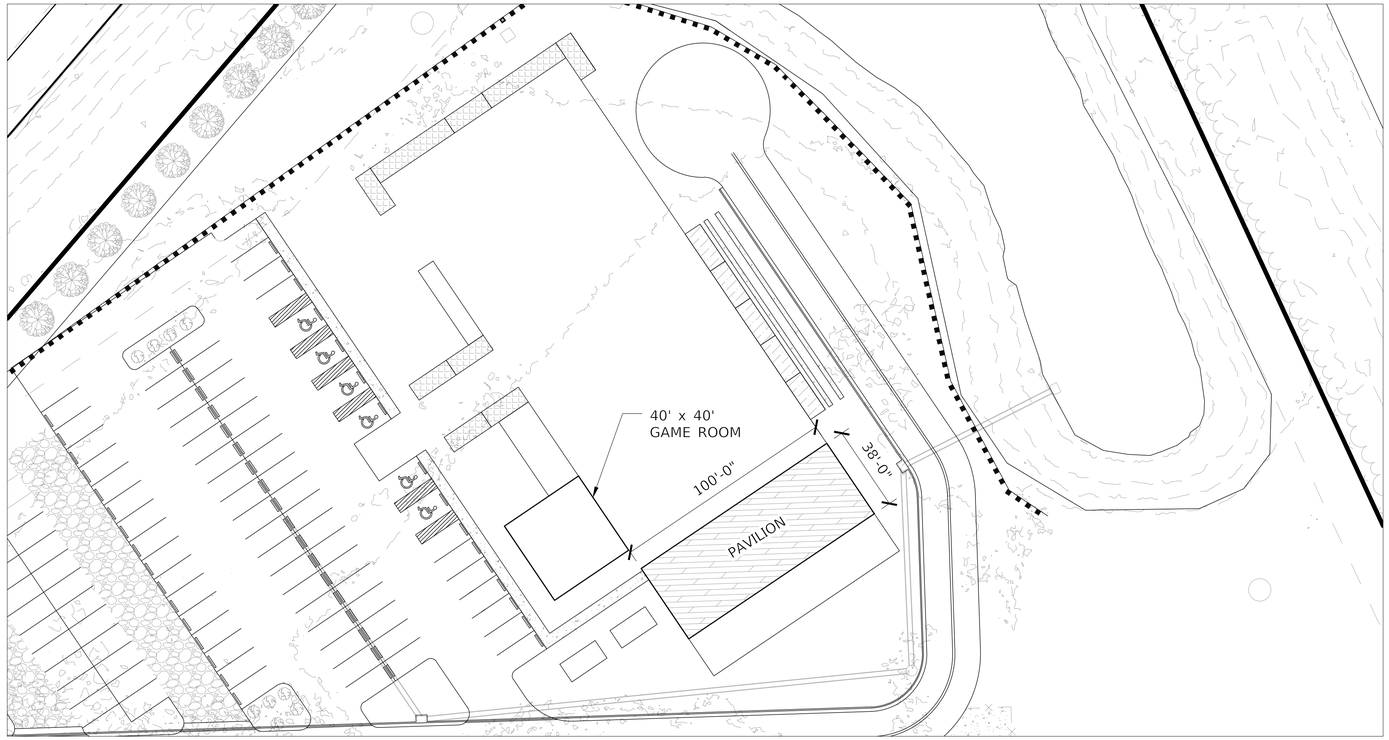
THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA



1 **KEY PLAN**
SCALE: 1/32" = 1'-0"

N

MICHAEL WOODS
520 STEEDLEY DRIVE,
LAKE CITY, FLORIDA
(386) 755-9314

REVISIONS	
DATE	DESCRIPTION

NORTH FLORIDA PROFESSIONAL SERVICES, INC.
P.O. BOX 3823
LAKE CITY, FL 32056
PH. 386-752-4675
LIC NO. LB8356
2551 BLAIRSTONE PINES DR.
TALLAHASSEE, FL 32301
WWW.NFPS.NET
CA# 29011

JOB NUMBER: L210802SPA
EOR: ROBERT P. BISHOP, JR.
P.E. NO.: 38546

COVER
THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO.
T1

D:\NFPS Projects\90324 L210802SPA The Woods Container Park\90324 L210802SPA The Woods Container Park.dwg T1 COVER

ROBERT P. BISHOP, JR.
LICENSE
No. 38546
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

THIS DOCUMENT HAS BEEN DIGITALLY SIGNED AND SEALED BY: ROBERT P. BISHOP, JR.

Robert P Bishop

Digitally signed by Robert P Bishop
DN: CN=Robert P Bishop, dnQualifier=A01410D000001872413ECF1000636D7, O=NORTH FLORIDA PROFESSIONAL SERVICES INC., C=US
Date: 2024.09.12 16:17:24-04'00'

ON THE DATE ADJACENT TO THE SEAL

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NORTH FLORIDA PROFESSIONAL SERVICES INC.
P.O. BOX 3823
LAKE CITY, FL 32056
CERTIFICATE OF AUTHORIZATION: 29011
ROBERT P. BISHOP, JR., P.E. NO. 38546

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

BUILDING SHEET LIST TABLE

SHEET NUMBER	SHEET TITLE
T1	COVER
SS1	SIGNATURE SHEET
B1	PAVILION FLOOR PLANS
B2	GAME ROOM FLOOR PLAN
B3	PAVILION ELEVATIONS
B4	GAME ROOM ELEVATIONS

DAVID MORGAN CRAPPS
LICENSE
No. 60989
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

THIS DOCUMENT HAS BEEN DIGITALLY SIGNED AND SEALED BY: DAVID MORGAN CRAPPS

David M Crapps
Digitally signed by David M Crapps
Date: 2024.09.12 13:16:26-04'00'

ON THE DATE ADJACENT TO THE SEAL

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STRUCTURAL SHEET LIST TABLE

SHEET NUMBER	SHEET TITLE
SS1	SIGNATURE SHEET
S1	STRUCTURAL NOTES
S2	PAVILION FOUNDATION PLAN
S3	GAME ROOM FOUNDATION PLAN
S4	STRUCTURAL SECTIONS AND DETAILS

R E V I S I O N S

DATE	DESCRIPTION

NORTH FLORIDA PROFESSIONAL SERVICES, INC.

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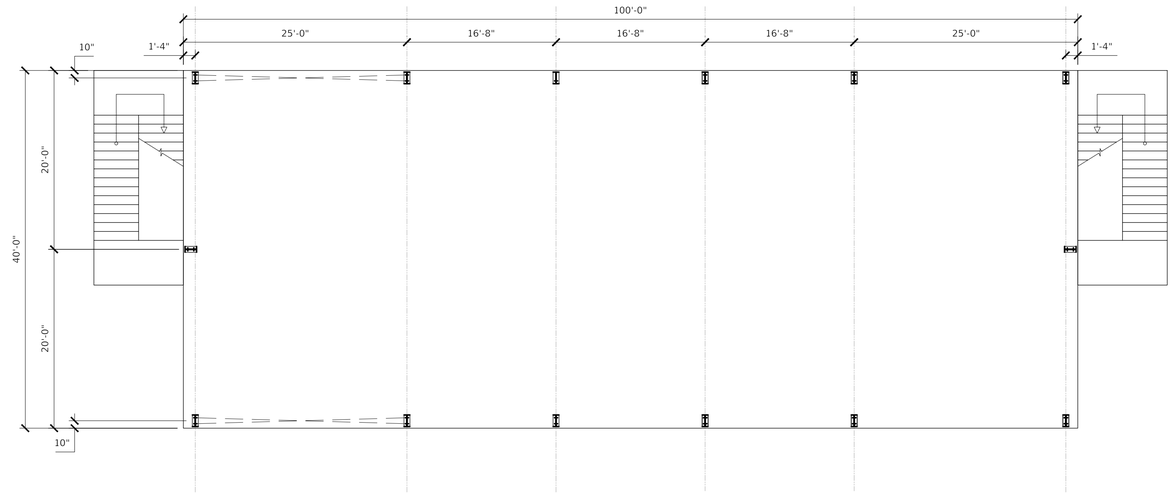
P.E. NO.: 38546

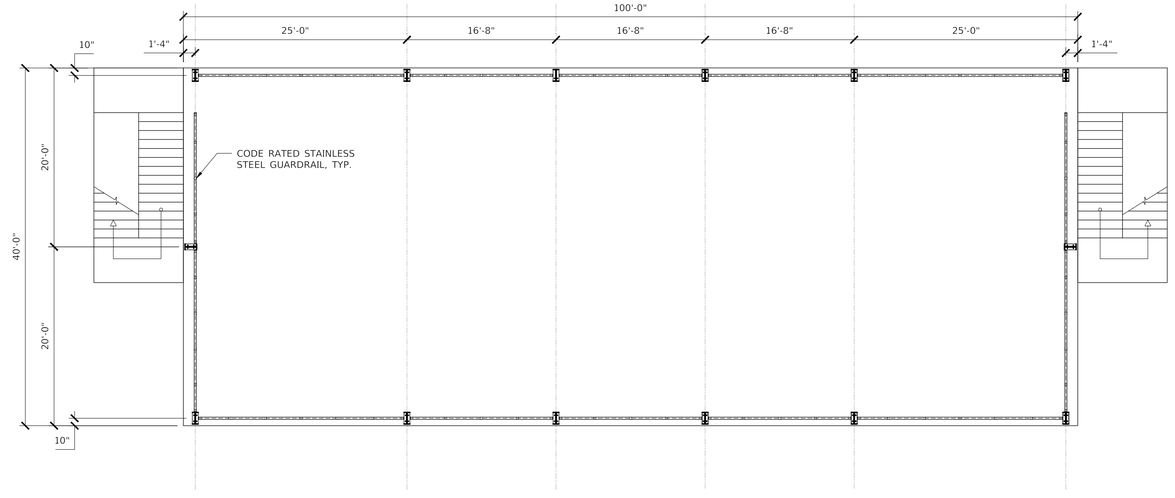
COVER
THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO. SS1



