		6635		2694		15965

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , Lake City, FL, 32024-

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.5

The higher the score, the more efficient the home.

Cady Residence, , Lake City, FL, 32024-

<p>1. New construction or existing New <input type="checkbox"/></p> <p>2. Single family or multi-family Single family <input type="checkbox"/></p> <p>3. Number of units, if multi-family 1 <input type="checkbox"/></p> <p>4. Number of Bedrooms 1 <input type="checkbox"/></p> <p>5. Is this a worst case? No <input type="checkbox"/></p> <p>6. Conditioned floor area (ft²) 1534 ft² <input type="checkbox"/></p> <p>7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default)</p> <p style="padding-left: 20px;">a. U-factor: Description Area</p> <p style="padding-left: 40px;">(or Single or Double DEFAULT) 7a. (Dble Default) 178.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">b. SHGC:</p> <p style="padding-left: 40px;">(or Clear or Tint DEFAULT) 7b. (Clear) 178.0 ft² <input type="checkbox"/></p> <p>8. Floor types</p> <p style="padding-left: 20px;">a. Slab-On-Grade Edge Insulation R=5.0, 84.0(p) ft <input type="checkbox"/></p> <p style="padding-left: 20px;">b. Raised Wood, Adjacent R=30.0, 1142.0ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>9. Wall types</p> <p style="padding-left: 20px;">a. Frame, Wood, Exterior R=19.0, 546.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">b. Frame, Wood, Adjacent R=13.0, 290.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">c. Frame, Wood, Exterior R=19.0, 744.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">d. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">e. N/A <input type="checkbox"/></p> <p>10. Ceiling types</p> <p style="padding-left: 20px;">a. Under Attic R=30.0, 1350.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">b. Under Attic R=30.0, 392.0 ft² <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>11. Ducts</p> <p style="padding-left: 20px;">a. Sup: Unc. Ret: Unc. AH: Interior Sup. R=6.0, 35.0 ft <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p>	<p>12. Cooling systems</p> <p style="padding-left: 20px;">a. Central Unit Cap: 31.0 kBtu/hr <input type="checkbox"/></p> <p style="padding-left: 40px;">SEER: 13.00 <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>13. Heating systems</p> <p style="padding-left: 20px;">a. Electric Heat Pump Cap: 31.0 kBtu/hr <input type="checkbox"/></p> <p style="padding-left: 40px;">HSPF: 7.70 <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. N/A <input type="checkbox"/></p> <p>14. Hot water systems</p> <p style="padding-left: 20px;">a. Electric Resistance Cap: 50.0 gallons <input type="checkbox"/></p> <p style="padding-left: 40px;">EF: 0.90 <input type="checkbox"/></p> <p style="padding-left: 20px;">b. N/A <input type="checkbox"/></p> <p style="padding-left: 20px;">c. Conservation credits <input type="checkbox"/></p> <p style="padding-left: 40px;">(HR-Heat recovery, Solar</p> <p style="padding-left: 40px;">DHP-Dedicated heat pump)</p> <p>15. HVAC credits PT, <input type="checkbox"/></p> <p style="padding-left: 20px;">(CF-Ceiling fan, CV-Cross ventilation,</p> <p style="padding-left: 20px;">HF-Whole house fan,</p> <p style="padding-left: 20px;">PT-Programmable Thermostat,</p> <p style="padding-left: 20px;">MZ-C-Multizone cooling,</p> <p style="padding-left: 20px;">MZ-H-Multizone heating)</p>
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I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCPB v4.5.2)

0802-24

Mike Cady

FROM : COLUMBIA CO BUILDING + ZONING FAX NO. : 386-758-2160

Nov. 30 2007 10:23AM P1



COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT
FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR
LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

- Single Family Dwelling
Other
Two-Family Residence
Farm Outbuilding
Addition, Alteration, Modification or other Improvement

I, Michael Cady, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number

Michael Cady
Owner Builder Signature
Date 2-2-08

FLORIDA NOTARY

The above signer is personally known to me or produced identification

Notary Signature Linda Roder Date 2-2-08



Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded thru
Atlantic Bonding Co., Inc.

