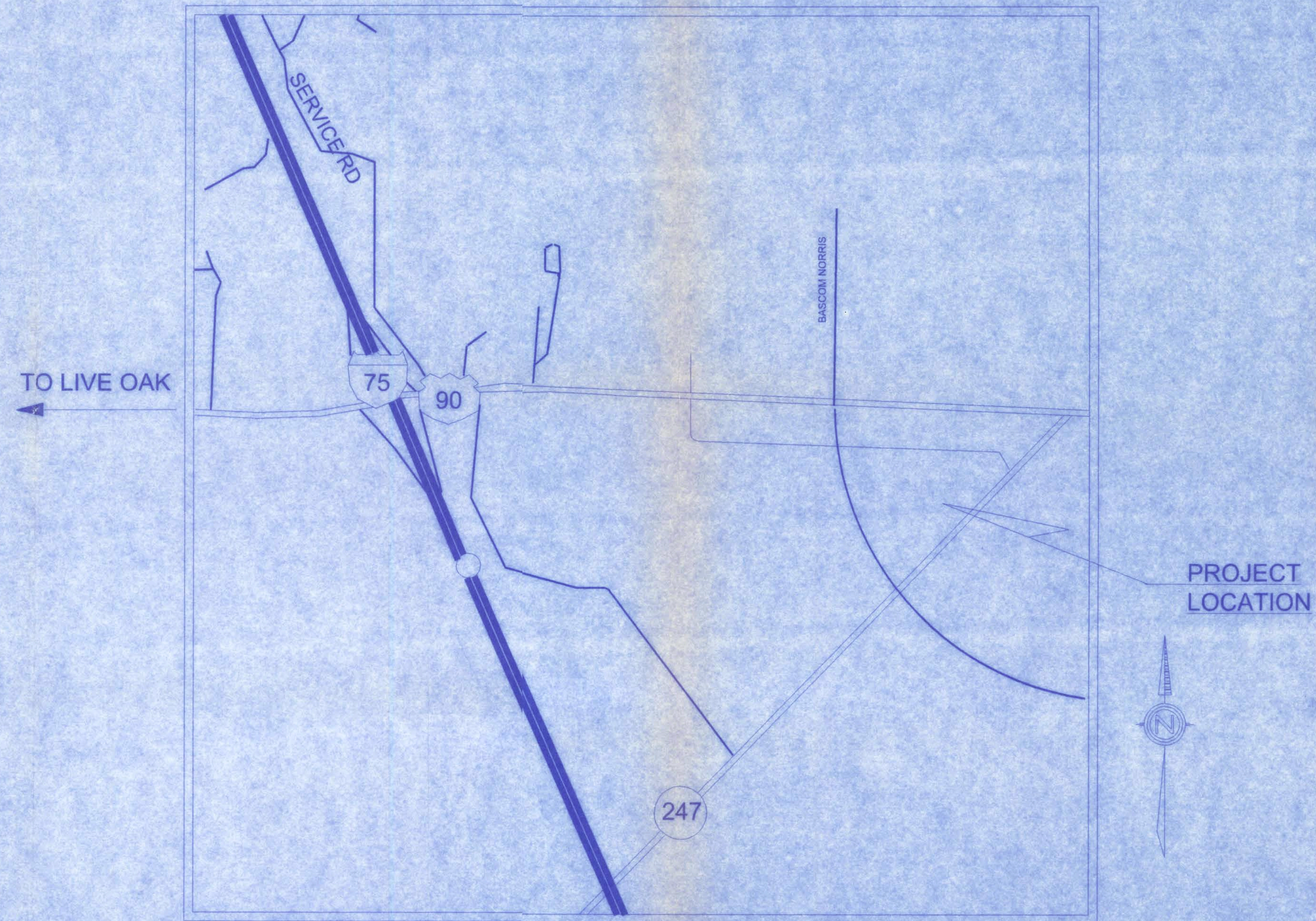


POLE BARN BUILDING
FOR
WILSON'S AXLE REPAIR

LAKE CITY, FLORIDA



INDEX OF SHEETS

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A-1	GENERAL NOTES
A-2	PLAN AND ELEVATIONS
A-3	ROOF PLAN AND DETAILS

PLANS PREPARED FOR:

WILSON'S AXLE SERVICE
LAKE CITY, FLORIDA

PLANS PREPARED BY:

Freeman
Design Group

181 N.W. MADISON STREET
SUITE #102
LAKE CITY, FL 33555
(888) 755-4229
DESIGNERS OF WILSON'S AXLE SERVICE

LOCATION MAP

Will H. Free
10/1/06

GENERAL NOTES

DESIGN CRITERIA

- D1 ALL WORK SHALL CONFORM TO AT LEAST THE MINIMUM STANDARD OF THE FOLLOWING CODES:
2004 FLORIDA BUILDING CODE, BUILDING
2004 FLORIDA BUILDING CODE, FUEL GAS
2004 FLORIDA BUILDING CODE, MECHANICAL
2004 FLORIDA BUILDING CODE, PLUMBING
2004 FLORIDA FIRE PREVENTION CODE
2002 NATIONAL ELECTRIC CODE.
- D2 DESIGN LOAD VALUES:
ROOF LIVE LOADS 20 PSF
ROOF DEAD LOADS 10 PSF
ASSUMED ALLOWABLE SOIL BEARING CAPACITY 2000 PSF
- D3 THE STRUCTURAL PLANS AND WIND SPEED HAVE BEEN DESIGNED IN ACCORDANCE WITH SECTION 1609 OF THE FLORIDA BUILDING CODE 2004 EDITION.

GENERAL

- G1 THE GENERAL CONTRACTOR SHALL REVIEW AND DETERMINE THAT DIMENSIONS ARE COORDINATED BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
- G2 THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, THE WORKPERSONS, AND OTHER PEOPLE DURING CONSTRUCTION. HE SHALL SUPERVISE AND DIRECT THE WORK AND BE RESPONSIBLE FOR ALL CONSTRUCTION & FOR ALL JOBSITE SAFETY.
- G3 NO STRUCTURAL MEMBER SHALL BE CUT NOTCHED OR OTHERWISE REDUCED IN STRENGTH.
- G4 THE GENERAL CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR ANCHORED, EMBEDDED AND SUPPORTED ITEMS WHICH AFFECT THE STRUCTURAL DRAWINGS AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES.
- G5 ANY SUBMITTALS RECEIVED BY A/E THAT HAVE NOT BEEN CHECKED BY THE GC AND HIS SUBCONTRACTOR SHALL BE RETURNED WITHOUT REVIEW.
- G6 ALL SECTIONS AND DETAILS SHALL BE CONSTRUED TO BE TYPICAL OR SIMILAR UNLESS ANOTHER SECTION OR DETAIL IS NOTED.
- G7 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING ALL EXISTING CONDITIONS, ELEVATIONS, AND DIMENSIONS PRIOR TO COMMENCING CONSTRUCTION, INCLUDING FABRICATION. ALL DISCREPANCIES SHALL BE REPORTED TO THE A/E FOR RESOLUTION.

