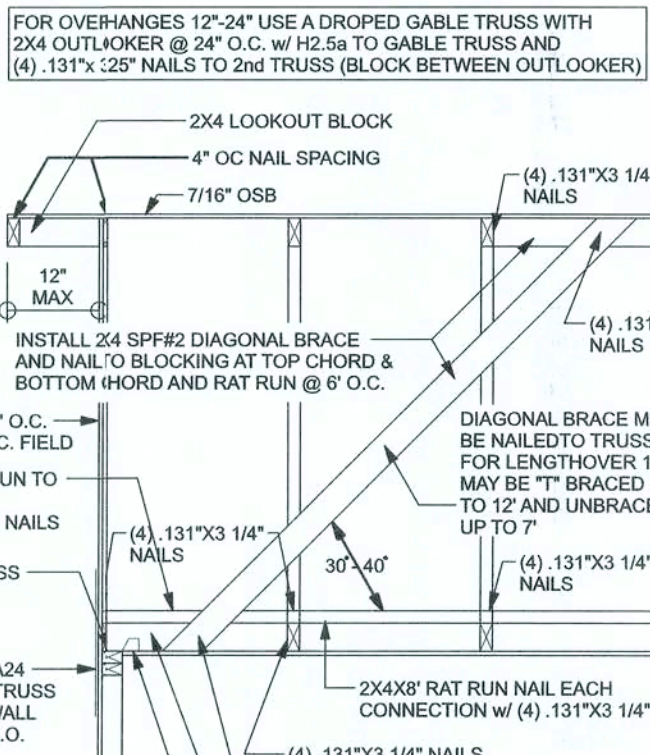


**ONE STORY WALL SECTION**  
SCALE: 3/4\" = 1'-0"

- RING SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:
- 0.113 INCH NOMINAL ROOT SHANK DIAMETER
  - RING DIAMETER OF 0.010 OVER SHANK DIAMETER
  - 16 TO 2 RINGS PER INCH
  - 0.280 INCH FULL ROUND HEAD DIAMETER
  - 2-3/8 INCH NAIL LENGTH

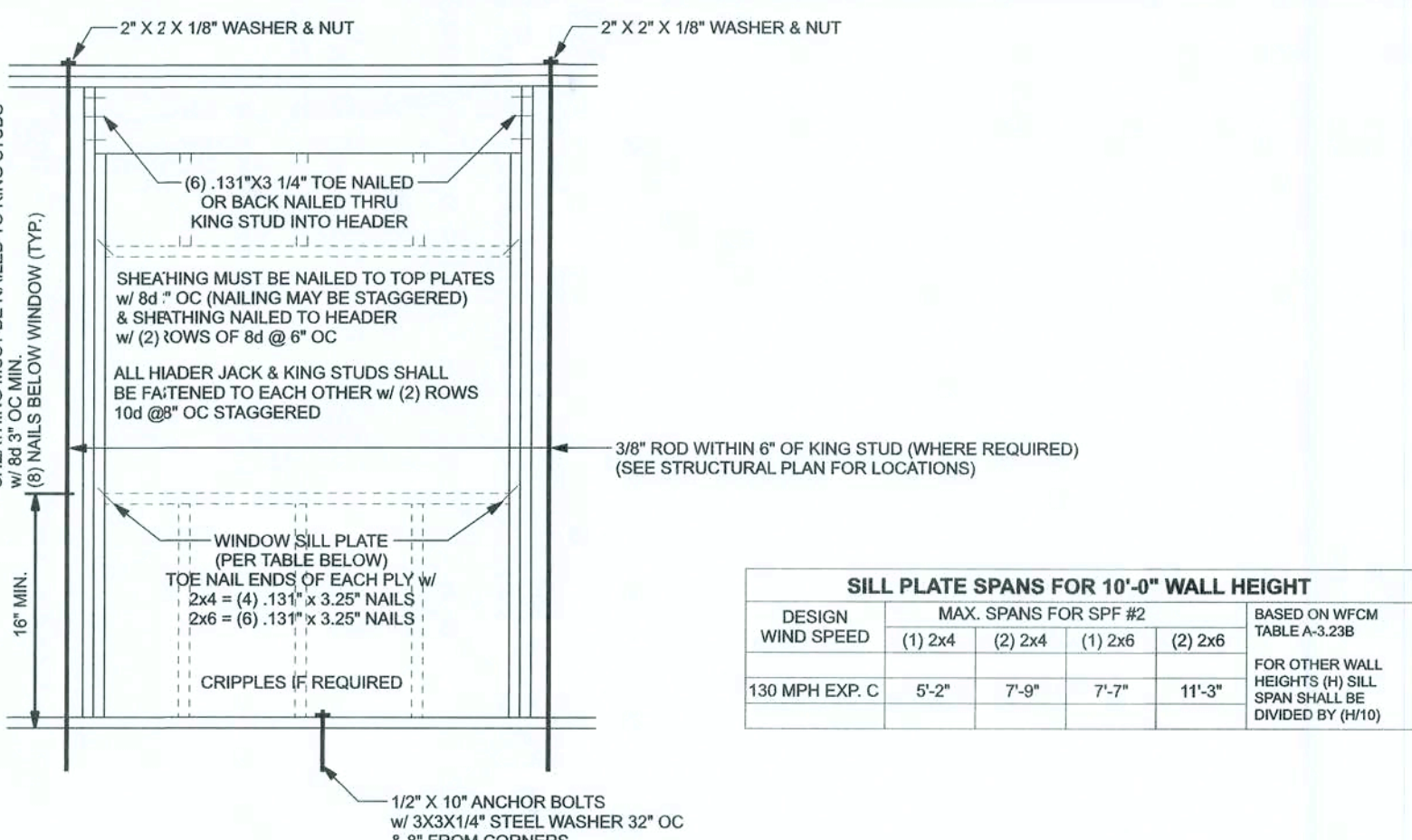
- NAILING PATTERN SHALL BE:
- 4\" GABLE END (SEE GABLE BRACING DETAIL)
  - 6\" OC @ EDGE ALL ZONES
  - 6\" OC @ INTERMEDIATE FRAMING IN ZONE 3
  - 12\" OC @ INTERMEDIATE FRAMING IN ZONE 1 & 2