1 PAVILION FIRST FLOOR PLAN

SCALE: 1/8" = 1'-0"



2 PAVILION SECOND FLOOR PLAN

SCALE: 1/8" = 1'-0"

REVISIONS	
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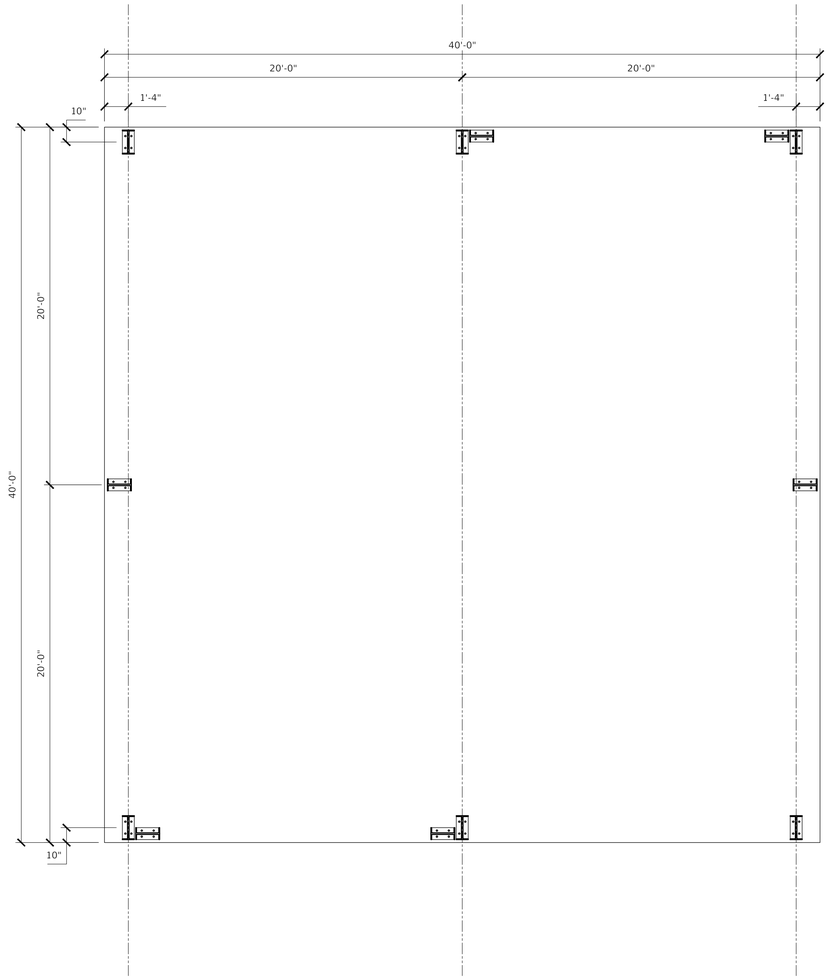
PAVILION FIRST FLOOR PLANS

THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO. B1

Joshua Galler 9/11/2024 9:14:55 AM D:\NFPS Projects\90324 L210802SPA The Woods Container Park\90324 L210802SPA The Woods Container Park.dwg B1 PAVILION FLOOR PLANS



1 GAME ROOM FLOOR PLAN

SCALE: 1/4" = 1'-0"

REVISIONS	
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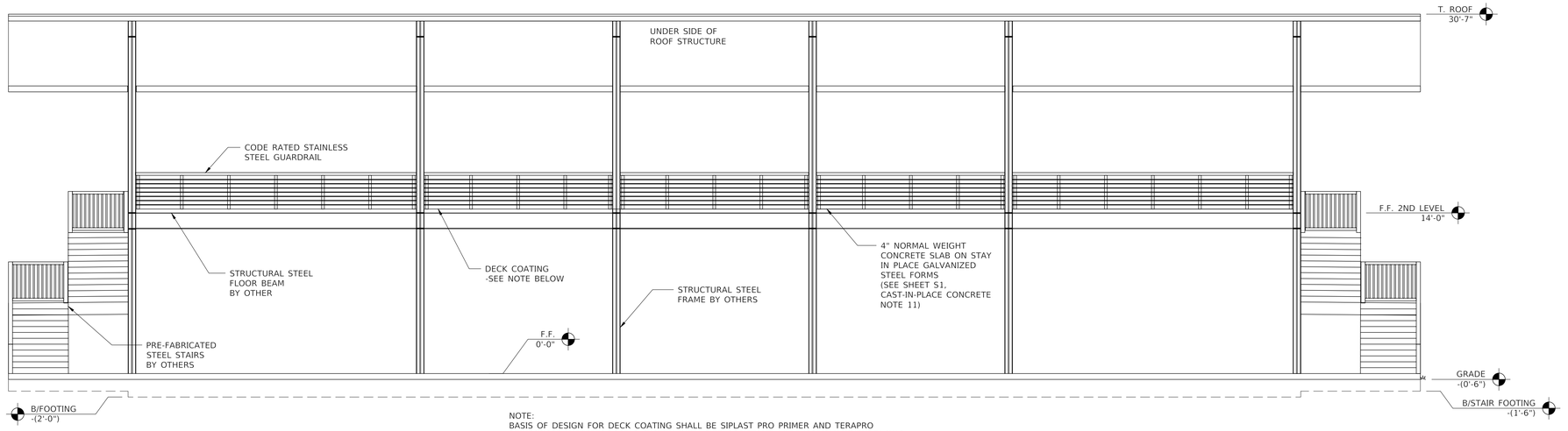
GAME ROOM FLOOR PLAN
THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO. B2

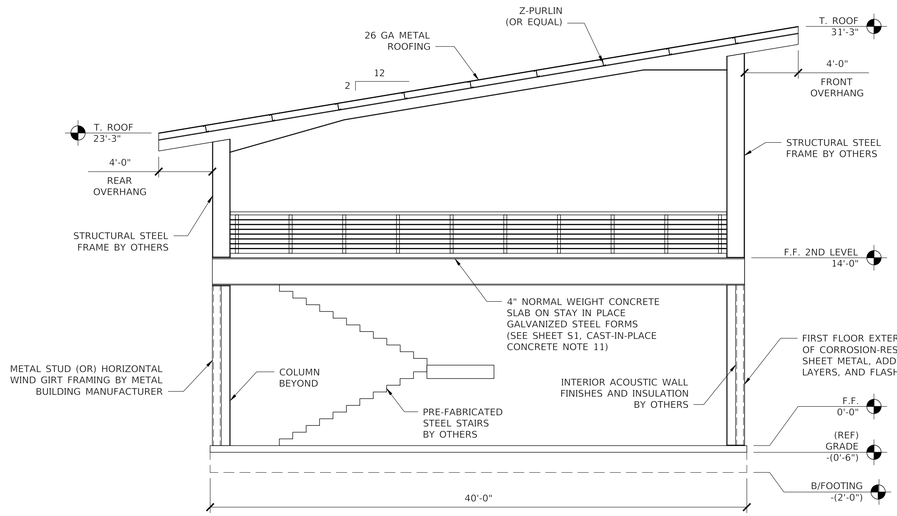
Joshua Galler 9/11/2024 9:15:37 AM D:\NFPS Projects\90324 L210802SPA The Woods Container Park\90324 L210802SPA The Woods Container Park.dwg B2 GAME ROOM FLOOR PLAN



NOTE:
BASIS OF DESIGN FOR DECK COATING SHALL BE SIPLAST PRO PRIMER AND TERAPRO REINFORCED WATERPROOFING SYSTEM WITH SKID RESISTANT / ADA COMPLIANT SURFACING. PROVIDE OWNER WITH 15-YEAR WARRANTY AND RECOMMENDED MAINTENANCE SCHEDULE.

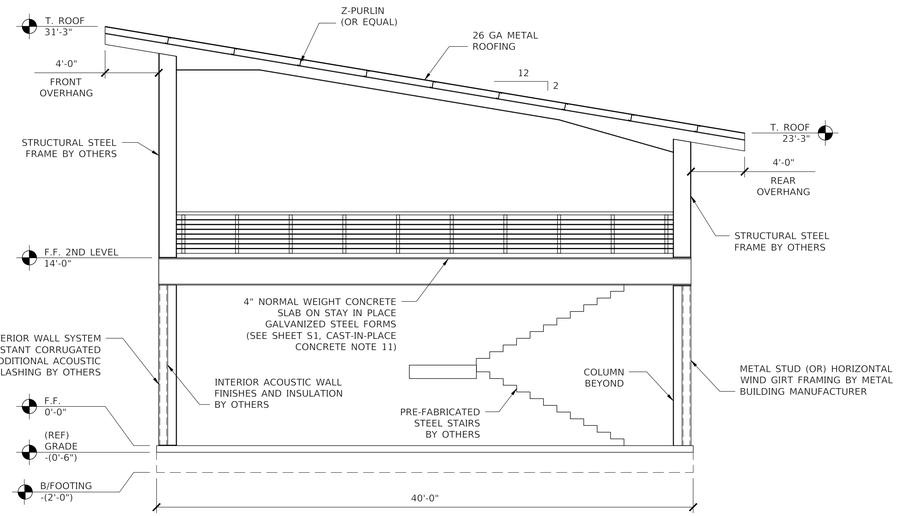
1 FRONT PAVILION ELEVATION

SCALE: 3/16" = 1'-0"



2 LEFT PAVILION ELEVATION

SCALE: 3/16" = 1'-0"



3 RIGHT PAVILION ELEVATION

SCALE: 3/16" = 1'-0"

