

5602 N.W. 13th STREET GAINESVILLE, FLORIDA 32653-2198

PO. BOX 5875 GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642 FAX (352) 373-9037

CERTIFICATE OF PROTECTIVE TREATMENT

Applicator.
Applicator:
10000
Joseph
10000
10000
10000
1000
10670
106%
106%
% Concentration: (6%) # Gallons Used: /50 Applicator:
1060%
1060%
1060%
1069/2
ation: 06%
10600
1060%
060%
060%
0690
, 069, # Gallons
750 # Gallons Used: 750
1069 # Gallons Used: 750
069, # Gallons Used: 750
060 # Gallons
1060 # Gallons Used: 750
1069 # Gallons Used: 750
Stratt Chemical Used: Bit
Chemical Used: Bit (1964) (196
Chemical Used: Bit (1964) Chemical Used: Bit (1964) Chemical Used:
Chemical Used: Bit (060) # Gallons Used: 750
Riffall T Chemical Used: Bit O60, # Gallons Used: 750
R. Frank T. Chemical Used: Bit O60, # Gallons Used: 750
Rith TT Chemical Used: Bit
Rithart Chemical Used: Bit
Chemical Used: Bit O600 # Gallons Used: 750
Riffer T Chemical Used: Bit Ober # Gallons Used: 750
Rithatt Chemical Used: Rith
Rithal Torch, Caracit Chemical Used: Bit O60 # Gallons Used: 750
Rith To The Chemical Used: Bit O60 # Gallons Used: 750
Rith I To Chemical Used: Rith (069) # Gallons Used: 750
Rith TT Chemical Used: Bit O62 # Gallons Used: 750
Rith Carage Rith This Chemical Used: Bit
Rith Tarage Chemical Used: Bit O600 # Gallons Used: 750
Rithat The Chemical Used: Bit Oben # Gallons Used: 750
Rithat The Chemical Used: Bit O600 # Gallons Used: 750
Rith Frih, Farage Rith IT Chemical Used: Rit 060 # Gallons Used: 750
Rith Brih, Garage Rith IT Chemical Used: Rit 069 # Gallons Used: 750
Rith T Chemical Used: Bit
Rith Carage Chemical Used: Bit Chemical Used: Bit Chemical Used: 750
Rith Faces Chemical Used: Bit O602 # Gallons Used: 750
Rith Brit, Garage Chemical Used: Bit O60 # Gallons Used: 750
Rith To Chemical Used: Rith Colons Used: Rith Chemical Used: Rith Chemical Used: 750
Rith Farage Chemical Used: Bit Colons Used: Bit Colons Used: 750
Rith To Chemical Used: Rith Colons & Chemical Used: Rith Colons Used: Rith Colons Used: 750
Living Porth, Garage Chemical Used: Rit Chemical Used: Rit Chemical Used: Rit Chemical Used: 750
Rith T Chemical Used: Bit (06%) # Gallons Used: 750
Living Porch, Cracair Ritar IT Chemical Used: Bit
1350 SW W. Krin Springs Bith Fareir Chemical Used: Bit
1350 SW Wilsin Springs Living Porth, Garage Rithat T Chemical Used: Bit 0692 # Gallons Used: 750
1350 SW Wilsin Springs Living Porth, Garage Riffer IT Chemical Used: Bit 060 # Gallons Used: 750
1350 SW Wilson Springs Living Brit, Farage Rithat T Chemical Used: Bit 060 # Gallons Used: 750
1350 SW Wilson Springs Rd Living Porch, Garage Ritartor Chemical Used: Bitarthrin John # Gallons Used: 750
Living Porch, Garage Chemical Used: Bitsthrin , 060 # Gallons Used: 750
Living Brih, Farage Rith T Chemical Used: Ritathria , 060 # Gallons Used: 750
Living Porch, Farage Chemical Used: Ritathria , 06% # Gallons Used: 750
1350 SW Wilsin Springs Had Biting Porth, Gracie Ritarthrin Chemical Used: Bitathrin 1069 # Gallons Used: 750
1350 SW Wilsin Springs Rd Biting Porch, Francis Used: Bitishrin Chemical Used: 750
1350 SW Wilsin Springs Rd Bitathin Chemical Used: Bitathin Color # Gallons Used: 750
1350 SW Milsin Springs Rd Living Porth, Garage Ritarthin Chemical Used: Bitathrin 1060 # Gallons Used: 750
1350 SW Wilsin Springs Rd Living Porch, Farage Ritar LT Chemical Used: 0692 # Gallons Used: 750
1350 SW With Springs Rd Rith Target Chemical Used: Bitathria 150
1350 SW WILL SOCIAGE RITH GALLE Chemical Used: Bit Colons Used: 750
1350 SW Wilson Springs Hall Living Porth, Garage Ritart Chemical Used: Bitathria 060 # Gallons Used: 750
1350 SW Wilson Springs Mal Living Porch, Garage Ritarto Chemical Used: Bitathria 060 # Gallons Used: 750
1350 SW Wilsin Springs Rd Living Porch, Garage Ritart Chemical Used: Bitathria , 0602 # Gallons Used: 750
1350 SW Wilsin Springs Rd Living Porch, Garrage Ritart Chemical Used: Bitathria , 0602 # Gallons Used: 750
1350 SW Wish Springs Rd Living Porch, Garage Ritart Chemical Used: Ritathria , 060 # Gallons Used: 750
1350 SW With Springs Rd Living Porch, Garage Rithat T Chemical Used: Rithrin 060 # Gallons Used: 750
1350 SW Wilson Springs Rd Living Porch, Garage Rithat T Chemical Used: Bitathria , 060 # Gallons Used: 750
1350 SW Wilson Springs Rd Living Porch, Garage Rithat T Chemical Used: Ritathria 069 # Gallons Used: 750
1350 SW Wilsin Springs Red Ritin T Chemical Used: Bitsthrin , 069 # Gallons Used: 750
1350 SW Wilsin Springs Rd Living Porch, Francis Used: Bitsthrin Ritin T Chemical Used: 750
1350 SW Wilsin Springs Rd Living Porch, Gracia Ritzthrin , 0622 # Gallons Used: 750
1350 SW Milsin Springs Rd Living Porth, Garage Rital T Chemical Used: Bitathrin , 0602 # Gallons Used: 750