FOUNDATIONS

- F1 A GEOTECHNICAL REPORT FOR THIS PROJECT WAS NOT PROVIDED BY THE OWNER. THESE PLANS ARE BASED UPON AN ASSUMED ALLOWABLE BEARING CAPACITY OF 2000 PSF. THE CONTRACTOR SHALL ENGAGE A QUALIFIED AND CERTIFIED GEOTECHNICAL ENGINEER TO DETERMINE THE ALLOWABLE SOIL BEARING CAPACITY. A COPY OF THE REPORT SHALL BE PROVIDED TO THE A/E. IF THE DETERMINED ALLOWABLE BEARING CAPACITY IS LESS THAN THE ASSUMED VALUE, MODIFICATIONS TO THE FOUNDATIONS MAY BE REQUIRED.
- F2 THE GEOTECHNICAL ENGINEER SHALL MAKE A FIELD INVESTIGATION TO DETERMINE IF ANY SOIL CONDITIONS ARE PRESENT THAT MAY ADVERSELY AFFECT THE PROJECT. THE CONTRACTOR SHALL REMOVE ALL SUCH MATERIAL AND REPLACE IT WITH APPROVED FILL.
- F3 SUBGRADE UNDER FOOTINGS AND SLABS SHALL BE COMPACTED TO AT LEAST 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D1557). SUBMIT DENSITY TESTS TO THE A/E.

SLABS ON GRADE

- S1 PROVIDE 6 MIL POLYETHYLENE SHEETING UNDER ALL SLABS ON GRADE.
- S2 PROVIDE CONTROL JOINTS (1/4" WIDE BY 3/8" DEEP) AS INDICATED ON PLAN. FILL JOINTS WITH POURED RUBBER. IF JOINTS ARE SAWCUT, SAWCUTTING MUST BE DONE THE SAME DAY THE CONCRETE IS PLACED.
- S3 PROVIDE 1/2" EXPANSION JOINTS AT ALL LOCATIONS WHERE SLABS ABUT STRUCTURES (WALLS, COLUMNS, ETC.)

CONCRETE AND REINFORCING

- C1 THE GENERAL CONTRACTOR SHALL ENGAGE A CERTIFIED TESTING AGENCY TO PERFORM INDUSTRY STANDARD TESTING INCLUDING SLUMP TESTS AND CYLINDER BREAKS TO ENSURE CONFORMANCE WITH PLANS. SUBMIT REPORTS TO A/E.
- C2 CONCRETE WORK & MIX DESIGN SHALL CONFORM TO ACI 301 (LATEST EDITION) "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS". MIXING SHALL BE IN ACCORDANCE WITH ASTM C94.
- C3 MINIMUM 28-DAY COMPRESSIVE STRENGTH:
FOOTINGS AND TIE BEAMS 2,500 PSI
COLUMNS AND SLABS 2,500 PSI
- C4 SPLICES OF REINFORCING (EXCEPT AS SHOWN ON PLANS)
#4 BARS 20 INCHES
#5 BARS 27 INCHES
WELDED WIRE MESH 6"
- C5 AT ALL CORNERS OF TIE BEAMS AND WALL FOOTINGS, PROVIDE CORNER BARS (30 INCH MINIMUM LEGS) TO MATCH HORIZONTAL BARS.
- C6 REINFORCING BARS SHALL CONFORM TO ASTM A615-96a GRADE 40. WELDED WIRE MESH SHALL CONFORM TO ASTM A-185. LAP WELDED WIRE MESH ON MESH + 2" WHERE SPLICED.
- C7 MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED.

LOCATION	MIN. COVER
BOTTOM OF FOOTINGS	3"
SIDES OF FOOTINGS	3"
COLUMNS	1 1/2"
TIE BEAMS	1 1/2"
SLABS	AS NOTED

PRE-FABRICATED WOOD TRUSSES

- WT1 WOOD TRUSSES SHALL BE DESIGNED, SIGNED & SEALED BY A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. TRUSSES SHALL BE FABRICATED IN CONFORMANCE WITH THE THE "QUALITY CONTROL MANUAL" BY TRUSS PLATE INSTITUTE (TPI).
- WT2 HANDLING, ERECTION AND BRACING OF WOOD TRUSSES SHALL BE IN ACCORDANCE WITH "HANDLING AND ERECTING WOOD TRUSSES" (HET80) AND "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS" (BWT-76) BY THE TRUSS PLATE INSTITUTE (TPI).
- WT3 PERMANENT BRACING SHALL BE INDICATED IN THE TRUSS LAYOUT DRAWINGS AND SHALL BE SUPPLIED AND INSTALLED BY THE FRAMING CONTRACTOR.
- WT4 TRUSSES SHALL BE DESIGNED PER ASCE 7-98 FOR THE FOLLOWING LOADS:
DEAD LOAD 10 PSF
LIVE LOAD 20 PSF
WIND 110 MPH W/ 3 SECOND WIND GUST
- WT5 PRE-FABRICATED WOOD TRUSSES SHALL BE FABRICATED FROM SOUTHERN PINE (SPIB) KILN DRIED #2 GRADE OR BETTER FOR CHORD AND #3 GRADE OR BETTER FOR WEBS.
- WT6 TRUSS BEARING SHALL BE 4" NOMINAL UNLESS NOTED OTHERWISE. BEARING LOCATIONS MUST BE MARKED ON TRUSS BY FABRICATOR TO INSURE PROPER INSTALLATION.
- WT7 SHOP DRAWINGS SHALL BE SUBMITTED WHICH INDICATE DESIGN LOADS, DURATION FACTOR, TRUSS LAYOUT, TRUSS CONFIGURATION AND TRUSS TO TRUSS CONNECTION. SHOP DRAWINGS SHALL SHOW PIECE MARKS, MEMBER SIZE AND GRADE AND CONNECTION DETAILS.
- WT8 NO WANE KNOTS, SKIPS OR OTHER DEFECTS SHALL OCCUR IN THE PLATE CONTACT AREA OR SCARFED AREA OF WEB MEMBERS. PLATES SHALL BE CENTERED WITH ONE REQUIRED EACH SIDE OR TRUSS.
- WT9 DESIGN OF METAL CONNECTED WOOD ROOF TRUSSES TO COMPLY WITH STANDARD BLDG. CODE NFPA'S "NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADED LUBER AND ITS FASTENINGS", AND TRUSS PLATE INSTITUTE'S "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES".
- WT10 WOOD BLOCKING AT TRUSS BEARING SHALL BE LAP SPLICED 4'-0" MIN. AND NAILED WITH (20) 10d NAILS AT SPLICE, 10d NAILS @ 16" O.C. ELSEWHERE.

MASONRY

- M1 MASONRY CONSTRUCTION SHALL CONFORM TO ACI STANDARD BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES (ACI 530-88/ASCE 5-88) AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-88/ASCE 6-88)

NOTE:
ALL WIND LOADS ARE IN ACCORDANCE WITH SECTION 1609, FLORIDA BUILDING CODE, 2004 EDITION.

