



Alpine, an ITW Company 6750 Forum Drive, Suite 305 Orlando, FL 32821 Phone: (800)755-6001 www.alpineitw.com

Site Information:

Customer: W. B. Howland Company, Inc.

Job Number: 21-6560

Job Description: ARLENA 7/12 // Sunset

Address: FL

Job Engineering Criteria:			
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.01.01A		
	JRef #: 1Xbc2150008		
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00		
Building Type: Closed			

This package contains general notes pages, 41 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss
1	349.21.0813.22087	A01
3	349.21.0813.27377	A03
5	349.21.0813.32767	B02
7	349.21.0813.37323	C01
9	349.21.0813.53627	C03
11	349.21.0814.02467	D02
13	349.21.0814.08847	D04
15	349.21.0814.15653	D06
17	349.21.0814.20703	D08
19	349.21.0814.25727	D10
21	349.21.0814.30417	D12
23	349.21.0814.39077	D14
25	349.21.0814.44130	D16
27	349.21.0814.59030	D18
29	349.21.0815.04197	G01
31	349.21.0815.14483	J01
33	349.21.0815.19483	J03
35	349.21.0815.22410	J05
37	349.21.0815.26777	J07
39	349.21.0815.30633	J09
41	349.21.0815.36497	J11
43	GBLLETIN0118	
45	BRCLBSUB0119	
47	CNNAILSP1014	

Item	Drawing Number	Truss
2	349.21.0813.25463	A02
4	349.21.0813.31033	B01
6	349.21.0813.34633	B03
8	349.21.0813.39703	C02
10	349.21.0814.00097	D01
12	349.21.0814.04617	D03
14	349.21.0814.13370	D05
16	349.21.0814.18270	D07
18	349.21.0814.23263	D09
20	349.21.0814.27980	D11
22	349.21.0814.32917	D13
24	349.21.0814.41253	D15
26	349.21.0814.48770	D17
28	349.21.0815.01573	D19
30	349.21.0815.12163	G02
32	349.21.0815.17980	J02
34	349.21.0815.21020	J04
36	349.21.0815.24617	J06
38	349.21.0815.28820	J08
40	349.21.0815.32393	J10
42	A14015ENC160118	
44	S14015ENC160118	
46	DEFLCAMB1014	

### **General Notes**

### Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

### Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

### **General Notes** (continued)

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

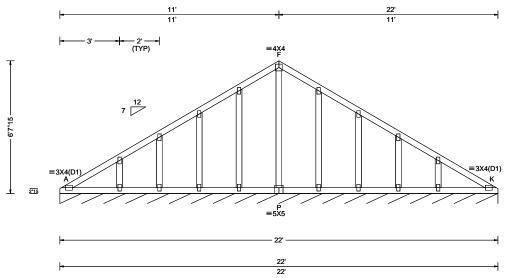
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

### References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 409465 GABL Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T16 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0813.22087 Truss Label: A01 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 K 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 K 999 180
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 K
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.002 E
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.031
l	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.026
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.040
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20
Lumber	•		

▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Loc R+ /R /Rw /U /RL K\* 83 /-/-Wind reactions based on MWFRS K Brg Wid = 264 Min Req = Bearing A is a rigid surface. Members not listed have forces less than 375#

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Plating Notes**

All plates are 2X4 except as noted.

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

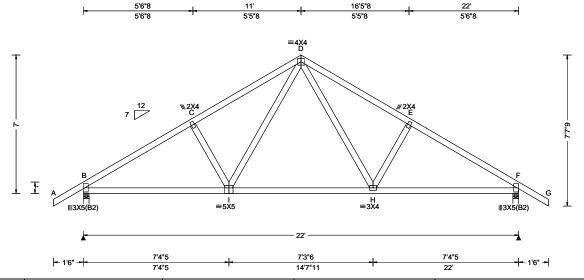
\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

6750 Forum Drive Suite 305 Orlando FL, 32821 SEQN: 409466 COMN Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T29 FROM: CDM Qty: 9 ARLENA 7/12 // Sunset DrwNo: 349.21.0813.25463 Truss Label: A02 / YK 12/15/2021



TCLL: 20.00	Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	4
WEWY 04 04 04 0504 00	TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	PP Deflection in loc L/defl L/# VERT(LL): 0.047 H 999 240 VERT(CL): 0.090 H 999 180 HORZ(LL): 0.024 F HORZ(TL): 0.047 F Creep Factor: 2.0 Max TC CSI: 0.207 Max BC CSI: 0.251	
Lumber	London		, , , ,	VIEW Ver: 21.01.01A.0521.20	E

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL 1075 /-/610 /175 /210 1075 /610 /175 /-Wind reactions based on MWFRS Brg Wid = 3.5 Min Reg = 1.5Brg Wid = 3.5 Min Reg = 1.5 Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords 450 - 1473 488 - 1309 489 - 1308 449 - 1475

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

# Maximum Bot Chord Forces Per Ply (lbs)

Jilolus	rens.comp.	Cilolus	i elis. (	Jonep.	
3 - I	1185 - 247	H-F	1186	- 259	
- H	822 -62	)			

### Maximum Web Forces Per Ply (lbs)

vvebs	rens.Comp.	webs	rens. Comp.	
I-D	492 - 160	D-H	404	- 150



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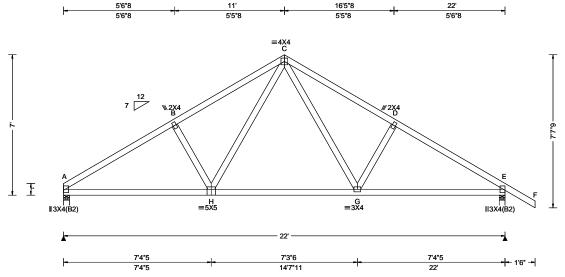
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SEQN: 409467 COMN Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T31 FROM: CDM ARLENA 7/12 // Sunset DrwNo: 349.21.0813.27377 Qty: 1 Truss Label: A03 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffii: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf BCDL: 5.0 psf WWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.038 G 999 240 VERT(CL): 0.077 G 999 180 HORZ(LL): 0.019 E HORZ(TL): 0.039 E Creep Factor: 2.0 Max TC CSI: 0.187 Max BC CSI: 0.210 Max Web CSI: 0.059
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

Maximum Reactions (lbs) Gravity Non-Gravity .oc R+ /R /Rh /Rw /U /RL 911 /524 /193 1020 /610 /-/17 Vind reactions based on MWFRS Brg Wid = 3.5 Min Reg = 1.5Brg Wid = 3.5 Min Req = 1.5 Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) chords Tens.Comp. Chords Tens. Comp. 281 - 1371 312 - 1194 318 - 1206 D-E 275 - 1359

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Wind

Wind loads based on MWFRS with additional C&C member design.

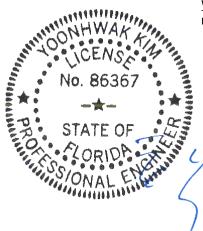
Wind loading based on both gable and hip roof types.

### Maximum Bot Chord Forces Per Ply (lbs)

JIIUIUS	16115.0	onip.	Cilolus	i elis. V	Jonep.	
۸ - H	1105	- 142	G-E	1090	- 135	
1-6	75/	- 1				

#### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. C	omp.
H-C	444 - 100	C - G	428	- 90



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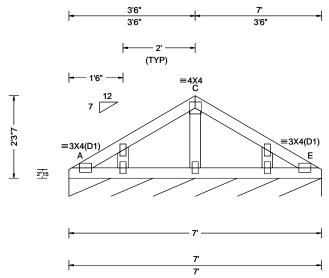
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SEQN: 409468 GABL Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T20 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0813.31033 Truss Label: B01 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.000 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.000 E 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 B
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.000 B
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.027
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.011
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.014
-	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20
Lumber			

#### ▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL E\* 82 /-/-/44 /12 Wind reactions based on MWFRS Brg Wid = 84.0 Min Req = Bearing A is a rigid surface. Members not listed have forces less than 375#

### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Plating Notes**

All plates are 2X4 except as noted.

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

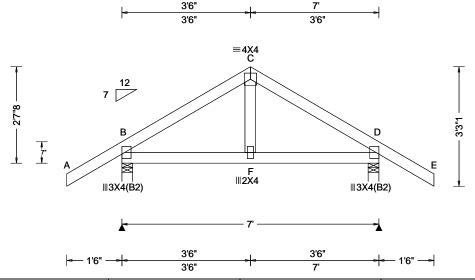
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SEQN: 409469 COMN Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T54 FROM: CDM DrwNo: 349.21.0813.32767 Qty: 1 ARLENA 7/12 // Sunset Truss Label: B02 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	Ī
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code:	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.004 F 999 240 VERT(CL): 0.007 F 999 180 HORZ(LL): 0.002 D HORZ(TL): 0.004 D Creep Factor: 2.0 Max TC CSI: 0.087 Max BC CSI: 0.043 Max Web CSI: 0.013  VIEW Ver: 21.01.01A.0521.20	
1		100,002		L

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 393 /258 393 /-/258 /-Wind reactions based on MWFRS В Brg Wid = 3.5 Min Reg = 1.5Brg Wid = 3.5 Min Req = 1.5 Bearings B & D are a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

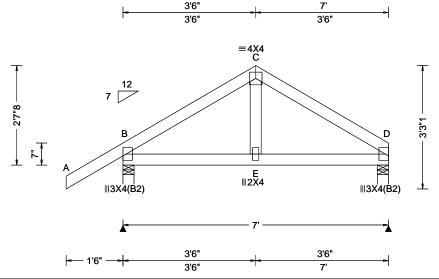
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SEQN: 409470 COMN Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T1 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0813.34633 Truss Label: B03 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 E 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.006 E 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 D
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 D
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.077
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.048
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.013
-	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 404 /-/258 /80 279 /-/165 /42 /-Wind reactions based on MWFRS Brg Wid = 3.5 Min Reg = 1.5Brg Wid = 3.5 Min Req = 1.5 Bearings B & D are a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

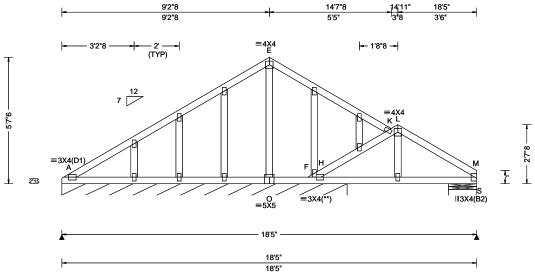
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SEQN: 409471 GABL Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 ТЗ FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0813.37323 Truss Label: C01 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 I 999 240	Loc R+ /R- /Rh /Rw /U /RL
DOLL. 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.007 I 999 180	A* 99 /- /- /54 /16 /11
10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.002 J	S 267 /- /- /162 /37 /-
Dec  d   10 00	EXP: C Kzt: NA		HORZ(TL): 0.003 J	Wind reactions based on MWFRS
NODOLL, 40 00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	A Brg Wid = 151 Min Req = -
0.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.047	S Brg Wid = 15.0 Min Req = 1.5
	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.029	Bearings A & S are a rigid surface.
1	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.042	Members not listed have forces less than 375#
1 ' '	Loc. from endwall: Any	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.
	GCpi: 0.18	Plate Type(s):		Chords Tens.comp.
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	F-K 337 -383

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Plating Notes**

All plates are 2X4 except as noted.

(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

### Loading

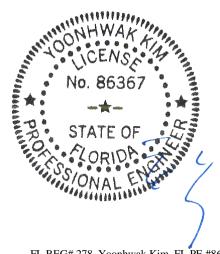
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

### **Additional Notes**

See DWGS S14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



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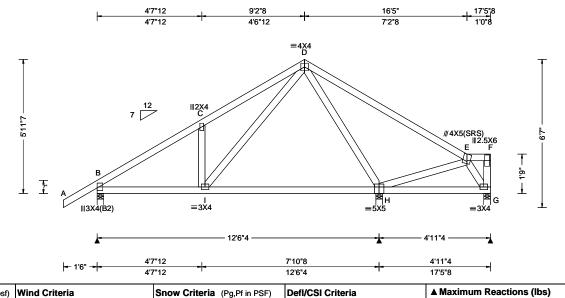
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SEQN: 409472 COMN Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T2 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0813.39703 Truss Label: C02 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)	
TCLL: 20.00	Wind Std: ASCE 7-16	Pa: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity No	on-G
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.012 C 999 240	Loc R+ /R- /Rh /Rw	/ U
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.025 C 999 180	B 575 /- /- /360	/10
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 C	H 912 /- /- /521	/14
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.007 C	G 123 /-35 /- /51	/40
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS	_
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.290	B Brg Wid = 3.5 Min Req = 1.5	
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.148	H Brg Wid = 3.5 Min Req = 1.5 G Bra Wid = 3.5 Min Rea = 1.5	-
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.303	Bearings B, H, & G are a rigid surfa	-
-	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		Members not listed have forces less	
	GCpi: 0.18	Plate Type(s):		Maximum Top Chord Forces Per	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20		Ten
Lumban				-	

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Non-Gravity

/104

/140 /-

221

- 598

/RL

/155

C-D

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp.