From: The Columbia County Building & Zoning Department

Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529

Reference to a building permit application Number: 0604-64

Waters Custom Homes Owner Peter Herrick 1350 SW Wilson Springs Road

On the date of April 24, 2006 application 0604-64 and plans for placement of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0604-64 when making reference to this application.

- 1. Please detail the header beams which will span the three open sides of the carport. Also detail the total number of studs and jack studs which will support the header beams and the method in which they will be attached to the foundation.
- 2. If the open deck area will be constructed in conjunction with the structure please submit structural plans which detail the designs to be used to show compliance with the FBC-2004 sections 1609.

- 3. Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.
- Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be preformed by the Columbia County Building Department.

Thank you,

Joe Haltiwanger
Plan Examiner

Columbia County Building Department



Tommy Waters Custom Homes, inc.
5325 S.W. 91* Terrace
Gainesville, Florida 3260£
E-mail: TominyWatersHomes@act.com
Website, www.twch.nc.com

Fnone: (352) 336-7600 Fax: (352) 336-7633

Fax Cover Sheet

Dide: 4-25-06
MR. JOE HALTIWANGER
Company: COLUMBIA COUNTY BLDG DEPT
FAX # 386-754-7088
From KEN SMITH
Total # Of Pages Lichtding Cover Sheet:
Massage: RE: APPLIC. # 0604-64
IN RESPONSE TO YOUR FAX OF 4-24-06
ITEM 1 - HEADER BEAMS AT CARDONS
2-PLY 1 3/4" x 16" LVL CAM REAM
THE JACKS & TRIMMER STUDS ARE
CAMED OUT IN THE WIND CODE ENGINEERING
(3 JACKS & 4 STUDS AT BACH SIDE CONNECTORS
ARE ALSO SPECIFIED IN THE HEADED TARIE
ITEM 2 - THE OPEN DECK AREA HAS BEEN
- QMITTED
ITEM 3 - A & B. CONSTRUCTION (ROCKY FORD) WILL BE
- INSTALLING THE SEPTIC SYSTEM
ITEM 4 - NOTICE OF COMMENCEMENT FORTHCOMINE
FROM FLORIDA CITIZENS BANK
THOUKS
(MIS)

Trepared by ReCord-Return To: (IK. Cent Lopy To:
Jenny Madson - Fla-Citizens Bank Tommy waters
2810-B NW 43 relst,
At: 1000
19150 CIK. Cent.
150 CIK. Cent.
PERMIT No.

Tommy waters Cust Ilms att: Icon Smith 5225 SW 91st Terr, Garls Ville, FL 32608

24505

NOTICE OF COMMENCEMENT

The undersigned, after being first duly sworn, states as follows and verifies that the information set forth in this Notice of Commencement is true to the best of the undersigned's knowledge, information and belief:

Description of Property (legal and street address):

SEE ATTACHMENT FOR LENGTHY LEGAL DESCH

STATE OF FLORIDA, COUNTY OF COLUMBIA I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office. P. DeWITT CASON, CLERK OF COURTS

1350 SW WILSON SPRINGS ROAD, FORT WH

General Description of Property: CONSTRUCT SINGLE FAMILY RESIDENCE

Name of Borrower(s): PETER R. HERRICK DIANA R. HERRICK Address of Borrower(s): 1350 SW WILSON SPRINGS ROAD

FORT WHITE, FL 32038

Borrower(s) interest in Property: PRIMARY RESIDENCE

Name & Address of Fee simple titleholder (if other than Borrower):

Builder's Name: TOMMY WATERS CUSTOM HOMES, INC Builder's Address:

5225 SW 91ST TERRACE, GAINESVILLE, FL 32608

7. Name and address of all lending institutions which provide financing for the improvements: FLORIDA CITIZENS BANK

2810-B NW 43RD STREET

GAINESVILLE FL 32606

Name and address of the designee, if any, of the Borrower:

is specified: PRICE R. HOMEL A Borrower PETER R. HERRICK BY DIANA R. HERRICK AS ATTORNEY	Y Ph Date IN FACT	2006 Cas R. W. MOO Borrower DIANA R. HERRICK	6-12-2006 Date
Borrower	Date	Borrower	Date
STATE OF FLORIDA COUNTY OF COLUMBIA The foregoing instrument was subscribed JUNE , 2006 . BY AND DIANA R. HERRICK, INDIVI	PETER R.	pefore me this 12TH HERRICK BY DIANA R. HERRIC My Commission Expires:	day of CK ATTORNEY IN FACT

Expiration date of this Notice of Commencement is one year from date of recording unless a different date

-1310 (9609)

9.