BASIC WIND SPEED	110 MPH
IMPORTANCE FACTOR	1.0
BUILDING CATEGORY	2
EXPOSURE	B
INTERNAL PRESSURE COEFFICIENT	0.0
COMPONENT AND CLADDING PRESSURE	WALLS +18.4/-25.8 PSF ROOF +10/-38.7 PSF OVERHANGS -71.6 PSF
TYPE OF STRUCTURE	ENCLOSED

BUILDING USE, CLASSIFICATION & OCCUPANCY AS PER TABLES 503 & 1004.1.2, FLORIDA BUILDING CODE, 2004 ED.

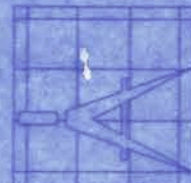
BUILDING GROUP OCCUPANCY	GROUP U
TABLE 500 TYPE OF CONSTRUCTION	TYPE V - UNPRO.
TABLE 500 AREA/HEIGHT LIMITATIONS	5.5 KSF/1 STORY
OCCUPANCY	
LOAD CAPACITY: 500/GROSS = 600 SF/500 = 1.2	2 OCCUPANTS

ALLOWABLE DEFLECTION OF STRUCTURAL MEMBERS

STRUCTURAL MEMBER	ALLOWABLE DEFLECTION
rafters having slopes greater than 2/12 with no finished ceiling attached to rafters	L/180
interior walls and partitions	H/180
floors and plastered ceilings	L/360
all other structural members	L/240
exterior walls with plaster or stucco finish	H/360
exterior walls - wind loads with brittle finishes	L/240
exterior walls - wind loads with flexible finishes	L/120

POLE BARN BUILDING FOR
WILSON'S AXLE REPAIR

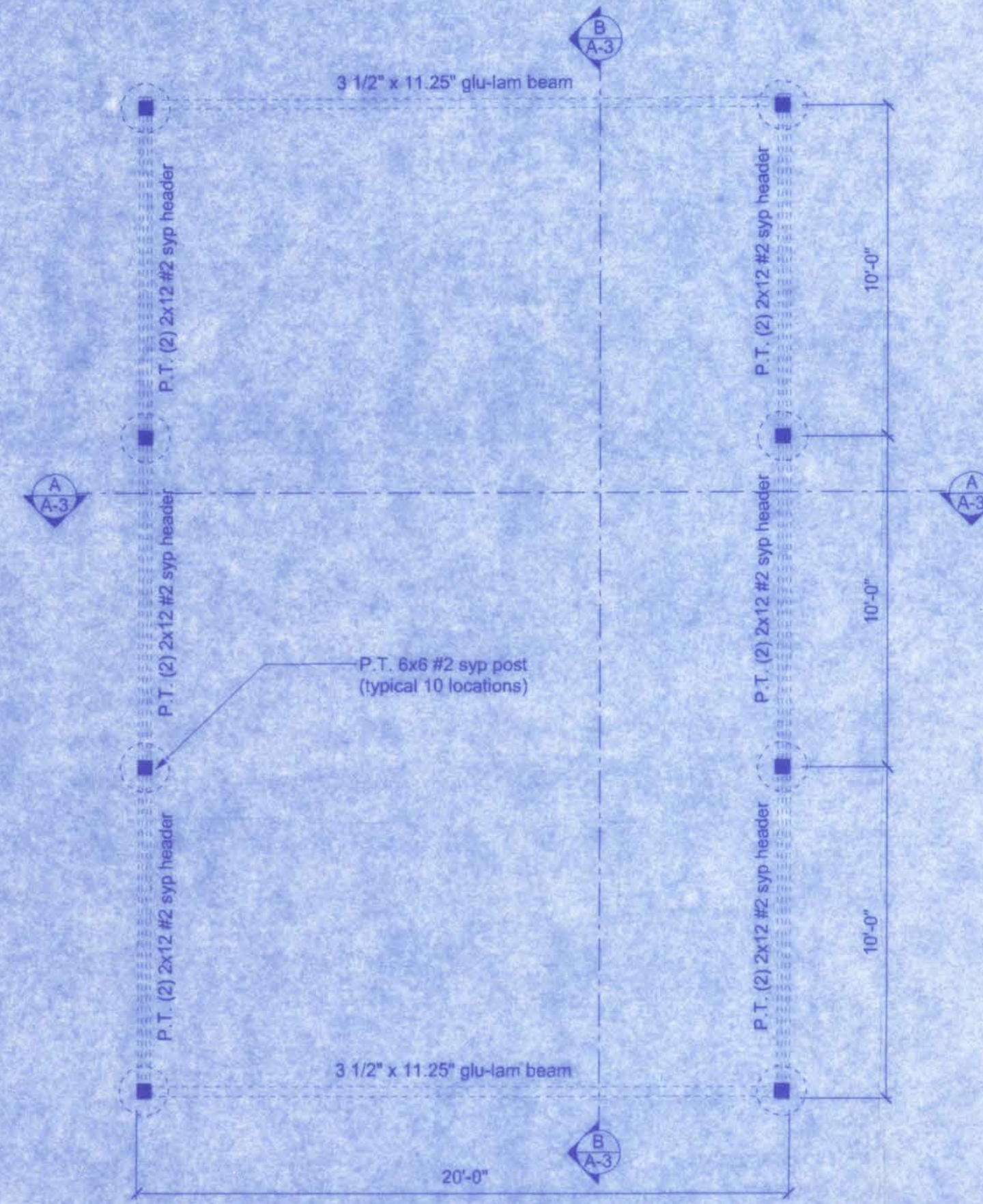
161 NW MADISON STREET
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LAKE CITY, FL 32055
(386) 758-4209



Freeman
Design Group Inc.