B - C

B - I 464

Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs

110 -613

Tens. Comp. I - D 523 - 147 H-E 273 - 462 D-H 173 - 696

### Lumber

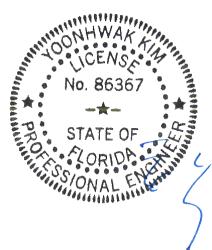
Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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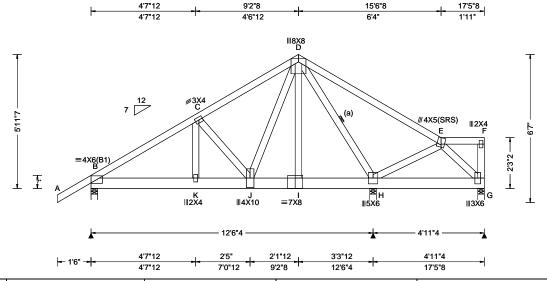
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SEQN: 409505 COMN Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T4 FROM: CDM DrwNo: 349.21.0813.53627 Qty: 1 ARLENA 7/12 // Sunset Truss Label: C03 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.042 J 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.085 J 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.010 C
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.021 C
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.260
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.330
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case	Max Web CSI: 0.431
'	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP M-31;

### **Bracing**

(a) Continuous lateral restraint equally spaced on

### **Special Loads**

(Lumber	Dur.Fac.=1	.25 / Plate [	Dur.Fac.=1.2	25)
TC: From	63 plf at	-1.50 to	63 plf at	7.06
TC: From	32 plf at	7.06 to	32 plf at	11.06
TC: From	63 plf at	11.06 to	63 plf at	17.46
BC: From	5 plf at	-1.50 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	7.06
BC: From	10 plf at	7.06 to	10 plf at	11.06
BC: From	20 plf at	11.06 to	20 plf at	17.46
BC: 2346 lb	Conc. Load	at 7.06		
BC: 1090 lb	Conc. Load	at 9.06		
BC: 1093 lb	Conc. Load	at 11.06		

#### **Purlins**

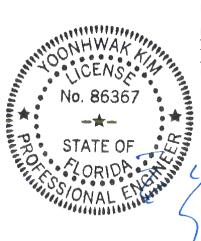
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

### **Additional Notes**

Negative reaction(s) of -203# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.



#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw / U /RL В 1934 /350 /-4092 /-/-/-/-/712 98 /-203 /-/13 Wind reactions based on MWFRS Brg Wid = 3.5 Min Req = 1.6 Brg Wid = 3.5 Min Req = 3.0Brg Wid = 3.5 Min Rea = 1.5Bearings B, H, & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens. Comp. Chords Tens.Comp.

B - C	510 - 2913	D-E	473	- 102
C-D	498 - 2837			

### Maximum Bot Chord Forces Per Ply (lbs)

ıp.
160 160

#### Maximum Web Forces Per Ply (lbs) Tens Comp Tens Comp Webs

******	rono.comp.	******	rono. Comp.
J - D D - I	2602 - 407 1596 - 297	D - H H - E	642 - 3534 125 - 387

FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

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FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.00097 Truss Label: D01 / YK 12/15/2021 13'5"15 19'10"2 26'4" 6'5"15 6'4"3 6'5"15 **∥7X6** =7<u>X</u>6 ₩6X8 =3X4 D <u>\*</u> 5'3"9 =5X6(B3) 7. G ⊪3X8∥ J ⊪3X4 =7X6 H ≡7X10 26'4 6'5"15 6'4"3 6'5"15 13'5"15 19'10"2 26'4" ▲ Maximum Reactions (lbs)

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.094 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.197 D 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.024 H
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.050 H
NCBCLL: 0.00	Mean Height: 11.28 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.357
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.399
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: No	Max Web CSI: 0.646
'	Loc. from endwall: NA	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20
Lumber		Wind	

Job Number: 21-6560

#### Wind

Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

### Maximum Bot Chord Forces Per Ply (lbs)

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

/Rh

Min Req = 1.9

Chords

E-F

/-

Wind reactions based on MWFRS

585 - 3543

608 - 3821

Chords	Tens.Comp.	Chords	Tens. Comp.	
B - J J - I	2964 - 471 2984 - 470	I - H	3836 - 623	

Non-Gravity

/381

/375

/RL

/-

Tens. Comp.

456 - 2875

456 - 2875

/Rw /U

Cust: R 215 JRef: 1Xbc2150008

T25

### Maximum Web Forces Per Ply (lbs)

Gravity

Brg Wid = 3.5

Brg Wid = -Bearing B is a rigid surface.

Chords Tens.Comp.

Loc R+

2236 /-

2346

В

В

B - C

C - D

Webs	Tens.Comp.	Webs	Tens. Comp.
C-J	589 0	E-H	347 - 912
C - I	1004 - 165	H - F	3426 - 544
D - H	201 - 1161	F-G	429 - 2222

### Hangers / Ties

Webs: 2x4 SP M-31;

SEQN: 409502

HIPM

Ply: 1

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP M-31; Bot chord: 2x6 SP 2400f-2.0E;

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=26'1" uses the following support conditions: 26'1" Bearing G (26'1", 9'1"2) HUS26 Supporting Member: (1)2x6 SP 2400f-2.0E (14) 0.148"x3" nails into supporting member. (6) 0.148"x3" nails into supported

#### Loading

member.

#1 hip supports 7-0-0 jacks with no webs.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

> FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

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SEQN: 409473 HIPM Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T30 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.02467 Truss Label: D02 / YK 12/15/2021 4'6"8 9' 14'9"15 20'6"2 26'4" 4'6"8 4'5"8 5'9"15 5'8"3 5'9"15 =4X5 D ∥2X4 E ≡5X6 ≡4X4 G ∥2X4 5'10" 7 H ∥2.5X6 K ≡3X4 ≡5X10 ≡4X4 =4X4(B2) 26'4" 4'6"8 10'3"7 5'8"3 5'9"15 - 1'6" -4'6"8 14'9"15 20'6"2 26'4" ▲ Maximum Reactions (lbs)

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
Loading Criteria (psf)   TCLL: 20.00   TCDL: 10.00   BCLL: 0.00   BCDL: 10.00   Des Ld: 40.00   NCBCLL: 10.00   Soffit: 2.00   Load Duration: 1.25   Spacing: 24.0   "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	Defl/CSI Criteria
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=26'1" uses the following support conditions: 26'1" Bearing H (26'1", 9'1"2) HUS26 Supporting Member: (1)2x6 SP 2400f-2.0E (14) 0.148"x3" nails into supporting member. (4) 0.148"x3" nails into supported member.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

601 - 1738 C - D 679 - 1678 D-E 721 - 1326

# Maximum Bot Chord Forces Per Ply (lbs)

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

/Rh

/-

Wind reactions based on MWFRS Brg Wid = 3.5

Gravity

Loc R+

1201 /-

Brg Wid = -Bearing B is a rigid surface.

Chords Tens.Comp.

1090

В

B - K 1412 - 641 J - I 950 - 537	_
K - J 1160 - 629	

Non-Gravity

/196

/219 /-

Tens. Comp.

508

720 - 1326

/RL

/210

/Rw /U

/733

/568

Min Reg = 1.5

Chords

### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens.	Comp.
K-D	433	- 67	I-G	1270	- 710
J - F	531	- 264	G - H	648	- 1040
F-I	553	- 784			



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SEQN: 409474 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T34 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.04617 Truss Label: D03 / YK 12/15/2021 5'6"8 18'4' 22'0"8 26'4" 5'6"8 5'5"8 7'4" 3'8"8 4'3"8 ≢5X5 /// 4X5(SRS) ∥2X<u>4</u> 7

I ≡5X5

26'4"

9'1"9

H ≡3X4

3'8"

VIEW Ver: 21.01.01A.0521.20

	5'6"8	14'8"	18'4"	
	568	148	184	
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	<b>A</b>
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	1.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.044 B 999 240	Lo
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.092 B 999 180	Α
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.019 G	G
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.039 G	W
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	Α
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.231	G
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.275	Be
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.473	Ma
• •	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		Ch
	GCpi: 0.18	Plate Type(s):		=

WAVE

J ≡3X4

5'6"8

	▲ Maxir	num Rea	ctions (	lbs)					
		Gravity		No	on-Grav	/ity			
0	Loc R+	- /R-	/ Rh	/ Rw	/ U	/ RL			
0	A 109	7 /-	/-	/649	/172	/165			
-	G 109	3 /-	/-	/563	/206	/-			
-	Wind re	actions b	ased on	MWFRS					
	A Brg	Wid = 3.	5 Min	Req = 1.5	5				
	G Brg	Wid = -		-					
	Bearing	A is a rig	id surfac	e.					
	Members not listed have forces less than 375#								
	Maximum Top Chord Forces Per Ply (lbs)								
	Chords	Tens.Co	mp.	Chords	Tens.	Ćomp.			
	A - B	522 -	1729	C-D	498	- 1044			
	B-C	632 -	-	D-E	505	- 1142			

G ∥2.5X6

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Wind Duration: 1.60

**∥3X5(B2)** 

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=26'1" uses the following support conditions: 26'1" Bearing G (26'1", 9'1"2) HUS26 Supporting Member: (1)2x6 SP 2400f-2.0E (14) 0.148"x3" nails into supporting member. (4) 0.148"x3" nails into supported member.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

# Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.	
A - J	1406 - 534	I - H	951 - 405	
J - I	1063 - 464	H - G	855 - 397	

### Maximum Web Forces Per Ply (lbs)

vvebs	rens.Comp.	vvebs	Tens. Comp.		
I-C	524 - 135	F-G	578 - 1244		



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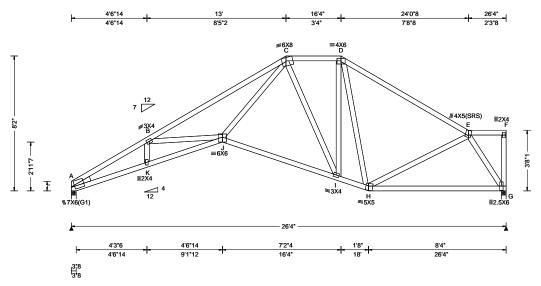
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SEQN: 409475 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T38 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.08847 Truss Label: D04 / YK 12/15/2021



Loading Criteri	a (psf) Wind Criteria	a	Snow Cri	<b>iteria</b> (Pg	,Pf in PSF)	DefI/CSI Cri	iteria		
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: Spacing: 24.0 "	Wind Std: A Speed: 130 i Enclosure: CI Risk Category EXP: C Kzt: Mean Height: TCDL: 5.0 psi BCDL: 5.0 psi BCDL: 5.0 psi MWFRS Para C&C Dist a: 3 Loc. from enc	ASCE 7-16 mph losed y; II : NA 15.00 ft f f f flallel Dist: h/2 to h 0.00 ft dwall: not in 9.00 ft	Pg: NA Pf: NA Lu: NA Snow Dui Building ( FBC 7th E TPI Std: Rep Fac: FT/RT:20 Plate Typ	Ct: NA  Cs: NA ration: NA  Code: Ed. 2020 F 2014 Yes (0)/10(0)	CAT: NA Ce: NA	PP Deflectic VERT(LL): VERT(CL): HORZ(LL): HORZ(TL): Creep Factc Max TC CSI Max BC CSI Max Web CSI	0.135 J 0.283 J 0.092 G 0.192 G or: 2.0 : 0.540 : 0.430	999 999 - -	240
	Wind Duration	n: 1.60	WAVE			VIEW Ver: 2	1.01.01A.	0521.2	20
Lumber	•	•				•			

▲ M	aximu	ım Rea	ctions	(lbs)			
	G	ravity		N	on-Grav	vity	
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
Α	1108	/-	/-	/651	/171	/195	
G	1102	/-	/-	/577	/191	/-	
Win	d read	tions ba	sed on	MWFRS			
Α	Brg V	/id = 3.5	5 Mir	Reg = 1.	5		
G	Brg V	/id = 3.5	5 Mir	Reg = 1.	5		
Bea	rings /	4 & G a	re a riq	id surface.			
Members not listed have forces less than 375#							
Maximum Top Chord Forces Per Ply (lbs)							
				Chords		•	
A -	 В	917 - 3	438	C-D	411	- 1000	
B -		785 - 2		D-E	367		

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31; Lt Stub Wedge: 2x4 SP M-31;

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.



Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp. Chords Tens. Comp. A - K 3009 - 890 1030 - 261 K-J 3063 - 907 H-G 730 - 256 1296 - 346 J - I

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.		Webs	Tens.	Comp.
B-J	288	- 482	I - D	500	- 106
J - C	1928	- 428	E-G	463	- 1283
C - I	193	- 5/12			

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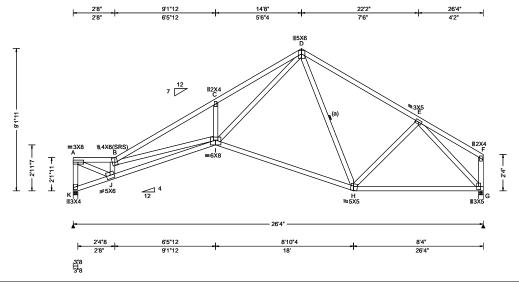
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SEQN: 409476 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T42 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.13370 Truss Label: D05 / YK 12/15/2021



Loading Criteria (psf) Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "  Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf BCDL: 5.0 psf WWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes	PP Deflection in loc L/defl L/# VERT(LL): 0.133 C 999 240 VERT(CL): 0.279 C 999 180 HORZ(LL): 0.087 G HORZ(TL): 0.183 G Creep Factor: 2.0 Max TC CSI: 0.307 Max BC CSI: 0.330 Max Web CSI: 0.468  VIEW Ver: 21.01.01A.0521.20

▲ Maximum Reactions (lbs)								
	G	ravity		N	on-Grav	vity		
Loc	R+	/ R-	/Rh	/ Rw	/ U	/ RL		
K 1	1108	/-	/-	/607	/13	/180		
G 1	1102	/-	/-	/609	/10	/-		
Wind	Wind reactions based on MWFRS							
K	Brg W	/id = 3.5	Min	Req = 1.5	5			
G I	•							
Bear	Bearings K & G are a rigid surface.							
Members not listed have forces less than 375#								
Maximum Top Chord Forces Per Ply (lbs)								
Chor	ds T	ens.Con	np.	Chords	Tens.	Ćomp.		
A - B	3	494 - 22	233	C-D	698	- 3077		
B - C	;	558 - 3	101	D-E	307	- 1171		

#### Lumbe

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Bracing**

(a) Continuous lateral restraint equally spaced on

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

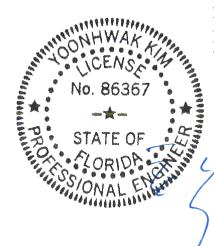
### Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Max JT VERT DEFL: LL: 0.13" DL: 0.15". See detail DEFLCAMB1014 for camber recommendations. Provide for adequate drainage of roof.