VMP MORTGAGE FORMS - (800)521-7291

9/96







REPORT ON IN-PLACE DENSITY TESTS

4475 S.W. 35th Terrace • Gainesville, Florida 32608 • (352) 372-3392

Table 2014 / 1 miles	# 24505
PROJECT: 1350 SW Wison Spains DD	
PROJECT: 1000 000 000 000 000 000	
AREA TESTED: Fill & prop bldg. pod	
course: F/G	DEPTH OF TEST:
TYPE OF TEST: Astmd 2920	DATE TESTED: 06-22-06
NOTE: The below tests DO/DO NOT meet the minimum of maximum density.	95 % compaction requirements
REMARKS:	

LOCATION OF TESTS	DRY DEN.	MAX. DEN.	% MAX. DEN.	MOIST.	OPT. MOIST.
		105.5			10.0
1	I.s.	1		//	
PROP BILLS pad	104.1		98.7	4.8	
prop. Isldy pad					
ADD Center of Suttering	105.3		99.8	4.8	
Apa Center of Suttering					
A. 10' ct (C 111)	124.3		97.9	3. 1	
App. 10' SF OF NW Coines OF	109.		/ Ca.: /		
707 330 330					
					4
		প			
					4404

TECH.	
IECH.	

MAR-23-2007 11:14

From: TOMMYWATERSHOMES

To:13867582160

Page:2/

FROM : ARROW EXT

FAX NO. :352 373 9037

Mar. 23 2007 10:12AM P2/2



5602 N.W. 13th STREET GAINESVILLE, FLORIDA 32653-2188

P.O. BOX 5875 GAINESVILLE, FLORIDA 32827-5875

PHONE (352) 373-3642 FAX (352) 373-9037

Parmid 24505

CERTIFICATE OF COMPLIANCE OF TERMITE PROTECTION

(as required by Florida Building Code (FBC) 1816.1.7)

ARROW EXTERMINATORS, INC (352) 373-3642

Tommy Waters Customs Homes 1350 SW Wilson Spring Road, Ft. White Address of Treatment or Lot/Block of Treatment

Soil Parrier

Method of Termite Prevention Treatment-soil barrier, wood treatment, bait system, other (describe)

The building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws established by the Florida Department of Agriculture and Consumer Services.

Authorized Signature

March 23, 2007

Date



OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 05-7S-16-04137-017

Building permit No. 000024505

Use Classification SFD/UTILITY

Waste: 0.00

Fire:

0.00

Total: 0.00



Location: 1350 SW WILSON SPRINGS RD

Owner of Building PETER & DIANA HERRICK

Permit Holder ROBERT D. WATERS

Date: 03/23/2007

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



RE: WAHERR - HERRICK RESIDENCE

MiTek Industries, Inc.

1801 Massaro Blvd. Tampa, Fl 33619 Phone: 813/675-1200 Fax: 813/675-1148

Site Information:

Project Customer:

Project Name:

Lot/Block:

Subdivision:

Address:

City:

State:

Name Address and License # of Structural Engineer of Record, If there is one, for the building. License #:

Name:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 6.2

Wind Code: ASCE 7-98 Wind Speed: 110 mph

Design Method: Main Wind Force Resisting System ASCE 7-98

Roof Load: 40.0 psf

Floor Load: 55.0 psf

This package includes 10 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date
1	T1856354	WAHERR	Α	11/10/05
2	T1856355	WAHERR	A1	11/10/05
3	T1856356	WAHERR	A2	11/10/05
4	T1856357	WAHERR	A2ET	11/10/05
5	T1856358	WAHERR	AET	11/10/05
6	T1856359	WAHERR	В	11/10/05
7	T1856360	WAHERR	BET	11/10/05
8	T1856361	WAHERR	С	11/10/05
9	T1856362	WAHERR	CET	11/10/05
10	T1856363	WAHERR	CET1	11/10/05

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie

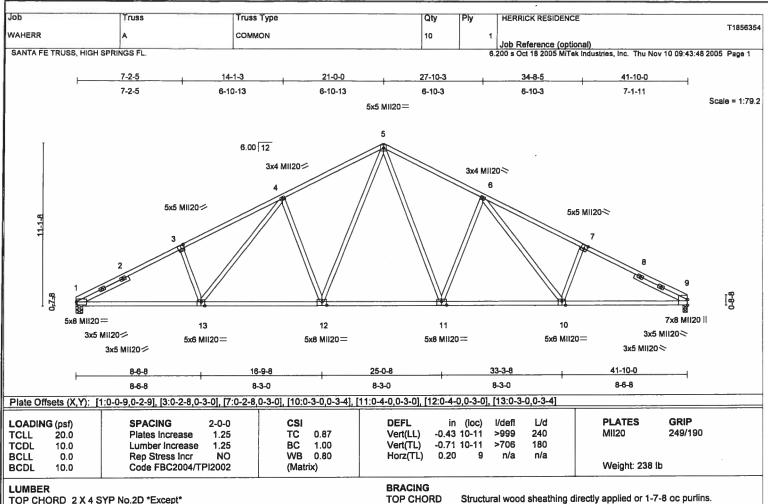
My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

1 of 2

Zhang, Guofjie

November 10,2005



BOT CHORD

Rigid ceiling directly applied or 8-3-15 oc bracing.