CERTIFICATE OF AUTHORIZATION # 00008701

DATE 10/04/08	DRAWN BY W.H.F.
REVISIONS	
SHEET A-1	
OF 3	
PROJECT NO. 06.C009	

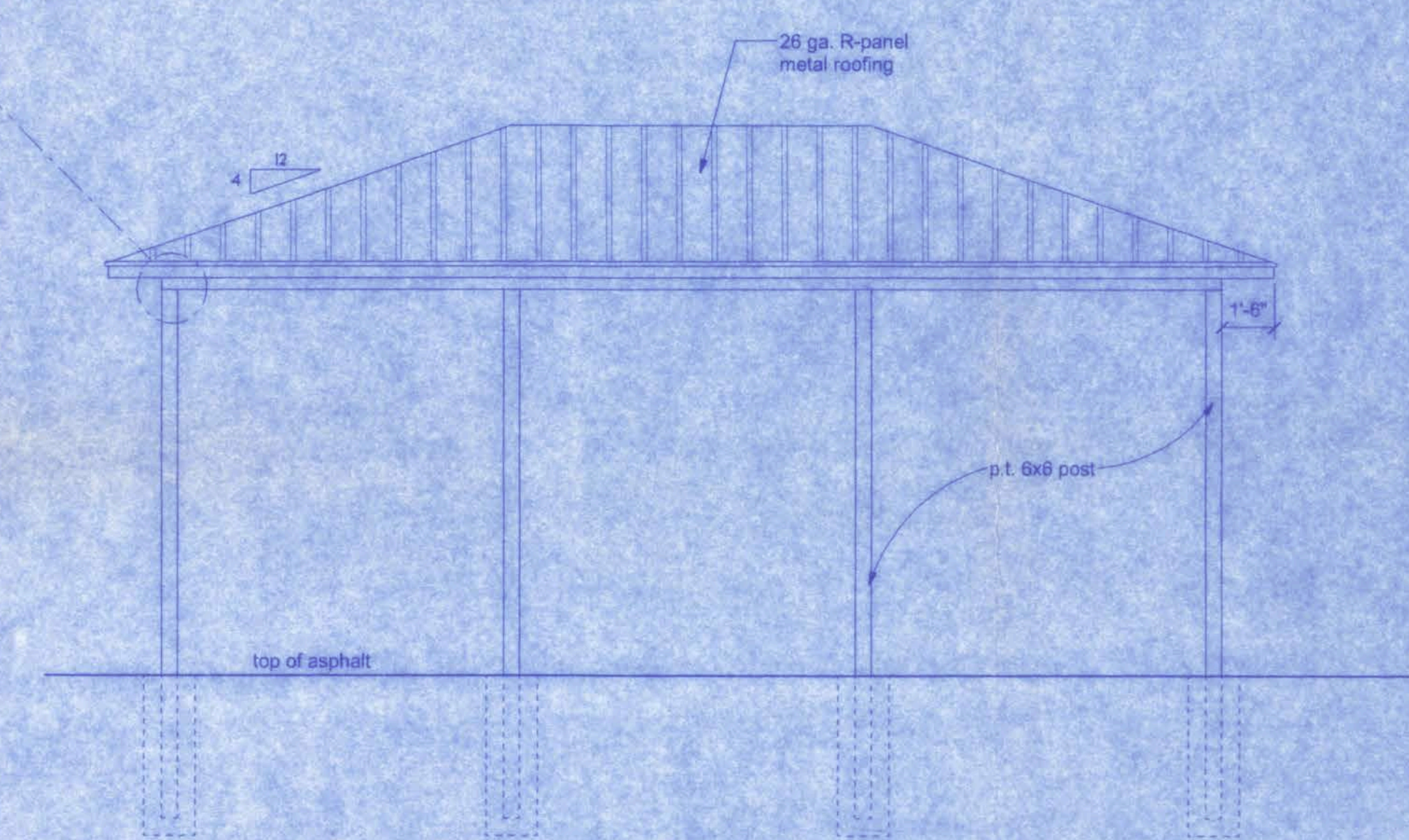
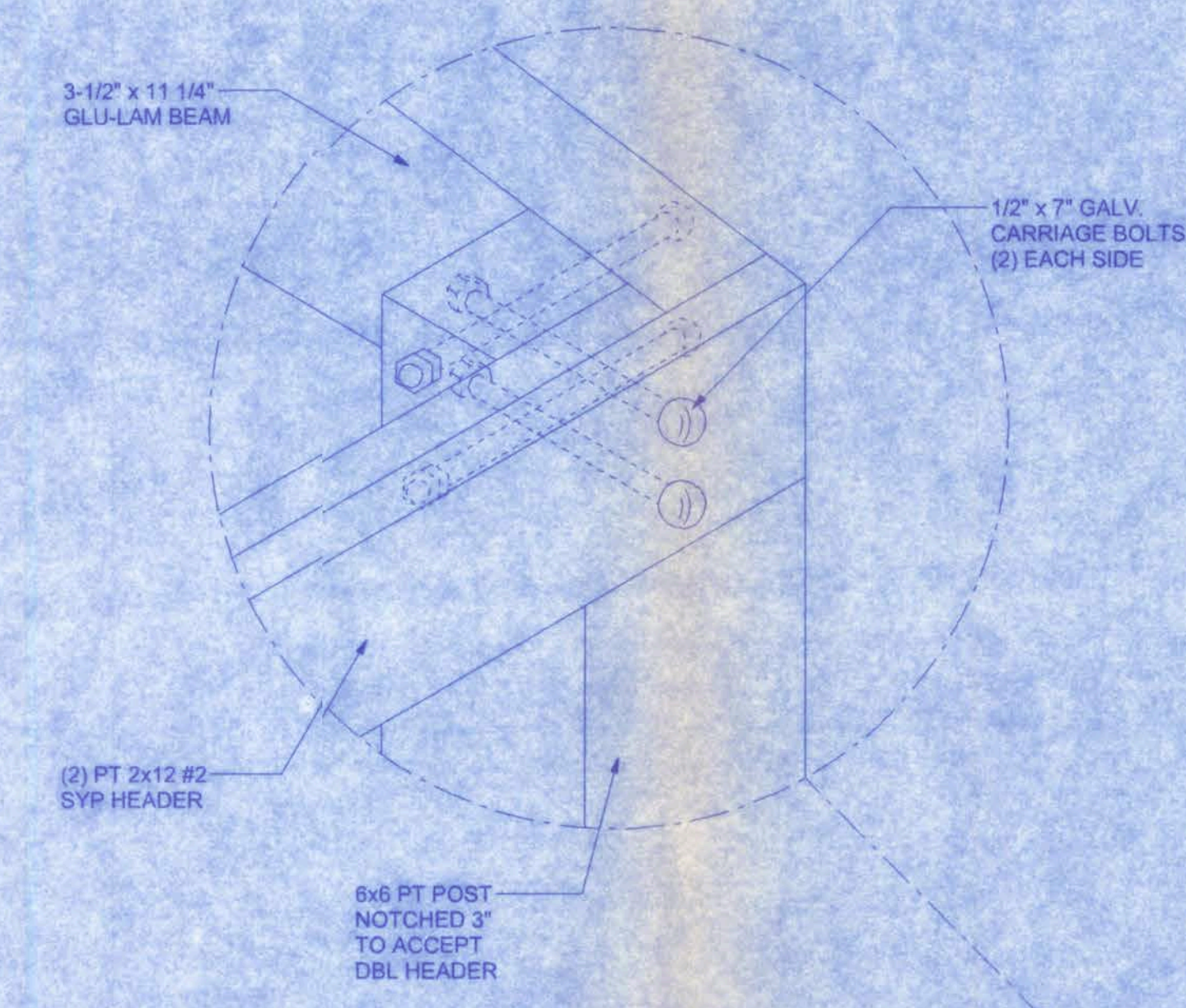


FLOOR PLAN LAYOUT
SCALE: 1/4" = 1'-0"

