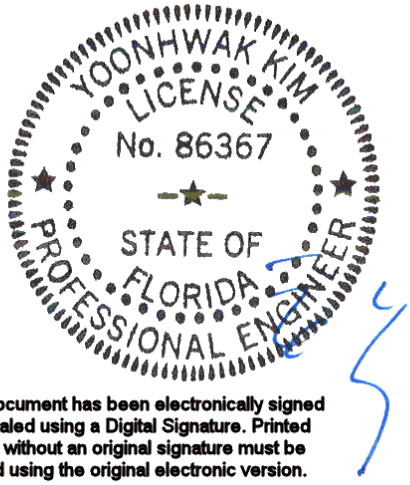

This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic version.

ALPINE
AN ITW COMPANY

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155 Harlem Ave
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Glenview, IL 60025
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www.alpineitw.com

FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

02/09/2023

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	*Job Number:* 22-8690
Job Description: JBC Builders (Busscher) - Futral	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 21.02.00B through 21.02.01 *JRef #:* 1XN22150004
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 150 *Building Type:* Closed	*Design Loading (psf):* 40.00, 65.00

This package contains general notes pages, 5 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	040.23.1306.19660	A01
3	040.23.1306.24223	A03
5	040.23.1306.28513	F02
7	BRCLBSUB0119	
9	STRBRIBR1014	

Item	Drawing Number	Truss
2	040.23.1306.21997	A02
4	040.23.1306.25687	F01
6	A16030ENC160118	
8	GBLLETIN0118	
10	DEFLCAMB1014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

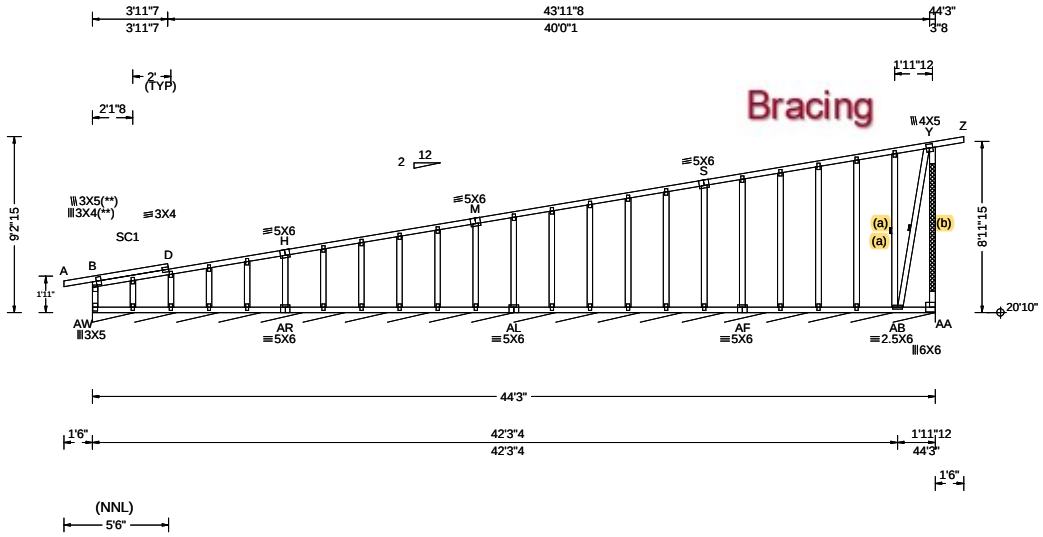
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 122218 GABL FROM:	Ply: 1 Qty: 2	**Job Number:** 22-8690 JBC Builders (Busscher) - Futral **Truss Label:** A01	Cust: R 215 JRef:1XN22150004 T6 DrwNo: 040.23.1306.19660 SSB / YK 02/09/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 150 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 26.27 ft
TCDL: 4.2 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.43 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.015	Y	999	240
VERT(CL): -0.019	Y	999	180
HORZ(LL): -0.220	K	-	-
HORZ(TL): 0.246	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.321
Max BC CSI: 0.247
Max Web CSI: 0.963

VIEW Ver: 21.02.00B.1108.20

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AA*	85	/-	/-	/44	/46	/15

Wind reactions based on MWFRS
AA Brg Wid = 531 Min Req = -
Bearing AW is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AR-AL	474	-575	AF-AB	475	-573
AL-AF	474	-574			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
AB- Y	627	-1758

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
Y -AA	1805	-514

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

(b) (2) #3 or better scab reinforcing members. Same size & 80% length of web member. Attach one to each face w/10d Box or Gun (0.128"x3",min.) nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

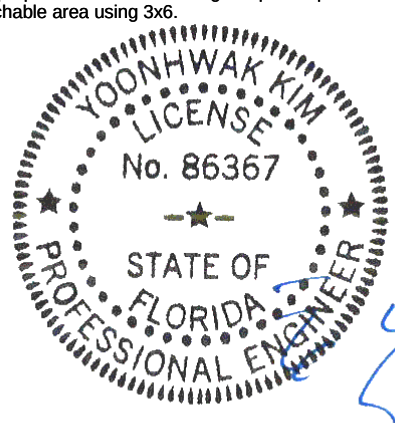
Right end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A16030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999
02/09/2023