REVISIONS	
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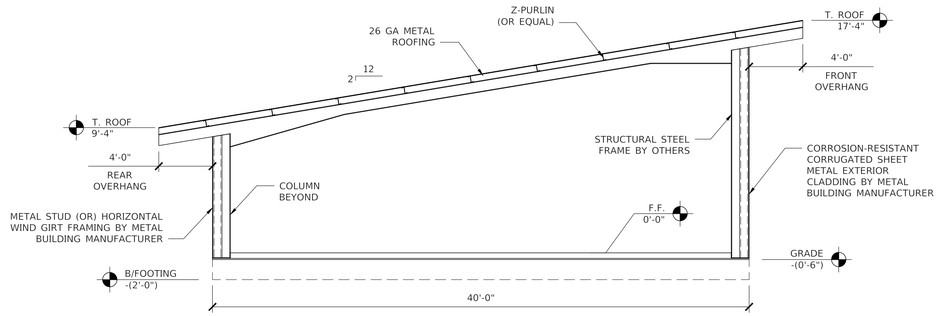
P.E. NO.: 38546

PAVILION ELEVATIONS

THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO. B3

Joshua Galler 9/11/2024 9:16:22 AM D:\NFPS Projects\90324 L210802SPA The Woods Container Park\90324 L210802SPA The Woods Container Park.dwg B3 PAVILION ELEVATIONS



1 SIDE GAME ROOM ELEVATION

SCALE: 3/16" = 1'-0"

REVISIONS	
DATE	DESCRIPTION

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P.E. NO.: 38546

GAME ROOM ELEVATIONS

THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO.: B4

GENERAL NOTES

1. AS USED IN THESE GENERAL NOTES:
   "DRAWINGS" MEANS THE LATEST STRUCTURAL DESIGN DRAWINGS, UON.
   "CONTRACT DOCUMENTS" IS DEFINED AS THE DESIGN DRAWINGS AND ALL GENERAL NOTES.
   "SER" IS DEFINED AS THE STRUCTURAL ENGINEER OF RECORD FOR THE STRUCTURE IN ITS FINAL CONDITION.
   "DESIGN PROFESSIONALS" IS DEFINED AS THE OWNER'S ARCHITECT AND SER.
   "MEP" INCLUDES, BUT IS NOT LIMITED TO MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION.
   "CONTRACTOR" IS DEFINED TO INCLUDE ANY OF THE FOLLOWING: GENERAL CONTRACTOR AND THEIR SUBCONTRACTORS, CONSTRUCTION MANAGER AND THEIR SUBCONTRACTORS.
2. THESE NOTES APPLY TO THE ENTIRE PROJECT UNLESS NOTED OTHERWISE IN THE CONTRACT DOCUMENTS.
3. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF THE STRUCTURAL WORK WITH THE ARCHITECTURAL, CIVIL, MEP CONTRACT DOCUMENTS, AS WELL AS ANY OTHER APPLICABLE TRADES.
4. THE CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF THE STRUCTURE UNTIL THE CONSTRUCTION OF THE STRUCTURE REACHES ITS FINAL CONDITION.
5. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, INSTALLATION, AND REMOVAL OF TEMPORARY BRACING AND CONSTRUCTION SUPPORTS, FOR NEW AND EXISTING STRUCTURES, AS NECESSARY TO COMPLETE THE PROJECT. NO PORTION OF THE PROJECT WHILE UNDER CONSTRUCTION IS INTENDED TO BE STABLE IN THE ABSENCE OF THE CONTRACTOR'S TEMPORARY SUPPORTS AND BRACES. CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED TO DESIGN TEMPORARY BRACING AND CONSTRUCTION SUPPORTS.
6. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE GENERAL NOTES. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.
7. THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS AND COORDINATE WITH THE STRUCTURAL DRAWINGS, ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER CONSULTANTS, PROJECT SHOP DRAWINGS AND FIELD CONDITIONS.
8. IN CASES OF CONFLICT BETWEEN DRAWINGS AND OTHER DISCIPLINES OR EXISTING CONDITIONS, CONTRACTOR SHALL NOTIFY THE DESIGN PROFESSIONALS AND OBTAIN CLARIFICATION PRIOR TO BIDDING AND PROCEEDING WITH WORK.
9. APPLY DETAILS, SECTIONS, AND NOTES ON THE DRAWINGS WHERE CONDITIONS ARE SIMILAR TO THOSE INDICATED BY DETAIL, DETAIL TITLE OR NOTE.
10. ONLY USE DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT SCALE DRAWINGS. ASSUME EQUAL SPACING BETWEEN ESTABLISHED DIMENSIONS, IF NOT INDICATED ON DRAWINGS.
11. CENTERLINES OF FRAMING MEMBERS COINCIDE WITH COLUMN CENTERLINES, UON.
12. THE CONTRACTOR SHALL VERIFY THAT CONSTRUCTION LOADS DO NOT EXCEED THE CAPACITY OF THE STRUCTURE AT THE TIME THE LOAD IS APPLIED.
13. THE CONTRACTOR SHALL VERIFY ALL OPENING SIZES AND LOCATIONS WITH OTHER DISCIPLINES. THE DRAWINGS DO NOT SHOW ALL OPENINGS REQUIRED. ADDITIONAL OPENINGS, BLOCKOUTS AND SLEEVES MAY BE REQUIRED BY OTHER DISCIPLINES AND SHALL BE CONSTRUCTED USING THE TYPICAL DETAILS AND/OR THE CRITERIA INDICATED ON THE DRAWINGS. OPENINGS REQUIRED BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS MUST BE APPROVED BY THE STRUCTURAL ENGINEER.
14. DO NOT CUT OR MODIFY STRUCTURAL MEMBERS FOR PIPES, DUCTS, ETC, UNLESS SPECIFICALLY DETAILED OR APPROVED IN WRITING BY THE ENGINEER.
15. ELEVATIONS INDICATED ON STRUCTURAL DRAWINGS ARE BASED ON A PROJECT DATUM INDICATED ON THE CIVIL DRAWINGS.

CODES AND DESIGN CRITERIA

1. PERFORM ALL CONSTRUCTION IN CONFORMANCE WITH THE BUILDING AND DESIGN CODES REFERENCED WITHIN THESE DOCUMENTS. THE PROJECT DOCUMENTS REFER TO THE FOLLOWING CODES AND STANDARDS, UON:
2. AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7-2022: "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES"
   2023 FLORIDA BUILDING CODE WITH AMENDMENTS
   STRUCTURAL CONCRETE:
   "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
   AMERICAN CONCRETE INSTITUTE (AC 318-19)
   STRUCTURAL STEEL:
   STEEL CONSTRUCTION MINIMUM - FIFTEENTH EDITION BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-16)
   CONCRETE MASONRY:
   "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
   THE MASONRY SOCIETY (TMS 402/602-16)
3. LIVE LOADS:

ROOF	20 PSF (REDUCIBLE PER CODE)
SECOND FLOOR DECK	100 PSF (NON-REDUCIBLE)
STAIRS	100 PSF (NON-REDUCIBLE)

4. SUPERIMPOSED DEAD LOADS:

COLLATERAL (ROOF)	5 PSF
COLLATERAL (SECOND FLOOR)	15 PSF

5. WIND LOAD DESIGN DATA:
   WIND LOADS SHALL BE IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE (REFERENCING ASCE 7-22).
   MAIN WIND FORCE RESISTING SYSTEM
   WIND DESIGN DATA:

a. ULTIMATE DESIGN WIND SPEED, 3 SECOND GUSTS, VULT.	120 MPH
b. HURRICANE PRONE REGION	YES
c. WINDBORNE DEBRIS REGION	NO
d. BUILDING RISK CATEGORY	II
e. WIND EXPOSURE CATEGORY	C
f. WIND TOPOGRAPHIC FACTOR (KZT)	1.0
g. GROUND ELEVATION FACTOR	1.0
h. ENCLOSURE CATEGORY	PARTIALLY OPEN @ GAME ROOM; PARTIALLY ENCLOSED @ PAVILION
i. INTERNAL PRESSURE COEFFICIENT	± 0.18 @ GAME ROOM; ± 0.55 @ PAVILION
j. MEAN ROOF HEIGHT (H)	15 FEET @ GAME ROOM; 28 FEET @ PAVILION
k. WIND DIRECTIONALITY FACTOR, KD	0.85
l. VELOCITY PRESSURE COEFFICIENT (KH)	0.85 @ GAME ROOM; 0.96 @ PAVILION
m. ULTIMATE VELOCITY PRESSURE (QH[ULT])	31.3 PSF @ GAME ROOM; 35.5 PSF @ PAVILION
n. COMPONENT & CLADDING WIND PRESSURES:	SEE TABLES THIS SHEET
o. DIMENSION A	4 FEET

6. RAIN LOADS: DESIGN RAIN LOAD INTENSITY IS 4.5 INCHES PER HOUR
7. IN CASES WHERE THE CONTRACTOR DETERMINES THAT SUSPENDED OR FLOOR MOUNTED MEP EQUIPMENT LOADS EXIST WHICH EXCEED DESIGN LOADS INDICATED ON CONTRACT DOCUMENTS, CONTRACTOR SHALL SUBMIT LOAD DATA TO DESIGN PROFESSIONALS FOR REVIEW PRIOR TO PROCEEDING WITH WORK.
8. DISTRIBUTE THE MAXIMUM LOAD HUNG FROM ANY STRUCTURAL MEMBER FOR MEP DUCTWORK,PIPING ETC OVER THE MEMBER'S TRIBUTARY AREA IN A WAY THAT THE DESIGN SUPERIMPOSED DEAD LOADS LISTED IN CONTRACT DOCUMENTS ARE NOT EXCEEDED. THE CONTRACTOR SHALL COORDINATE THE LOADS OF ALL TRADES AND PROVIDE ADDITIONAL SUPPORT OR DISTRIBUTION FRAMING AS REQUIRED TO ACHIEVE THE ALLOWABLE LOAD DISTRIBUTION.
9. STRUCTURAL COMPONENTS ARE NOT DESIGNED FOR VIBRATING EQUIPMENT. MOUNT VIBRATING EQUIPMENT ON VIBRATION ISOLATORS.
10. CONNECTIONS OF SYSTEMS DESIGNED BY CONTRACTOR'S ENGINEER SUCH AS, BUT NOT LIMITED TO, CLADDING, STAIRS, ELEVATORS, AND MEP LOADS ARE ASSUMED TO IMPOSE VERTICAL AND/OR HORIZONTAL LOADS ON THE BASE BUILDING STRUCTURAL MEMBERS WITHOUT GENERATING TORSION IN THE SUPPORTING STRUCTURAL MEMBERS. CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING ALL SUPPLEMENTARY BRACING MEMBERS AS REQUIRED TO PREVENT TORSION ON THE BASE BUILDING STRUCTURE.