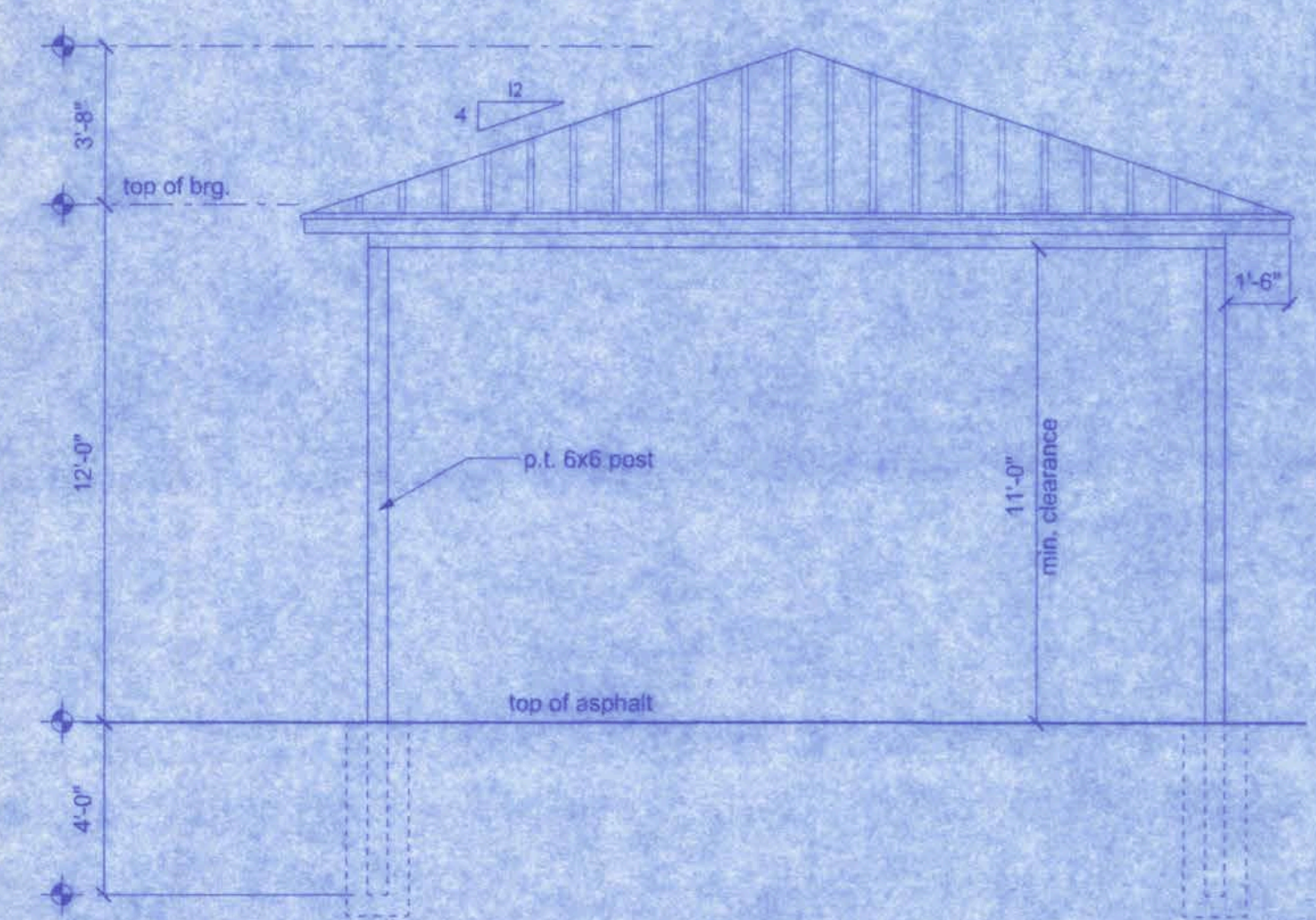
AREA SUMMARY	
UNDER ROOF AREA	600 SQ. FT.

- STEEL COATING RECOMMENDATIONS IN PRESSURE TREATED WOOD:**
- Thicker galvanizing generally extends service life of a product. The treated wood industry recommends use of Stainless Steel and hot-dip galvanized connectors and fasteners with treated wood.
 - Due to the uncertainties, which are out of the specifiers control, in regard to the chemicals used in pressure treated wood, Simpson recommends the use of stainless steel fasteners, anchors and connectors with treated wood when possible. At a minimum, customers should use ZMAX (G185 HDG per ASTM A653), Bath/Post Hot-Dip Galvanized (per ASTM A123 for connectors and ASTM A153 for fasteners), or mechanically galvanized fasteners (per ASTM B695, Class 55 or greater), product with the newer alternative treated woods.
 - G60 galvanized products should not be used with treated woods.
 - G60 galvanized connectors can be used with Sodium Borate (DOT - Disodium Octaborate Tetrahydrate) treated woods. Sodium Borate treated woods are not suitable for applications where moisture exposure is likely. They are suitable for mudsill applications when transported, stored, and installed appropriately.
 - When using stainless steel or hot-dip galvanized connectors, the connectors and fasteners should be made of the same material.

Simpson Strong-Tie Product Finishes	Untreated Wood	Chromated Copper Arsenate (CCA-C)	DOT Sodium Borate (SBX)	Alkaline Copper Quat ACO-C and ACO-D (Carbonate)	Copper Azole (CA-A and CA-B)	SBX (DOT) with NASIO	Ammoniacal Copper Zinc Arsenate (ACZA)	Other Pressure Treated Woods
Standard (G90)	X	X	X					
ZMAX (G185)	X	X	X	X	X	X		
Post Hot-Dip Galvanized (HDG)	X	X	X	X	X	X	X	X
SST300 (Stainless Steel)	X	X	X	X	X	X	X	X



SIDE ELEVATION
SCALE: 1/4" = 1'-0"



END ELEVATION
SCALE: 1/4" = 1'-0"

W. H. F.
10/14/06

**POLE BARN BUILDING FOR
WILSON'S AXLE REPAIR**

161 NW MADISON STREET
SUITE #102
LAKE CITY, FL 32055
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CERTIFICATE OF AUTHORIZATION # 00030701

Freeman
Design Group Inc.