#### ROOF SHEATHING FASTENING



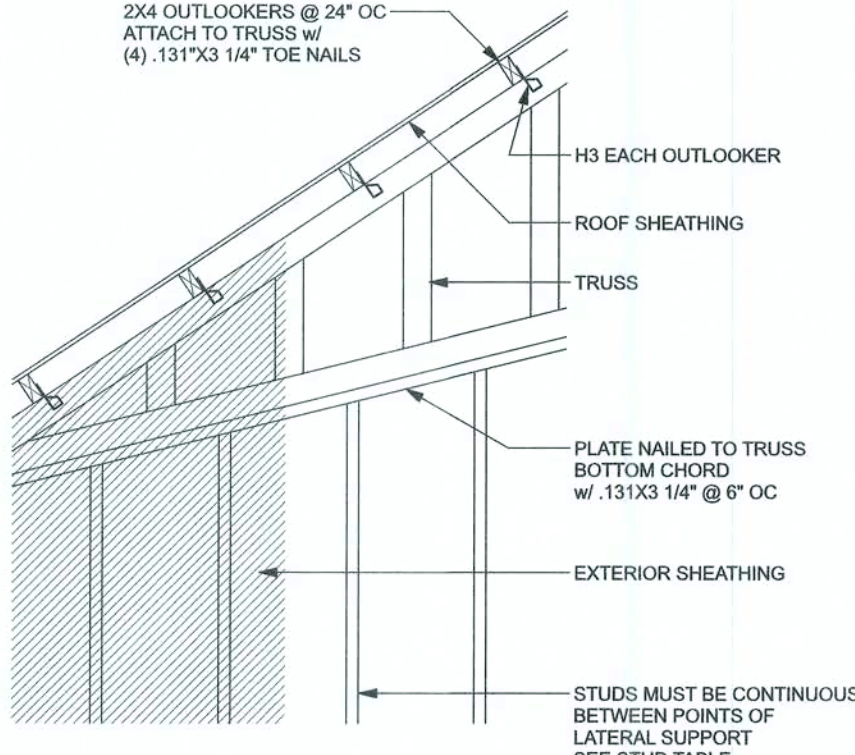
SPACE RAIL RUN & DIAGONAL BRACE 6'-0\" O.C.  
FOR GABLE HEIGHT UP TO 25'-0\" 130 MPH, EXP. C, ENCLOSED

