DATE 05/05/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024466

APPLICANT CHARESE NORTON PHONE 752-3331

ADDRESS 3367 S US HIGHWAY 441 LAKE CITY FL 32025

OWNER ELMER & RITA NICHOLS PHONE 752-1221

ADDRESS 1381 SW BEDENBAUGH LANE LAKE CITY FL 32025

CONTRACTOR JAMES NORTON PHONE 752-3331

LOCATION OF PROPERTY 441S, TR ON CR 131, TR ON BEDENBAUGH LANE, SITE ON LEFT
JUST BEFORE PAVEMENT ENDS

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 76650.00

HEATED FLOOR AREA 1533.00 TOTAL AREA 1921.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 19

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 30-4S-17-08914-002 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES

		RB0031780	X Charese J. North	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
EXISTING	06-0415-N	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD
SPECIAL FAMILY LOT PERMIT, DAUGHTER TO MOTHER

Check # or Cash 21608

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 385.00 CERTIFICATION FEE $ 9.61 SURCHARGE FEE $ 9.61

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 479.22

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0604-85 Date Received 4/26/06 By LH Permit # 24464

Application Approved by - Zoning Official BLK Date 04.05.06 Plans Examiner OK JTH Date 5-1-06

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments Section 14.9 Special Family Lot Permit Daughter to mother

NOC, EH CK# 2608 06-0415-N

Applicants Name Charese Norton, @ Norton Home Improvement Phone 386-752-3331

Address 3367 S US HWY 441, Ste 101, Lake City, FL 32025

Owners Name Elmer & Rita Nichols Phone 386-752-1221

911 Address 1381 SW Bedenbaugh Lane, Lake City, FL 32025

Contractors Name James H. Norton Phone 386-752-3331

Address 3367 S US HWY 441, Ste 101, Lake City, FL 32025

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address Mark Disoway, P.O. Box 868, Lake City, FL 32056 Tim Delbene, 192 SW Sagewood Gln, Lake City, FL 32024

Mortgage Lenders Name & Address 1st Fed., 4705 W. US. Hwy 90, Lake City, FL 32056

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 30-4S-17-08914-002 Estimated Cost of Construction $100,000.00

Subdivision Name — Lot ___ Block ___ Unit ___ Phase ___

Driving Directions 441 South to 131, turn Right go down to Bedenbaugh lane, turn Right and site @ is on left just before pavement ends.

Type of Construction SFD, new home construct. Number of Existing Dwellings on Property 0

Total Acreage .50 Lot Size ___ Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive)

Actual Distance of Structure from Property Lines - Front 97.3' Side 37.7' Side 37.7' Rear 36'

Total Building Height 19' Number of Stories 1 Heated Floor Area 1533 Roof Pitch 6/12

Porch 388 TOTAL 1921

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James H. Norton

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 26th day of April 2006.

Personally known ✓ or Produced Identification ___

Patricia T Peeler
My Commission DD129966
Expires September 05, 2006

James H. Norton

Contractor Signature

Contractors License Number RB0031780

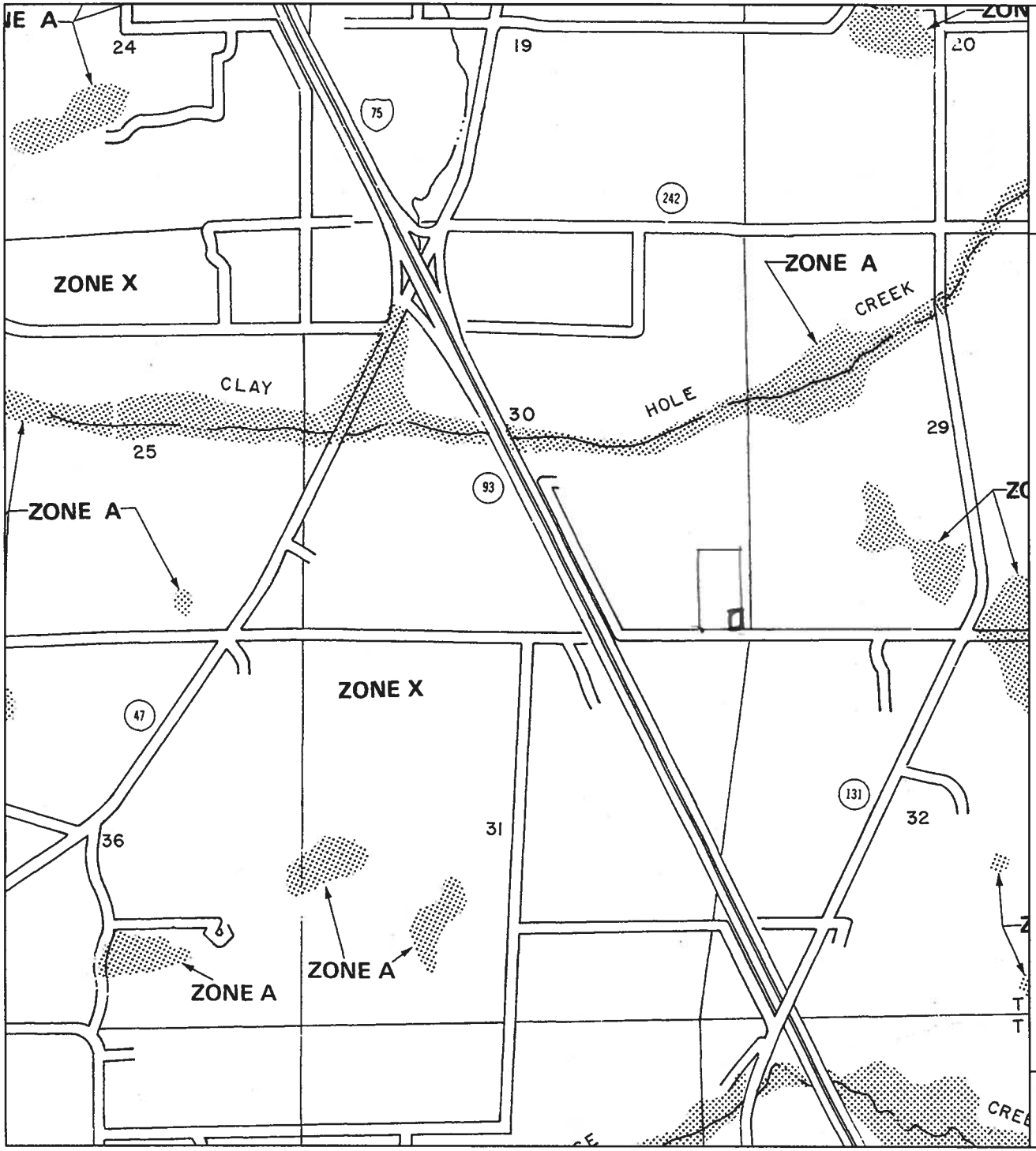
Competency Card Number 5553

NOTARY STAMP/SEAL

Patricia T. Peeler

Notary Signature

JW called CHARESE 5.5.06 -



N

APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

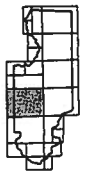
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 175 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0175 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 5/1/2006 (printed at scale and type A)

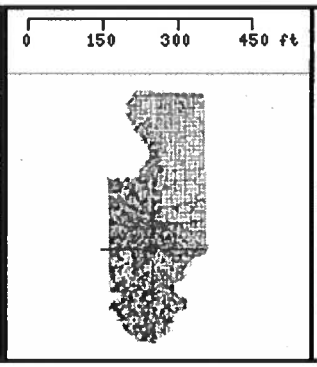
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

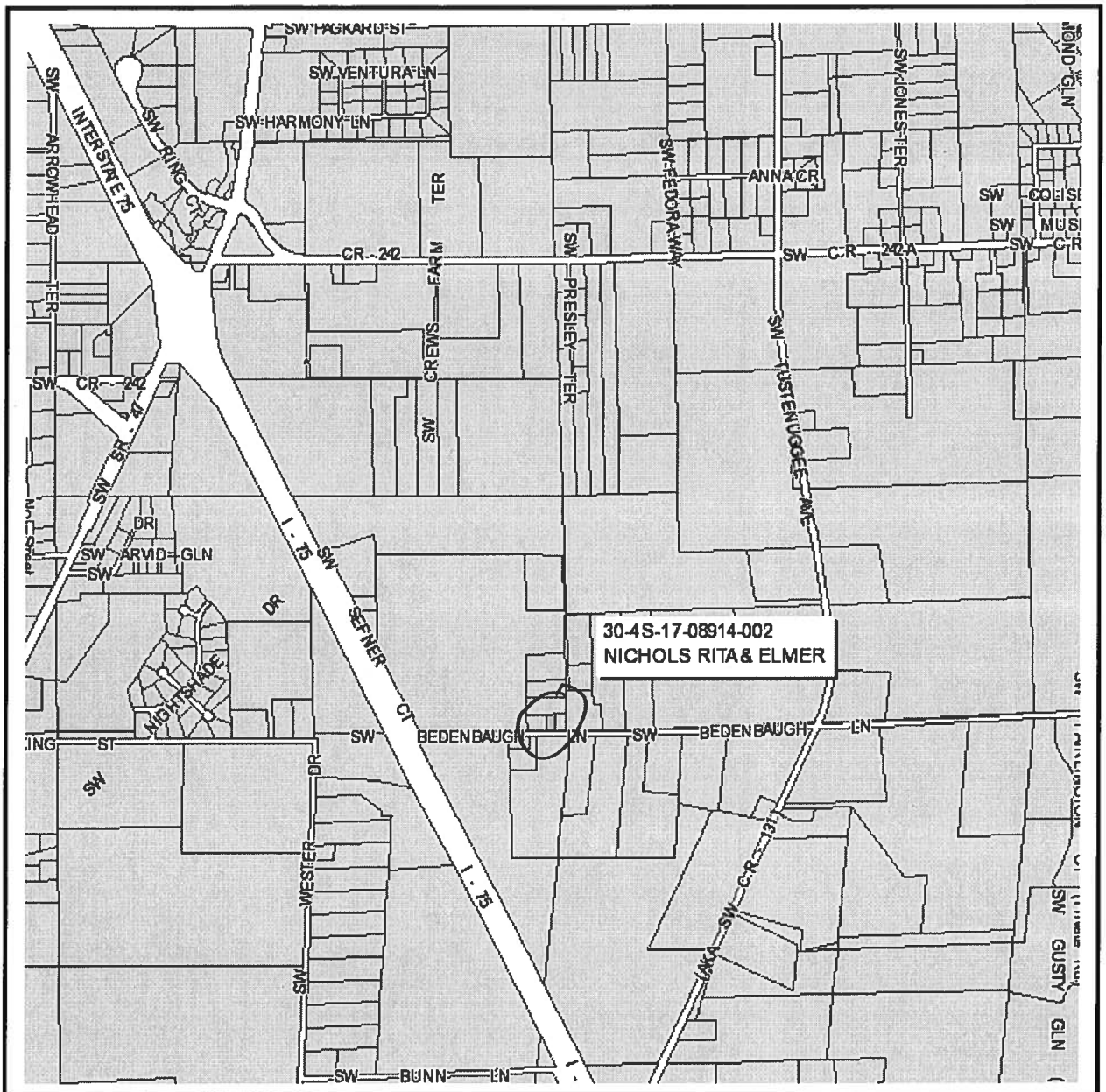
PARCEL: 30-4S-17-08914-002 - NO AG ACRE (009900)

THE W1/2 OF E1/2 OF THE FOLLOWNG DESC:A PARCEL 449.4 FT E & W BY 194.7 FT N & S IN

Name:	NICHOLS RITA & ELMER	LandVal	$12,000.00
Site:		BldgVal	$0.00
	439 SE LILLIAN LOOP	ApprVal	$12,000.00
Mail:	APT 104	JustVal	$12,000.00
	LAKE CITY, FL 32025	Assd	$12,000.00
Sales	2/9/2006 $100.00 V / U	Exmpt	$0.00
Info	12/28/2000 $100.00 V / U	Taxable	$12,000.00



This information, GIS Map Updated: 4/6/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.



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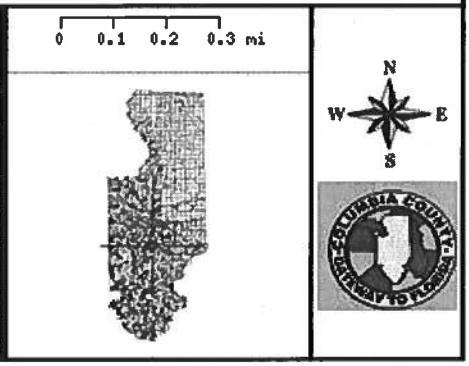

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Columbia County Property Appraiser

DB Last Updated: 3/7/2006

2006 Proposed Values

Parcel: 30-4S-17-08914-002 Tax Record Property Card Interactive GIS Map Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	NICHOLS RITA & ELMER
Site Address	
Mailing Address	439 SE LILLIAN LOOP APT 104 LAKE CITY, FL 32025
Brief Legal	THE W1/2 OF E1/2 OF THE FOLLOWNG DESC:A PARCEL 449.4 FT E & W BY 194.7 FT N & S IN

Use Desc. (code)	NO AG ACRE (009900)
Neighborhood	30417.00
Tax District	2
UD Codes	MKTA01
Market Area	01
Total Land Area	0.500 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$12,000.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$12,000.00

Just Value	$12,000.00
Class Value	$0.00
Assessed Value	$12,000.00
Exempt Value	$0.00
Total Taxable Value	$12,000.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
2/9/2006	1073/2066	QC	V	U	01	$100.00
12/28/2000	918/1522	WD	V	U	03	$100.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	.500 AC	1.00/1.00/1.50/1.00	$24,000.00	$12,000.00

Columbia County Property Appraiser DB Last Updated: 3/7/2006

1 of 1

Inst:2006003329 Date:02/10/2006 Time:15:15
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1073 P:2066

Above Space Reserved for Recording
[If required by your jurisdiction, list above the name & address of: 1) where to return this form; 2) preparer; 3) party requesting recording.]

Quitclaim Deed

Date of this Document: 2/9/2006

Reference Number of Any Related Documents: _____

Grantor:

Name KATHY HOUK
Street Address 1401 SW BEDENBAUGH LN
City/State/Zip LAKE CITY, FL 32025

Grantee:

Name RITA + ELMER NICHOLS
Street Address 439 SE LILLIAN LOOP APT. #104
City/State/Zip LAKE CITY, FL 32025

Abbreviated Legal Description (i.e., lot, block, plat *or* section, township, range, quarter/quarter *or* unit, building and condo name): SEE Attatchment A

Assessor's Property Tax Parcel/Account Number(s): 30-4S-17-08914-002

THIS QUITCLAIM DEED, executed this NINTH day of FEBRUARY, 20 06, by first party, Grantor, KATHY HOUK, whose mailing address is 1401 SW BEDENBAUGH LN, to second party, Grantee, RITA + ELMER NICHOLS, whose mailing address is 439 SE LILLIAN LOOP APT. #104, LAKE CITY FL.