SUBMITTALS

1. 10 WORKING DAYS PRIOR TO SUBMITTING SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT FOR STRUCTURAL ENGINEER'S REVIEW A SCHEDULE WHICH DETAILS THE ESTIMATED QUANTITY OF SHOP DRAWINGS AND THE DATE THE SHOP DRAWINGS WILL BE RECEIVED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER SHALL HAVE THE OPPORTUNITY TO REVIEW THE PROPOSED SCHEDULE AND SUBMIT COMMENTS TO THE CONTRACTOR. THE FINAL SHOP DRAWING SCHEDULE SHALL BE DEVELOPED AND SUBMITTED TO THE STRUCTURAL ENGINEER. IN ACCORDANCE WITH THE SHOP DRAWING SCHEDULE, THE STRUCTURAL ENGINEER WILL RETURN THE SHOP DRAWING ITEMS WITHIN TEN WORKING DAYS AFTER HAVING RECEIVED THE REPRODUCIBLE SHOP DRAWING.
2. THE CONTRACTOR SHALL REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO ARCHITECT AND STRUCTURAL ENGINEER AND SHALL STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING IS ADDRESSED:
   a. THE SHOP DRAWING IS REQUESTED.
   b. THE SHOP DRAWING IS BASED ON THE LATEST DESIGN.
   c. THE ARCHITECT'S AND STRUCTURAL ENGINEER'S COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
   d. THE WORK IS COORDINATED AMONG ALL CONSTRUCTION TRADES.
   e. REVISIONS FROM PREVIOUS SUBMITTALS ARE CLEARLY MARKED BY CIRCLING OR CLOUDS.
   f. SUBMITTAL IS COMPLETE.
   g. SUBMITTAL DOES NOT INCLUDE SUBSTITUTION REQUEST.
   h. SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL, SPECIFICATION SECTION NUMBER.
3. THE STRUCTURAL ENGINEER SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS. THE STRUCTURAL ENGINEER'S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT. NO WORK SHALL BE STARTED WITHOUT SUCH REVIEW.
4. FOR COMPONENTS THAT REQUIRE ENGINEERING BY THE CONTRACTOR, PROVIDE A NOTE ON EACH SHOP DRAWING, WRITTEN AND SIGNED BY THE SUPPLIER'S ENGINEER, INDICATING THAT THE SHOP DRAWING IS IN CONFORMANCE WITH THE CALCULATIONS OF THE CONTRACTOR'S. THE FOLLOWING ITEMS REQUIRE SUBMITTALS FOR STRUCTURAL REVIEW AS OUTLINED IN THE SPECIFICATIONS:

S	CALC	PRE-ENGINEERED METAL BUILDINGS, WITH COLUMN REACTIONS
S		CONCRETE REINFORCING LAYOUT
S		CONCRETE MIX DESIGNS
S		STAINLESS STEEL CABLE GUARDRAIL

   S = SHOP DRAWINGS REQUIRED
   CALC = SUPPORTING CALCULATIONS REQUIRED, SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED.
5. SUBMITTAL FOR SPECIAL STRUCTURAL, LOAD-CARRYING ITEMS THAT ARE REQUIRED BY CODES OR STANDARDS TO RESIST FORCES MUST BE PREPARED BY, OR UNDER THE DIRECT SUPERVISION OF, A DELEGATED ENGINEER. EXAMPLES INCLUDE BUT ARE NOT LIMITED TO, STRUCTURAL LIGHT GAGE STEEL FRAMING, EXTERIOR ENCLOSURE SYSTEMS, STEEL STAIRS, PRECAST CONCRETE PILES.
6. A DELEGATED ENGINEER IS DEFINED AS A FLORIDA LICENSED ENGINEER WHO SPECIALIZES IN AND UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS OR STRUCTURAL SYSTEMS INCLUDED IN A SPECIFIC SUBMITTAL PREPARED FOR THIS PROJECT AND IS AN EMPLOYEE OR OFFICER OF, OR CONSULTANT TO, THE CONTRACTOR OR FABRICATOR RESPONSIBLE FOR THE SUBMITTAL. THE DELEGATED ENGINEER SHALL SIGN, SEAL, AND DATE THE SUBMITTAL, INCLUDING CALCULATIONS AND DRAWINGS.

STRUCTURAL TESTING/INSPECTION AGENCY SERVICES

1. NEITHER THE OBSERVATION OF THE ARCHITECT/STRUCTURAL ENGINEER IN THE ADMINISTRATION OF THE CONTRACT, NOR TESTS/INSPECTIONS BY THE TESTING/INSPECTION AGENCY, NOR APPROVALS BY PERSONS OTHER THAN THE ARCHITECT/STRUCTURAL ENGINEER SHALL RELIEVE THE CONTRACTOR FROM HIS OBLIGATION TO PERFORM THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
2. OWNER WILL EMPLOY AND PAY FOR THE STRUCTURAL TESTING/INSPECTION SERVICES THAT ARE REQUIRED BY THE CONTRACT DOCUMENTS.
3. CONTRACTOR SHALL PAY FOR ANY ADDITIONAL STRUCTURAL TESTING/INSPECTION REQUIRED FOR WORK OR MATERIALS NOT COMPLYING WITH CONTRACT DOCUMENTS DUE TO NEGLIGENCE OR NONCONFORMANCE.
4. CONTRACTOR SHALL PAY FOR ANY ADDITIONAL STRUCTURAL TESTING/INSPECTION REQUIRED FOR HIS CONVENIENCE.
5. REFER TO THE OTHER GENERAL NOTES SECTIONS FOR STRUCTURAL TESTING/ INSPECTION REQUIREMENTS.
6. STRUCTURAL TESTING/INSPECTION AGENCY'S QUALIFICATIONS
   a. PROVIDE PERSONNEL WITH A MINIMUM OF TWO YEARS' EXPERIENCE AND QUALIFIED TO PERFORM THE STRUCTURAL TESTING/INSPECTION REQUIRED BY THE CONTRACT DOCUMENTS.
   b. COMPLY WITH THE AMERICAN COUNCIL OF INDEPENDENT LABORATORIES' RECOMMENDED REQUIREMENTS.
   c. COMPLY WITH ASTM E329.
   d. MAINTAIN PROPERLY CALIBRATED EQUIPMENT; CALIBRATED WITHIN THE PAST 12 MONTHS WITH DEVICES OF ACCURACY TRACEABLE TO EITHER NATIONAL BUREAU OF STANDARDS (NBS) STANDARDS OR ACCEPTED VALUES OF NATURAL PHYSICAL CONSTANTS.
7. STRUCTURAL TESTING/INSPECTION AGENCY'S RESPONSIBILITIES
   a. COOPERATE WITH THE CONTRACTOR AND PROVIDE TIMELY SERVICE.
   b. UPON ARRIVING AT THE CONSTRUCTION SITE, SIGN IN AND NOTIFY THE CONTRACTOR OF PRESENCE.
   c. SELECT THE REPRESENTATIVE SAMPLES THAT ARE TO BE TESTED/INSPECTED.
   d. PERFORM TESTS/INSPECTIONS AS OUTLINED IN CONTRACT DOCUMENTS, THE APPLICABLE CODES, AND AS DIRECTED BY THE STRUCTURAL ENGINEER.
   e. REPORT WORK AND MATERIALS NOT COMPLYING WITH CONTRACT DOCUMENTS IMMEDIATELY TO THE CONTRACTOR AND STRUCTURAL ENGINEER.
   f. LEAVE COPIES OF FIELD NOTES WITH THE CONTRACTOR PRIOR TO LEAVING THE CONSTRUCTION SITE. FIELD NOTES SHALL INCLUDE THE MESSAGE GIVEN TO THE CONTRACTOR, DATE, TIME OF MESSAGE, NAME OF CONTRACTOR'S REPRESENTATIVE INFORMED, TYPE AND LOCATION OF WORK OR MATERIALS TESTED/INSPECTED, WHETHER THE WORK OR MATERIALS COMPLIES WITH CONTRACT DOCUMENTS AND NAME OF THE STRUCTURAL TESTING/INSPECTION AGENCY'S REPRESENTATIVE.
   g. REPORT AND DISTRIBUTE RESULTS OF TESTS/INSPECTIONS PROMPTLY IN THE FORM OF WRITTEN REPORTS. COPIES OF THE REPORTS FOR THIS PROJECT WILL BE FURNISHED TO THE OWNER, CONTRACTOR, ARCHITECT, STRUCTURAL ENGINEER, AND THE LOCAL BUILDING AUTHORITIES.
   h. STRUCTURAL TESTING/INSPECTION AGENCY SHALL NOT ALTER REQUIREMENTS OF CONTRACT DOCUMENTS, APPROVE OR REJECT ANY PORTION OF THE WORK, OR PERFORM DUTIES OF THE CONTRACTOR.
8. CONTRACTOR'S RESPONSIBILITIES
   a. PROVIDE COPY OF CONTRACT DOCUMENTS TO THE STRUCTURAL TESTING/INSPECTION AGENCY.
   b. ARRANGE THE PRECONSTRUCTION MEETING TO DISCUSS QUALITY ISSUES.
   c. NOTIFY THE STRUCTURAL TESTING/INSPECTION AGENCY SUFFICIENTLY IN ADVANCE OF OPERATIONS TO ALLOW ASSIGNMENT OF PERSONNEL AND SCHEDULING OF TESTS.
   d. COOPERATE WITH STRUCTURAL TESTING/INSPECTION AGENCY AND PROVIDE ACCESS TO WORK.
   e. PROVIDE SAMPLES OF MATERIALS TO BE TESTED IN REQUIRED QUANTITIES.
   f. FURNISH COPIES OF MILL TEST REPORTS WHEN REQUESTED.
   g. PROVIDE STORAGE SPACE FOR STRUCTURAL TESTING/INSPECTION AGENCY'S EXCLUSIVE USE, SUCH AS FOR STORING AND CURING CONCRETE TESTING SAMPLES.
   h. PROVIDE LABOR TO ASSIST THE STRUCTURAL TESTING/INSPECTION AGENCY IN PERFORMING TESTS/INSPECTIONS.