# Maximum Bot Chord Forces Per Ply (lbs)

Choras	rens.comp.	Choras	rens. Comp.	
J - I	2806 - 653	H-G	903 - 181	
I - H	1032 - 104			

### Maximum Web Forces Per Ply (lbs)

webs	rens.comp.	webs	rens. Comp.	
A - K	282 - 1079	C - I	224 - 376	
A - J	2459 - 542	I - D	2384 - 464	
J - B	506 - 1883	E-G	274 - 1304	

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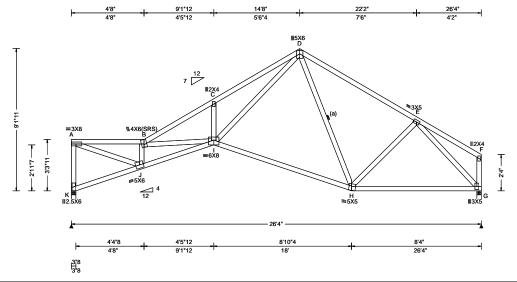
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SEQN: 409477 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T43 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.15653 Truss Label: D06 / YK 12/15/2021



BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "  Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf SDL: 5.0 psf C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft  Enclosure: Closed Snow Duration: NA Snow Duration: NA HORZ(LL): 0.282 C 999 180  VERT(CL): 0.282 C 999 180  HORZ(LL): 0.085 G ENCRETCAL SNA SNOW Duration: NA HORZ(LL): 0.079 G ENCRETCAL SNA HORZ(LL): 0.085 G ENCRETCAL SNA HORZ(LL): 0.	Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
Wind Duration: 1.60 WAVE VIEW Ver: 21.01.01A.0521.20	TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.135 C 999 240 VERT(CL): 0.282 C 999 180 HORZ(LL): 0.085 G HORZ(TL): 0.179 G Creep Factor: 2.0 Max TC CSI: 0.218 Max BC CSI: 0.318 Max Web CSI: 0.468	

▲ Ma	▲ Maximum Reactions (Ibs)								
	G	ravity		. N	Ion-Gra	vity			
Loc	R+	/ R-	/Rh	/ Rw	/ U	/ RL			
κ	1108	/-	/-	/582	/38	/175			
G	1102	/-	/-	/613	/7	/-			
Win	d reac	tions bas	sed on	<b>MWFRS</b>					
K	Brg W	/id = 3.5	Min	Req = 1	.5				
G	Brg W	/id = 3.5	Min	Req = 1.	.5				
Bea	rings k		e a rigi	d surface					
Men	bers	not listed	d have	forces les	s than	375#			
Max	imum	Top Ch	ord F	orces Pe	r Plv (lb	s)			
				Chords		•			
A - E	3	625 - 25	521	C-D	717	- 3023			
B - 0	)	604 - 30	023	D - E	315	- 1172			

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Bracing**

(a) Continuous lateral restraint equally spaced on

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

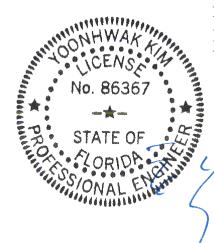
### Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Max JT VERT DEFL: LL: 0.13" DL: 0.15". See detail DEFLCAMB1014 for camber recommendations. Provide for adequate drainage of roof.



I-H 1033 - 115 Maximum Web Forces Per Ply (lbs)

Chords

Tens. Comp.

- 186

902

Maximum Bot Chord Forces Per Ply (lbs)

Webs

2963 - 709

Chords Tens.Comp.

Tens. Comp. Webs Tens.Comp. A - K 360 - 1058 I - D 2324 - 479 A - J 2674 - 659 E-G 281 - 1302 J - B 516 - 1799

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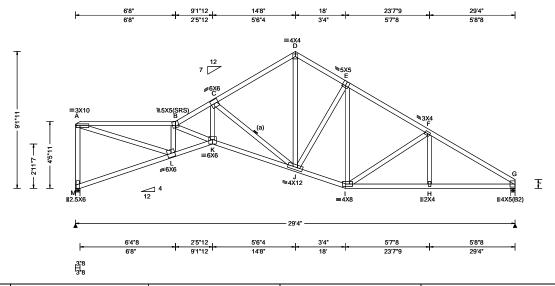
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SEQN: 409478 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T12 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.18270 Truss Label: D07 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	Ī
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.164 K 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.344 K 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.105 G	
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.220 G	
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.455	
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.333	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.323	
'	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	
Louishau		•		_

	4.84. *** B (U)								
▲ M	▲ Maximum Reactions (lbs)								
	(	Gravity		Non-Gravity					
Loc	R+	/ R-	/ Rh	/ Rw	/U	/ RL			
М	1230	) /-	/-	/637	/72	/220			
G	1227	' /-	/-	/717	/8	/-			
Win	nd rea	ctions b	ased on	MWFRS					
М	Brg \	Wid = 3.	5 Min	Req = 1.5	5				
G	Brg \	Wid = 3.	5 Min	Req = 1.5	5				
Bea	irings	M & G	are a rig	id surface.					
Mer	mbers	not liste	ed have	forces less	than	375#			
Max	kimui	m Top C	hord F	orces Per	Ply (lb	s)			
	Chords Tens.Comp. Chords Tens.Comp.								
Α-	В	699 -	2970	D-E	401	- 1430			
B -	С	723 -	3455	E-F	385	- 1551			
C-	D	384 -	1468	F-G	385	- 1943			

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Bracing**

(a) Continuous lateral restraint equally spaced on

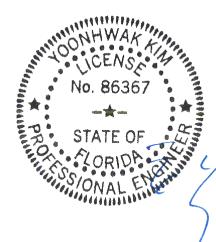
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

Max JT VERT DEFL: LL: 0.16" DL: 0.18". See detail DEFLCAMB1014 for camber recommendations. Provide for adequate drainage of roof.



# Maximum Bot Chord Forces Per Ply (lbs) Tens. Comp. Chords Tens.Comp. Chords

I - H

H - G

1588

1590

- 262

- 260

### Maximum Web Forces Per Ply (lbs)

- 451

1342 - 145

3368 - 661

3054

K - J

.1 - 1.

Webs	Tens.Comp.	Webs	Tens. Comp.		
M - A	385 - 1176	C-J	461 - 2198		
A - L	3109 - 729	J - D	1098 - 266		
L-B	553 - 1938	I-F	154 - 390		
K-C	2100 - 405				

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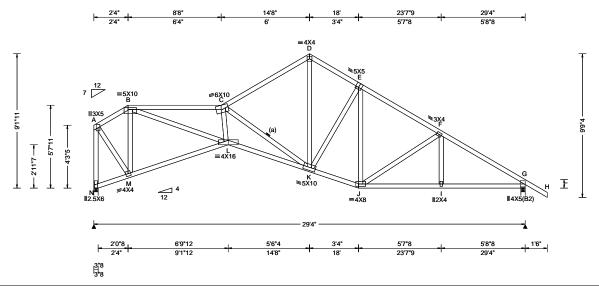
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SEQN: 409479 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T35 FROM: CDM DrwNo: 349.21.0814.20703 Qty: 1 ARLENA 7/12 // Sunset Truss Label: D08 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
Loading Criteria (psf)	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res.	PP Deflection in loc L/defl L/# VERT(LL): 0.168 L 999 240 VERT(CL): 0.349 L 999 180 HORZ(LL): 0.108 G HORZ(TL): 0.225 G Creep Factor: 2.0 Max TC CSI: 0.309
Load Duration: 1.25 Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Max BC CSI: 0.372 Max Web CSI: 0.410 VIEW Ver: 21.01.01A.0521.20
Lumber	1	IMAAL	

▲ Ma	▲ Maximum Reactions (Ibs)								
	G	ravity		No	n-Grav	vity			
Loc	R+	/ R-	/Rh	/ Rw	/ U	/ RL			
N ·	1229	/-	/-	/638	/61	/237			
G ·	1332	/-	/-	/806	/30	/-			
Wind	d reac	tions bas	sed on	MWFRS					
N	Brg W	/id = 3.5	Min	Req = 1.5	5				
G	Brg W	/id = 3.5	Min	Req = 1.5	5				
Bear	ings N	N & G are	e a rigi	d surface.					
Mem	ibers i	not listed	l have	forces less	than 3	375#			
Max	imum	Top Ch	ord Fo	orces Per	Ply (lb	s)			
Chor	rds T	ens.Con	ıp.	Chords	Tens.	Ćomp.			
A - E	3	215 - 7	736	D-E	428	- 1424			
<sup>]</sup> в - с	;	836 - 33	345	E-F	410	- 1541			
C - E	)	413 - 14	176	F-G	404	- 1922			

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Bracing**

(a) Continuous lateral restraint equally spaced on

In lieu of structural panels use purlins to brace all flat

### Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

#### Maximum Bot Chord Forces Per Ply (lbs) Tens. Comp. Chords Tens.Comp. Chords M - L 673 - 115 1565 - 243 3649 - 640 1 - G - 242 L-K 1566 K - J 1335 - 135

# Maximum Web Forces Per Ply (lbs)

vebs	rens.comp.	Webs	rens. Comp.	
A - N	338 - 1244	C-L	575 -3	
A - M	1070 - 306	C - K	667 - 2759	
3 - M	363 - 920	K - D	1081 - 301	
3 - L	2885 - 646			



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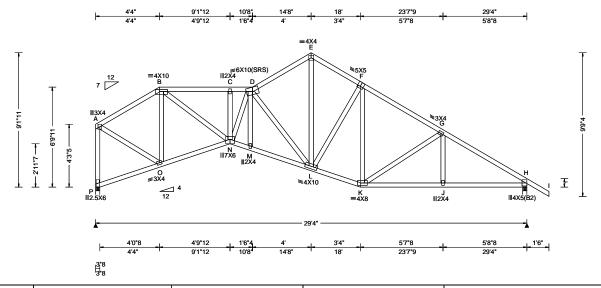
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SEQN: 409480 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T14 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.23263 Truss Label: D09 / YK 12/15/2021



Loading Criteria (psf) Wind Criteria		Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria		
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#		
1.0220.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.083 D 999 240		
DCLL. 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.173 D 999 180		
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.058 H		
Dec I d: 10 00	EXP: C Kzt: NA		HORZ(TL): 0.121 H		
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0		
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.249		
	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.351		
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.653		
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)			
	GCpi: 0.18	Plate Type(s):			
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20		

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

	▲ Maximum Reactions (lbs)								
		Gravity		Non-Gravity					
0	Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL			
0	P 122	9 /-	/-	/639	/51	/237			
.	H 133	2 /-	/-	/810	/40	/-			
.	Wind re	actions b	ased on N	<b>MWFRS</b>					
	P Brg	Wid = 3	.5 Min F	Req = 1.5	5				
	H Brg	Wid = 3	.5 Min F	Req = 1.5	5				
	Bearing	sP&Ha	re a rigid	surface.					
	Member	s not list	ed have fo	orces less	than 3	375#			
	Maximu	m Top (	hord Fo	rces Per	Ply (lb	s)			
	Chords	Tens.Co	omp. (	Chords	Tens.	Ćomp.			
	A - B	222	4400	E - F	400	- 1421			
				Е-Г F-G	460				
	B-C			_	437	- 1543			
	C - D	633 -	2188 (	G - H	430	- 1921			

451 - 1424

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Cnoras	rens.C	omp.	Cnoras	rens. (	Jomp.
O - N	976	- 145	L-K	1334	- 160
N - M	2176	- 376	K-J	1564	- 265
M - L	2182	- 382	J - H	1565	- 263

### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - P	340 - 1195	B - N	1581 - 378
A - O	1071 - 279	D-L	432 - 1479
B - O	276 - 730	L-E	1138 - 357



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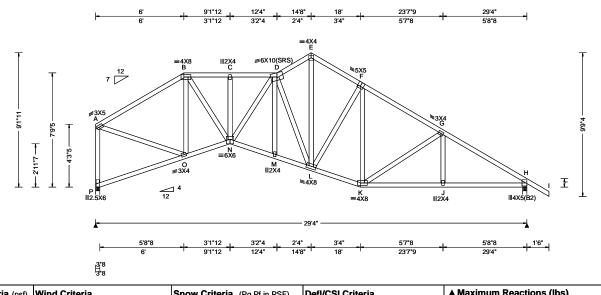
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6750 Forum Drive Suite 305 Orlando FL, 32821

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 409481 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T10 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.25727 Truss Label: D10 / YK 12/15/2021



Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
Pf: NA Ce: NA	VERT(LL): 0.070 D 999 240
Lu: NA Cs: NA	VERT(CL): 0.146 D 999 180
Snow Duration: NA	HORZ(LL): 0.048 H
	HORZ(TL): 0.100 H
Building Code:	Creep Factor: 2.0
FBC 7th Ed. 2020 Res.	Max TC CSI: 0.249
TPI Std: 2014	Max BC CSI: 0.350
Rep Fac: Yes	Max Web CSI: 0.518
FT/RT:20(0)/10(0)	
Plate Type(s):	
WAVE	VIEW Ver: 21.01.01A.0521.20
	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