WITNESSETH that the said first party, for good consideration and for the sum of ONE HUNDRED Dollars ($ 100.00) paid by the said second party, the receipt whereof is hereby acknowledged, does hereby remise, release and quitclaim unto the said second party forever, all the right, title, interest and claim,

which the said first party has in and to the following described parcel of land, and improvements and appurtenances thereto in the County of Columbia, State of FLORIDA to wit: _____

IN WITNESS WHEREOF, the said first party has signed and sealed these presents the day and year first written above. Signed, sealed and delivered in the presence of:

Inst:2006003328 Date:02/10/2006 Time:15:15
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1073 P:2067

Signature of Witness _____

Print Name of Witness _____

Signature of Witness Kenneth Green

Print Name of Witness Kenneth Green

Signature of Grantor Kathy R Houk

Print Name of Grantor KATHY R HOUK

State of Florida)
County of Columbia)

On February 9, 2006, before me, Kathy R. Houk, appeared _____, personally known to me (or proved to me on the basis of satisfactory evidence) to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

WITNESS my hand and official seal.

W. Dale Menninger
Signature of Notary

Affiant ✓ Known ____ Produced ID
Type of ID _____
(Seal)

W. Dale Menninger
Commission # DD289041
Expires May 13, 2008
Bonded Thru Pain - Insurance, Inc. 800-385-7019

Columbia County Tax Collector

generated on 2/9/2006 4:41:19 PM EST

Tax Record

Attatchment A

DATA VIEW AS OF: 2/9/2006 4:41:19 PM ET

Ad Valorem Taxes and Non-Ad Valorem Assessments

The information contained herein does not constitute a title search and should not be relied on as such.

Account Number	Tax Type	Tax Year
R08914-002	Real Estate	2005

Mailing Address
HOUK KATHY R
LAKE CITY FL 32025

Folio
125515.0000

Assessed Value	Exempt Amount	Taxable Value
$7,200.00	$0.00	$7,200.00

Exemption Detail
NO EXEMPTIONS

Millage Rate
002 19.06040

Legal Description
THE W1/2 OF E1/2 OF THE FOLLOWNG DESC:A PARCEL 449.4 FT E & W BY 194.7 FT N & S IN SE COR OF SE1/4 OF SE1/4. ORB 523-323 THRU 325, 678-614, 918-1519, 918-1522,

Tax Districts Detail

Code	Description	Exemption	Amount
C001	BOARD OF COUNTY COMMISSIONERS	$0.00	$62.83
S002	COLUMBIA COUNTY SCHOOL BOARD	$0.00	$57.27
W SR	SUWANNEE RIVER WATER MGT DIST	$0.00	$3.54
HLSH	LAKE SHORE HOSPITAL AUTH	$0.00	$12.60
IIDA	INDUSTRIAL DEVELOPEMENT AUTH	$0.00	$0.99
FFIR	FIRE ASSESSMENTS	$0.00	$28.09

Total Gross	$165.32
Discount	($1.65)
Total	$163.67

If Paid By	Amount Due
02/28/2006	$163.67
03/31/2006	$165.32

Prior Year Taxes Due

NO DELINQUENT TAXES

Click Here to Pay Now

Inst:200600329 Date:02/10/2006 Time:15:15
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1073 P:2068

Rec 15.00
due .70

FILED AND RECORDED IN PUBLIC RECORDS OF COLUMBIA COUNTY, FL.

'01 JAN 22 PM 4:02

BK 0918 PG 1522 01-01230

OFFICIAL RECORDS

WARRANTY DEED

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

THIS INDENTURE, made this 23 day of December, 2000, between **TOMMY D. HOUK**, a single person, and **RITA NICHOLS f/k/a RITA HOUK**, a married woman not residing on the property, parties of the first part, Grantor, and **KATHY R. HOUK**, (Social Security No. 590-40-1327), whose mailing address is Route 12, Box 10-S, Lake City, Florida 32025, party of the second part, Grantee,

W I T N E S S E T H:

That said grantor, for and in consideration of the sum of **TEN AND NO/100 ($10.00) DOLLARS**, and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs, successors and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

TOWNSHIP 4 SOUTH, RANGE 17 EAST

Section 30: The W 1/2 of the E 1/2 of the following described real property:

449.4 feet East and West by 194.7 feet North and South in the Southeast corner of SE 1/4 of SE 1/4, Page 672 of 1976 tax roll; more particularly described as follows: Commence at the SE Corner of Section 30, Township 4 South, Range 17 East, Columbia County, Florida, and run N 2°54'37" W along the East line of said Section 30, a distance of 219.49 feet; thence S 87°58'49" W 53.4 feet to the **POINT OF BEGINNING**; thence S 1°20'05" E 194.70 feet; thence S 87°58'49" W 449.40 feet; thence N 1°20'05" W 194.70 feet; thence N 87°58'49" E 449.40 feet to the **POINT OF BEGINNING**.

Prepared by Marlin M. Feagle
Attorney at Law
Post Office Box 1653
Lake City, Florida 32055
(as to form only)

Documentary Stamp .70
Intangible Tax
P. DeWitt Cason
Clerk of Court
By DC

SUBJECT TO reservations, restrictions and easement of record, if any, together with ad valorem taxes and non-ad valorem assessments subsequent to December 31, 2000.

Tax Parcel No.: 30-4S-17-08914-000

and said grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claim of all persons whatsoever.

IN WITNESS WHEREOF, Grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered
in the presence of:

Becky Westberry
Witness
Becky Westberry
(Print or type name)

Bobbi Goss
Witness
Bobbi Goss
(Print or type name)

Tommy D. Houk (SEAL)
TOMMY D. HOUK

Signed, sealed and delivered
in the presence of:

Becky Westberry
Witness
Becky Westberry
(Print or type name)

Bobbi Goss
Witness
Bobbi Goss
(Print or type name)

Rita Nichols (SEAL)
RITA NICHOLS
f/k/a Rita Houk

STATE OF FLORIDA
COUNTY OF COLUMBIA

I HEREBY CERTIFY that on this day before me, an officer duly qualified to take acknowledgments, personally appeared **TOMMY D. HOUK** who is personally known to me or who has produced Personally Known as identification.

WITNESS my hand and official seal in the County and State last aforesaid this 29 day of December, 2000.

Donna S. Higgs
NOTARY PUBLIC
(NOTARIAL SEAL) **MY COMMISSION EXPIRES:**

Donna S. Higgs
MY COMMISSION # CC[illegible] EXPIRES
March 31, 200[illegible]
BONDED THRU TROY FAIN INSURANCE, INC.

STATE OF FLORIDA
COUNTY OF COLUMBIA

I HEREBY CERTIFY that on this day before me, an officer duly qualified to take acknowledgments, personally appeared **RITA NICHOLS f/k/a RITA HOUK** who is personally known to me or who has produced Personally Known as identification.

WITNESS my hand and official seal in the County and State last aforesaid this 29 day of December, 2000.

Donna S. Higgs
NOTARY PUBLIC
(NOTARIAL SEAL) **MY COMMISSION EXPIRES:**

Donna S. Higgs
MY COMMISSION # CC[illegible] EXPIRES
March 31, 200[illegible]
BONDED THRU TROY FAIN INSURANCE, INC.

COLUMBIA COUNTY 9-1-1 ADDRESSIN(

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.co

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 1/27/2006 **DATE ISSUED:** 2/7/200

ENHANCED 9-1-1 ADDRESS:

1381 SW BEDENBAUGH LN
LAKE CITY FL 32025

PROPERTY APPRAISER PARCEL NUMBER:

30-4S-17-08914-002

Remarks:

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

56

COLUMBIA COUNT
9-1-1 ADDRESSIN
APPROVED

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL 32025
Phone 386-752-6677
Fax 386-752-1477

Building Permit #_____ Owner's Name Nichols

Well Depth_____ Ft. Casing Depth _____Ft. Water Level _____Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Aermotor Pump Model S20-100 HP 1

System Pressure (PSI) _____On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 20 (GPM)

Tank Installation: (Bladder) Galvanized Make Challenger
Model PC244 Size 81

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

4-20-06
Date

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Nichols Residence**	Builder:	**Jim Norton**
Address:	**SW Bedebaugh Lane**	Permitting Office:	**Columbia Co.**
City, State:	**Lake City, FL 32055-**	Permit Number:	**24466**
Owner:	**Nichols**	Jurisdiction Number:	**~~12~~1000**
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		No	
6. Conditioned floor area (ft²)		1533 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	141.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 170.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1177.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1533.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 20.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 35.0 kBtu/hr	
	SEER: 14.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	
	HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 30.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.09

Total as-built points: 18656
Total base points: 24301

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene

DATE: 3/11/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Bedebaugh Lane, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1533.0	20.04	5529.8	Double, Clear	N	2.0	5.0	9.0	19.20	0.87	150.5
				Double, Clear	E	2.0	5.0	9.0	42.06	0.80	301.7
				Double, Clear	E	2.0	7.0	30.0	42.06	0.89	1117.9
				Double, Clear	S	8.0	7.0	30.0	35.87	0.50	538.2
				Double, Clear	S	2.0	7.0	30.0	35.87	0.82	882.5
				Double, Clear	W	2.0	3.0	3.0	38.52	0.64	73.8
				Double, Clear	W	2.0	7.0	30.0	38.52	0.89	1024.8
				As-Built Total:				**141.0**			**4089.4**

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1177.0	1.50	1765.5
Exterior	1177.0	1.70	2000.9					
Base Total:	**1177.0**		**2000.9**	**As-Built Total:**		**1177.0**		**1765.5**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Exterior Insulated	21.0	4.10	86.1
Exterior	42.0	6.10	256.2	Exterior Insulated	21.0	4.10	86.1
Base Total:	**42.0**		**256.2**	**As-Built Total:**	**42.0**		**172.2**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	1533.0	1.73	2652.1	Under Attic	30.0	1533.0	1.73 X 1.00	2652.1
Base Total:	**1533.0**		**2652.1**	**As-Built Total:**		**1533.0**		**2652.1**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	170.0(p)	-37.0	-6290.0	Slab-On-Grade Edge Insulation	0.0	170.0(p	-41.20	-7004.0
Raised	0.0	0.00	0.0					
Base Total:			**-6290.0**	**As-Built Total:**		**170.0**		**-7004.0**

INFILTRATION	Area X	BSPM	= Points		Area X	SPM	= Points
	1533.0	10.21	15651.9		1533.0	10.21	15651.9

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Bedebaugh Lane, Lake City, FL, 32055-**	PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**19801.0**	**Summer As-Built Points:**					**17327.1**
Total Summer Points	System Multiplier	Cooling Points	Total Component	Cap Ratio	Duct Multiplier (DM x DSM x AHU)	System Multiplier	Credit Multiplier	Cooling Points
19801.0	**0.4266**	**8447.1**	17327.1 **17327.1**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.244 **0.244**	0.902 **0.902**	4337.2 **4337.2**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Bedebaugh Lane, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1533.0	12.74	3515.5	Double, Clear	N	2.0	5.0	9.0	24.58	1.01	222.7	
				Double, Clear	E	2.0	5.0	9.0	18.79	1.08	183.2	
				Double, Clear	E	2.0	7.0	30.0	18.79	1.05	589.4	
				Double, Clear	S	8.0	7.0	30.0	13.30	2.96	1182.5	
				Double, Clear	S	2.0	7.0	30.0	13.30	1.17	467.1	
				Double, Clear	W	2.0	3.0	3.0	20.73	1.12	69.6	
				Double, Clear	W	2.0	7.0	30.0	20.73	1.03	641.3	
				As-Built Total:				**141.0**			**3355.9**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM	=	Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1177.0	3.40		4001.8	
Exterior	1177.0	3.70	4354.9									
Base Total:	**1177.0**		**4354.9**	**As-Built Total:**				**1177.0**			**4001.8**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM	=	Points	
Adjacent	0.0	0.00	0.0	Exterior Insulated				21.0	8.40		176.4	
Exterior	42.0	12.30	516.6	Exterior Insulated				21.0	8.40		176.4	
Base Total:	**42.0**		**516.6**	**As-Built Total:**				**42.0**			**352.8**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1533.0	2.05	3142.6	Under Attic			30.0	1533.0	2.05 X 1.00		3142.6	
Base Total:	**1533.0**		**3142.6**	**As-Built Total:**				**1533.0**			**3142.6**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM	=	Points	
Slab	170.0(p)	8.9	1513.0	Slab-On-Grade Edge Insulation			0.0	170.0(p	18.80		3196.0	
Raised	0.0	0.00	0.0									
Base Total:			**1513.0**	**As-Built Total:**				**170.0**			**3196.0**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM	=	Points	
	1533.0	-0.59	-904.5					1533.0	-0.59		-904.5	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Bedebaugh Lane, Lake City, FL, 32055-**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**12138.2**	**Winter As-Built Points:**					**13144.6**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
12138.2	**0.6274**	**7615.5**	13144.6 **13144.6**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.432 **0.432**	0.950 **0.950**	6264.3 **6264.3**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Bedebaugh Lane, Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2746.00	8238.0	30.0	0.90	3	1.00	2684.98	1.00	8054.9	
				As-Built Total:						**8054.9**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
8447	**7615**	**8238**	**24301**	**4337**	**6264**	**8055**	**18656**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **SW Bedebaugh Lane, Lake City, FL, 32055-**	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	N/A
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

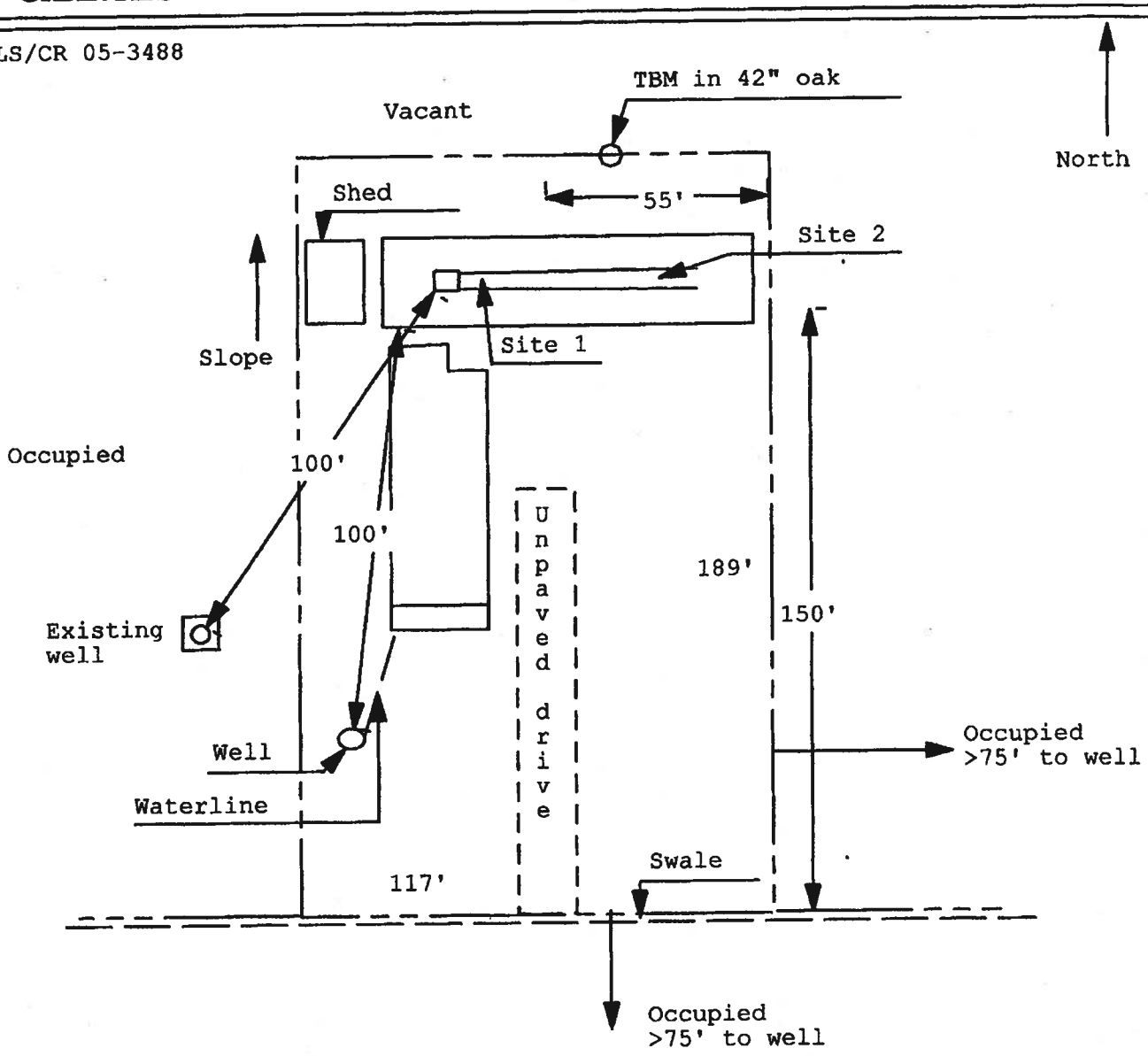
COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-045N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