FOUNDATION NOTES

1. ANY FILL REQUIRED TO BACKFILL EXCAVATED AREA OR ACHIEVE FINISHED GRADE IN STRUCTURAL AREAS SHALL BE INORGANIC, NON-PLASTIC GRANULAR SOIL (CLEAN SANDS). THE FILL SHALL BE PLACED IN LEVEL LIFTS NOT TO EXCEED 12 INCHES LOOSE THICKNESS AND COMPACTED TO A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM SPECIFICATION D-1557. IN-PLACE DENSITY TESTS SHALL BE PERFORMED ON EACH LIFT BY AN EXPERIENCED ENGINEERING TECHNICIAN TO VERIFY THAT THE REQUIRED DEGREE OF COMPACTION HAS BEEN ACHIEVED. A SOIL COMPACTION TEST SHALL BE PERFORMED IN EVERY SPREAD FOOTING PAD SUB-GRADE.
2. REMOVE FREE WATER FROM EXCAVATIONS BEFORE PLACING CONCRETE.
3. CONTRACTOR ALONG WITH GEOTECHNICAL FIELD REPRESENTATIVE SHALL ENSURE THAT CLAYEY SOILS OR EXPANSIVE SOILS ARE NOT PRESENT.
4. CONTRACTOR IN CONJUNCTION WITH GEOTECHNICAL FIELD REPRESENTATIVE, SHALL DETERMINE IF ANY SOILS OR UNSTABLE CONDITIONS ARE DISCOVERED DURING EXCAVATION WHICH WOULD PREVENT ATTAINMENT OF THE DESIGN SOIL PRESSURE RECOMMENDED BY THE SOILS REPORT.
5. PROVIDE COMPACTION TESTING OF THE SUBGRADE PRIOR TO LAYING STEEL OR PLACING CONCRETE. COMPACTION SHALL ACHIEVE A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM SPECIFICATION D-1557. ALL SOIL PREPARATION SHALL CONFORM TO THE RECOMMENDATIONS CONTAINED IN THE SOILS REPORT FOR THE PROJECT AND SPECIFICATIONS.
6. FOUNDATIONS HAVE BEEN DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF, IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE TABLE 1806.2.

STRUCTURAL STEEL NOTES

1. STEEL WORK SHALL CONFORM TO THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - ALLOWABLE STRESS DESIGN AND AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.
2. MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:

ROLLED SHAPES, PLATES, AND BARS:	ASTM A36. EXCEPT,
WIDE FLANGE SECTIONS:	ASTM A992, GRADE 50. FY=50KSI.
MACHINE BOLTS:	ASTM A307.
PIPE COLUMNS:	ASTM A53, GRADE B. FY=35KSI.
STRUCTURAL STEEL TUBING:	ASTM A500, GRADE B. FY=46KSI (OR) GRADE C FY=50KSI.
HIGH STRENGTH BOLTS:	ASTM A325 U.N.O.
HEADED ANCHOR STUDS:	ASTM A108 (ULT TENSILE STR = 60,000PSI).

3. CONNECTIONS:
   a. UNLESS OTHERWISE NOTED, BOLTS SHALL BE HIGH-STRENGTH, BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANES. BOLTS SHALL BE PRE-TENSIONED WITH TWIST-OFF TENSION CONTROL (OR) TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD (SNUG-TIGHT PLUS 1/2 TURN). USE LOCK WASHERS.
   b. WELDING ELECTRODES FOR ALL STEEL SHALL BE E70XX. RETURN FILLET WELDS FOR FRAMED CONNECTIONS 1/2" AT EACH END.
   c. SHOP CONNECTIONS SHALL BE WELDED OR BOLTED.
   d. FIELD CONNECTIONS SHALL BE MADE WITH 3/4" BOLTS, EXCEPT AS NOTED OTHERWISE.
   e. ALL CAP PLATES AND BASE PLATES SHALL BE CONTINUOUSLY WELDED TO COLUMNS W/ MAX WELD SIZE UNO.
4. HIGH-STRENGTH FIELD-BOLTED CONNECTIONS SHALL BE INSTALLED, TIGHTENED, TESTED, AND INSPECTED ACCORDING TO "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" BY RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RCSC). CONNECTIONS SHALL NOT BE CLASSIFIED AS SLIP-CRITICAL (SC) UNLESS INDICATED ON PLANS AS SUCH. "SNUG-TIGHT", AS DEFINED IN THE SPECIFICATION, IS SUFFICIENT FOR ALL BOLTED CONNECTIONS UNLESS THE BOLTS IN SUCH A CONNECTION ARE INDICATED AS SLIP-CRITICAL (SC). SLIP-CRITICAL BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION.
5. BRACE AND MAINTAIN ALL STEEL IN ALIGNMENT UNTIL OTHER PARTS OF CONSTRUCTION NECESSARY FOR PERMANENT SUPPORT ARE COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING TEMPORARY SHORING AS REQUIRED FOR THE STABILITY OF THE STEEL FRAME UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN COMPLETED AND BUILDING IS ENCLOSED
6. ALL WELDING IN THE SHOP OR IN THE FIELD SHALL BE PERFORMED BY CERTIFIED WELDERS ONLY. CERTIFICATION DOCUMENT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR HIS REVIEW. ALL WELDS SHALL BE PRE-QUALIFIED PER AWS D1.1, LATEST EDITION. WELDED SPLICES OF ROLLED SHAPES MADE IN THE SHOP ARE ACCEPTABLE PROVIDED RADIOGRAPHED NDT EXAMINATION RESULTS ARE IN ACCORDANCE WITH AWS ACCEPTANCE STANDARDS AND WRITTEN REPORTS VERIFYING SUCH RESULTS ARE SUBMITTED TO THE STRUCTURAL ENGINEER FOR HIS APPROVAL. MINIMUM FILLET WELDS SHALL BE 3/16 UNLESS OTHERWISE SHOWN ON THE DRAWINGS.
7. THE STEEL STRUCTURE IS DESIGNED FOR STABILITY IN ITS COMPLETED CONDITION PER THE DRAWINGS, SPECIFICATIONS AND THESE NOTES. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING, GUYING AND OTHER MEANS OF SUPPORT DURING CONSTRUCTION SUFFICIENT TO WITHSTAND WEATHER CONDITIONS AND MEET ALL APPLICABLE SAFETY REQUIREMENTS DURING CONSTRUCTION.
8. DETAILING OF STRUCTURAL STEEL AND CONNECTIONS SHALL BE SHOWN ON SHOP AND ERECTION DRAWINGS PREPARED BY THE FABRICATOR FOR THE STRUCTURAL ENGINEER'S REVIEW, PRIOR TO FABRICATION.
9. UNLESS NOTED OTHERWISE, ALL STEEL SHALL RECEIVE A SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) WHERE EXPOSED TO VIEW. ALL OTHER AREAS, INCLUDING THOSE WHICH WILL RECEIVE SPRAY-ON-FIRE PROTECTION, OR WHERE HEADED STUDS ARE TO BE WELDED, SHALL NOT BE PAINTED.
10. HOT DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL ITEMS AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO EARTH AND/OR TO WEATHER. GALVANIZING SHALL BE PER ASTM A123 FOR MEMBERS AND ASTM A153 FOR CONNECTION ELEMENTS. (SEE DRAWINGS FOR OTHER STRUCTURAL ITEMS TO BE HOT DIP GALVANIZED).
11. PROVIDE CURB ANGLES 3X3X1/4 TO SUPPORT ROOF DECK AT OPENINGS UNO.