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
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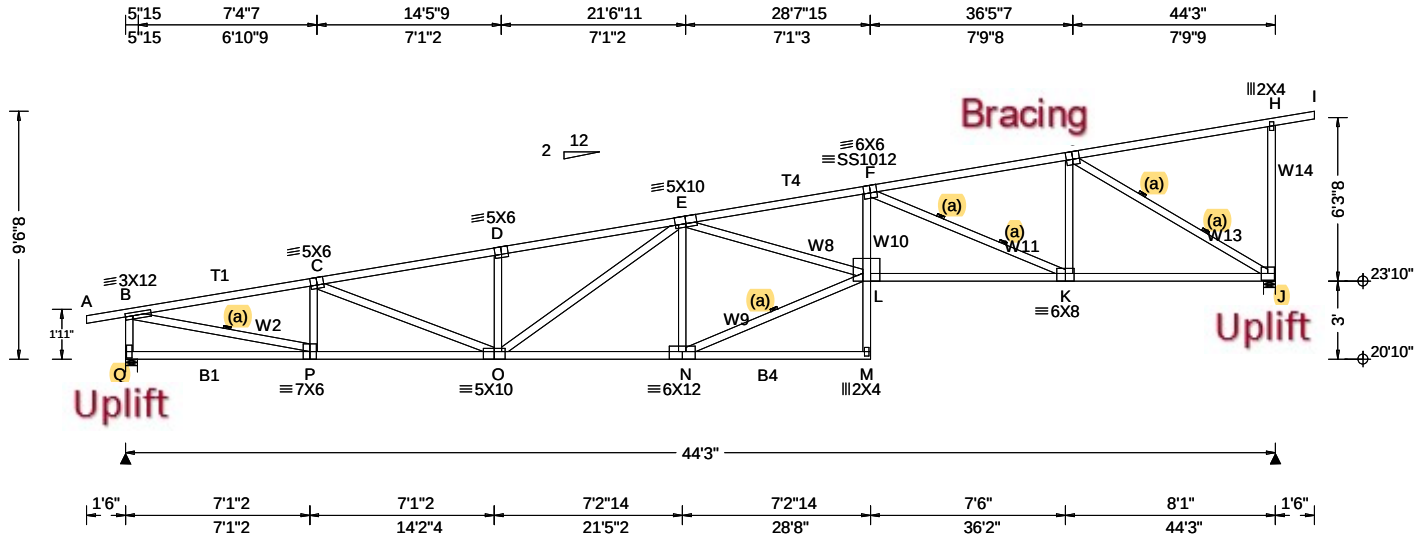
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 103939 MONO FROM:	Ply: 1 Qty: 8	**Job Number:** 22-8690 JBC Builders (Busscher) - Futral **Truss Label:** A02	Cust: R 215 JRef:1XN22150004 T5 DrwNo: 040.23.1306.21997 SSB / YK 02/09/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 150 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 26.44 ft
TCDL: 4.2 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.43 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.473 E	999	240
VERT(CL):	0.938 E	566	180
HORZ(LL):	-0.104 J	-	-
HORZ(TL):	0.199 J	-	-

Creep Factor: 2.0
Max TC CSI: 0.878
Max BC CSI: 0.618
Max Web CSI: 0.877

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Q	1871	/-	/-	/987	/1020	/603
J	1870	/-	/-	/977	/1085	/-

Wind reactions based on MWFRS
Q Brg Wid = 5.5 Min Req = 2.2 (Truss)
J Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings Q & J are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	2020	- 3918	E - F	3050	- 5680
C - D	2318	- 4428	F - G	1439	- 2669
D - E	2381	- 4424			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
Q - P	332	- 626	L - K	5572	- 3273
P - O	3899	- 2461	K - J	2517	- 1499
O - N	3830	- 2239			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - Q	1034	- 1805	E - L	1857	- 1108
B - P	3884	- 1971	N - L	4098	- 2397
P - C	494	- 718	L - F	1411	- 716
C - O	469	- 190	F - K	1904	- 3261
D - O	340	- 409	K - G	1421	- 659
O - E	621	- 417	G - J	1654	- 2936
E - N	1032	- 1526			

Lumber

Top chord: 2x4 SP #2; T1,T4 2x4 SP M-31;
Bot chord: 2x4 SP M-31; B1,B4 2x4 SP #2;
Webs: 2x4 SP #3; W2,W11,W13 2x4 SP #2; W8,W9,W10, W14 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

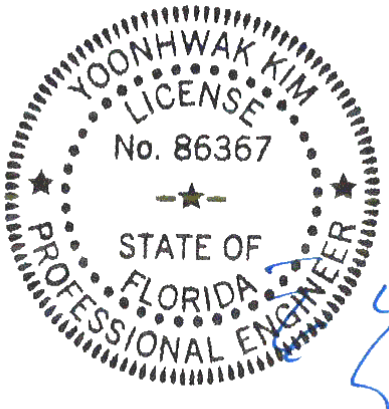
Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Right end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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02/09/2023

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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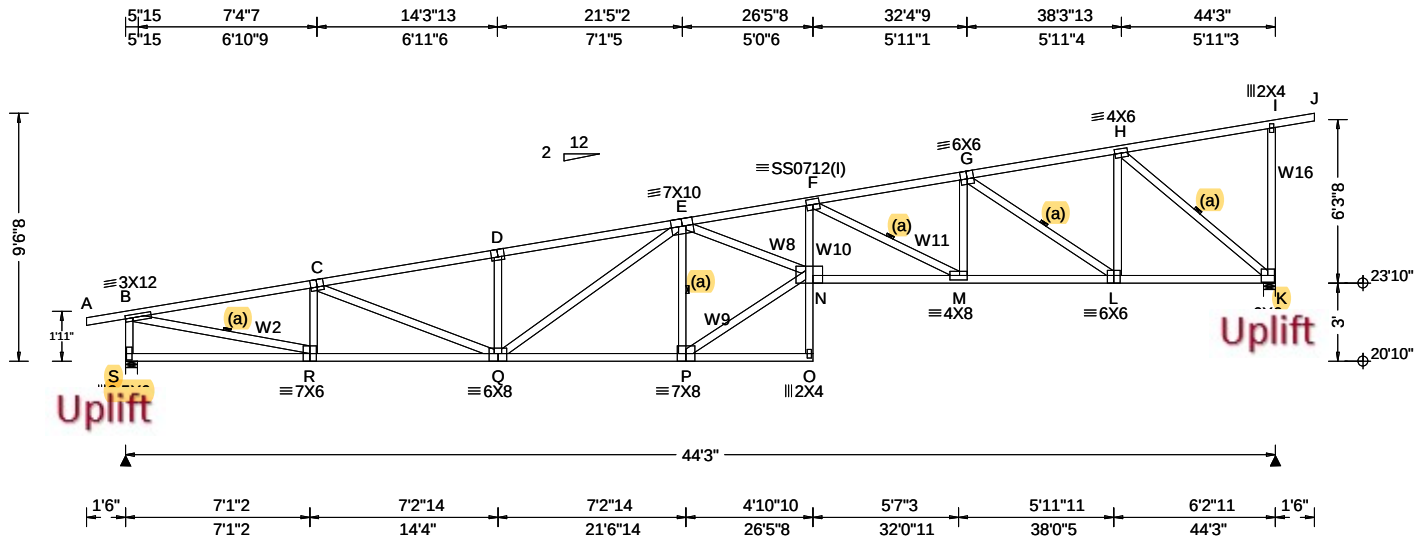
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 103945 MONO FROM:	Ply: 1 Qty: 6	**Job Number:** 22-8690 JBC Builders (Busscher) - Futral **Truss Label:** A03	Cust: R 215 JRef:1XN22150004 T3 DrwNo: 040.23.1306.24223 SSB / YK 02/09/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 150 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 26.44 ft
TCDL: 4.2 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 4.43 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.548	N	969	240
VERT(CL): 1.085	N	489	180
HORZ(LL): 0.131	K	-	-
HORZ(TL): 0.259	K	-	-