NICHOLS/CR 05-3488



1 inch = 40 feet

Site Plan Submitted By Paul Land **Date** 4/21/06

Plan Approved ✓ **Not Approved** ____ **Date** 5/1/06

By [signature] (Columbia) **CPHU**

Notes: ____

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE-------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE--------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORESUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
✓	•	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
✓	•	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
✓	•	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
✓	•	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
✓	•	**Elevations including:**
✓	•	a) All sides
✓	•	b) Roof pitch
✓	•	c) Overhang dimensions and detail with attic ventilation
NA	•	d) Location, size and height above roof of chimneys
NA	•	e) Location and size of skylights
✓	•	f) Building height
✓	•	e) Number of stories

Floor Plan including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

b) Floor joist size and spacing

c) Girder size and spacing

d) Attachment of joist to girder

e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

b) Ceiling fans

c) Smoke detectors

d) Service panel and sub-panel size and location(s)

e) Meter location with type of service entrance (overhead or underground)

f) Appliances and HVAC equipment

g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

a) Manual J sizing equipment or equivalent computation

b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

need: recorded NOC
ENV. HEALTH form
Manual J.

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING

BUILDING CODE COMPLIANCE OFFICE
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

CONTRACTOR LICENSING SECTION
(305) 375-2527 FAX (305) 375-2558

CONTRACTOR ENFORCEMENT DIVISION
(305) 375-2966 FAX (305) 375-2908

PRODUCT CONTROL DIVISION
(305) 375-2902 FAX (305) 372-6339

PRODUCT CONTROL NOTICE OF ACCEPTANCE

Premdor Entry Systems
911 E. Jeferson, P.O. Box 76
Pittsburgh ,KS 66762

Your application for Notice of Acceptance (NOA) of:
Series Entergy 6-8 S-W/E Inswing Opaque Residential Insulated Steel Door w/H. M. Frame
under Chapter 8 of the Code of Miami-Dade County governing the use of Alternate Materials and Types of Construction, and completely described herein, has been recommended for acceptance by the Miami-Dade County Building Code Compliance Office (BCCO) under the conditions specified herein.

This NOA shall not be valid after the expiration date stated below. BCCO reserves the right to secure this product or material at any time from a jobsite or manufacturer's plant for quality control testing. If this product or material fails to perform in the approved manner, BCCO may revoke, modify, or suspend the use of such product or material immediately. BCCO reserves the right to revoke this approval, if it is determined by BCCO that this product or material fails to meet the requirements of the South Florida Building Code.

The expense of such testing will be incurred by the manufacturer.

ACCEPTANCE NO.: 01-0313.06
EXPIRES: 02/19/2006

Raul Rodriguez
Chief Product Control Division

THIS IS THE COVERSHEET, SEE ADDITIONAL PAGES FOR SPECIFIC AND GENERAL CONDITIONS

BUILDING CODE & PRODUCT REVIEW COMMITTEE

This application for Product Approval has been reviewed by the BCCO and approved by the Building Code and Product Review Committee to be used in Miami-Dade County, Florida under the conditions set forth above.

RECEIVED
APR 27 2001
PITTSBURG, KANSAS

APPROVED: 04/19/2001

Francisco J. Quintana, R.A.
Director
Miami-Dade County
Building Code Compliance Office

Premdor Entry Systems

ACCEPTANCE NO.: 01-0313.06

APPROVED : APR 19 2001

EXPIRES : February 19, 2006

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. DESCRIPTION OF UNIT

1.1 This renews the Notice of Acceptance No. 97-0910.11 which was issued on February 19, 1998. It approves a residential insulated steel door, as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.

2. PRODUCT DESCRIPTION

2.1 The Series Entergy 6' 8" S-W/E Inswing Opaque Residential Insulated Steel Door in a Hollow Metal Frame-Impact and its components shall be constructed in strict compliance with the following documents: Drawing No 31-1032-EW-I, Sheets 1 through 5 of 5; titled "Premdor (Entergy Brand Wood Edge) 3'0" x 6'8" Steel door in a Hollow Metal Frame (Inswing)" dated 6/25/97 with revision C. dated 3/20/01, bearing the Miami-Dade County Product Control approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. LIMITATIONS

3.1 This approval applies to single unit applications of single door only, as shown in approved drawings.

3.2 Unit shall be installed only at locations protected by a canopy or overhang such that the angle between the edge of canopy or overhang to sill is less than 45 degrees. Unless unit is installed in non-habitable areas where the unit and the area are designed to accept water infiltration.

4. INSTALLATION

4.1 The residential insulated steel door and its components shall be installed in strict compliance with the approved drawings.

4.2 Hurricane protection system (shutters): the installation of this unit will not require a hurricane protection system.

5. LABELING

5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".

6. BUILDING PERMIT REQUIREMENTS

6.1 Application for building permit shall be accompanied by copies of the following:

6.1.1 This Notice of Acceptance

6.1.2 Duplicate copies of the approved drawings, as identified in Section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.

6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.

Manuel Perez, P.E. Product Control Examiner
Product Control Division

AMTROL INC.

WEL-FLO®
Pre-pressurized Water System Tanks

- Proven Diaphragm Design
- Tough Gloss Finish
- Sizes from 14 to 119 Gallons
- Outstanding Value

NEW Home CONST ONLY

Pump and Tank Code
Section 613
Well Pumps and Tanks used for private potable water systems
~~July 1, 2001~~ March 1, 2002

613.1 Pumps. Well pumps used for potable water shall comply with sections 613.1.1 and 613.1.2
613.1.1 Pump Installation. Pumps shall be installed for operation without re-priming or breaking suction. Pumps shall be connected to the well head by means of a union, companion flange or compression coupling in such a manner that it is accessible for maintenance, repair and, removal.
613.1.2 Pump Sizing. Minimum pump size shall be determined by table 613.1.

Table 613.1
Minimum Private Potable Water System Pump Size

	Bathrooms in Home				
	1	1 ½	2-2 ½	3-4	5-6
Minimum Pump Size	7gpm	10gpm	14gpm	17gpm	21gpm

Notes:
1. Values given are average and do not include high and low extremes
2. Installations over 6 bathrooms shall be approved by the code official

613.2 Pressure Tanks. Tanks relying on expansion of a flexible membrane within a restricting container, or tanks with direct water- to- air interface to provide pressure in the water system shall be used. All pressure tanks for storing potable water under pressure, including those having an airspace for pressure for expansion shall be identified by seal, label, or plate indicating the manufacturer's name and model number and shall meet the following specifications:

1. Pressure tank drawdown shall be a minimum of 1 gallon for every gallon produced by the pump (Example: 20 gallon per minute pump will require a draw of 20 gallons usable). Exceptions: Pump start applications, constant pressure devices and variable speed pumps.
2. Pressure tanks must be constructed of steel, fiberglass, or comparable materials. Tanks to be buried shall have a minimum wall thickness of ¼ inch and be built by the manufacture specifically for underground use. Fiberglass or other non-metallic tanks to be buried shall have the structural strength to prevent collapse.

613.3 Piping. Piping associated with well pumps and tanks shall comply with Sections 613.3.1 through 613.3.

613.3.1 Drop Pipe. The Drop pipe from the submersible pump to the first fitting past the well seal shall be either galvanized steel, stainless steel, or PVC Schedule 80 threaded/coupled or lock joint pipe. The drop pipe for a single (pipe) jet pump shall be either galvanized steel, or stainless steel. The drop pipe for a double (pipe) jet shall be galvanized steel, stainless steel on the suction side and/or minimum PVC Schedule 40 on the pressure side.

613.3.2 Pump Discharge pipe sizing. For submersible pumps, pipe size shall be equal to the pump discharge. Piping for all other types of pumps shall be sized in accordance to the manufacturers' specifications.

613.3.3 Pressure Tank Pipe Sizing. Piping size for the offset of the pressure tank shall use the piping friction loss charts for the piping material used.

613.4 Electrical wiring. All wiring shall be installed in accordance with chapter 27 of the Florida Building code and NFPA 70.

613.5 Disinfection. The pump installer shall disinfect any potable well and water system in accordance with Section 610.

613.6 Valves. A pressure relief valve shall be installed on any pumping system that can produce pressures of 75 psi or greater. A check valve shall be installed at the well head of submersible pumps.

* Cycle STOP VALVE'S ARE CONSTANT PRESS DEVICE
* Countys MAY Add Higher Demands

'ROL INC.

WELL-X-TROL 5

essurized Diaphragm Well Tanks

CHAMPION, WEL-FLO, PRO-LINE *See Rat Sheet* (A)

Model / Part No.	List Price ($)	Diameter (ins)	Dimensions Height (ins)	Total Volume (gals)	Max. Accept. Factor	System Drawdown 20/40 (gals)	30/50 (gals)	40/60 (gals)	Shipping Wt. (Vol.) lbs (cu ft)
CH 4202/WF60/CA4202	213.00	15¾	31⅛	20.0	0.57	8.0	6.8	5.9	33 (4.9)
CH 6000/WF80/CA6000	225.00	15¾	38¼	26.0	0.44	10.5	8.8	7.6	36.0
CH 8003/WF100/CA8003	364.00	15¾	46½	32.0	0.35	–	10.9	9.4	43 (7.0)
CH 8205/WF110/CA8205	399.00	22	29⅝	34.0	1.00	13.7	11.6	10.0	61 (9.5)
CH 10050/WF140/CA10050	461.00	22	36	44.0	0.77	17.7	15.0	13.0	69 (11.0)
CH 12051/WF200/CA12051	545.00	22	46¼	62.0	0.55	24.9	21.1	18.3	92 (13.9)
CH 17255/WF255/CA17255	585.00	22	56⅛	81.0	0.41	32.6	27.5	23.9	103
CH 17252/WF252/CA17252	663.00	22	62¼	86.0	0.39	34.6	29.2	25.4	114 (18.1)
CH 17002/WF260/CA17002	647.00	26	47¼	86.0	0.54	34.6	29.2	25.4	123 (18.9)
CH 22050/WF360/CA22050	822.00	26	51⅛	119.0	0.39	47.8	40.5	35.1	165 (24.5)

CH4202, CH6000, CH8003, WF60, WF80, WI100, CA 4202, CA6000, & CA8003 have a 1" NPTF system connection and a 28 psig pre-charge.

[illegible] CH22050, WF110, WF260, CA8205, CA22050 have a 1¼" NPTF system connection and a 38

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

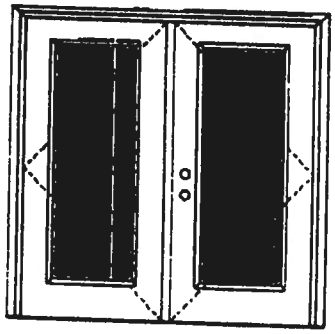
2300 35th STREET P.O. BOX 2149 TUSCALO

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


130, 135 Series


136 Series


880 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


120 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 5-panel with scroll.

Johnson Entry Systems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

PREMDOOR Collection

Exclusively from Masonite

Masonite International Corporation

XX
Glazed Inswing Unit

COP-WI-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


100 Series


114, 120, 122 Series


152 Series


140 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Bathy

State of Florida, Professional Engineer
Kurt Bathazor, P.E. – License Number 56533

Johnson™ EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product details subject to change without notice.