TOP CHORD 2 X 4 SYP No.2D *Except*

7-9 2 X 4 SYP SS

BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **WEBS**

SLIDER Left 2 X 4 SYP No.3 3-11-7, Right 2 X 4 SYP No.3 3-11-8

REACTIONS (lb/size) 1=2168/0-5-8, 9=2168/0-3-8

Max Horz 1=-175(load case 3)

Max Uplift1=-329(load case 5), 9=-328(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-4069/573, 2-3=-3977/594, 3-4=-3906/653, 4-5=-3181/555, 5-6=-3153/548, 6-7=-3805/635, 7-8=-3903/585, TOP CHORD

8-9=-4010/564 **BOT CHORD**

1-13=-588/3503, 12-13=-392/3005, 11-12=-179/2299, 10-11=-253/2981, 9-10=-417/3422

3-13=-256/244, 4-13=-187/713, 4-12=-735/346, 5-12=-260/1291, 5-11=-253/1266, 6-11=-718/342, 6-10=-170/648, **WEBS**

7-10=-215/236

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 1 and 328 lb uplift at joint

- 6) Load case(s) 1, 7, 8 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-5=-60, 5-9=-60, 1-13=-20, 10-13=-60(F=-40), 9-10=-20

7) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-20, 1-13=-20, 10-13=-60(F=-40), 9-10=-20

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634 November 10,2005

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for gateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

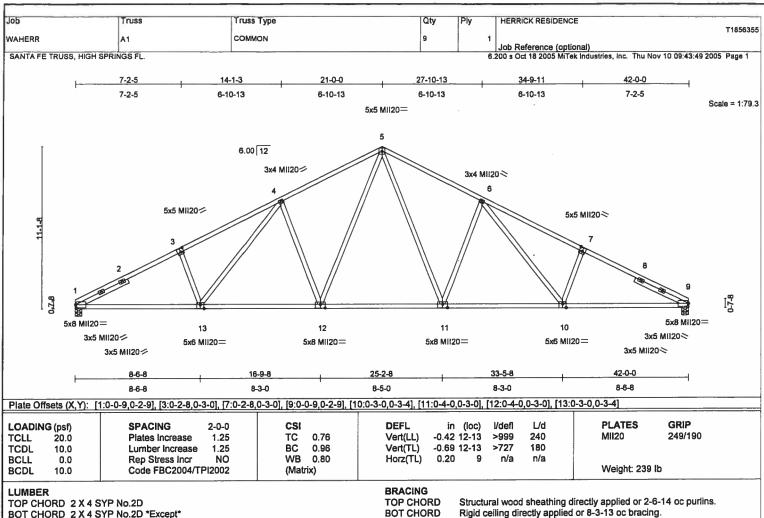


	Truss	Truss Type	Qty	Ply	HERRICK RESIDENCE	T18563
ERR	A	COMMON	10		1 leb Deference (actions)	
TA FE TRUSS, HIGH SPR	INGS FL.				Job Reference (optional) 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu No	v 10 09:43:48 2005 Page 2
AD CASE(S) Standar and unbalanced Regul Jniform Loads (plf) Vert: 1-5=-20,	d ar: Lumber Increase=1.2 5-9=-60, 1-13=-20, 10-13	5, Plate Increase=1.25 3=-60(F=-40), 9-10=-20				
					Sec.	
					(20)	
					F	
					ş	

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability-during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





BOT CHORD 2 X 4 SYP No.2D *Except*

10-11 2 X 4 SYP SS, 12-13 2 X 4 SYP SS 2 X 4 SYP No.3

WEBS

Left 2 X 4 SYP No.3 3-11-7, Right 2 X 4 SYP No.3 3-11-7 SLIDER

REACTIONS (lb/size) 1=2178/0-5-8, 9=2178/0-5-8

Max Horz 1=-175(load case 3)

Max Uplift1=-330(load case 5), 9=-330(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-4089/575, 2-3=-3996/595, 3-4=-3925/655, 4-5=-3204/557, 5-6=-3204/557, 6-7=-3925/655, 7-8=-3996/596, TOP CHORD

8-9=-4089/575

1-13=-590/3520, 12-13=-394/3024, 11-12=-181/2318, 10-11=-256/3024, 9-10=-429/3520 **BOT CHORD**

3-13=-256/244, 4-13=-187/709, 4-12=-732/346, 5-12=-259/1296, 5-11=-259/1296, 6-11=-732/346, 6-10=-187/709, **WEBS**

7-10=-256/244

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 330 lb uplift at joint 1 and 330 lb uplift at joint 9
- 6) Load case(s) 1, 7, 8 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 1-13=-20, 10-13=-60(F=-40), 9-10=-20 7) 1st unbalanced Regular. Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-20, 1-13=-20, 10-13=-60(F=-40), 9-10=-20