Creep Factor: 2.0
Max TC CSI: 0.494
Max BC CSI: 0.675
Max Web CSI: 0.882

VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
S	1869	/-	/-	/987	/1020	/603
K	1870	/-	/-	/977	/1085	/-

Wind reactions based on MWFRS
S Brg Wid = 5.5 Min Req = 1.5 (Truss)
K Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings S & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	2019	- 3910	E - F	3827	- 7129
C - D	2321	- 4430	F - G	2172	- 4039
D - E	2386	- 4428	G - H	1104	- 1996

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
S - R	331	- 627	N - M	7010	- 4106
R - Q	3891	- 2460	M - L	3870	- 2274
Q - P	3810	- 2232	L - K	1860	- 1129

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - S	1034	- 1802	P - N	4502	- 2633
B - R	3877	- 1970	N - F	1850	- 1007
R - C	495	- 721	F - M	2016	- 3443
C - Q	479	- 194	M - G	1336	- 672
D - Q	351	- 421	G - L	1355	- 2366
Q - E	648	- 431	L - H	1506	- 744
E - P	1542	- 2436	H - K	1369	- 2430
E - N	3490	- 2050			

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3; W2,W10 2x4 SP #2; W8,W9,W11, W16 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 5X6 except as noted.

(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

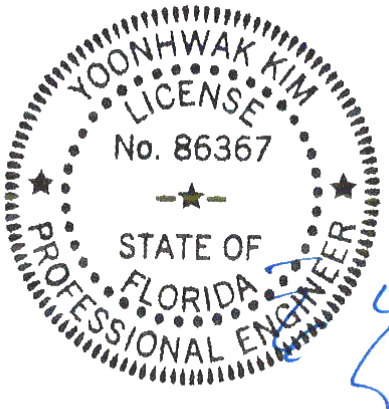
Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Right end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL 1999
02/09/2023

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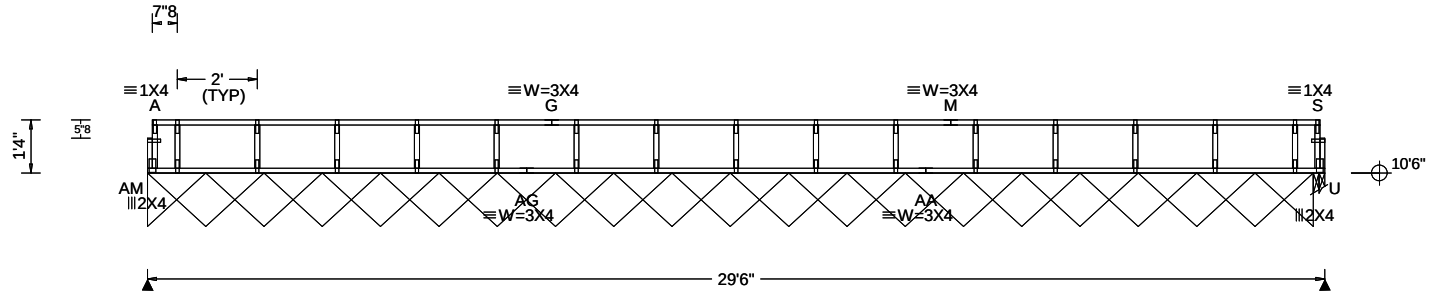
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 103890 SY42 FROM:	Ply: 1 Qty: 2	**Job Number:** 22-8690 JBC Builders (Busscher) - Futral **Truss Label:** F01	Cust: R 215 JRef:1XN22150004 T2 DrwNo: 040.23.1306.25687 SSB / YK 02/09/2023



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 40.00 TCDL: 15.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 65.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 C 999 480 VERT(CL): 0.000 C 999 360 HORZ(LL): -0.000 U - - HORZ(TL): 0.000 U - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.058 Max Web CSI: 0.052 VIEW Ver: 21.02.01.1216.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AM*131		/-	/-	/-	/-	/-
U	-	/-9	/-	/-	/-	/-

AM Brg Wid = 350 Min Req = -
U Brg Wid = 3.5 Min Req = 1.5
Bearings AM & U are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 4x2 SP #2;
Bot chord: 4x2 SP #2;
Webs: 4x2 SP #3;

Bracing

Sheathing is required for any longitudinal(drag) forces. All connections to be designed by the building designer.

Fasten rated sheathing to one face of this frame.

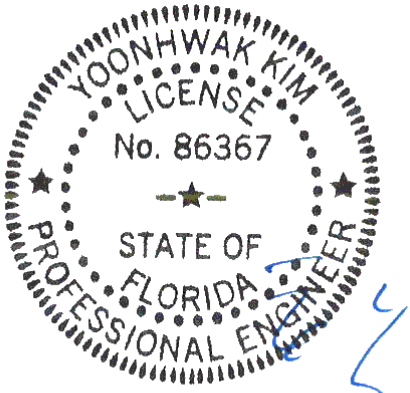
Plating Notes

All plates are 1X4 except as noted.