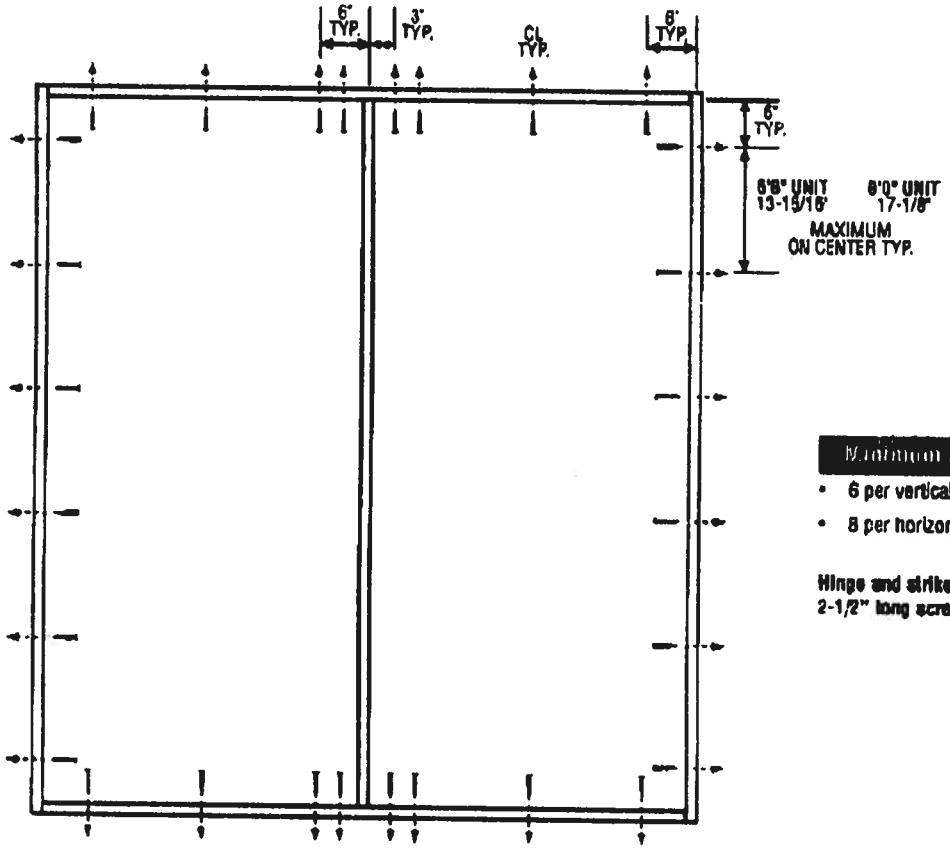
PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and penetrement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
CERTIFICATE
NO. 18354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

ALLEN N. REEVES
CERTIFICATE
NO. 19256
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PROFESSIONAL ENGINEER

Allen N. Reeves
7 JUNE 2002

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

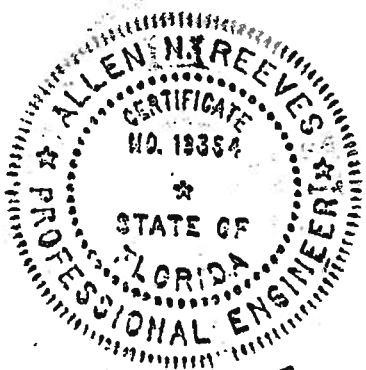
Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



Allen N. Reeves
7 JUNE 2002

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 13354 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.1	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the mullion)		
	(Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.15"	0.41" max.
	@ 15.0 psf (negative)	0.29"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the mullion)		
	(Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.29" max.
	@ 22.5 psf (negative)	0.01"	0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88)		
	In operating direction at 70 lbs		
	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES ★ CERTIFICATE NO. 19394 ★ STATE OF FLORIDA ★ PROFESSIONAL ENGINEER ★

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
7 June 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

MAH:nlb
01-41641.01

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

ALLEN M. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves
1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

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Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
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STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES
CERTIFICATE NO. 19[illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Allen N. Reeves
1 APRIL 2002

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NO. 1337[illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987) In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
@ 67.5 psf (positive)	0.05"	0.18" max.
@ 70.8 psf (negative)	0.05"	0.18" max.

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE NO. 19334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:nlb
01-41134.01

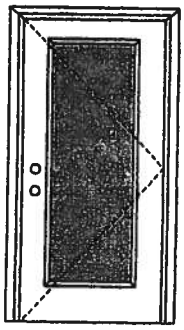
ALLEN N. REEVES
CERTIFICATE NO. 1[illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


160 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


105, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR® Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH

Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR® Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

THIS INSTRUMENT PREPARED BY
AND RETURN TO:
TITLE OFFICES, LLC
1089 SW MAIN BLVD.
LAKE CITY, FLORIDA 32025

24466

Parcel I.D. #: ***08914-002***

06-04124

SPACE ABOVE THIS LINE FOR PROCESSING DATA *SPACE ABOVE THIS LINE FOR RECORDING DATA*

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713.13, Florida Statutes, the following information is provided in this Notice of Commencement. This Notice shall be void and of no force and effect if construction is not commenced within ninety (90) days after recordation.

1. Description of property: (Legal description of property, and street address if available)

1381 SW BEDENBAUGH LANE, LAKE CITY, FLORIDA 32025
TOWNSHIP 4 SOUTH, RANGE 17 EAST

SECTION 30: THE W ½ OF THE E ½ OF THE FOLLOWING DESCRIBED REAL PROPERTY: 449.40 FEET EAST AND WEST BY 194.70 FEET NORTH AND SOUTH IN THE SOUTHEAST CORNER OF SE ¼ OF SE ¼, PAGE 672 OF 1976 TAX ROLL; MORE PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCE AT THE SE CORNER OF SECTION 30, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA AND RUN N 2°54'37" W ALONG THE EAST LINE OF SAID SECTION 30, A DISTANCE OF 219.49 FEET; THENCE S 87°58'49" W 53.40 FEET TO THE POINT OF BEGINNING; THENCE S 1°20'05" E 194.70 FEET; THENCE S 87°58'49" W 449.40 FEET; THENCE N 1°20'05" W 194.70 FEET; THENCE N 87°58'49" E 449.40 FEET TO THE POINT OF BEGINNING.

2. General description of improvement: **construction of single family dwelling**

3. Owner information:
   a. Name and address:
      ELMER M. NICHOLS, JR. and RITA J. NICHOLS
      439 SE LILLIAN LOOP, APT. 104, LAKE CITY, FLORIDA 32025
   b. Interest in property: **Fee Simple**
   c. Name and Address of Fee Simple Titleholder (if other than owner):

4. Contractor: (Name and Address)
   NORTON HOME IMPROVEMENT CO., INC.
   3367 S. US HWY. 441, SUITE 101, LAKE CITY, FLORIDA 32025
   Telephone Number: **(386) 752-3331**

Inst:200600S982 Date:04/25/2006 Time:15:04
J.F. DC,P.DeWitt Cason,Columbia County B:1081 P:1593

5. Surety (if any):
   a. Name and Address:
      Telephone Number: _____
   b. Amount of Bond $_____

6. Lender: (Name and Address)
   FIRST FEDERAL SAVINGS BANK OF FLORIDA
   4705 WEST U.S. HWY 90, P.O. BOX 2029, LAKE CITY, FL 32056
   Telephone Number: **755-0600**

7. Persons within the State of Florida designated by Owner upon whom notice or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: (Name and Address)
   N/A

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: (Name and Address) **PAULA HACKER**
   FIRST FEDERAL SAVINGS BANK OF FLORIDA
   4705 WEST U.S. HWY 90, P.O. BOX 2029, LAKE CITY, FL 32056
   Telephone Number: **755-0600**

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

_____ {SEAL}
ELMER M. NICHOLS, JR.

_____ {SEAL}
RITA J. NICHOLS

Sworn to and subscribed before me this 21st day of April, 2006, by ELMER M. NICHOLS, JR. and RITA J. NICHOLS, who are personally known to me or who have produced

_____ as identification.

Notary Public
My Commission Expires: _____

Martha Bryan
MY COMMISSION # DD232534 EXPIRES
August 10, 2007
TROY FAIN INSURANCE, INC.

Inst:2006009992 Date:04/25/2006 Time:15:04
DC,P.DeWitt Cason,Columbia County B:1081 P:1594

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 30-4S-17-08914-002 **Building permit No.** 000024466

Use Classification SFD,UTILITY **Fire:** 18.17

Permit Holder JAMES NORTON **Waste:** 0.00

Owner of Building ELMER & RITA NICHOLS **Total:** 18.17

Location: 1381 SW BEDENBAUGH LANE

Date: 09/12/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L156415		
Builder:	Norton Home	Date:	4/6/2006
Lot:	N/A	Start Number:	**1723**
Subdivision:	1381 SW Bedenbaugh Lane		
County or City:	Columbia County		
Truss Page Count:	13		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORTON, JAMES H. RB 0031780
Address: RT 28 BOX 388A
HIGH SPRINGS, FL. 32643 Designer: **101**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	T01G	0406061723	4/6/2006				
2	T02	0406061724	4/6/2006				
3	T03	0406061725	4/6/2006				
4	T04	0406061726	4/6/2006				
5	T05G	0406061727	4/6/2006				
6	T06	0406061728	4/6/2006				
7	T07G	0406061729	4/6/2006				
8	T08	0406061730	4/6/2006				
9	T09G	0406061731	4/6/2006				
10	T10	0406061732	4/6/2006				
11	T11	0406061733	4/6/2006				
12	T12	0406061734	4/6/2006				
13	T01G	0406061735	4/6/2006				

APR 06 2006

01:57:03 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**NORTON, JAMES H** (Primary Name) **NORTON HOME IMPROVEMENT COMPANY INC** (DBA Name)
Main Address:	**RT 28 BOX 388A LAKE CITY, Florida 32025**
Lic. Location:	**RT 28 BOX 388A LAKE CITY, FL 32025 Columbia**

License Information

License Type:	**Registered Building Contractor**
Rank:	**Reg Building**
License Number:	**RB0031780**
Status:	**Current, Active**
Licensure Date:	**02/16/1978**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	02/20/2004

View Related License Information
View License Complaint

New Search Back

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061723

Job	Truss	Truss Type	Qty	Ply	
L156415	T01G	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:06 2006 Page 1

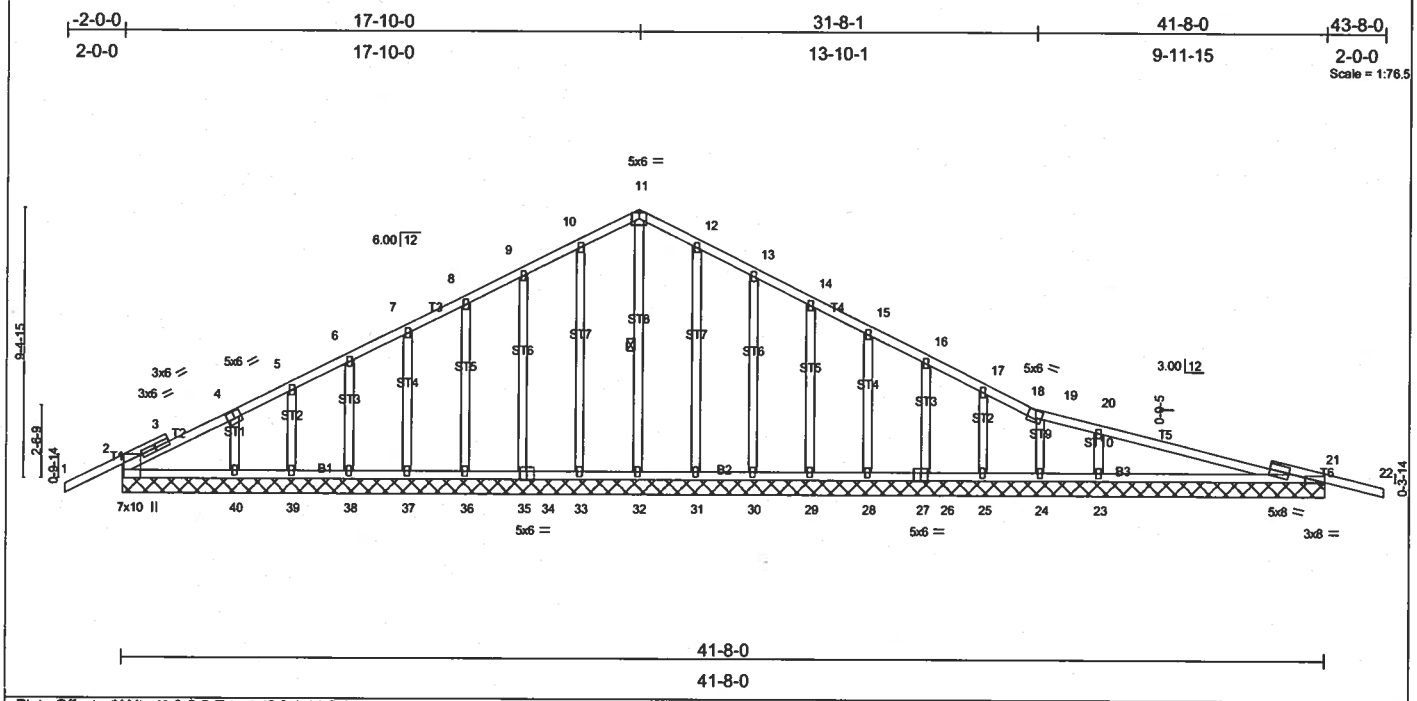


Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-1-11,0-1-8], [4:0-3-0,0-3-0], [18:0-3-0,0-1-9], [21:0-5-0,Edge], [21:0-1-12,0-2-8], [27:0-3-0,0-0-4], [34:0-3-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.13	22	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	0.22	22	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.01	21	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 257 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T5 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 11-32

REACTIONS (lb/size) 21=722/41-8-0, 2=550/41-8-0, 32=265/41-8-0, 33=287/41-8-0, 35=288/41-8-0, 36=288/41-8-0, 37=285/41-8-0, 38=299/41-8-0, 39=244/41-8-0, 40=421/41-8-0, 31=287/41-8-0, 30=289/41-8-0, 29=286/41-8-0, 28=296/41-8-0, 26=255/41-8-0, 25=429/41-8-0, 24=-293/41-8-0, 23=1257/41-8-0
Max Horz 2=147(load case 5)
Max Uplift21=-353(load case 4), 2=-215(load case 5), 33=-118(load case 5), 35=-137(load case 5), 36=-131(load case 5), 37=-131(load case 5), 38=-135(load case 5), 39=-120(load case 5), 40=-178(load case 5), 31=-113(load case 6), 30=-139(load case 6), 29=-130(load case 6), 28=-134(load case 6), 26=-122(load case 6), 25=-177(load case 6), 24=-294(load case 10), 23=-463(load case 4)
Max Grav 21=722(load case 1), 2=550(load case 1), 32=265(load case 1), 33=291(load case 9), 35=288(load case 1), 36=289(load case 9), 37=285(load case 1), 38=299(load case 9), 39=244(load case 1), 40=422(load case 9), 31=291(load case 10), 30=289(load case 1), 29=286(load case 10), 28=296(load case 1), 26=255(load case 10), 25=429(load case 1), 24=102(load case 4), 23=1257(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/54, 2-3=-167/76, 3-4=-153/72, 4-5=-91/102, 5-6=-75/131, 6-7=-78/168, 7-8=-78/204, 8-9=-78/242, 9-10=-78/303, 10-11=-78/349, 11-12=-78/349, 12-13=-78/303, 13-14=-78/242, 14-15=-78/186, 15-16=-76/128, 16-17=-85/94, 17-18=-33/62, 18-19=-32/59, 19-20=-132/87, 20-21=-112/82, 21-22=-8/53
BOT CHORD 2-40=0/158, 39-40=0/165, 38-39=0/165, 37-38=0/165, 36-37=0/165, 35-36=0/165, 34-35=0/165, 33-34=0/165, 32-33=0/165, 31-32=0/165, 30-31=0/165, 29-30=0/165, 28-29=0/165, 27-28=0/165, 26-27=0/165, 25-26=0/165, 24-25=0/165, 23-24=0/165, 21-23=0/165
WEBS 11-32=-205/0, 10-33=-231/139, 9-35=-228/171, 8-36=-228/161, 7-37=-227/162, 6-38=-234/166, 5-39=-201/151, 4-40=-320/210, 12-31=-231/139, 13-30=-229/171, 14-29=-227/161, 15-28=-232/165, 16-26=-208/152, 17-25=-320/212, 19-24=-83/172, 20-23=-922/526

JOINT STRESS INDEX
2 = 0.71, 3 = 0.00, 3 = 0.34, 3 = 0.34, 4 = 0.32, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.18, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.36, 19 = 0.00, 20 = 0.34, 21 = 0.92, 21 = 0.13, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.00, 27 = 0.30, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.30, 35 = 0.00, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34 and 40 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 21 and 21.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 353 lb uplift at joint 21, 215 lb uplift at joint 2, 118 lb uplift at joint 33, 137 lb uplift at joint 35, 131 lb uplift at joint 36, 131 lb uplift at joint 37, 135 lb uplift at joint 38, 120 lb uplift at joint 39, 178 lb uplift at joint 40, 113 lb uplift at joint 31, 139 lb uplift at joint 30, 130 lb uplift at joint 29, 134 lb uplift at joint 28, 122 lb uplift at joint 26, 177 lb uplift at joint 25, 294 lb uplift at joint 24 and 463 lb uplift at joint 23.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