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634 November 10,2005

Continued on page 2

A WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is tacifateral support of individual-web members only. Additional temporary bracing to insure stability during-construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	HERRICK RESIDENCE	
WAHERR	44	COMMON			T18	56355
WANERK	^1	COMMON	9	1	Job Reference (optional)	
SANTA FE TRUSS, HIGH SPRI	NGS FL.		•	6.	200 s Oct 18 2005 MiTek Industries, Inc. Thu Nov 10 09:43:49 2005 Pag	je 2

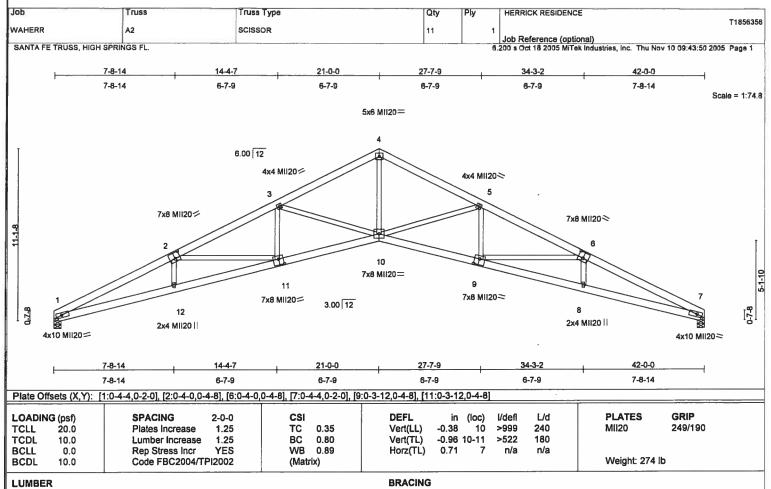
LOAD CASE(S) Standard
8) 2nd unbalanced Regular. Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
Vert: 1-5=-20, 5-9=-60, 1-13=-20, 10-13=-60(F=-40), 9-10=-20

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown Applicability of design parameters and proper incorporation for the steady should be a construction as a significant responsibility of the steady should be a constructed by the steady should be stead







TOP CHORD 2 X 6 SYP No.2 BOT CHORD 2 X 6 SYP No.2 WEBS 2 X 4 SYP No.3 TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-1-2 oc purlins.

Rigid ceiling directly applied or 7-5-7 oc bracing.

REACTIONS (lb/size) 1=1662/0-5-8, 7=1662/0-5-8

Max Horz 1=-173(load case 3)

Max Uplift1=-326(load case 5), 7=-326(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-5453/1112, 2-3=-4648/892, 3-4=-3651/623, 4-5=-3651/645, 5-6=-4648/729, 6-7=-5453/965

BOT CHORD 1-12=-1084/4864, 11-12=-1080/4873, 10-11=-744/4235, 9-10=-485/4235, 8-9=-784/4873, 7-8=-788/4864

WEBS 2-12=0/272, 2-11=-694/327, 3-11=-40/426, 3-10=-994/398, 4-10=-396/2767, 5-10=-994/402, 5-9=-44/426, 6-9=-694/344,

6-8=0/272

NOTES

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 326 lb uplift at joint 1 and 326 lb uplift at joint

LOAD CASE(S) Standard

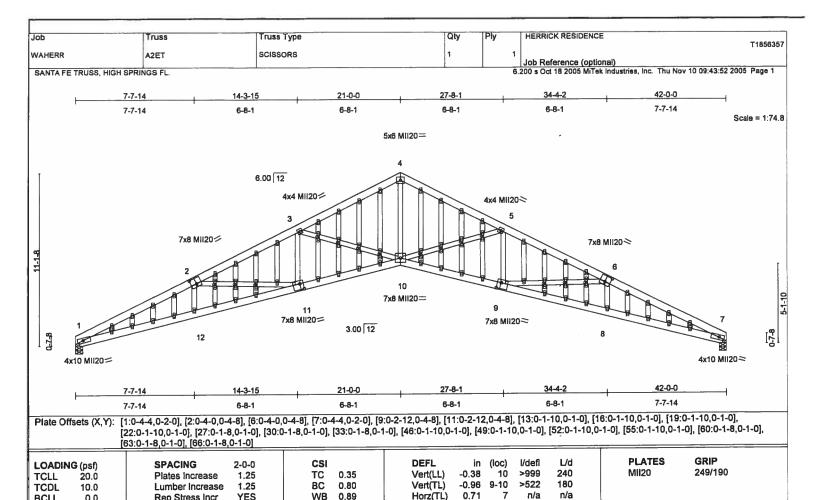
Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

November 10,2005

WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for laterate property of individual web members only. Additional temporary to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





LUMBER

BCLL

BCDL

TOP CHORD 2 X 6 SYP No.2 BOT CHORD 2 X 6 SYP No.2 2 X 4 SYP No.3 **WEBS** 2 X 4 SYP No.3 **OTHERS**

0.0

10.0

BRACING

Horz(TL)

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-1-2 oc purlins.

Weight: 361 lb

Rigid ceiling directly applied or 7-5-7 oc bracing.