DATE 10/04/06	DRAWN BY W.H.F.
REVISIONS	
SHEET A-2	OF 3
PROJECT NO. 06.C009	

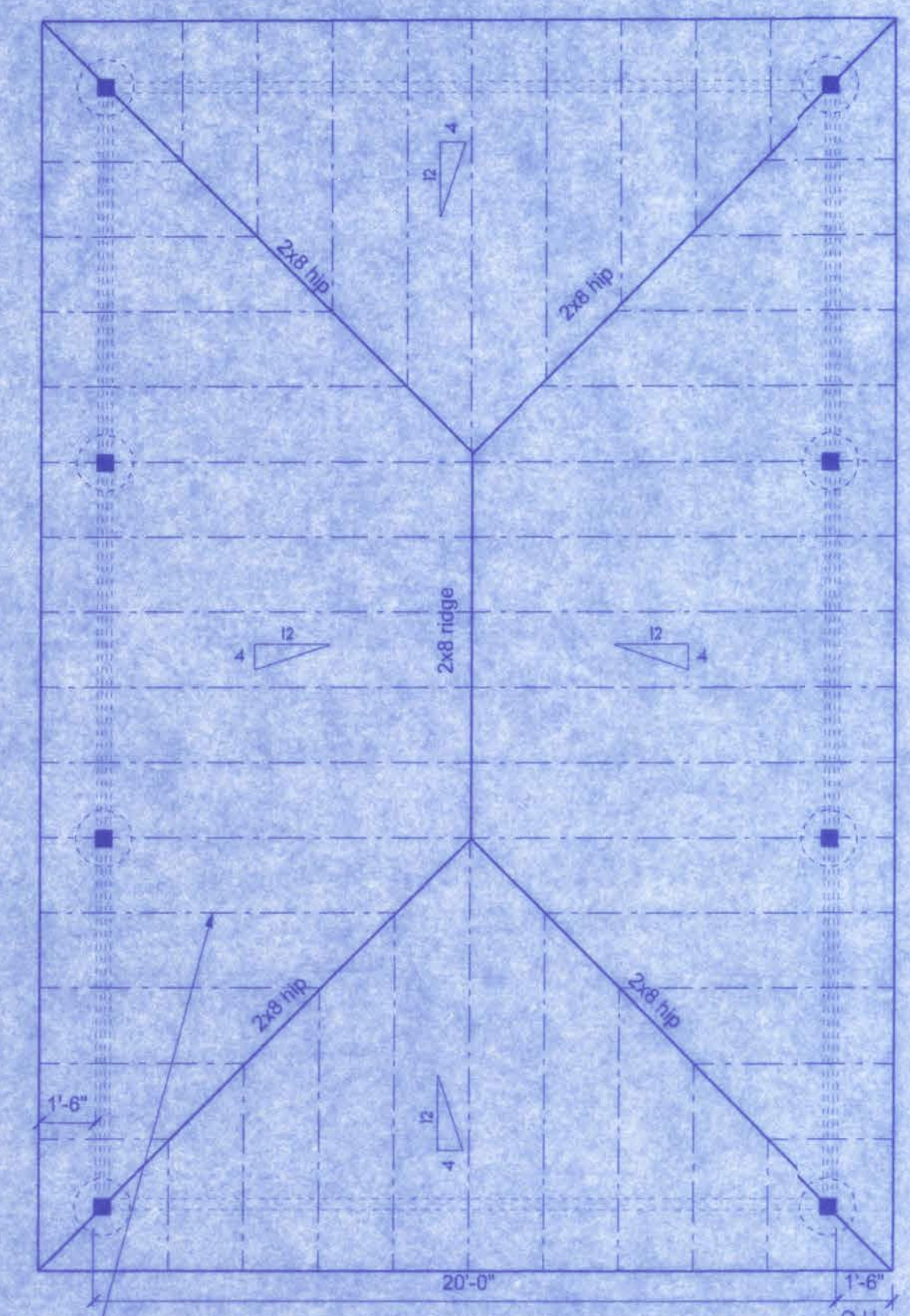
15/4/06

POLE BARN BUILDING FOR
WILSON'S AXLE REPAIR

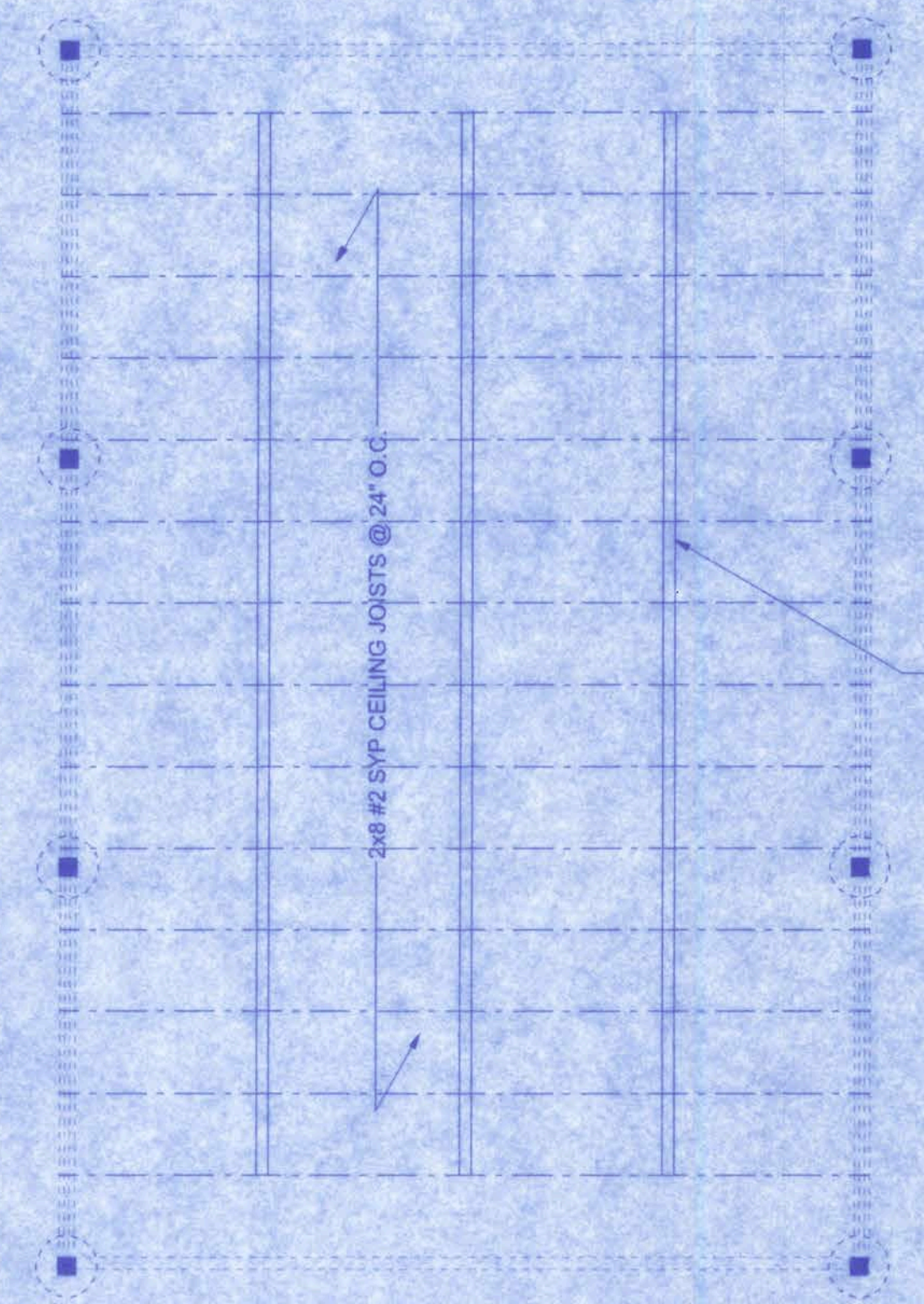
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DATE	DRAWN BY
10/04/06	W.H.F.
REVISIONS	
SHEET A-3	
OF 3	
PROJECT NO. 06.C009	