	A IVIA	AIIIIU	IIII VE	<b>20110113</b>	(IDS)			
		G	ravity		N	on-Grav	vity	
)	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
)	P 1	229	/-	/-	/641	/42	/237	
	H 1	332	/-	/-	/814	/49	/-	
	Wind	reac	tions b	ased o	n MWFRS			
	PE	3rg W	/id = 3	.5 Mi	n Req = 1.	5		
	н в	3rg W	/id = 3	.5 Mi	n Req = 1.	5		
	Beari	ngs F	Р& Н а	are a ric	id surface.			
	Mem	bers	not list	ed have	forces les	s than 3	375#	
	Maxi	mum	Top (	Chord F	orces Per	Ply (lb	s)	
	Chor	ds T	ens.C	omp.	Chords	Tens.	Ćomp.	_
-	A - B		380 -	1358	E-F	488	- 1419	
	B-C			1707	F-G	460	- 1543	
			0.0			700	1070	

546 - 1706

493 - 1388

C-D

Cnoras	rens.c	omp.	Cnoras	rens. (	∍omp.
O - N	1158	- 163	L-K	1334	- 182
N - M	1665	- 269	K-J	1563	- 283
M - L	1662	- 268	J - H	1565	- 282

G - H

451 - 1920

### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - P	337 - 1177	B - N	1065 - 278
A - O	1163 - 281	D-L	395 - 1158
B - O	241 - 603	L-E	1187 - 421



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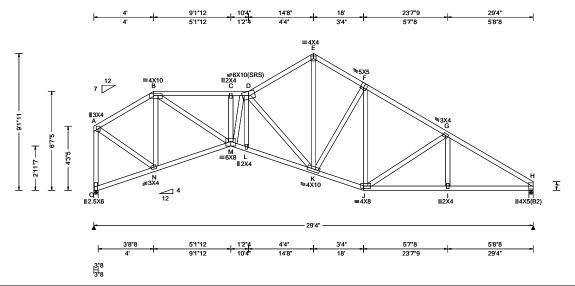
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SEQN: 409482 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T24 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.27980 Truss Label: D11 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.087 D 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.183 D 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.060 H
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.126 H
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.210
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.334
1	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.718
'	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind loads based on MWFRS with additional C&C

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

	▲ Maxim	um Rea	ctions (	lbs)		
	(	avity		No	on-Gra	vity
,	Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL
1	O 1232	/-	/-	/639	/53	/220
	H 1227	/-	/-	/725	/27	/-
	Wind rea	ctions ba	ased on	MWFRS		
	O Brg \	Vid = 3.9	5 Min	Req = 1.5	5	
	H Brg \	Nid = 3.8	5 Min	Reg = 1.5	5	
	Bearings	O & H a	re a rigi	d surface.		
	Members	not liste	d have	forces les	s than :	375#
	Maximur	n Top C	hord Fo	orces Per	Ply (lb	s)
	Chords '	Tens.Co	mp.	Chords	Tens.	Ćomp.
	A - B	328 -	1070	E-F	463	- 1426
	B-C	689 - 2	2329	F-G	436	- 1552
	C - D	688 - 2	2329	G-H	433	- 1942
	D-E	452 -	1438			

### Maximum Bot Chord Forces Per Ply (lbs)

Choras	rens.comp.		Choras	rens. (	Jomp.
N - M	935	- 196	K-J	1341	- 193
M - L	2332	- 451	J - I	1588	- 303
L-K	2340	- 459	I - H	1589	- 302

### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. (	Comp.
A - O	352 - 1202	D-K	486	- 1597
A - N	1060 - 291	K - E	1133	- 353
B - N	295 - 760	J - G	149	- 388
B - M	1735 - 436			



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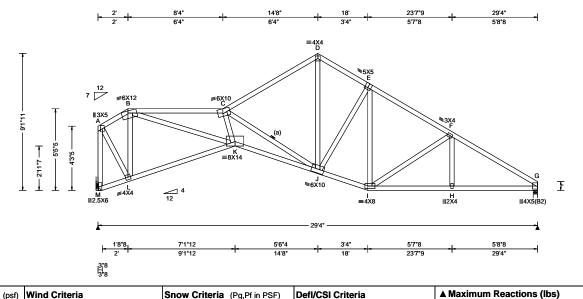
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SEQN: 409483 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T37 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.30417 Truss Label: D12 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
1.0220.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.208 K 999 240
DCLL. 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.435 K 807 180
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.129 G
Dec 1 d · 10 00	EXP: C Kzt: NA		HORZ(TL): 0.270 G
NODOLL, 40 00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
0.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.333
I	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.428
1	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.497
' "	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

#### **Bracing**

(a) Continuous lateral restraint equally spaced on member.

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=0' uses the following support conditions: 0' Bearing M (0', 9'1"2) HUS28 Supporting Member: (1)2x8 SP #2 (22) 0.148"x3" nails into supporting member, (4) 0.148"x3" nails into supported

member.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

#### Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1232 /-/638 /220 1227 /-/720 /-/16 Wind reactions based on MWFRS Brg Wid = -М Min Rea = -Brg Wid = 3.5 Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 431 - 1431 192 - 657 B - C 958 - 3679 E-F 408 - 1550 C-D 414 - 1491 F-G 408 - 1943

Maximum Bot Chord Forces Per Ply (lbs)					
Chords	Tens.C	Comp.	Chords	Tens. (	Comp.
L-K	611	- 104	I-H	1589	- 281
K - J	4141	- 810	H-G	1590	- 280
1 1	1212	160			

# Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - M	347 - 1266	C-K	724 0
A - L	1105 - 328	C-J	795 - 3217
B - L	394 - 960	J - D	1068 - 293
B - K	3258 - 784	I-F	153 - 393



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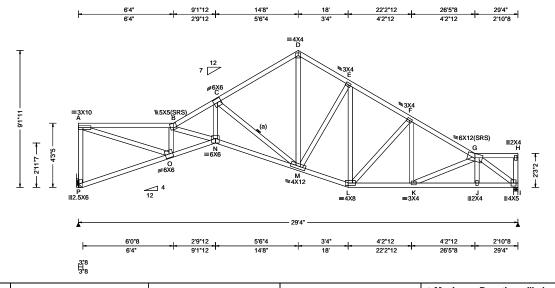
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SEQN: 409484 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T5 Qty: 1 DrwNo: 349.21.0814.32917 FROM: CDM ARLENA 7/12 // Sunset Truss Label: D13 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.161 N 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.337 N 999 180
	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.101 I
Dec 1 d · 40 00	EXP: C Kzt: NA		HORZ(TL): 0.212 I
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.419
	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.326
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.317
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Bracing**

(a) Continuous lateral restraint equally spaced on member.

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information

Bearing at location x=0' uses the following support conditions: 0' Bearing P (0', 9'1"2) HUS28 Supporting Member: (1)2x8 SP #2 (22) 0.148"x3" nails into supporting member, (4) 0.148"x3" nails into supported member.

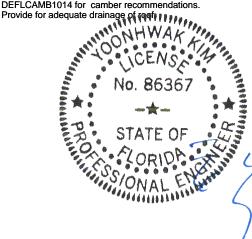
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure. Wind loading based on both gable and hip roof types.

Max JT VERT DEFL: LL: 0.16" DL: 0.18". See detail DEFLCAMB1014 for camber recommendations.



#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity /Rw /U Loc R+ /Rh /RL Р 1233 /-/177 1226 /674 /-/12 Wind reactions based on MWFRS Brg Wid = -Min Rea = Brg Wid = 3.5 Bearing I is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 755 - 2968 411 - 1426 B - C 791 - 3463 E-F 398 - 1523 C-D 397 - 1473 F-G 405 - 1830

### Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.C	Comp.	Chords	Tens. (	Comp.
O - N	3382	- 801	L-K	1523	- 297
N - M	3060	- 585	K - J	1638	- 383
M - L	1337	- 211	J - I	1642	- 380

### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp	١.
A - P	415 - 1173	C - M	531 - 219	9
A - O	3130 - 796	M - D	1083 - 27	1
O - B	597 - 1965	L-F	148 - 39	6
N - C	2184 - 465	G - I	456 - 1979	9

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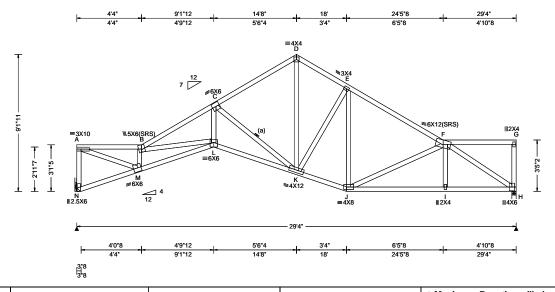
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SEQN: 409485 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T39 Qty: 1 DrwNo: 349.21.0814.39077 FROM: CDM ARLENA 7/12 // Sunset Truss Label: D14 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.161 L 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.338 L 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.106 H
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.222 H
NCBCLL: 10.00	Mean Height: 15.22 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.189
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.356
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.592
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

#### **Bracing**

(a) Continuous lateral restraint equally spaced on member.

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=0' uses the following support conditions: 0' Bearing N (0', 9'1"2) HUS28 Supporting Member: (1)2x8 SP #2 (22) 0.148"x3" nails into supporting member, (4) 0.148"x3" nails into supported member.

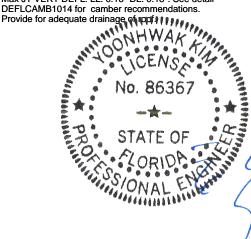
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

# Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure. Wind loading based on both gable and hip roof types.

Max JT VERT DEFL: LL: 0.16" DL: 0.18". See detail DEFLCAMB1014 for camber recommendations.



#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1233 /-/652 /155 1226 /-/643 /-/43 Wind reactions based on MWFRS Brg Wid = -Min Rea = -Brg Wid = 3.5 Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 706 - 2826 426 - 1441 B - C 785 - 3515 388 - 1577 C-D 406 - 1475

#### Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

M - L	3337 - 865	J - I	1663	- 436
L-K	3097 - 660	I - H	1668	- 433
K - J	1355 - 268			

### Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.	
A - N	380 - 1188	C-K	566 - 2249	
A - M	3009 - 749	K-D	1136 - 306	
M - B	596 - 2050	J-F	208 - 430	
L-C	2126 - 415	F-H	508 - 1971	

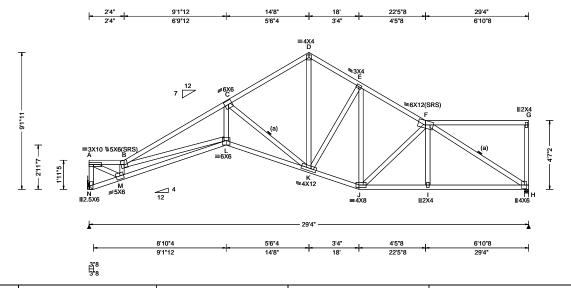
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.158 L 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.330 L 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.107 H
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.224 H
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.383
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.372
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.330
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

#### **Bracing**

(a) Continuous lateral restraint equally spaced on member.

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information

Bearing at location x=0' uses the following support conditions: 0' Bearing N (0', 9'1"2) HUS28 Supporting Member: (1)2x8 SP #2 (22) 0.148"x3" nails into supporting member, (4) 0.148"x3" nails into supported member.

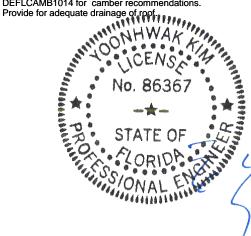
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure. Wind loading based on both gable and hip roof types.

Max JT VERT DEFL: LL: 0.16" DL: 0.17". See detail DEFLCAMB1014 for camber recommendations.



#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity /Rw /U Loc R+ /R /Rh /RL 1233 /-/684 /185 1226 /-/643 /-/84 Wind reactions based on MWFRS Brg Wid = -Min Rea = -Brg Wid = 3.5 Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 580 - 2398 427 - 1422 B - C 829 - 3597 - 1537 C-D 425 - 1482

#### Maximum Bot Chord Forces Per Ply (lbs) Tens. Comp. Chords Tens.Comp. Chords M - L 3068 - 878 J - I 1570 - 462 I - H 1574 3143 - 763 - 460 L-K 1334 - 315 K - J

#### Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - N C - K - 2304 319 - 1215 621 A - M 2677 - 646 K-D 1116 - 306 M - B 627 - 2122 J - F 237 - 442 542 2111 - 1856

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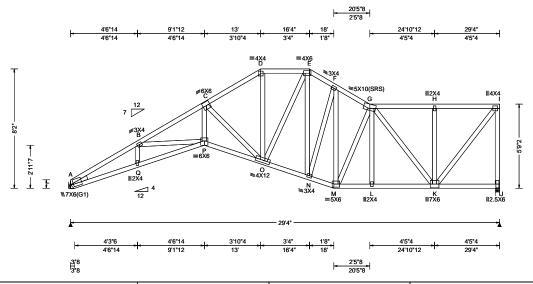
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SEQN: 409487 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T28 DrwNo: 349.21.0814.44130 FROM: CDM ARLENA 7/12 // Sunset Qty: 1 Truss Label: D16 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.163 P 999 240
DCLL. 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.341 P 999 180
DCDL. 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.106 K
Dec I d: 40 00	EXP: C Kzt: NA		HORZ(TL): 0.222 K
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.228
	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.426
	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.651
. •	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31; Lt Stub Wedge: 2x4 SP M-31;

### Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=0' uses the following support conditions: 0' Support conditions. 0 Bearing A (0', 9'1"2) HUS28 Supporting Member: (1)2x8 SP #2 (22) 0.148"x3" nails into supporting (4) 0.148"x3" nails into supported member.