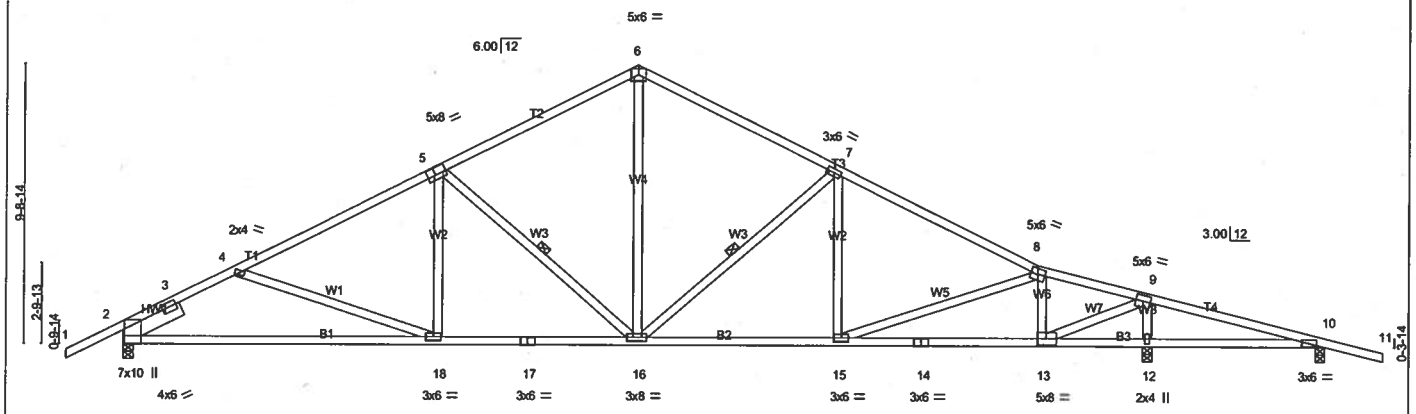
Dwg.#0406061724

Job	Truss	Truss Type	Qty	Ply	
L156415	T02	SPECIAL	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:07 2006 Page 1

-2-0-0	3-11-14	10-10-15	17-10-0	24-9-1	31-8-1	35-6-0	41-8-0	43-8-0
2-0-0	3-11-14	6-11-1	6-11-1	6-11-0	6-11-0	3-9-15	6-2-0	2-0-0

Scale = 1:76.5



10-10-15	17-10-0	24-9-1	31-8-1	35-6-0	41-8-0
10-10-15	6-11-1	6-11-0	6-11-0	3-9-15	6-2-0

Plate Offsets (X,Y): [2:0-5-15,Edge], [5:0-3-8,0-3-0], [13:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.27	2-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.46	2-18	>929	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.09	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-2-10

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-16, 7-16

REACTIONS (lb/size) 2=1552/0-4-0, 12=2063/0-4-0, 10=95/0-4-0
Max Horz 2=152(load case 5)
Max Uplift2=-584(load case 5), 12=-740(load case 6), 10=-279(load case 4)
Max Grav 2=1552(load case 1), 12=2063(load case 1), 10=144(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-2323/1052, 3-4=-2274/1053, 4-5=-2158/963, 5-6=-1582/832, 6-7=-1578/833, 7-8=-1999/912, 8-9=-1475/658, 9-10=-401/1014, 10-11=0/25
BOT CHORD 2-18=-769/1959, 17-18=-609/1875, 16-17=-609/1875, 15-16=-556/1720, 14-15=-522/1455, 13-14=-522/1455, 12-13=-939/460, 10-12=-939/460
WEBS 4-18=-140/176, 5-18=0/315, 5-16=-730/400, 6-16=-442/966, 7-16=-567/336, 7-15=0/174, 8-15=-65/282, 8-13=-871/481, 9-13=-1037/2547, 9-12=-1878/866

JOINT STRESS INDEX
2 = 0.77, 2 = 0.85, 3 = 0.00, 4 = 0.34, 5 = 0.70, 6 = 0.62, 7 = 0.41, 8 = 0.70, 9 = 0.77, 10 = 0.30, 12 = 0.69, 13 = 0.57, 14 = 0.65, 15 = 0.35, 16 = 0.57, 17 = 0.67 and 18 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 584 lb uplift at joint 2, 740 lb uplift at joint 12 and 279 lb uplift at joint 10.

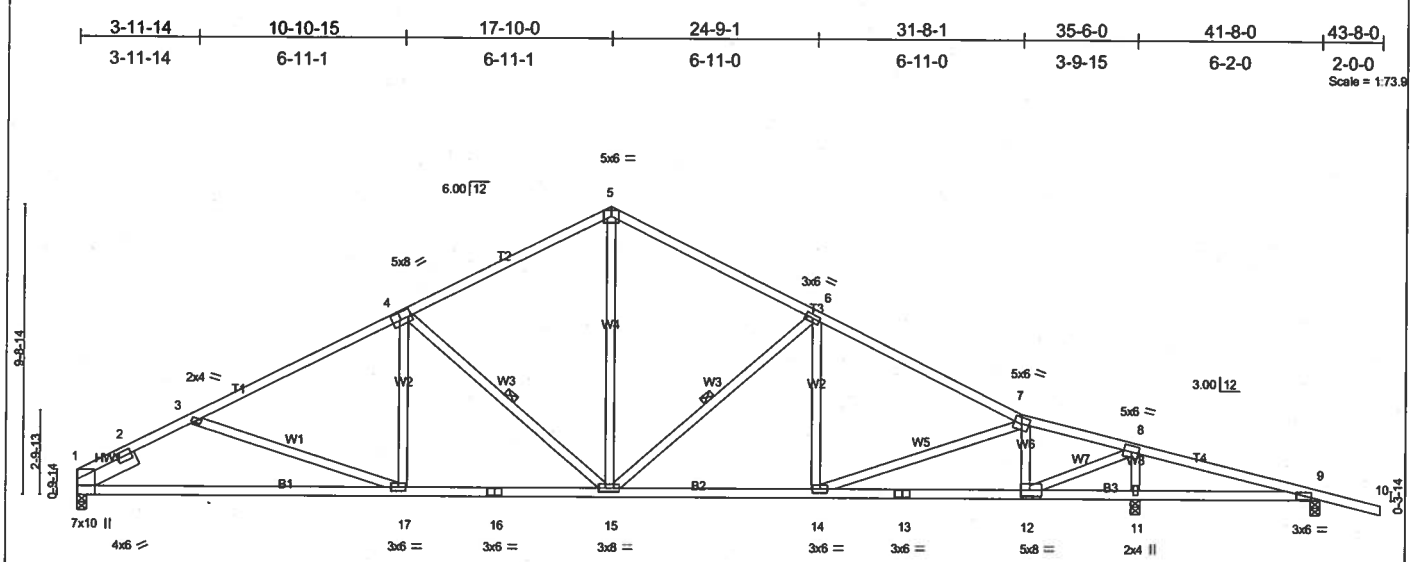
LOAD CASE(S) Standard

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061725

Job	Truss	Truss Type	Qty	Ply	
L156415	T03	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:07 2006 Page 1



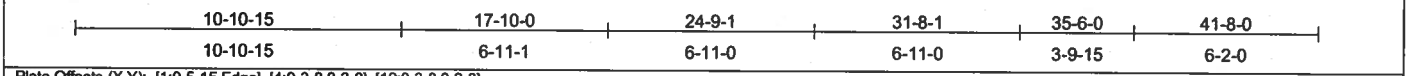


Plate Offsets (X,Y): [1:0-5-15,Edge], [4:0-3-8,0-3-0], [12:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.27	1-17	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.47	1-17	>914	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.09	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 227 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-2-10

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 6-15

REACTIONS (lb/size) 1=1441/0-4-0, 11=2067/0-4-0, 9=95/0-4-0
Max Horz 1=-163(load case 6)
Max Uplift1=-470(load case 5), 11=-655(load case 6), 9=-212(load case 4)
Max Grav 1=1441(load case 1), 11=2067(load case 1), 9=144(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2355/1102, 2-3=-2311/1111, 3-4=-2173/988, 4-5=-1588/846, 5-6=-1584/847, 6-7=-2005/931, 7-8=-1479/702, 8-9=-417/1015, 9-10=0/25
BOT CHORD 1-17=-834/2001, 16-17=-632/1888, 15-16=-632/1888, 14-15=-572/1726, 13-14=-566/1459, 12-13=-566/1459, 11-12=-940/480, 9-11=-940/480
WEBS 3-17=-171/213, 4-17=-7/326, 4-15=-739/413, 5-15=-454/971, 6-15=-567/341, 6-14=0/173, 7-14=-48/284, 7-12=-873/476, 8-12=-1107/2553, 8-11=-1881/887

JOINT STRESS INDEX
1 = 0.75, 1 = 0.85, 2 = 0.00, 3 = 0.34, 4 = 0.69, 5 = 0.63, 6 = 0.41, 7 = 0.72, 8 = 0.77, 9 = 0.28, 11 = 0.69, 12 = 0.57, 13 = 0.65, 14 = 0.35, 15 = 0.57, 16 = 0.67 and 17 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 470 lb uplift at joint 1, 655 lb uplift at joint 11 and 212 lb uplift at joint 9.

LOAD CASE(S) Standard

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061726

Job	Truss	Truss Type	Qty	Ply	
L156415	T04	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:08 2006 Page 1

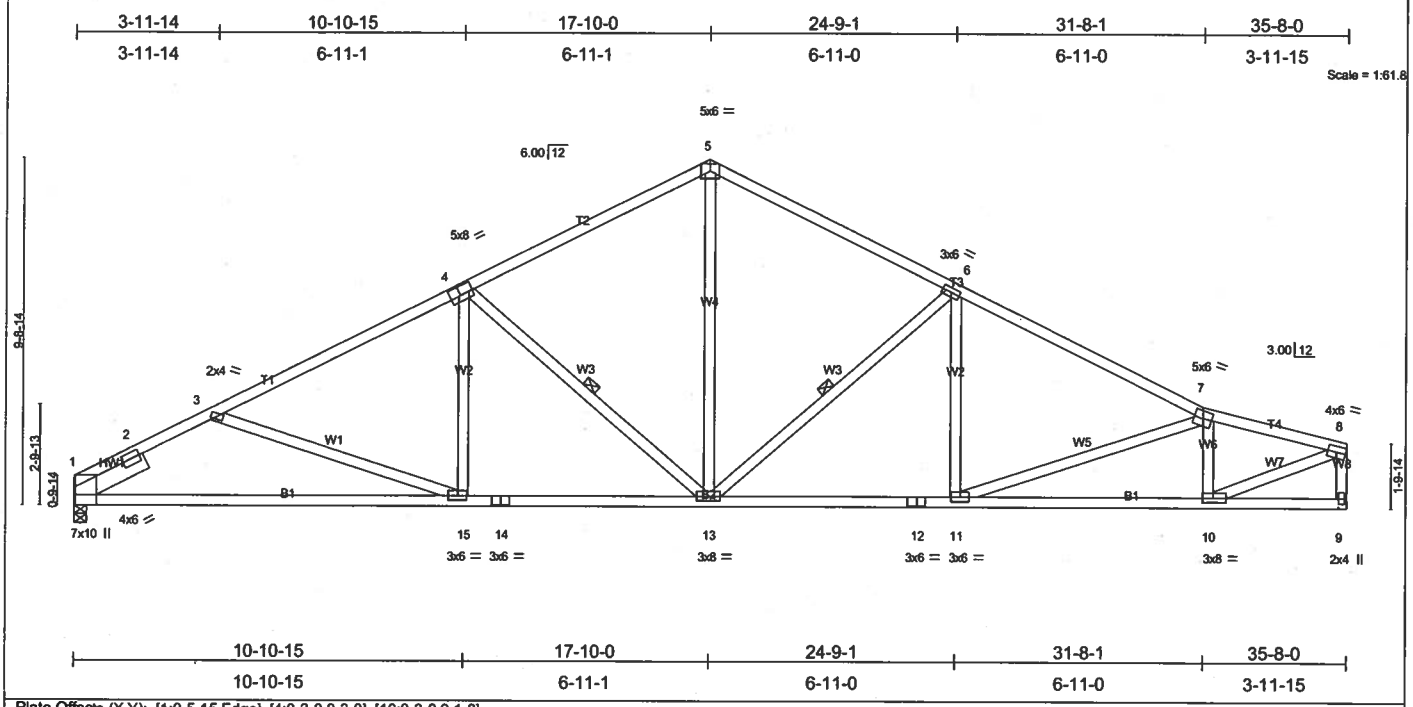


Plate Offsets (X,Y): [1:0-5-15,Edge], [4:0-3-8,0-3-0], [10:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.28	1-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.47	1-15	>899	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.11	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 206 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-2-10

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-5 oc bracing.
WEBS 1 Row at midpt 4-13, 6-13

REACTIONS (lb/size) 1=1492/0-4-0, 9=1492/Mechanical
Max Horz 1=182(load case 5)
Max Uplift1=-483(load case 5), 9=-485(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2448/1137, 2-3=-2403/1145, 3-4=-2276/1027, 4-5=-1695/886, 5-6=-1691/887, 6-7=-2240/1020, 7-8=-2148/950, 8-9=-1438/646
BOT CHORD 1-15=-980/2079, 14-15=-785/1980, 13-14=-785/1980, 12-13=-769/1935, 11-12=-769/1935, 10-11=-926/2106, 9-10=-32/68
WEBS 3-15=-161/207, 4-15=-6/322, 4-13=-736/411, 5-13=-489/1066, 6-13=-691/397, 6-11=-15/305, 7-11=-230/170, 7-10=-691/419, 8-10=-951/2168

JOINT STRESS INDEX
1 = 0.76, 1 = 0.89, 2 = 0.00, 3 = 0.34, 4 = 0.70, 5 = 0.63, 6 = 0.41, 7 = 0.77, 8 = 0.82, 9 = 0.69, 10 = 0.86, 11 = 0.35, 12 = 0.70, 13 = 0.57, 14 = 0.84 and 15 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 483 lb uplift at joint 1 and 485 lb uplift at joint 9.

