MATERIALS

. fc 8" precast lintel = 3500 psi . fc prestressed lintel = 6000 psi . Grout per ASTM C476 fc = 3000 osi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump ASTM C90 w/minimum net area

. Concrete Masonry Units (CMU) per compressive strength = 1900 psi . Rebar per ASTM A615 grade 60 . Prestressing strand per ASTM A416 grade 270 low relaxation . Mortar per ASTM C270 type M or S

GENERAL NOTES

 Provide full mortar bed and head joints. Shore filled lintels as required. 3. Installation of lintel must comply with the architectural and/or structural documents. 4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting 5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection. 6. Bottom field added rebar to be located at the bottom

of the lintel cavity. 7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage. 3. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

9. Safe load rating based on rational design analysis per ACI 318 and ACI 530 10. Product Approvals: Miami-Dade County, Florida No.

03-0606.05 11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating. 12. Lintels loaded simultaneously with vertical (gravity or

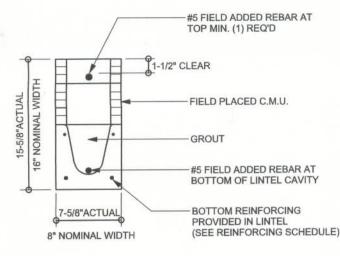
uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation: Applied vertical load Safe vertical load Safe horizontal load

Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

TYPE DESIGNATION

F = FILLED WITH GROUT / U = UNFILLED / S = SOLID **OUANTITY OF #5 FIELD ADDED REBAR AT** BOTTOM OF LINTEL CAVITY 8F16-1B/1T-4'-0"

- QUANTITY OF #5 FIELD NOMINAL HEIGHT ADDED REBAR AT TOP #5 FIELD ADDED REBAR AT TOP MIN. (1) REQ'D



SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch

nominal bearing.

Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches. 2. N.R. = Not Rated Safe loads are superimposed allowable loads. 4. Safe loads based on grade 40 or grade 60 field rebar. 5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only 6. The designer may evaluate concentrated loads from the safe load tables by

For composite lintel heights not shown, use safe load from next lower height shown. 8. For lintels lengths not shown, use safe load from next longest length shown 9. All safe loads in units of pounds per linear

calculating the maximum resisting moment

and shear at d-away from face of support.

10. All safe loads based on simply supported 11. The number in the the parenthesis

indicates the percent reduction for grade 40 field added rebar. Example 7'-6" lintel type 8F32-1B safe gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15% reduction $6472 \Rightarrow (.85) = 5501$ plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