REACTIONS (lb/size) 1=1662/0-5-8, 7=1662/0-5-8

Max Horz 1=-173(load case 3)

Max Uplift1=-326(load case 5), 7=-326(load case 6)

Rep Stress Incr

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-5454/1110, 2-3=-4650/893, 3-4=-3652/623, 4-5=-3652/645, 5-6=-4650/730, 6-7=-5454/963 TOP CHORD

YES

1-12=-1082/4865, 11-12=-1081/4869, 10-11=-746/4242, 9-10=-485/4242, 8-9=-785/4869, 7-8=-786/4865 **BOT CHORD**

4-10=-394/2763, 2-12=0/275, 3-11=-39/425, 5-9=-43/425, 6-8=0/275, 2-11=-687/327, 3-10=-999/398, 5-10=-999/402, **WEBS**

0.89

(Matrix)

6-9=-687/344

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail*
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

5) All plates are 2x4 Mil20 unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Gable studs spaced at 1-4-0 oc.

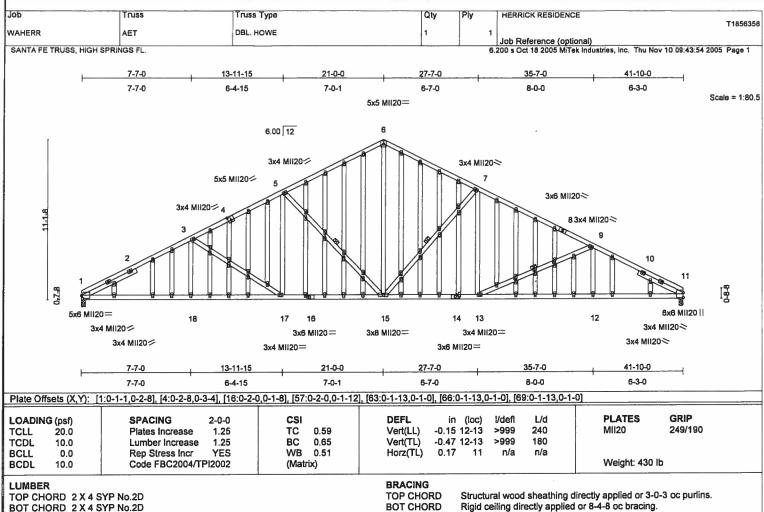
- 8) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 326 lb uplift at joint 1 and 326 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634 November 10,2005

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WFBS

1 Row at midpt

BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **WEBS**

OTHERS 2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.3 4-1-15, Right 2 X 4 SYP No.3 3-5-7

REACTIONS (lb/size) 1=1673/0-3-8, 11=1673/0-3-8

Max Horz 1=-175(load case 3)

Max Uplift1=-329(load case 5), 11=-328(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2 = -3009/560, 2-3 = -2883/581, 3-4 = -2508/497, 4-5 = -2429/523, 5-6 = -1934/463, 6-7 = -1934/468, 7-8 = -2397/505, 3-6 = -1934/468, 7-8 = -2397/505, 3-6 = -1934/468, 7-8 = -2397/505, 3-6 = -1934/468, 3-6TOP CHORD

8-9=-2495/479, 9-10=-2939/585, 10-11=-3031/562

1-18=-571/2579, 17-18=-571/2579, 16-17=-385/2173, 15-16=-385/2173, 14-15=-243/2144, 13-14=-243/2144, **BOT CHORD**

12-13=-430/2579. 11-12=-430/2579

3-18=0/272, 5-17=-49/464, 6-15=-247/1254, 7-13=-21/447, 9-12=0/276, 3-17=-504/221, 5-15=-781/307, 7-15=-784/310,

9-13=-508/236

NOTES

WEBS

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MII20 unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Gable studs spaced at 1-4-0 oc.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 1 and 328 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634 November 10,2005

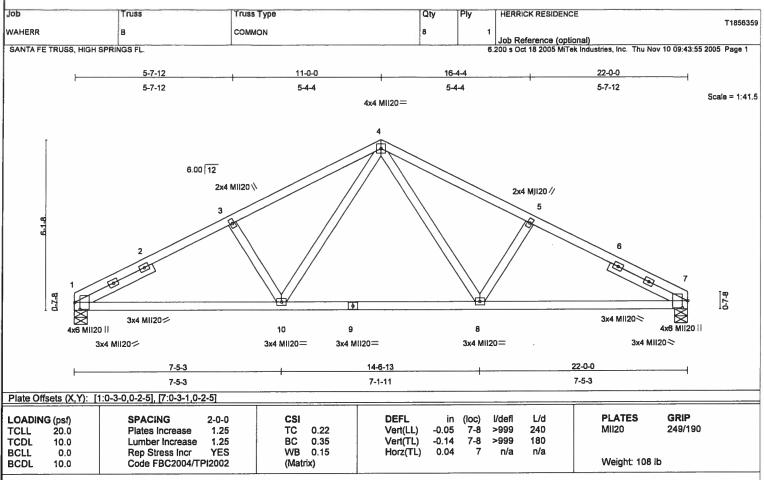
5-15, 7-15, 9-13

1801 Massaro Blvd.

Tampa, FL 33619

MARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

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LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

WEBS 2 X 4 SYP No.3

SLIDER

Left 2 X 4 SYP No.3 3-1-1, Right 2 X 4 SYP No.3 3-1-1

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-7-10 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=880/0-5-8, 7=880/0-5-8

Max Horz 1=-94(load case 3)