FOR BUILDING DEPARTMENT USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date Building Official/Representative

0802-24

Mike Cady

FROM : COLUMBIA CD BUILDING + ZONING FAX NO. : 386-758-2160

Nov. 30 2007 10:23AM P1



COLUMBIA COUNTY BUILDING DEPARTMENT

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT
FOR OWNER/BUILDER WHEN ACTING AS THEIR OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR
LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

- Single Family Dwelling
Other
Two-Family Residence
Farm Outbuilding
Addition, Alteration, Modification or other Improvement

I, Michael Cady, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number

Michael Cady (Signature)
Owner Builder Signature
Date 2-22-08

FLORIDA NOTARY

The above signer is personally known to me or produced identification

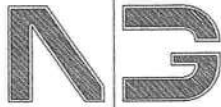
Notary Signature Linda Roder Date 2-22-08



Linda R. Roder
Commission #DD303275
Expires: Mar 24, 2008
Bonded Thru Atlantic Bonding Co., Inc.

FOR BUILDING DEPARTMENT USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date Building Official/Representative



**NICHOLAS
PAUL
GEISLER**
ARCHITECT
N.C.A.R.B. Certified

■ 1758 NW Brown Road
■ Lake City, FL 32055
■ 386/755-9021

17 APRIL 2008

JOHNNY KEARSE, BUILDING OFFICIAL
COLUMBIA COUNTY, BUILDING DEPT.
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA 32055

RE: CADY GARAGE ADDITION
PERMIT Nr.: 26825

DEAR SIR:

PLEASE BE ADVISED OF THE FOLLOWING CORRECTION TO THE CONSTRUCTION DOCUMENTS FOR THE ABOVE REFERENCED PROJECT:

1. THE DETAIL AND/OR NOTES THAT REFER TO 3 #5 REBAR AT THE BOTTOM OF THE PROPOSED FOOTINGS SHOULD READ 2 #5 REBAR.
2. PLEASE NOTE THAT IT IS THE OWNER'S DESIRE TO INSTALL A THIRD #5 REBAR AT THE TOP OF THE FOOTING. THIS SHALL BE ACCOMPLISHED BY PLACING THE THIRD REBAR AT THE TIME OF THE POUR. THE EXACT LOCATION IS NOT IMPORTANT AS TO THE STRUCTURAL CONSIDERATIONS FOR THE PROJECT.
3. HAVING INSPECTED THE PREPARED FOOTINGS AT THE JOB SITE, I HAVE DIRECTED THE CONTRACTOR TO SHIFT THE PAD FOOTING AT THE COVERED WALK THAT IS NEAREST TO THE NEW GARAGE AS NEEDED TO THE RIGHT AS WHEN FACING THE GARAGE, SO AS TO CLEAR THE CONDUIT BUNDLE THAT WOULD OTHERWISE BE ENCASED WITHIN THE FOOTING. THIS FOOTING IS PRIMERILEY INTENDED AS A DEAD LOAD FOR THE ROOF UPLIFT LOADS OF THE COVERED WALK AND AS SUCH WILL PROVIDE SATISFACTORY UPLIFT PERFORMANCE WHILE SHIFTED AS NOTED ABOVE.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#26825

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: P.O. Box 1795 City Lake City State FL Zip 32905
Company Business License No. JE109576 Company Phone No. 352-752-3811 • 352-494-5751
FHA/VA Case No. (if any) _____

Section 2: Builder Information

Company Name: Mike Cady Company Phone No. 755-6237

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 300 SW Sweetbriar Ln. Lake City, FL 32024
Type of Construction (More than one box may be checked) Slab Basement Crawl Other _____
Approximate Depth of Footing: Outside _____ Inside _____ Type of Fill Monolithic

Section 4: Treatment Information

Date(s) of Treatment(s) 4/14/08
Brand Name of Product(s) Used Termidor
EPA Registration No. _____
Approximate Final Mix Solution % 0.06%
Approximate Size of Treatment Area: Sq. ft. 1534 Linear ft. _____ Linear ft. of Masonry Voids _____
Approximate Total Gallons of Solution Applied 150 gals.
Was treatment completed on exterior? Yes No
Service Agreement Available? Yes No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____

Comments _____

Name of Applicator(s) S. Gregory Certification No. (if required by State law) JE109576

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Shannon Gregory Date 4/14/08

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3122)

Form NPCA-99-B may still be used form HUD-NPCA-99-B (04/2003)

Order Product #2581 from CROWNMAX • 1-800-252-4011