Additional Notes

See detail STRBRIBR1014 for bracing and bridging recommendations.

Truss must be installed as shown with top chord up.



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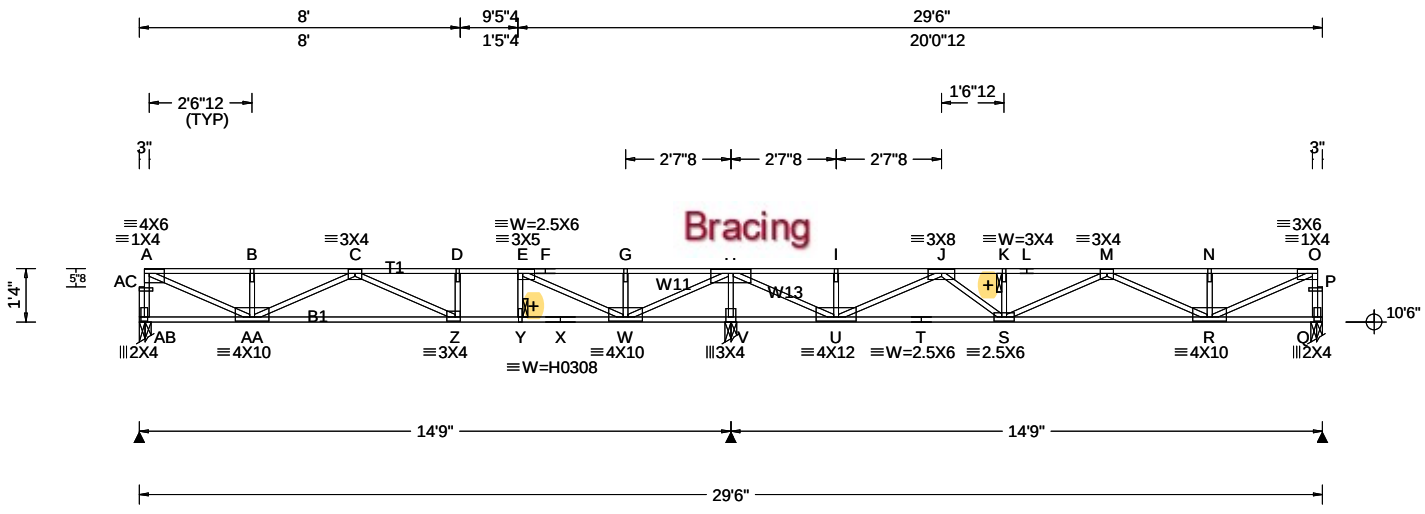
North Building, 4th Floor
Glenview, IL 60025

SEQN: 103896 SY42 FROM:	Ply: 1 Qty: 21	**Job Number:** 22-8690 JBC Builders (Busscher) - Futral **Truss Label:** F02	Cust: R 215 JRef:1XN22150004 T4 DrwNo: 040.23.1306.28513 SSB / YK 02/09/2023



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 40.00	Wind Std: NA	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 15.00	Speed: NA mph	Pf: NA Ce: NA	VERT(LL): 0.225 D 776 480
BCLL: 0.00	Enclosure: NA	Lu: NA Cs: NA	VERT(CL): 0.353 D 495 360
BCDL: 10.00	Category: NA	Snow Duration: NA	HORZ(LL): 0.029 B - -
Des Ld: 65.00	EXP: NA Kzt: NA		HORZ(TL): 0.047 B - -
NCBCLL: 10.00	Mean Height: NA ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: NA psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.586
Load Duration: 1.00	BCDL: NA psf	TPI Std: 2014	Max BC CSI: 0.808
Spacing: 24.0 "	MWFRS Parallel Dist: NA	Rep Fac: Yes	Max Web CSI: 0.833
	C&C Dist a: NA ft	FT/RT:12(0)/10(0)	
	Loc. from endwall: NA	Plate Type(s):	VIEW Ver: 21.02.01.1216.15
	I: NA GCpi: NA	WAVE, HS	
	Wind Duration: NA		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AB	887	/-	/-	/-	/-	/-
V	2170	/-	/-	/-	/-	/-
Q	865	/-	/-	/-	/-	/-

AB Brg Wid = 3.5 Min Req = 1.5 (Truss)
V Brg Wid = 3.5 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings AB, V, & Q are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	0	-1587	H - I	184	-693
B - C	0	-1587	I - J	184	-693
C - D	0	-2180	J - K	0	-2212
D - E	0	-2181	K - L	0	-2212
E - F	420	-871	L - M	0	-2212
F - G	420	-871	M - N	0	-1526
G - H	420	-871	N - O	0	-1526

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AA- Z	2267	0	V - U	0	-1571
Z - Y	2181	0	U - T	1821	0
Y - X	2171	0	T - S	1821	0
X - W	2171	0	S - R	2172	0
W - V	0	-1571			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
AC-AB	0	-880	H - U	2117	0
A -AC	0	-855	U - J	0	-1332
A -AA	1749	0	J - S	550	0
AA- C	0	-752	M - R	0	-714
C - Z	178	-411	R - O	1682	0
E - W	0	-1627	O - P	0	-823
W - H	2038	0	P - Q	0	-833
H - V	0	-2065			

Lumber

Top chord: 4x2 SP #2; T1 4x2 SP M-31;
Bot chord: 4x2 SP #2; B1 4x2 SP M-31;
Webs: 4x2 SP #3; W11,W13 4x2 SP M-31;

Plating Notes

All plates are 1X4 except as noted.

Deflection

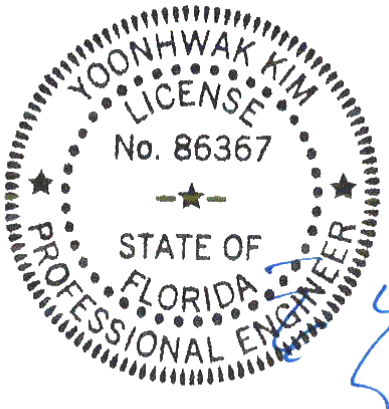
Max JT VERT DEFL: LL: 0.22" DL: 0.23". See detail DEFLCAMB1014 for camber recommendations.