LOAD CASE(S) Standard

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061727

Job	Truss	Truss Type	Qty	Ply	
L156415	T05G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:09 2006 Page 1

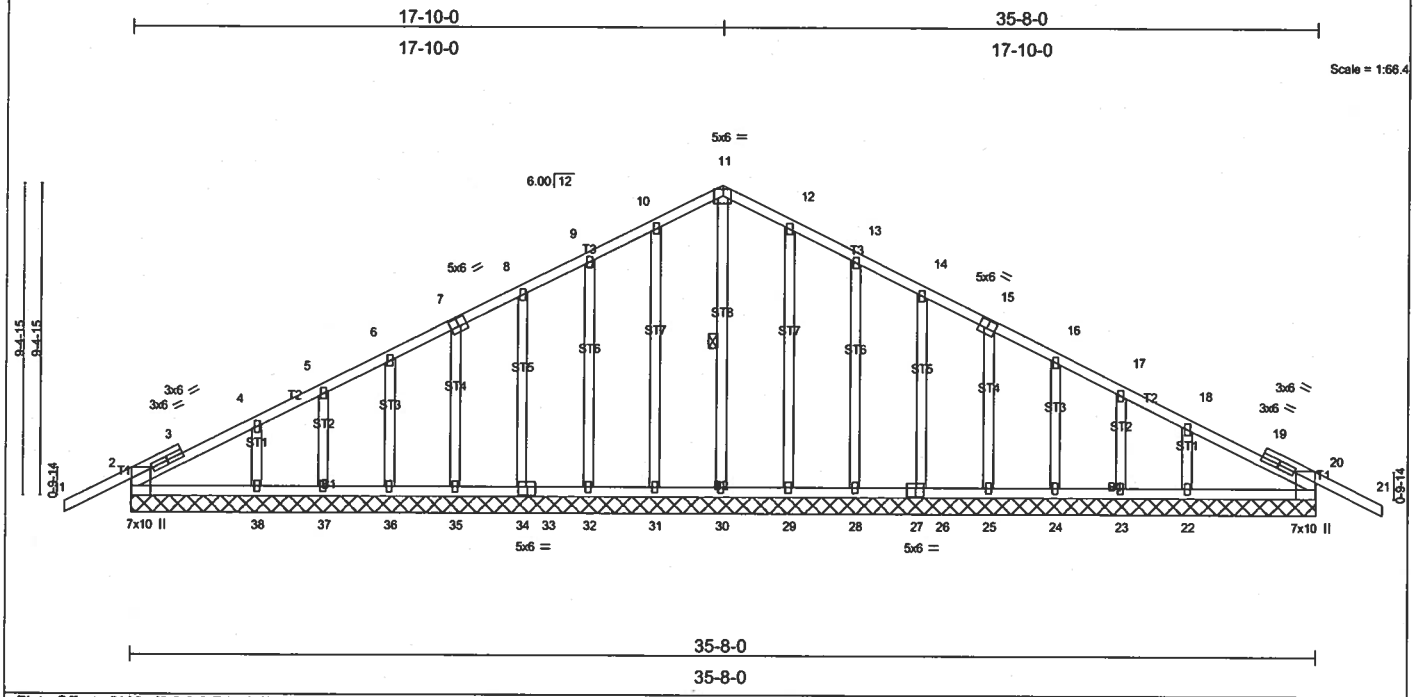


Plate Offsets (X,Y): [2:0-3-8,Edge], [3:0-1-11,0-1-8], [7:0-3-0,0-3-0], [15:0-3-0,0-3-0], [19:0-1-11,0-1-8], [20:0-3-8,Edge], [27:0-3-0,0-0-4], [33:0-3-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	21	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.05	21	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.01	20	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 239 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 11-30

REACTIONS (lb/size) 2=557/35-8-0, 30=246/35-8-0, 31=286/35-8-0, 32=289/35-8-0, 34=287/35-8-0, 35=285/35-8-0, 36=302/35-8-0, 37=235/35-8-0, 38=431/35-8-0, 29=286/35-8-0, 28=289/35-8-0, 26=287/35-8-0, 25=285/35-8-0, 24=302/35-8-0, 23=235/35-8-0, 22=431/35-8-0, 20=557/35-8-0
Max Horz 2=-147(load case 6)
Max Uplift2=-214(load case 5), 31=-118(load case 5), 32=-138(load case 5), 34=-130(load case 5), 35=-131(load case 5), 36=-137(load case 5), 37=-113(load case 5), 38=-185(load case 5), 29=-114(load case 6), 28=-139(load case 6), 26=-130(load case 6), 25=-131(load case 6), 24=-136(load case 6), 23=-115(load case 6), 22=-180(load case 6), 20=-245(load case 6)
Max Grav 2=557(load case 1), 30=246(load case 1), 31=290(load case 9), 32=289(load case 1), 34=287(load case 9), 35=285(load case 1), 36=302(load case 9), 37=235(load case 1), 38=432(load case 9), 29=290(load case 10), 28=289(load case 1), 26=287(load case 10), 25=285(load case 1), 24=302(load case 10), 23=235(load case 1), 22=432(load case 10), 20=557(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/54, 2-3=-181/66, 3-4=-157/64, 4-5=-113/96, 5-6=-97/124, 6-7=-101/162, 7-8=-101/198, 8-9=-101/237, 9-10=-102/297, 10-11=-100/344, 11-12=-100/344, 12-13=-102/297, 13-14=-101/237, 14-15=-101/180, 15-16=-102/123, 16-17=-97/64, 17-18=-113/35, 18-19=-94/9, 19-20=-181/12, 20-21=-8/54
BOT CHORD 2-38=0/171, 37-38=0/171, 36-37=0/171, 35-36=0/171, 34-35=0/171, 33-34=0/171, 32-33=0/171, 31-32=0/171, 30-31=0/171, 29-30=0/171, 28-29=0/171, 27-28=0/171, 26-27=0/171, 25-26=0/171, 24-25=0/171, 23-24=0/171, 22-23=0/171, 20-22=0/171
WEBS 11-30=-186/0, 10-31=-230/139, 9-32=-229/171, 8-34=-227/160, 7-35=-227/162, 6-36=-238/168, 5-37=-192/144, 4-38=-329/217, 12-29=-230/139, 13-28=-229/171, 14-26=-227/161, 15-25=-227/162, 16-24=-237/168, 17-23=-192/144, 18-22=-329/217

JOINT STRESS INDEX
2 = 0.71, 3 = 0.00, 3 = 0.35, 3 = 0.35, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.21, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.18, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.21, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.00, 19 = 0.35, 19 = 0.35, 20 = 0.71, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.00, 27 = 0.30, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.30, 34 = 0.00, 35 = 0.34, 36 = 0.34, 37 = 0.34 and 38 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 2, 118 lb uplift at joint 31, 138 lb uplift at joint 32, 130 lb uplift at joint 34, 131 lb uplift at joint 35, 137 lb uplift at joint 36, 113 lb uplift at joint 37, 185 lb uplift at joint 38, 114 lb uplift at joint 29, 139 lb uplift at joint 28, 130 lb uplift at joint 26, 131 lb uplift at joint 25, 136 lb uplift at joint 24, 115 lb uplift at joint 23, 180 lb uplift at joint 22 and 245 lb uplift at joint 20.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-11=-114(F=-60), 11-21=-114(F=-60), 2-20=-30

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

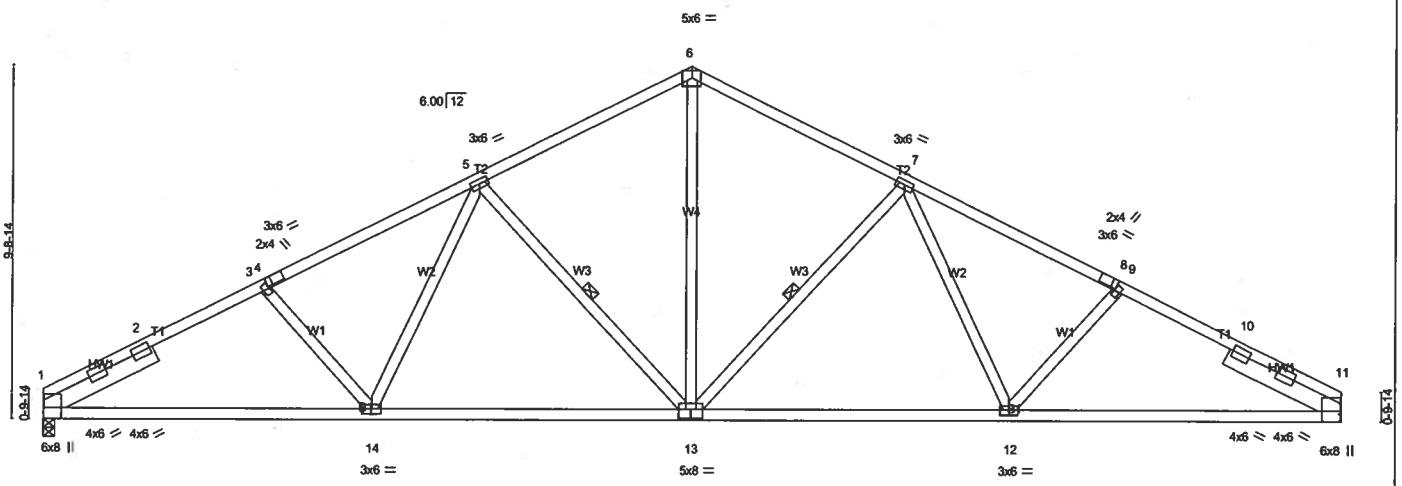
Dwg.#0406061728

Job	Truss	Truss Type	Qty	Ply	
L156415	T06	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:10 2006 Page 1

6-1-11	11-11-14	17-10-0	23-8-3	29-6-5	35-8-0
6-1-11	5-10-3	5-10-3	5-10-3	5-10-3	6-1-11

Scale = 1:60.6



9-0-12	17-10-0	26-7-4	35-8-0
9-0-12	8-9-4	8-9-4	9-0-12

Plate Offsets (X,Y): [1:0-5-15,Edge], [4:0-1-12,Edge], [7:0-0-0,0-0-0], [8:0-1-12,Edge], [9:0-0-0,0-0-0], [11:0-5-15,Edge], [11:0-0-0,0-0-0], [11:0-0-0,0-0-0], [13:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.23	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.37	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.12	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-5-6, Right 2 X 6 SYP No.1D 3-5-6

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-9-10 oc bracing.
WEBS 1 Row at midpt 5-13, 7-13

REACTIONS (lb/size) 1=1498/0-4-0, 11=1498/Mechanical
Max Horz 1=-137(load case 3)
Max Uplift1=-486(load case 5), 11=-486(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2510/1111, 2-3=-2425/1124, 3-4=-2314/1067, 4-5=-2300/1089, 5-6=-1698/900, 6-7=-1698/900, 7-8=-2299/1089, 8-9=-2313/1066, 9-10=-2425/1123, 10-11=-2509/1110
BOT CHORD 1-14=-867/2114, 13-14=-658/1848, 12-13=-658/1848, 11-12=-866/2113
WEBS 3-14=-177/236, 5-14=-134/433, 5-13=-617/404, 6-13=-551/1148, 7-13=-616/404, 7-12=-134/433, 9-12=-177/236

JOINT STRESS INDEX
1 = 0.77, 1 = 0.46, 1 = 0.46, 2 = 0.00, 3 = 0.34, 4 = 0.63, 5 = 0.41, 6 = 0.46, 7 = 0.41, 8 = 0.63, 9 = 0.34, 10 = 0.00, 11 = 0.77, 11 = 0.46, 11 = 0.46, 12 = 0.47, 13 = 0.65 and 14 = 0.47

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 486 lb uplift at joint 1 and 486 lb uplift at joint 11.

LOAD CASE(S) Standard

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061729

Job	Truss	Truss Type	Qty	Ply	
L156415	T07G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:11 2006 Page 1

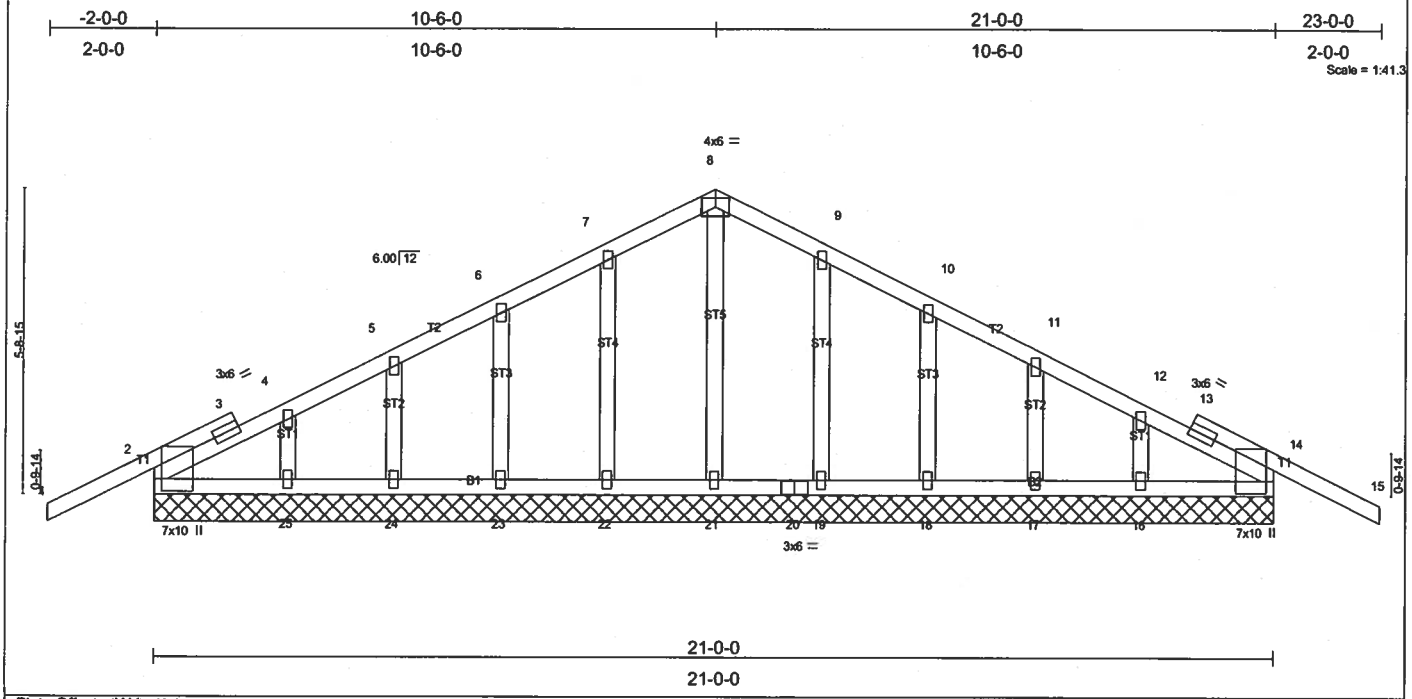


Plate Offsets (X,Y): [2:0-2-12,0-1-12], [14:0-2-12,0-1-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.04	15	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.06	15	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.11	Horz(TL)	0.00	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 118 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=501/21-0-0, 14=501/21-0-0, 21=265/21-0-0, 22=289/21-0-0, 23=285/21-0-0, 24=303/21-0-0, 25=231/21-0-0, 19=289/21-0-0, 18=285/21-0-0, 17=303/21-0-0, 16=231/21-0-0
Max Horz 2=95(load case 5)
Max Uplift2=-242(load case 5), 14=-264(load case 6), 22=-129(load case 5), 23=-130(load case 5), 24=-145(load case 5), 25=-81(load case 5), 19=-127(load case 6), 18=-131(load case 6), 17=-146(load case 6), 16=-78(load case 5)
Max Grav 2=501(load case 1), 14=501(load case 1), 21=265(load case 1), 22=293(load case 9), 23=285(load case 1), 24=303(load case 9), 25=231(load case 9), 19=293(load case 10), 18=285(load case 1), 17=303(load case 10), 16=231(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/54, 2-3=-133/49, 3-4=-77/47, 4-5=-73/71, 5-6=-77/108, 6-7=-76/147, 7-8=-76/200, 8-9=-76/200, 9-10=-76/147, 10-11=-77/90, 11-12=-73/34, 12-13=-36/14, 13-14=-133/16, 14-15=-8/54
BOT CHORD 2-25=0/116, 24-25=0/116, 23-24=0/116, 22-23=0/116, 21-22=0/116, 20-21=0/116, 19-20=0/116, 18-19=0/116, 17-18=0/116, 16-17=0/116, 14-16=0/116
WEBS 8-21=-205/0, 7-22=-233/154, 6-23=-226/164, 5-24=-238/173, 4-25=-184/114, 9-19=-233/154, 10-18=-226/164, 11-17=-238/173, 12-16=-184/114