2'-10" (34") PRECAST

3'-6" (42") PRECAST

4'-0" (48") PRECAST

4'-6" (54") PRECAST

5'-4" (64") PRECAST

5'-10" (70") PRECAST

6'-6" (78") PRECAST

7'-6" (90") PRECAST

9'-4" (112") PRECAST

10'-6" (126") PRECAST

11'-4" (136") PRECAST

12'-0" (144") PRECAST

13'-4" (160") PRECAST

14'-0" (168") PRECAST

14'-8" (176") PRESTRESSED

15'-4" (184") PRESTRESSED

17'-4" (208") PRESTRESSED

19'-4" (232") PRESTRESSED

21'-4" (256") PRESTRESSED

22'-0" (264") PRESTRESSED

24'-0" (288") PRESTRESSED

7	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0B		
	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1B		
Ì	3069	4605	6113	7547	8974	10394	11809		
	3069	4605	6113	7547	8974	10394	11809		
	3069	3719	5163	6607	8054	9502	10951		
	3069	4605	6113	7547	8974	10394	11809		
	2561	2751	3820	4890	5961	7034	8107		
	2693	4605	6113	7547	8974	10394	11809		
	1969	2110	2931	3753	4576	5400	6224		
l	2189	4375	6113	7547 (7)	8672	10294	11809		
	1349	1438	1999	2560	3123	3686	8F32-1B 11809 11809 10951 11809 8107 11809 6224 11809 4249 10127(19) 3470 8328 (20) 5061 8825 (14) 3685 6472 (15) 3302 6390(47) 2536 4754 (45) 4006 4006 3552 3552 2883 2883 2630 NR 2712 NR		
	1663	3090	5365	7547(36)	7342 (19)	8733 (19)	8F32-1B 11809 11809 10951 11809 8107 11809 6224 11809 4249 10127(19) 3470 8328 (20) 5061 8825 (14) 3685 6472 (15) 3302 6390(47) 2536 4754 (45) 4006 4006 3552 3852 2883 2883 2630 2630 NR 2712		
	1105	1173	1631	2090	2549	3009			
	1451	2622	4360	7168 (45)	6036(19)	7181 (19)	8328 (20		
	1238	2177	3480	3031	3707	4383			
	1238	2177	3480	5381	8360	10394(37)	8825 (14)		
	1011	1729	2632	2205	2698	3191			
	1011	1729	2661	3898	5681	8467(44)	6472 (15)		
	699	1160	1625	2564	3486	2818			
	752	1245	1843	2564	3486	4705(37)	6390(47)		
	535	890	1247	2093	2777	2163			
	643	1052	1533	2093	2781	3643 (38)	4754 (45)		
	582	945	1366	1846	2423	3127			
ŀ	582	945	1366	1846	2423	3127	4006		
	540	873	1254	1684	2193	2805	3552		
	540	873	1254	1684	2193	2805	3552		
	471	755	1075	1428	1838	2316	3552		
	471	755	1075	1428	1838	2316	3552 3552 2883		
	424	706	1002	1326	1697	2127	2630		
l	442	706	1002	1326	1697	2127	2630		
	NR	NR	NR	NR	NR	NR	NR		
	458	783	1370	1902	2245	2517	2712		
İ	NR	NR	NR	NR	NR	NR	NR		
İ	412	710	1250	1733	2058	2320	2513		
	NR	NR	NR	NR	NR	NR			
Ì	300	536	950	1326	1609	1849	2047		
	NR	NR	NR	NR	NR	NR			
	235	418	750	1037	1282	1515	1716		
	NR	NR	NR	NR	NR	NR	NR		
	180	340	598	845	1114	1359	1468		
	NR	NR	NR	NR	NR	NR	NR		
١	165	315	550	784	1047	1285	1399		
١	NR	NR	NR	NR	NR	NR	NR		
	129	250	450	654	884	1092	1222		