METAL BUILDING

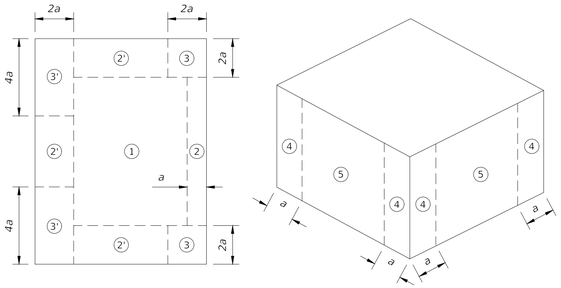
1. STRUCTURAL ENGINEER IS ONLY RESPONSIBLE FOR FOUNDATION DESIGN AND ELEVATED CONCRETE DESIGN. ALL OTHER STRUCTURAL COMPONENTS ARE TO BE DESIGNED BY THE METAL BUILDING ENGINEER.
   SIGNED AND SEALED METAL SHOP DRAWINGS SHALL BE SUBMITTED. SHOP DRAWINGS SHALL INCLUDE ALL COLUMN REACTION LOADS SO THAT THE FOOTINGS MAY BE VERIFIED
2. MAX LATERAL STORY DRIFT SHALL BE LIMITED TO H/240. THE METAL BUILDING DESIGN SHALL ALSO PROVIDE CONSIDERATION FOR OCCUPANT COMFORT DUE TO THE EFFECTS OF RHYTHMIC CROWD MOVEMENTS ON THE STRUCTURE
3. ALL EXPOSED STEEL COMPONENTS AT THE PAVILION INCLUDING FRAMES, GIRTS, METAL SIDING, PURLINS, GIRDERS, SAG RODS, X-BRACING, ROOF DECK AND CONNECTIONS SHALL BE FINISHED WITH HOT-DIPPED GALVANIZATION, DESIGNED AND FABRICATED BY A METAL BUILDING MANUFACTURER.
4. SHOP DRAWINGS INDICATING THE SIZE LAYOUT, FLASHING DETAILS, STEEL GRADES, AND GAUGE THICKNESS OF ALL COMPONENTS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. ALL DIMENSIONS SHALL BE COORDINATED WITH THE FLOOR PLAN AND ELEVATIONS PRIOR TO FABRICATION
5. TESTING LABORATORY SHALL OBSERVE PLACEMENT OF COLUMN ANCHOR BOLTS.
6. SEE NOTE 11 OF CAST-IN-PLACE CONCRETE.

WIND PRESSURE DIAGRAM

1. DESIGN WIND PRESSURES TO BE USED IN THE DESIGN OF ALL COMPONENTS AND CLADDING ELEMENTS.
   REFER TO WIND PRESSURE DIAGRAM FOR ZONE LOCATIONS AND EXTENTS.
2. POSITIVE PRESSURES ACT TOWARD COMPONENT SURFACES AND NEGATIVE PRESSURES ACT AWAY FROM COMPONENT SURFACES.
3. LINEAR INTERPOLATION BETWEEN EFFECTIVE WIND AREAS MAY BE USED TO OBTAIN THE REQUIRED COMPONENT AND CLADDING DESIGN WIND PRESSURE.
4. WIND PRESSURES SHOWN ARE UNFACTORED. MULTIPLY BY A FACTOR OF 0.6 FOR ALLOWABLE STRESS DESIGN (ASD). MULTIPLY BY A FACTOR OF 1.0 FOR LOAD AND RESISTANCE FACTOR DESIGN (LRFD).

WINDOWS AND DOORS

FOR THE SELECTION OF WINDOW AND DOOR PRODUCTS, TABULATED VALUES ARE NORMALLY MULTIPLIED BY 0.6 PRIOR TO COMPARISON WITH THE POSITIVE AND NEGATIVE PRESSURE RATINGS PROVIDED IN EACH FLORIDA PRODUCT APPROVAL. IT IS RECOMMENDED THAT THE MANUFACTURER'S REPRESENTATIVE REVIEW THESE DRAWINGS FOR VERIFICATION.



CAST-IN-PLACE CONCRETE (FOUNDATION AND ELEVATED SLAB)

1. NORMAL WEIGHT STRUCTURAL CONCRETE MINIMUM 28-DAY COMPRESSIVE STRENGTH, F'C: 3,000 PSI
2. PROVIDE NORMAL WEIGHT CONCRETE WITH CURED DENSITY OF 145 +/- 5 PCF, AND AGGREGATE CONFORMING TO ASTM C33, UON.
3. THE USE OF CALCIUM CHLORIDE AND OTHER CHLORIDE CONTAINING AGENTS IS PROHIBITED. THE USE OF RECYCLE CONCRETE IS PROHIBITED.
4. PLACEMENT WITHIN AND CONTACT BETWEEN ALUMINUM ITEMS, INCLUDING ALUMINUM CONDUIT, AND CONCRETE IS PROHIBITED.
5. ALL CAST-IN-PLACE CONCRETE WILL EXPERIENCE DIFFERING VARIATIONS OF CRACKING. ANY ELEMENT EXPOSED TO DIRECT WEATHER AND/OR TEMPERATURE VARIATIONS DURING CONSTRUCTION OR IN THE FINAL CONDITION IS TO BE TREATED AND REGULARLY MAINTAINED TO PREVENT PROPAGATION OF CRACKS AND WATER PENETRATION. THE CONTRACTOR SHALL DEVELOP A REGULAR MAINTENANCE PROGRAM AND SUBMIT IT TO THE OWNER.
6. MAXIMUM W/C RATIO OF 0.50 FOR FOOTINGS AND 0.45 FOR OTHER CONCRETE. CMU GROUT SHALL HAVE W/C RATIO OF 0.60 OR HIGHER.
7. ALL FORMWORK SHALL BE DESIGNED, ERECTED, SUPPORTED, BRACED, AND MAINTAINED ACCORDING TO ACI 347, RECOMMENDED STANDARD PRACTICE FOR CONCRETE FORMWORK.
8. RESPONSIBILITY: THE DESIGN, CONSTRUCTION, AND SAFETY OF ALL FORMWORK SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.
9. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED UNLESS OTHERWISE SHOWN ON THE STRUCTURAL DRAWINGS.
10. THE CONTRACTOR SHALL EMPLOY A TESTING LABORATORY TO PREPARE TEST CYLINDERS REPRESENTING CONCRETE POURED EVERY DAY, ONE SET PER DAY OR ONE SET MINIMUM FOR EACH 50 CUBIC YARDS POURED. THE TESTING LABORATORY TECHNICIAN SHALL BE PRESENT AT THE BEGINNING OF EACH POUR. LABORATORY REPORT SHALL BE FURNISHED TO THE STRUCTURAL ENGINEER SHOWING STRENGTH.
11. ELEVATED SLAB SHALL BE NORMAL WEIGHT CONCRETE ON 22-GAUGE, HOT-DIPPED GALVANIZED 1.0C CONFORM DECK BY VULCRAFT (OR EQUAL), WITH A TOTAL DEPTH OF 4 INCHES. REINFORCE SLAB WITH 4X4-W2.9XW2.9 WELDED WIRE MESH PLACED 1.5 INCHES BELOW THE TOP OF SLAB. SEE CODE AND DESIGN CRITERIA NOTES 3 AND 4 FOR LIVE LOAD AND SUPERIMPOSED DEAD LOAD.

STEEL REINFORCING

1. REINFORCING BARS: ASTM A615, GRADE 60
   a. REINFORCEMENT PLACEMENT (UNO) WELDED PLAIN WIRE MESH: ASTM A185, MINIMUM YIELD STRESS OF 60 KSI
   a. CONCRETE REINFORCEMENT COVER BELOW GRADE: UNFORMED 3" CLEAR FORMED 2" CLEAR CENTER REBAR IN MASONRY CELLS UON.
2. REINFORCEMENT SPLICE
   a. LAP REINFORCEMENT 48 BAR DIAMETER
   b. LAP WELDED WIRE MESH: ONE GRID SPACE PLUS 2".
4. DO NOT USE REBAR STAKES AS CHAIRS. CHAIRS SHALL BE MASONRY OR NON-CORROSIVE SUPPORTS SUCH AS PLASTIC

ABBREVIATIONS

PT	PRESSURE TREATED
GALV.	GALVANIZED
A.B.	ANCHOR BOLT
F.B.C.	FLORIDA BUILDING CODE
U.N.O.	UNLESS NOTED OTHERWISE
EJ	EXPANSION JOINT

POST-INSTALLED ANCHORS

1. ANCHOR PRODUCTS APPROVED FOR USE ON THIS PROJECT ARE LISTED BELOW UNLESS OTHERWISE SPECIFIED IN SECTIONS/DETAILS:
   a. HILTI "HIT-HY 200" ADHESIVE (ICC-ES ESR-3187)
   b. HILTI "HIT-RE 500-SD" ADHESIVE (ICC-ES ESR2322)
   c. EPCON "G5" ADHESIVE (ICC-ES ESR1137)
   d. SIMPSON STRONG-TIE "SET-XP" ADHESIVE (ICC-ES ESR2508)
   e. SIMPSON STRONG-TIE "AT-XP" ADHESIVE (IAPMO-ES ER263)
   f. EPCON "S7" ADHESIVE (ICC-ES ESR2308)
   g. SIMPSON STRONG-TIE "SET" (ICC-ES ESR3342)
   h. SIMPSON STRONG TIE "SET-XP" (ICC PENDING)
2. OVERHEAD AND/OR CONSTANT TENSION EPOXY ANCHOR INSTALLATIONS NOT SHOWN ON THE DRAWINGS SHALL NOT BE PERMITTED UNLESS EACH CONDITION IS REVIEWED AND APPROVED IN WRITING BY THE SER.
3. INSTALL ANCHORS TO MEET THE REQUIREMENTS INDICATED IN THE CONTRACT DOCUMENTS AND THE MANUFACTURER'S RECOMMENDATIONS.
4. LOCATE, BY NON-DESTRUCTIVE MEANS, AND AVOID ALL EXISTING REINFORCEMENT PRIOR TO INSTALLATION OF ANCHORS. IF EXISTING REINFORCING LAYOUT PROHIBITS THE INSTALLATION OF ANCHORS AS INDICATED IN THE DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE DESIGN PROFESSIONALS IMMEDIATELY.
5. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM STRUCTURAL ENGINEER OF RECORD (SER) PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
6. ANCHOR INSTALLER SHALL BE TRAINED BY THE MANUFACTURER ON PROPER INSTALLATION METHODS. CARE SHALL BE EXERCISED TO AVOID CONFLICTS WITH EXISTING REINFORCING WHEN DRILLING HOLES. PILOT HOLES SHALL BE INSTALLED AS REQUIRED. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE OR ON THE STRUCTURAL DRAWINGS EMBEDMENT SHALL BE THE MINIMUM SPECIFIED ON THE STRUCTURAL DRAWINGS.