JOINT STRESS INDEX
2 = 0.70, 3 = 0.00, 3 = 0.00, 3 = 0.29, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.25, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.00, 13 = 0.29, 13 = 0.00, 14 = 0.70, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.15, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34 and 25 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 242 lb uplift at joint 2, 264 lb uplift at joint 14, 129 lb uplift at joint 22, 130 lb uplift at joint 23, 145 lb uplift at joint 24, 81 lb uplift at joint 25, 127 lb uplift at joint 19, 131 lb uplift at joint 18, 146 lb uplift at joint 17 and 78 lb uplift at joint 16.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-8=-114(F=-60), 8-15=-114(F=-60), 2-14=-30

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061730

Job	Truss	Truss Type	Qty	Ply	
L156415	T08	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:12 2006 Page 1

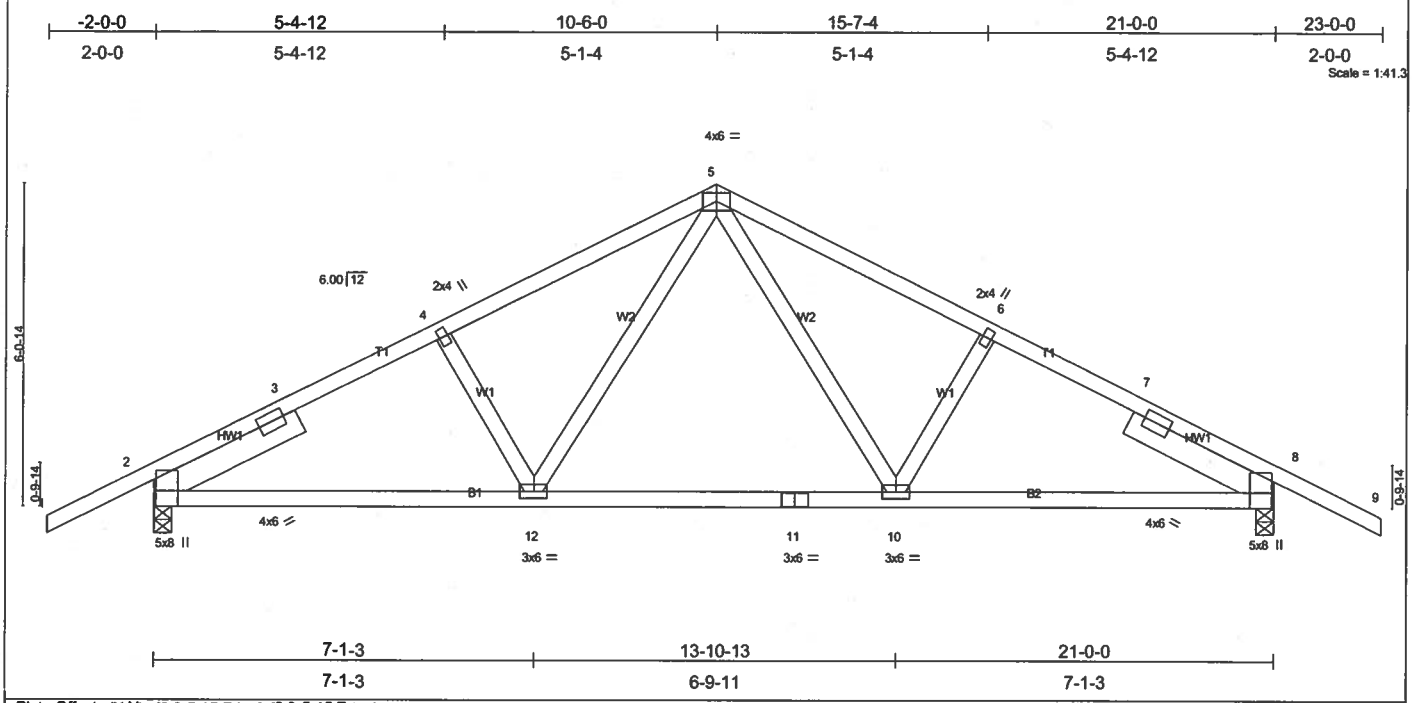


Plate Offsets (X,Y): [2:0-5-15,Edge], [8:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.15	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.12	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 117 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-0-7, Right 2 X 6 SYP No.1D 3-0-7

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-11 oc bracing.

REACTIONS (lb/size) 2=990/0-4-0, 8=990/0-4-0
Max Horz 2=100(load case 5)
Max Uplift2=-665(load case 5), 8=-665(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-1311/1336, 3-4=-1244/1350, 4-5=-1170/1361, 5-6=-1170/1361, 6-7=-1244/1350, 7-8=-1311/1336, 8-9=0/26
BOT CHORD 2-12=-1031/1075, 11-12=-670/802, 10-11=-670/802, 8-10=-1031/1075
WEBS 4-12=-185/178, 5-12=-552/424, 5-10=-552/424, 6-10=-185/179

JOINT STRESS INDEX
2 = 0.64, 2 = 0.49, 3 = 0.00, 4 = 0.34, 5 = 0.72, 6 = 0.34, 7 = 0.00, 8 = 0.64, 8 = 0.49, 10 = 0.44, 11 = 0.33 and 12 = 0.44

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 665 lb uplift at joint 2 and 665 lb uplift at joint 8.

LOAD CASE(S) Standard

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061731

Job	Truss	Truss Type	Qty	Ply	
L156415	T09G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:13 2006 Page 1

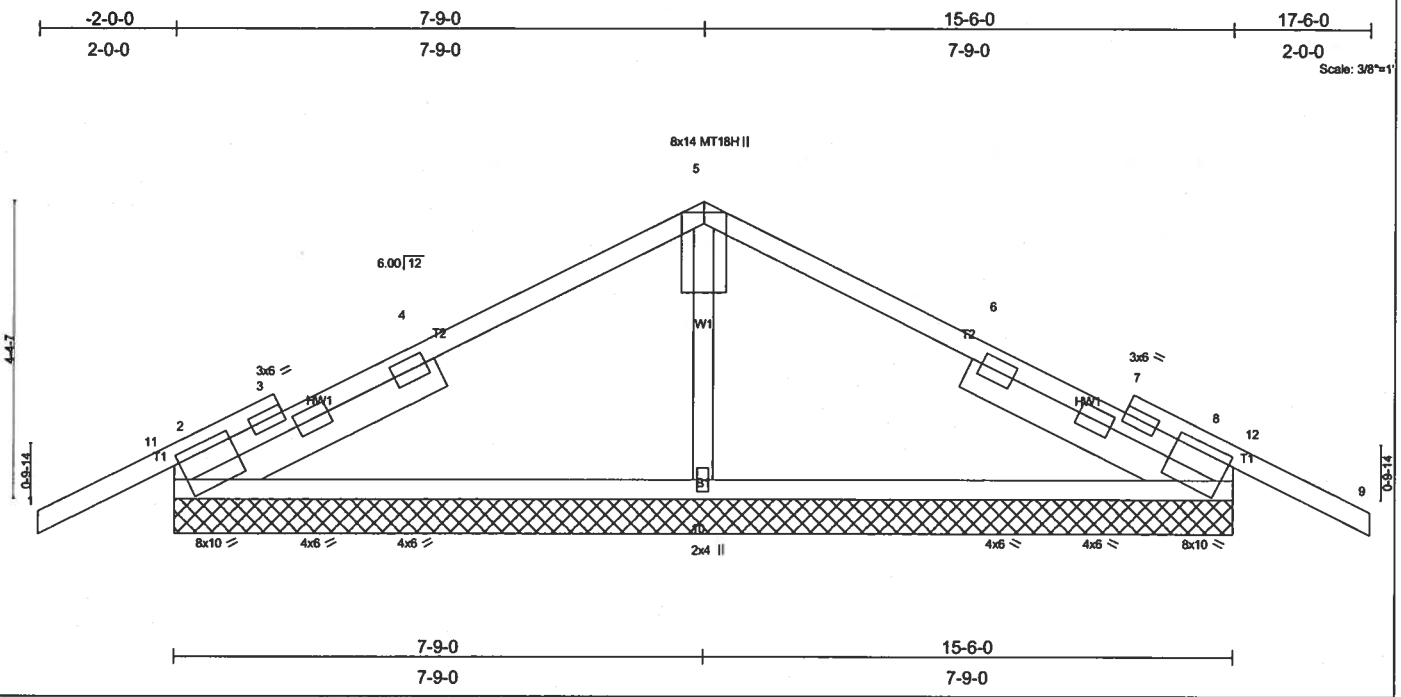

Plate Offsets (X,Y): [2:0-0-12,0-5-0], [8:0-0-12,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	0.03	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	0.04	8-9	n/r	90	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.17	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 85 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-11-11, Right 2 X 6 SYP No.1D 3-11-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=943/15-6-0, 8=943/15-6-0, 10=771/15-6-0
Max Horz 2=-85(load case 6)
Max Uplift2=-483(load case 5), 8=-499(load case 6), 10=-172(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-11=-12/72, 2-11=-15/90, 2-3=-543/316, 3-4=-379/294, 4-5=-365/288, 5-6=-365/288, 6-7=-379/294, 7-8=-543/316, 8-12=-15/90, 9-12=-12/72
BOT CHORD 2-10=-104/330, 8-10=-104/330
WEBS 5-10=-529/230

JOINT STRESS INDEX
2 = 0.92, 2 = 0.02, 2 = 0.02, 3 = 0.00, 3 = 0.10, 4 = 0.00, 5 = 0.96, 6 = 0.00, 7 = 0.00, 7 = 0.10, 8 = 0.92, 8 = 0.02, 8 = 0.02 and 10 = 0.18

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 483 lb uplift at joint 2, 499 lb uplift at joint 8 and 172 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061732

Job	Truss	Truss Type	Qty	Ply	
L156415	T10	COMMON	3	1	Job Reference (optional)

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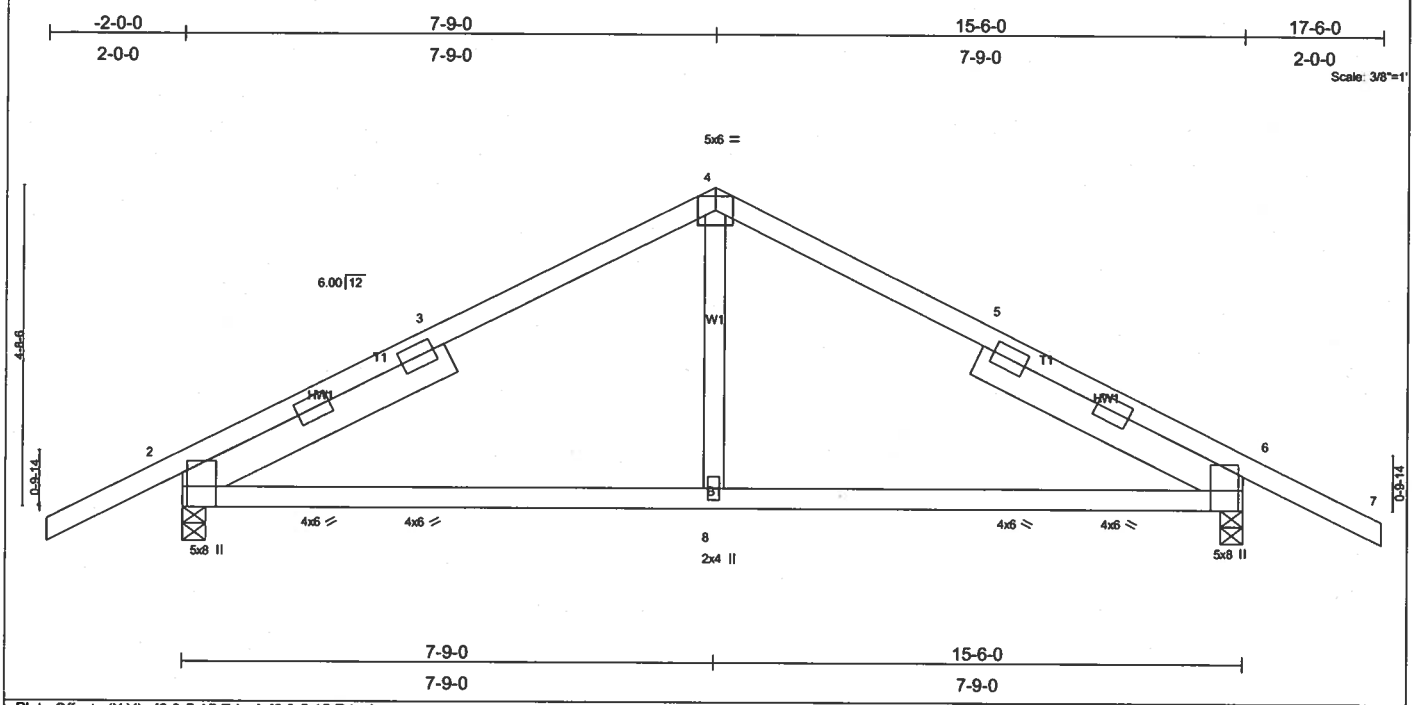

Plate Offsets (X,Y): [2:0-5-15,Edge], [6:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.09	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.13	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 82 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 4-4-2, Right 2 X 6 SYP No.1D 4-4-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=759/0-4-0, 6=759/0-4-0
Max Horz 2=81(load case 5)
Max Uplift2=-328(load case 5), 6=-328(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-813/335, 3-4=-718/352, 4-5=-718/352, 5-6=-813/335, 6-7=0/26
BOT CHORD 2-8=-129/643, 6-8=-129/643
WEBS 4-8=0/255

JOINT STRESS INDEX
2 = 0.47, 2 = 0.16, 2 = 0.16, 3 = 0.00, 4 = 0.71, 5 = 0.00, 6 = 0.47, 6 = 0.16, 6 = 0.16 and 8 = 0.19

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 2 and 328 lb uplift at joint 6.