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

<	CAST	-Carra	SAFE	ELOAD	- POUN	NDS PE	R LINEA	R FOO	Т
		TYPE	8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-1T
LENG	TH		8F8-2T	8F12-2T	8F16-2T	8F20-2T	8F24-2T	8F28-2T	8F32-2T
01.401	(C 411)	PPECACT	1972	3173	4460	5747	7034	8321	9608
2'-10"	(34")	PRECAST	1972	3173	4460	5747	7034	8321	9608
01.01	/ / OII)	DDECAGE	1569	2524	3547	4569	5591	6613	7636
3'-6"	(42")	PRECAST	1569	2524	3547	4569	5591	6613	7636
41.01	(40II)	DDECAST	1363	2192	3079	3966	4853	5740	6627
4'-0"	(48")	PRECAST	1363	2192	3079	3966	4853	5740	6627
4'-6"	(54")	PRECAST	1207	1940	2724	3508	4292	5077	5861
4-0	(34)	FRECASI	1207	1940	2724	3508	4292	5077	5861
F1 48	(0.411)	PPEGAGE	1016	1632	2290	2949	3607	4265	4924
5'-4"	(64")	PRECAST	1016	1632	2290	2949	3607	4265	4924
EL 401	(7011)	DDECAGE	909	1492	2093	2694	3295	3897	4498
5'-10"	(70")	PRECAST	929	1492	2093	2694	3295	3897	4498
C1 C11	/70"\	DDECAST	835 (12)	1340	1880	2419	2959	3498	4038
6'-6"	(78")	PRECAST	835	1340	1880	2419	2959	3498	4038
71.01	(0.0II)	DDECAGE	727 (23)	1021	1634 (12)	2102 (11)	2571(10)	3039 (10)	3508 (9
7'-6"	(90")	PRECAST	727	1166	1634	2102	2571	3039	3508
01.411	(44011)	DDECAST	591	680	1133 (15)	1471 (15)	1811(15)	2152 (16)	2494 (15
9'-4"	(112)	PRECAST	591	851	1326	1705	2084	2463	2842
401.01	(4000)	DDECAST	530	552	914 (15)	1185 (15)	1458 (15)	1732 (15)	2007 (15
10'-6"	(126")	PRECAST	530	686	1183	1526	1865	2204	2544
4.4148	(40011)	DDECAGE	474	485	798 (15)	1034 (15)	1272 (15)	1510 (15)	1749 (15
11'-4"	(136")	PRECAST	494	599	1028	1422	1738	2053	2369
			470 (9)	441	723 (14)	936 (14)	1151(15)	1366 (15)	1582 (15
12'-0"	(144")	PRECAST	470	543	928	1349	1649	1948	2247
401.48	(4000)	DDEGAGE	418 (15)	373	606 (14)	783 (14)	962 (14)	1141 (14)	1321 (14
13'-4"	(160")	PRECAST	428	455	770	1145	1444	1718	1993
4.41.00	/400W	DDEGAGE	384 (15)	346	559 (14)	723 (14)	887 (14)	1052 (14)	1218 (14
14'-0"	(168")	PRECAST	410	420	709	1050	1434 (8)	1694 (8)	1954 (7
	/ / man	DDECTDECCE	239	323	519 (13)	671 (13)	823 (13)	976 (14)	1129 (14
14'-8"	(176")	PRESTRESSE	246	390	655	968	1324 (8)	1625 (11)	1874 (11
		DDECTDECCE	224	302	485 (13)	626 (13)	767 (13)	909 (13)	1052 (13
15'-4"	(184")	PRESTRESSE	230	364	609	897	1224 (8)	1562 (14)	1801 (14
4-71 411	/000II)	DDECTDECCE	187	255	404 (12)	520 (12)	637 (12)	754 (12)	872 (12
17'-4"	(208")	PRESTRESSE	192	303	500	732	993 (8)	1268 (14)	1470 (14
401.411	(020")	DDECTDECCE	162	222	347 (11)	446 (11)	546 (12)	646 (12)	746 (12
19-4"	(232)	PRESTRESSED	166	261	424	616	831 (8)	1057 (14)	1225 (14
241.41	(DEC!!)	DDESTDESSE	142	198	306 (11)	393 (11)	480 (11)	567 (11)	654 (11
21-4"	(200")	PRESTRESSE	142	230	369	531	713 (7)	100000000000000000000000000000000000000	1046 (13
201.01	/DC/III	DDECTDECCE	137	192	295 (10)	378 (11)	461 (10)		The second second second
22-0"	(204")	PRESTRESSE	137	221	354	508	681 (7)	861 (13)	997 (13
n 41	(0.0.0	DDEGTOCOC	124	175	267 (10)	341 (10)	416 (10)	491 (10)	
24'-0"	(288")	PRESTRESSE	124	200	316	450	600 (7)		Manager 1