ANCHOR BOLTS

1. ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 36 WITH ASTM A563 NUTS AND ASTM F436 WASHERS.
2. HOT DIP GALVANIZE ALL ANCHOR BOLTS, WASHERS, NUTS AND SHIMS PER ASTM A123 OR A153.

PAVILION

COMPONENTS AND CLADDING WIND PRESSURES ON ROOF AND WALLS (PSF)

ZONE	ALL ROOF ZONES	1	2	2'	3	3'	4		5	
TRIB AREA	(+)	(-)	(-)	(-)	(-)	(-)	(+)	(-)	(+)	(-)
10	26	-50	-56	-65	-71	-95	44	-47	44	-55
20	25	-50	-55	-64	-66	-87	43	-45	43	-52
50	24	-50	-54	-63	-58	-74	41	-43	41	-48
100	23	-50	-53	-62	-53	-65	39	-42	39	-46
200	23	-50	-53	-62	-53	-65	38	-41	38	-42
500	23	-50	-53	-62	-53	-65	36	-38	36	-38

GAME ROOM

COMPONENTS AND CLADDING WIND PRESSURES ON ROOF AND WALLS (PSF)

ZONE	ALL ROOF ZONES	1	2	2'	3	3'	4		5	
TRIB AREA	(+)	(-)	(-)	(-)	(-)	(-)	(+)	(-)	(+)	(-)
10	13	-34	-39	-47	-53	-74	29	-31	29	-38
20	12	-34	-39	-47	-48	-67	28	-30	28	-36
50	11	-34	-38	-46	-42	-56	26	-28	26	-33
100	10	-34	-37	-45	-37	-47	25	-27	25	-30
200	10	-34	-37	-45	-37	-47	23	-26	23	-28
500	10	-34	-37	-45	-37	-47	22	-24	22	-24

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY DAVID MORGAN CRAPPS, ON THE DATE ADJACENT TO THE SEAL.
PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

REVISIONS	
DATE	DESCRIPTION

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PH. 386-752-4675
LIC NO. LB8356

2551 BLAIRSTONE PINES DR.
TALLAHASSEE, FL 32301
WWW.NFPS.NET
CA# 29011

JOB NUMBER:
L210802SPA
EOR:
DAVID MORGAN CRAPPS
P.E. NO.:
60989

STRUCTURAL NOTES
THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

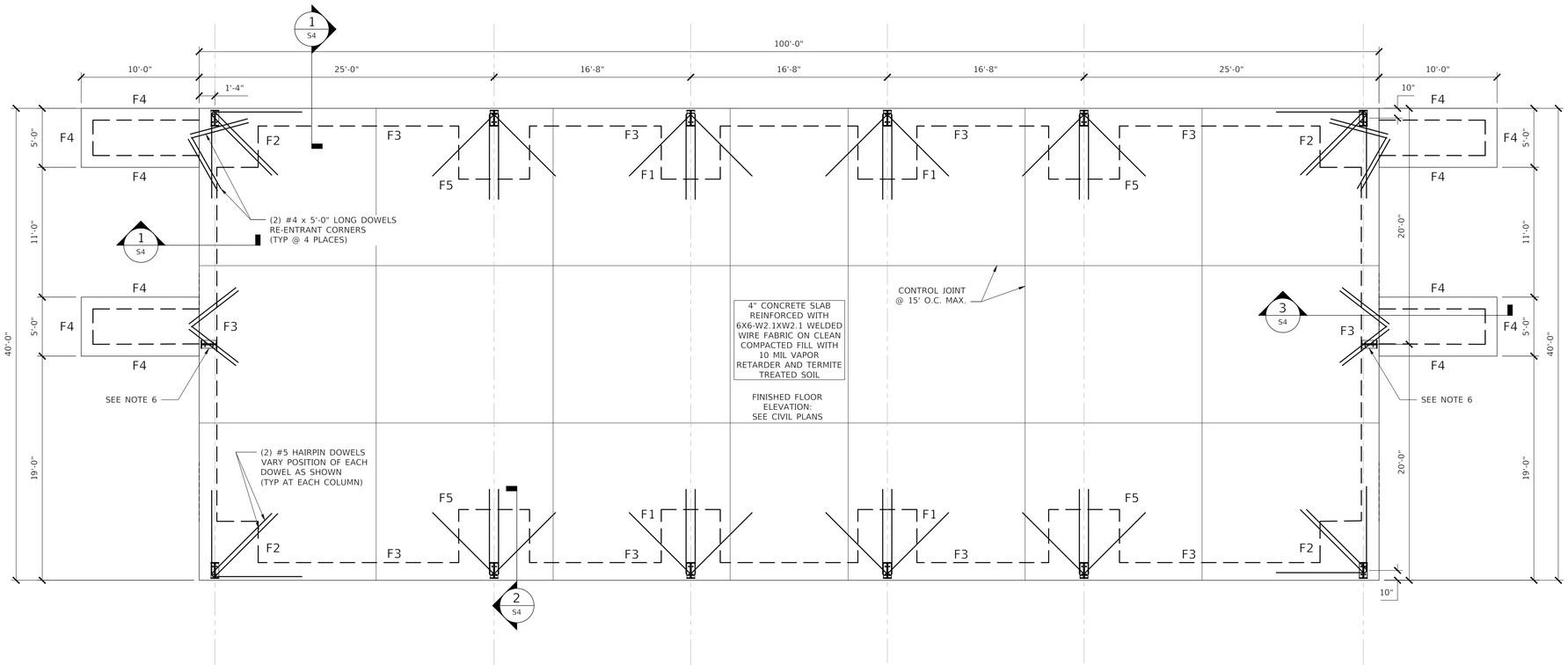
SHEET NO.
S1

PLAN NOTES:

1. LOCATE EXISTING UTILITIES PRIOR TO EXCAVATION FOR NEW FOOTINGS.
2. MAINTAIN POSITIVE SLOPE FOR FINISHED GRADE AWAY FROM NEW FOUNDATIONS PER CODE.
3. FIELD VERIFY DIMENSIONS AS REQUIRED.
4. COMPACT SUB-GRADE PER SHEET S1 "FOUNDATION NOTES" AND PROVIDE TERMITE TREATMENT.
5. CONTRACTOR TO VERIFY ANCHOR BOLT LAYOUT WITH METAL BUILDING SHOP DRAWINGS PRIOR TO DETAILING HAIRPINS. PROVIDE 2" MIN. SIDE COVER AT SLAB EDGE AND 1" MIN. CLEARANCE FROM ANCHOR BOLTS.
6. PROVIDE NOT LESS THAN 4" EDGE DISTANCE FROM CENTERLINE OF ANCHOR BOLTS TO EDGE OF FOOTING, TYPICAL, AT METAL BUILDING COLUMNS.

FOOTING SCHEDULE

MARK	TYPE	WIDTH	LENGTH	THICK	REINFORCEMENT
F1	COLUMN	5'-0"	6'-0"	2'-0"	#5 BARS @ 6" O.C. EA. WAY (TOP & BOTTOM)
F2	COLUMN	5'-0"	5'-0"	2'-0"	#5 BARS @ 6" O.C. EA. WAY (TOP & BOTTOM)
F3	STRIP	1'-6"	CONT.	2'-0"	3 - #5 BARS CONTINUOUS (TOP & BOTTOM)
F4	STRIP	1'-0"	CONT.	1'-6"	1 - #5 BAR CONTINUOUS (TOP & BOTTOM)
F5	COLUMN	6'-0"	6'-0"	2'-0"	#5 BARS @ 6" O.C. WA. WAY (TOP & BOTTOM)



1 PAVILION FOUNDATION
SCALE: 3/16" = 1'-0"

REVISIONS	
DATE	DESCRIPTION

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PAVILION FOUNDATION
THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