Additional Notes

+ 2x6 continuous strongback. See detail STRBRIBR1014 for bracing and bridging recommendations.

Maximum panel length exceeds 30". TPI allows non-bearing partition walls to be supported at any point when panels are 30" or less.

Truss must be installed as shown with top chord up.



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155 Harlem Ave
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Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 160 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 140 MPH Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	3' 8"	6' 3"	6' 6"	7' 5"	7' 8"	8' 9"	9' 2"	11' 7"	12' 0"	13' 9"	14' 0"
		#3	3' 6"	5' 5"	5' 9"	7' 2"	7' 7"	8' 8"	9' 1"	11' 3"	11' 11"	13' 7"	14' 0"
		Stud	3' 6"	5' 4"	5' 8"	7' 2"	7' 7"	8' 8"	9' 1"	11' 2"	11' 11"	13' 7"	14' 0"
		Standard	3' 6"	4' 7"	4' 11"	6' 2"	6' 7"	8' 4"	8' 11"	9' 8"	10' 4"	13' 1"	14' 0"
	SP DFL	#1	3' 10"	6' 4"	6' 7"	7' 6"	7' 9"	8' 11"	9' 3"	11' 9"	12' 2"	13' 11"	14' 0"
		#2	3' 8"	6' 1"	6' 6"	7' 5"	7' 8"	8' 9"	9' 2"	11' 7"	12' 0"	13' 9"	14' 0"
		#3	3' 7"	4' 10"	5' 2"	6' 5"	6' 11"	8' 9"	9' 1"	10' 1"	10' 10"	13' 8"	14' 0"
		Stud	3' 7"	4' 10"	5' 2"	6' 5"	6' 11"	8' 9"	9' 1"	10' 1"	10' 10"	13' 8"	14' 0"
		Standard	3' 3"	4' 3"	4' 7"	5' 8"	6' 1"	7' 9"	8' 3"	8' 11"	9' 7"	12' 2"	13' 0"
16" o.c.	SPF HF	#1 / #2	4' 2"	7' 2"	7' 5"	8' 5"	8' 9"	10' 1"	10' 6"	13' 3"	13' 9"	14' 0"	14' 0"
		#3	4' 0"	6' 7"	7' 5"	8' 4"	8' 8"	9' 11"	10' 4"	13' 1"	13' 7"	14' 0"	14' 0"
		Stud	4' 0"	6' 7"	7' 0"	8' 4"	8' 8"	9' 11"	10' 4"	13' 1"	13' 7"	14' 0"	14' 0"
		Standard	4' 0"	5' 8"	6' 0"	7' 6"	8' 0"	9' 11"	10' 4"	11' 10"	12' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	7' 3"	7' 6"	8' 7"	8' 11"	10' 2"	10' 7"	13' 5"	13' 11"	14' 0"	14' 0"
		#2	4' 2"	7' 2"	7' 5"	8' 5"	8' 9"	10' 1"	10' 6"	13' 3"	13' 9"	14' 0"	14' 0"
		#3	4' 1"	5' 11"	6' 4"	7' 11"	8' 5"	10' 0"	10' 5"	12' 5"	13' 3"	14' 0"	14' 0"
		Stud	4' 1"	5' 11"	6' 4"	7' 11"	8' 5"	10' 0"	10' 5"	12' 5"	13' 3"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 5"	10' 2"	11' 0"	11' 9"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	4' 8"	7' 11"	8' 2"	9' 4"	9' 8"	10' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 7"	8' 1"	9' 2"	9' 6"	10' 11"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 5"	7' 7"	8' 1"	9' 2"	9' 6"	10' 11"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 6"	6' 11"	8' 8"	9' 3"	10' 11"	11' 5"	13' 7"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	8' 0"	8' 3"	9' 5"	9' 9"	11' 2"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 8"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	6' 10"	7' 3"	9' 1"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 6"	6' 10"	7' 3"	9' 1"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 1"	6' 5"	8' 1"	8' 7"	10' 11"	11' 5"	12' 8"	13' 7"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 135 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

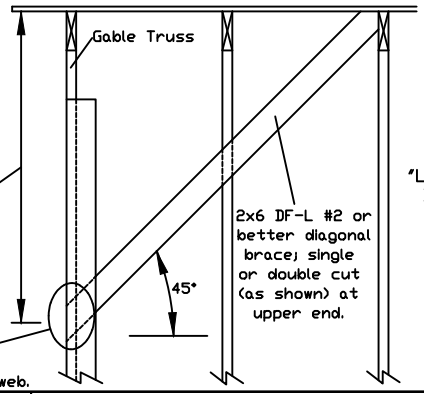
Gable Vertical Plate Sizes

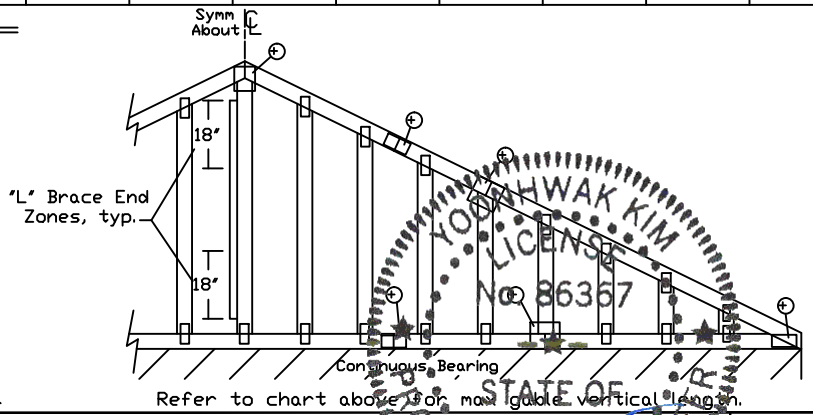
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 12' 0"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 690# at each end. Max web total length is 14'.