#### (TYP.) GABLE BRACING DETAIL WOOD FRAME

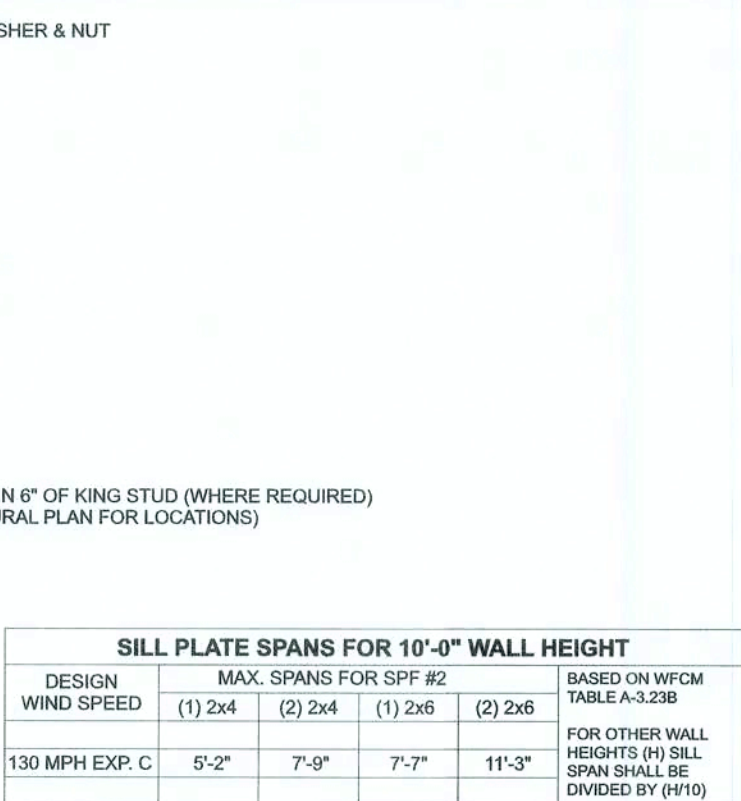


**TYPICAL HEADER STRAPPING DETAIL**  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

#### SHEATHING FOR UPLIFT ATTACHMENT DETAILS ONE STORY WOOD FRAME

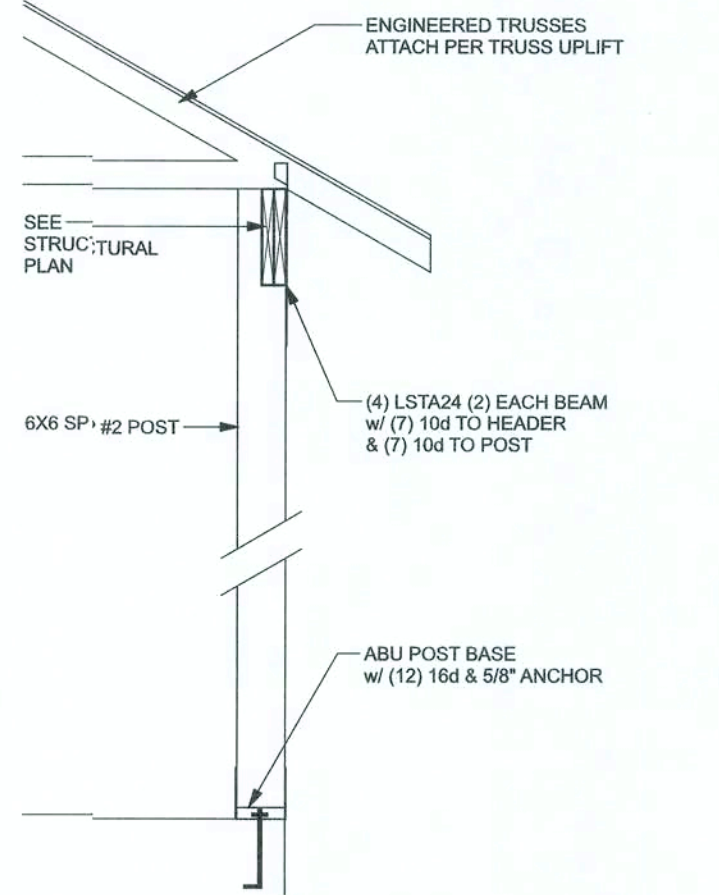


#### (TYP.) GABLE WALL w/ VAULTED CEILING WOOD FRAME

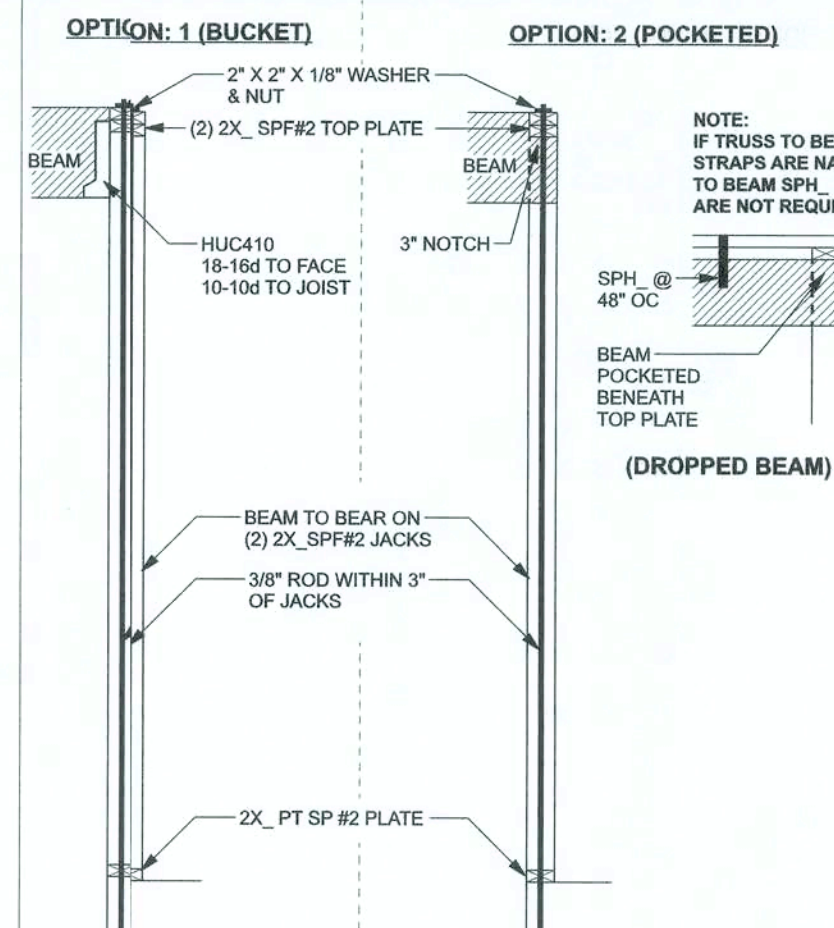