Max Uplift1=-172(load case 5), 7=-172(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1434/275, 2-3=-1307/290, 3-4=-1272/299, 4-5=-1272/300, 5-6=-1307/291, 6-7=-1434/275

BOT CHORD 1-10=-265/1207, 9-10=-100/854, 8-9=-100/854, 7-8=-178/1207

WEBS 3-10=-279/200, 4-10=-119/455, 4-8=-119/455, 5-8=-279/200

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

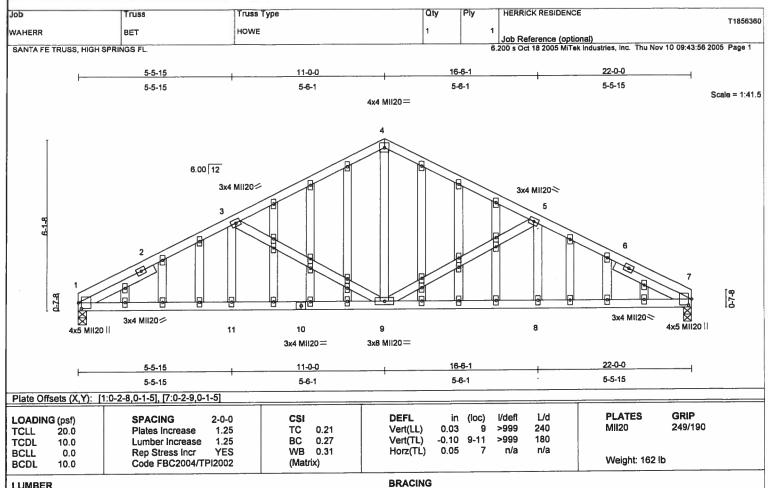
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 172 lb uplift at joint 1 and 172 lb uplift at joint 7

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634 November 10,2005

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TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 **WEBS OTHERS** 2 X 4 SYP No.3

Left 2 X 4 SYP No.3 2-11-15, Right 2 X 4 SYP No.3 2-11-15 SLIDER

Max Horz 1=-94(load case 3)

REACTIONS (lb/size) 1=880/0-3-8, 7=880/0-3-8

Max Uplift1=-172(load case 5), 7=-172(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-1461/265, 2-3=-1335/281, 3-4=-1043/239, 4-5=-1043/239, 5-6=-1334/282, 6-7=-1461/266 TOP CHORD

1-11=-259/1228, 10-11=-259/1228, 9-10=-259/1228, 8-9=-173/1228, 7-8=-173/1228 BOT CHORD

3-11=0/215, 4-9=-65/523, 5-8=0/215, 3-9=-441/191, 5-9=-441/192 **WEBS**

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MII20 unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Gable studs spaced at 1-4-0 oc.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 172 lb uplift at joint 1 and 172 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634 November 10,2005

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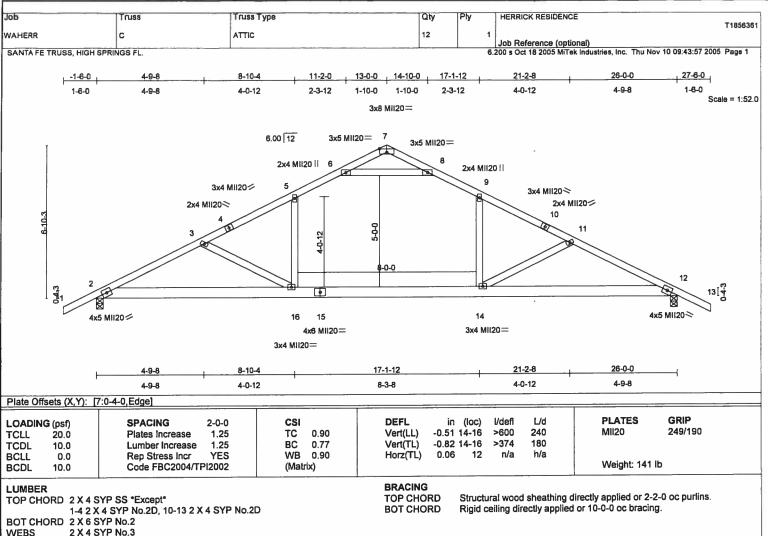
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1801 Massaro Blvd. Tampa, FL 33619

Structural wood sheathing directly applied or 5-7-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





2 X 4 SYP No.3

REACTIONS (lb/size) 2=1503/0-3-8, 12=1503/0-3-8

Max Horz 2=133(load case 5)

Max Uplift2=-268(load case 5), 12=-268(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

 $1-2=0/43,\ 2-3=-2814/301,\ 3-4=-2372/211,\ 4-5=-2270/223,\ 5-6=-1906/238,\ 6-7=-41/871,\ 7-8=-41/871,\ 8-9=-1906/237,\ 3-4=-2372/211,\ 4-5=-2270/223,\ 5-6=-1906/238,\ 6-7=-41/871,\ 7-8=-41/871,\ 8-9=-1906/237,\ 3-4=-2372/211,\ 4-5=-2270/223,\ 5-6=-1906/238,\ 6-7=-41/871,\ 7-8=-41/871,\ 8-9=-1906/237,\ 3-4=-2372/211,\ 4-5=-2270/223,\ 5-6=-1906/238,\ 6-7=-41/871,\ 7-8=-41/871,\ 8-9=-1906/237,\ 3-4=-2372/211,\ 4-5=-2270/223,\ 5-6=-1906/238,\ 6-7=-41/871,\ 7-8=-41/871,\ 8-9=-1906/237,\ 8-9=-$ TOP CHORD