Refer to chart above for max gable vertical length.

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB16030
DATE	01/26/2018
DRWG	A16030ENC160118

1/29/2018, Yoonhwak Kim, FL PE #86367

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

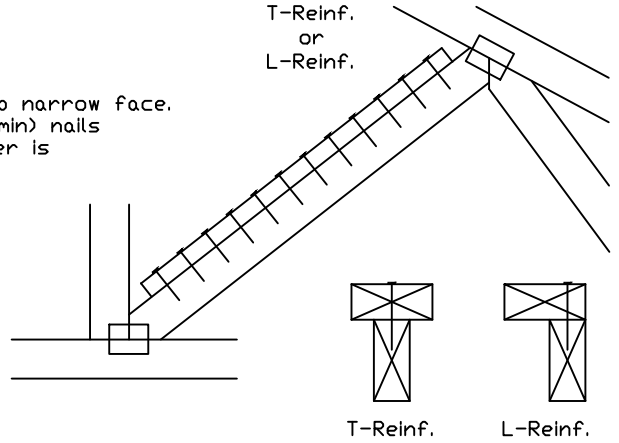
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

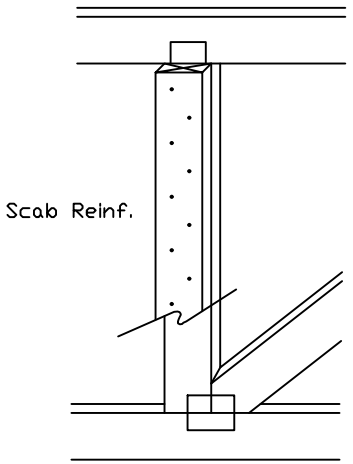
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



YOONHWAK KIM LICENSE No. 86367 STATE OF FLORIDA PROFESSIONAL ENGINEER

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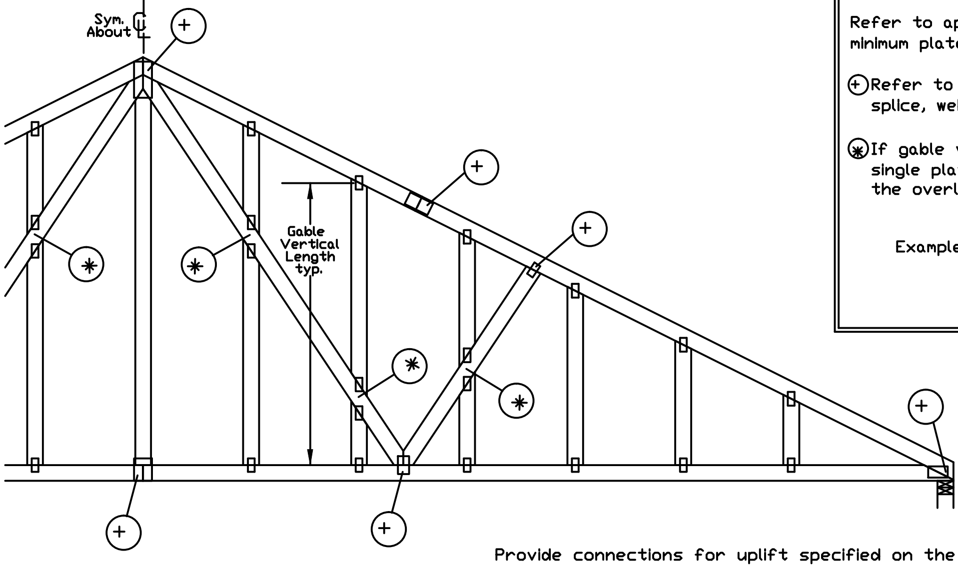
02/09/2023
FL Reg. #278, Yoonhwak Kim, FL PE #86367

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Detail

For Let-in Verticals



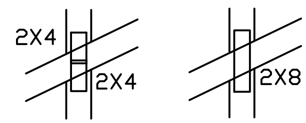
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

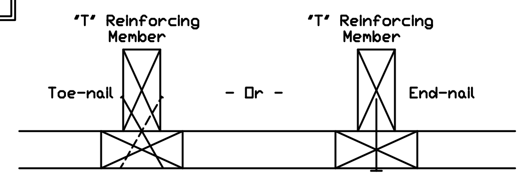
(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

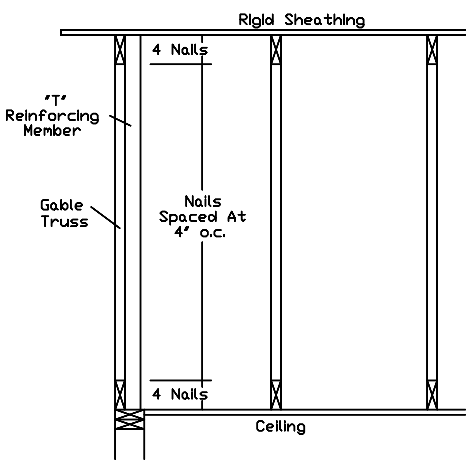
Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ "T" Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with
End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

YOONHWAK KIM LICENSE No. 86367 STATE OF FLORIDA PROFESSIONAL ENGINEER

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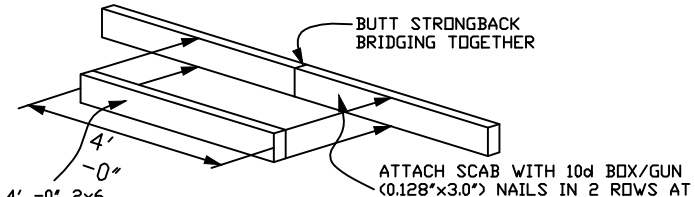
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