SILL PLATE SPANS FOR 10'-0\" WALL HEIGHT				
DESIGN	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A3.2.8		
WIND SPEED	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"

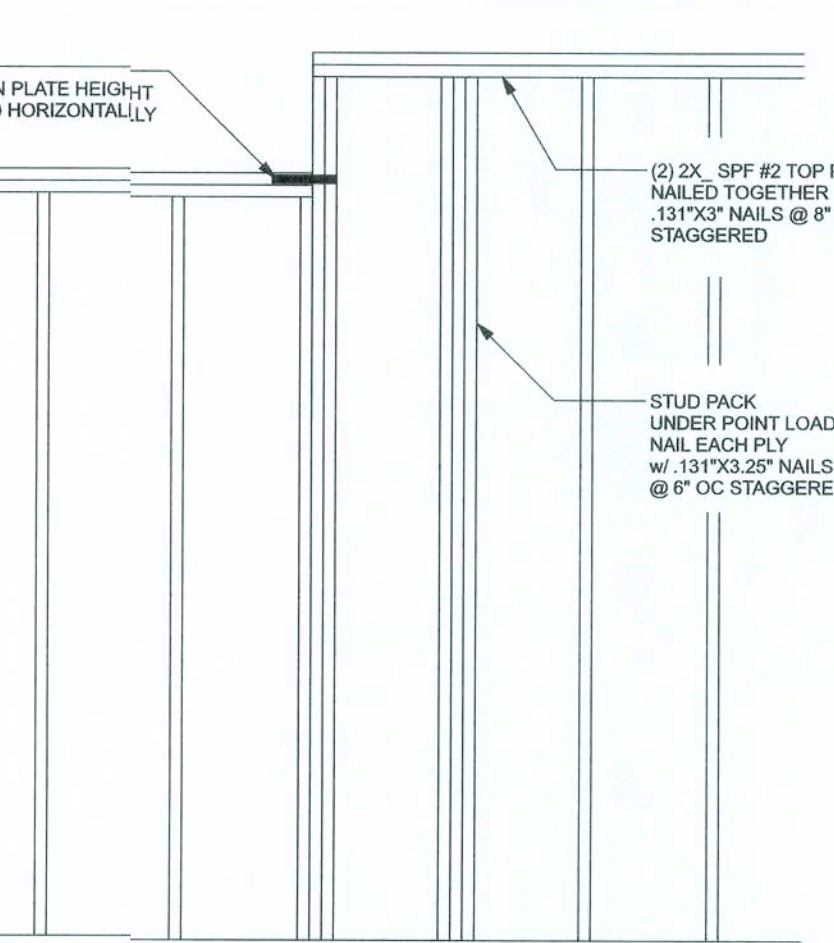
#### (TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ RODS