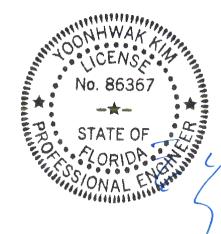
#### **Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads based on MWFRS with additional C&C member design

Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.



#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1233 /-/728 /183 /195 1226 /647 /231 /-Wind reactions based on MWFRS Brg Wid = -Min Rea = Brg Wid = 3.5 Bearing J is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 1112 - 3759 - 1444 B - C 1134 - 3492 F-G 568 - 1485 C-D 641 - 1731 G-H 374 - 876 D-E 586 - 1448 - 876

### Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. (	Jomp.	
A - Q	3269 - 1129	N - M	1321	- 482	
Q-P	3317 - 1147	M - L	1402	- 541	
P - O	3071 - 1054	L-K	1404	- 540	
O - N	1305 - 465				

### Maximum Web Forces Per Ply (lbs)

Tens.Comp.	Webs	Tens. Comp.
2110 - 652	M - G	211 - 377
715 - 2104	G-K	263 - 836
615 - 171	K-I	1390 - 593
484 - 137	I-J	562 - 1189
	2110 - 652 715 - 2104 615 - 171	2110 -652 M - G 715 -2104 G - K 615 -171 K - I

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SEQN: 409488 SPEC Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T41 FROM: CDM DrwNo: 349.21.0814.48770 Qty: 1 ARLENA 7/12 // Sunset Truss Label: D17 / YK 12/15/2021 5'6"8 11' 18'4" 23'10" 29'4" 5'5"8 3'8' 3'8" 5'6" 5'6" =4X10 =5X6 ∥2X4 E ≡3X4 G ≡4X4 H **∌3X**4 7 N ∥2X4  $\equiv 3X4$ ≡3X4 ∏I ∥2.5X6 ≡5X6 ≡4X4 **∥4X5(B2)** 29'4"

3'8"

14'8'

3'8"

18'4'

5'6"

23'10'

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Ī
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.058 M 999 240 VERT(CL): 0.119 M 999 180 HORZ(LL): 0.021 J HORZ(TL): 0.044 J Creep Factor: 2.0 Max TC CSI: 0.246 Max BC CSI: 0.351 Max Web CSI: 0.522  VIEW Ver: 21.01.01A.0521.20	ı
Lumber	Wild Bulddon. 1.00	WAVE	VIEW VOI. 21.01.017.0021.20	l

5'5"8

▲ M	▲ Maximum Reactions (lbs)							
	G	avity		Non-Gravity				
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
В	1326	/-	/-	/813	/137	/251		
1	1215	/-	/-	/631	/-	/-		
Win	d read	ctions b	ased or	MWFRS				
В	Brg V	Vid = 3.	5 Mir	Req = 1.	5			
1	Brg V	Vid = 3.	5 Mir	n Reg = 1.	5			
Bea	rings	B & I ar	e a rigio	d surface.				
Men	nbers	not liste	ed have	forces les	s than 3	375#		
Max	imun	n Top C	hord F	orces Per	Ply (lb	s)		
Cho	rds <sup>-</sup>	Tens.Co	mp.	Chords	Tens.	Ćomp.		
B - 0	С	528 -	1910	E-F	558	- 1333		
C - I	D	549 -	1552	F-G	510	- 1268		
D - I	F	557 -	1333	G - H	362	- 833		

5'6"

29'4'

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

5'6"8

5'6"8

Wind loads based on MWFRS with additional C&C

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

#### Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Tens. Comp. Chords B - N 1557 -615 1267 - 509 L-K N - M 1556 - 616 K-J 873 - 385 M - L 1266 - 517

# Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.		
F-K	336 - 392	J - H	1326	- 577	
K - G	628 - 372	H-I	583	- 1173	
GI	514 - 907				



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FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0814.59030 Truss Label: D18 / YK 12/15/2021 11'11"8 17'9" 23'4"12 29'4" 9' 2'11"8 5'9"8 5'7"12 5'11"4 ∥2X4 E =5<u>¥</u>5 ≡3X4 G 5'10" 6'7"14 <u>T</u> N ∥2X4 МП K ≡4X10 =5X6 ≡3X4(A1) 112 5X6 29'4' 2'3"8 6'8"8 2'11"8 5'9"8 5'7"12 5'11"4 11'11"8 17'9" 23'4"12 29'4"

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.216 O 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.444 O 788 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.163 J
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.336 J
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.604
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.300
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.487
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20
Lumber			

Job Number: 21-6560

#### ▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL 1331 /-/800 /220 /209 1210 /-/623 /240 /-Wind reactions based on MWFRS Brg Wid = 3.5Min Reg = 1.5Brg Wid = 3.5 Min Req = 1.5 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 218 - 847 1014 - 1939 C - D 865 - 2019 F-G 818 - 1558 D-E 1021 - 1952 G-H 560 - 1053

Cust: R 215 JRef: 1Xbc2150008

T7

SEQN: 409489

HIPM

Ply: 1

Top chord: 2x4 SP M-31; T1 2x6 SP 2400f-2.0E; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### **Plating Notes**

(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

#### Maximum Bot Chord Forces Per Ply (lbs) Tens. Comp. Chords Tens.Comp. Chords C - P 1440 - 654 N-L 1753 - 853 P - N 1749 - 855 K-J 1100 - 591 Maximum Web Forces Per Ply (lbs) Tens. Comp. Webs Tens.Comp. Webs L-F 465 - 235 G - J 601 -882

1551 - 826

651 - 331

463 - 650

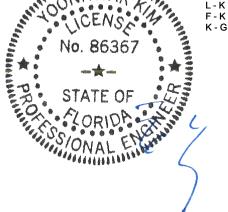
J - H

H - I

1458

679 - 1163

- 775



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SEQN: 409503 HIPM Ply: 2 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 FROM: CDM ARLENA 7/12 // Sunset DrwNo: 349.21.0815.01573 Qty: 1 Truss Label: D19 / YK 12/15/2021 2 Complete Trusses Required 11'11"8 17'9" 23'4"12 29'4" 4'11"8 5'7"12 5'11"4 5'9"8 ≡5X10 D ∥2X4 E =7X6 ≡3X4 G ≡4X6 H <u>\*</u>8 5'5"1 N ∥2X4 7 K =6X8 =3X4(A1) ∥2.5X6 ≡7X8 ||12.5X6 ≡7X8 **∥2.5**X6 29'4"

5'9"8

17'9'

Loading	Criteria (psf)	Wind Criteria	Snow Cr	riteria (Pg	,Pf in PSF)	Defl/CSI Cr	iteria			Γ.
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection	n in loc	L/defl	L/#	
TCDL:	10.00	Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL):	0.226 N	999	240	١.
BCLL:	0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL):	0.472 N	741	180	
BCDL:	10.00	Risk Category: II	Snow Du	ration: NA		HORZ(LL):	0.172 J	-	-	
Des Ld:	40.00	EXP: C Kzt: NA				HORZ(TL):	0.359 J	-	-	ľ
NCBCLL:	0.00	Mean Height: 11.28 ft TCDL: 5.0 psf	Building (	Code:		Creep Facto	r: 2.0			
Soffit:	2.00	BCDL: 5.0 psf	FBC 7th	Ed. 2020 I	Res.	Max TC CS	: 0.935	5		
Load Dura	ation: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std:	2014		Max BC CS	l: 0.264	ļ		
Spacing:		C&C Dist a: 3.00 ft	Rep Fac:	No		Max Web C	SI: 0.209	)		
' -		Loc. from endwall: NA	FT/RT:20	0(0)/10(0)						l,
		GCpi: 0.18	Plate Typ	oe(s):						١.
		Wind Duration: 1.60	WAVE			VIEW Ver: 2	21.01.01A	.0521.:	20	l

4'11"8

11'11"8

▲ Maxin	▲ Maximum Reactions (lbs)							
	Gravity		Non-Gravity					
Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL			
B 2504	4 /-	/-	/-	/424	/-			
1 260	5 /-	/-	/-	/416	/-			
Wind rea	actions b	ased on	MWFRS					
B Brg	Wid = 3.	5 Min	Req = 1.5	5				
I Brg	Wid = 3.	5 Min	Req = 1.5	5				
Bearings	B&lar	e a rigid	surface.					
Member	s not liste	ed have f	forces less	s than 3	375#			
Maximu	m Top C	hord Fo	rces Per	Ply (lb	s)			
Chords	Tens.Co	mp.	Chords	Tens.	Ćomp.			
в-с	112	- 681	E-F	490	- 3019			
C-D		2595		354	- 2229			
D-E	493 -	3042	G-H	237	- 1490			

5'11"4

29'4"

5'7"12

23'4"12

#### Lumber

Top chord: 2x6 SP 2400f-2.0E; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP M-31;

1'6"

2'3"8

2'3"8

4'8"8

### **Nailnote**

Nail Schedule:0.131"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @12.00" o.c. :1 Row @ 4" o.c. Use equal spacing between rows and stagger nails

in each row to avoid splitting.

#### Loading

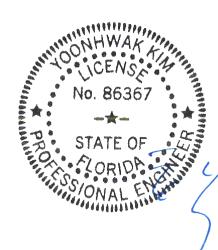
#1 hip supports 7-0-0 jacks W/2 panel TC and no end vert.

### **Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.



#### Maximum Bot Chord Forces Per Ply (lbs) Tens. Comp. Chords Tens.Comp. Chords C - N 2423 - 385 K - J 1546 - 250 N - L 2437 - 386

#### Maximum Web Forces Per Ply (lbs) Tens. Comp. Webs Tens.Comp. Webs D-L 708 - 125 K - G 860 - 130 877 - 149 L-F G-J 188 - 805 L-K 2199 - 350 J - H 1842 - 293 165 - 692 H - I 213 - 1194

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SEQN: 409504 HIPS Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T23 FROM: CDM ARLENA 7/12 // Sunset DrwNo: 349.21.0815.04197 Qty: 1 Truss Label: G01 / YK 12/15/2021 10'1"12 15 20' 25 5'1"12 4'10"4 **=**4X4 ≢7X6 B ∥2X4 C =5X6 E T2 D 3'6" =2X4(A1)  $\equiv 3X4(A1)$ ₹H G ∥2X4 J ∥2X4 **≡**8X8 =3X4 15'9"12 9'2"4 5'8" 5 5'1"12 4'2"4 5' 15'9"12 10'1"12 20' 25

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	l
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	l
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.017 C 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.036 C 999 180	l
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B	l
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 11.14 ft		HORZ(TL): 0.009 B	
NCBCLL: 0.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	ı
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.147	l
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.115	ı
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: No	Max Web CSI: 0.228	ı
	Loc. from endwall: NA	FT/RT:20(0)/10(0)		ı
	GCpi: 0.18	Plate Type(s):		ł
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	l
Lumber				_

Top chord: 2x4 SP M-31; T2 2x6 SP 2400f-2.0E; Bot chord: 2x6 SP 2400f-2.0E; Webs: 2x4 SP M-31;

### Loading

#1 hip supports 5-0-0 jacks with no webs.

Left side jacks have 5-0-0 setback with 0-0-0 cant and 1-6-0 overhang. End jacks have 5-0-0 setback with 0-0-0 cant and 1-6-0 overhang. Right side jacks have 5-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

#### Wind

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C-D A - B 227 - 1315 162 - 981 B-C 162 - 982 D-E 513 -74

Min Req = 1.5

Min Req = 1.5

Min Rea = 1.5

Non-Gravity

/143 /-

/304 /-

/63 /-

/RL

/Rw /U

#### Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

A - J 1073 - 173 1087 - 171

### Maximum Web Forces Per Ply (lbs)

▲ Maximum Reactions (lbs) Gravity

/Rh

/-

Wind reactions based on MWFRS Brg Wid = 3.5

Bearings A, H, & F are a rigid surface. Members not listed have forces less than 375#

Brg Wid = 3.5

Brg Wid = 3.5

Loc R+

2022 /-

341

Α 879

Mens	Tens.C	omp.	webs	i ens.	Comp.
C - I I - D			D - H H - E		- 1330 - 906



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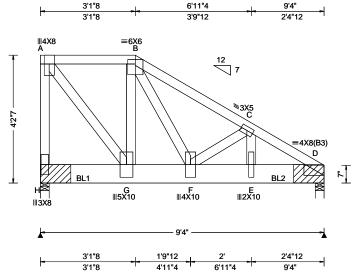
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SEQN: 409498 HIPM Ply: 1 Job Number: 21-6560 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset Truss Label: G02

Cust: R 215 JRef: 1Xbc2150008 T27 DrwNo: 349.21.0815.12163 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	1
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.033 F 999 240	L
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.066 F 999 180	ŀ
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.011 A	[
Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case	HORZ(TL): 0.021 A Creep Factor: 2.0  Max TC CSI: 0.137  Max BC CSI: 0.726  Max Web CSI: 0.269	V E N
	Loc. from endwall: not in 4.50 ft GCpi: 0.18	FT/RT:20(0)/10(0) Plate Type(s):		2
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	] {
Lumber	-			- 6

	<b>▲</b> Ma	ximu	ım Rea	ctions	(lbs)	)				
	Gravity					Non-Gravity				
	Loc	R+	/ R-	/ Rh		/Rw	/ U	/ RL		
	Н 3	132	/-	/-		/-	/192	/-		
	D 3	594	/-	/-		/-	/294	/-		
	Wind	reac	tions ba	ased or	n MW	/FRS				
	НЕ	3rg W	/id = 3.9	5 Mir	n Red	<b>1</b> = -				
	DE	3rg W	/id = 3.9	5 Mir	n Red	i = -				
	Beari	nas I	∃&Da	re a rio	id su	ırface.				
	Memi	bers	not liste	d have	forc	es les	s than 3	375#		
	Maxi	mum	Top C	hord F	orce	s Per	Ply (lb	s)		
								Comp.		
_	A - B		95 - 1	1710	C -	D	190	- 3839		
	B-C		150 -3	-	9	_	130	5555		

Top chord: 2x4 SP M-31; Bot chord: 2x8 SP #2; Webs: 2x4 SP M-31;

### **Special Loads**

(Lumber	Dur.Fac.=1.	25 / Plate [	Our.Fac.=1.2	(5)
TC: From	63 plf at	0.00 to	63 plf at	3.12
TC: From	32 plf at	3.12 to	32 plf at	6.94
TC: From	63 plf at	6.94 to	63 plf at	9.33
BC: From	10 plf at	0.00 to	10 plf at	9.33
BC: 1232 lb	Conc. Load	at 0.94		
BC: 1233 lb	Conc Load	at 2 94 4	94 6 94 8 9	14

### **Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

#### Wind

Wind loads and reactions based on MWFRS. Left end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.