03/09/2023 #1336378, Yoonhwak Kim, FL PE #86367

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

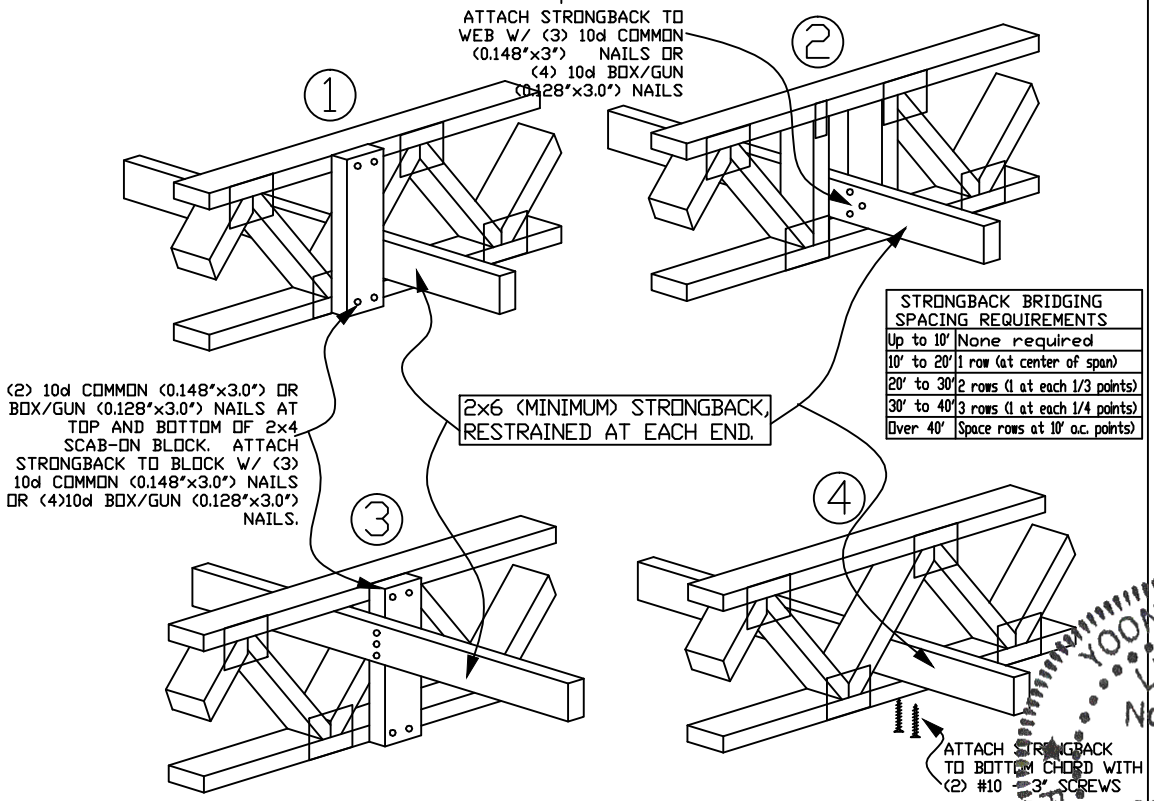
MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

STRONGBACK BRIDGING RECOMMENDATIONS



STRONGBACK BRIDGING SPLICE DETAIL

NOTE: Details 1 and 2 are the preferred attachment methods



STRONGBACK BRIDGING SPACING REQUIREMENTS	
Up to 10'	None required
10' to 20'	1 row (at center of span)
20' to 30'	2 rows (1 at each 1/3 points)
30' to 40'	3 rows (1 at each 1/4 points)
Over 40'	Space rows at 10' o.c. points)

STRONGBACK BRIDGING ATTACHMENT ALTERNATIVES

- All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
- All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
- The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' -0" o.c. (max.)
- The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.

The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended by Alpine.

For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).

YOONHWAK KIM LICENSE No. 86367 STATE OF FLORIDA PROFESSIONAL ENGINEER

ALPINE AN ITW COMPANY

155 Harlem Ave
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TC LL	PSF	REF STRONGBACK
TC DL	PSF	DATE 10/01/14
BC DL	PSF	DRWG STRBRIBR1014
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.	1.00	
SPACING		

FL REG# 278, Yoonhwak Kim, FL PE #86367

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (inches)
D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)

Truss Type	Recommended Camber
Pitched Trusses	1.00 x Deflection from Actual Dead Load
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)

Note: The actual dead load may be considerably less than the design dead load.

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02/09/2023
FL REG#278, Yoonhwak Kim, FL PE #86367