#### (TYP.) PORCH POST ONE STORY WOOD



**(TYP.) BEAM TO WALL**  
WOOD FRAME w/ RODS



CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	-	-	-
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"	-
575	485	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"	-
1340	1015	H16A	9-10d1 1/2"	9-10d1 1/2"	-
720	850	LTS12-30	6-10d1 1/2"	6-10d1 1/2"	-
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	-
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	-
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LST421	9-10d	9-10d	-
1640	1455	MST424	9-10d	9-10d	-
1030	1030	CS20	7-10d	7-10d	-
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
565	535	SP1	6-10d	4-10d	-
1065	605	SP2	6-10d	6-10d	-
771	771	LST424	10-10d	wrap under or over plate	-
1235	1235	LST424	14-10d	wrap under or over plate	-
Uplift SP	Uplift SPF	Holdowns @ Stemmwall	To Stud / Post	Anchor	
1625	1800	DT122	8-505 1/4\" x 1 1/2"	1/2\" x 12\" Titen HD	-
4235	3640	HTT4	18-16d x 2 1/2"	1/2\" x 12\" Titen HD	-
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1625	1800	DT122	8-505 1/4\" x 1 1/2"	1/2\" x 12\" Titen HD	-
4235	3640	HTT4	18-16d x 2 1/2"	1/2\" x 12\" Titen HD	-
Uplift SP	Uplift SPF	Post Bases @ Stemmwall	To Post	Anchor	
2200		ABU44	12-16d	5/8\" x 12\" Drill & Epoxy	-
2300		ABU66	12-16d	5/8\" x 12\" Drill & Epoxy	-
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200		ABU44	12-16d	5/8\" x 7\" Drill & Epoxy	-
2300		ABU66	12-16d	5/8\" x 7\" Drill & Epoxy	-

#### EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 12\" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16\" O.C. x 0.8 = 12.8\" O.C.)

(1) 2x4 @ 16\" OC	TO 10'-1\" STUD HEIGHT
(1) 2x4 @ 12\" OC	TO 11'-2\" STUD HEIGHT
(1) 2x6 @ 16\" OC	TO 15'-7\" STUD HEIGHT
(1) 2x6 @ 12\" OC	TO 17'-3\" STUD HEIGHT

#### GRADE & SPECIES TABLE

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

#### GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI

WELDED WIRE REINFORCED SLAB: 6\" x 6\" W1.4 x W1.4 FB = 89KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACT APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI. ALL LAP SPICES 40\" DB (2\" FOR #6 BARS); UNCL. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16\" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\" IN CONCRETE OR REINFORCED BOND BEAM OR 16\" IN GROUTED CMU.

#### BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

#### ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DARNED RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



#### DESIGN CRITERIA & LOADS:

BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
<b>WIND LOADS</b>	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	1
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C&amp;C DESIGN PRESSURES</b>	SEE TABLE
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b>	1500 PSF
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE

#### COMPONENT & CLADING DESIGN PRESSURES 130 MP (EXP C) (VUL)

EFFECTIVE WIND AREA (F <sub>T</sub> )	ZONE 4 INTERIOR	ZONE 5 END & FRONTAL OUTSIDE CORNER
0 - 20	+42.6 -46.2	+42.6 -57
<b>GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)</b>		
8'x7' GARAGE DOOR	+22.8 -25.5	
16'x7' GARAGE DOOR	+21.7 -24.1	

B&B Homes

Fonsa & Tawanna Bryant Res.

PROJECT ADDRESS:  
Briley Road  
Columbia, FL

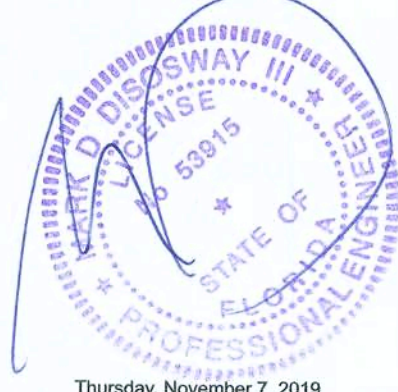
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