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

(AST-(्रैक्रकाख		SAFE	LOAD	- POUN	DS PEF	R LINEAL	R F001	Г
		TYPE		8RF6-0B	8RF10-0B	8RF14-0B	8RF18-0B	8RF22-0B	8RF26-0B	8RF30-0B
LENG	TH		8RU6	8RF6-1B	8RF10-1B	8RF14-1B	8RF18-1B	8RF22-1B	8RF26-1B	8RF30-1E
60171020	(United States			1749	3355	3280	4349	5421	6493	7567
4'-4"	(52")	PRECAST	1635	1891	3699	5206	6639	8060	9479	10893
arvean			4404	1596	3063	2992	3968	4946	5924	6904
4'-6"	(54")	PRECAST	1494	1756	3699	5206	6639	8060	9479	10893
E1 08	(0.0H)	DDECAGE		920	1770	1716	2277	2839	3402	3966
5'-8"	(68")	PRECAST	866	1167	2481	4567	6389	8060 (34)	7917 (18)	9311 (11
EL 401	/70"\	DDECAST	0.10	859	1653	1600	2124	2649	3174	3700
5'-10"	(10)	PRECASI	810	1113	2342	4242	6639 (10)	8060 (39)	7402 (19)	8706 (1
	(0.0T)	DDECLOT	707	901	1825	3120	5048	7747	9448	7360
6'-8"	(80")	PRECAST	797	901	1825	3120	5048	7915	9479	10893 (33
71.01	(00!!)	DDECAGE	000	755	1490	2459	3776	5743	7239	5623
7'-6"	(90")	PRECASI	669	755	1490	2459	3776	5743	8998 (19)	10893 (4)
01.01	 (52") PRECAST (54") PRECAST (68") PRECAST (70") PRECAST (80") PRECAST (90") PRECAST (116") PRECAST 	144	466	999	1568	2253	3129	4091	3146	
9'-8"	(110)	FRECASI	411	FOC	000	4500	2252	2120	4450	E901

526 999 1568 2253 3129 4150 5891 (4) SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

<	Cast		SA	FE LOA	D - POL	JNDS P	ER LINE	EAR FO	ОТ
	_	TYPE	8RF6-1T	8RF10-1T	8RF14-1T	8RF18-1T	8RF22-1T	8RF26-1T	8RF30-1T
LENG	TH		8RF6-2T	8RF10-2T	8RF14-2T	8RF18-2T	8RF22-2T	8RF26-2T	8RF30-2T
	/===W		905	1748	2635	3522	4409	5296	6183
4'-4"	(52")	PRECAST	905	1748	2635	3522	4409	5296	6183
			867	1675	2525	3374	4224	5074	5924
4'-6" (54")	PRECAST	867	1675	2525	3374	4224	5074	5924	
CI OII	/CO!!\	DDECACT	675	1301	1960	2618	3277	3935	4594
5'-8"	(68")	PRECAST	675	1301	1960	2618	3277	3935	4594
E! 40!	(70")	PRECAST	655	1262	1900	2538	3176	3815	4453
5'-10"	(70")	PRECASI	655	1262	1900	2538	3176	3815	4453
01 01	(0.011)	DDECAGE	570	1012	1651	2204	2758	3312	3865
6'-8"	(80")	PRECAST	570	1097	1651	2204	2758	3312	3865
71.01	(00II)	DDECACT	506	797	1462 (8)	1952 (7)	2442 (6)	2931 (6)	3257
7'-6"	(90")	PRECAST	506	967	1462	8RF18-2T 8RF22-2T 8RF26-2T 8RF3 3522 4409 5296 618 3522 4409 5296 618 3374 4224 5074 592 2618 3277 3935 459 2618 3277 3935 459 2538 3176 3815 445 2538 3176 3815 445 2204 2758 3312 386 2204 2758 3312 386 0 1952 (7) 2442 (8) 2931 (8) 325	3421		
0' 9"	/4.4 CW)	DDECAST	395	491	931 (12)	1301 (15)	1640 (15)	1980 (15)	2322 (16

395 589 1135 1514 1893 2272 2652

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. TUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

4" CONCRETE SLAB -

7/16" OSB ROOF SHEATHING UNBLOCKED

@ 6" OC ON EDGES & INTERMEDIATE SUPPORTS

ENGINEERED TRUSSES

(4) .131"X3" TOE NAILS

2X8 PT SYP#2 PLATE

1/2" X 10" ANCHOR BOLT

@ 48" OC & 8" FROM CORNERS

- 8" BOND BEAM POURED SOLID

-#5 VERT. IN FULLY GROUTED

CELLS IN CORNERS & EACH SIDE OF OPENINGS & @ 72" O UNO SEE STRUCTUAL PLANS

INSPECTOR TO VERIFY THAT ALL REBAR IS GRADE 60

ALL LAPS TO BE 30" MIN.