### Bearing Block(s)

Brg blocks:0.131"x3", min. nails x-loc #blocks length/blk #nails/blk 0.000' 1 12" 4 9.041' 1 12" 5 brg wall plate Rigid Surface Rigid Surface Brg block to be same size and species as chord. Refer to drawing CNNAILSP1014 for more information.



# Maximum Bot Chord Forces Per Ply (lbs)

onoras	rens.comp.		Choras	rens. Comp.		
G - F	1743	- 98	E-D	3264	- 156	
F-E	3223	- 156				

### Maximum Web Forces Per Ply (lbs)

vvebs	rens.comp.	vvebs	rens. Comp.	
A - H	157 - 2341	B - F	1937	- 52
A - G	2828 - 155	F-C	46	- 817
G-B	42 - 426	C-E	866	- 1

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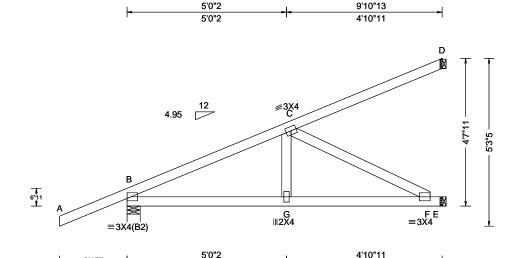
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SEQN: 409499 HIP\_ Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T21 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0815.14483 Truss Label: J01 / YK 12/15/2021



L	oading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	▲ Maximum Reactions (lbs)	
lΤ	CLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity No	
lΤ	CDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.021 G 999 240	Loc R+ /R- /Rh /Rw	
В	CLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.043 G 999 180	B 462 /- /- /-	
В	CDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.006 D	E 359 /- /- /-	
D	es Ld: 40.00	EXP: C Kzt: NA Mean Height: 0.00 ft		HORZ(TL): 0.013 D	D 272 /- /- /-	
N	CBCLL: 0.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS	
L	oad Duration: 1.25 pacing: 24.0 "	2.00 BCDL: 5.0 psf uration: 1.25 MWFRS Parallel Dist: 0 to h/2	FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0)	Max TC CSI: 0.281 Max BC CSI: 0.227 Max Web CSI: 0.166	B Brg Wid = 4.9 Min Req = 1.5 E Brg Wid = 1.5 D Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less	
		GCpi: 0.18	Plate Type(s):		Maximum Top Chord Forces Per I	
		Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	Chords Tens.Comp.	

5'0"2

– 2'1"7 ––<del>-|-</del>

# Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Loading

Hipjack supports 7-0-0 setback jacks with no webs.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

Chords Tens.Comp. B - C 101 - 629

9'10"13

#### Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

B - G 553 G-F 548 - 89

Non-Gravity

/93 /-

/100

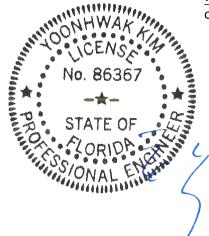
/RL

/-/-

/Rw /U

#### Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. C-F 100 - 621



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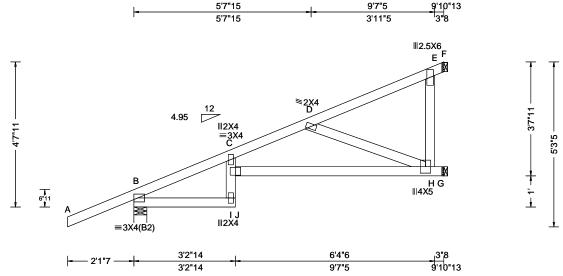
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SEQN: 409500 HIP\_ Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T26 FROM: CDM Qty: 1 ARLENA 7/12 // Sunset DrwNo: 349.21.0815.17980 Truss Label: J02 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.096 I 999 240 VERT(CL): 0.195 I 606 180 HORZ(LL): 0.042 C HORZ(TL): 0.086 C Creep Factor: 2.0 Max TC CSI: 0.304 Max BC CSI: 0.258 Max Web CSI: 0.238  VIEW Ver: 21.01.01A.0521.20	Grav Loc R+ / B 462 /- G 127 /- F 504 /- Wind reactio B Brg Wid G Brg Wid F Brg Wid Bearing B is Members no Maximum T Chords Ter
Lumber				

#### ▲ Maximum Reactions (lbs) avity Non-Gravity /R /Rh /Rw / U /RL 1\_ /-/-/215 /-/113 ions based on MWFRS Min Req = 1.5 id = 4.9id = 1.5id = 1.5s a rigid surface. ot listed have forces less than 375# Top Chord Forces Per Ply (lbs) ens.Comp. Chords Tens. Comp.

#### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Loading

Hipjack supports 7-0-0 setback jacks with no webs.

Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.

#### Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp.

C-D

- 816

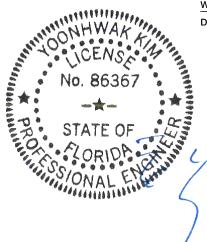
I - H 812 - 170

B - C

#### Maximum Web Forces Per Ply (lbs)

82 - 386

Webs Tens.Comp. D - H 183 - 845



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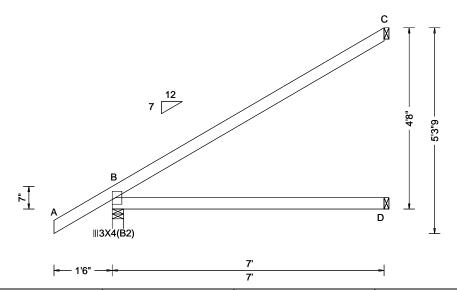
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SEQN: 409490 **EJAC** Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 Т9 FROM: CDM DrwNo: 349.21.0815.19483 Qty: 19 ARLENA 7/12 // Sunset Truss Label: J03 / YK 12/15/2021

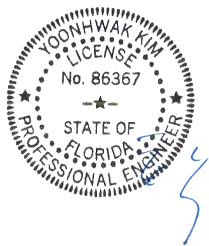


Loading Criteria (psf) V	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)	
TCLL: 20.00 V TCDL: 10.00 S BCLL: 0.00 E BCDL: 10.00 R	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.009 B HORZ(TL): 0.018 B	Gravity ` 1	/- /-
NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	ICDL: 5.0 psf BCDL: 5.0 psf WWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft	5.0 psf 5.0 psf 5.0 psf FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes pm endwall: not in 4.50 ft FT/RT:20(0)/10(0)	wind reactions based on MWFRS ax TC CSI: 0.314 ax BC CSI: 0.216 ax Web CSI: 0.000  Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface. Members not listed have forces less the	.5	
V	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20		

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

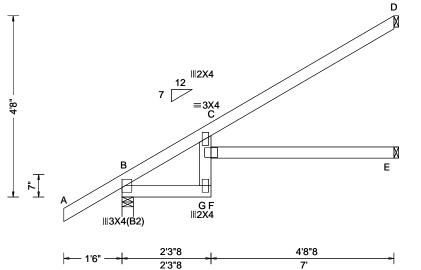
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SEQN: 409491 **EJAC** Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T48 FROM: CDM Qty: 3 ARLENA 7/12 // Sunset DrwNo: 349.21.0815.21020 Truss Label: J04 / YK 12/15/2021



1	Ī
3'8"	
<u></u>	- 5.36

	Criteria	Snow Criteria (	Pg,Pf in PSF)	Defl/CSI Criteria		
	Std: ASCE 7-16 d: 130 mph	Pg: NA Ct: N/ Pf: NA	A CAT: NA Ce: NA	PP Deflection in loc VERT(LL): 0.140 F		
BCLL: 0.00 Enclo	sure: Closed Category: II	Lu: NA Cs: N	A	VERT(CL): 0.282 F	296	180
Des I d: 40.00 EXP:	C Kzt: NA Height: 15.00 ft	Snow Duration: N	NA	HORZ(LL): 0.093 C HORZ(TL): 0.186 C		-
NCBCLL: 10.00 TCDI	.: 5.0 psf .: 5.0 psf	Building Code: FBC 7th Ed. 202	0 Res.	Creep Factor: 2.0 Max TC CSI: 0.37	3	
Load Duration: 1.25 MWF	RS Parallel Dist: h/2 to h	TPI Std: 2014 Rep Fac: Yes		Max BC CSI: 0.13	-	
	Dist a: 3.00 ft from endwall: not in 4.50 ft	FT/RT:20(0)/10(0	))	IVIAX VVED CSI. 0.00	,	
Wind	GCpi: 0.18 Duration: 1.60	Plate Type(s): WAVE		VIEW Ver: 21.01.01A	.0521.	20

▲ Maximum Reactions (lbs)						
	G	avity		No	on-Grav	/ity
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/RL
В	406	/-	/-	/274	/31	/169
Е	107	/-	/-	/61	/-	/-
D	210	/-	/-	/142	/104	/-
Wind reactions based on MWFRS						
B Brg Wid = 3.5 Min Reg = 1.5						
E Brg Wid = 1.5						
D Brg Wid = 1.5						
Bearing B is a rigid surface.						
Members not listed have forces less than 375#						

## Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

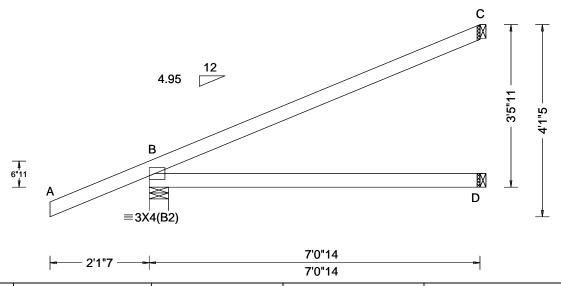
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SEQN: 409501 HIP\_ Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T19 FROM: CDM DrwNo: 349.21.0815.22410 Qty: 2 ARLENA 7/12 // Sunset Truss Label: J05 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pa: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B 307 /- /- /- /68 /-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.004 B	D 80 /- /- /34 /- /-
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 0.00 ft		HORZ(TL): 0.008 B	C 220 /- /- /- /81 /-
NCBCLL: 0.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.296	B Brg Wid = 4.9 Min Req = 1.5
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.121	D Brg Wid = 1.5 C Brg Wid = 1.5
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: No	Max Web CSI: 0.000	Bearing B is a rigid surface.
1	Loc. from endwall: NA	FT/RT:20(0)/10(0)		Members not listed have forces less than 375#
	GCpi: 0.18	Plate Type(s):		mornado not notos nato foros less than or on
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	

### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31;

Hipjack supports 5-0-0 setback jacks with no webs.

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

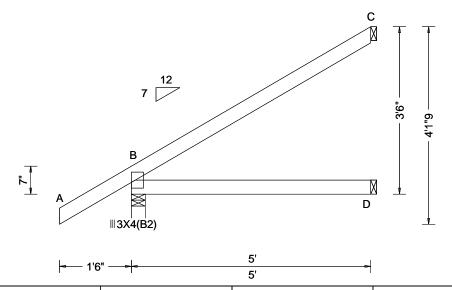
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SEQN: 409492 **EJAC** Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T55 FROM: CDM DrwNo: 349.21.0815.24617 Qty: 9 ARLENA 7/12 // Sunset Truss Label: J06 / YK 12/15/2021



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph Enclosure: Closed	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00 BCDL: 10.00	Risk Category: II	Lu: NA Cs: NA Snow Duration: NA	VERT(CL): NA HORZ(LL): -0.003 C	B 327 /- /- /225 /29 /127 D 95 /- /- /51 /- /-
Des Ld: 40.00	EXP: C Kzt: NA	Show Duration, NA	HORZ(TL): 0.005 B	D 95 /- /- /51 /- /-   C 138 /- /- /89 /81 /-
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS  B Brg Wid = 3.5 Min Reg = 1.5
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res. TPI Std: 2014	Max TC CSI: 0.159 Max BC CSI: 0.104	B   Brg Wid = 3.5   Min Req = 1.5   D   Brg Wid = 1.5
	MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000	C Brg Wid = 1.5
opusg	Loc. from endwall: not in 4.50 ft GCpi: 0.18	FT/RT:20(0)/10(0) Plate Type(s):		Bearing B is a rigid surface.  Members not listed have forces less than 375#
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	
Lumbor				

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

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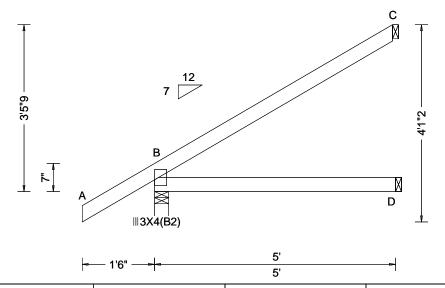
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 409493 **EJAC** Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T45 FROM: CDM DrwNo: 349.21.0815.26777 Qty: 2 ARLENA 7/12 // Sunset Truss Label: J07 / YK 12/15/2021



Loading Criteria (psf) Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00 Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00 Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B 326 /- /- /225 /29 /126
BCDL: 10.00 Risk Category: II	Snow Duration: NA	HORZ(LL): -0.003 C	C 136 /- /- /87 /80 /-
Des Ld: 40.00 EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.005 B	D 95 /- /- /51 /- /-
NCBCLL: 10.00 TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00 BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.155	B Brg Wid = 3.5 Min Req = 1.5 C Bra Wid = 1.5
Load Duration: 1.25 MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.104	D Bra Wid = 1.5
Spacing: 24.0 " C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000	Bearing B is a rigid surface.
Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		Members not listed have forces less than 375#
GCpi: 0.18	Plate Type(s):		
Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	

### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.