Thursday, November 7, 2019

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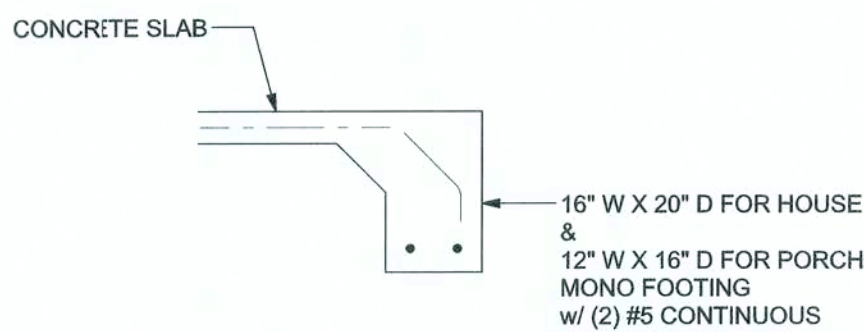
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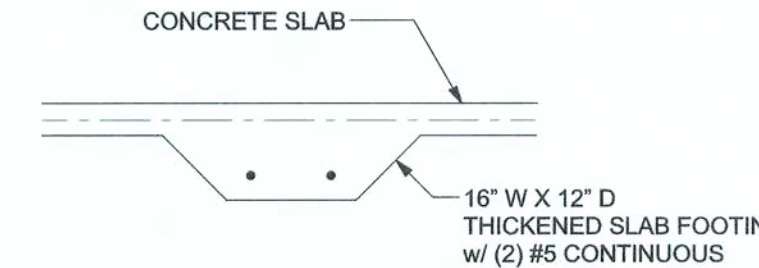
S-1

OF 3 SHEETS

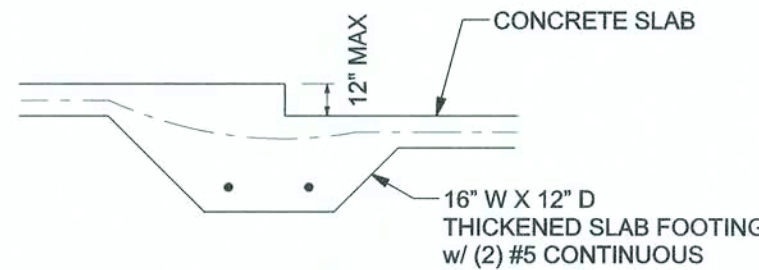




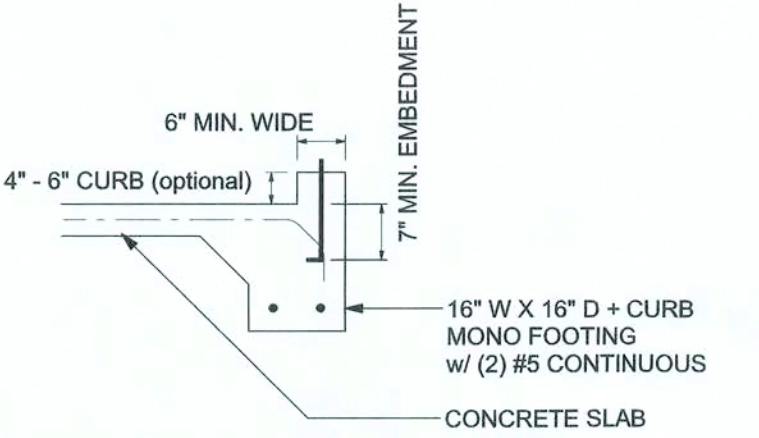
F1 S-2  
OPTIONAL MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



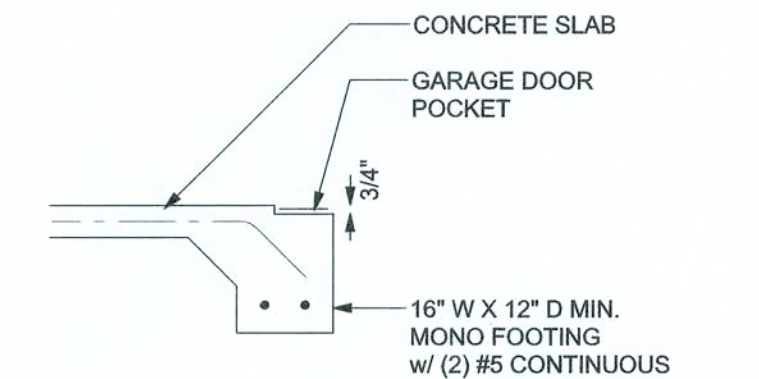
F2 S-2  
INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