#5 STEEL DOWEL

GRADE

-20" X 10" POURED

EXTERIOR WALL

ONE STORY CMU

CONCRETE STRIP FOOTING

- (2) #5 REBAR CONTINUOUS

STD HOOK IN FOOTING

#5 REBAR CONTINUOUS

2" FROM TOP

ATTACH PER TRUSS UPLIFT

NAILED TO ROOF FRAMING W

.113" X 2 3/8" RING SHANK NAILS

DOOR & WINDOW BUCK ATTACHMENT

Fb (psi) | E (10⁶ psi)

1.6

1.6

1.6

1.8

1.7

2.0

2.0

1200

1050

975

2400

1700

2900

2900

TAPCON IN FACE OF CMU 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT 3" MIN. SPACING

3/16" TAPCONS @ 14" O.C.

1/4" TAPCONS @ 14" O.C.

WINDOWS & DOORS UP TO 6'X8'

GRADE & SPECIES TABLE

SYP #2

SYP #2

SYP #2

24F-V3 SP

TIMBERSTRAND

MICROLAM

PARALAM

2x12

GLB

1/4" TAPCONS @ 21" O.C. WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 9" O.C.

SLIDERS UP TO 8'HX20'W

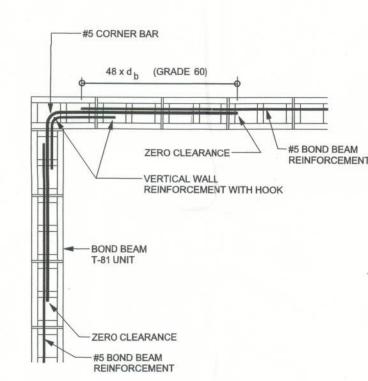
3/16" TAPCONS @ 7" O.C. 1/4" TAPCONS @ 10" O.C.

GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS & 9" O.C. (2) 1/4" TAPCONS & 14" O.C.

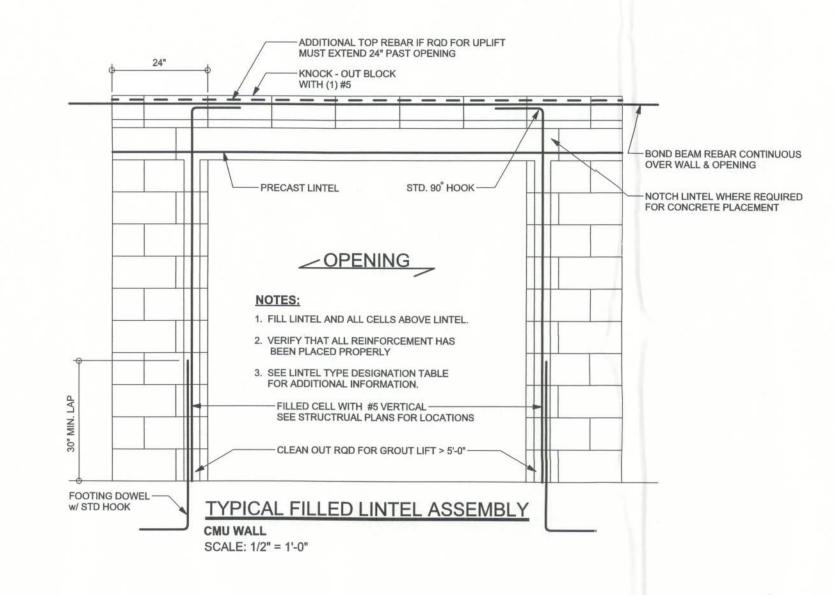
GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 4" O.C. (2) 1/4" TAPCONS & 7" O.C.