12/15/2021

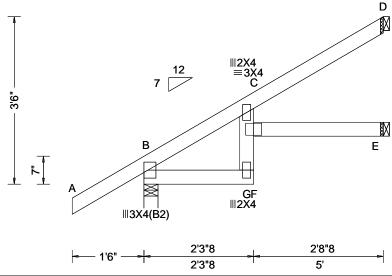
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SEQN: 409494 JACK Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T18 FROM: CDM Qty: 2 ARLENA 7/12 // Sunset DrwNo: 349.21.0815.28820 Truss Label: J08 / YK 12/15/2021



41"9	-
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TCLL: 20.00	Wind Criteria Wind Std: ASCE 7-16	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA	Defl/CSI Criteria PP Deflection in loc L/defl L/#	
DCDL	Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA	Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	VERT(LL): 0.041 F 999 240 VERT(CL): 0.082 F 726 180 HORZ(LL): 0.027 C	
Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	HORZ(TL): 0.054 C Creep Factor: 2.0  Max TC CSI: 0.173  Max BC CSI: 0.047  Max Web CSI: 0.030	
Lumbor	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 21.01.01A.0521.20	1

▲ Max	imum Re	actions (II	os)		
	Gravity	-	No	on-Gra	vity
Loc R	+ /R-	/ Rh	/ Rw	/ U	/ RL
B 32	7 /-	/-	/226	/30	/127
E 65	/-	/-	/37	/-	/-
D 14	9 /-	/-	/102	/70	/-
Wind r	eactions l	pased on N	/WFRS		
B Br	g Wid = 3	.5 Min F	Req = 1.5	5	
E Br	g Wid = 1	.5	-		
D Br	g Wid = 1	.5			
Bearing	g B is a ri	gid surface	<b>)</b> .		
Membe	ers not list	ed have fo	orces les	s than	375#

## Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

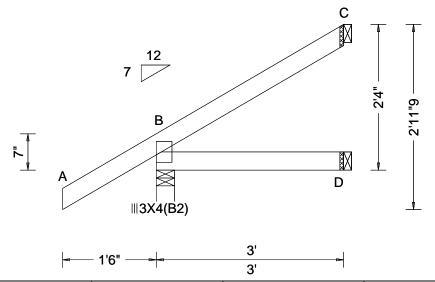
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SEQN: 409495 JACK Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T17 FROM: CDM DrwNo: 349.21.0815.30633 Qty: 6 ARLENA 7/12 // Sunset Truss Label: J09 / YK 12/15/2021



TCLL: 20.00	Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs	s)
Spacing: 24.0 " C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Rep Fac: Yes Max Web CSI: 0.000 Bearing B is a rigid surface. Members not listed have forces less than 375#	TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft	Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.001 C HORZ(TL): 0.001 C Creep Factor: 2.0 Max TC CSI: 0.072	Gravity Loc R+ / R- / Rh  B 255 /- /- D 56 /- /- C 69 /- /- Wind reactions based on MN B Brg Wid = 3.5 Min Re D Brg Wid = 1.5 C Brg Wid = 1.5 Bearing B is a rigid surface.	Non-Gravity / Rw / U / RL  /184 /31 /86 /31 /- /- /42 /46 /- WFRS eq = 1.5
Wind Duration: 1.60   WAVE   VIEW Ver: 21.01.01A.0521.20		Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20		

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

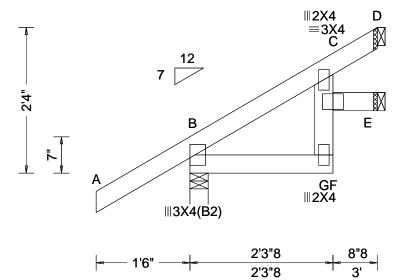
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SEQN: 409496 JACK Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 T13 FROM: CDM Qty: 2 ARLENA 7/12 // Sunset DrwNo: 349.21.0815.32393 Truss Label: J10 / YK 12/15/2021



-1	2'11"9
<u></u>	2'

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	4
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 F 999 240	[
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.006 F 999 180	ı
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 C	E
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.003 C	[
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	١
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.072	H
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.018	Н
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.008	H
' - '	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)		1,
	GCpi: 0.18	Plate Type(s):		┨.
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	

▲ M	laxim	um Rea	ctions (II	os)				
	G	avity		No	on-Gra	vity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL		
В	255	/-	/-	/184	/31	/86		
Е	22	/-	/-	/16	/1	/-		
D	80	/-	/-	/56	/33	/-		
Wir	nd read	ctions b	ased on N	/WFRS				
В	Brg V	Vid = 3.	5 Min F	Req = 1.5	5			
Е	Brg V	Vid = 1.	.5	-				
D	Brg V	Vid = 1.	.5					
Bearing B is a rigid surface.								
Members not listed have forces less than 375#								

### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;

### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

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SEQN: 409497 JACK Ply: 1 Job Number: 21-6560 Cust: R 215 JRef: 1Xbc2150008 FROM: CDM Qty: 8 ARLENA 7/12 // Sunset DrwNo: 349.21.0815.36497 Truss Label: J11 / YK

> C В D **||3X4(B2)**



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (Ib	s)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity	Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA	Loc R+ /R- /Rh	/Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA	B 229 /- /-	/180 /51 /45
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 C	D 14 /-1 /-	/11 /2 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.001 C	C - /-43 /-	/30 /44 /-
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	Wind reactions based on M	-
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.093	B Brg Wid = 3.5 Min R	eq = 1.5
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.010	D Brg Wid = 1.5 C Bra Wid = 1.5	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.000	Bearing B is a rigid surface.	
	Loc. from endwall: Any	FT/RT:20(0)/10(0)		Members not listed have for	
	GCpi: 0.18	Plate Type(s):			
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20		

### Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31;

Wind loads based on MWFRS with additional C&C

Wind loading based on both gable and hip roof types.



FL REG# 278, Yoonhwak Kim, FL PE #86367 12/15/2021

\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!

\*\*IMPORTANT\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



T36

12/15/2021

## Gable Stud Reinforcement Detail

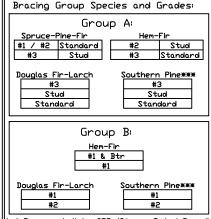
ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

		2×4	Brace	No	(1) 1×4 "L	Brace *	(1) 2×4 "L		(2) 2×4 *L		(1) 2×6 'L	* Brace *	(2) 2×6 *L	Brace **
$\parallel$ $_{-}$	Spacing	Vertico  Species	Grade		Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
유		CDE	#1 / #2	4′ 3″	7′ 3″	7' 7"	8′ 7 <b>″</b>	8′ 11 <b>″</b>	10′ 3″	10′ 8″	13′ 6″	14' 0"	14′ 0″	14′ 0″
		SPF	#3	4′ 1″	6′ 7 <b>″</b>	7′ 1″	8′ 6 <b>″</b>	8′ 10 <b>″</b>	10′ 1″	10′ 6″	13′ 4″	13′ 10″	14′ 0″	14′ 0″
D	ب ا	HF	Stud	4′ 1″	6′ 7 <b>″</b>	7′ 0 <b>″</b>	8' 6 <b>"</b>	8′ 10 <b>″</b>	10′ 1″	10′ 6″	13′ 4″	13′ 10 <b>″</b>	14′ 0″	14′ 0″
<u>C</u>		1 11	Standard	4′ 1″	5′ 8 <b>″</b>	6′ 0 <b>″</b>	7′ 7″	8′ 1 <b>″</b>	10′ 1″	10′ 6″	11′ 10″	12′ 8″	14′ 0″	14′ 0″
a			#1	4′ 6″	7′ 4″	7′ 8 <b>″</b>	8′ 8 <b>″</b>	9′ 0″	10′ 4″	10′ 9″	13′ 8″	14′ 0″	14′ 0″	14′ 0″
$-$	*	SP	#2	4′ 3″	7′ 3″	7′ 7″	8′ 7 <b>″</b>	8′ 11 <b>″</b>	10′ 3″	10′ 8″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
	4	<sub>D</sub> _,	#3	4′ 2″	6′ 0 <b>″</b>	6′ 4″	7′ 11 <b>″</b>	8′ 6 <b>″</b>	10′ 2″	10′ 7″	12′ 5″	13′ 4″	14′ 0″	14′ 0″
g	N	DFL	Stud	4′ 2″	6′ 0″	6′ 4″	7′ 11″	8′ 6″	10′ 2″	10′ 7″	12′ 5″	13′ 4″	14′ 0″	14′ 0″
$\Pi \cong$			Standard	4′ 0″	5′ 3″	5′ 7 <b>′</b>	7′ 0″	7′ 6″	9′ 6″	10′ 2″	11′ 0″	11′ 10″	14′ 0″	14′ 0″
∏ -≌		SPF	#1 / #2	4′ 11″	8′ 4″	8′ 8 <b>″</b>	9′ 10″	10′ 3″	11′ 8″	12′ 2″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
+>	l . <del>.</del>	12LL	#3	4′ 8″	8′ 1″	8′ 8 <b>″</b>	9′ 8″	10′ 1″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
$\sqcup$	Ų	l HF	Stud	4′ 8″	8′ 1″	8′ 6 <b>′</b>	9′ 8″	10′ 1″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
ΠŌ	lō	1 11	Standard	4′ 8″	6′ 11 <b>′</b>	7′ 5 <b>′</b>	9′ 3″	9′ 11″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
~			#1	5′ 1 <b>″</b>	8′ 5″	8′ 9 <b>″</b>	9′ 11″	10′ 4″	11′ 10″	12′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
/		SP	#2	4′ 11″	8′ 4″	8′ 8 <b>′</b>	9′ 10″	10′ 3″	11′ 8″	12′ 2″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	9		#3	4′ 9″	7′ 4″	7′ 9″	9′ 9″	10′ 2″	11′ 8″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
O	<del> </del>	DFL	Stud	4′ 9″	7′ 4″	7′ 9″	9′ 9″	10′ 2″	11′ 8″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	4′ 8″	6′ 5″	6′ 10″	8′ 7″	9′ 2″	11′ 7″	12′ 1″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
		SPF	#1 / #2	5′ 5″	9′ 2″	9′ 6 <b>″</b> 9′ 4 <b>″</b>	10′ 10″	11′ 3″	11′ 8″	13′ 5 <b>″</b> 13′ 3 <b>″</b>	14′ 0″	14′ 0″	14′ 0″	14′ 0″
ˈd			#3	5′ 1″	9′ 0″		10′ 8″	11′ 1″	12′ 9″		14′ 0″	14′ 0″	14′ 0″	14′ 0″
0	Ų	HF	Stud	5′ 1″	9′ 0″	9′ 4″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
		· '''	Standard	5′ 1″	8′ 0″	8′ 6″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
X	-		#1	5′ 8″	9′ 3″	9′ 8″	10′ 11″	11′ 4″	13′ 0″	13′ 6″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
ll ơ		SP	#2	5′ 5 <b>″</b> 5′ 3 <b>″</b>	9′ 2″	9′ 6″	10′ 10″	11′ 3″	12′ 11″	13′ 5″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
M Q	N		#3		8′ 5″	9′ 0″	10′ 9″	11′ 2″	12′ 10″	13′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	~	DFL	Stud	5′ 3″	8′ 5″	9′ 0″	10′ 9″	11′ 2″	12′ 10″	13′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	5′ 1 <b>″</b>	7′ 5″	7′ 11″	9′ 11″	10′ 7″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″



1x4 Braces shall be SRB (Stress-Rated Board) \*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

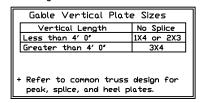
Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0' outlookers with 2'0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. ¥ For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩₩For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.



Refer to the Building Designer for conditions not addressed by this detail.

#### Symm C Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x4 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Constituous Bearing Connect diagonal at Refer to chart above son midpoint of vertical web.

\*\*\*VARNINGI\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING \*\*\*IMPORTANT\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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For more information see this job's general notes page and these web sites;
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.fbcacomponents.com; ICC: www.fbcacomponents.components.components.com; ICC: www.fbcacomponents.components.components.components.components.components.components.components.

MAX, TOT, LD, 60 PSF MAX. SPACING 24.0"

ak Kim EL DE #86367

514 Earth City Expressway Suite 242 Earth City, MO 63045

ASCE7-16-GAB14015 DATE 01/26/2018 DRWG A14015ENC160118

## Gable Detail For Let-in Verticals Gable Truss Plate Sizes Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs. (+) Refer to Engineered truss design for peak, splice, web, and heel plates. \*If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web. Gable Example: Length typ. (\* )

Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A14015ENC100118,

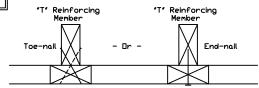
A18015ENC100118, A12015ENC100118, A12015ENC100118, A12015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A12003ENC100118, A12003ENC100118, A120030ENC100118, A120030ENC100118,

\$18015ENC100118, \$20015ENC100118, \$20015END100118, \$20015PED100118 \$11530ENC100118, \$12030ENC100118, \$14030ENC100118, \$12030ENC100118)

\$18030ENC100118, \$20030ENC100118, \$20030END100118, \$20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical

#### "T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

### Web Length Increase w/ "T" Brace

"T" Reinf.	<b>'</b> T'
Mbr. Size	Increase
2×4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30 (1) 2x4 "L" Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length  $1.30 \times 8' \ 7'' = 11' \ 2''$ 

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Refer to drawings 160A-Z for standard plate positions.