F3 S2  
INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"



F4 S-2  
OPTIONAL MONOLITHIC CURB FOOTING  
SCALE: 1/2" = 1'-0"

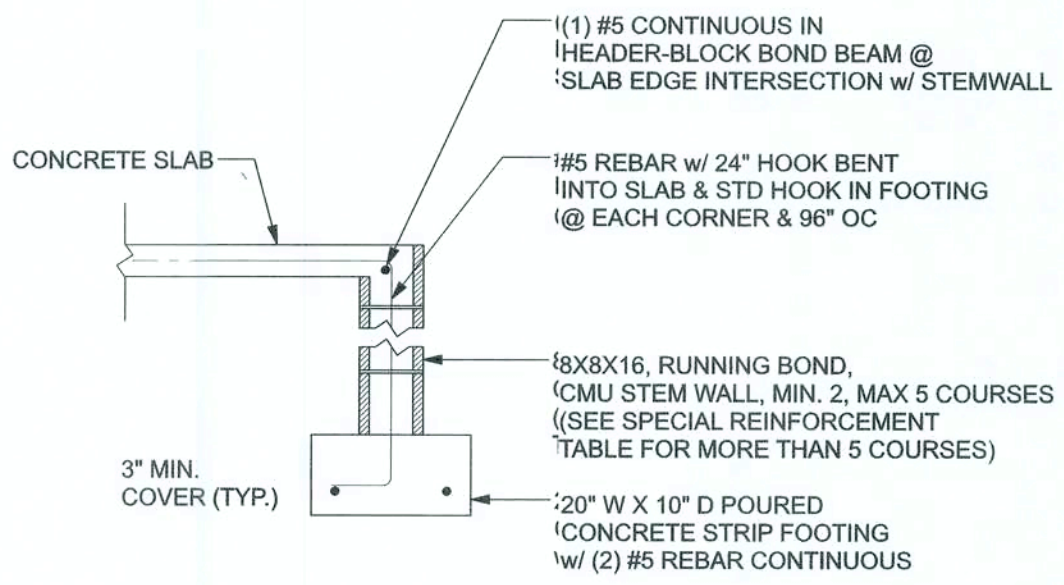


F5 S-2  
GARAGE DOOR POCKET FOOTING  
SCALE: 1/2" = 1'-0"

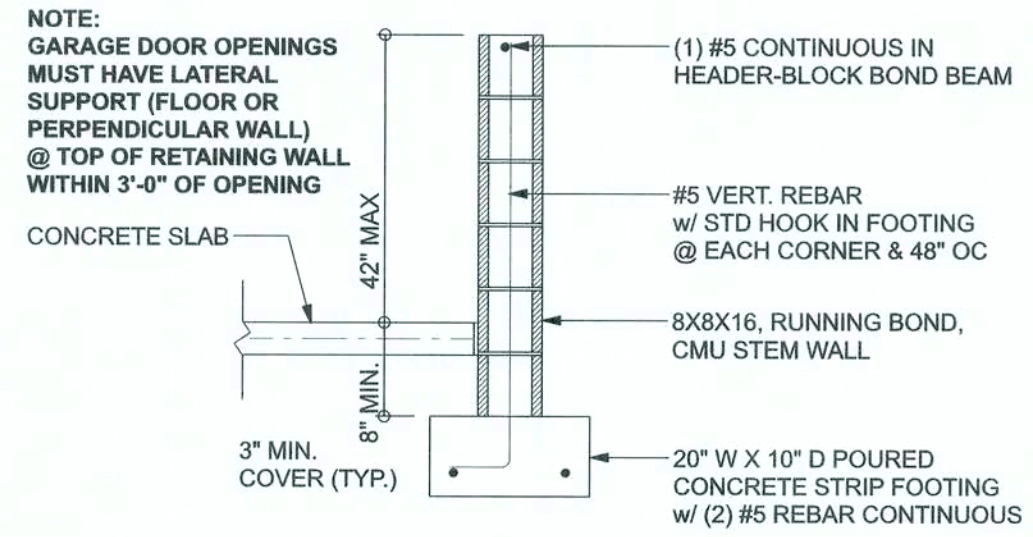
FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES  
FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISCOWAY DESIGN GROUP OR MARK DISCOWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.  
FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.  
FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ #5-1414 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 8" LAPS SEALED w/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL.