9-10=-2270/222, 10-11=-2372/211, 11-12=-2814/302, 12-13=0/43 2-16=-297/2476, 15-16=-91/1985, 14-15=-91/1985, 12-14=-180/2476

BOT CHORD 6-8=-2959/291, 5-16=-4/788, 9-14=-4/788, 3-16=-581/235, 11-14=-581/235 **WEBS**

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Ceiling dead load (5.0 psf) on member(s). 5-6, 8-9, 6-8

- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 14-16
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 2 and 268 lb uplift at joint

LOAD CASE(S) Standard

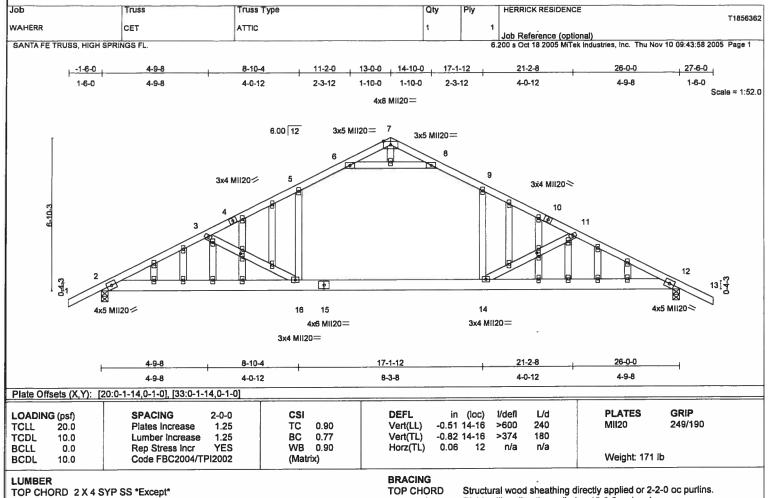
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November 10,2005

1801 Massaro Blvd Tampa, FL 33619

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1-4 2 X 4 SYP No.2D, 10-13 2 X 4 SYP No.2D

BOT CHORD 2 X 6 SYP No.2 **WEBS** 2 X 4 SYP No.3 **OTHERS** 2 X 4 SYP No.3

Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD**

REACTIONS (lb/size) 2=1503/0-3-8, 12=1503/0-3-8

Max Horz 2=133(load case 5)

Max Uplift2=-268(load case 5), 12=-268(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

 $1-2=0/43,\ 2-3=2814/301,\ 3-4=2372/211,\ 4-5=-2270/223,\ 5-6=-1906/238,\ 6-7=-41/871,\ 7-8=-41/871,\ 8-9=-1906/237,\ 3-1$ TOP CHORD

9-10=-2270/222, 10-11=-2372/211, 11-12=-2814/302, 12-13=0/43

2-16=-297/2476, 15-16=-91/1985, 14-15=-91/1985, 12-14=-180/2476 **BOT CHORD WEBS** 6-8=-2959/291, 5-16=-4/788, 9-14=-4/788, 3-16=-581/235, 11-14=-581/235

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 Mil20 unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Gable studs spaced at 1-4-0 oc.

8) Ceiling dead load (5.0 psf) on member(s), 5-6, 8-9, 6-8

- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 14-16
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 2 and 268 lb uplift at joint

LOAD CASE(S) Standard

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MARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

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Job HERRICK RESIDENCE Truss Truss Type Qty T1856363 WAHERR CET1 KINGPOST Job Reference (optional)
6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Nov 10 09:43:59 2005 Page 1 SANTA FE TRUSS, HIGH SPRINGS FL -1-6-0 6-9-4 13-0-0 15-9-8 1-6-0 6-9-4 6-2-12 2-9-8 5x5 MII20= Scale = 1:42.8 4 6.00 12 6x14 MII20 \\ 5x5 M1120/ 3x4 MII20 / 3x4 MII20= 3x5 MI120= 15-9-8 8-0-4 7-9-4 8-0-4 Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-2-8,0-3-0], [3:0-1-12,0-0-8]

LOADING (psf) SPACING 2-0-0 TCLL 20.0 Plates Increase 1.25 TCDL 10.0 Lumber Increase 1.25 BCLL 0.0 Rep Stress Incr YES BCDL 10.0 Code FBC2004/TPl2002	CSI DEFL in TC 0.39 Vert(LL) -0.07 BC 0.38 Vert(TL) -0.20 WB 0.46 Horz(TL) 0.01 (Matrix)		PLATES GRIP MII20 249/190 Weight: 144 lb
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LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.3 BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=724/0-3-8, 6=615/0-3-8

Max Horz 2=276(load case 5)

Max Uplift2=-206(load case 5), 6=-159(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/39, 2-3=-926/138, 3-4=-791/197, 4-5=-50/39, 5-6=-80/27

BOT CHORD 2-7=-270/750, 6-7=-76/217

WEBS 3-7=-375/243, 4-7=-186/677, 4-6=-544/225

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MII20 unless otherwise indicated.

6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

7) Gable studs spaced at 1-4-0 oc.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 2 and 159 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

November 10,2005

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