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engineering responsibility solely for the design shown. The suitability and use of this for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

IREF LET-IN VERT DATE 01/02/2018 DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY MAX. SPACING 24.0"

514 Earth City Expressway Suite 242

Earth City, MO 63045

Rigid Sheathing

Ceiling

4 Nails

Nails

Spaced At

4 Nails

Reinforcing Member

Gable

Truss

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com, TPI: www.tpinstorg, SBCA: www.sbcacomponents.com, ICC\_100/1/5/50020030

## Gable Stud Reinforcement Detail for Stucco Cladding

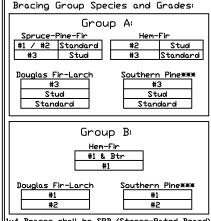
ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

						•						•		
		2×4 · Vertica	Brace	No	(1) 1×4 "L	" Brace *	(1) 2×4 *L	" Brace *	(2) 2×4 *L	" Brace **	(1) 2×6 *L	" Brace *	(2) 2x6 <b>1</b> L	Brace *
ا کا	Spacing	Species	Grade	Braces	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
		CDE	#1 / #2	3′ 9″	6′ 4″	6′ 7″	7′ 6″	7′ 10″	8′ 11″	9′ 4″	11′ 9″	12′ 3″	14′ 0″	14′ 0″
	1 -	SPF	#3	3′ 7″	6′ 3″	6′ 6″	7′ 5 <b>″</b>	7′ 8″	8′ 10 <b>″</b>	9′ 2″	11′ 7″	12′ 1″	13′ 10″	14′ 0″
<u>@</u>	Ų	HF	Stud	3′ 7″	6′ 3″	6′ 6″	7′ 5 <b>″</b>	7′ 8″	8′ 10 <b>″</b>	9′ 2″	11′ 7″	12′ 1″	13′ 10″	14′ 0″
	0		Standard	3′ 7″	5′ 8 <b>″</b>	6′ 0″	7′ 5 <b>″</b>	7′ 8″	8′ 10 <b>″</b>	9′ 2″	11′ 7″	12′ 1″	13′ 10″	14′ 0″
ا به اا			#1	3′ 11″	6′ 5 <b>″</b>	6′ 8″	7′ 7″	7′ 11″	9′ 0″	9′ 5″	11′ 11″	12′ 4″	14′ 0″	14′ 0″
	*	SP	#2	3′ 9″	6′ 4″	6′ 7″	7′ 6″	7′ 10″	8′ 11″	9′ 4″	11′ 9″	12′ 3″	14′ 0″	14′ 0″
	4		#3	3′ 8″	6′ 0 <b>″</b>	6′ 4″	7′ 5 <b>″</b>	7′ 9″	8′ 11″	9′ 3″	11′ 8″	12′ 2″	13′ 11″	14′ 0″
	$\Omega$	IDFL	Stud	3′ 8″	6′ 0″	6′ 4″	7′ 5″	7′ 9″	8′ 11″	9′ 3″	11′ 8″	12′ 2″	13′ 11″	14′ 0″
[] 전 [			Standard	3′ 7″	5′ 3″	5′ 7 <b>″</b>	7′ 0″	7′ 6″	8′ 10 <b>″</b>	9′ 2″	11′ 0″	11′ 10″	13′ 10″	14′ 0″
II .U I		lone.	#1 / #2	4′ 3″	7′ 3″	7′ 7″	8′ 7 <b>″</b>	8′ 11″	10′ 3″	10′ 8 <b>″</b>	13′ 6″	14′ 0″	14′ 0″	14′ 0″
+	_	SPF	#3	4′ 1″	7′ 2″	7′ 7″	8′ 6 <b>″</b>	8′ 10 <b>″</b>	10′ 1″	10′ 6″	13′ 4″	13′ 10′	14′ 0″	14′ 0″
	U	HF	Stud	4′ 1″	7′ 2″	7′ 5″	8′ 6 <b>″</b>	8′ 10 <b>″</b>	10′ 1″	10′ 6″	13′ 4″	13′ 10′		14′ 0″
ΠàΙ	Ō	1 11	Standard	4′ 1″	6′ 11″	7′ 5″	8′ 6 <b>″</b>	8′ 10 <b>″</b>	10′ 1″	10′ 6″	13′ 4″	13′ 10′		14′ 0″
$\Pi \vee \Pi$			#1	4′ 6″	7′ 4″	7′ 8″	8′ 8 <b>″</b>	9′ 0″	10′ 4″	10′ 9″	13′ 8″	14′ 0″	14′ 0″	14′ 0″
>	*	l SP	#2	4′ 3″	7′ 3″	7′ 7″	8′ 7 <b>″</b>	8′ 11″	10′ 3″	10′ 8″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
	ý Q		#3	4′ 2″	7′ 3″	7′ 6″	8′ 6 <b>″</b>	8′ 10 <b>″</b>	10′ 2″	10′ 7″	13′ 5″	13′ 11″		14′ 0″
ll orl	16	IDFL	Stud	4′ 2″	7′ 3″	7′ 6″	8′ 6 <b>″</b>	8′ 10 <b>″</b>	10′ 2″	10′ 7″	13′ 5 <b>″</b>	13′ 11″		14′ 0″
I I 🖳			Standard	4′ 1″	6′ 5″	6′ 10 <b>″</b>	8′ 6 <b>″</b>	8′ 10″	10′ 1″	10′ 6″	13′ 4″	13′ 10′		14′ 0″
ᄓᄱ		SPF	#1 / #2	4′ 8″	8′ 0 <b>″</b>	8′ 4″	9′ 5″	9′ 10″	10′ 3″	11′ 9″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	-		#3	4′ 6″	7′ 11″	8′ 2 <b>″</b>	9′ 4″	9′ 8″	11′ 1″	11′ 7″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
l	U	HF	Stud	4′ 6″	7′ 11″	8′ 2 <b>″</b>	9′ 4″	9′ 8″	11′ 1″	11′ 7″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	Ō	1 11	Standard	4′ 6″	7′ 11″	8′ 2 <b>″</b>	9′ 4″	9′ 8″	11′ 1″	11′ 7″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
$H \times H$			#1	4' 11"	8′ 1″	8′ 5 <b>″</b>	9′ 7″	9′ 11″	11′ 4″	11′ 10″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
ĉ	*	SP	#2	4′ 8″	8′ 0″	8′ 4″	9′ 5″	9′ 10″	11′ 3″	11′ 9″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
$  \breve{\Sigma} $	ù		#3	4′ 7″	7′ 11″	8′ 3 <b>″</b>	9′ 5″	9′ 9″	11′ 2″	11′ 8″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
	1,	IDFL	Stud	4′ 7″	7′ 11″	8′ 3 <b>″</b>	9′ 5″		11′ 2″	11′ 8″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	4′ 6″	7′ 5″	7′ 11″	9′ 4″	9′ 8″	11′ 1″	11′ 7″	14′ 0″	14′ 0″	14′ 0″	14′ 0″



1x4 Braces shall be SRB (Stress-Rated Board) \*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

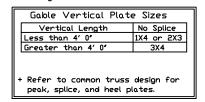
Gable Truss Detail Notes: Wind Load deflection criterion is 1/360.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0' outlookers with 2' 0' overhang, or 12' plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. \* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩₩For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.



Refer to the Building Designer for conditions not addressed by this detail.

#### Symm C Gable Truss Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web "L" Brace End total length is 14'. Zones, typ. 2x4 DF-L #2 or better diagonal brace; single Vertical length shown or double cut in table above. (as shown) at upper end. Constituous Bearing Connect diagonal at Refer to chart above son midpoint of vertical web.

\*\*\*VARNINGI\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING \*\*\*IMPORTANT\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and macing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI inless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Applicable to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any fallure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

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For more information see this job's general notes page and these web sites 15/2021 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.

MAX, TOT, LD, 60 PSF MAX. SPACING 24.0"

514 Earth City Expressway Suite 242 Earth City, MO 63045

ASCE7-16-GAB14015 DATE 01/26/2018 DRWG S14015ENC160118

# CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

## Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scale reinforcement.

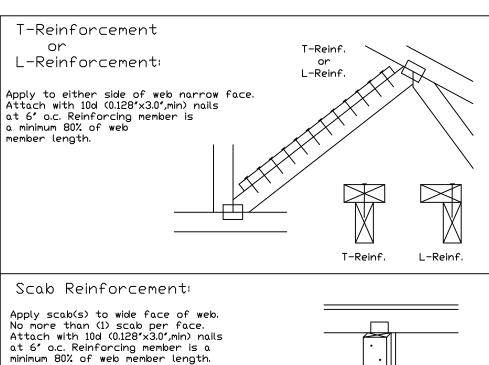
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

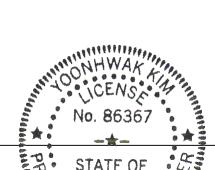
Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir			
Size	Restraint	T- or L- Reinf.			
2x3 or 2x4	1 row	2×4	1-2×4		
2x3 or 2x4	2 rows	2×6	2-2×4		
2×6	1 row	2×4	1-2×6		
2×6	2 rows	2×6	2-2×4( <b>米</b> )		
2×8	1 row	2×6	1-2×8		
2×8	2 rows	2×6	2-2×6( <b>*</b> )		

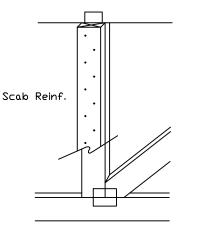
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(\*\*) Center scab on wide face of web. Apply (1) scab to each face of web.





278 Yoonhwak Kim, FL PE #86367



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Trusses require extreme care in fabricating, handling, shipping, installing into Installing and follow the latest edition of BCSI (Buldling Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installing and by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and botton chord shall have a properly attached rigid celling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or BIO, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-7 for standard plate positions.

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For more information see this job's general notes page and these web sites 5.44.

ALPINE: www.alpineitw.com; TPI: www.tpinstorg; SBCA: www.sbcacomponents.com; ICC: www.tccsareo.

TC LL PSF REF CLR Subst.

DATE 01/02/19

BC DL PSF DRWG BRCLBSUB0119

BC LL PSF PSF

DUR. FAC.

SPACING

AN ITW COMPA 514 Earth City Expressway Suite 242

Earth City, MO 63045

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

## Commentary: Deflection and Camber

L = Span of Truss (inches)

D = Depth of Truss at Deflection Point (inches)

## Recommended Truss Deflection Limits

<u>Truss Type</u>	L/D	<u>Deflection Limits</u>				
		<u>Live Load</u>	<u>Total Load</u>			
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)			
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)			
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)			
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)			
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)			
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)			

Truss Type Recommended Camber

Pitched Trusses 1.00 x Deflection from Actual Dead Load

Sloping Parallel 1.5 x Vertical Deflection from

Chord Trusses Actual Dead Load

Floor Trusses (0.25 x Deflection from Live Load) +

Actual Dead Load

Flat Roof №5 x Deflection from Live Load) + Design Dead Load Deflection)

dead load may be considerably less than

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Refer to drawings 160A-Z for standard plate positions.

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For more information see this job's general notes page and these web sites 15/3021-78, Yoonhwak Kim, FL PE #86367

**I**REF DEFLEC/CAMB DATE 10/01/14 DRWG DEFLCAMB1014



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## NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

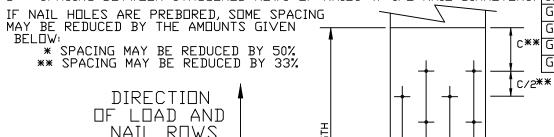
BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

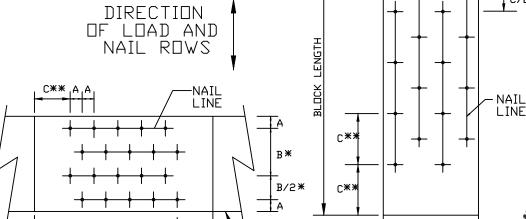
## LOAD PERPENDICULAR TO GRAIN

- A EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C END DISTANCE (15 NAIL DIAMETERS)

## LOAD PARALLEL TO GRAIN

- A EDGE DISTANCE (6 NAIL DIAMETERS)
- C SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
- D SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)





LOAD APPLIED PERPENDICULAR TO GRAIN

BLOCK LENGTH

LOAD APPLIED PARALLEL TO GAIN

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Earth City, MO 63045

# \*\*\*\*VARNING\*\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING \*\*\*\*\*IMPORTANT\*\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

TRUSS **MEMBER** 

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Bulding Component Safety Information, bry FII and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid celling. Locations shown for pernament lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or BIO, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise.

Refer to drawings 160A-Z for standard plate positions. WAS ONAL IN

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation a bracing of trusses.

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For more information see this job's general notes page and these web sites 15/2001. ALPINE: www.alpineitw.com; TPI: www.tpinstorg; SBCA: www.sbcacomponents.com; ICC: while the sites 1878, Yoonhwak Kim, FL PE #86367

MINIMUM NAIL SPACING DISTANCES

	DIS	TANCES		
NAIL TYPE	Α	Вж	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8″
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d CDMMDN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d C□MM□N (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"

REF NAIL SPACE DATE 10/01/14

DRWG CNNAILSP1014