TYPICAL BOND BEAM CORNER

SCALE: 1/2" = 1'-0"



WOOD ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
0000	< 2200	ABU44	12-16d		1/2" AB
< 2200					N. C.
< 2200	< 2300	ABU66	12-16d		1/2" AB

MASONRY TRUSS ANCHOR TABLE **OBTAIN UPLIFT REQUIREMENTS FROM TRUSS**

MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIFICA	R AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARI LY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SIT BACKFILL HE	CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND SHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MAT REQUIREMEN	RIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 S FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE	NTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU LAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL DENGINEER IMMEDIATELY.
DESIGN, PLAC	RUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS EMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, USS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL ATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS BOOK SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

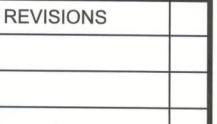
WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

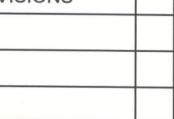
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;

DESIGN DATA

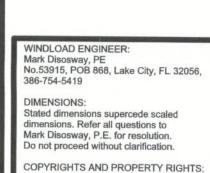
BUILDING IS NOT IN THE HIGH VELOCITY HUR	RICANE ZON	E											
BUILDING IS NOT IN THE WIND-BORNE DEBRIS	S REGION												
1.) BASIC WIND SPEED = 110 MPH	S-10-10-10-10-10-10-10-10-10-10-10-10-10-												
.) WIND EXPOSURE = C .) WIND IMPORTANCE FACTOR = 1.0 .) BUILDING CATEGORY = II													
										5.) ROOF ANGLE = 10-45 DEGREES			
										6.) MEAN ROOF HEIGHT = <30 FT			
7.) INTERNAL PRESSURE COEFFICIENT = N/A) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)												
8.) COMPONENTS AND CLADDING DESIGN W	/IND PRESSU	RES (T	ABLE F	R301.2(2))								
					1.1.0								
***	Zone	_		ind Are									
	1		-30.5		100 -25.3								
2 2	2		-30.5	25.3 25.3	-25.3								
	2 O'hg		-56.8	20,0	-56.8								
2 2 2 2 5	3	27.8	-35.7	25.3	-30.5								
4 3 4	3 O'hg		-95.6		-59.3								
555	4		-33.0	25.9	-28.5								
The state of the s	5	30.5	-40.7	25.9	-31.6								
	Doors	& Wind	lows	30.5	-40.7								
2	1.53-53-55	Worst Case (Zone 5, 10 ft2) 8x7 Garage Door											
5 3 1					20.0								
2 4 5	16x7 Ga		27.3 25.9	-32.0									
4 1				20.0	20,4								
55 22													

DESIGN LOADS													
FLOOR 40 PSF (ALL OTHER DWELLING ROOM	MS)												
30 PSF (SLEEPING ROOMS)													
	30 PSF (ATTICS WITH STORAGE)												
10 PSF (ATTICS WITHOUT STORAGE,	, <3:12)												
ROOF 20 PSF (FLAT OR <4:12)													
16 PSF (4:12 TO <12:12)													
12 PSF (12:12 AND GREATER)													
STAIRS 40 PSF (ONE & TWO FAMILY DWELLING	GS)												
SOIL BEARING CAPACITY 1000PSF													
NOT IN FLOOD ZONE (BUILDER TO VERIFY)													









Mark Disosway, P.E. hereby expressly

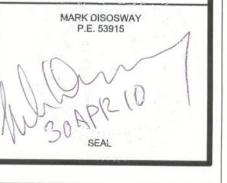
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or copied in any form or manner without first

the express written permission and consent

of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Aaron Simque Homes, Inc.

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FINALS DATE: 30Apr10

JOB NUMBER: 1004068 DRAWING NUMBER

> **S-1** OF 3 SHEETS