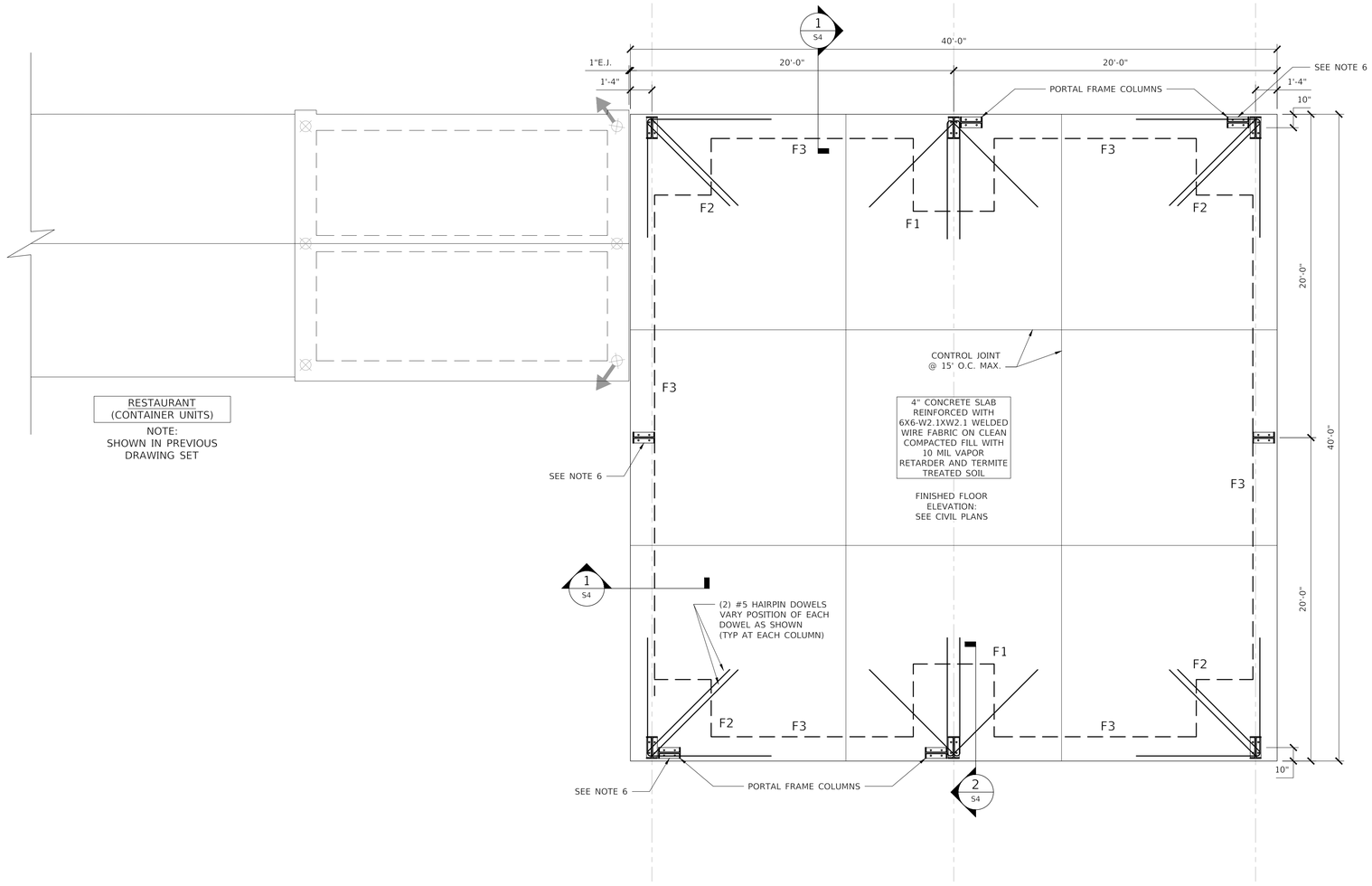
SHEET NO. S2

PLAN NOTES:

1. LOCATE EXISTING UTILITIES PRIOR TO EXCAVATION FOR NEW FOOTINGS.
2. MAINTAIN POSITIVE SLOPE FOR FINISHED GRADE AWAY FROM NEW FOUNDATIONS PER CODE.
3. FIELD VERIFY DIMENSIONS AS REQUIRED.
4. COMPACT SUB-GRADE PER SHEET S1 "FOUNDATION NOTES" AND PROVIDE TERMITE TREATMENT.
5. CONTRACTOR TO VERIFY ANCHOR BOLT LAYOUT WITH METAL BUILDING SHOP DRAWINGS PRIOR TO DETAILING HAIRPINS. PROVIDE 2" MIN. SIDE COVER AT SLAB EDGE AND 1" MIN. CLEARANCE FROM ANCHOR BOLTS.
6. PROVIDE NOT LESS THAN 4" EDGE DISTANCE FROM CENTERLINE OF ANCHOR BOLTS TO EDGE OF FOOTING, TYPICAL, AT METAL BUILDING COLUMNS.

FOOTING SCHEDULE

MARK	TYPE	WIDTH	LENGTH	THICK	REINFORCEMENT
F1	COLUMN	5'-0"	6'-0"	2'-0"	#5 BARS @ 6" O.C. EA. WAY (TOP & BOTTOM)
F2	COLUMN	5'-0"	5'-0"	2'-0"	#5 BARS @ 6" O.C. EA. WAY (TOP & BOTTOM)
F3	STRIP	1'-6"	CONT.	2'-0"	3 - #5 BARS CONTINUOUS (TOP & BOTTOM)



1 GAME ROOM FOUNDATION
SCALE: 1/4" = 1'-0"

REVISIONS	
DATE	DESCRIPTION

NORTH FLORIDA PROFESSIONAL SERVICES, INC.
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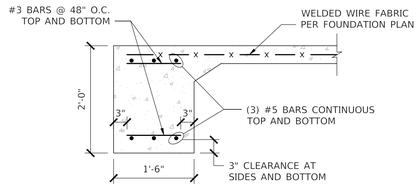
GAME ROOM FOUNDATION
THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO. S3

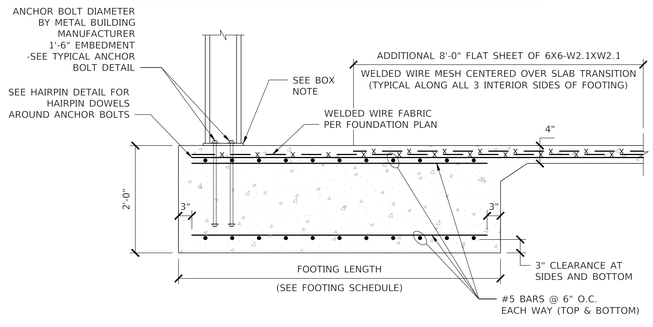


1 FOOTING #3 DETAIL

SCALE: 3/4" = 1'-0"

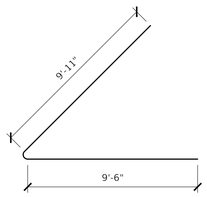
NOTE: WALL STRUCTURE NOT SHOWN

NOTE:
CONTRACTOR TO CONFIRM WITH METAL BUILDING MANUFACTURER IF NONSHRINK GROUT IS REQUIRED UNDER THE COLUMN BASE PLATE, WITH DUE CONSIDERATION FOR THE MAGNITUDE OF THE COLUMN REACTIONS AND THE ANTICIPATED CONSTRUCTION TOLERANCES FOR CONCRETE FINISHING OPERATIONS. COLUMN LEVELING PROCEDURES DURING ERECTION SHALL NOT INDUCE RESIDUAL STRESSES IN COLUMN BASE AND ANCHOR BOLTS THAT HAVE NOT BEEN ACCOUNTED FOR IN THE DESIGN.



2 FOOTING DETAIL

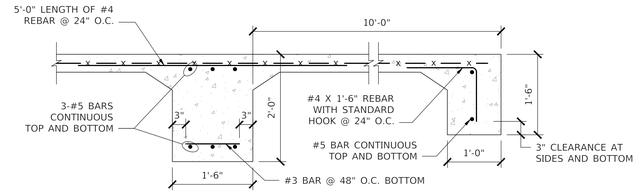
SCALE: 3/4" = 1'-0"



HAIRPIN DETAIL

SCALE: 1/4" = 1'-0"

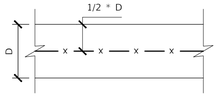
NOTE:
LOCATE STAIR COLUMNS (NOT SHOWN) OVER FOOTINGS / TURNDOWN EDGES. PROVIDE A MINIMUM CONCRETE EDGE DISTANCE OF 3-1/2" FROM CENTERLINE OF ANCHOR BOLTS AT STAIR COLUMS.



3 FOUNDATION DETAIL

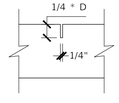
SCALE: 3/4" = 1'-0"

NOTE: WALL STRUCTURE NOT SHOWN



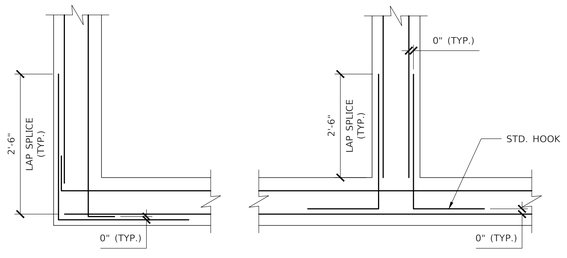
SLAB DETAIL

SCALE: 1 1/2" = 1'-0"



CONTROL JOINT

SCALE: 1 1/2" = 1'-0"

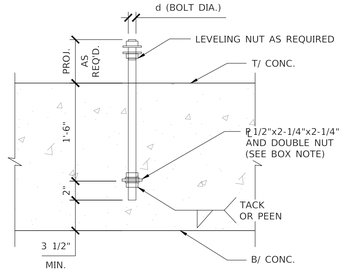


CORNER INTERSECTION

NOTE:
PROVIDE MATCHING CORNER BARS FOR ALL HORIZONTAL REINFORCING AT INTERSECTIONS & CORNERS OF ALL: FOOTINGS, THICKENED SLABS ON GRADE, & HORIZ. WALL REINFORCEMENT UNLESS NOTED OTHERWISE.

TYPICAL REINFORCEMENT DETAIL

SCALE: N.T.S.



NOTE:
ASTM F1554 HEX HEADED BOLT WITH ℄ 1/2"x2-1/4"x2-1/4" AND NUT MAY BE SUBSTITUTED FOR THREADED ROD AT CONTRACTOR'S OPTION. USE GRADE 36 UNLESS SPECIFIED OTHERWISE BY THE METAL BUILDING MANUFACTURER.

TYPICAL ANCHOR BOLT DETAIL

SCALE: N.T.S.

REVISIONS	
DATE	DESCRIPTION

NORTH FLORIDA PROFESSIONAL SERVICES, INC.
P.O. BOX 3823
LAKE CITY, FL 32056
PH. 386-752-4675
LIC NO. LB8356
2551 BLAIRSTONE PINES DR.
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WWW.NFPS.NET
CA# 29011

JOB NUMBER: L210802SPA
EOR: DAVID MORGAN CRAPPS
P.E. NO.: 60989

STRUCTURAL SECTIONS AND DETAILS

THE WOODS PAVILION AND GAME ROOM
COLUMBIA COUNTY, FLORIDA

SHEET NO. S4