LOAD CASE(S) Standard

APRIL 06, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061733

Job	Truss	Truss Type	Qty	Ply	
L156415	T11	COMMON	2	1	Job Reference (optional)

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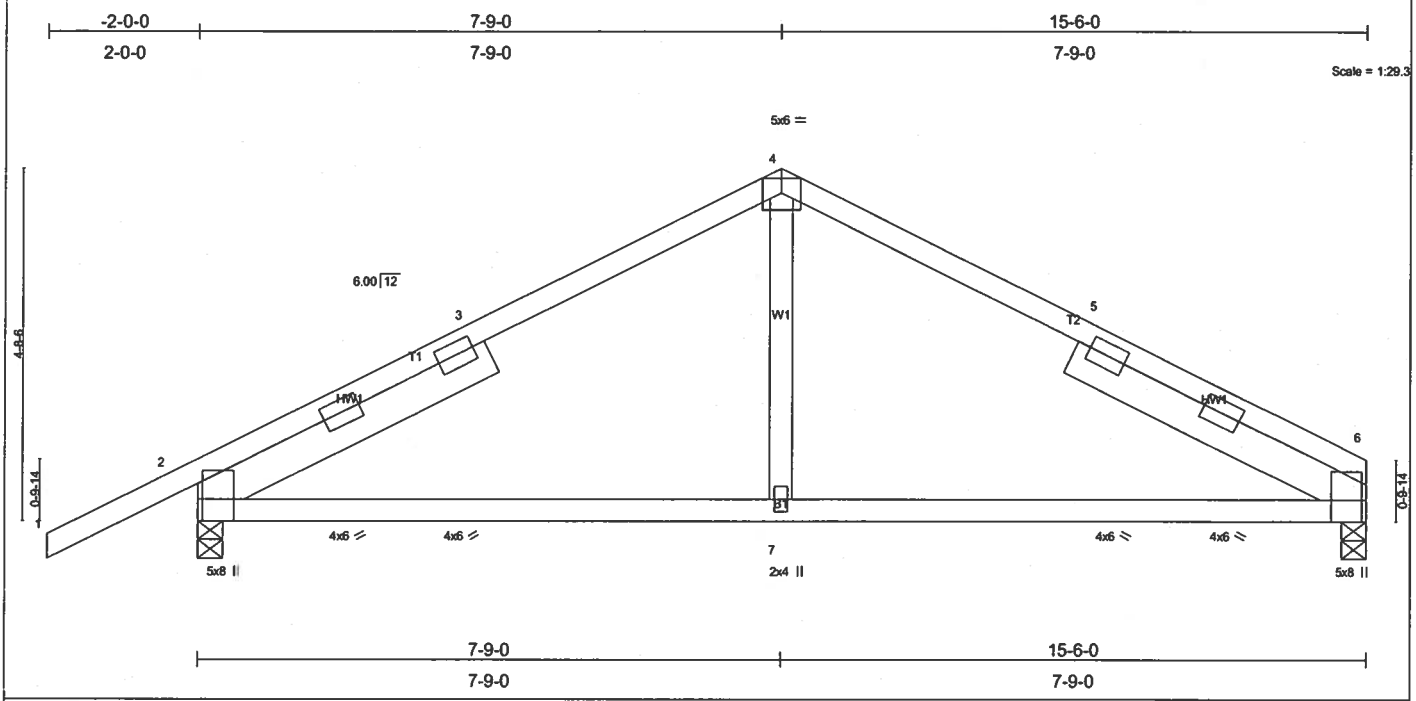

Plate Offsets (X,Y): [2:0-5-15,Edge], [6:0-5-15,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.14	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 79 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 4-4-2, Right 2 X 6 SYP No.1D 4-4-2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=644/0-4-0, 2=766/0-4-0
Max Horz 2=93(load case 5)
Max Uplift6=-208(load case 6), 2=-330(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-831/360, 3-4=-736/377, 4-5=-736/370, 5-6=-825/350
BOT CHORD 2-7=-192/659, 6-7=-192/659
WEBS 4-7=0/258

JOINT STRESS INDEX
2 = 0.47, 2 = 0.16, 2 = 0.16, 3 = 0.00, 4 = 0.75, 5 = 0.00, 6 = 0.47, 6 = 0.16, 6 = 0.16 and 7 = 0.19

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 208 lb uplift at joint 6 and 330 lb uplift at joint 2.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061734

Job	Truss	Truss Type	Qty	Ply	
L156415	T12	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Apr 06 14:29:15 2006 Page 1

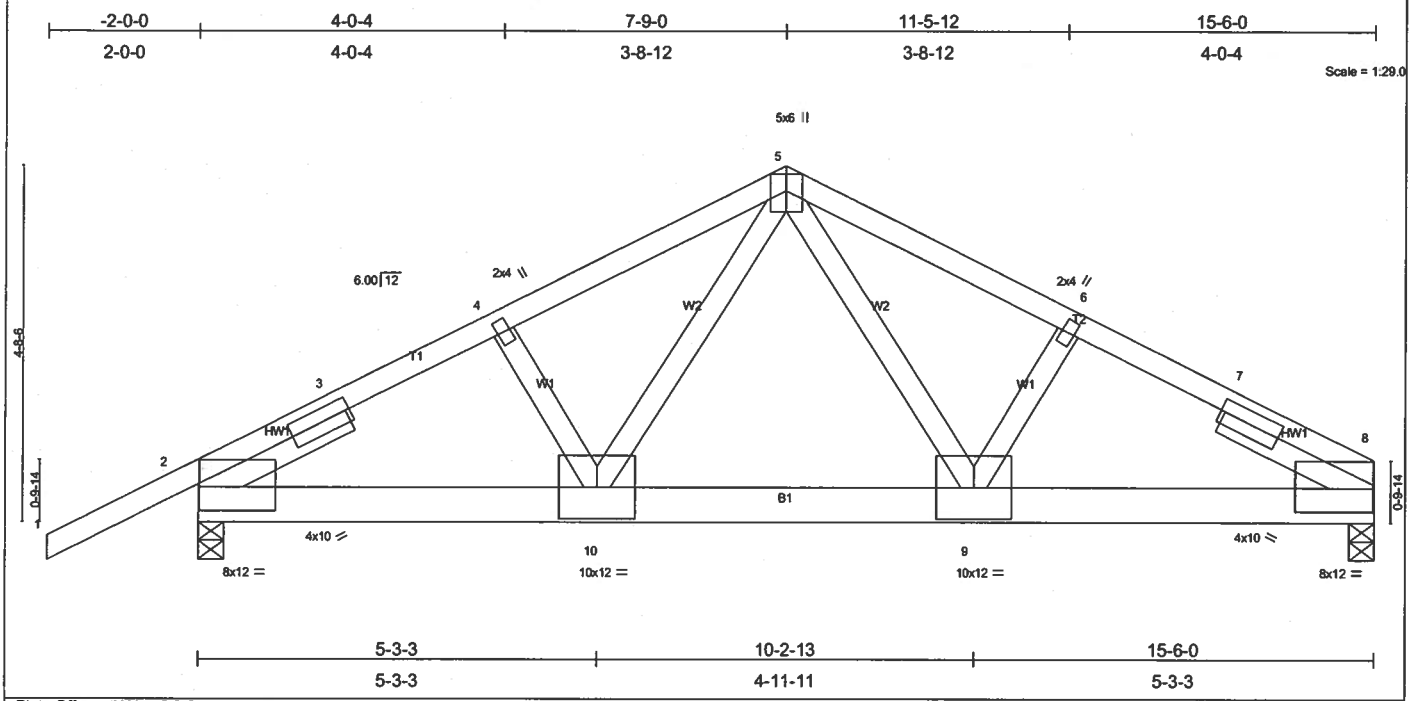

Plate Offsets (X,Y): [2:0-0-3,0-4-3], [8:0-0-3,0-4-3]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.13	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.21	9-10	>895	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.63	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 4 SYP No.3 2-2-5, Right 2 X 4 SYP No.3 2-2-5

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=6069/0-4-0, 2=6191/0-4-0
Max Horz 2=97(load case 4)
Max Uplift8=-2256(load case 5), 2=-2379(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-3=-8130/2980, 3-4=-8112/3009, 4-5=-8076/3024, 5-6=-8098/3059, 6-7=-8135/3044, 7-8=-8151/3012
BOT CHORD 2-10=-2608/7068, 9-10=-1899/5302, 8-9=-2584/7092
WEBS 4-10=-162/476, 5-10=-1453/3904, 5-9=-1492/3938, 6-9=-156/473

JOINT STRESS INDEX
2 = 0.94, 2 = 0.95, 3 = 0.00, 4 = 0.17, 5 = 0.71, 6 = 0.17, 7 = 0.00, 8 = 0.94, 8 = 0.95, 9 = 0.32 and 10 = 0.32

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2256 lb uplift at joint 8 and 2379 lb uplift at joint 2.
7) Girder carries tie-in span(s): 35-8-0 from 0-0-0 to 15-6-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-8=-730(F=-700)

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0406061735

Job	Truss	Truss Type	Qty	Ply	
L156415	T01G	SPECIAL	1	1	Job Reference (optional)

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-11=-114(F=-60), 11-18=-114(F=-60), 18-22=-114(F=-60), 2-21=-30

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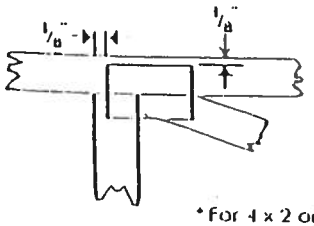
Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

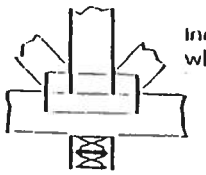
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

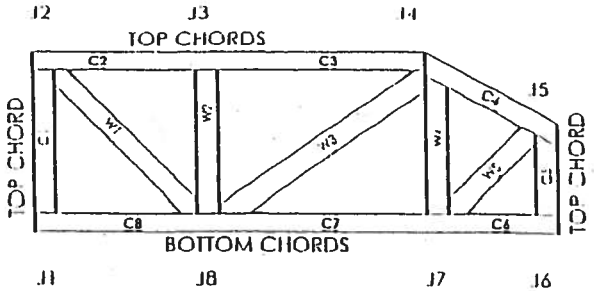
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

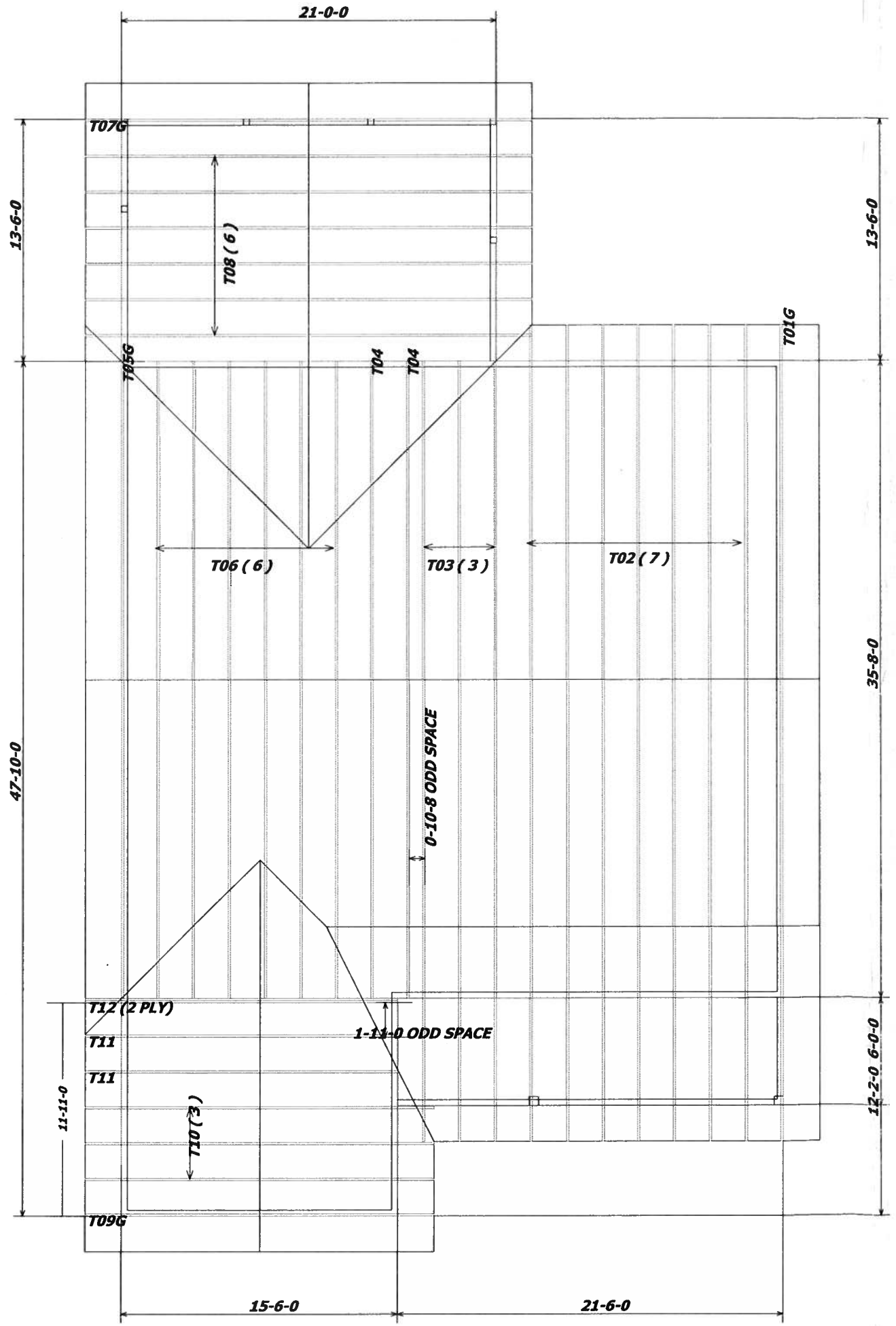
BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

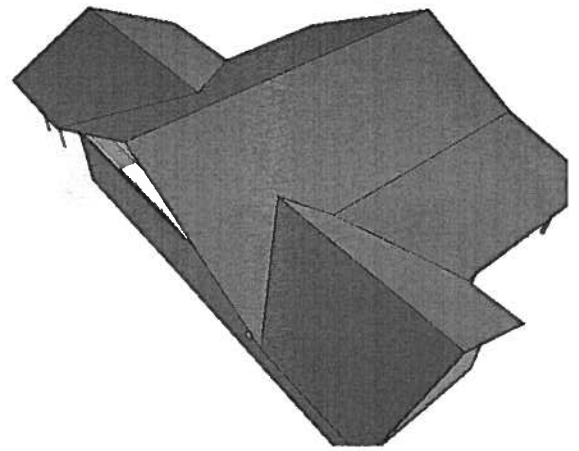
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.





*** HAGNER SCHEDULE ***

8 - HTU26

BEARING HEIGHT SCHEDULE

8'-0"

6/12 PITCH

(3/12 PITCH -- FRONT PORCH)

2' OVERHANGS

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: NORTON HOME IMP. CO. INC.

LEGAL ADDRESS: NICHOLS RESIDENCE

MODEL:		REVISION:
CUSTOM		SCALE: NTS
DATE: 03-20-06	DRAWN BY: RAL	JOB #: L156415