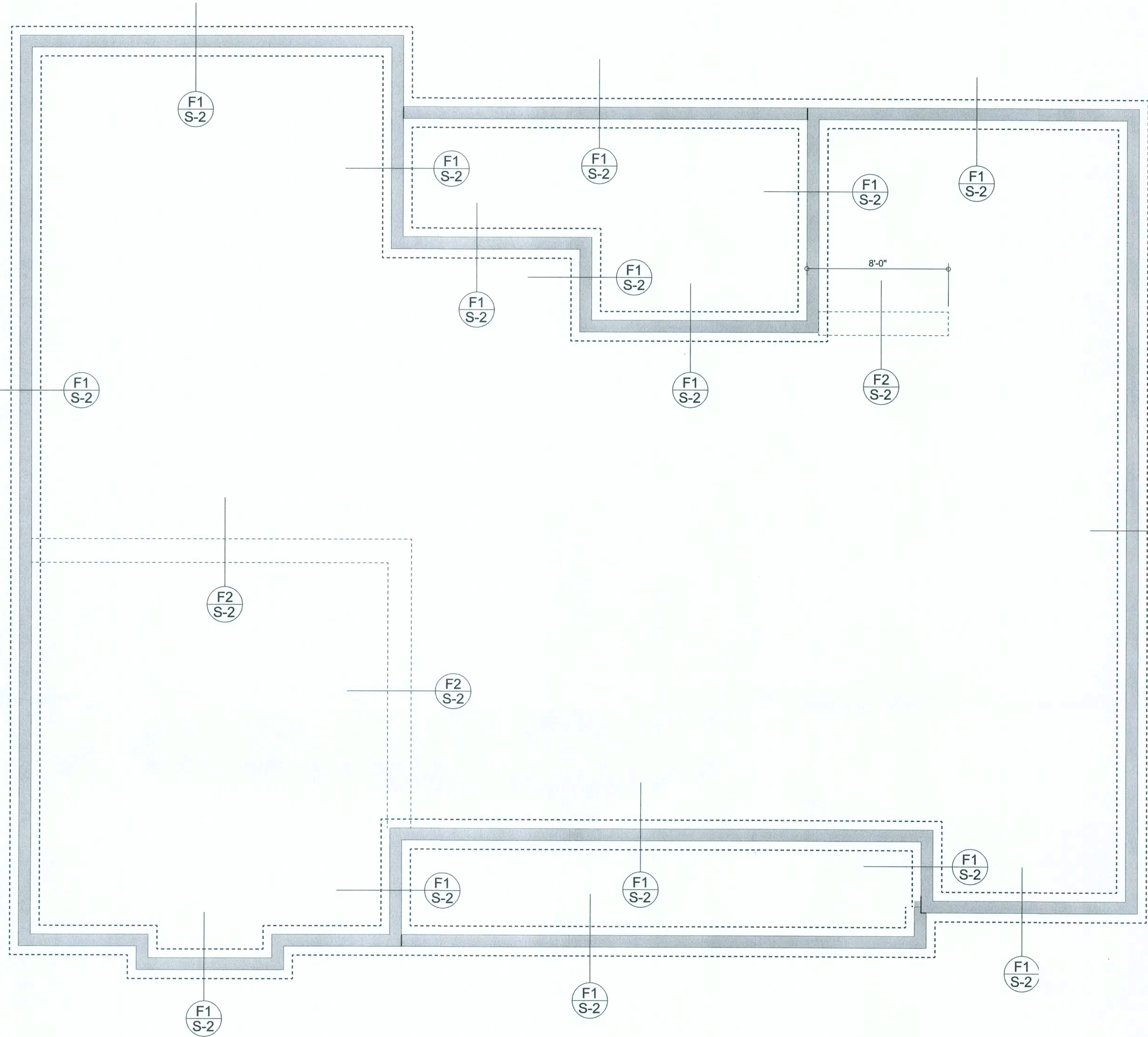


F1 S-2  
STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



F4 S-2  
STEM WALL CURB FOOTING  
SCALE: 1/2" = 1'-0"

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL PER FBC 2017-RES. SECTION R403.1.4



TALL STEM WALL TABLE:  
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Diaphragm ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls Fm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x16"x16" running bond end 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FB5, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM A615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A305, Class G80, 0.80 oz/lb2 = 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb2 or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

B&B Homes

Fonse & Tawanna Bryant Res.

PROJECT ADDRESS:  
Biny Road  
Columbia, FL

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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MARK DISCOWAY P.E. 03915

Thursday, November 7, 2019

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JOB NUMBER:  
191216

S-2  
OF 3 SHEETS