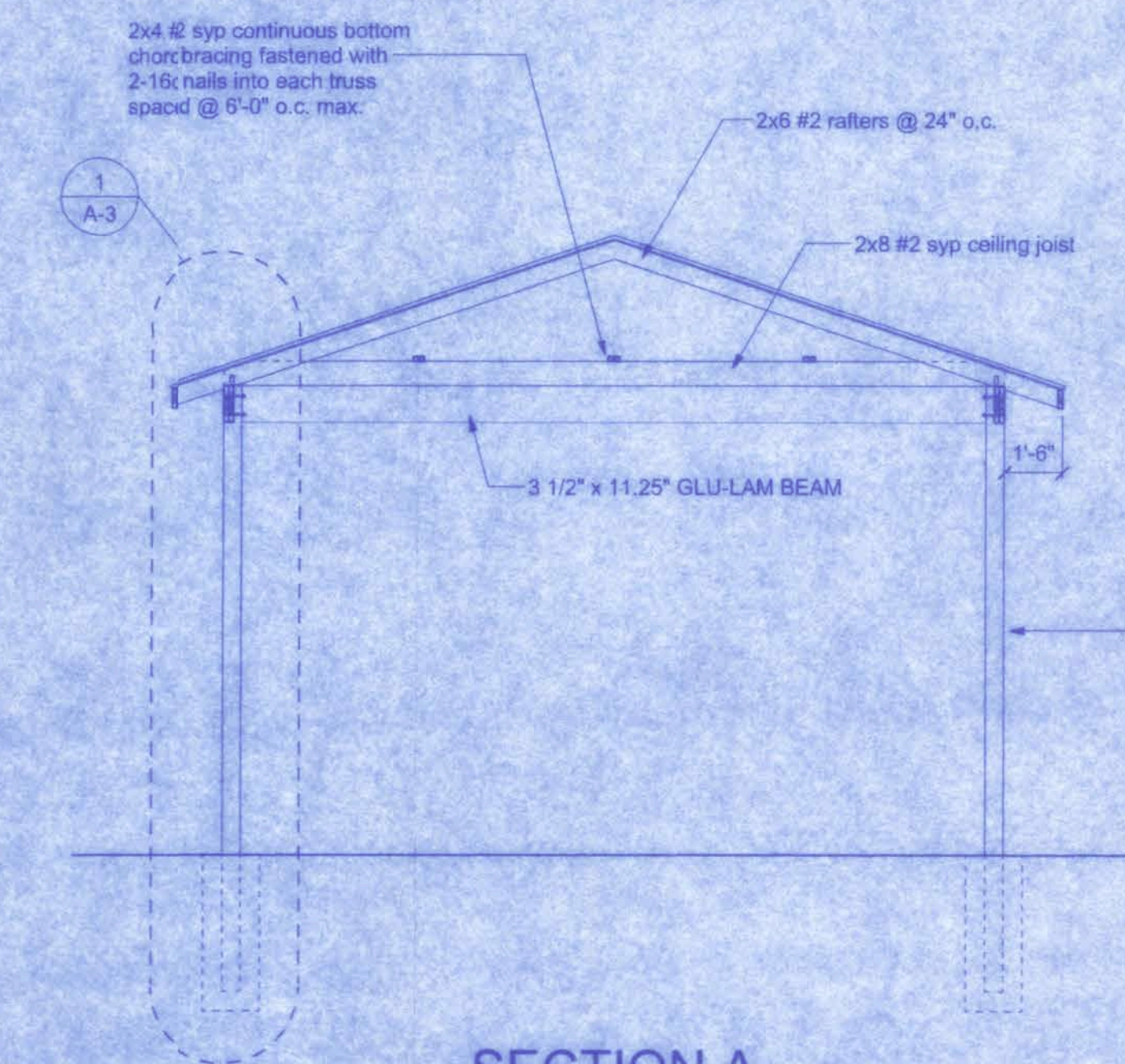


ROOF PLAN
SCALE: 1/4" = 1'-0"

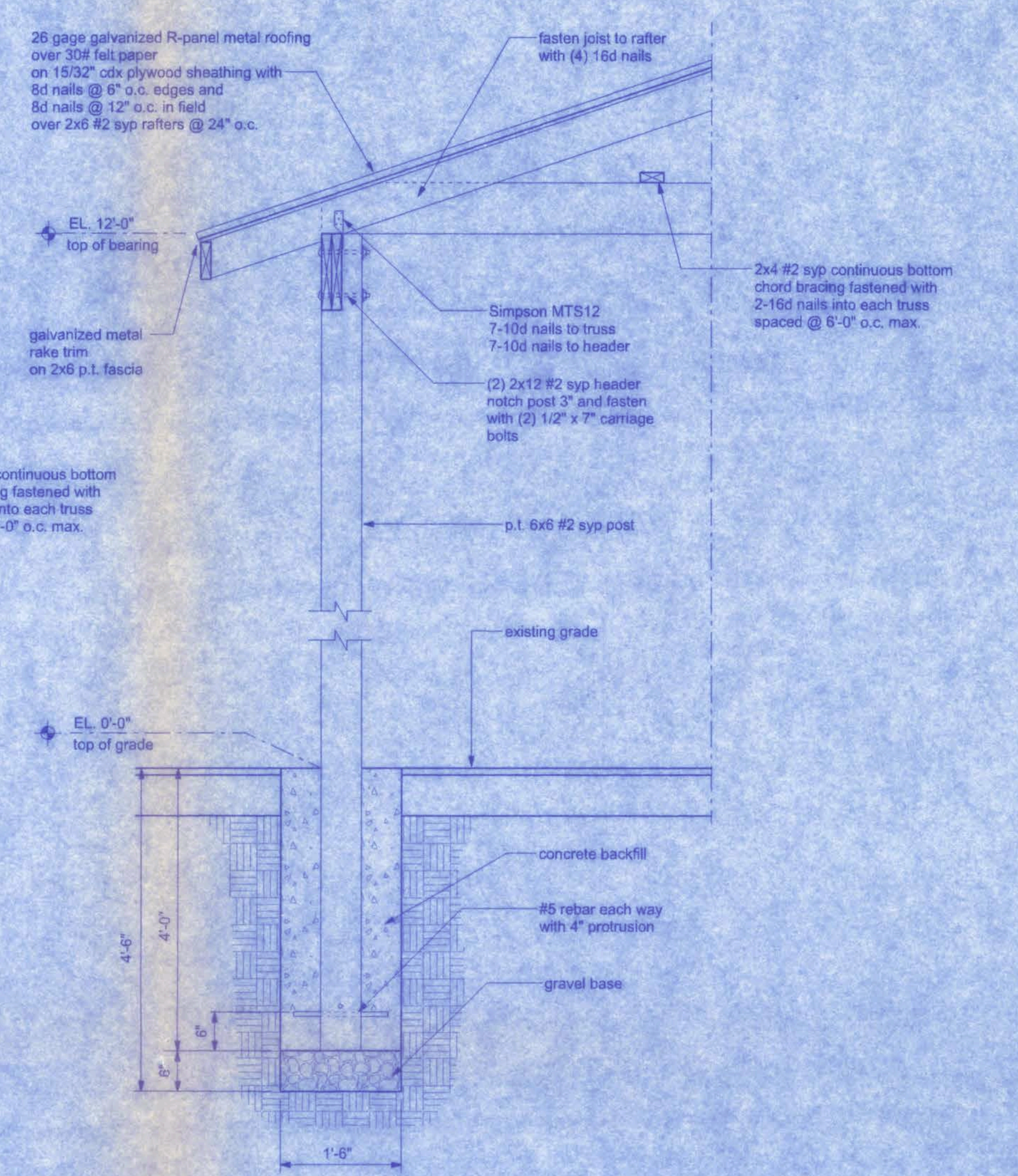


CEILING PLAN
SCALE: 1/4" = 1'-0"