ALPINE
AN ITW COMPANY
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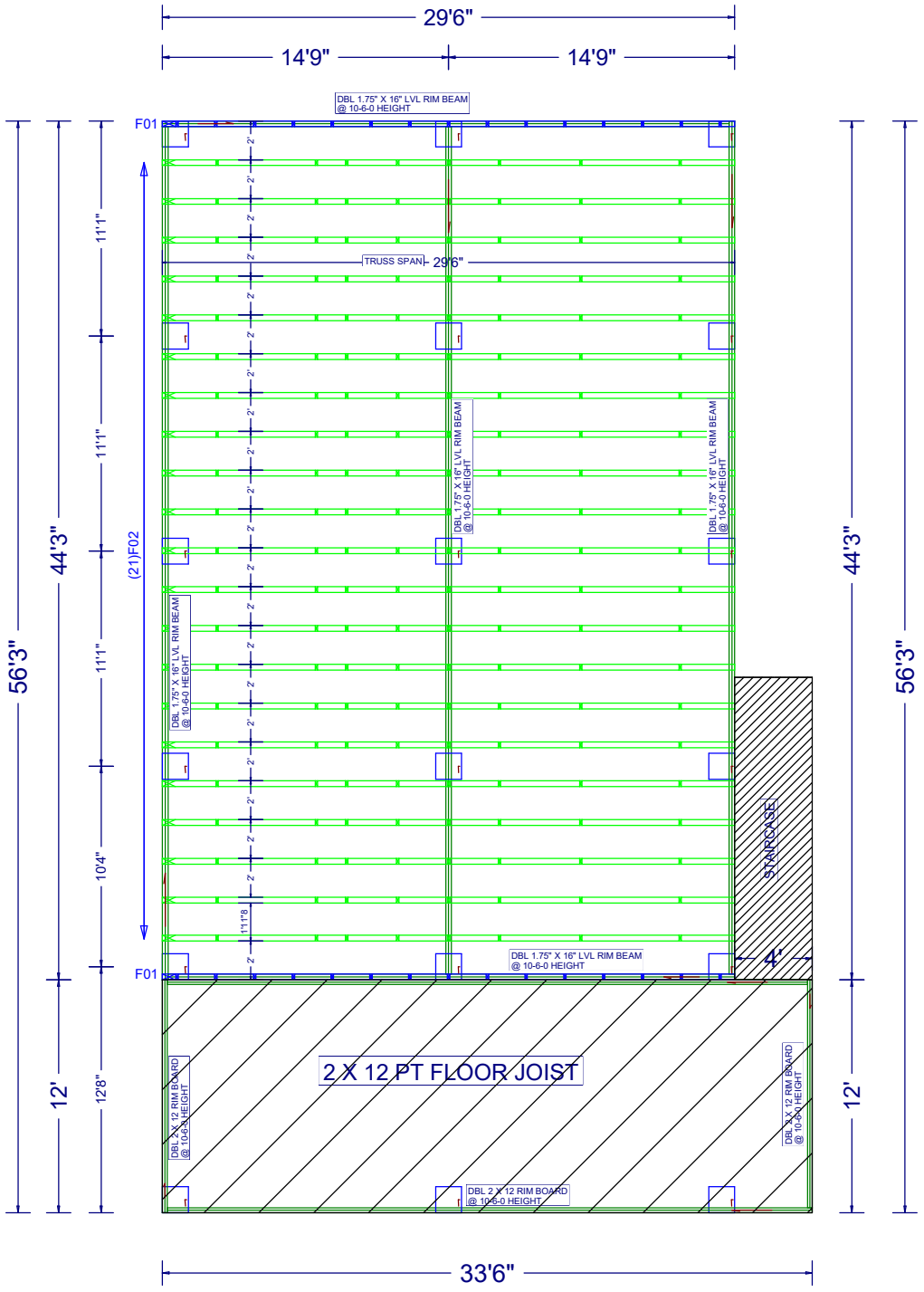
Glenview, IL 60025

REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLCAMB1014



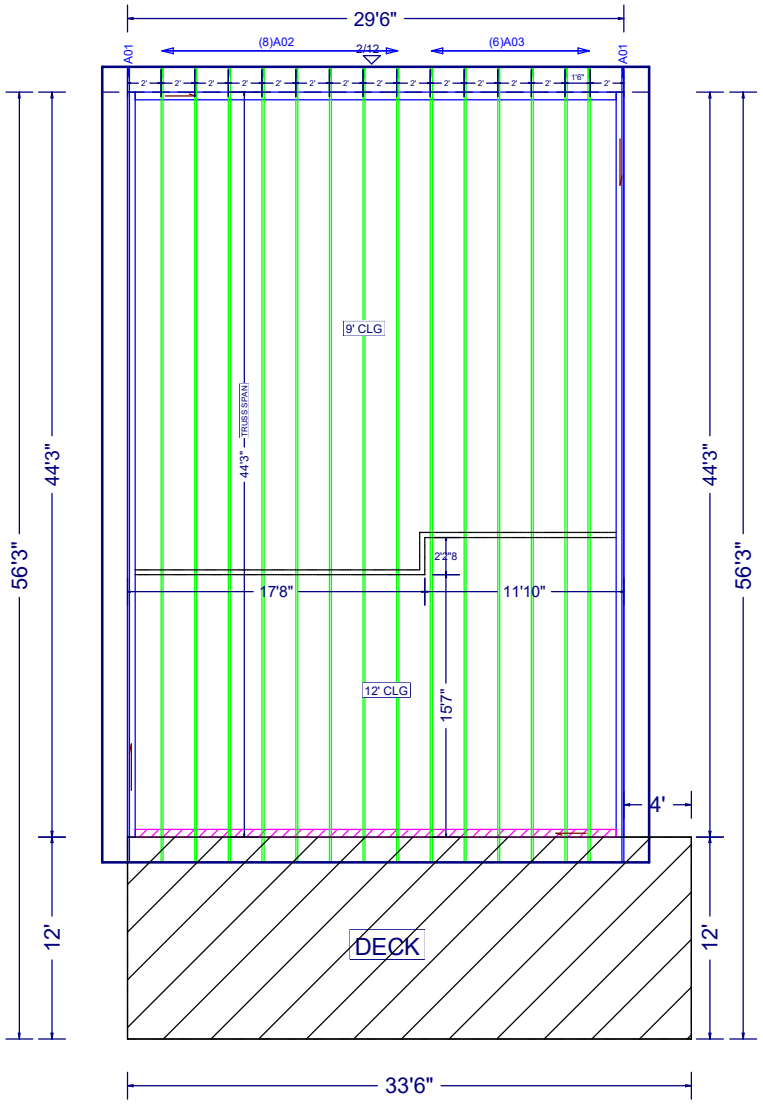
Total Truss Quantity = 23.

JOB #: 22-8690

Job Name: JBC Builders (Busscher) -
Customer: Contractor
Designer: Fill in later
ADDRESS:
SALESMAN: Fill in later
: <Not Found>

JOB NO:
22-8690

PAGE NO:
1 OF 1



Total Truss Quantity = 16.

BEARING WALLS &BEAM HEIGHTS
- 00'11"04 ELEVATION 2 PLY
- 01'04"00 ELEVATION 2 PLY
- 09'00"00 ELEVATION
- 12'00"00 ELEVATION

NOTE:

1. Here the Overhang is not mentioned in the plans,
   So we scaled the plans and considered as 1-6-0 and designed.
   Can you please check and confirm it.
2. In this project there is discripency in between floor plan and Elevations
   about wall height,So we followed and matched with elevations and designed.

JOB #: 22-8690

Job Name: JBC Builders (Busscher) -
Customer: Contractor
Designer: Fill in later
ADDRESS:
SALESMAN: Fill in later
: <Not Found>

JOB NO:
22-8690