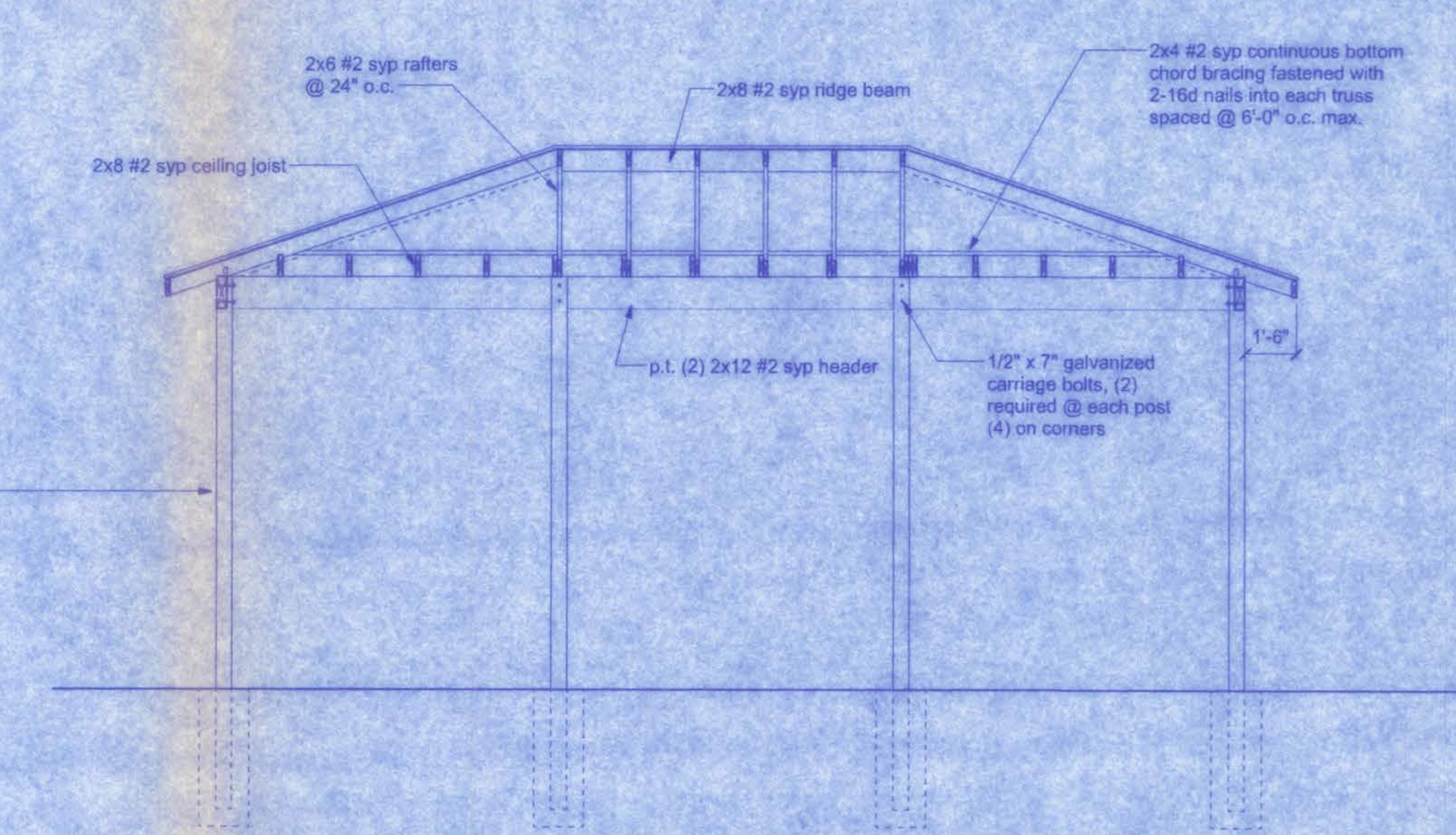
NOTE:
ALL FRAMING SHALL BE
#2 SYP OR BETTER



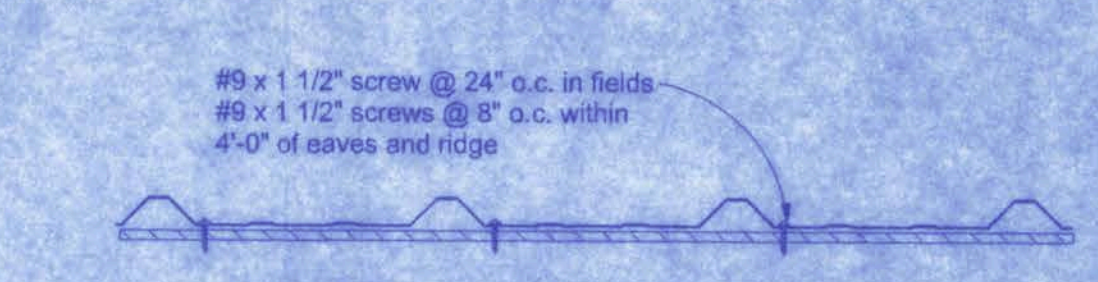
SECTION A
SCALE: 1/4" = 1'-0"



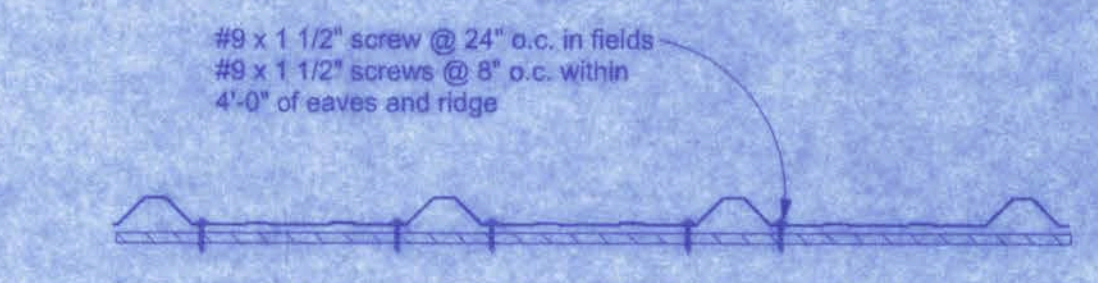
1 POST DETAIL
SCALE: 3/4" = 1'-0"



SECTION B
SCALE: 1/4" = 1'-0"



PBR AND R PANEL FASTENING SCHEDULE FOR ROOF FIELD
SCALE: 1 1/2" = 1'



PBR AND R PANEL FASTENING SCHEDULE AT EAVES AND VALLEYS
SCALE: 1 1/2" = 1'

ROOF FASTENING DETAIL
SCALE: 3/4" = 1'-0"